Layout Name

Design Method

Description

Created

Sales Rep

Designer

Shipping

1.4A 7X4

Job Path

905-642-4400

Ground Floor

Design Method

Deflection Joist

LL Span L/

TL Span L/

LL Cant 2L/

TL Cant 2L/

LL Span L/

TL Span L/

LL Cant 2L/

TL Cant 2L/

Decking

Thickness

Fastener

Vibration

Deck

Deflection Girder

Floor

Loads

Live

Dead

Builder's Project

14 Anderson Blvd

Stouffville, Ontario Canada

Kott Lumber Company

D:\Users\rochavillo\WORK FROM HOME\GREEN YORK HOMES

\GRANELLI HOME CORP\MODELS

LSD

2012

40

15

480

360

480

360

360

240

480

360

OSB

3/4"

Nailed & Glued

\LIANA 3\LIANA 3 ELEV 1\FLOOR

Building Code NBCC 2010 / OBC

Project

RCO

Builder

May 29, 2018

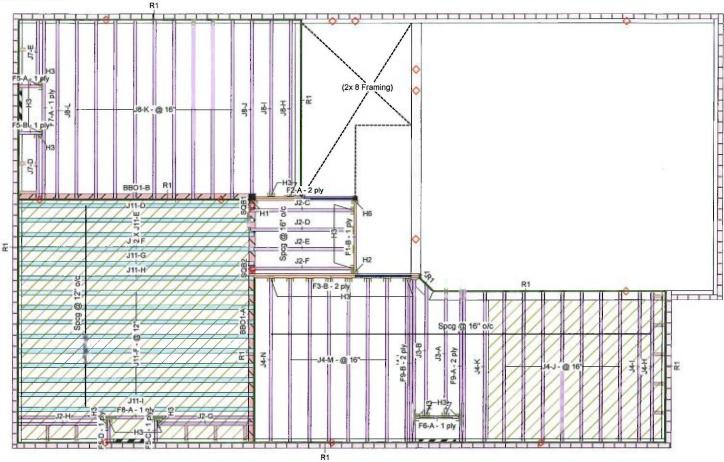
LIANA 3 (ELEV.1 & 2)

GRANELLI HOME CORP.

GREEN YORK HOMES

BRAMPTON, ONT.

Ground Floor





Engineered floor joists shall be installed in accordance with the supplier's layout and specifications forming part of the permit drawings.

Building Code O. Reg. 332/

All work shall conform to

THIS CERTIFICATION IS TO CONFIRM THAT:

- 1. THE LOADS USED IN THE CALCULATION OF THE ATTACHED APPROVED COMPONENTS CONFORM TO THE FLOOR ASSEMBLY SHOWN ON THIS LAYOUT.
- 2. THE FLOOR JOISTS COMPLY WITH THE NASCOR SPAN TABLE FOR THE LOADS AND SPACING SHOWN ON THIS LAYOUT.

THE FLOOR SYSTEM MUST BE ASSEMBLED IN ACCORDANCE TO THE NASCOR SPECIFIER GUIDE. MULTI-PLY MEMBERS MUST BE ATTACHED TOGETHER AS PER THE INCLUDED MULTIPLE MEMBER CONNECTION DETAIL.

ALL OTHER COMPONENTS AND STRUCTURAL ELEMENTS SUPPORTING THE FLOOR SYSTEM SUCH AS BEAMS, WALLS, **COLUMNS AND FOUNDATION WALLS AND FOOTINGS** INCLUDING ANCHORAGE OF COMPONENTS AND BRACING FOR LATERAL STABILITY ARE THE RESPONSIBILITY OF OTHERS.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS. PASS THRU FRAMING SQUASH

BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS





Load from Above Wall Opening Norbord Rimboard Plus 1.125 X 9.5 NJ60U 9.5 N.IH 9.5 Forex 2.0E-3000Fb LVL 1.75 X 9.5

- 1. OBC 2012 O.Reg 332/12 as amended
- 2. Nascor CCMC 13535-R
- 3. LVL CCMC -14056-R
- 4. CAN/CSA-O86-09
- 5. CCMC -12787-R APA PR-L310(C)

	L (I lus	311/					
Label	Descr	iption	1				
F3	Forex						
	2.0E-3	2.0E-3000Fb LVL					
F2	Forex		Г				
	2.0E-3	000Fb LVL					
F1	Forex						
		000Fb LVL					
I Joist (Flush)						
Label	Descr	iption	١				
J11	NJ60U						
F8	NJH	1					
F7	NJH	1					
F9	NJH	1					
F6	NJH	1					
F5	NJH						
J8	NJH						
J4	NJH						
J3	NJH						
J2	NJH						
J7	NJH						
Rim Bo							
	Descri	intion	٧				
R1		d Rimboard	٧				
K.I		125 X 9.5					
Blockin		.20 A 0.0					
Label	_	intion	1				
BLK1	Descri NJH	μιστ	٧				
Hanger	INOU		_				
riangei							
Label	Pcs	Description	n				
H1	1	Unknown					
		Hanger					
H2	1	HUS1.81/10)				
НЗ	26	LT259					
H6	1	HUCQ1.81/ SDS	9-				
NOTES:							
 Double another Install Install rimboa Refer t Squas all first two lev Load to It shall 	e joist or er memb 2x4 bloc single-p ird/rimjo to Nasco h blocks level jo vels floo ransfer l be the	ify dimension of the control of the	ler. o/d low juic dec upp ins				
		Member Co					

Ground Floor LVL/LSL (Flush)

nsions on the architectural drawings. e filler/backer ply when supporting

a face-mounted hanger. 24" o/c under parallel non-load bearing walls.

Width Depth

Width Depth

9.5

9.5

9.5

9.5

9.5

9.5

9.5

9.5

9.5

9.5

9.5

9.5

9.5

9.5

9.5

2.5 9.5 LinFt

Skew Slope

Width Depth Qty Plies

Depth

1.75

1 75

1.75

3.5

2.5

2.5

2.5

2.5

2.5

2.5

2.5

2.5

2.5

2.5

Width

1.125

Qty

2

Plies

2

2

2

Qty Plies

Qty Plies

Beam/Girder

fasteners

30 16d

4 10dx1 1/2

Pcs Length

Pcs Length

8-0-0

6-0-0

18-0-0

18-0-0

14-0-0

12-0-0

4-0-0

2-0-0

14-0-0

12-0-0

10-0-0

8-0-0

6-0-0

2

2

15

1

1

4

13

18

2

Pcs Length

Pcs Length

Varies | 15-0-0

Supported

Member

fasteners

10 16d

2 10dx1 1/2

6

14

- vindow header along inside face of
- er guide for installation works.
- nended to be installed at end bearing on ch support loading from above exceeding
- be installed under all point loads.
- responsibility that floor joists and beams are nger manufacturer's standards.

Connection Detail to ply to ply nailing or

Rim parallel to joists: 1-1/8" rimboard with 2"x 4" block (1/16" longer than rim depth @ 16" o/c). All other components and structural elements supporting the floor system such as beams, walls, columns, and foundation walls and footings including anchorage of components and bracing for lateral stability are the responsibility of Others.

Hatch area represents ceramic tiled floor with an addtional dead load of 5 PSF

The framing shown on this layout may deviate from the architectural and structural drawings. Project Engineer to review and approve the deviation prior to construction

ARCHITECTURAL DRAWINGS:

JARDIN DESIGN GROUP INC. 64 Jardin Dr., Suite 3A, Vaughan, ON Date: Rev.2; May 22,2018 Project No: 17-55 Model: Liana 3

19-444489.000.00.RR.







Second Floor BBO2-A F1-A - 1 ply J6-F J11-F J11-D F4-A - 1 J6-E J6-D J6-C Spcg @ 16" o/c Spcq @ 16" o/c

THIS CERTIFICATION IS TO CONFIRM THAT:

- 1. THE LOADS USED IN THE CALCULATION OF THE ATTACHED APPROVED COMPONENTS CONFORM TO THE FLOOR ASSEMBLY SHOWN ON THIS LAYOUT.
- 2. THE FLOOR JOISTS COMPLY WITH THE NASCOR SPAN TABLE FOR THE LOADS AND SPACING SHOWN ON THIS LAYOUT.

THE FLOOR SYSTEM MUST BE ASSEMBLED IN ACCORDANCE TO THE NASCOR SPECIFIER GUIDE. MULTI-PLY MEMBERS MUST BE ATTACHED TOGETHER AS PER THE INCLUDED MULTIPLE MEMBER CONNECTION DETAIL.

ALL OTHER COMPONENTS AND STRUCTURAL ELEMENTS SUPPORTING THE FLOOR SYSTEM SUCH AS BEAMS, WALLS, COLUMNS AND FOUNDATION WALLS AND FOOTINGS INCLUDING ANCHORAGE OF COMPONENTS AND BRACING FOR LATERAL STABILITY ARE THE RESPONSIBILITY OF OTHERS.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS. PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS



ARCHITECTURAL DRAWINGS:

JARDIN DESIGN GROUP INC. 64 Jardin Dr., Suite 3A, Vaughan, ON Date: Rev.2; May 22,2018 Project No: 17-55 Model: Liana 3

Legend



Load from Above Wall

Norbord Rimboard Plus 1.125 X 9.5 NJ60H 9.5

NJ60U 9 5 NJH 9.5

Forex 2.0E-3000Fb LVL 1.75 X 9.5 (Dropped) Forex 2.0E-3000Fb LVL 1.75 X 9.5

- 1. OBC 2012 O.Reg 332/12 as amended
- 2. Nascor CCMC 13535-R
- 3. LVL CCMC -14056-R
- 4. CAN/CSA-O86-09
- 5. CCMC -12787-R APA PR-L310(C)

Second											
LVL/LS											ALA COMP
	Descr		Width		epth	Q	-	Plies	Pcs	Length	NASCOR
F12	Forex 2.0E-3	3000Fb LVL	1.7	5	9.5	1	1	2	2	14-0-0	
F3	Forex 2.0E-3	3000Fb LVL	1.75	ŝ	9.5	1	1	2	2	12-0-0	Layout Name LIANA 3 (ELEV.2)
F11	Forex 2.0E-3	3000Fb LVL	1.78	5	9.5	1	1	2	2	6-0-0	Design Method LSD
F1	Forex 2.0E-3	3000Fb LVL	1.75	ō	9.5				1	6-0-0	Description
F10	Forex 2.0E-3	3000Fb LVL	1.75	5	9.5				1	4-0-0	GRANELLI HOME CORP. BRAMPTON, ONT.
F4	Forex 2.0E-3	3000Fb LVL	1.75	5	9.5				2	2-0-0	Created May 29, 2018
LVL/LS	L (Dro	pped)									
Label	Descr	ription	Width	n De	epth	Qt	ty	Plies	Pcs	Length	Builder
BBO4			1.75	5	9.5	1	\cap	2	2	14-0-0	GREEN YORK HOMES
	-	3000Fb LVL									Sales Rep
	(Flush)									To the state of	RM
	Descr	<u> </u>	Width	_	epth	Qt	ty	Plies	Pcs	Length	Designer
J5	NJ60H		2.5	_	9.5			\longrightarrow	8	14-0-0	RCO
J11	NJ60U		3.5		9.5			\longrightarrow	17	18-0-0	Shipping
J6	NJ60U	J	3.5	_	9.5	-			10	16-0-0	
J8	NJH		2.5		9.5	<u> </u>	\rightarrow		17	14-0-0	Project
J4	NJH		2.5	-	9.5	-	-		15	12-0-0	Builder's Project
J2	NJH		2.5	_	9.5	-	-		3	8-0-0 6-0-0	Kott Lumber Company
J1 J9	NJH		2.5	_	9.5		-		9	6-0-0 4-0-0	14 Anderson Blvd
Rim Bo			4.0	,	9.5				9	4-0-0	Stouffville, Ontario
	Descr	Intion	Width	- I De	epth	Qt	4.,	Plies	Pcs	Length	Canada
R1		rd Rimboard		-	9.5	- Q	·y	Files	19	Length 12	L4A 7X4
N.I		.125 X 9.5	1.120	1	9.5				15	14	905-642-4400
Blockin											Job Path
	Descr	iption	Width	ı De	epth	Qt	ty	Plies	Pcs	Length	D:\Users\rochavillo\WORK FROM
BLK1	NJH		2.5	5	9.5	Lin	Ft		Varies	8-0-0	HOME\GREEN YORK HOMES
Hanger	f					- 3	Bea	am/Girder	Sur	oported	\GRANELLI HOME CORP\MODELS \LIANA 3\LIANA 3 ELEV 2\FLOOR
										ember	Second Floor
Label	Pcs	Description	n	Skew	Slo	ре	fa	steners	fas	teners	Design Method LSD
H1	1	Unknown Hanger									Building Code NBCC 2010 / OBC 2012
H2	2	HUS1.81/10	0			\perp		30 16d		0 16d	Floor
НЗ	24	LT259						10dx1 1/2		Odx1 1/2	Loads
H4	1	HGUS410	\rightarrow			\perp		46 16d		6 16d	Live 40
H5	5	LT359	1					4 10d	2 10	0dx1 1/2	Dead 15

NOTES:

- Framer to verify dimensions on the architectural drawings.
- 2. Double joist only require filler/backer ply when supporting
- another member using a face-mounted hanger.
- 3. Install 2x4 blocking @ 24" o/c under parallel non-load bearing walls. 4. Install single-ply flush window header along inside face of rimboard/rimjoist
- Refer to Nascor specifier guide for installation works.
- . Squash blocks recommended to be installed at end bearing on all first level joists which support loading from above exceeding two levels floor or roof.
- '. Load transfer blocks to be installed under all point loads.
- B. It shall be the framer's responsibility that floor joists and beams are fastened as per the hanger manufacturer's standards.

Refer to Multiple Member Connection Detail to ply to ply nailing or bolting requirements.

Rim parallel to joists: 1-1/8" rimboard with 2"x 4" block (1/16" longer than rim depth @ 16" o/c). All other components and structural elements supporting the floor system such as beams, walls, columns, and foundation walls and footings including anchorage of components and bracing for lateral stability are the responsibility of Others.

Hatch area represents ceramic tiled floor with an addtional dead load of 5 PSF

The framing shown on this layout may deviate from the architectural and structural drawings. Project Engineer to review and approve the deviation prior to construction.

Deflection Joist

LL Span L/

TL Span L/

LL Cant 2L/

TL Cant 2L/

LL Span L/

TL Span L/

LL Cant 2L/

TL Cant 2L/

Decking

Thickness

Fastener

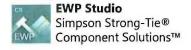
Vibration

Ceiling:

Deck

Deflection Girder







480

360

480

360

360

240

480

360

OSB

5/8"

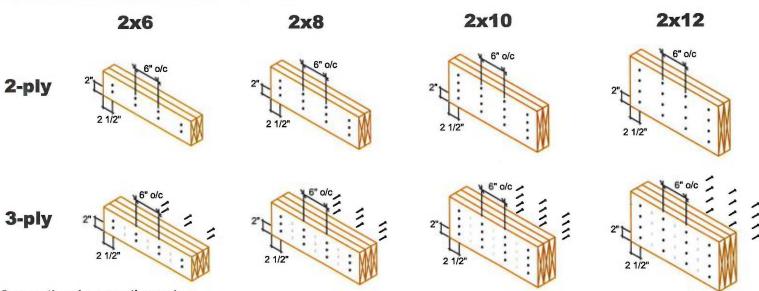
Nailed & Glued

Gypsum 1/2"

INEUU 10-UZZ

MULTIPLE MEMBER CONNECTIONS

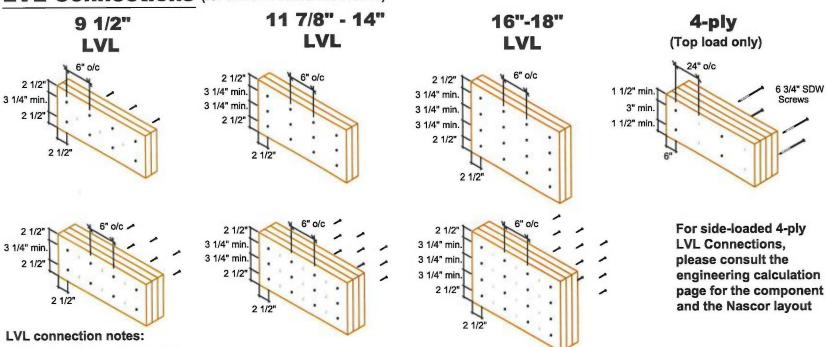
Conventional Connections (for uniform distributed loads)



Conventional connection notes:

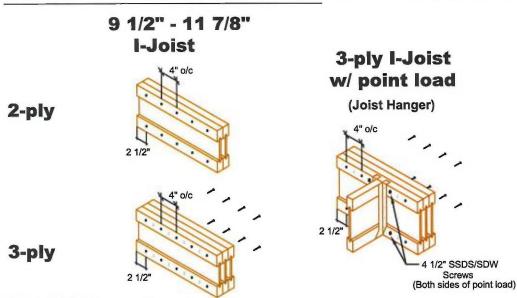
- -Nails to be 3" 10d spiral wire nails.
- -Nails to be located a minimum of 2" from the top and bottom of the member. Start all nails a minimum of 2 1/2" in from ends.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail driven from the opposite side.

LVL Connections (for uniform distributed loads)



- -Nails to be 3 1/2" spiral wire nails.
- -Nails to be located a minimum of 2 1/2" from the top and bottom of the member. Start all nails a minimum of 2 1/2" in from ends.
- -Minimum 3 1/4" spacing between rows.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail or screw driven from the opposite side.

Vertical I-Joist Connections (for uniform distributed loads)



Vertical I-Joist connection notes:

- -Nails to be 3" spiral wire nails.
- -Nails to be located at centre of top and bottom flanges. Start all nails a minimum of 2 1/2" in from ends.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail driven from the opposite side.



MULTI-PLY CONNECTION **DETAILS**

Date: November 30, 2016 Scale: NTS

KOTT 3228 Moodie Drive Ottawa, ON K2H 7V1 Ph: 613-838-2775

FAGE 2 OF 30

Fx: 613-838-4751

Engineering Note Page (ENP-2) M-2057

REVISION 2009-10-09

LOT 19

Please read all notes prior to installation of the component

DESIGN INFORMATION

This building component is certified as an individual component for the loads and conditions shown on the calculation and drawing page.

The responsibility of the undersigned engineer is only limited to the calculation of this building component for the loads and conditions shown on this drawing.

The responsibility of the undersigned is limited to the verification of the structural capacity of the NASCOR floor joists and LVL beams based on placement as shown on the layout. The loads applied are limited to the gravity effects of the specified loads. The structural integrity of the building and the effect of wind, uplift, seismic, lateral or other forces, calculation of adequate support and anchorage of components, as well as the dimensions and design loads used to calculate components are the responsibility of the overall building designer.

Floor joists and OSB rim board are designed to carry uniformly distributed loads only. Point loads should be transferred through the floor cavity with squash blocks. Structural elements such as walls, posts, connectors, and squash blocks are the responsibility of the overall building designer.

The undersigned engineer disclaims any responsibility for damages as a result of being furnished faulty or incorrect information, specifications and/or designs.

Installation of NASCOR joists is to be carried out in accordance with the current edition of the manufacturer's approved literature available at http://www.nascor.ca.

CODE

This building component is designed in accordance with the National Building Code of Canada, the Ontario Building Code, CCMC and Canadian Standards Association guidelines.

COMPONENT

- 1. The building component used in construction must be the same as indicated on the drawings.
- 2. The building component must be installed and assembled as per specification shown on the drawing and in accordance with the manufacturer's assembly and installation.
- 3. Members consisting of multiple plies must be connected as per the document "Multi-ply Connection Details".
- 4. Pass-thru squash block framing is required at all point loads over bearings.

HANDLING AND INSTALLATION

Do not drill any hole, cut or notch a certified building component without a written preauthorization.





Client: Project:

Address:

GREEN YORK HOMES

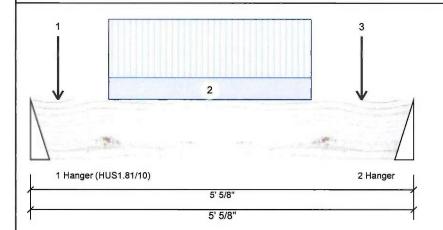
Date: 5/31/2018

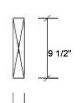
Designer: RCO

Job Name: LIANA 3 (ELEV.1)

Project #:

Level: Ground Floor Forex 2.0E-3000Fb LVL 1.750" X 9.500" - PASSED





0

0

Ld. Comb.

1.25D+1.5L

1.25D+1.5L

Member Information Type: Girder Application: Floor (Residential) Plies: Design Method: Moisture Condition: Dry NBCC 2010 / OBC 2012 **Building Code:** Deflection LL: Load Sharing: 360 No Deflection TL: Not Checked 240 Deck: Not Checked Importance: Vibration: Normal General Load 40 PSF Floor Live: Dead: **15 PSF**

omactored Reactions ONPATTERINED ID (Opint)								
Brg	Live	Dead	Snow	Wind				

Cap. React D/L lb

136

127

0

0

Total Ld. Case

677 L

629 L

Unfortered Denetions UNDATTEDALED IL (Unlife)

338

314

Bearing Length

1 -Hanger

2 -Hanger 3.000"

3.000"

Bearings and Factored Reactions	

170 / 507

159 / 471

Analysis Results

ı	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
l	Moment	716 ft-lb	2'6 7/16"	11362 ft-lb	0.063 (6%)	1.25D+1.5L	L
l	Unbraced	716 ft-lb	2'6 7/16"	7908 ft-lb	0.091 (9%)	1.25D+1.5L	L
l	Shear	672 lb	11 3/4"	4638 lb	0.145 (14%)	1.25D+1.5L	L
	Perm Defl in.	0.003 (L/17255)	2'6 7/16"	0.156 (L/360)	0.020 (2%)	D	Uniform
l	LL Defl inch	0.008 (L/6957)	2'6 7/16"	0.156 (L/360)	0.050 (5%)	L	L
l	TL Defl inch	0.011 (L/4958)	2'6 7/16"	0.234 (L/240)	0.050 (5%)	D+L	L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA

17%

16%

USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL**



Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.

4 Bottom	4 Bottom braced at bearings.						POINT LOADS OVER BEARINGS.				
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments		
1	Point	0-4-6		Far Face	48 lb	128 lb	0 lb	0 lb	J2		
2	Part. Uniform	1-0-6 to 3-8-6		Far Face	52 PLF	139 PLF	0 PLF	0 PLF			
3	Point	4-4-6		Far Face	57 lb	153 lb	0 ІЬ	0 lb	J2		
	Self Weight				4 PLF						

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. Lumber

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- chemicals Handling & Installation

 - LVL beams must not be cut or drilled Refer to manufacturer's product informatior regarding installation requirements, multi-ply fastening details, beam strength values, and code

 - approvals
 Damaged Bearns must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent

Manufacturer Info Forex APA: PR-L318







Client: Project:

Address:

GREEN YORK HOMES

Date: 5/31/2018 Designer: RCO

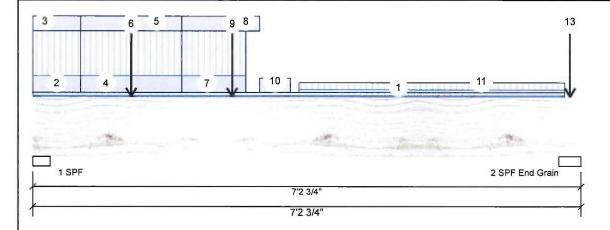
> Job Name: LIANA 3 (ELEV.1)

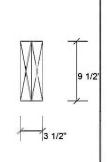
Project #:

F2-A Forex 2.0E-3000Fb LVL 1.750" X 9.500"

2-Ply - PASSED

Level: Ground Floor





Wind

Page 1 of 2

Member I	nformat	tion
----------	---------	------

Type:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	
Floor Live:	40 PSF

15 PSF

Application: Design Method:

Building Code:

Load Sharing:

LSD

NBCC 2010 / OBC 2012 No

Floor (Residential)

Deck: Not Checked Vibration: Not Checked

Unfactored Reactions UNPATTERNED Ib (Uplift) Live Dead Snow

1	1088	645	0	0
2	1613	756	0	0

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	2.750"	41%	807 / 1632	2438	L	1.25D+1.5L
2 - SPF End	3.500"	37%	945 / 2419	3364	L	1.25D+1.5L

Grain

Analysis Results

Dead:

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	3003 ft-lb	2'7 9/16"	22724 ft-lb	0.132 (13%)	1.25D+1.5L	L
Unbraced	3003 ft-lb	2'7 9/16"	21802 ft-lb	0.138 (14%)	1.25D+1.5L	L
Shear	1838 lb	11 1/2"	9277 lb	0.198 (20%)	1.25D+1.5L	L
Perm Defl in.	0.014 (L/5673)	3'1 13/16"	0.228 (L/360)	0.060 (6%)	D	Uniform
LL Defl inch	0.024 (L/3355)	3'1 9/16"	0.228 (L/360)	0.110 (11%)	L	L
TL Defl inch	0.039 (L/2108)	3'1 5/8"	0.342 (L/240)	0.110 (11%)	D+L	L

Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on full section width.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments	
1	Tie-In	0-0-0 to 7-0-2	(Span)0-10-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
2	Part. Uniform	0-0-0 to 0-7-9		Тор	92 PLF	246 PLF	0 PLF	0 PLF	J8	
3	Part. Uniform	0-0-0 to 0-7-9		Тор	80 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight	
4	Part. Uniform	0-7-9 to 1-11-9		Тор	92 PLF	246 PLF	0 PLF	0 PLF	J8	
5	Part. Uniform	0-7-9 to 1-11-9		Тор	80 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight	

Continued on page 2...

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the Intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- chemicals
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply regarding installation requirements, multi-pay fastening details, beam strength values, and code
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info APA: PR-L318







Client: Project: Address: GREEN YORK HOMES

Date:

5/31/2018

Designer: RCO

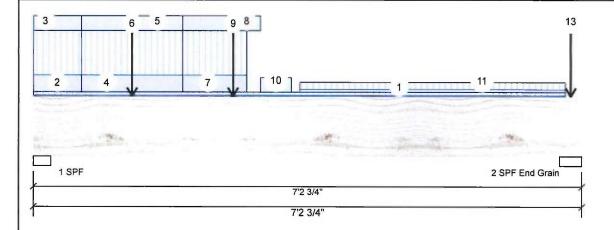
Job Name: LIANA 3 (ELEV.1)

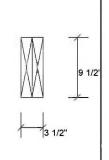
Project #:

F2-A Forex 2.0E-3000Fb LVL 1.750" X 9.500"

2-Ply - PASSED

Level: Ground Floor





Page 2 of 2

Continued	from page 1								
ID	Load Type	Location T	Trib Width	Side	Dead	Live	Snow	Wind	Comments
6	Point	1-3-9		Far Face	121 lb	322 lb	0 lb	0 lb	J8
7	Part. Uniform	1-11-9 to 2-9-11		Тор	92 PLF	246 PLF	0 PLF	0 PLF	J8
8	Part. Uniform	1-11-9 to 2-11-14		Тор	80 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
9	Point	2-7-9		Far Face	93 lb	248 lb	0 lb	0 lb	J8
10	Part, Uniform	2-11-14 to 3-4-12		Тор	80 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
11	Part. Uniform	3-6-2 to 7-0-2		Тор	15 PLF	40 PLF	0 PLF	0 PLF	
12	Point	7-1-0		Near Face	127 lb	314 lb	0 lb	0 lb	F1
13	Point	7-1-0		Тор	379 lb	869 lb	0 lb	0 lb	C3
	Self Weight				8 PLF				

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design ortena and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- chemicals

- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastering details, beam strength values, and code

- parprovals

 Damaged Beams must not be used

 Design assumes top edge is laterally restrained

 Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex APA: PR-L318







Client: Project: Address: **GREEN YORK HOMES**

Date: 5/31/2018 Designer: RCO

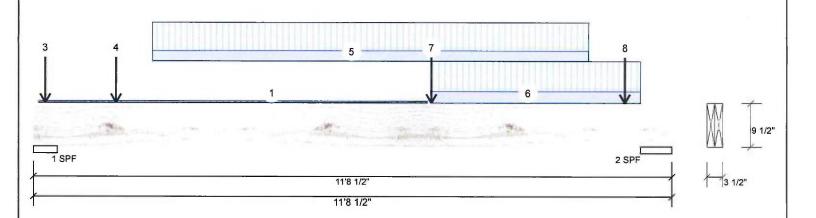
Job Name: LIANA 3 (ELEV.1)

Project #:

F3-B Forex 2.0E-3000Fb LVL

1.750" X 9.500"

2-Ply - PASSED Level: Ground Floor



/lember Inforn	nation			Unfactor	ed React	tions U	NPATTERNI	ED lb (Uplift)	
Туре:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snov	V	Wind
Plies:	2	Design Method:	LSD	1	1523		642		0	0
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012	2	2170		865		0	0
Deflection LL:	360	Load Sharing:	No	***						
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal	Vibration:	Not Checked							
General Load										
Floor Live:	40 PSF			Bearings	and Fac	tored F	Reactions			
Dead:	15 PSF			Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.
				1 - SPF	5.250"	27%	803 / 2285	3088	L	1.25D+1.5L
				2-SPF	6.875"	29%	1081 / 3255	4335	L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	10162 ft-lb	6'11 3/4"	22724 ft-lb	0.447 (45%)	1.25D+1.5L	L
Unbrace	d 10162 ft-lb	6'11 3/4"	20411 ft-lb	0.498 (50%)	1.25D+1.5L	L
Shear	3978 lb	10'4 7/8"	9277 lb	0.429 (43%)	1.25D+1.5L	L
Perm De	efl in. 0.091 (L/1425)	6'	0.361 (L/360)	0.250 (25%)	D	Uniform
LL Defl i	nch 0.227 (L/571)	6' 1/8"	0.361 (L/360)	0.630 (63%)	L	L
TL Defl i	nch 0.319 (L/408)	6' 1/8"	0.541 (L/240)	0.590 (59%)	D+L	L

Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on full section width

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Page 1 of 2

/ Lateral	sienderness ratio based o	on full section wiath.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-1-2 to 7-2-12	(Span)0-6-2	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	0-2-10		Тор	37 lb	99 lb	0 lb	0 lb	J11
3	Point	0-2-10		Тор	24 lb	0 lb	0 lb	0 lb	Wall Self Weight
4	Point	1-6-3		Near Face	103 lb	276 lb	0 lb	0 lb	J4
5	Part. Uniform	2-2-3 to 10-2-3		Near Face	84 PLF	224 PLF	0 PLF	0 PLF	

Continued on page 2...

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation

Manufacturer Info For flat roofs provide proper drainage to prevent ponding

APA: PR-L318





Client: Project: Address: GREEN YORK HOMES

5/31/2018 Date:

Designer: RCO

Job Name: LIANA 3 (ELEV.1)

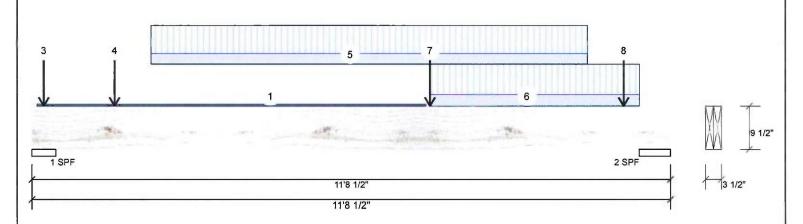
Project #:

Forex 2.0E-3000Fb LVL F3-B

1.750" X 9.500"

2-Ply - PASSED

Level: Ground Floor



Continued fr	om page 1								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
6	Part. Uniform	7-3-10 to 11-1-10		Тор	90 PLF	240 PLF	0 PLF	0 PLF	
7	Point	7-3-10		Far Face	136 lb	338 lb	0 lb	0 lb	F1
8	Point	10-10-3		Near Face	73 lb	195 lb	0 lb	0 lb	J4
	Self Weight				8 PLF				

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS

NOtes:

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- andling & installation

 LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
 approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info Forex APA: PR-L318



Kott Lumber Company 14 Anderson Blvd, Ontario Canada L4A 7X4 905-642-4400





Page 2 of 2

EWP Studio Simpson Strong-Tie® Component Solutions™

Client:

GREEN YORK HOMES

Project: Address: Date: 5/31/2018

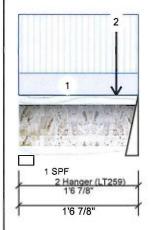
RCO Designer:

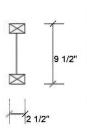
LIANA 3 (ELEV.1) Job Name:

Project #:

9.500" - PASSED F5-A NJH

Level: Ground Floor





Mind

Member Inform	nation
Туре:	Girder
Plies:	1
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	
Floor Live:	40 PSF
Dead:	15 PSF

Application: Floor (Residential) Design Method: **Building Code:**

NBCC 2010 / OBC 2012

Bra

Load Sharing: Deck: Not Checked

Not Checked Vibration:

Unfactored	Reactions	UNPATTERNED	lb	(Uplift)
------------	-----------	-------------	----	----------

Dig	LIVE	Dead	SHOW	VVIIIG	
1	62	23	0	0	
2	115	43	0	0	

Snow

Dead

Bearings and Factored Reactions

Live

Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	2.375"	8%	29 / 93	122	L	1.25D+1.5L
2 - Hanger	2.000"	14%	53 / 173	226	L	1.25D+1.5L

Analysis Results

l	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
	Moment	43 ft-lb	11 9/16"	3830 ft-lb	0.011 (1%)	1.25D+1.5L	L
l	Unbraced	43 ft-lb	11 9/16"	3779 ft-lb	0.011 (1%)	1.25D+1.5L	L
	Shear	213 lb	1'5 5/8"	1580 lb	0.135 (13%)	1.25D+1.5L	L
	Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
	LL Defl inch	0.001 (L/31491)	11 3/8"	0.044 (L/360)	0.010 (1%)	L	L
	TL Defl inch	0.001 (L/22944)	11 3/8"	0.067 (L/240)	0.010 (1%)	D+L	L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top flange unbraced.
- 4 Bottom flange braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-6-14	(Span)3-2-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	1-3-7		Far Face	28 lb	76 lb	0 lb	0 lb	J7

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design orther and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component sultability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

chemicals

- Libist flanges must not be cut or drilled Refer to latest copy of the IJoist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-ply fastening details and handling/erection details Damaged IJoists must not be used Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.
- 5 Provide lateral support at bearing points to avoid
- lateral displacement and rotation
 6. Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 7. For flat roofs provide proper drainage to prevent

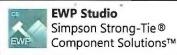
Manufacturer Info

Nascor by Kott









Client: Project:

Address:

GREEN YORK HOMES

Date: 5/31/2018

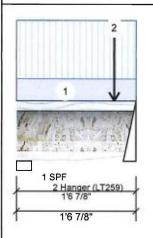
Designer: RCO

Job Name: LIANA 3 (ELEV.1)

Level: Ground Floor

Project #:

F5-B 9.500" - PASSED NJH



9 1/2"

Mem	ber l	Info	rmati	ion
-----	-------	------	-------	-----

Type:	Girder
Plies;	1
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	
Floor Live:	40 PSF
Dead:	15 PSF

Application:

Design Method: **Building Code:** NBCC 2010 / OBC 2012 Load Sharing: No

Floor (Residential)

Not Checked Deck: Vibration: Not Checked

Unfactored Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	Wind
1	62	23	0	0
2	113	43	0	0

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	2.375"	8%	29 / 92	121	L	1.25D+1.5L
2 - Hanger	2.000"	14%	53 / 170	223	L	1.25D+1.5L

Analysis Results

l	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
l	Moment	43 ft-lb	11 1/2"	3830 ft-lb	0.011 (1%)	1.25D+1.5L	L
l	Unbraced	43 ft-lb	11 1/2"	3779 ft-lb	0.011 (1%)	1.25D+1.5L	L
l	Shear	210 lb	1'5 5/8"	1580 lb	0.133 (13%)	1.25D+1.5L	L
l	Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
	LL Defl inch	0.001 (L/31811)	11 3/8"	0.044 (L/360)	0.010 (1%)	L	L
	TL Defl inch	0.001 (L/23114)	11 3/8"	0.067 (L/240)	0.010 (1%)	D+L	L.

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top flange unbraced.
- 4 Bottom flange braced at bearings.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



al	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-6-14	(Span)3-2-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	1-3-7		Near Face	28 lb	74 lb	0 lb	0 lb	J1

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 Use I Joist not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- I. Julist flanges must not be cut or drilled
 Refer to latest copy of the Lloist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-ply fastening details and handling/srection details
 Damaged Ljoists must not be used
 Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing lengths—3.5 inches
 For flat roofs provide proper drainage to prevent

Manufacturer info

Nascor by Kott







Client: Project:

Address:

GREEN YORK HOMES

Date: 5/31/2018

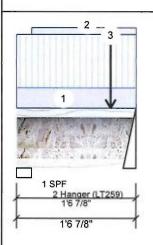
Designer: RCO

Job Name: LIANA 3 (ELEV.1)

Level: Ground Floor

Project #:

9.500" - PASSED F5-C NJH



9 1/2"

	Information
Туре:	Girder

Plies: Moisture Condition: Dry Deflection LL: Deflection TL: 240

General Load Floor Live: 40 PSF Dead: 15 PSF

Normal

Application: Design Method:

Building Code:

Load Sharing:

Deck:

Vibration:

LSD

NBCC 2010 / OBC 2012 No

Floor (Residential)

Not Checked Not Checked

Unfactored Reactions UNPATTERNED Ib (Uplift)	Unfactored	Reactions	UNPATTERNED	lb	(Uplift)
--	------------	-----------	-------------	----	----------

Brg	Live	Dead	Snow	Wind	_
1	70	33	0	0	
2	141	69	0	0	

Bearings and Factored Reactions

Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	2.375"	9%	42 / 106	147	L	1.25D+1.5L
2 - Hanger	2.000"	19%	87 / 211	298	L	1.25D+1.5L

Analysis Results

Importance:

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	63 ft-lb	1'1 1/8"	3830 ft-lb	0.016 (2%)	1.25D+1.5L	L
Unbraced	63 ft-lb	1'1 1/8"	3779 ft-lb	0.017 (2%)	1.25D+1.5L	L
Shear	283 lb	1'5 5/8"	1580 lb	0.179 (18%)	1.25D+1.5L	L
Perm Defl in.	0.000 (L/47832)	1' 5/8"	0.044 (L/360)	0.010 (1%)	D	Uniform
LL Defl inch	0.001 (L/23647)	1' 5/8"	0.044 (L/360)	0.020 (2%)	L	L
TL Defl inch	0.001 (L/15824)	1' 5/8"	0.067 (L/240)	0.020 (2%)	D+L	L

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top flange unbraced.
- 4 Bottom flange braced at bearings.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-6-14	(Span)3-2-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Part. Uniform	0-2-6 to 1-6-14		Тор	8 PLF	0 PLF	0 PLF	0 PLF	
3	Point	1-2-14		Near Face	54 lb	110 lb	0 lb	0 lb	J2

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. Upost flanges must not be cut or drilled
 2. Refer to latest copy of the Lipids product information details for framing details, sufficient tables, web hole chart, bridging details, multi-ply tastening details and handling/erection details
 2. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

Manufacturer Info

Nascor by Kott









Client:

GREEN YORK HOMES

Project: Address: Date:

5/31/2018

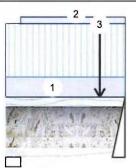
Designer: RCO

Job Name: LIANA 3 (ELEV.1)

Level: Ground Floor

Project #:

9.500" - PASSED F5-D NJH



9 1/2"

		- 3 -
	1	V
		P
	1000	301
	SPF	L
	2 Hange 1'6 7/8	(LT259)
le.		"

Type: Girder Plies: Moisture Condition: Dry Deflection LL: 360 Deflection TL: 240 Importance: Normal General Load

Member Information

40 PSF 15 PSF Application: Design Method:

Vibration:

Floor (Residential)

NBCC 2010 / OBC 2012 **Building Code:** No

Load Sharing: Deck:

Not Checked Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live Dead		Snow	Wind
1	70	33	0	0
2	136	67	0	0
1				

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.	
1 - SPF	2.375"	9%	41 / 104	145	L	1.25D+1.5L	
2 - Hanger	2.000"	18%	84 / 205	288	L	1.25D+1.5L	

Analysis Results

Floor Live:

Dead:

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	61 ft-lb	1'1"	3830 ft-lb	0.016 (2%)	1.25D+1.5L	L
Unbraced	61 ft-lb	1'1"	3779 ft-lb	0.016 (2%)	1.25D+1.5L	L
Shear	274 lb	1'5 5/8"	1580 lb	0.173 (17%)	1.25D+1.5L	L
Perm Defl in.	0.000 (L/49408)	1' 7/16"	0.044 (L/360)	0.010 (1%)	D	Uniform
LL Defl inch	0.001 (L/24289)	1' 9/16"	0.044 (L/360)	0.010 (1%)	L	L
TL Defl inch	0.001 (L/16284)	1' 1/2"	0.067 (L/240)	0.010 (1%)	D+L	L

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top flange unbraced.
- 4 Bottom flange braced at bearings.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-6-14	(Span)3-2-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Part. Uniform	0-2-6 to 1-6-14		Тор	8 PLF	0 PLF	0 PLF	0 PLF	
3	Point	1-2-14		Far Face	51 lb	105 lb	0 lb	0 lb	J2

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. Lumber

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- arruting a Installation.

 Lioist flanges must not be cut or drilled.

 Refer to latest copy of the IJoist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-liy fastening details and handling/erection details.

 Damaged IJoists must not be used.

 Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.
- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length=3.5 inches
 For flat roofs provide proper drainage to prevent

Manufacturer Info

Nascor by Kott









Client: Project: **GREEN YORK HOMES**

Address:

5/31/2018 Date:

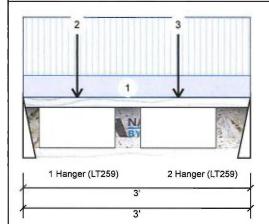
RCO Designer:

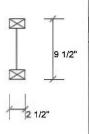
Job Name: LIANA 3 (ELEV.1)

Project #:

9.500" - PASSED F6-A NJH

Level: Ground Floor





Wind

PROFESSIONAL

0

0

Member Inform	nation		
Туре:	Girder	Application:	Floor (Residential)
Plies;	1	Design Method:	LSD
Moisture Condition	: Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

Bearings and Factored Reactions									
Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.			
1 - Hanger	2.000"	32%	122 / 390	512	L	1.25D+1.5L			
2 -	2.000"	29%	109 / 350	459	L	1.25D+1.5L			

Unfactored Reactions UNPATTERNED Ib (Uplift)

97

88

Live

260

233

1 2

Hanger

Analysis Results Actual Analysis Location Allowed Capacity Comb. Case Moment 360 ft-lb 2' 9/16" 3830 ft-lb 0.094 (9%) 1.25D+1.5L L Unbraced 360 ft-lb 2' 9/16" 3411 ft-lb 0.105 (11%) 1.25D+1.5L L 505 lb 0.319 (32%) 1.25D+1.5L L Shear 1 1/4" 1580 lb Perm Defl in. 0.002 1'8 1/4" 0.093 (L/360) 0.020 (2%) D Uniform (L/16963) LL Defl inch 0.005 (L/6359) 1'8 1/4" 0.093 (L/360) 0.060 (6%) L TL Defl inch 0.007 (L/4625) 1'8 1/4" 0.140 (L/240) 0.050 (5%) D+L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO P NAILING OR BOLTING REQUIREMEN

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS

LY		
ITS.	******	
		(

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top flange unbraced.
- 4 Bottom flange braced at bearings.

Live Snow Wind Comment	ts
40 PSF 0 PSF 0 PSF	
188 lb 0 lb 0 lb J3	
202 lb 0 lb 0 lb J3	
	40 PSF 0 PSF 0 PSF 188 lb 0 lb 0 lb J3



Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 Idoist not to be treated with fire retardant or corrosive

chemicals

- Handling & Installation
- Identaling & Installation

 I Joist flanges must not be cut or drilled

 Refer to latest copy of the Judist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-by fastening details and handling/erection details

 Damaged Joists must not be used

 Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.
- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roofs provide proper drainage to prevent

Manufacturer Info

Nascor by Kott





Client: Project: Address:

GREEN YORK HOMES

5/31/2018 Date:

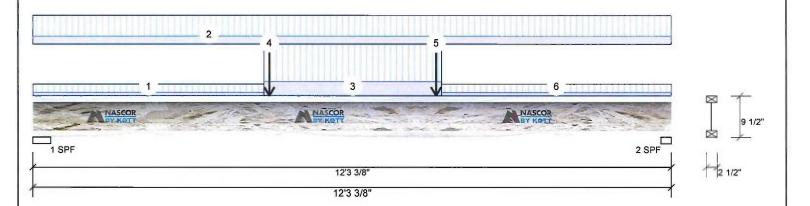
Designer: RCO

Job Name: LIANA 3 (ELEV.1)

Project #:

9.500" - PASSED F7-A NJH

Level: Ground Floor



Member Information **Unfactored Reactions UNPATTERNED Ib (Uplift)** Type: Girder Application: Floor (Residential) Live Dead Snow Wind Plies: Design Method: 327 123 0 0 1 Moisture Condition: Dry NBCC 2010 / OBC 2012 **Building Code:** 2 320 120 0 0 Deflection LL: 360 Load Sharing: No Deflection TL: Not Checked 240 Deck: Importance: Not Checked Normal Vibration: General Load **Bearings and Factored Reactions** 40 PSF Floor Live: Dead: **15 PSF** Cap. React D/L lb Ld. Comb. Bearing Length Total Ld. Case 41% 154 / 490 1 - SPF 4.125" 644 L 1.25D+1.5L 2 - SPF 2.375" 40% 150 / 480 630 L 1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2352 ft-lb	6'2 11/16"	3830 ft-lb	0.614 (61%)	1.25D+1.5L	L
Unbraced	2352 ft-lb	6'2 11/16"	2368 ft-lb	0.993 (99%)	1.25D+1.5L	L
Shear	629 lb	3 3/8"	1580 lb	0.398 (40%)	1.25D+1.5L	L
Perm Defl in.	0.071 (L/2010)	6'2 1/2"	0.395 (L/360)	0.180 (18%)	D	Uniform
LL Defl inch	0.188 (L/756)	6'2 9/16"	0.395 (L/360)	0.480 (48%)	L	L
TI Deflinch	0.259 (L/549)	6'2 9/16"	0.593 (L/240)	0.440 (44%)	D+L	L

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Top flange must be laterally braced at a maximum of 4'9" o.c.

3 Bottom flange braced at bearings

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

ASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARING



ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 4-5-5	(Span)0-4-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 12-3-6	(Span)0-11-5	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Tie-In	4-5-5 to 7-10-5	(Span)1-8-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
4	Point	4-6-9		Far Face	43 lb	113 lb	0 lb	0 lb	F5
5	Point	7-9-1		Far Face	43 lb	115 lb	0 lb	0 lb	F5
6	Tie-In	7-10-5 to 12-3-6	(Span)0-4-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. Lumber

Dry service conditions, unless noted otherwise
 Usist not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- Lioist flanges must not be cut or drilled Refer to latest copy of the Lioist product information details for framing details, stiffener subles, web hole chart, bridging details, multi-ply fastening details and handling/erection details
 Damaged Lioists must not be used. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.
- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing lengther 3.5 inches
 For flat roofs provide proper drainage to prevent

Manufacturer Info

Nascor by Kott





Client:

GREEN YORK HOMES

Project: Address:

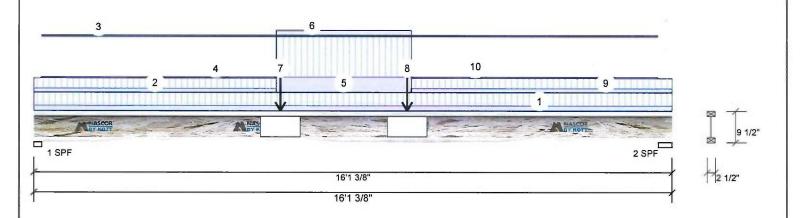
5/31/2018 Date:

RCO Designer:

Job Name: LIANA 3 (ELEV.1)

Project #:

F8-A NJH 9.500" - PASSED Level: Ground Floor



Member Inform	nation			Unfactored Reactions UNPATTERNED Ib (Uplift)				
Туре:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind
Plies:	1	Design Method:	LSD	1	345	167	0	0
Moisture Condition:	: Dry	Building Code:	NBCC 2010 / OBC 2012	2	342	164	0	0
Deflection LL:	360	Load Sharing:	No	70				
Deflection TL:	240	Deck:	Not Checked					
Importance:	Normal	Vibration:	Not Checked					
General Load								
Floor Live:	40 PSF			Bearings	and Fac	tored Reactions		
Dead:	15 PSF			Bearing	Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
				1 - SPF	2.375"	46% 208 / 518	726 L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	3768 ft-lb	8' 3/16"	3830 ft-lb	0.984 (98%)	1.25D+1.5L	L
Unbraced	3768 ft-lb	8' 3/16"	3771 ft-lb	0.999 (100%)	1.25D+1.5L	L
Shear	721 lb	1 5/8"	1580 lb	0.456 (46%)	1.25D+1.5L	L
Perm Defl in.	0.221 (L/852)	7'11 5/8"	0.523 (L/360)	0.420 (42%)	D	Uniform
LL Defl inch	0.456 (L/413)	7'11 5/8"	0.523 (L/360)	0.870 (87%)	L	L
TL Defl inch	0.677 (L/278)	7'11 5/8"	0.785 (L/240)	0.860 (86%)	D+L	L
•	Moment Unbraced Shear Perm Defl in. LL Defl inch	Moment 3768 ft-lb Unbraced 3768 ft-lb	Moment 3768 ft-lb 8' 3/16" Unbraced 3768 ft-lb 8' 3/16" Shear 721 lb 1 5/8" Perm Defl in. 0.221 (L/852) 7'11 5/8" LL Defl inch 0.456 (L/413) 7'11 5/8"	Moment 3768 ft-lb 8' 3/16" 3830 ft-lb Unbraced 3768 ft-lb 8' 3/16" 3771 ft-lb Shear 721 lb 1 5/8" 1580 lb Perm Defl in. 0.221 (L/852) 7'11 5/8" 0.523 (L/360) LL Defl inch 0.456 (L/413) 7'11 5/8" 0.523 (L/360)	Moment 3768 ft-lb 8' 3/16" 3830 ft-lb 0.984 (98%) Unbraced 3768 ft-lb 8' 3/16" 3771 ft-lb 0.999 (100%) Shear 721 lb 1 5/8" 1580 lb 0.456 (46%) Perm Defl in. 0.221 (L/852) 7'11 5/8" 0.523 (L/360) 0.420 (42%) LL Defl inch 0.456 (L/413) 7'11 5/8" 0.523 (L/360) 0.870 (87%)	Moment 3768 ft-lb 8' 3/16" 3830 ft-lb 0.984 (98%) 1.25D+1.5L Unbraced 3768 ft-lb 8' 3/16" 3771 ft-lb 0.999 (100%) 1.25D+1.5L Shear 721 lb 1 5/8" 1580 lb 0.456 (46%) 1.25D+1.5L Perm Defl in. 0.221 (L/852) 7'11 5/8" 0.523 (L/360) 0.420 (42%) D LL Defl inch 0.456 (L/413) 7'11 5/8" 0.523 (L/360) 0.870 (87%) L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

45%

206 / 513

718 L

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.

2 - SPF 4.125"



1.25D+1.5L

FAGE 10 OF 30

Page 1 of 2

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Top flange must be laterally braced at a maximum of 1'5" o.c.

3 Bottom flange braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 16-1-6	(Span)0-6-12	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 6-1-6	(Ѕрап)0-5-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Part. Uniform	0-2-6 to 15-9-1		Тор	1 PLF	0 PLF	0 PLF	0 PLF	
4	Part, Uniform	0-2-6 to 6-1-6		Тор	1 PLF	0 PLF	0 PLF	0 PLF	
5	Tie-In	6-1-6 to 9-6-6	(Span)1-8-11 to 1-8-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
6	Part, Uniform	6-1-6 to 9-6-6		Тор	4 PLF	0 PLF	0 PLF	0 PLF	
7	Point	6-2-10		Near Face	67 lb	136 lb	0 lb	0 lb	F5
•	1 Onic	0210		itcai i acc	07 10	10015	0 10	0 10	1 5

Continued on page 2...

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design cnteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. Joist flanges must not be out or drilled

 2. Refer to latest copy of the IJoist product information details for framing details, siftener tables, web hole chart, bridging details, multi-ply fastening details and handling/erection details

 3. Damaged IJoists must not be used

 4. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.
- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length=3.5 inches
 For flat roofs provide proper drainage to prevent

Manufacturer Info

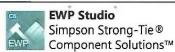
Nascor by Kott







Page 2 of 2



Client:

GREEN YORK HOMES

Project: Address: Date:

5/31/2018

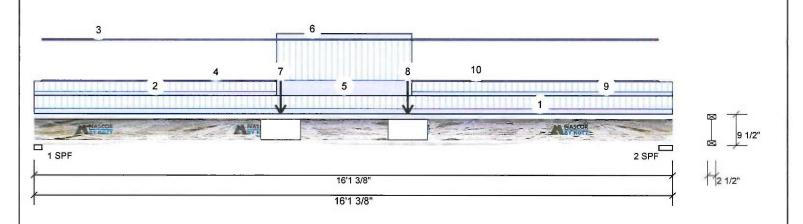
Designer: RCO

Job Name: LIANA 3 (ELEV.1)

Project #:

9.500" - PASSED NJH F8-A

Level: Ground Floor



Continued from									
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
8	Point	9-5-2		Near Face	69 lb	141 lb	0 lb	0 lb	F5
9	Tie-In	9-6-6 to 16-1-6	(Span)0-5-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
10	Part. Uniform	9-6-6 to 15-9-1		Тор	1 PLF	0 PLF	0 PLF	0 PLF	

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design cntern and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

chemicals

- Handling & Installation
- andling & installation

 Lioist flanges must not be cut or drilled

 Refer to latest copy of the Lioist product information details for framing details, stiffens tables, web hole chart, bridging details, multi-ply fastening details and handling/erection details

 Damaged lobist must not be used

 Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length=3.5 inches
 For flat roofs provide proper drainage to prevent

Manufacturer Info

Nascor by Kott







EWP Studio Simpson Strong-Tie® Component Solutions™ Client: Project: Address:

GREEN YORK HOMES

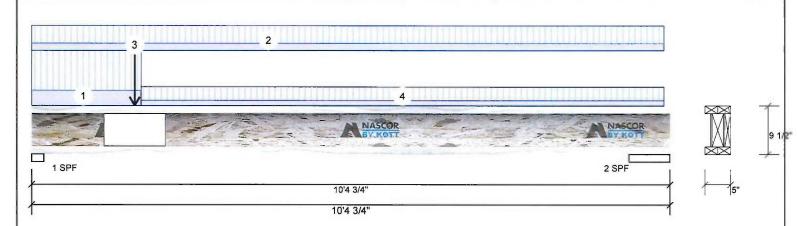
Date: 5/31/2018

Designer: RCO Job Name: LIANA 3 (ELEV.1)

Project #:

9.500" 2-Ply - PASSED F9-A NJH

Level: Ground Floor



Member Information **Unfactored Reactions UNPATTERNED Ib (Uplift)** Type: Girder Application: Floor (Residential) Live Snow Wind Dead Plies: 2 Design Method: LSD 535 201 0 0 Moisture Condition: Dry **Building Code:** NBCC 2010 / OBC 2012 2 328 123 0 0 Load Sharing: Deflection LL: 360 No Deflection TL: 240 Deck: Not Checked Importance: Normal Vibration: Not Checked General Load Bearings and Factored Reactions Floor Live: 40 PSF Dead: 15 PSF Total Ld. Case Cap. React D/L lb Ld. Comb. Bearing Length 33% 251 / 802 1 - SPF 2.375" 1053 L 1.25D+1.5L 2 - SPF 8.000" 20% 154 / 492 646 L 1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1673 ft-lb	4'1 3/4"	7660 ft-lb	0.218 (22%)	1.25D+1.5L	L
Unbraced	1673 ft-lb	4'1 3/4"	1685 ft-lb	0.993 (99%)	1.25D+1.5L	L
Shear	1027 lb	1 5/8"	3160 lb	0.325 (32%)	1.25D+1.5L	L
Perm Defl in.	0.018 (L/6275)	4'8 7/16"	0.322 (L/360)	0.060 (6%)	D	Uniform
LL Defl inch	0.049 (L/2357)	4'8 7/16"	0.322 (L/360)	0.150 (15%)	L	L
TL Defl inch	0.068 (L/1713)	4'8 7/16"	0.483 (L/240)	0.140 (14%)	D+L	L

Design Notes

- 1 Gir
- 2 Mul
- 3 Тор
- 4 Top

5 Bot

gn Notes	REFER TO MULTIPLE MEMBER TO MEMBER				
irders are designed to be supported on the bottom edge only.	CONNECTION DETAIL FOR PLY TO PLY				
ultiple plies must be fastened together as per manufacturer's details.	NAILING OR BOLTING REQUIREMENTS.				
op loads must be supported equally by all plies.	PASS THRU FRAMING SQUASH				
op flange must be laterally braced at a maximum of 8'7" o.c.	BLOCK IS REQUIRED AT ALL				
ottom flange braced at bearings.	POINT LOADS OVER BEARINGS.				



		•							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-9-6	(Span)3-5-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 10-3-9	(Span)1-6-1	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	1-8-2		Far Face	88 lb	233 lb	0 lb	0 lb	F6
4	Tie-In	1-9-6 to 10-3-9	(Span)1-1-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design critoria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the Intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

- Lifeting & Instantation
 Lioist langes must not be cut or drilled
 Refer to latest copy of the Lloist product information
 details for framing details, stiffener tables, web hole
 chart, bridging details, multi-ply fastening details and
 handling/erection details
 Damaged Lloists must not be used
 Design assumes top flange to be laterally restrained
 by attached sheathing or as specified in engineering
 notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stifferers for point load as shown Minimum point load bearing length=3.5 inches
 For flat roofs provide proper drainage to prevent appoints.

Nascor by Kott

Manufacturer Info

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA

USED IN THE DESIGN OF THIS COMPONENT.







Client: Project: Address: GREEN YORK HOMES

5/31/2018 Date:

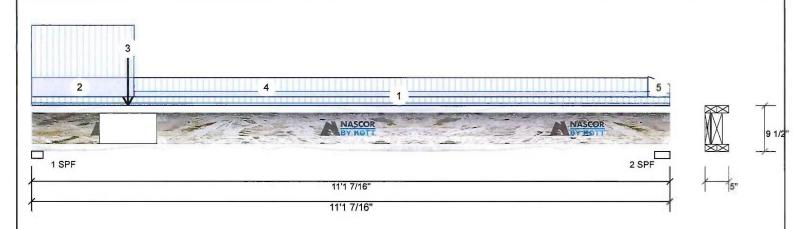
RCO Designer:

Job Name: LIANA 3 (ELEV.1)

Project #:

9.500" 2-Ply - PASSED F9-B NJH

Level: Ground Floor



Member Information **Unfactored Reactions UNPATTERNED Ib (Uplift)** Type: Girder Application: Floor (Residential) Live Dead Plies: 2 Design Method: 453 169 Moisture Condition: Dry **Building Code:** NBCC 2010 / OBC 2012 2 192 72 0 Deflection LL: Load Sharing: Deflection TL: 240 Deck: Not Checked Importance: Normal Vibration: Not Checked General Load Bearings and Factored Reactions Floor Live: 40 PSF Dead: 15 PSF Bearing Length Cap. React D/L lb Total Ld. Case 1 - SPF 2.375" 28% 212 / 679 891 2 - SPF 3.188" 12% 90 / 288 377 L

Analysis Results

ſ	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
ı	Moment	1293 ft-lb	3'10 13/16"	7660 ft-lb	0.169 (17%)	1.25D+1.5L	L
l	Unbraced	1293 ft-lb	3'10 13/16"	1300 ft-lb	0.994 (99%)	1.25D+1.5L	L
l	Shear	871 lb	1 5/8"	3160 lb	0.275 (28%)	1.25D+1.5L	L
ı	Perm Defl in.	0.017 (L/7653)	5'1 5/16"	0.359 (L/360)	0.050 (5%)	D	Uniform
ı	LL Defl inch	0.045 (L/2865)	5'1 1/4"	0.359 (L/360)	0.130 (13%)	L	L
l	TL Defl inch	0.062 (L/2084)	5'1 1/4"	0.539 (L/240)	0.120 (12%)	D+L	Ĺ

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 9'6" o.c.

5 Bottom flange braced at bearings.

		READ ALL NOTES ON THIS PAGE AND ON
	П	ENGINEERING NOTE PAGE ENP-2. THIS
		NOTE PAGE IS AN INTEGRAL PART OF THIS
		CALCULATION SUMMARY PAGE AS IT
		CONTAINS SPECIFICATIONS AND CRITERIA
		USED IN THE DESIGN OF THIS COMPONENT.
ı		

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARING



Wind

0

Ld. Comb.

1.25D+1.5L

1.25D+1.5L

		,-,							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 11-1-7	(Span)0-4-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 1-9-6	(Span)3-5-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	1-8-2		Near Face	97 lb	260 lb	0 lb	0 lb	F6
4	Tie-In	1-9-6 to 10-8-14	(Span)0-11-1	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
5	Tie-In	10-8-14 to 11-1-7	(Span) 0-11-14 to 0-7-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- Licinity on Installation

 Licinity langes must not be cut or drilled

 Refer to latest copy of the Licinity product information

 details for framing details, stiffener tables, web hole

 chart, bridging details, multiply fastening details and

 handling/erection details

 Damaged Licinitism must not be used

 Design assumes top flange to be laterally restrained

 by attached sheathing or as specified in engineering

 notes.
- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load earing lengths—3.5 inches
 For flat roofs provide proper drainage to prevent

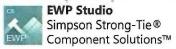
Manufacturer Info

Nascor by Kott









Client:

GREEN YORK HOMES

Project: Address:

5/31/2018

Designer: RCO

Job Name: LIANA 3 (ELEV.1)

Project #:

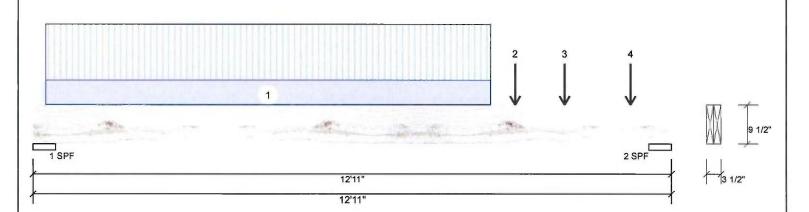
Date:

Forex 2.0E-3000Fb LVL BBO4-A

1.750" X 9.500"

2-Ply - PASSED

Level: Second Floor



nation			Unfacto	red Reac	tions U	INPATTERN	ED lb (Uplift)	
Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snov	v	Wind
2	Design Method:	LSD	1	1515		707	()	0
Dry	Building Code:	NBCC 2010 / OBC 2012	2	1536		676	(0	0
360	Load Sharing:	No							
240	Deck:	Not Checked							
Normal	Vibration:	Not Checked							
40 PSF			Bearings	and Fac	tored	Reactions			
15 PSF			Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
			1 - SPF	5.500"	27%	884 / 2273	3156	L	1.25D+1.5L
	4		2-SPF	5.500"	27%	845 / 2304	3148	L	1.25D+1.5L
	Girder 2 Dry 360 240 Normal	Girder 2 Design Method: Building Code: 360 Load Sharing: 240 Deck: Normal Vibration: 40 PSF 15 PSF	Girder Application: Floor (Residential) 2 Design Method: LSD Building Code: NBCC 2010 / OBC 2012 Load Sharing: No Deck: Not Checked Vibration: Not Checked	Application: Floor (Residential) Brg	Application: Floor (Residential) Brg Live	Application: Floor (Residential) Brg Live	Application: Floor (Residential) Brg Live Dead	Application: Floor (Residential) Brg Live Dead Snow	Application: Floor (Residential) Brg Live Dead Snow

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	9326 ft-lb	6'5 3/8"	22724 ft-lb	0.410 (41%)	1.25D+1.5L	L
Unbraced	9326 ft-lb	6'5 3/8"	19820 ft-lb	0.471 (47%)	1.25D+1.5L	L
Shear	2846 lb	11'8 3/4"	9277 lb	0.307 (31%)	1.25D+1.5L	L
Perm Defl in.	0.116 (L/1250)	6'5 5/16"	0.404 (L/360)	0.290 (29%)	D	Uniform
LL Defl inch	0.253 (L/574)	6'5 1/2"	0.404 (L/360)	0.630 (63%)	L	L
TL Defl inch	0.370 (L/393)	6'5 1/2"	0.606 (L/240)	0.610 (61%)	D+L	L

Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.

7 Lateral slenderness ratio based on full section width.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



/ Lateral Sienderness ratio based on full section width.										
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments	
1	Part. Uniform	0-3-1 to 9-3-1		Тор	107 PLF	244 PLF	0 PLF	0 PLF		
2	Point	9-9-1		Тор	92 lb	244 lb	dl 0	0 lb	J8	
3	Point	10-9-1		Тор	107 lb	285 lb	0 lb	0 lb	J8	
4	Point	12-1-1		Тор	122 lb	326 lb	0 lb	0 lb	J8	
	Self Weight				8 PLF					

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info







EWP Studio Simpson Strong-Tie® Component Solutions™

Client: Project: Address: **GREEN YORK HOMES**

5/31/2018 Date:

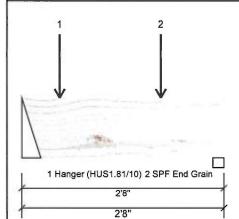
Designer: RCO

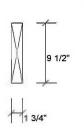
Job Name: LIANA 3 (ELEV.1)

Project #:

F10-A Forex 2.0E-3000Fb LVL 1.750" X 9.500" - PASSED

Level: Second Floor





Wind

wember	intormation
Type:	Girder

Piles:	7
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	
Floor Live:	40 PSF
Dead:	15 PSF

Application: Design Method:

Building Code:

Load Sharing:

Deck:

Floor (Residential) NBCC 2010 / OBC 2012

No

Not Checked Not Checked Vibration:

Unfactored Reactions UNPATTERNED Ib (Uplift)

2	139	57	0	0

Dead

Analysis Results

Ī	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case	
	Moment	208 ft-lb	1'10"	11362 ft-lb	0.018 (2%)	1.25D+1.5L	L	
	Unbraced	208 ft-lb	1'10"	10456 ft-lb	0.020 (2%)	1.25D+1.5L	L	
	Shear	344 lb	11 3/4"	4638 lb	0.074 (7%)	1.25D+1.5L	L	
	Perm Defl in.	0.000 (L/68055)	1'9 1/8"	0.080 (L/360)	0.010 (1%)	D	Uniform	
	LL Defl inch	0.001 (L/27103)	1'9 7/8"	0.080 (L/360)	0.010 (1%)	L	L	
	TL Defl inch	0.001 (L/19389)	1'9 11/16"	0.120 (L/240)	0.010 (1%)	D+L	L	

Bearings and Factored Reactions

Live

174

1

_							
Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.	
1 - Hanger	3.000"	9%	88 / 261	349	L	1.25D+1.5L	
2 - SPF End Grain	1.750"	12%	71 / 209	280	L	1.25D+1.5L	

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.
- 4 Bottom braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Point	0-6-0		Far Face	51 lb	136 lb	0 lb	0 lb	J2
2	Point	1-10-0		Far Face	66 lb	177 lb	0 lb	0 lb	J2
	Self Weight				4 PLF				

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- chemicals

Handling & Installation

- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- approvals

 Damaged Beams must not be used

 Design assumes top edge is laterally restrained

 Provide lateral support at bearing points to avoid

 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent

Manufacturer Info

APA: PR-L318







Client: Project: Address: GREEN YORK HOMES

5/31/2018

RCO Designer:

Job Name: LIANA 3 (ELEV.1)

Project #:

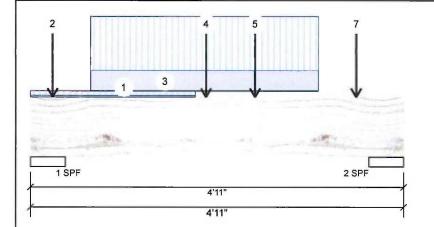
Date:

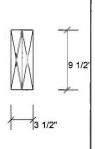
Forex 2.0E-3000Fb LVL

1.750" X 9.500"

2-Ply - PASSED

Level: Second Floor





Mind

Page 1 of 2

Member Information

Туре:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	
Floor Live:	40 PSF
Dead:	15 PSF

Application: Design Method:

Floor (Residential)

Building Code: NBCC 2010 / OBC 2012

Load Sharing: No

Deck: Not Checked Vibration: Not Checked

Unfactored Reactions UNPATTERNED Ib (Uplift)

Dig	LIVC	Dead	CHOW	4 411 ICI
1	1661	666	0	0
2	1662	664	0	0

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1-SPF	5.500"	28%	833 / 2492	3325	L	1.25D+1.5L
2 - SPF	5.500"	28%	830 / 2493	3324	L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	4701 ft-lb	2'3 3/4"	22724 ft-lb	0.207 (21%)	1.25D+1.5L	L
Unbraced	4701 ft-lb	2'3 3/4"	22724 ft-lb	0.207 (21%)	1.25D+1.5L	L
Shear	3520 lb	3'8 3/4"	9277 lb	0.379 (38%)	1.25D+1.5L	L
Perm Defl in.	0.008 (L/5989)	2'3 3/4"	0.138 (L/360)	0.060 (6%)	D	Uniform
LL Defl inch	0.021 (L/2406)	2'3 3/4"	0.138 (L/360)	0.150 (15%)	L	L
TL Defl inch	0.029 (L/1716)	2'3 3/4"	0.206 (L/240)	0.140 (14%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on full section width.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 2-2-0	(Span)1-3-9	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	0-3-8		Far Face	79 lb	211 lb	0 lb	0 lb	J6
3	Part. Uniform	0-9-8 to 3-9-8		Far Face	119 PLF	316 PLF	0 PLF	0 PLF	
4	Point	2-3-12		Near Face	616 lb	1523 lb	0 lb	0 lb	F3
5	Point	2-11-8		Near Face	51 lb	136 lb	0 lb	0 lb	J2
6	Point	4-3-8		Far Face	108 lb	287 lb	0 lb	0 lb	J6

Continued on page 2...

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design cnteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code fastening ordiner, so-many approvals approvals Damaged Beams must not be used Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent

Manufacturer Info

APA: PR-L318







Page 2 of 2

EWP Studio Simpson Strong-Tie® Component Solutions™

Project:

Address:

GREEN YORK HOMES

Date:

5/31/2018 RCO Designer:

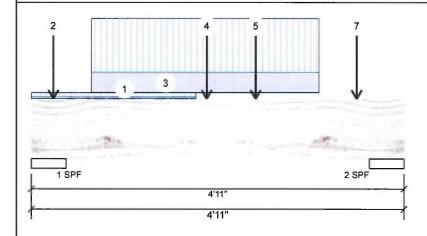
Job Name: LIANA 3 (ELEV.1)

Project #:

F11-A Forex 2.0E-3000Fb LVL 1.750" X 9.500"

2-Ply - PASSED

Level: Second Floor



.Continued from page 1

ID Load Type 7 Point

Location Trib Width 4-3-8

Side Near Face Dead 61 lb

Live 162 lb Snow 0 lb Wind Comments 0 lb J2

Self Weight 8 PLF

> READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARING



Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- approvals

 Damaged Beams must not be used

 Design assumes top edge is laterally restrained

 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent

Manufacturer Info

Forex APA: PR-L318







Member Information

Page 1 of 1



Client: Project:

Address:

GREEN YORK HOMES

5/31/2018

Designer: RCO

Job Name: LIANA 3 (ELEV.1)

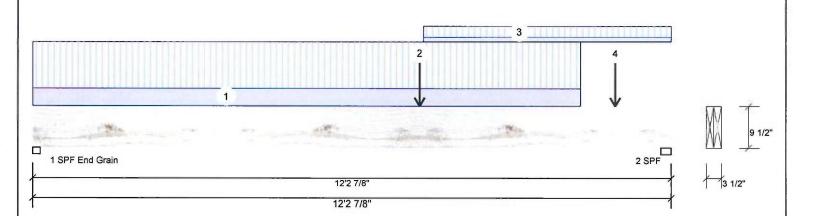
Project #:

Date:

F12-A Forex 2.0E-3000Fb LVL 1.750" X 9.500"

2-Ply - PASSED

Level: Second Floor



Туре:	Girder	Application:	Floor (Residential)	Brg	Live	
Plies:	2	Design Method:	LSD	1	730	
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012	2	894	
Deflection LL:	360	Load Sharing:	No	1 -		
Deflection TL:	240	Deck:	Not Checked			
Importance:	Normal	Vibration:	Not Checked			
General Load						
Floor Live:	40 PSF			Bearings	and Fac	tore
Dead:	15 PSF	1		Bearing	Length	C
				1 - SPF	1.750"	3

Analysis Results										
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case				
Moment	6660 ft-lb	7'4 15/16"	22724 ft-lb	0.293 (29%)	1.25D+1.5L	L				
Unbraced	6660 ft-lb	7'4 15/16"	19870 ft-lb	0.335 (34%)	1.25D+1.5L	L				
Shear	1898 lb	11'3 3/4"	9277 lb	0.205 (20%)	1.25D+1.5L	L				
Perm Defl in.	0.070 (L/2071)	6'4 7/16"	0.401 (L/360)	0.170 (17%)	D	Uniform				
LL Defl inch	0.163 (L/883)	6'4 3/4"	0.401 (L/360)	0.410 (41%)	L	L				

6'4 5/8" 0.601 (L/240) 0.390 (39%) D+L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.

TL Defl inch 0.233 (L/619)

6 Lateral slenderness ratio based on full section width.

Jnfactored	Reactions	UNPAT	TERNED	lb	(Uplift)	

DIG	Live	Dead	Show	VVIIIO
1	730	322	0	0
2	894	386	0	0

Cnow

Dood

ed Reactions

ı								
	Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.	
	1 - SPF End Grain	1.750"	33%	403 / 1095	1497	L	1.25D+1.5L	
ı	2 CDE	2 375"	36%	482 / 1340	1822	1	1 25D+1 5I	

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Mind

o Editorial Storiation octobrotham Social Walter										
I	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
I	1	Part. Uniform	0-0-0 to 10-6-0		Far Face	28 PLF	75 PLF	0 PLF	0 PLF	
I	2	Point	7-4-15		Near Face	255 lb	661 lb	0 lb	0 lb	F1
I	3	Tie-In	7-5-13 to 12-2-14	(Span)0-10-9	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
I	4	Point	11-2-0		Far Face	34 lb	91 lb	0 lb	0 lb	J9
I		Self Weight				8 PLF				

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

LVL beams must not be out or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code

approvals

Damaged Beams must not be used

Design assumes top edge is laterally restrained

Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex APA: PR-L318





Client: Project:

Address:

GREEN YORK HOMES

Date: 5/31/2018

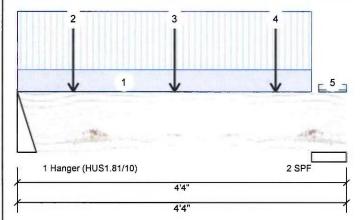
Designer: RCO

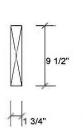
Job Name: LIANA 3 (ELEV.1)

Project #

1.750" X 9.500" - PASSED Forex 2.0E-3000Fb LVL F1-A

Level: Second Floor





Wind

AOFESSION

Member Information							
Туре:	Girder						
Plies:	1						
Moisture Condition:	Dry						
Deflection LL:	360						
Deflection TL:	240						
Importance:	Normal						

40 PSF

15 PSF

Application: Design Method: **Building Code:** Load Sharing:

Deck:

Vibration:

Floor (Residential) NBCC 2010 / OBC 2012 No

Not Checked Not Checked

Unfactored	Reactions	UNPATTERNED	lb (Uplift)
Bra	Live	Dead	Snow

3	-10-			
1	661	255	0	0
2	604	235	0	0

Bearings and Factored Reactions

I	Bearing	Length	Cap. F	React D/L lb	Total	Ld. Case	Ld. Comb.	
	1 - Hanger	3.000"	34%	319 / 992	1311	L	1.25D+1.5L	
1	•							
ı	2 - SPF	5 500"	20%	294 / 907	1200	1	1 25D+1 5L	

Analysis Results

General Load

Floor Live: Dead:

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1179 ft-lb	2' 13/16"	11362 ft-lb	0.104 (10%)	1.25D+1.5L	L
Unbraced	1179 ft-lb	2' 13/16"	9138 ft-lb	0.129 (13%)	1.25D+1.5L	L
Shear	844 lb	11 3/4"	4638 lb	0.182 (18%)	1.25D+1.5L	L
Perm Defl in.	0.004 (L/11669)	2' 7/8"	0.125 (L/360)	0.030 (3%)	D	Uniform
LL Defl inch	0.010 (L/4503)	2' 7/8"	0.125 (L/360)	0.080 (8%)	L	L
TL Defl inch	0.014 (L/3249)	2' 7/8"	0.188 (L/240)	0.070 (7%)	D+L	L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY

NAILING OR BOLTING REQUIREM	EN'
PASS THRU FRAMING SQUASH	
BLOCK IS REQUIRED AT ALL	
DON'T LOADS OVER BEADINGS	

	Ji
Comments	J
J7	
J7	
J7	

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.

4 Bottom	4 Bottom braced at bearings.					POINT LOAI	DS OVER BE	ARINGS.	
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Part, Uniform	0-0-0 to 3-10-8		Тор	90 PLF	240 PLF	0 PLF	0 PLF	
2	Point	0-8-13		Far Face	39 lb	105 lb	0 lb	0 lb	J7
3	Point	2-0-13		Far Face	47 lb	126 lb	0 lb	0 lb	J7
4	Point	3-4-13		Far Face	36 lb	96 lb	0 lb	0 lb	J7
5	Tie-In	3-11-10 to 4-4-0	(Span)1-1-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	Self Weight				4 PLF				



Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- approves
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info







Client: Project: Address: **GREEN YORK HOMES**

Date: 5/31/2018

Designer: RCO

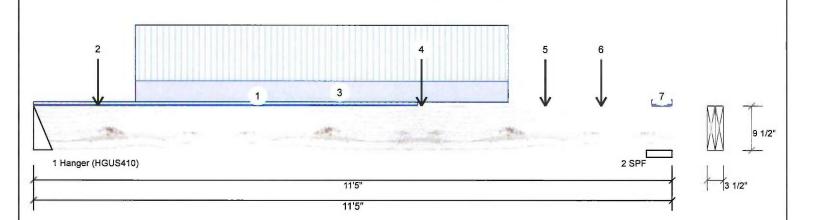
Job Name: LIANA 3 (ELEV.1)

Project #:

F3-A Forex 2.0E-3000Fb LVL 1.750" X 9.500"

2-Ply - PASSED

Level: Second Floor



Type.	Gildei	Application.	Floor (Residential)	pry	Live		Dead	2110	N	vvina
Plies:	2	Design Method:	LSD	1	1523		616		0	0
Moisture Conditio	n: Dry	Building Code:	NBCC 2010 / OBC 2012	2	1514		622		0	0
Deflection LL:	360	Load Sharing:	No							
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal	Vibration:	Not Checked							
General Load										
Floor Live:	40 PSF			Bearings	s and Fac	tored I	Reactions			
Dead:	15 PSF			Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
				1 -	4.000"	29%	770 / 2284	3055	L	1.25D+1.5L
				Hanger						

Case

2 - SPF 5.500"

Floor (Residential)

Analysis	Actual	Location	Allowed	Capacity	Comb.	Ca
Moment	8863 ft-lb	5'10"	22724 ft-lb	0.390 (39%)	1.25D+1.5L	L
Unbraced	8863 ft-lb	5'10"	20442 ft-lb	0.434 (43%)	1.25D+1.5L	L
Shear	3021 lb	10'2 3/4"	9277 lb	0.326 (33%)	1.25D+1.5L	L

L 5'8 3/16" 0.358 (L/360) 0.220 (22%) D Perm Defl in. 0.080 (L/1615) Uniform LL Defl inch 0.198 (L/650) 5'8 1/8" 0.358 (L/360) 0.550 (55%) L L TL Defl inch 0.278 (L/464) 5'8 1/8" 0.538 (L/240) 0.520 (52%) D+L L

Application:

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Unfactored Reactions UNPATTERNED Ib (Uplift)

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



3048 L

777 / 2271

Mind

1.25D+1.5L

Page 1 of 2

Design Notes

Analysis Results

1 Fill all hanger nailing holes.

Member Information

- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on full section width.

· Latera	Cicilacilloco Iddo Dacoa	on ran occaon mach.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 6-10-6	(Span)0-7-12	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	1-1-13		Near Face	129 lb	344 lb	0 lb	0 lb	J5
3	Part. Uniform	1-9-13 to 8-5-13		Near Face	103 PLF	275 PLF	0 PLF	0 PLF	
4	Point	6-11-4		Far Face	70 lb	174 lb	0 lb	0 lb	F10
5	Point	9-1-13		Near Face	120 lb	321 lb	0 lb	016	J5
6	Point	10-1-13		Near Face	109 lb	268 lb	0 lb	0 lb	J5

Continued on page 2...

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding









Page 2 of 2

EWP Studio Simpson Strong-Tie® Component Solutions™ Client:

GREEN YORK HOMES

Project: Address: Date: 5/31/2018

RÇO Designer:

Job Name: LIANA 3 (ELEV.1)

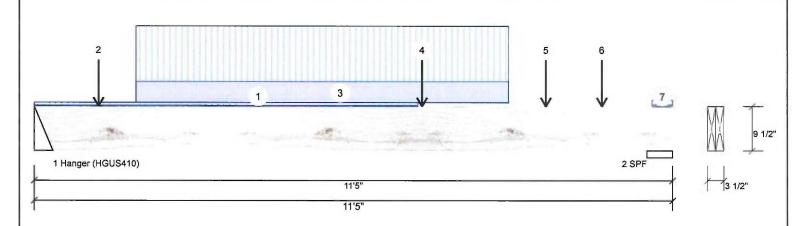
Project #:

Forex 2.0E-3000Fb LVL F3-A

1.750" X 9.500"

2-Ply - PASSED

Level: Second Floor



..Continued from page 1

ID Load Type 7 Tie-In

Self Weight

Location Trib Width Side 11-0-10 to 11-5-0 (Span)1-1-3 Top

Dead 15 PSF

8 PLF

Live 40 PSF

Snow 0 PSF Wind Comments

0 PSF

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component sultability of the intended application, and to verify the dimensions and loads.

Lumber

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- chemicals Handling & Installation
- LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code

- approvals

 Damaged Beams must not be used

 Damaged Beams must not be used

 Design assumes top edge is laterally restrained

 Provide lateral support at bearing points to avoid

 lateral displacement and rotation

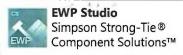
For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex







Client:

GREEN YORK HOMES

Project: Address:

5/31/2018 Date:

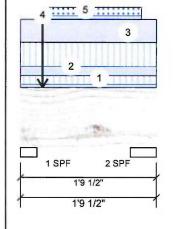
Designer: RCO

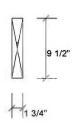
Job Name: LIANA 3 (ELEV.2)

Project #:

1.750" X 9.500" - PASSED Forex 2.0E-3000Fb LVL

Level: Second Floor





Member Inform	nation			Unfactored Reactions UNPATTERNED Ib (Uplift)							
Туре:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snov	N	Wind	
Plies:	1	Design Method:	LSD	1	78		255	32	3	0	
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012	2	90		124	5	3	0	
Deflection LL:	360	Load Sharing:	No	100							
Deflection TL:	240	Deck:	Not Checked								
Importance:	Normal	Vibration:	Not Checked								
General Load											
Floor Live:	40 PSF			Bearing:	s and Fac	tored	Reactions				
Dead:	15 PSF			Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.	
				1 - SPF	2.625"	33%	319 / 524	843	L	1.25D+1.5S +0.5L	
Analysis Results					4.125"	8%	156 / 135	290	L	1.25D+1.5L	

Location Allowed Analysis Actual Capacity Comb. Moment 114 ft-lb 6" 11362 ft-lb 0.010 (1%) 1.25D+1.5S L +0.5L Unbraced 114 ft-lb 6" 11362 ft-lb 0.010 (1%) 1.25D+1.5S L +0.5L 8 5/8" 4638 lb Shear 54 lb 0.012 (1%) 1.25D+1.5S L Perm Defl in. 0.000 8 7/16" 0.045 (L/360) 0.010 (1%) D Uniform (L/49777) LL Defl inch 0.000 6 3/16" 0.045 (L/360) 0.010 (1%) S+0.5L (L/43590) TL Defl inch 0.001 7 1/2" 0.068 (L/240) 0.010 (1%) D+S+0.5L (L/23529)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.

4 Bottom br	aced at bearings.								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-9-8	(Span)1-2-9	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 1-9-8	(Span)3-5-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Part. Uniform	0-0-0 to 1-9-8		Тор	64 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
4	Point	0-3-8		Тор	182 lb	0 lb	345 lb	0 lb	F14 F14
5	Part. Uniform	0-3-10 to 1-7-2		Тор	10 PLF	0 PLF	24 PLF	0 PLF	
	Self Weight				4 PLF				

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals Damaged Beams must not be used
- Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding



Manufacturer Info

Kott Lumber Company 14 Anderson Blvd, Ontario Canada 14A7X4





EWP Studio Simpson Strong-Tie® Component Solutions™

GREEN YORK HOMES

Project: Address:

5/31/2018

RCO Designer:

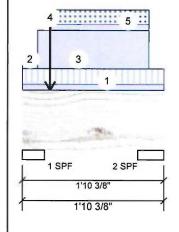
Job Name: LIANA 3 (ELEV.2)

Project #:

Date:

F4-B Forex 2.0E-3000Fb LVL

1.750" X 9.500" - PASSED Level: Second Floor



Member Info	rmation			Unfacto	red Reac	tions U	INPATTERN	ED Ib (Uplift)	
Type:	Girder	Application:	Floor (Residential)	Brg	Live	1	Dead	Snow	v .	Wind
Plies:	1	Design Method:	LSD	1	24		231	323	3	0
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	25		87	53	3	0
Deflection LL:	360	Load Sharing:	No							
Deflection TL:	240	Deck:	Not Checked	1						
Importance:	Normal	Vibration:	Not Checked							
General Load										
Floor Live:	40 PSF			Bearing:	s and Fac	tored	Reactions			
Dead:	15 PSF			Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.
				1 - SPF	3.500"	25%	289 / 497	786	L	1.25D+1.5S +0.5L
Analysis Resu	lts			2 - SPF	4.125"	5%	109 / 38	147	L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	104 ft-lb	5 1/4"	11362 ft-lb	0.009 (1%)	1.25D+1.5S +0.5L	L
Unbraced	104 ft-lb	5 1/4"	11362 ft-lb	0.009 (1%)	1.25D+1.5S +0.5L	L
Shear	57 lb	9 1/2"	4638 lb	0.012 (1%)	1.25D+1.5S	L
Perm Defl in.	0.000 (L/57884)	9"	0.045 (L/360)	0.010 (1%)	D	Uniform
LL Defl inch	0.000 (L/49057)	5 1/16"	0.045 (L/360)	0.010 (1%)	S+0.5L	L
TL Defl inch	0.001 (L/27348)	7 3/8"	0.068 (L/240)	0.010 (1%)	D+S+0.5L	L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARING



Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.

4 5000111	braceu at bearings.								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-10-6	(Span)1-3-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Part. Uniform	0-0-0 to 0-2-6		Тор	21 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
3	Part. Uniform	0-2-6 to 1-8-0		Тор	64 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
4	Point	0-4-6		Тор	182 lb	0 lb	345 lb	0 lb	F14 F14
5	Part. Uniform	0-4-8 to 1-8-0		Тор	10 PLF	0 PLF	24 PLF	0 PLF	
	Self Weight				4 PLF				

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- andling & installation

 LVL beams must not be out or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
 approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

Manufacturer Info For flat roofs provide proper drainage to prevent ponding





