Engineering Note Page (ENP-2)

REVISION 2009-10-09

Please read all notes prior to installation of the component

DESIGN INFORMATION

This building component is certified as an individual component for the loads and conditions shown on the calculation and drawing page.

The responsibility of the undersigned engineer is <u>only</u> limited to the calculation of this building component for the loads and conditions shown on this drawing.

The responsibility of the undersigned is limited to the verification of the structural capacity of the NASCOR floor joists and LVL beams based on placement as shown on the layout. The loads applied are limited to the gravity effects of the specified loads. The structural integrity of the building and the effect of wind, uplift, seismic, lateral or other forces, calculation of adequate support and anchorage of components, as well as the dimensions and design loads used to calculate components are the responsibility of the overall building designer.

Floor joists and OSB rim board are designed to carry uniformly distributed loads only. Point loads should be transferred through the floor cavity with squash blocks. Structural elements such as walls, posts, connectors, and squash blocks are the responsibility of the overall building designer.

The undersigned engineer disclaims any responsibility for damages as a result of being furnished faulty or incorrect information, specifications and/or designs.

Installation of NASCOR joists is to be carried out in accordance with the current edition of the manufacturer's approved literature available at http://www.nascor.ca.

CODE

This building component is designed in accordance with the National Building Code of Canada, the Ontario Building Code, CCMC and Canadian Standards Association guidelines.

COMPONENT

- 1. The building component used in construction must be the same as indicated on the drawings.
- 2. The building component must be installed and assembled as per specification shown on the drawing and in accordance with the manufacturer's assembly and installation.
- 3. Members consisting of multiple plies must be connected as per the document "Multi-ply Connection Details".
- 4. Pass-thru squash block framing is required at all point loads over bearings.

HANDLING AND INSTALLATION

Do not drill any hole, cut or notch a certified building component without a written preauthorization.



Client: Project: **GREEN YORK HOMES**

Address:

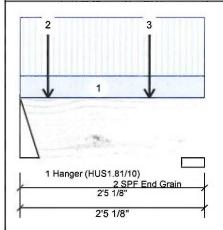
Date:

5/31/2018 Designer: **RCO**

Job Name: LIANA 2 (ELEV.1)

Project #:

Level: Ground Floor Forex 2.0E-3000Fb LVL 1.750" X 9.500" - PASSED



Manual and Indianasia Com-

Page 1 of 1

lember Infor	mation		
Type:	Girder	Application:	Floor (Residential)
Plies:	1	Design Method:	LSD
Moisture Conditio	n: Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

Unfacto	red Reactions	SUNPATTER	NED lb (Uplif	t)
Brg	Live	Dead	Snow	Wind
1	299	116	0	0
2	258	101	0	0

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	214 ft-lb	1'6 7/8"	11362 ft-lb	0.019 (2%)	1.25D+1.5L	L
Unbraced	214 ft-lb	1'6 7/8"	10729 ft-lb	0.020 (2%)	1.25D+1.5L	L
Shear	438 lb	11 3/4"	4638 lb	0.094 (9%)	1.25D+1.5L	L
Perm Defl in.	0.000 (L/58713)	1'4 3/8"	0.067 (L/360)	0.010 (1%)	D	Uniforn
LL Defl inch	0.001 (L/23005)	1'4 9/16"	0.067 (L/360)	0.020 (2%)	L	L
TL Defl inch	0,001 (L/16529)	1'4 7/16"	0.100 (L/240)	0.010 (1%)	D+L	L

Bearings and Factored Reactions

7								
	Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.	
	1 - Hanger	3.000"	15%	145 / 449	594	L	1.25D+1.5L	
	2 - SPF End Grain	3.625"	11%	127 / 387	514	L	1.25D+1.5L	

Design Notes

1 Fill all hanger nailing holes.

- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.

4 Bottom braced at bearings.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS



ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 2-5-2	(Span)3-11-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	0-4-6		Far Face	65 lb	175 lb	0 lb	0 lb	J3
3	Point	1-8-6		Far Face	72 lb	192 lb	0 lb	0 lb	J3
	Self Weight				4 PLF				

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

LVI beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code 1.

4. 5.

tasterning details, oearn strengin values, and code approvals

Damaged Beams must not be used

Design assumes top edge is laterally restrained

Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex APA: PR-L318







Client:

GREEN YORK HOMES

Project: Address:

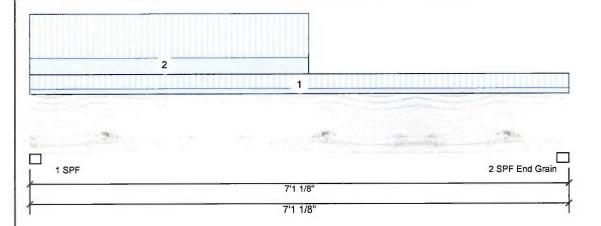
5/31/2018 Date:

RCO Designer:

Job Name: LIANA 2 (ELEV.1)

Project #:

Level: Ground Floor 1.750" X 9.500" - PASSED Forex 2.0E-3000Fb LVL F2-A





Page 1 of 1

Member	Information
--------	-------------

Type:	Girder
Plies:	1
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	

40 PSF

15 PSF

Application: Floor (Residential) Design Method: NBCC 2010 / OBC 2012 **Building Code:** Load Sharing: No Not Checked Deck:

Not Checked

Brg	Live	Dead	Snow	Wind
1	156	72	0	0
2	84	45	0	0

Analysis Results

Floor Live:

Dead:

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	453 ft-lb	2'11 9/16"	11362 ft-lb	0.040 (4%)	1.25D+1.5L	L
Unbraced	453 ft-lb	2'11 9/16"	5389 ft-lb	0.084 (8%)	1,25D+1.5L	L
Shear	228 lb	10 1/2"	4638 lb	0.049 (5%)	1.25D+1.5L	L
Perm Defl in.	0.004 (L/20045)	3'4 1/8"	0.231 (L/360)	0.020 (2%)	D	Uniform
LL Defl inch	0.009 (L/9718)	3'3 1/2"	0.231 (L/360)	0.040 (4%)	L	L
TL Defl inch	0.013 (L/6545)	3'3 11/16"	0.346 (L/240)	0.040 (4%)	D+L	L

Vibration:

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	1.750"	17%	90 / 234	324	L	1.25D+1.5L
2 - SPF End	1.875"	7%	56 / 125	182	L	1.25D+1.5L

Grain

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Top braced at bearings.
- 3 Bottom braced at bearings

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 7-1-2	(Span)0-7-14	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Part. Uniform	0-0-0 to 3-8-0		Тор	15 PLF	40 PLF	0 PLF	0 PLF	
	Self Weight				4 PLF				

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- featuring approvals

 Damaged Beams must not be used

 Design assumes top edge is laterally restrained

 Provide lateral support at bearing points to avoid lateral displacement and rotation
- For flat roofs provide proper drainage to prevent pending

Manufacturer Info

Forex APA: PR-L318









Client: Project:

Address:

GREEN YORK HOMES

5/31/2018

RCO Designer:

Job Name: LIANA 2 (ELEV.1)

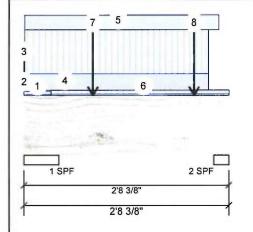
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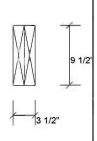
Date:

F3-A Forex 2.0E-3000Fb LVL 1.750" X 9.500"

2-Ply - PASSED

Level: Ground Floor





Page 1 of 2

Member Infor	mation			Unfactore	d React	ions U	NPATTERNI	ED lb (Uplift)	
Туре:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snov	٧	Wind
Plies:	2	Design Method:	LSD	1	669		380		ס	0
Moisture Conditio	n: Dry	Building Code:	NBCC 2010 / OBC 2012	2	523		292)	0
Deflection LL:	360	Load Sharing:	No							
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal	Vibration:	Not Checked							
General Load							_			
Floor Live:	40 PSF			Bearings a	ind Fact	tored R	eactions			
Dead:	15 PSF			Bearing Lo	ength	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
				1 - SPF 5.	.500"	12%	475 / 1003	1478	L	1.25D+1.5L
				2 - SPF 2.	.375"	22%	365 / 784	1150	L	1.25D+1.5L
D	4							_		

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	607 ft-lb	1'4 1/8"	22724 ft-lb	0.027 (3%)	1.25D+1.5L	L
Unbraced	607 ft-lb	1'4 1/8"	22724 ft-lb	0.027 (3%)	1.25D+1.5L	L
Shear	729 lb	1'2 1/4"	9277 lb	0.079 (8%)	1.25D+1.5L	L
Perm Defl in.	0.001 (L/32357)	1'5 1/8"	0.072 (L/360)	0.010 (1%)	D	Uniform
LL Defl inch	0.001 (L/18072)	1'4 11/16"	0.072 (L/360)	0.020 (2%)	L	L
TL Defl inch	0.002 (L/11598)	1'4 13/16"	0.108 (L/240)	0.020 (2%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- Lateral slanderness ratio based on full section width

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



	if full section width.							
Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
Tie-In	0-0-0 to 0-4-4	(Span)0-10-7	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
Part. Uniform	0-0-0 to 0-0-2		Тор	46 PLF	123 PLF	0 PLF	0 PLF	J5
Part. Uniform	0-0-0 to 0-0-2		Тор	40 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
Part. Uniform	0-0-2 to 2-5-2		Тор	92 PLF	246 PLF	0 PLF	0 PLF	J5
Part. Uniform	0-0-2 to 2-6-12		Тор	80 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
	Tie-In Part. Uniform Part. Uniform Part. Uniform	Tie-In 0-0-0 to 0-4-4 Part. Uniform 0-0-0 to 0-0-2 Part. Uniform 0-0-0 to 0-0-2 Part. Uniform 0-0-2 to 2-5-2	Tie-In 0-0-0 to 0-4-4 (Span)0-10-7 Part. Uniform 0-0-0 to 0-0-2 Part. Uniform 0-0-0 to 0-0-2 Part. Uniform 0-0-2 to 2-5-2	Tie-In 0-0-0 to 0-4-4 (Span)0-10-7 Top Part. Uniform 0-0-0 to 0-0-2 Top Part. Uniform 0-0-0 to 0-0-2 Top Part. Uniform 0-0-2 to 2-5-2 Top	Tie-In 0-0-0 to 0-4-4 (Span)0-10-7 Top 15 PSF Part. Uniform 0-0-0 to 0-0-2 Top 46 PLF Part. Uniform 0-0-0 to 0-0-2 Top 40 PLF Part. Uniform 0-0-2 to 2-5-2 Top 92 PLF	Tie-In 0-0-0 to 0-4-4 (Span)0-10-7 Top 15 PSF 40 PSF Part. Uniform 0-0-0 to 0-0-2 Top 46 PLF 123 PLF Part. Uniform 0-0-0 to 0-0-2 Top 40 PLF 0 PLF Part. Uniform 0-0-2 to 2-5-2 Top 92 PLF 246 PLF	Tie-In 0-0-0 to 0-4-4 (Span)0-10-7 Top 15 PSF 40 PSF 0 PSF Part. Uniform 0-0-0 to 0-0-2 Top 46 PLF 123 PLF 0 PLF Part. Uniform 0-0-0 to 0-0-2 Top 40 PLF 0 PLF 0 PLF Part. Uniform 0-0-2 to 2-5-2 Top 92 PLF 246 PLF 0 PLF	Tie-In 0-0-0 to 0-4-4 (Span)0-10-7 Top 15 PSF 40 PSF 0 PSF 0 PSF Part. Uniform 0-0-0 to 0-0-2 Top 46 PLF 123 PLF 0 PLF 0 PLF Part. Uniform 0-0-0 to 0-0-2 Top 40 PLF 0 PLF 0 PLF 0 PLF Part. Uniform 0-0-2 to 2-5-2 Top 92 PLF 246 PLF 0 PLF 0 PLF

Continued on page 2...

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
 approvals
- approvals
 Damaged Seams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318







Client: Project: Address: **GREEN YORK HOMES**

5/31/2018 Date:

Designer: RCO

Job Name: LIANA 2 (ELEV.1)

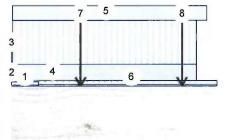
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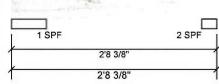
Forex 2.0E-3000Fb LVL

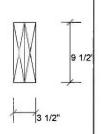
1.750" X 9.500"

2-Ply - PASSED

Level: Ground Floor







Page 2 of 2

.Continued from page 1 Location Trib Width ID Load Type Side Dead Live Snow Wind Comments 6 Tie-In 0-4-4 to 2-8-6 (Span)1-0-15 Top 15 PSF 40 PSF 0 PSF 0 PSF Point 0-10-14 Far Face 120 lb 319 lb 0 lb 0 lb J5 7 8 2-2-14 Far Face 83 lb 220 lb 0 lb 0 lb J5 Point Self Weight 8 PLF

> REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- chemicals
- Handling & Installation
 - LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
 - tessening source, approvals approvals approvals Damaged Beams must not be used Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex APA: PR-L318









Client:

Address:

GREEN YORK HOMES

Project:

5/31/2018 Date:

Designer: RCO

Job Name: LIANA 2 (ELEV.1)

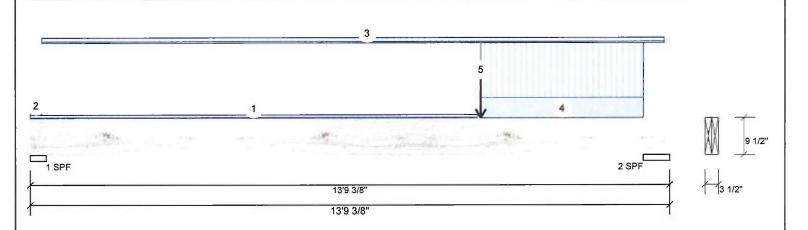
Project #:

Forex 2.0E-3000Fb LVL

1.750" X 9.500"

2-Ply - PASSED

Level: Ground Floor



Member Infor	mation			Unfacto	red Reac	tions U	NPATTERNI	ED lb (U	plift)	
Туре:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snow		Wind
Plies:	2	Design Method:	LSD	1	374		193	0		0
Moisture Condition	i: Dry	Building Code:	NBCC 2010 / OBC 2012	2	1087		464	0		0
Deflection LL:	360	Load Sharing:	No							
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal	Vibration:	Not Checked							
General Load										
Floor Live:	40 PSF			Bearing:	s and Fac	tored I	Reactions			
Dead:	15 PSF			Bearing	Length	Cap.	React D/L lb	Total I	.d. Case	Ld. Comb.
				1 - SPF	4.125"	9%	241 / 560	801 L	_	1.25D+1.5L
				2 - SPF	6.875"	15%	580 / 1631	2211 L		1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	4654 ft-lb	9'8 1/2"	22724 ft-lb	0.205 (20%)	1.25D+1.5L	L
Unbraced	4654 ft-lb	9'8 1/2"	19392 ft-lb	0.240 (24%)	1.25D+1.5L	L
Shear	1816 lb	12'5 3/4"	9277 lb	0.196 (20%)	1.25D+1.5L	L
Perm Defl in.	0.058 (L/2669)	7'3 7/8"	0.433 (L/360)	0.130 (13%)	D	Uniform
LL Defl inch	0.127 (L/1225)	7'5 1/4"	0.433 (L/360)	0.290 (29%)	L	L
TL Defl inch	0.186 (L/840)	7'4 13/16"	0.649 (L/240)	0.290 (29%)	D+L	L

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on full section width.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



I	6 Lateral siende	rness ratio based on	full section width.							
I	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
l	1	Tie-In	0-0-0 to 9-7-10	(Ѕрап)0-6-2	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
ı	2	Tie-In	0-0-0 to 0-3-0	(Span)0-7-6	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	3	Tie-In	0-3-0 to 13-7-15	(Span)0-9-14	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
ı	4	Part. Uniform	9-8-8 to 13-2-8		Тор	90 PLF	240 PLF	0 PLF	0 PLF	
	5	Point	9-8-8		Far Face	116 lb	299 lb	0 lb	0 lb	F1
ı		Self Weight				8 PLF				

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to resure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

LVL beams must not be cut or drilled
Refer to manufacturer's product information
regarding installation requirements, multi-ply
fastening details, beam strength values, and code

approvals

Damaged Beams must not be used
Design assumes top edge is laterally restrained
Provide lateral support at bearing points to
lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex APA: PR-L318









Client:

Address:

GREEN YORK HOMES Project:

5/31/2018 Date:

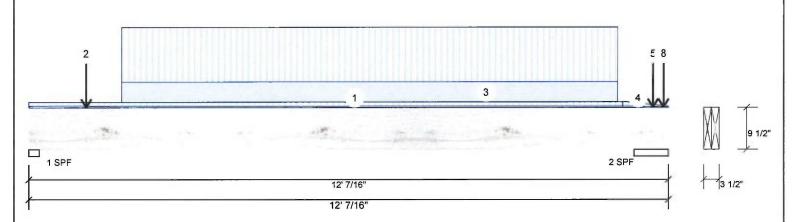
Designer: RCO LIANA 2 (ELEV.1) Job Name:

Project #:

F4-D Forex 2.0E-3000Fb LVL 1.750" X 9.500"

2-Ply - PASSED

Level: Ground Floor



Туре:	Girder	Application:	Floor (Residential)
Plies:	2	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

Unfactored	Reactions	UNPATTERNED	lb	(Uplift)
------------	-----------	--------------------	----	----------

Brg	Live	Dead	Snow	Wind
1	1545	622	0	0
2	1762	732	0	0

Bearings	and	Factored	Reactions
Dooring	Longit	Con	Booot D/L II

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	2.375"	61%	777 / 2317	3094	L	1.25D+1.5L
2 - SPF	7.754"	21%	915 / 2643	3559	L	1.25D+1.5L

Analysis Results

Member Information

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	9242 ft-lb	5'9 9/16"	22724 ft-lb	0.407 (41%)	1.25D+1.5L	L
Unbraced	9242 ft-lb	5'9 9/16"	20195 ft-lb	0.458 (46%)	1.25D+1.5L	L
Shear	3271 lb	11 1/8"	9277 lb	0.353 (35%)	1.25D+1.5L	L ^a
Perm Defl in.	0.092 (L/1482)	5'9 9/16"	0.377 (L/360)	0.240 (24%)	D	Uniform
LL Defl inch	0.229 (L/594)	5'9 9/16"	0.377 (L/360)	0.610 (61%)	L	L
TL Defl inch	0.320 (L/424)	5'9 9/16"	0.566 (L/240)	0.570 (57%)	D+L	L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.

7 Lateral slenderness ratio based on full section width.							
ID	Load Type	Location	Trib Width	Side			
1	Tie-In	0-0-0 to 11-2-3	(Span)1-0-4	Тор			
2	Point	1-0-15		Far Face			

Dead Live Snow Wind Comments 15 PSF 0 PSF 40 PSF 0 PSF 124 lb 332 lb 0 lb 0 lb J5 0 PLF 0 PLF 3 101 PLF 270 PLF Part Uniform 1-8-15 to 11-0-15 Far Face 15 PSF 40 PSF 0 PSF 0 PSF 11-2-3 to 12-0-7 (Span)1-1-0 Tie-In Top to 0-2-12 Point 11-8-15 Far Face 48 lb 128 lb 0 lb 0 lb J5 5

Continued on page 2...

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

1.

LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

approvals
Damaged Beams must not be used
Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation 4.

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318





Page 2 of 2

EWP Studio Simpson Strong-Tie® Component Solutions™

Client: Project: Address: **GREEN YORK HOMES**

Date: 5/31/2018 Designer: RCO

Job Name: LIANA 2 (ELEV.1)

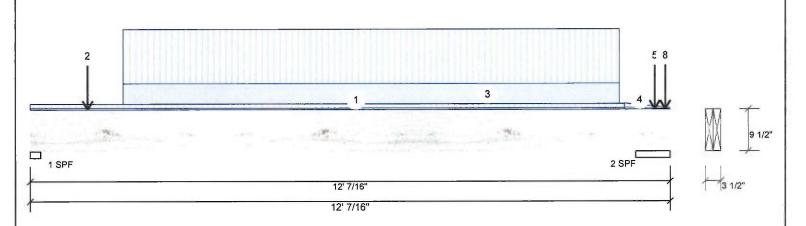
Project #:

Forex 2.0E-3000Fb LVL F4-D

1.750" X 9.500"

2-Ply - PASSED

Level: Ground Floor



	Continued from	m page 1							
	ID	Load Type	Location Trib Width	Side	Dead	Live	Snow	Wind	Comments
	6	Point	11-11-5	Тор	16 lb	34 lb	0 lb	0 lb	J4
	7	Point	11-11-5	Тор	20 lb	54 lb	0 lb	0 lb	J5
1	8	Point	11-11-5	Тор	22 lb	0 lb	0 lb	0 lb	Wall Self Weight
		Self Weight			8 PLF				

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS.

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

Handling & Installation

1. LVL beams must not be cut or drilled

2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastering details, beam strength values, and code approvals

3. Damaged Beams must not be used

4. Design assumes top edge is laterally restrained

5. Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex APA: PR-L318







Client: Project: Address: **GREEN YORK HOMES**

Date: 5/31/2018

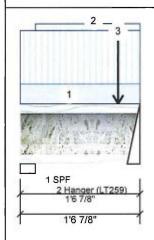
Designer: RCO

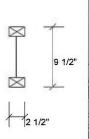
Job Name: LIANA 2 (ELEV.1)

Project #:

9.500" - PASSED F5-A NJH

Level: Ground Floor





Wind

0

1.25D+1.5L

1.25D+1.5L

Page 1 of 1

Туре:	Girder
Plies:	1
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	
Floor Live:	40 PSF
Dead:	15 PSF

alunia Danulta

Application: Floor (Residential) Design Method: LSD NBCC 2010 / OBC 2012 **Building Code:** Load Sharing: Not Checked Vibration: Not Checked

2	135	67		0	0
Bearings	and Fact	ored Reactions			
Bearing	Lenath	Cap. React D/L lb	Total	Ld. Case	Ld. Comb.

39 / 98

84 / 202

31

Snow

136 L

286 L

0

Unfactored Reactions UNPATTERNED Ib (Uplift) Dead

Live

65

Brg

1

1 - SPF 2.375"

2.000"

2 -

Hanger

۲	anaiysis kes	ialysis Results										
	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case					
	Moment	52 ft-lb	1' 1/8"	3830 ft-lb	0.014 (1%)	1.25D+1.5L	L					
	Unbraced	52 ft-lb	1' 1/8"	3779 ft-lb	0.014 (1%)	1.25D+1.5L	L					
	Shear	272 lb	1'5 5/8"	1580 lb	0.172 (17%)	1.25D+1.5L	L					
	Perm Defl in.	0.000 (L/56622)	11 7/8"	0.044 (L/360)	0.010 (1%)	D	Uniform					
	LL Defl inch	0.001 (L/28154)	11 7/8"	0.044 (L/360)	0.010 (1%)	L	L					
	TL Defl inch	0.001 (L/18804)	11 7/8"	0.067 (L/240)	0.010 (1%)	D+L	L					
7												

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

9%

18%

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS.



Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top flange unbraced.
- 4 Bottom flange braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-6-14	(Span)3-2-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Part. Uniform	0-2-6 to 1-6-14		Тор	8 PLF	0 PLF	0 PLF	0 PLF	
3	Point	1-3-7		Near Face	49 lb	99 lb	0 lb	0 lb	J2

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

- Loist flanges must not be cut or drilled Refer to latest copy of the Jubist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-ply fastening details and handling/erection details.
- nanciing/erection details

 Damaged IJoists must not be used

 Design assumes top flange to be laterally restrained
 by attached sheathing or as specified in engineering
 notes.
- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Nascor by Kott



Kott Lumber Company 14 Anderson Blvd, Ontario Canada





Client:

GREEN YORK HOMES

Project: Address: Date: 5/31/2018

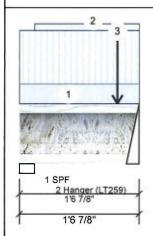
Designer: RCO

Job Name: LIANA 2 (ELEV.1)

Project #:

9.500" - PASSED F5-B NJH

Level: Ground Floor



9 1/2'

Wind

0

Page 1 of 1

Member Info	rmation			Unfactored Reactions UNPATTERNED lb (Uplift)					
Туре:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow		
Plies:	1	Design Method:	LSD	1	63	30	0		
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	120	59	0		
Deflection LL:	360	Load Sharing:	No	18					
Deflection TL:	240	Deck:	Not Checked						
Importance:	Normal	Vibration:	Not Checked						
General Load				_					
Floor Live:	40 PSF			Bearing	s and Fact	ored Reactions			
Dead:	15 PSF	l l		Bearing	Length	Can React D/L lb	Total 1d Case		

Bearings	and	Factored	Reactions

_							
Bearing L	_ength	Cap. I	React D/L lb	Total	Ld. Case	Ld. Comb.	
1-SPF 2	2.375"	8%	37 / 94	131	L	1.25D+1.5L	
2 - 2 Hanger	2.000"	16%	74 / 180	254	L	1.25D+1.5L	

Analysis Results

Γ	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
l	Moment	48 ft-lb	11 11/16"	3830 ft-lb	0.013 (1%)	1.25D+1.5L	L
l	Unbraced	48 ft-lb	11 11/16"	3779 ft-lb	0.013 (1%)	1.25D+1.5L	L
l	Shear	240 lb	1'5 5/8"	1580 lb	0.152 (15%)	1.25D+1.5L	L
	Perm Defl in.	0.000 (L/61792)	11 7/16"	0.044 (L/360)	0.010 (1%)	D	Uniform
	LL Defl inch	0.001 (L/30562)	11 1/2"	0.044 (L/360)	0.010 (1%)	L	L
	TL Defl inch	0.001 (L/20448)	11 1/2"	0.067 (L/240)	0.010 (1%)	D+L	L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top flange unbraced.
- 4 Bottom flange braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-6-14	(Span)3-2-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Part. Uniform	0-2-6 to 1-6-14		Тор	8 PLF	0 PLF	0 PLF	0 PLF	
3	Point	1-3-7		Far Face	40 lb	82 lb	0 lb	0 lb	J2

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

- Licist flanges must not be cut or drilled Refer to latest copy of the IJoist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-ply fastening details and handling/erection details
- Damaged Loists must not be used
 Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.
- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Nascor by Kott







Client: Project: **GREEN YORK HOMES**

Address:

5/31/2018 Date:

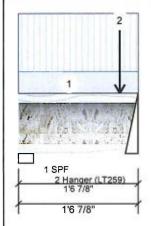
Designer: RCO

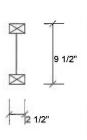
Job Name: LIANA 2 (ELEV.1)

Project #:

9.500" - PASSED F5-C NJH

Level: Ground Floor





Wind

0

0

1.25D+1.5L

1.25D+1.5L

Page 1 of 1

Member Information							
Туре:	Girder						
Plies:	1						
Moisture Condition:	Dry						
Deflection LL:	360						
Deflection TL:	240						
Importance:	Normal						
General Load							
Floor Live:	40 PSF						
Dead:	15 PSF						

Application: Floor (Residential) Design Method:

Building Code:

Load Sharing:

Deck: Vibration: NBCC 2010 / OBC 2012

Not Checked Not Checked

Bearing Length

1 - SPF 2.375"

2.000"

2

2 -

Hanger

Unfactored Reactions UNPATTERNED Ib (Uplift) Dead Snow 0

44

28 / 88

55 / 173

Bearings and Factored Reactions Cap. React D/L lb Ld. Comb. Total Ld. Case

116 L

227 L

0

Analysis Results

Γ	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
ı	Moment	39 ft-lb	11"	3830 ft-lb	0.010 (1%)	1.25D+1.5L	L
ı	Unbraced	39 ft-lb	11"	3779 ft-lb	0.010 (1%)	1.25D+1.5L	L
ı	Shear	214 lb	1'5 5/8"	1580 lb	0.135 (14%)	1.25D+1.5L	L
ı	Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
l	LL Defl inch	0.000 (L/35261)	10 7/8"	0.044 (L/360)	0.010 (1%)	L	L
	TL Defl inch	0.001 (L/25598)	10 7/8"	0.067 (L/240)	0.010 (1%)	D+L	L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

7%

14%

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS

Live

59

115



Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top flange unbraced.
- 4 Bottom flange braced at bearings.

	main nange ziaeet at zeeninge.								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-6-14	(Span)3-2-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	1-4-0		Near Face	28 lb	73 lb	0 lb	0 lb	J2

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Handling & Installation

- andling & Installation
 Libids flanges must not be cut or drilled
 Refer to latest copy of the Joist product information
 details for framing details, stiffener tables, web hole
 chart, bridging details, multi-ply fastening details and
 handling/erection details
 Damaged Libids must not be used
 Design assumes top flange to be laterally restrained
 by attached sheathing or as specified in engineering
 notes.
- Provide lateral support at bearing points to avoid lateral displacement and rotation.
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Nascor by Kott







Client:

GREEN YORK HOMES

Project: Address: Date: 5/31/2018

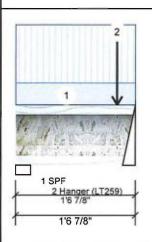
Designer: **RCO**

Job Name: LIANA 2 (ELEV.1)

Level: Ground Floor

Project #:

9.500" - PASSED F5-D NJH



9 1/2"

Page 1 of 1

Member Inform	nation
Type:	Girder
Plies:	1
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	
Floor Live:	40 PSF
Dead:	15 PSF

Actual

39 ft-lb

39 ft-lb

218 lb

(L/34841)

(L/25376)

Perm Defl in. 0.000 (L/999)

LL Defl inch 0.000

TL Defl inch 0.001

Application: Floor (Residential) LSD Design Method: Building Code: NBCC 2010 / OBC 2012 Load Sharing: No Deck: Not Checked Vibration: Not Checked

Unfactored Reactions UNPATTERNED Ib (Uplift) Live Dead Snow Wind Brg 22 59 0 0 1 2 118 0 0

Cap. React D/L lb

28 / 89

55 / 177

Location Allowed Capacity Comb. Case 11 1/16" 3830 ft-lb 0.010 (1%) 1.25D+1.5L L 11 1/16" 3779 ft-lb 0.010 (1%) 1.25D+1.5L L 1'5 5/8" 1580 lb 0.138 (14%) 1.25D+1.5L L 0 999.000 (L/0) 0.000 (0%) 10 15/16" 0.044 (L/360) 0.010 (1%) L 10 15/16" 0.067 (L/240) 0.010 (1%) D+L L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

7%

15%

Bearings and Factored Reactions

Bearing Length

1 - SPF 2.375"

Hanger

2.000"

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Ld. Comb.

1.25D+1.5L

1.25D+1.5L

Total Ld. Case

116 L

231 L

Design Notes

Analysis Results

Analysis

Moment

Unbraced

Shear

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top flange unbraced.
- 4 Bottom flange braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-6-14	(Span)3-2-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	1-4-0		Far Face	28 lb	76 lb	0 lb	0 lb	J2

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads

Lumber

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- Losist flanges must not be cut or drilled.
 Refer to latest copy of the Joist product information details for framing details, stflener tables, web hole chart, bridging details, multi-ply fastening details and handling/erection details
 Damaged Joists must not be used
 Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roofs provide proper drainage to prevent ponding.

Manufacturer Info

Nascor by Kott



Kott Lumber Company 14 Anderson Blvd, Ontario 14A 7X4



Client:

GREEN YORK HOMES

Project: Address: Date:

Designer: RCO

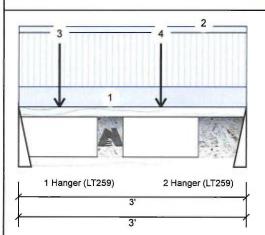
Job Name: LIANA 2 (ELEV.1)

5/31/2018

Level: Ground Floor

Project #:

9.500" - PASSED F6-A NJH



 \boxtimes 9 1/2'

Page 1 of 1

Member	Information
Туре:	Girder

Type:	Girder
Plies:	1
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	
Floor Live:	40 PSF
Dead:	15 PSF

Application: Design Method:

Floor (Residential) LSD

NBCC 2010 / OBC 2012 No

Load Sharing: Deck: Vibration:

Building Code:

Not Checked Not Checked

Unfactored Reactions UNPATTERNED Ib (Uplift)

135			
133	0	0	
105	0	0	
	105	105 0	105 0 0

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.	
1 - Hanger	2.000"	37%	169 / 411	581	L	1.25D+1.5L	
2 - Hanger	2.000"	29%	132 / 323	455	L	1.25D+1.5L	

Analysis Actual Location Allowed Capacity Comb. Case 420 ft-lb Moment 1'10 7/16" 3830 ft-lb 0.110 (11%) 1.25D+1.5L L 420 ft-lb 1'10 7/16" 3411 ft-lb Unbraced 0.123 (12%) 1.25D+1.5L L Shear 573 lb 1 1/4" 1580 lb 0.363 (36%) 1.25D+1.5L L Perm Defl in. 0.003 1'10 7/16" 0.093 (L/360) 0.030 (3%) D Uniform (L/12262)

LL Defl inch 0.006 (L/6011) 1'10 7/16" 0.093 (L/360) 0.060 (6%) L TL Defl inch 0.008 (L/4033) 1'10 7/16" 0.140 (L/240) 0.060 (6%) D+L READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Design Notes

Analysis Results

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top flange unbraced.

4 Bottom flange braced at bearings

T DOLLOTTI III	ange braces at bearings.									
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments	_
1	Tie-In	0-0-0 to 3-0-0	(Span)1-8-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
2	Part. Uniform	0-0-0 to 3-0-0		Тор	4 PLF	0 PLF	0 PLF	0 PLF		
3	Point	0-6-7		Far Face	84 lb	169 lb	0 lb	0 lb	J3	
4	Point	1-10-7		Far Face	106 lb	217 lb	0 lb	0 lb	J3	

L

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. Lumber

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- Libist flanges must not be cut or drilled Refer to latest copy of the I Joist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-ply fastening details and handling/erection details Damaged I Joists must not be used Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.
- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length >= 3.5 inches
 For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Nascor by Kott







EWP Studio Simpson Strong-Tie® Component Solutions™

Client:

GREEN YORK HOMES

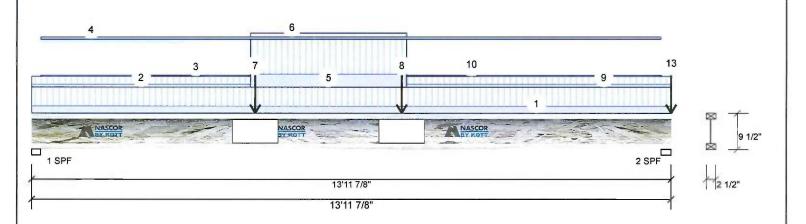
Project: Address: Date: 5/31/2018

Designer: **RCO**

Job Name: LIANA 2 (ELEV.1)

Project #:

F7-A NJH 9.500" - PASSED Level: Ground Floor



Туре:	Girder	Application:	Floor (Residential)
Plies:	1	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

Unfactored Reactions	UNPATTERNED	lb	(Uplift)
----------------------	-------------	----	----------

Brg	Live	Dead	Snow	Wind
1	370	181	0	0
2	533	273	0	0

Bearing	s and Fac	tored I	Reactions			
Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	2.375"	49%	226 / 555	781	L	1.25D+1.5L
2-SPF	2.625"	72%	342 / 800	1141	L	1.25D+1.5L

Analysis Results

Member Information

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	3313 ft-lb	7'1"	3830 ft-lb	0.865 (87%)	1.25D+1.5L	L
Unbraced	3313 ft-lb	7'1"	3323 ft-lb	0.997 (100%)	1.25D+1.5L	L
Shear	774 lb	1 5/8"	1580 lb	0.490 (49%)	1.25D+1.5L	L
Perm Defl in.	0.155 (L/1064)	6'11 3/8"	0.457 (L/360)	0.340 (34%)	D	Uniform
LL Defl inch	0.315 (L/522)	6'11 3/8"	0.457 (L/360)	0.690 (69%)	L	L
TL Defl inch	0.470 (L/350)	6'11 3/8"	0.685 (L/240)	0.690 (69%)	D+L	L

ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

READ ALL NOTES ON THIS PAGE AND ON

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS



Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Top flange must be laterally braced at a maximum of 3' o.c.

3 Bottom flange braced at bearings.

3 DOLLOTT	nange braced at bearing	33.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 13-11-14	(Span)0-11-5	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 4-9-6	(Span)0-4-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Part. Uniform	0-2-6 to 4-9-6		Тор	1 PLF	0 PLF	0 PLF	0 PLF	
4	Part. Uniform	0-2-6 to 13-9-2		Тор	2 PLF	0 PLF	0 PLF	0 PLF	
5	Tie-In	4-9-6 to 8-2-6	(Span)1-8-11 to 1-8-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
6	Part. Uniform	4-9-6 to 8-2-6		Тор	4 PLF	0 PLF	0 PLF	0 PLF	
7	Point	4-10-10		Near Face	59 lb	120 lb	0 lb	0 lb	F5
ontinued or	n page 2								

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads

Lumber

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

I. Judist flanges must not be cut or drilled.

Refer to latest copy of the I.Joist product information details for framing details, suffener tables, wee hole chart, bridging details, multi-ply fastening details and handling/erection details

Damaged Judist must not be used

Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing lengths—3.5 inches
 For flat roofs provide proper drainage to prevent
- ponding

Manufacturer Info

Nascor by Kott



Kott Lumber Company 14 Anderson Blvd, Ontario Canada 14A7X4





Continued from some

Page 2 of 2

EWP Studio Simpson Strong-Tie® Component Solutions™

Client: Project: Address: **GREEN YORK HOMES**

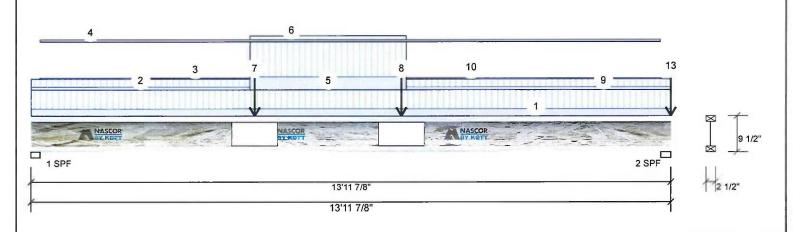
Date: 5/31/2018

Designer: **RCO** Job Name: LIANA 2 (ELEV.1)

Project #:

9.500" - PASSED F7-A NJH

Level: Ground Floor



Continued from	1 page 1								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
8	Point	8-1-2		Near Face	67 lb	135 lb	0 lb	0 lb	F5
9	Tie-In	8-2-6 to 13-11-14	(Span)0-4-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
10	Part. Uniform	8-2-6 to 13-9-2		Тор	1 PLF	0 PLF	0 PLF	0 PLF	
11	Point	13-11-14		Тор	35 lb	91 lb	0 lb	0 lb	J5
12	Point	13-11-14		Тор	41 lb	93 lb	0 lb	0 lb	J5
13	Point	13-11-14		Тор	27 lb	0 lb	0 lb	0 lb	Wall Self Weight

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

Handling & Installation

1. Uloist flanges must not be cut or drilled

2. Refer to latest copy of the IJoist product information details for framing details, siftener tables, web hole chart, bridging details, multi-ply fastening details and handling/erection details

3. Damaged IJoists must not be used

4. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Nascor by Kott







EWP Studio Simpson Strong-Tie® Component Solutions™

Client:

GREEN YORK HOMES

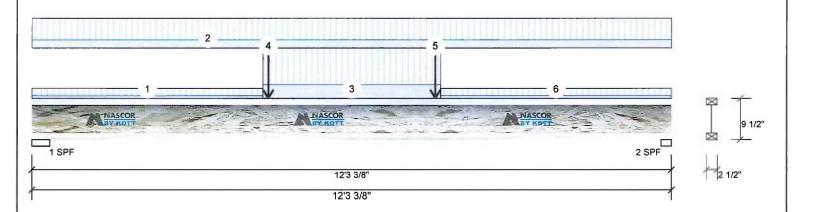
Project: Address: Date: 5/31/2018

Designer: RCO

Job Name: LIANA 2 (ELEV.1)

Project #:

F7-B NJH 9.500" - PASSED Level: Ground Floor



Member Info	rmation			Unfacto	red Reac	tions L	INPATTERN	ED lb (Uplift)	
Туре:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snov	N	Wind
Plies:	1	Design Method:	LSD	1	331		125		0	0
Moisture Conditi	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	324		122		0	0
Deflection LL:	360	Load Sharing:	No	1						
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal	Vibration:	Not Checked							
General Load		-11								
Floor Live:	40 PSF			Bearing:	s and Fac	tored	Reactions			
Dead:	15 PSF			Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
				1 - SPF	4.125"	41%	156 / 496	652	L	1.25D+1.5L
				2-SPF	2.375"	40%	152 / 486	638	L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2390 ft-lb	6'2 3/4"	3830 ft-lb	0.624 (62%)	1.25D+1.5L	L
Unbraced	2390 ft-lb	6'2 3/4"	2404 ft-lb	0.994 (99%)	1.25D+1.5L	L
Shear	637 lb	3 3/8"	1580 lb	0.403 (40%)	1.25D+1.5L	L
Perm Defl in.	0.072 (L/1978)	6'2 1/2"	0.395 (L/360)	0.180 (18%)	D	Uniform
LL Defl inch	0.191 (L/744)	6'2 9/16"	0.395 (L/360)	0.480 (48%)	L	L
TL Defl inch	0.263 (L/541)	6'2 9/16"	0.593 (L/240)	0.440 (44%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Top flange must be laterally braced at a maximum of 4'9" o.c.

3 Bottom flange braced at bearings.

	riidiyolo	Actual	LOCATION	Allowed	Capacity	COMB.	Case
	Moment	2390 ft-lb	6'2 3/4"	3830 ft-lb	0.624 (62%)	1.25D+1.5L	L
	Unbraced	2390 ft-lb	6'2 3/4"	2404 ft-lb	0.994 (99%)	1.25D+1.5L	L
	Shear	637 lb	3 3/8"	1580 lb	0.403 (40%)	1.25D+1.5L	L
	Perm Defl in.	0.072 (L/1978)	6'2 1/2"	0.395 (L/360)	0.180 (18%)	D	Uniform
	LL Defl inch	0.191 (L/744)	6'2 9/16"	0.395 (L/360)	0.480 (48%)	L	L
	TL Defl inch	0.263 (L/541)	6'2 9/16"	0.593 (L/240)	0.440 (44%)	D+L	L
ī		-					

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS



ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 4-5-3	(Span)0-4-2	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 12-3-6	(Span) 0-11-14	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Tie-In	4-5-3 to 7-10-3	(Span)1-8-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
4	Point	4-6-7		Far Face	44 lb	115 lb	0 lb	0 lb	F5
5	Point	7-8-15		Far Face	44 lb	118 lb	0 lb	0 lb	F5
6	Tie-In	7-10-3 to 12-3-6	(Span)0-4-2	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- Hartidiation
 Liulist flangers must not be out or drilled
 Refer to latest copy of the I Joist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-ply fastening details and handling/erection details
 Damaged Lipliats must not be used
 Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.
- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length ≥ 3.5 inches
 For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Nascor by Kott





EWP Studio Simpson Strong-Tie® Component Solutions™

Client: Project: GREEN YORK HOMES

Address:

Date: 5/31/2018

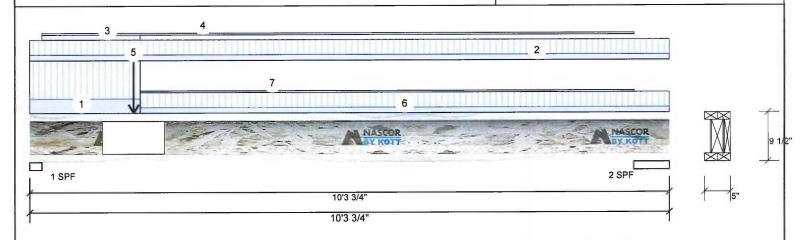
Designer: RCO

Job Name: LIANA 2 (ELEV.1)

Project #:

2-Ply - PASSED 9.500" F8-A NJH

Level: Ground Floor



Туре:	Girder	Application:	Floor (Residential)
Plies:	2	Design Method:	LSD
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF	İ	

Unfactored Reactions UNPATT	ERNED lb (Uplift)
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Brg	Live	Dead	Snow	Wind
1	514	250	0	0
2	325	155	0	0

Bearings and Factored Reactions

Bearing Length	Cap. R	leact D/L lb	Total	Ld. Case	Ld. Comb.	
1 - SPF 2.375"	34%	313 / 771	1084	L	1.25D+1.5L	
2 - SPF 6.875"	22%	194 / 488	682	L	1.25D+1.5L	

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1756 ft-lb	4'2 9/16"	7660 ft-lb	0.229 (23%)	1.25D+1.5L	L
Unbraced	1756 ft-lb	4'2 9/16"	1759 ft-lb	0.999 (100%)	1.25D+1.5L	L
Shear	1059 lb	1 5/8"	3160 lb	0.335 (34%)	1.25D+1.5L	L
Perm Defl in.	0.024 (L/4916)	4'8 11/16"	0.322 (L/360)	0.070 (7%)	D	Uniform
LL Defl inch	0.048 (L/2404)	4'8 11/16"	0.322 (L/360)	0.150 (15%)	L	L
TL Defl inch	0.072 (L/1614)	4'8 11/16"	0.483 (L/240)	0.150 (15%)	D+L	L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 8'5" o.c.

5 Bottom flange braced at bearings.

5 Bottom	flange braced at bearing	S							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-9-6	(Span)3-5-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 10-3-12	(Span)1-3-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Part. Uniform	0-2-6 to 1-9-6		Тор	9 PLF	0 PLF	0 PLF	0 PLF	
4	Part. Uniform	0-2-6 to 9-8-15		Тор	3 PLF	0 PLF	0 PLF	0 PLF	
5	Point	1-8-2		Far Face	105 lb	215 lb	0 lb	0 lb	F6
6	Tie-In	1-9-6 to 10-3-12	(Span)1-4-1	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
7	Part. Uniform	1-9-6 to 9-8-15		Тор	3 PLF	0 PLF	0 PLF	0 PLF	

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

- Handling & Installation
- IJoist flanges must not be cut or drilled Refer to latest copy of the IJoist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-ply fastening details and handling/erection details
- Damaged IJoiets must not be used Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length ≥ 3.5 inches
 For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Nascor by Kott



Kott Lumber Company 14 Anderson Blvd, Ontario L4A 7X4





Project:

GREEN YORK HOMES

Address:

Date: 5/31/2018

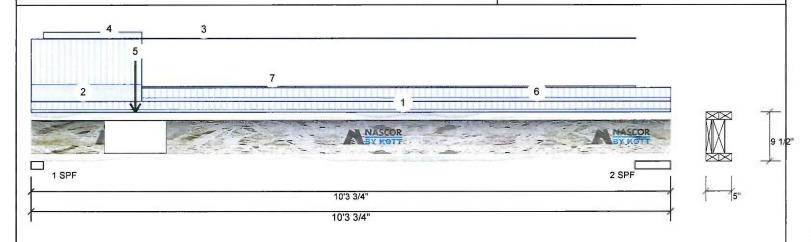
Designer: RCO

Job Name: LIANA 2 (ELEV.1)

Project #:

9.500" 2-Ply - PASSED F8-B NJH

Level: Ground Floor



MEHIDEI IIIIOIII	lation
Туре:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection II:	260

Member Information

Deflection LL: 360 Deflection TL: 240 Importance: Normal General Load Floor Live: 40 PSF Dead: 15 PSF Application: Design Method:

Floor (Residential) LSD

NBCC 2010 / OBC 2012 **Building Code:** No

Load Sharing: Deck:

Not Checked Vibration: Not Checked

Unfactored Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	vvina
1	451	220	0	0
2	194	93	0	0

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	2.375"	30%	276 / 676	951	L	1.25D+1.5L
2-SPF	6.875"	13%	116 / 291	407	L	1.25D+1.5L

Analysis Results

	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
	Moment	1286 ft-lb	3' 1/2"	7660 ft-lb	0.168 (17%)	1.25D+1.5L	L
	Unbraced	1286 ft-lb	3' 1/2"	1290 ft-lb	0.997 (100%)	1.25D+1.5L	L
l	Shear	930 lb	1 5/8"	3160 lb	0.294 (29%)	1.25D+1.5L	L
I	Perm Defl in.	0.017 (L/6925)	4'6"	0.322 (L/360)	0.050 (5%)	D	Uniform
I	LL Defl inch	0.034 (L/3400)	4'6"	0.322 (L/360)	0.110 (11%)	L	L
I	TL Defl inch	0.051 (L/2280)	4'6"	0.483 (L/240)	0.110 (11%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 9'7" o.c.

5 Bottom flange braced at bearings.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Page 1 of 1

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 10-3-12	(Span)0-7-1	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 1-9-6	(Span)3-5-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Part. Uniform	0-2-6 to 9-9-0		Тор	1 PLF	0 PLF	0 PLF	0 PLF	
4	Part. Uniform	0-2-6 to 1-9-6		Тор	9 PLF	0 PLF	0 PLF	0 PLF	
5	Point	1-8-2		Near Face	135 lb	274 lb	0 lb	0 lb	F6
6	Tie-In	1-9-6 to 10-3-12	(Span)0-8-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
7	Part. Uniform	1-9-6 to 9-9-0		Тор	2 PLF	0 PLF	0 PLF	0 PLF	

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 Upoist not to be treated with fire retardant or corrosive

Handling & Installation

andling & Installation
Lioist flanges must not be cut or drilled
Refer to latest copy of the Lioist product information
details for framing details, stiffener tables, web hole
chart, bridging details, multi-ply fastening details and
handling/erection details
Damaged Lioists must not be used
Design assumes top flange to be laterally restrained
by attached sheathing or as specified in engineering
notes.

Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roofs provide proper drainage to prevent pending

Manufacturer Info

Nascor by Kott







EWP Studio Simpson Strong-Tie® Component Solutions™

Project: Address: **GREEN YORK HOMES**

Date: 5/31/2018

Designer: RCO

Job Name: LIANA 2 (ELEV.1)

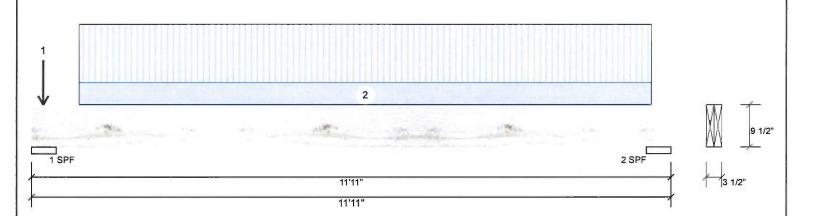
Project #:

BBO4-A Forex 2.0E-3000Fb LVL

1.750" X 9.500"

2-Ply - PASSED

Level: Second Floor



Member Info	rmation			Unfactore	ed React	ions UNPATTERN	IED lb (Uplift)		
Туре:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind	
Plies:	2	Design Method:	LSD	1	1378	565	0	0	
Moisture Conditi	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	1362	559	0	0	
Deflection LL:	360	Load Sharing:	No	=.3723					
Deflection TL:	240	Deck:	Not Checked						
Importance:	Normal	Vibration:	Not Checked						
General Load									_
Floor Live:	40 PSF			Bearings	and Fac	tored Reactions			
Dead:	15 PSF			Bearing L	_ength	Cap. React D/L lb	Total Ld. Case	Ld. Comb.	_
				1 - SPF 5	5.500"	23% 706 / 2068	2774 L	1.25D+1.5L	
		7		2 - SPF 5	5 500"	23% 699 / 2043	2742 L	1.25D+1.5L	

Analysis Results

ľ	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
l	Moment	7560 ft-lb	5'11 5/8"	22724 ft-lb	0.333 (33%)	1.25D+1.5L	L
l	Unbraced	7560 ft-lb	5'11 5/8"	20280 ft-lb	0.373 (37%)	1.25D+1.5L	L
l	Shear	2345 lb	1'2 1/4"	9277 lb	0.253 (25%)	1.25D+1.5L	L
l	Perm Defl in.	0.074 (L/1812)	5'11 9/16"	0.371 (L/360)	0.200 (20%)	D	Uniform
l	LL Defl inch	0.180 (L/740)	5'11 9/16"	0.371 (L/360)	0.490 (49%)	L	L
	TL Defl inch	0.254 (L/525)	5'11 9/16"	0.556 (L/240)	0.460 (46%)	D+L	L

Design Notes

- Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size; beam width X 4.5.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on full section width.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS



ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Point	0-2-10		Тор	52 lb	138 lb	0 lb	016	J5
2	Part. Uniform	0-10-10 to 11-6-10		Тор	92 PLF	244 PLF	0 PLF	0 PLF	
	Self Weight				8 PLF				

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

Andling & Installation
LVL beams must not be cut or drilled
Refer to manufacturer's product information
regarding installation requirements, multi-ply
fastening details, beam strength values, and code
approvals
Damaged Beams must not be used
Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent

Manufacturer Info

Forex APA: PR-L318









Client:

GREEN YORK HOMES

Project: Address:

5/31/2018 Date:

RCO Designer:

Job Name: LIANA 2 (ELEV.1)

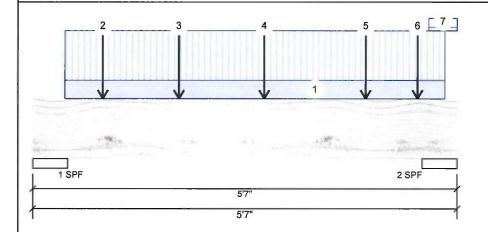
Project #:

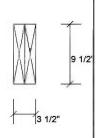
Forex 2.0E-3000Fb LVL

1.750" X 9.500"

2-Ply - PASSED

Level: Second Floor





Page 1 of 2

Туре:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	
Floor Live:	40 PSF

Member Information

Floor (Residential) Application: Design Method: LSD **Building Code:** NBCC 2010 / OBC 2012 Load Sharing: Not Checked Deck:

Unfactored Reactions UNPATTERNED lb (Uplift) Live Dead Snow Wind 1315 1 519 0 2 1964 809 0 0

Vibration: Not Checked 15 PSF Dead:

Bearings and Factored Reactions

Bearing Le	ength Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF 5.5	500" 22%	649 / 1973	2622	L	1.25D+1.5L
2 - SPF 5.5	500" 33%	1012 / 2947	3959	L	1.25D+1.5L

Analysis Results

Γ	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
l	Moment	3183 ft-lb	3' 9/16"	22724 ft-lb	0.140 (14%)	1.25D+1.5L	L
l	Unbraced	3183 ft-lb	3' 9/16"	22724 ft-lb	0.140 (14%)	1.25D+1.5L	L
ı	Shear	3768 lb	4'4 3/4"	9277 lb	0.406 (41%)	1.25D+1.5L	L
l	Perm Defl in.	0.007 (L/7855)	2'10 1/8"	0.160 (L/360)	0.050 (5%)	D	Uniform
l	LL Defl inch	0.019 (L/3082)	2'10 1/8"	0.160 (L/360)	0.120 (12%)	L	L
	TL Defl inch	0.026 (L/2214)	2'10 1/8"	0.240 (L/240)	0.110 (11%)	D+L	L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.

6 Lateral slenderness ratio based on full section width.

	ordinadiriodd ratio badda d	i ian occupi mani.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Part. Uniform	0-5-1 to 5-5-1		Far Face	104 PLF	276 PLF	0 PLF	0 PLF	
2	Point	0-11-1		Near Face	112 lb	292 lb	0 lb	0 lb	J5
3	Point	1-11-1		Near Face	105 lb	281 lb	0 lb	0 lb	J5
4	Point	3-0-9		Near Face	123 lb	327 lb	0 lb	0 lb	J5
5	Point	4-4-9		Near Face	100 lb	268 lb	0 lb	0 lb	J5
6	Point	5-0-12		Near Face	319 lb	713 lb	0 lb	0 lb	F4
ontinued or	n page 2								

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component sultability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals Damaged Beams must not be used

Daniga Beans must not be used
Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex APA: PR-L318







Client: Project: Address: **GREEN YORK HOMES**

Date:

5/31/2018

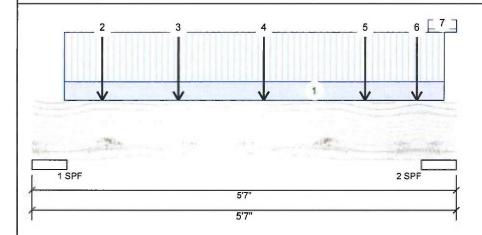
Designer: RCO Job Name: LIANA 2 (ELEV.1)

Project #:

F10-A Forex 2.0E-3000Fb LVL 1.750" X 9.500"

2-Ply - PASSED

Level: Second Floor



9 1/2

Page 2 of 2

.Continued from page 1

ID Load Type 7 Tie-In

Location Trib Width 5-2-8 to 5-7-0 (Span)2-6-3

Dead 15 PSF

Side

Top

Live 40 PSF

Snow 0 PSF Wind Comments

0 PSF

Self Weight

8 PLF

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS.

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

LVL beams must not be cut or drilled
Refer to manufacturer's product information
regarding installation requirements, multi-ply
fastening details, beam strength values, and code
approvals

approves

Damaged Beams must not be used

Design assumes top edge is laterally restrained

Provide lateral support at bearing points to avoid

lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318









Client:

GREEN YORK HOMES

Project: Address: Date: 5/31/2018

Designer: RCO

Job Name: LIANA 2 (ELEV.1)

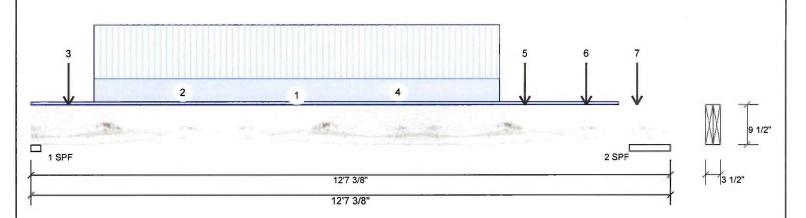
Project #:

Forex 2.0E-3000Fb LVL

1.750" X 9.500"

2-Ply - PASSED

Level: Second Floor



Brg

Туре:	Girder	Application:	Floor (Residential)
Plies:	2	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

Unfactored Reaction	ons UNPATTERNE	D lb (Uplift)
----------------------------	----------------	---------------

Live

1	1567	722	0	0
2	1790	804	0	0
7		155.00		155

Snow

Dead

Bearings	Bearings and Factored Reactions									
Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.				
1 - SPF	2.375"	64%	902 / 2351	3253	L	1.25D+1.5L				
2 - SPF	9.714"	18%	1005 / 2685	3690	L	1.25D+1.5L				

Analysis Results

Member Information

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	9822 ft-lb	6'	22724 ft-lb	0.432 (43%)	1.25D+1.5L	L
Unbraced	9822 ft-lb	6'	20006 ft-lb	0.491 (49%)	1.25D+1.5L	L
Shear	3223 lb	11 1/8"	9277 lb	0.347 (35%)	1.25D+1.5L	L
Perm Defl in.	0.115 (L/1224)	6' 1/16"	0.391 (L/360)	0.290 (29%)	D	Uniform
LL Defl inch	0.251 (L/561)	6'	0.391 (L/360)	0.640 (64%)	L	L
TL Defl inch	0.366 (L/385)	6' 1/16"	0.587 (L/240)	0.620 (62%)	D+L	L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Wind

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.

	6 Lateral	slenderness ratio based o	n full section width.	
	ID	Load Type	Location	Trib \
1	4	Tie In	0.0.0 to 11.7.2	/Snan

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 11-7-2	(Span)0-6-7	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Part. Uniform	0-2-7 to 11-1-5		Тор	1 PLF	0 PLF	0 PLF	0 PLF	
3	Point	0-8-15		Far Face	104 lb	238 lb	0 lb	0 lb	J5
4	Part. Uniform	1-2-15 to 9-2-15		Far Face	114 PLF	266 PLF	0 PLF	0 PLF	
5	Point	9-8-15		Far Face	128 lb	294 lb	0 lb	0 lb	J5
6	Point	10-11-7		Far Face	123 lb	292 lb	0 lb	0 lb	J5

Continued on page 2...

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

approvais
Damaged Beams must not be used
Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

For flat roofs provide proper drainage to prevent conding

Manufacturer Info

APA: PR-L318

Kott Lumber Company Canada L4A 7X4 905-642-4400





Page 2 of 2

EWP Studio Simpson Strong-Tie® Component Solutions™

Client: Project:

Address:

GREEN YORK HOMES

5/31/2018

Designer: RCO

Job Name: LIANA 2 (ELEV.1)

Project #:

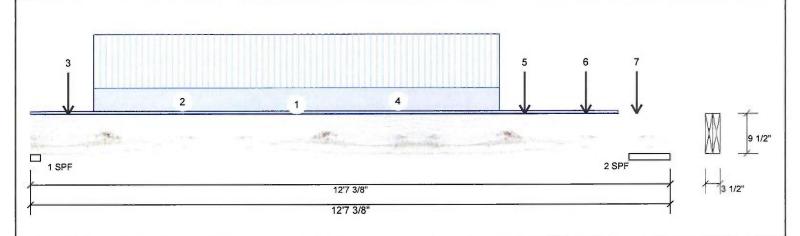
Date:

Forex 2.0E-3000Fb LVL F4-A

1.750" X 9.500"

2-Ply - PASSED

Level: Second Floor



.Continued from page 1

ID Load Type 7 Point

Self Weight

Location Trib Width 11-11-7

Side Far Face 105 lb

Dead Live 281 lb Snow di 0 Wind Comments 0 lb J5

8 PLF

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code appropriate

4. 5.

papervals

Damaged Beams must not be used

Design assumes top edge is laterally restrained

Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Forex

APA: PR-L318

Manufacturer Info







Client: Project: Address: **GREEN YORK HOMES**

5/31/2018

RCO Designer:

Job Name: LIANA 2 (ELEV.1)

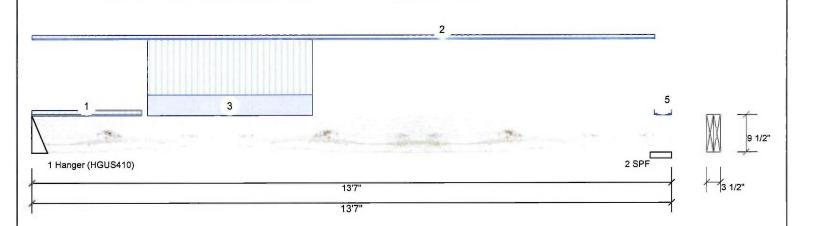
Project #:

Forex 2.0E-3000Fb LVL

1.750" X 9.500"

2-Ply - PASSED

Level: Second Floor



Member Inform	nation			Unfacto	red Reac	tions U	NPATTERN	ED lb ((Uplift)
Туре:	Girder	Application:	Floor (Residential)	Brg	Live	la e	Dead	Sno	w
Plies:	2	Design Method:	LSD	1	713		319		0
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012	2	357		186		0
Deflection LL:	360	Load Sharing:	No	F-1					
Deflection TL:	240	Deck:	Not Checked						
Importance:	Normal	Vibration:	Not Checked						
General Load									
Floor Live:	40 PSF			Bearing	s and Fac	tored	Reactions		
Dead:	15 PSF			Bearing	Length	Сар.	React D/L lb	Total	Ld. Case
				1 - Hanger	4.000"	14%	398 / 1070	1468	L

Analysis	Results
----------	---------

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	4658 ft-lb	5' 1/8"	22724 ft-lb	0.205 (20%)	1.25D+1.5L	L
Unbraced	4658 ft-lb	5' 1/8"	19429 ft-lb	0.240 (24%)	1.25D+1.5L	L
Shear	1393 lb	1' 3/4"	9277 lb	0.150 (15%)	1.25D+1.5L	L
Perm Defl in.	0.058 (L/2674)	6'2 5/8"	0.431 (L/360)	0.130 (13%)	D	Uniform
LL Defl inch	0.128 (L/1212)	6'1 5/16"	0.431 (L/360)	0.300 (30%)	L	L
TL Defl inch	0.186 (L/834)	6'1 3/4"	0.646 (L/240)	0.290 (29%)	D+L	L

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on full section width.

3rg	Live	Dead	Snow	
	740	040	•	

1	713	319	0	0
2	357	186	0	0

_							
Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.	
1 -	4.000"	14%	398 / 1070	1468	L	1.25D+1.5L	
Hanger							
2 - SPF	5.500"	6%	233 / 535	768	L	1.25D+1.5L	

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Wind

Page 1 of 1

/ Lateral Si	icidei ileas fatio based	Of full Scotlott Width.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 2-3-14	(Span)0-10-5	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 13-2-12	(Span)0-8-3	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Part. Uniform	2-5-7 to 5-11-7		Тор	90 PLF	240 PLF	0 PLF	0 PLF	
4	Tie-In	13-2-10 to 13-7-0	(Span)0-10-5	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
5	Tie-In	13-2-12 to 13-7-0	(Span)0-5-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
- A29	Self Weight				8 PLF				

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

andling & Installation
LVL beams must not be cut or drilled
Refer to manufacturer's product information
regarding installation requirements, multi-ply
fastening details, beam strength values, and code
approvals
Damaged Beams must not be used
Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex APA: PR-L318







Client:

GREEN YORK HOMES

Project: Address:

5/31/2018 Date:

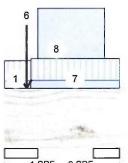
RCO Designer:

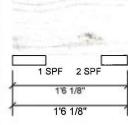
Job Name: LIANA 2 (ELEV.1)

Project #:

Forex 2.0E-3000Fb LVL F9-A

1.750" X 9.500" - PASSED Level: Second Floor





1.25D+1.5L

PROFESSIONAL STA

2018

Page 1 of 2

Member Infor	mation			Unfactore	d React	ions U	NPATTERNI	ED lb (Uplift)	
Туре:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snov	v	Wind
Plies:	1	Design Method:	LSD	1	283		457	56	6	0
Moisture Conditio	n: Dry	Building Code:	NBCC 2010 / OBC 2012	2	20		43		0	0
Deflection LL:	360	Load Sharing:	No							
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal	Vibration:	Not Checked							
General Load										
Floor Live:	40 PSF			Bearings a	and Fact	tored F	Reactions			
Dead:	15 PSF			Bearing L	ength	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
				1 - SPF 5	.250"	37%	571 / 991	1561	L	1.25D+1.5S +0.5L

2 - SPF 4.125"

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	10 ft-lb	9 5/8"	7385 ft-lb	0.001 (0%)	1.4D	Uniform
Unbraced	10 ft-lb	9 5/8"	7385 ft-lb	0.001 (0%)	1.4D	Uniform
Shear	40 lb	5 1/4"	3015 lb	0.013 (1%)	1.4D	Uniform
Perm Defl in	n. 0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		

1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

LL Defl inch 0.000 (L/999) TL Defl inch 0.000 (L/999) Design Notes

Analysis Results

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

width X 4.5. 2 Girders are

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS

- 3 Top braced

e designed to be supported on the bottom edge only.	
d at bearings.	
aced at bearings.	

0 999.000 (L/0) 0.000 (0%)

0 999.000 (L/0) 0.000 (0%)

4 Bottom braced at bearings.									
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 0-4-2	(Span)1-3-7	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	0-3-8		Тор	10 lb	0 lb	23 lb	0 lb	
3	Point	0-3-8		Тор	18 lb	0 lb	0 lb	0 lb	Wall Self Weight
4	Point	0-3-8		Тор	386 lb	262 lb	537 lb	0 lb	F12 F12
5	Point	0-3-8		Тор	2 lb	0 lb	6 lb	0 lb	
6	Point	0-3-8		Тор	6 lb	0 lb	d1 0	0 lb	Wall Self Weight
7	Tie-In	0-4-2 to 1-6-2	(Span)1-4-9	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
Continued or	nage 2								

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- andling & Installation
 LVL beams must not be cut or drilled
 Refer to manufacturers product information
 regarding installation requirements, multi-ply
 testiening details, beam strength values, and code
 approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex APA: PR-L318



Kott Lumber Company 14 Anderson Blvd, Ontario Canada L4A 7X4 905-642-4400

83 L





Client:

GREEN YORK HOMES

Project: Address: Date:

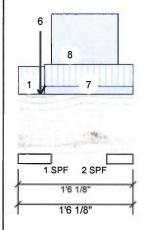
5/31/2018 Designer: RCO

Job Name: LIANA 2 (ELEV.1)

Project #:

F9-A Forex 2.0E-3000Fb LVL 1.750" X 9.500" - PASSED

Level: Second Floor



Page 2 of 2

.Continued from page 1

ID Load Type 8 Part. Uniform

Self Weight

0-5-4 to 1-3-12

Location Trib Width

Side Top

Dead 64 PLF

4 PLF

Live 0 PLF Snow 0 PLF

Wind Comments 0 PLF

Wall Self Weight

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

LVI beams must not be cut or drilled
Refer to manufacturer's product information
regarding installation requirements, multi-ply
fastening details, beam strength values, and code
approvals

approves
Damaged Beams must not be used
Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation 4. 5.

For flat roofs provide proper drainage to prevent pending

Manufacturer Info

APA; PR-L318









Client:

GREEN YORK HOMES

Project: Address:

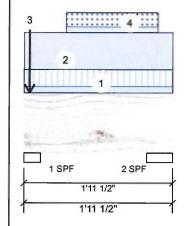
Date: 5/31/2018

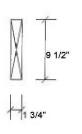
Designer: RCO

Job Name: LIANA 2 (ELEV.2)

Project #:

F9-A Forex 2.0E-3000Fb LVL 1.750" X 9.500" - PASSED Level: Second Floor





Page 1 of 1

Member Information							
Туре:	Girder						
Plies:	1						
Moisture Condition:	Dry						
Deflection LL:	360						
Deflection TL:	240						
Importance:	Normal						
General Load							
Floor Live;	40 PSF						
Dead:	15 PSF						

Application: Design Method:

Building Code:

Load Sharing:

0 999.000 (L/0) 0.000 (0%)

Deck:

Floor (Residential) NBCC 2010 / OBC 2012

No

Not Checked

Vibration: Not Checked

Unfactored Reactions UNPATTERNED I	lb	(Uplift)	
------------------------------------	----	----------	--

Brg	Live	Dead	Snow	Wind
1	184	324	365	0
2	31	90	19	0

Bearings and Factored Reactions

	Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.	_	
	1 - SPF	2.625"	41%	406 / 639	1045	L	1.25D+1.5S +0.5L		
	2-SPF	4.125"	4%	126 / 0	126	Uniform	1.4D		

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	35 ft-lb	11 1/16"	7385 ft-lb	0.005 (0%)	1.4D	Uniform
Unbraced	35 ft-lb	11 1/16"	7285 ft-lb	0.005 (0%)	1.4D	Uniform
Shear	d 8	10 5/8"	4638 lb	0.002 (0%)	1.25D+1.5S +0.5L	L
Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
LL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.
- 4 Bottom braced at bearings

TL Defl inch 0.000 (L/999)

. 5000	mi biassa at boarnigo.									
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments	•
1	Tie-In	0-0-0 to 1-11-8	(Span)1-5-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
2	Part. Uniform	0-0-0 to 1-11-8		Тор	64 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight	
3	Point	0-0-14		Тор	248 lb	157 lb	355 lb	0 lb	F11 F11	
4	Part. Uniform	0-6-10 to 1-9-2		Тор	10 PLF	0 PLF	24 PLF	0 PLF		
	Self Weight				4 PLF					

Notes

Calculated Structured Designs is responsible only of the Structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Handling & Installation

- ATIONING & INSTALLATION
 LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
 approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318







Client:

GREEN YORK HOMES

Project: Address: Date:

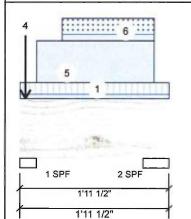
5/31/2018

RCO Designer:

Job Name: LIANA 2 (ELEV.2)

Project #:

1.750" X 9.500" - PASSED Level: Second Floor F9-B Forex 2.0E-3000Fb LVL



Page 1 of 1

lember Inform	nation			Unfactored Reactions UNPATTERNED Ib (Uplift)						
Туре:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind		
Plies:	1	Design Method:	LSD	1	173	318	370	0		
Moisture Condition:	: Dry	Building Code:	NBCC 2010 / OBC 2012	2	18	73	19	0		
Deflection LL:	360	Load Sharing:	No							
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal	Vibration:	Not Checked							
General Load										
Floor Live:	40 PSF			Bearings a	nd Facto	ored Reactions				
Dead:	15 PSF			Bearing Le	ngth	Cap. React D/L lb	Total Ld. Case	Ld. Comb.		
				1 - SPF 2.6	525"	41% 398 / 642	1040 L	1.25D+1.5S +0.5L		

	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
ı	Moment	34 ft-lb	11 1/16"	7385 ft-lb	0.005 (0%)	1.4D	Uniform
l	Unbraced	34 ft-lb	11 1/16"	7285 ft-lb	0.005 (0%)	1.4D	Uniform
	Shear	7 lb	10 5/8"	4638 lb	0.002 (0%)	1.25D+1.5S +0.5L	L
l	Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
	LL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		

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REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS

2 - SPF 4.125"



102 Uniform

Design Notes

Analysis Results

1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam

0 999.000 (L/0) 0.000 (0%)

- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.

TL Defl inch 0.000 (L/999)

4 Bottom	n braced at bearings.		_						
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-11-8	(Span)0-10-9	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	0-0-14		Тор	250 lb	157 lb	360 lb	0 lb	F11 F11
3	Point	0-0-14		Тор	5 lb	0 lb	0 lb	0 lb	Wall Self Weight
4	Point	0-0-14		Тор	5 lb	0 lb	0 lb	0 lb	Wall Self Weight
5	Part. Uniform	0-2-10 to 1-9-2		Тор	64 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
6	Part. Uniform	0-6-10 to 1-9-2		Тор	10 PLF	0 PLF	24 PLF	0 PLF	
	Self Weight				4 PLF				

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvalis

Damaged Beams must not be used

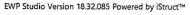
Dasign assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

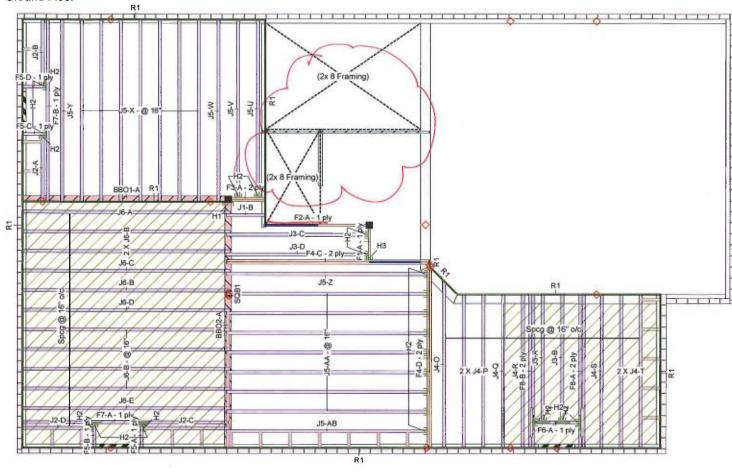
Manufacturer Info

APA: PR-L318





Ground Floor



THIS CERTIFICATION IS TO CONFIRM THAT:

- 1. THE LOADS USED IN THE CALCULATION OF THE ATTACHED APPROVED COMPONENTS CONFORM TO THE FLOOR ASSEMBLY SHOWN ON THIS LAYOUT.
- 2. THE FLOOR JOISTS COMPLY WITH THE NASCOR SPAN TABLE FOR THE LOADS AND SPACING SHOWN ON THIS LAYOUT.

THE FLOOR SYSTEM MUST BE ASSEMBLED IN ACCORDANCE TO THE NASCOR SPECIFIER GUIDE. MULTI-PLY MEMBERS MUST BE ATTACHED TOGETHER AS PER THE INCLUDED MULTIPLE MEMBER CONNECTION DETAIL.

ALL OTHER COMPONENTS AND STRUCTURAL ELEMENTS SUPPORTING THE FLOOR SYSTEM SUCH AS BEAMS, WALLS, **COLUMNS AND FOUNDATION WALLS AND FOOTINGS** INCLUDING ANCHORAGE OF COMPONENTS AND BRACING FOR LATERAL STABILITY ARE THE RESPONSIBILITY OF OTHERS.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS. PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL**

POINT LOADS OVER BEARINGS



Legend



Load from Above Wall Wall Opening Norbord Rimboard Plus 1.125 X 9.5 NJH 9.5

- 3. LVL CCMC -14056-R
- 4. CAN/CSA-O86-09
- 5 CCMC -12787-R APA PR-L310(C)



- 1. OBC 2012 O.Reg 332/12 as amended
- 2. Nascor CCMC 13535-R

Label	Descr	iption	Width	De	pth	Qty	Plies	Pcs	Length	NASCOR
F4	Forex 2.0E-3	000Fb LVL	1.75		9.5	2	2	4	14-0-0	
F2	Forex 2.0E-3	000Fb LVL	1.75		9.5			1	8-0-0	Layout Name LIANA 2 (ELEV.1&2)
F3	Forex 2.0E-3	000Fb LVL	1.75	y	9.5	1	2	2	4-0-0	Design Method
F1	Forex 2.0E-3	000Fb LVL	1.75	- 1	9.5			1	4-0-0	Description
I Joist (Flush)									GRANELLI HOMES CORP.
Label	Descr		Width	De	pth	Qty	Plies	Pcs	Length	BRAMPTON, ONT.
F7	NJH		2.5		9.5			2	14-0-0	Created
F8	NJH		2.5		9.5	2	2	4	12-0-0	May 29, 2018
F6	NJH		2.5		9.5			1	4-0-0	Builder
F5	J6 NJH J5 NJH		2.5		9.5			4	2-0-0	GREEN YORK HOMES
J6			2.5		9.5			11	16-0-0	
J5			2.5		9.5			20	14-0-0	Sales Rep
J4	NJH		2.5		9.5			8	12-0-0	RM
J3	NJH		2.5		9.5			4	10-0-0	Designer
	J2 NJH		2.5		9.5			4	6-0-0	RCO
J1	NJH		2.5		9.5			1	4-0-0	Shipping
Rim Bo	ard						3		30	Project
Label	Descr	iption	Width	De	pth	Qty	Plies	Pcs	Length	
R1		d Rimboard	1.125		9.5			13	12	Builder's Project
		125 X 9.5								Kott Lumber Company
Blockin										14 Anderson Blvd
Label		iption	Width	De		Qty	Plies	Pcs	Length	Stouffville, Ontario
BLK1	NJH		2.5		9.5	LinFt		Varies	29-0-0	Canada
Hanger						Be	am/Girder		ported ember	L4A 7X4 905-642-4400
Label	Pcs	Description	n SI	kew	Slop	e fa	steners	fas	teners	Job Path
H1	1	Unknown Hanger								D:\Users\rochavillo\WORK FROM HOME\GREEN YORK HOMES
H2 25 LT259 H3 1 HUS1.81/1		LT259	59 4 10dx1 1/2		10dx1 1/2	2 10dx1 1/2		\GRANELLI HOME CORP\MODELS \LIANA 2\LIANA 2 ELEV 1\FLOOR		
		1/10			_	30 16d		0 16d	LIANA ZILIANA Z ELEV TIFLOOR	

- . Framer to verify dimensions on the architectural drawings.
- 2. Double joist only require filler/backer ply when supporting another member using a face-mounted hanger.
- Install 2x4 blocking @ 24" o/c under parallel non-load bearing walls.
- 4. Install single-ply flush window header along inside face of rimboard/rimioist
- 5. Refer to Nascor specifier guide for installation works.
- . Squash blocks recommended to be installed at end bearing on all first level joists which support loading from above exceeding two levels floor or roof.
- . Load transfer blocks to be installed under all point loads.
- It shall be the framer's responsibility that floor joists and beams are fastened as per the hanger manufacturer's standards.

Refer to Multiple Member Connection Detail to ply to ply nailing or

Rim parallel to joists: 1-1/8" rimboard with 2"x 4" block (1/16" longer than rim depth @ 16" o/c). All other components and structural elements supporting the floor system such as beams, walls, columns, and foundation walls and footings including anchorage of components and bracing for lateral stability are the responsibility of Others

Hatch area represents ceramic tiled floor with an addtional dead load of 5 PSF

The framing shown on this layout may deviate from the architectural and structural drawings. Project Engineer to review and approve the deviation prior to construction.

ARCHITECTURAL DRAWINGS:

JARDIN DESIGN GROUP INC. 64 Jardin Dr., Suite 3A, Vaughan, ON Date: Rev.1; May 22,2018 Project No: 17-55 Model: Liana 2

EWP Studio Simpson Strong-Tie® Component Solutions™ 2012

15

480

360

480

360

360

240

480

360

OSB

3/4"

Nailed & Glued

Building Code NBCC 2010 / OBC

Floor

Loads

Live

Dead

Deflection Joist

LL Span L/

TL Span L/

LL Cant 2L/

TL Cant 2L/

LL Span L/

TL Span L/

LL Cant 2L/

TL Cant 2L/

Decking

Thickness

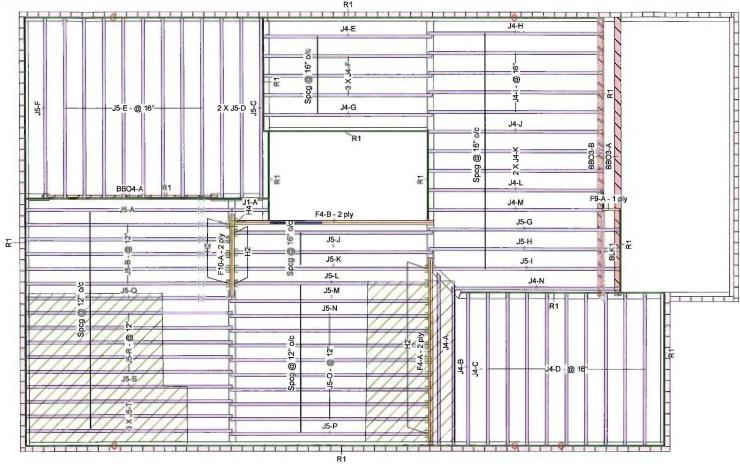
Fastener

Vibration

Deck

Deflection Girder

Second Floor



THIS CERTIFICATION IS TO CONFIRM THAT:

- 1. THE LOADS USED IN THE CALCULATION OF THE ATTACHED APPROVED COMPONENTS CONFORM TO THE FLOOR ASSEMBLY SHOWN ON THIS LAYOUT.
- 2. THE FLOOR JOISTS COMPLY WITH THE NASCOR SPAN TABLE FOR THE LOADS AND SPACING SHOWN ON THIS LAYOUT.

THE FLOOR SYSTEM MUST BE ASSEMBLED IN ACCORDANCE TO THE NASCOR SPECIFIER GUIDE. MULTI-PLY MEMBERS MUST BE ATTACHED TOGETHER AS PER THE INCLUDED MULTIPLE MEMBER CONNECTION DETAIL.

ALL OTHER COMPONENTS AND STRUCTURAL ELEMENTS SUPPORTING THE FLOOR SYSTEM SUCH AS BEAMS, WALLS, **COLUMNS AND FOUNDATION WALLS AND FOOTINGS** INCLUDING ANCHORAGE OF COMPONENTS AND BRACING FOR LATERAL STABILITY ARE THE RESPONSIBILITY OF OTHERS.

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REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS. PASS THRU FRAMING SQUASH

BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS.



Legend

Norbord Rimboard Plus 1.125 X 9.5 N.IH 9 5 Forex 2.0E-3000Fb LVL 1.75 X 9.5

Forex 2.0E-3000Fb LVL 1.75 X 9.5

3. LVL CCMC -14056-R



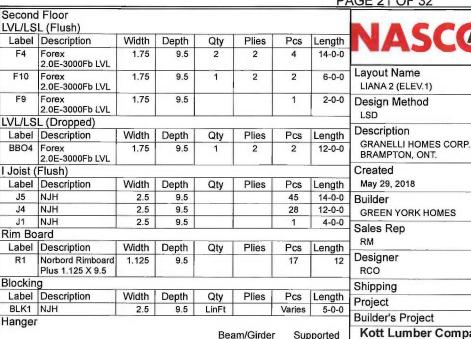
1. OBC 2012 O.Reg 332/12 as amended

2. Nascor CCMC - 13535-R

4. CAN/CSA-086-09

5. CCMC -12787-R APA PR-L310(C)





Skew Slope

H4 NOTES:

H2

- Framer to verify dimensions on the architectural drawings.
- 2. Double joist only require filler/backer ply when supporting another member using a face-mounted hanger.
- 3. Install 2x4 blocking @ 24" o/c under parallel non-load bearing walls.
- Install single-ply flush window header along inside face of rimboard/rimjoist.
- . Refer to Nascor specifier guide for installation works.

Pcs Description

21 LT259

1 HGUS410

- 6. Squash blocks recommended to be installed at end bearing on all first level joists which support loading from above exceeding two levels floor or roof.
- . Load transfer blocks to be installed under all point loads.
- B. It shall be the framer's responsibility that floor joists and beams are fastened as per the hanger manufacturer's standards.

Refer to Multiple Member Connection Detail to ply to ply nailing or bolting requirements

Rim parallel to joists: 1-1/8" rimboard with 2"x 4" block (1/16" longer than rim depth @ 16" o/c). All other components and structural elements supporting the floor system such as beams, walls, columns, and oundation walls and footings including anchorage of components and bracing for lateral stability are the responsibility of Others.

Hatch area represents ceramic tiled floor with an additional dead load

The framing shown on this layout may deviate from the architectural and structural drawings. Project Engineer to review and approve the deviation pric to construction.

ARCHITECTURAL DRAWINGS:

JARDIN DESIGN GROUP INC. 64 Jardin Dr., Suite 3A, Vaughan, ON Date: Rev.1; May 22,2018 Project No: 17-55 Model: Liana 2

Builder's Project **Kott Lumber Company**

14 Anderson Blvd Stouffville, Ontario 2 10dx1 1/2 Canada L4A 7X4 905-642-4400

Job Path

Member

fasteners

16 16d

fasteners

4 10dx1 1/2

46 16d

D:\Users\rochavillo\WORK FROM HOME\GREEN YORK HOMES \GRANELLI HOME CORP\MODELS **LIANA 2/LIANA 2 ELEV 1/FLOOR**

Second Floor

Design Method LSD Building Code NBCC 2010 / OBC 2012

Floor Loads Live Dead Deflection Joist LL Span L/ 480 TL Span L/ 360 LL Cant 2L/ 480

40

15

360

360

240

480

360

OSB

5/8"

Deflection Girder LL Span L/ TL Span L/ LL Cant 2L/

TL Cant 2L/

TL Cant 2L/ Decking

Deck Thickness Fastener Nailed & Glued

Vibration

Ceiling: Gypsum 1/2"



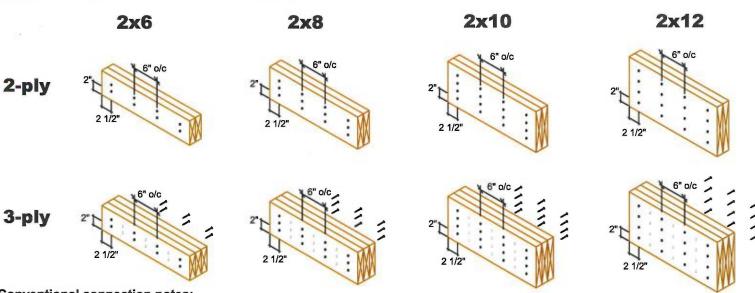


INEUU 10-UZ I

IPLE MEMBER CONNECTIONS

HOME CORP-LIANA 2 (ELEV.1&2

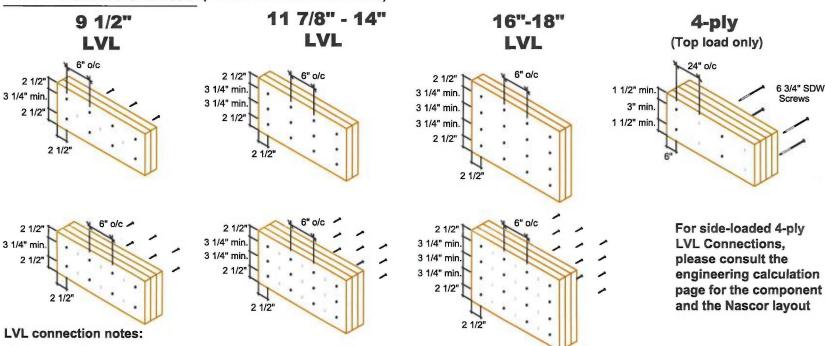
Conventional Connections (for uniform distributed loads)



Conventional connection notes:

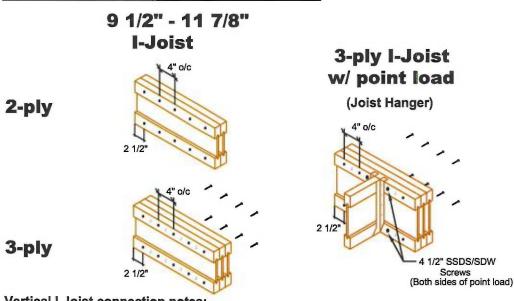
- -Nails to be 3" 10d spiral wire nails.
- -Nails to be located a minimum of 2" from the top and bottom of the member. Start all nails a minimum of 2 1/2" in from ends.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail driven from the opposite side.

LVL Connections (for uniform distributed loads)



- -Nails to be 3 1/2" spiral wire nails. -Nails to be located a minimum of 2 1/2" from the top and bottom of the member. Start all nails a minimum of 2 1/2" in from ends.
- -Minimum 3 1/4" spacing between rows.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail or screw driven from the opposite side.

Vertical I-Joist Connections (for uniform distributed loads)



Vertical I-Joist connection notes:

- -Nails to be 3" spiral wire nails.
- -Nails to be located at centre of top and bottom flanges. Start all nails a minimum of 2 1/2" in from ends.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail driven from the opposite side.

