Ground Floor

Layout Name

Design Method

LSD Description

Created May 30, 2018 Builder

Sales Rep

RM Designer RCO Shipping

Project

Builder's Project

14 Anderson Blvd Stouffville, Ontario Canada L4A 7X4 905-642-4400 Job Path

Kott Lumber Company

D:\Users\rochavillo\WORK FROM HOME\GREEN YORK HOMES

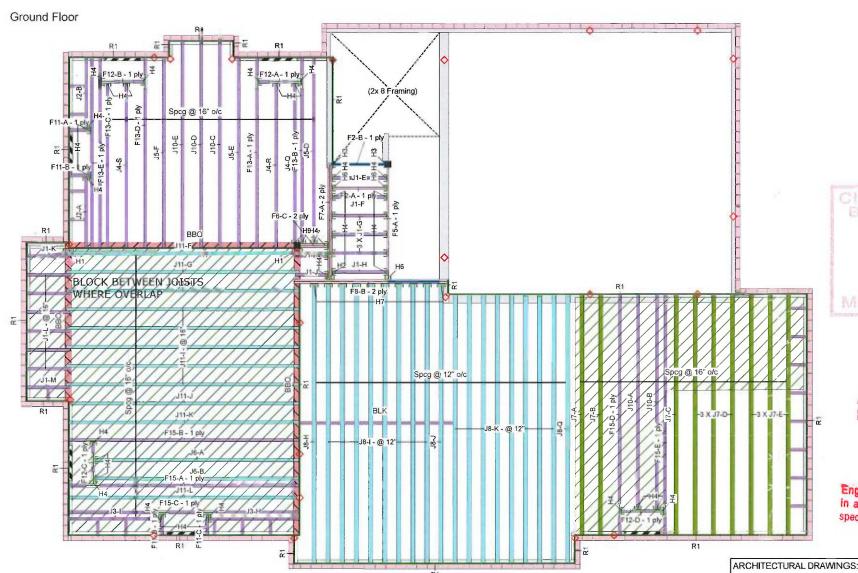
\GRANELLI HOME CORP\MODELS NOT 1 (CELESTIAL 2 ELEV.1)\FLOOR

Nailed & Glued

LOT 1 (CELESTIAL 2 EL.1)

GRANELLI HOME CORP. BRAMPTON, ONT.

GREEN YORK HOMES



OROFESSION4

T.L. WISE

100083566

December 17, 2018

CITY OF BRAMPTON MAR 15 7mg

> All work shall conform to th Building Code O. Reg. 332/12 as ar

in accordance with the supplier's layout and specifications forming part of the permit drawlr

JARDIN DESIGN GROUP INC. 64 Jardin Dr., Suite 3A, Vaughan, ON Date: Rev.5; Aug.30,2018 Project No: 17-55 Model: Celestial 2

Legend



Load from Above Wall Wall Opening Norbord Rimboard Plus 1.125 X 11.875 LPI 20Plus 11.875 NJ40U 11.875 NJ60H 11 875 NJ60U 11.875 Forex 2.0E-3000Fb LVL 1.75 X 11.875

- 1. OBC 2012 O.Reg 332/12 as amended
- 2. Nascor CCMC 13535-R
- 3. LVL CCMC -14056-R
- 4. CAN/CSA-086-09
- 5. CCMC -12787-R APA PR-L310(C)

Engineered floor joists shall be installed

- Framer to verify dimensions on the architectural drawings. 2. Double joist only require filler/backer ply when supporting another member using a face-mounted hanger.
- . Install 2x4 blocking @ 24" o/c under parallel non-load bearing walls.
- . Install single-ply flush window header along inside face of rimboard/rimjoist.
 5. Refer to Nascor specifier guide for installation works.
- Squash blocks recommended to be installed at end bearing on all first level joists which support loading from above exceeding two levels floor or roof.
- . Load transfer blocks to be installed under all point loads.
- 8. It shall be the framer's responsibility that floor joists and beams are fastened as per the hanger manufacturer's standards.

Refer to Multiple Member Connection Detail to ply to ply nailing or bolting requirements.

Rim parallel to joists: 1-1/8" rimboard with 2"x 4" block (1/16" longer than rim depth @ 16" o/c). All other components and structural elements supporting the floor system such as beams, walls, columns, and foundation walls and footings including anchorage of components and

Hatch area represents ceramic tiled floor with an addtional dead load

and structural drawings. Project Engineer to review and approve the deviation prior

_	Description	Width	Depth	Qty	Plies	Pcs	Length
F8	Forex 2.0E-3000Fb LVL	1.75	11.875	1	2	2	12-0-0
F7	Forex 2.0E-3000Fb LVL	1.75	11.875	1	2	2	10-0-0
F5	Forex 2.0E-3000Fb LVL	1.75	11.875			1	10-0-0
F2	Forex 2.0E-3000Fb LVL	1.75	11.875			2	4-0-0
F6	Forex 2.0E-3000Fb LVL	1.75	11.875	1	2	2	4-0-0
Joist (Flush)			<u> </u>			8
Label	Description	Width	Depth	Qty	Plies	Pcs	Length
F15	LPI 20Plus	2.5	11.875			5	18-0-0
F13	LPI 20Plus	2.5	11.875			5	14-0-0
F12	LPI 20Plus	2.5	11.875			4	4-0-0
F11	LPI 20Plus	2.5	11.875			4	2-0-0
J10	LPI 20Plus	2.5	11.875			5	16-0-0
J5	LPI 20Plus	2.5	11.875			3	14-0-0
J4	LPI 20Plus	2.5	11.875			4	12-0-0
J3	LPI 20Plus	2.5	11.875			2	8-0-0
J2	LPI 20Plus	2.5	11.875			2	6-0-0
J1	LPI 20Plus	2.5	11.875			16	4-0-0
J7	NJ40U	3.5	11.875			9	18-0-0
J11	NJ60H	2.5	11.875			11	18-0-0
J6	NJ60H	2.5	11.875			2	16-0-0
J8	NJ60U	3.5	11.875			19	20-0-0
Rim Bo	ard						
Label	Description	Width	Depth	Qty	Plies	Pcs	Length
R1	Norbord Rimboard Plus 1.125 X 11.875	1.125	11.875			17	12
3lockin	g						
Label	Description	Width	Depth	Qty	Plies	Pcs	Length
BLK1,	LPI 20 Plus	2.5	11.875	LinFt		Varies	29-0-0

anger					Beam/Girder	Supported	ILOT 1 (CELES	TIAL 2 EL.1).isl
Label Pcs C H1 2 L H2 2 H H3 2 H				Dealth Olidei	Member	Ground Floo	r	
Label	Pcs	Description	Skew	Slope	fasteners	fasteners	Design Method	LSD
H1	2	Unknown Hanger					Building Code	NBCC 2010 / OBC 2012
H2	2	HGUS410			46 16d	16 16d	Floor	
НЗ	2	HUCQ1.81/9-					Loads	
75.		SDS					Live	40
H4	40	LT251188			4 10dx1 1/2	2 10dx1 1/2	Dead	15
H6	3	HUS1.81/10			30 16d	10 16d	Deflection Jois	t
H7	10	LT351188			4 10dx1 1/2	2 10dx1 1/2	LL Span L/	480
H9	1	LT251188					TL Span L/	360
OTES:							II Cant 21/	480

	Floor	
	Loads	
	Live	40
	Dead	15
_	Deflection Joist	
	LL Span L/	480
-	TL Span L/	360
	LL Cant 2L/	480
	TL Cant 2L/	360
	Deflection Girder	
	LL Span L/	360
	TL Span L/	240
	LL Cant 2L/	480
	TL Cant 2L/	360
	Decking	
	Deck	SPF Plywood
	Thickness	3/4"

Fastener

Vibration

bracing for lateral stability are the responsibility of Others.

The framing shown on this layout may deviate from the architectural

columns and foundation walls and footings including anchorage of components and bracing for lateral stability are the responsibility of others.

This certification is to confirm that:

multiple member connection detail.

assembly shown on this layout.

layout.

approved components conform to the floor

2. The floor joists comply with the Nascor span

table for the loads and spacing shown on this

1. The loads used in the calculation of the attached

The floor system must be assembled in accordance

to the Nascor Specifier Guide. Multi-ply members

supporting the floor system such as beams, walls,

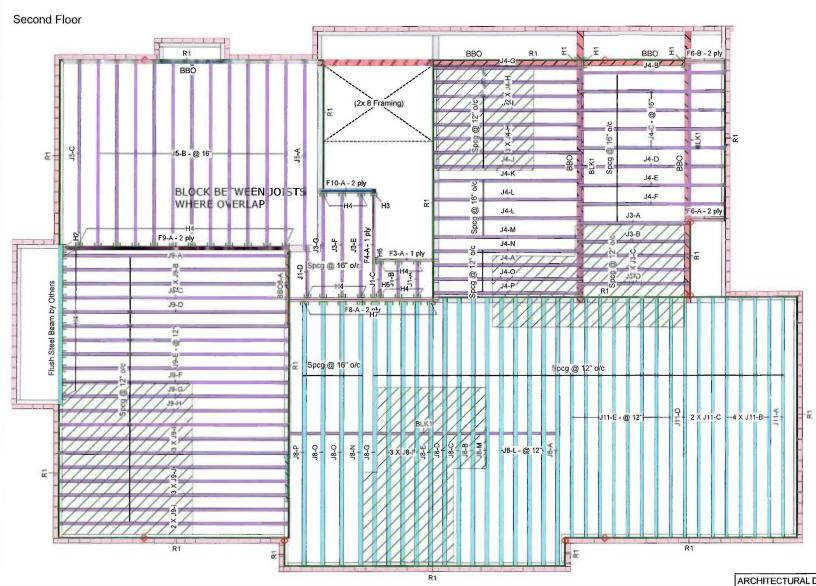
must be attached together as per the included

All other components and structural elements

Version 18.40.162 Powered by iStruct¹

This layout is to be used as an installation guide only. It is meant to be used in conjunction with the architectural and structural drawings, not to replace them

19-447131.000.00. RR. KOTT



This certification is to confirm that:

1. The loads used in the calculation of the attached approved components conform to the floor assembly shown on this layout.

2. The floor joists comply with the Nascor span table for the loads and spacing shown on this layout.

The floor system must be assembled in accordance to the Nascor Specifier Guide. Multi-ply members must be attached together as per the included multiple member connection detail. All other components and structural elements supporting the floor system such as beams, walls, columns and foundation walls and footings including anchorage of components and bracing for lateral stability are the responsibility of others.



ARCHITECTURAL DRAWINGS:

JARDIN DESIGN GROUP INC. 64 Jardin Dr., Suite 3A, Vaughan, ON Date: Rev.5; Aug.30,2018 Project No: 17-55 Model: Celestial 2

Legend



Load from Above Norbord Rimboard Plus 1.125 X 11.875 LPI 20Plus 11.875 NJ60H 11.875

NJ60U 11.875 Forex 2.0E-3000Fb LVL 1.75 X 9.5 (Dropped) Forex 2.0E-3000Fb LVL 1.75 X 11.875

- 1. OBC 2012 O.Reg 332/12 as amended
- 2. Nascor CCMC 13535-R
- 3. LVL CCMC -14056-R
- 4. CAN/CSA-086-09
- 5. CCMC -12787-R APA PR-L310(C)

	L (Flush)	1 A E al 41-	Dankle	Ohi	Dline	Doc	Longit	-
	Description	Width	Depth	Qty	Plies	Pcs	Length	4
F9	Forex 2.0E-3000Fb LVL	1.75	11.875	1	2	2	18-0-0	-
F8	Forex 2.0E-3000Fb LVL	1.75	11.875	1	2	2	12-0-0	
F4	Forex 2.0E-3000Fb LVL	1.75	11.875			1	8-0-0	
F10	Forex 2.0E-3000Fb LVL	1.75	11.875	1	2	2	6-0-0	
F3	Forex 2.0E-3000Fb LVL	1.75	11.875			1	6-0-0	1
F6	Forex 2.0E-3000Fb LVL	1.75	11.875	2	2	4	4-0-0	1
LVL/LS	L (Dropped)							1
	Description	Width	Depth	Qty	Plies	Pcs	Length	1
BBO8	Forex 2.0E-3000Fb LVL	1.75	9.5	1	2	2	6-0-0	1
Joist (Flush)							7
Label	Description	Width	Depth	Qty	Plies	Pcs	Length	7
J9	LPI 20Plus	2.5	11.875			20	18-0-0	٦
J5	LPI 20Plus	2.5	11.875			13	14-0-0	1
J4	LPI 20Plus	2.5	11.875			24	12-0-0	1
J3	LPI 20Plus	2.5	11.875			8	8-0-0	1
J1	LPI 20Plus	2.5	11.875			4	4-0-0]
J11	NJ60H	2.5	11.875			16	18-0-0	}
J8	NJ60U	3.5	11.875			18	20-0-0	╛
Rim Bo								4
Label	Description	Width	Depth	Qty	Plies	Pcs	Length	
R1	Norbord Rimboard Plus 1.125 X 11.875	1.125	11.875			19	12	
Blockin	g							7
Label	Description	Width	Depth	Qty	Plies	Pcs	Length	1
Label			11.875			Varies	33-0-0	1

					Bearin Olider	Member
Label	Pcs	Description	Skew	Stope	fasteners	fasteners
H1	3	Unknown Hanger				
H2	1	HGUS410			46 16d	16 16d
НЗ	1	HUCQ1.81/9- SDS				
H4	36	LT251188			4 10dx1 1/2	2 10dx1 1/2
H6	2	HUS1.81/10			30 10dx1 1/2	10 16d
H7	9	LT351188			4 10dx1 1/2	2 10dx1 1/2

- Framer to verify dimensions on the architectural drawings.
- . Double joist only require filler/backer ply when supporting another member using a face-mounted hanger.
- 3. Install 2x4 blocking @ 24" o/c under parallel non-load bearing walls. 4. Install single-ply flush window header along inside face of
- rimboard/rimjoist.

 Refer to Nascor specifier guide for installation works.
- . Squash blocks recommended to be installed at end bearing on all first level joists which support loading from above exceeding two levels floor or roof.
- . Load transfer blocks to be installed under all point loads. 3. It shall be the framer's responsibility that floor joists and beams are fastened as per the hanger manufacturer's standards.

Refer to Multiple Member Connection Detail to ply to ply nailing or bolting requirements.

Rim parallel to joists: 1-1/8" rimboard with 2"x 4" block (1/16" longer than rim depth @ 16" o/c). All other components and structural elements supporting the floor system such as beams, walls, columns, and foundation walls and footings including anchorage of components and oracing for lateral stability are the responsibility of Others.

Hatch area represents ceramic tiled floor with an additional dead load of 5 PSF

The framing shown on this layout may deviate from the architectural and structural drawings. Project Engineer to review and approve the deviation prior to construction.

AASCO	
Layout Name	-
LOT 1 (CELESTIAL 2 EL.1)	
Design Method	
LCD	

Description GRANELLI HOME CORP. BRAMPTON, ONT.

Created May 30, 2018

Builder GREEN YORK HOMES Sales Rep

RM Designer **RCO**

Shipping Project

Builder's Project Kott Lumber Company

14 Anderson Blvd Stouffville, Ontario Canada L4A 7X4 905-642-4400

Job Path

D:\Users\rochavillo\WORK FROM HOME\GREEN YORK HOMES IGRANELLI HOME CORPIMODELS \LOT 1 (CELESTIAL 2 ELEV.1)\FLOO \LOT 1 (CELESTIAL 2 EL.1).isl

Second Floor
Design Method LSD
Building Code NBCC 2010 / OBC 2012
Floor
Loads
Live 40
Dead 15
Deflection Joist
LL Span L/ 360

TL Span L/ 360 LL Cant 2L/ 480 TL Cant 2L/ 360 Deflection Girder LL Span L/ 360 240 TL Span L/ 480 LL Cant 2L/ TL Cant 2L/ 360 Decking

SPF Plywood Deck Thickness 5/8" Fastener Nailed & Glued

Vibration Gypsum 1/2" Ceiling:

Engineering Note Page (ENP-2)

REVISION 2018-10-17

Please read all notes prior to installation of the component

DESIGN INFORMATION

This building component is certified as an individual component for the loads and conditions shown on the calculation and drawing page.

The responsibility of the undersigned engineer is <u>only</u> limited to the calculation of this building component for the loads and conditions shown on this drawing.

The responsibility of the undersigned is limited to the verification of the structural capacity of the floor joists and LVL beams based on placement as shown on the layout. The loads applied are limited to the gravity effects of the specified loads. The structural integrity of the building and the effect of wind, uplift, seismic, lateral or other forces, calculation of adequate support and anchorage of components, as well as the dimensions and design loads used to calculate components are the responsibility of the overall building designer.

Floor joists and OSB rim board are designed to carry uniformly distributed loads only. Point loads should be transferred through the floor cavity with transfer blocks. Structural elements such as walls, posts, connectors, and transfer blocks are the responsibility of the overall building designer.

The undersigned engineer disclaims any responsibility for damages as a result of being furnished faulty or incorrect information, specifications and/or designs.

Installation of floor joists is to be carried out in accordance with the current edition of the manufacturer's literature available at http://www.kottgroup.com.

CODE

This building component is designed in accordance with the National Building Code of Canada, the Ontario Building Code, CCMC and Canadian Standards Association guidelines.

COMPONENT

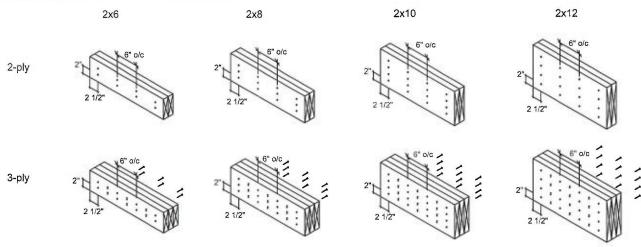
- 1. The building component used in construction must be the same as indicated on the drawings.
- 2. The building component must be installed and assembled as per specification shown on the drawing and in accordance with the manufacturer's assembly and installation.
- 3. Members consisting of multiple plies must be connected as per the document "Multi-ply Connection Details".
- 4. Pass-thru transfer block framing is required at all point loads over bearings.

HANDLING AND INSTALLATION

Do not drill any hole, cut or notch a certified building component without a written preauthorization.



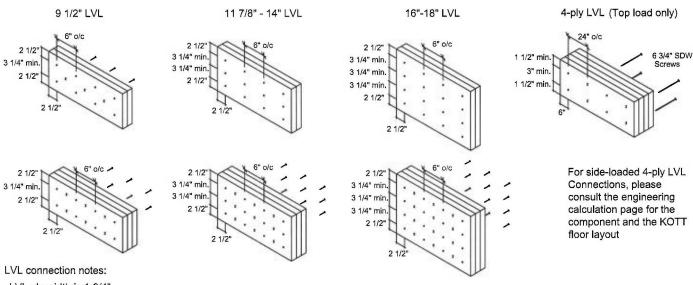
Conventional Connections



Conventional connection notes:

- -Nails to be 3" long wire nails.
- -Nails to be located 2" min. from the top and bottom of the member. Start all nails 2 1/2" min. from ends.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail driven from the opposite side.

LVL Connections



- -LVL ply width is 1-3/4"
- -Nails to be 3 1/2" common wire nails.
- -Nails to be located 2 1/2" min. from the top and bottom of the member. Start all nails 2 1/2" min. from ends.
- -Minimum 3 1/4" spacing between rows.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail or screw driven from the opposite side.

Multiple Member Connections

All connections are for uniformly distributed loads.

For multi-ply connections of I-joists, refer to Manufacturer's Installation Guide



KOTT Inc. 3228 Moodie Drive Ottawa, ON K2H 7V1 613-838-2775



Client:

GREEN YORK HOMES

Project: Address:

Date: 12/14/2018

Designer: RCO

Job Name: LOT 1 (CELESTIAL 2 EL.1)

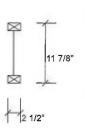
Project #:

LPI 20Plus 11.875" - PASSED

...... ...

Level: Ground Floor





Page 1 of 1

Member Info	rmation			Unfactored Reactions UNPATTERNED Ib (Uplift)						
Type:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind		
Plies:	1	Design Method:	LSD	1	61	23	0	0		
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	114	42	0	0		
Deflection LL:	360	Load Sharing:	No	-						
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal	Vibration:	Not Checked							
General Load										
Floor Live:	40 PSF			Bearings	s and Fac	tored Reactions				
Dead:	15 PSF			Bearing	Length	Cap. React D/L lb	Total Ld. Case	Ld. Cc		

ŀ	Analysis Res	ults					
Γ	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
l	Moment	42 ft-lb	11 7/16"	6250 ft-lb	0.007 (1%)	1.25D+1.5L	L
l	Shear	211 lb	1'4 3/8"	2345 lb	0.090 (9%)	1.25D+1.5L	L
l	Perm Defl in.	0.000 (L/999)	0	999,000 (L/0)	0.000 (0%)		
l	LL Defl inch	0.000 (L/36074)	11 5/16"	0.041 (L/360)	0.010 (1%)	L	L
l	TL Defl inch	0.001	11 5/16"	0.061 (L/240)	0.010 (1%)	D+L	L

Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1 - SPF 2.375" 7% 1.25D+1.5L 29 / 92 120 L 1.25D+1.5L 2.000" 14% 53 / 171 224 L Hanger

T.L. WISE 100083566 VCE OF ONTAR

December 17, 2018

						stability.
1	Provide	restraint a	supports	to ensure	lateral	stability.

- 2 Dead Load Deflection: Instant = 0.000", Long Term = 0.000"
- 3 Fill all hanger nailing holes.

Design Notes

- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top flange braced at bearings.
- 6 Bottom flange braced at bearings.

(L/26279)

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-5-10	(Span)3-2-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	1-2-0		Far Face	30 lb	81 lb	0 lb	0 lb	J2

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

Copyright 2016 All rights reserved by Louisiana Pacific Corp. 414 Union St Suite 2000, Nashville, TN 37219

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

This design is valid until 10/31/2020

Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325 www.lpcorp.com CCMC: 12412-R APA: PR-L238C





Client:

Project: Address: GREEN YORK HOMES

12/14/2018 Date:

RCO

Designer: Job Name: LOT 1 (CELESTIAL 2 EL.1)

Project #:

2.000"

Hanger

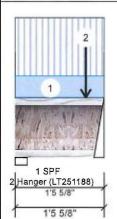
14%

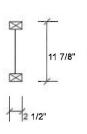
52 / 167

219 L

11.875" - PASSED LPI 20Plus

Level: Ground Floor





1.25D+1.5L

Page 1 of 1

Member Info	rmation			Unfactore	ed React	ions UN	PATTERNI	ED lb (Uplift)	
Туре:	Girder	 Application:	Floor (Residential)	Brg	Live		Dead	Snow	Wind
Plies:	1	Design Method:	LSD	1	61		23	0	0
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	112		42	0	0
Deflection LL:	360	Load Sharing:	No	1					
Deflection TL:	240	Deck:	Not Checked						
Importance:	Normal	Vibration:	Not Checked						
General Load									
Floor Live:	40 PSF			Bearings	and Fact	tored Re	actions		
Dead:	15 PSF			Bearing !	Length	Cap. R	eact D/L lb	Total Ld. Case	Ld. Comb.
				1 - SPF 2	2.375"	7%	28 / 91	119 L	1.25D+1.5L

Analysis Results										
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case				
Moment	41 ft-lb	11 5/16"	6250 ft-lb	0.007 (1%)	1.25D+1.5L	L				
Shear	206 lb	1'4 3/8"	2345 lb	0.088 (9%)	1.25D+1.5L	L				
Perm Defl ir	n. 0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)						
LL Defl inch	0.000 (L/36734)	11 3/16"	0.041 (L/360)	0.010 (1%)	L	L				
TL Defl inch	0.001	11 3/16"	0.061 (L/240)	0.010 (1%)	D+L	L				

T.L. WISE 100083566 100083566

December 17, 2018

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection; Instant = 0.000", Long Term = 0.000"
- 3 Fill all hanger nailing holes.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top flange braced at bearings.
- 6 Bottom flange braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-5-10	(Span)3-2-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	1-2-0		Near Face	29 lb	78 lb	0 lb	0 lb	J2

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

Copyright 2016 All rights reserved by Louisiana Pacific Corp. 414 Union St Suite 2000, Nashville, TN 37219

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

This design is valid until 10/31/2020

Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325 www.lpcorp.com CCMC: 12412-R APA: PR-L238C







Client: Project:

Address:

GREEN YORK HOMES

Date:

12/14/2018

Page 1 of 1

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Designer: RCO

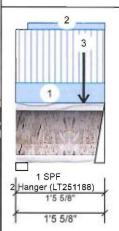
Job Name: LOT 1 (CELESTIAL 2 EL.1)

Project #:

LPI 20Plus

11.875" - PASSED

Level: Ground Floor



11 7/8

Member Information						
Туре:	Girder	Application				
Plies:	1	Design Me				
Moisture Condition:	Dry	Building C				
Deflection LL:	360	Load Shar				
Deflection TL:	240	Deck:				
Importance:	Normal	Vibration:				
General Load						
Floor Live:	40 PSF					
Dead:	15 PSF					

		_
tion:	Floor (Residential)	1
Method:	LSD	1
Code:	NBCC 2010 / OBC 2012	1
haring:	No	1
	Not Checked	
n:	Not Checked	-
		-

Brg	Live	Dead	Snow	Wind
1	68	32	0	0
2	127	63	0	0

			2 - Hang
Capacity	Comb.	Case	
0.009 (1%)	1.25D+1.5L	L	
0.109 (11%)	1.25D+1.5L	L	
0.010 (1%)	D	Uniform	
0.010 (1%)	L	L	
0.010 (1%)	D+L	L	

Bearings and Factored Reactions Cap. React D/L lb Total Ld. Case Ld. Comb. Bearing Length 1 - SPF 2.375" 9% 41 / 102 143 L 1.25D+1.5L 2.000" 17% 79 / 191 269 L 1.25D+1.5L ger

(L/18862) **Design Notes**

Analysis Results

Perm Defl in. 0.000

LL Defl inch 0.001

TL Defl inch 0.001

Actual

59 ft-lb

255 lb

(L/56853)

(L/28228)

Analysis

Moment

Shear

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0,000", Long Term = 0,000"
- 3 Fill all hanger nailing holes.
- 4 Girders are designed to be supported on the bottom edge only.

Location Allowed

1'4 3/8" 2345 lb

1' 3/4" 6250 ft-lb

1' 3/8" 0.041 (L/360) 0.010 (1%) D

1'7/16" 0.041 (L/360) 0.010 (1%) L

1' 3/8" 0.061 (L/240) 0.010 (1%) D+L

5 Top flange braced at bearings.

						flange braced at bearings.	6 Bottom
Snow	Live	Dead	Side	Trib Width	Location	Load Type	ID
0 PSF	40 PSF	15 PSF	Тор	(Span)3-2-8	0-0-0 to 1-5-10	Tie-In	1
0 PLF	0 PLF	8 PLF	Тор		0-2-6 to 1-5-10	Part, Uniform	2
0 lb	101 lb	50 lb	Near Face		1-1-7	Point	3
0 PLF	0 PLF	8 PLF	Тор	(Span)3-2-8	0-2-6 to 1-5-10	Part. Uniform	



December 17, 2018

0 PLF Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended, This analysis is valid only for the product listed.

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This design is valid until 10/31/2020

Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325 www.lpcorp.com CCMC: 12412-R APA: PR-L238C

Wind

0 PSF

Comments

Kott Lumber Company 14 Anderson Blvd, Ontario 14A 7X4





Client:

Address:

GREEN YORK HOMES

Project:

12/14/2018 Date:

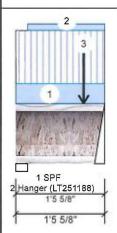
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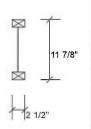
Job Name: LOT 1 (CELESTIAL 2 EL.1)

LPI 20Plus

11.875" - PASSED

Level: Ground Floor





1.25D+1.5L 1.25D+1.5L

Page 1 of 1

Member Info	rmation			Unfactore	ed Reactio	ns UNPATTERN	ED lb (Uplift)	
Туре:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind
Plies:	1	Design Method:	LSD	1	69	33	0	0
Moisture Condit	ion: Dry	Building Code:	NBCC 2010 / OBC 2012	2	129	65	0	0
Deflection LL:	360	Load Sharing:	No					
Deflection TL:	240	Deck:	Not Checked					
Importance:	Normal	Vibration:	Not Checked					
General Load								
Floor Live:	40 PSF			Bearings	and Facto	red Reactions		
Dead:	15 PSF			Bearing L	Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb

1 - SPF 2.375"

2.000"

2 -

Hanger

9%

17%

41 / 103

81 / 194

Analysis	Results
----------	---------

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	60 ft-lb	1' 7/8"	6250 ft-lb	0.010 (1%)	1.25D+1.5L	L
Shear	261 lb	1'4 3/8"	2345 lb	0.111 (11%)	1.25D+1.5L	L
Perm Defl in.	0.000 (L/55408)	1' 1/2"	0.041 (L/360)	0.010 (1%)	D	Uniform
LL Defl inch	0.001 (L/27690)	1' 1/2"	0.041 (L/360)	0.010 (1%)	L	L
TL Defl inch	0.001 (L/18463)	1' 1/2"	0.061 (L/240)	0.010 (1%)	D+L	L

T.L. WISE 100083566 100083566 December 17, 2018

144 L

275 L

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.000", Long Term = 0.000"
- 3 Fill all hanger nailing holes.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top flange braced at bearings.
- 6 Bottom flange braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind Comments	
1	Tie-In	0-0-0 to 1-5-10	(Span)3-2-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Part, Uniform	0-2-6 to 1-5-10		Тор	8 PLF	0 PLF	0 PLF	0 PLF	
3	Point	1-1-7		Far Face	52 lb	104 lb	dl 0	Pass-Thru Framing Squash Block is required at all point loads over bearing	

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

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This design is valid until 10/31/2020

Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325 www.lpcorp.com CCMC: 12412-R APA: PR-L238C





Client:

GREEN YORK HOMES

Project: Address:

12/14/2018

RCO

Designer: Job Name: LOT 1 (CELESTIAL 2 EL.1)

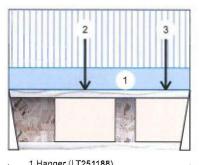
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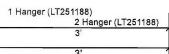
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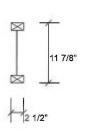
LPI 20Plus

11.875" - PASSED

Level: Ground Floor







Page 1 of 1

Member miloti	lation
Туре:	Girder
Plies:	1
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	
Floor Live:	40 PSF

15 PSF

Mambar Information

Application: Floor (Residential) Design Method: Building Code: NBCC 2010 / OBC 2012 Load Sharing: Not Checked Vibration: Not Checked

Live Wind Brg Dead Snow 256 96 0 0 1 366 137 2 0 0

Unfactored Reactions UNPATTERNED Ib (Uplift)

Analysis Results Analysis Actual Location Allowed Capacity Comb. 529 ft-lb Moment 1'3 1/8" 6250 ft-lb 0.085 (8%) 1.25D+1.5L L Shear 714 lb 2'10 3/4" 2345 lb 0.304 (30%) 1.25D+1.5L L Perm Defl in. 0.002 1'3 1/8" 0.093 (L/360) 0.020 (2%) D Uniform (L/15127) LL Defl inch 0.006 (L/5660) 1'3 1/8" 0.093 (L/360) 0.060 (6%) L TL Defl inch 0.008 (L/4119) 1'3 1/8" 0.140 (L/240) 0.060 (6%) D+L

Bearings and Factored Reactions

Bear	ring Length	Cap. F	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - Han	2.000" ger	32%	120 / 384	504	L	1.25D+1.5L
2 - Han	2.000" ger	45%	171 / 550	721	L	1.25D+1.5L



Dead:

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.002", Long Term = 0.003"
- 3 Fill all hanger nailing holes.
- 4 See manufacture installation guide note E4 for installation details
- 5 Girders are designed to be supported on the bottom edge only.
- 6 Top flange braced at bearings.

7 Bottom flange braced at bearings.



December 17, 2018

ı	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
١	1	Tie-In	0-0-0 to 3-0-0	(Span)1-8-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
l	2	Point	1-3-2		Near Face	116 lb	310 lb	0 lb	0 lb	J4
	3	Point	2-7-2		Near Face	78 lb	209 lb	0 lb		J4 Fhru Framing Squash Block is ed at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

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This design is valid until 10/31/2020

Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325 www.lpcorp.com CCMC: 12412-R APA: PR-L238C





Client:

GREEN YORK HOMES

Project: Address:

Date:

12/14/2018

Designer: RCO Job Name: LOT 1 (CELESTIAL 2 EL.1)

LPI 20Plus 11.875" - PASSED evel: Ground Floor

Unfactored Reactions UNPATTERNED lb (Uplift)

Dead

137

97

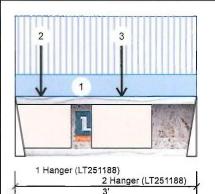
Live

366

259

1

2



3'

Member Information

11 7/8"

Wind

n

Page 1 of 1

Type:	Girder	Α
Plies:	1	D
Moisture Condition:	Dry	В
Deflection LL:	360	Le
Deflection TL:	240	D
Importance:	Normal	V
General Load		
Floor Live:	40 PSF	
Dead:	15 PSF	10

Application: Floor (Residential) Design Method: LSD Building Code: NBCC 2010 / OBC 2012 oad Sharing: No Not Checked Deck: /ibration: Not Checked

Bearings	s and Fa	ctored F	Reactions			
Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - Hanger	2.000"	45%	171 / 549	720	L	1.25D+1.5L
2 - Hanger	2.000"	32%	121 / 389	510	L	1.25D+1.5L

Snow

0

0

T.L. WISE

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NCE OF ON

December 17, 2018

Analysis Results Analysis Actual Location Allowed Capacity Comb. Case 1'9 1/8" 6250 ft-lb Moment 527 ft-lb 0.084 (8%) 1.25D+1.5L L Shear 713 lb 1 1/4" 2345 lb 0.304 (30%) 1.25D+1.5L L Perm Defl in. 0.002 1'9 1/8" 0.093 (L/360) 0.020 (2%) D Uniform (L/15183) LL Defl inch 0.006 (L/5673) 1'9 1/8" 0.093 (L/360) 0.060 (6%) L TL Defl inch 0.008 (L/4130) 1'9 1/8" 0.140 (L/240) 0.060 (6%) D+L

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.002", Long Term = 0.003"
- 3 Fill all hanger nailing holes.
- 4 See manufacture installation guide note E4 for installation details
- 5 Girders are designed to be supported on the bottom edge only.
- 6 Top flange braced at bearings.
- 7 Bottom flange braced at bearings

	bottom liange braced at bearings.								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 3-0-0	(Span)1-8-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	0-5-2		Near Face	80 lb	214 lb	0 lb	0 lb	J4
3	Point	1-9-2		Near Face	115 lb	308 lb	0 16		Thru Framing Squash Block is red at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

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This design is valid until 10/31/2020

Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325 www.lpcorp.com CCMC: 12412-R APA: PR-L238C Kott Lumber Company 14 Anderson Blvd, Ontario Canada





Address:

GREEN YORK HOMES

Project:

Date: 12/14/2018

RCO Designer:

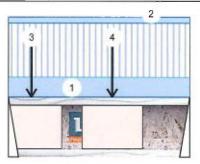
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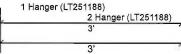
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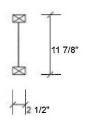
LPI 20Plus

11.875" - PASSED

Level: Ground Floor







Page 1 of 1

Memb	er	Inf	or	m	a	ti	or	١
				_	_			

Туре:	Girder
Plies:	1
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	
Floor Live:	40 PSF
Dead:	15 PSF

Application: Floor (Residential) Design Method: **Building Code:** NBCC 2010 / OBC 2012 Load Sharing: No Deck: Not Checked Not Checked Vibration:

451 1 2 302

Brg

Wind

0

0

Bearings and Factored Reactions

Live

Unfactored Reactions UNPATTERNED Ib (Uplift)

Dead

219

147

bearing:	s and rac	toreu Ke	eactions				
Bearing	Length	Cap. F	React D/L lb	Total	Ld. Case	Ld. Comb.	
1 - Hanger	2.000"	60%	274 / 677	950	L	1.25D+1.5L	
2 - Hanger	2.000"	40%	184 / 453	637	L	1.25D+1.5L	

Snow

0

0

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	693 ft-lb	1'8 9/16"	6250 ft-lb	0.111 (11%)	1.25D+1.5L	L
Shear	943 lb	1 1/4"	2345 lb	0.402 (40%)	1.25D+1.5L	L
Perm Defl in.	0.004 (L/9546)	1'8 9/16"	0.093 (L/360)	0.040 (4%)	D	Uniform
LL Defl inch	0.007 (L/4632)	1'8 9/16"	0.093 (L/360)	0.080 (8%)	L	L
TL Defl inch	0.011 (L/3119)	1'8 9/16"	0.140 (L/240)	0.080 (8%)	D+L	L

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.004", Long Term = 0.005"
- 3 Fill all hanger nailing holes.
- 4 See manufacture installation guide note E4 for installation details
- 5 Girders are designed to be supported on the bottom edge only.
- 6 Top flange braced at bearings.

7 Bottom flange braced at bearings



December 17, 2018

- 1		bracea at bearings.								
	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
	1	Tie-In	0-0-0 to 3-0-0	(Span)1-8-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	2	Part. Uniform	0-0-0 to 3-0-0		Тор	4 PLF	0 PLF	0 PLF	0 PLF	
ı	3	Point	0-4-9		Near Face	126 lb	260 lb	0 lb	0 lb	J6
	4	Point	1-8-9		Near Face	189 lb	390 lb	0 lb		ruFraming Squash Block is at all point loads over bearings

Refer to Multiple Member Connection

Detail for ply to ply nailing or bolting requirements

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This design is valid until 10/31/2020

Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325 www.lpcorp.com CCMC: 12412-R APA: PR-L238C





isDesign[™]

Client:

Address:

GREEN YORK HOMES

Project:

Date:

12/14/2018

RCO Designer:

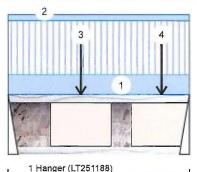
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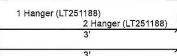
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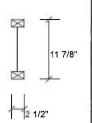
LPI 20Plus

11.875" - PASSED

Level: Ground Floor







Page 1 of 1

Member Information							
Girder	Application:	Floor (Residential)					
1	Design Method:	LSD					
Dry	Building Code:	NBCC 2010 / OBC 201:					
360	Load Sharing:	No					
240	Deck:	Not Checked					
Normal	Vibration:	Not Checked					
40 PSF							
15 PSF							
	Girder 1 Dry 360 240 Normal	Girder Application: 1 Design Method: Dry Building Code: Acod Sharing: Deck: Vibration: 40 PSF					

Applic	cation:	Floor (Residential)
Desig	n Method:	LSD
Buildí	ng Code:	NBCC 2010 / OBC 2012
Load	Sharing:	No
Deck:		Not Checked
Vibrat	tion:	Not Checked

Wind	Snow	Dead	Live	Brg
0	0	159	325	1
0	О	218	450	2

E	Bearings and Factored Reactions											
	Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.					
	1 - Hanger	2.000"	43%	198 / 487	686	L	1.25D+1.5L					
	2 - Hanger	2.000"	60%	273 / 676	948	L	1.25D+1.5L					

Analysis Results Analysis Actual Location Allowed Capacity Comb. 701 ft-lb 1'2 7/16" 6250 ft-lb 0.112 (11%) 1.25D+1.5L L Moment 2'10 3/4" 2345 lb 941 lb 0.401 (40%) 1.25D+1.5L L Shear Perm Defl in. 0.004 (L/9387) 1'2 7/16" 0.093 (L/360) 0.040 (4%) D Uniform LL Defl inch 0.007 (L/4578) 1'2 7/16" 0.093 (L/360) 0.080 (8%) L 1'2 7/16" 0.140 (L/240) 0.080 (8%) D+L TL Defl inch 0.011 (L/3077)

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.004", Long Term = 0.005"
- 3 Fill all hanger nailing holes.
- 4 See manufacture installation guide note E4 for installation details
- 5 Girders are designed to be supported on the bottom edge only.
- 6 Top flange braced at bearings.

7 Bottom flange braced at bearings

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T.L. WISE 100083566
TOUNCE OF ONTRE

December 17, 2018

7 Bottom	flange braced at bearings.								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 3-0-0	(Span)1-8-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Part, Uniform	0-0-0 to 3-0-0		Тор	4 PLF	0 PLF	0 PLF	0 PLF	
3	Point	1-2-7		Far Face	191 lb	391 lb	0 lb	dl 0	J10
4	Point	2-6-7		Far Face	135 lb	281 lb	0 lb		ru¹Framing Squash Block is I at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

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This design is valid until 10/31/2020

Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325 www.lpcorp.com CCMC: 12412-R APA: PR-L238C





Client:

GREEN YORK HOMES

Project: Address:

12/14/2018

RCO Designer:

Job Name: LOT 1 (CELESTIAL 2 EL.1)

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Page 1 of 1

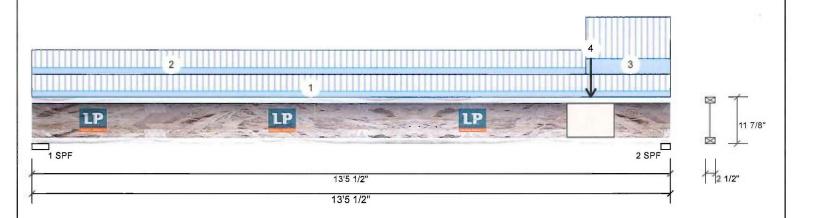
Project #:

Date:

LPI 20Plus

11.875" - PASSED

Level: Ground Floor



Member Inforn	nation			Unfacto	red Reac	tions U	NPATTERN	ED lb ((Uplift)
Type:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Sno	w
Plies:	1	Design Method:	LSD	1	397		149		0
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012	2	642		241		0
Deflection LL:	360	Load Sharing:	No						
Deflection TL:	240	Deck:	Not Checked						
Importance:	Normal	Vibration:	Not Checked	1					
General Load									
Floor Live:	40 PSF			Bearings	s and Fac	tored	Reactions		
Dead:	15 PSF			Bearing	Length	Сар.	React D/L lb	Total	Ld. Case
				1 - SPF	4.250"	43%	186 / 596	782	L
				2 - SPF	2.375"	77%	301 / 963	1265	L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2688 ft-lb	7'5 3/8"	6250 ft-lb	0.430 (43%)	1.25D+1.5L	L
Shear	1240 lb	13'3 7/8"	2345 lb	0.529 (53%)	1.25D+1.5L	L
Perm Defl in.	0.060 (L/2623)	7' 3/8"	0.434 (L/360)	0.140 (14%)	D	Uniform
LL Defl inch	0.159 (L/984)	7' 3/8"	0.434 (L/360)	0.370 (37%)	L	L
TL Defl inch	0.219 (L/715)	7' 3/8"	0.652 (L/240)	0.340 (34%)	D+L	L

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.060", Long Term = 0,089"
- 3 See manufacture installation guide note E4 for installation details
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top flange must be laterally braced at a maximum of 6'4" o.c.

6 Bottom flange braced at bearings.



Wind 0

Ld. Comb. 1.25D+1.5L 1.25D+1.5L

December 17, 2018

								Pass-Thr	ru Framing Squash
4	Point	11-9-6		Near Face	96 lb	256 lb	0 lb	0 lb	F12
3	Tie-In	11-8-2 to 13-5-8	(Span)3-2-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 11-8-2	(Span)1-4-6	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
1	Tie-In	0-0-0 to 13-5-8	(Span)1-3-10	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments

sh Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the revoked listed.

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This design is valid until 10/31/2020

Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325 www.lpcorp.com CCMC: 12412-R APA: PR-L238C





Client:

GREEN YORK HOMES

Project: Address:

Date: 12/14/2018

Designer: RCO

Job Name: LOT 1 (CELESTIAL 2 EL.1)

Project #:

2.000"

Hanger

2 - SPF 2.375"

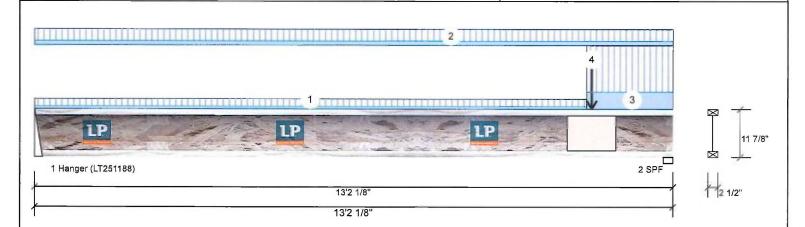
28%

71%

105 / 337

276 / 883

LPI 20Plus 11.875" - PASSED Level: Ground Floor



Member Info	rmation			Unfactored	Reaction	ns UNPATTERNI	ED lb (Uplift	t)
Туре:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	1
Plies:	1	Design Method:	LSD	1	225	84	0	
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	589	221	0	
Deflection LL:	360	Load Sharing:	No					
Deflection TL:	240	Deck:	Not Checked					
Importance:	Normal	Vibration:	Not Checked					
General Load								
Floor Live:	40 PSF			Bearings ar	nd Factor	ed Reactions		
Dead:	15 PSF			Bearing Le	ngth (Cap. React D/L lb	Total Ld. Ca	ase

Analysis Res	sults					
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1815 ft-lb	8'5 1/16"	6250 ft-lb	0.290 (29%)	1.25D+1.5L	L
Shear	1138 lb	13' 1/2"	2345 lb	0.485 (49%)	1.25D+1.5L	L
Perm Defl in.	0.039 (L/3941)	7' 15/16"	0.431 (L/360)	0.090 (9%)	D	Uniform
LL Defl inch	0.105 (L/1477)	7' 15/16"	0.431 (L/360)	0.240 (24%)	L	L

TL Defl inch 0.145 (L/1074) 7' 15/16" 0.647 (L/240) 0.220 (22%) D+L

T.L. WISE 100083566

442 L

1159 L

Wind 0 0

Ld. Comb.

1.25D+1.5L

1.25D+1.5L

Page 1 of 1

December 17, 2018

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.039", Long Term = 0.059"
- 3 Fill all hanger nailing holes.

Design Notes

- 4 See manufacture installation guide note E4 for installation details
- 5 Girders are designed to be supported on the bottom edge only.
- 6 Top flange must be laterally braced at a maximum of 7'7" o.c.

7 Bottom flange braced at bearings.

l	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
l	1	Tie-In	0-0-0 to 11-4-12	(Span)0-6-2	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
l	2	Tie-In	0-0-0 to 13-2-2	(Span)0-9-14	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
١	3	Tie-In	11-4-12 to 13-2-2	(Span)3-2-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
I	4	Point	11-6-0		Far Face	137 lb	366 lb			rtն tPraming Squash Block is at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product based. product listed.

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This design is valid until 10/31/2020

Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325 www.lpcorp.com CCMC: 12412-R APA: PR-L238C





Client:

GREEN YORK HOMES

Project: Address:

Date: Designer: 12/14/2018

RCO

Job Name: LOT 1 (CELESTIAL 2 EL.1)

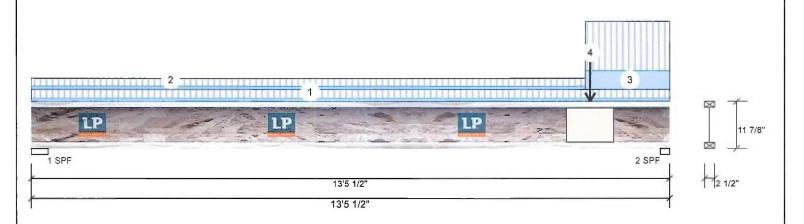
Page 1 of 1

Project #:

LPI 20Plus

11.875" - PASSED

Level: Ground Floor



Type: Girder Application: Floor (Residential) Plies: Design Method: Moisture Condition: Dry Building Code: NBCC 2010 / OBC 2012 Deflection LL: Load Sharing: 360 No Deflection TL: Not Checked 240 Deck: Importance: Vibration: Not Checked Normal General Load Floor Live: 40 PSF Dead: 15 PSF

Unfactored Reactions UNPATTERNED Ib (Uplift)

Br	g Live	Dead	Show	vviria	
1	198	74	0	0	
2	558	209	0	0	

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	4.250"	21%	93 / 297	389	L	1.25D+1.5L
2 - SPF	2.375"	67%	261 / 837	1098	L	1.25D+1.5L

Analysis Results

Member Information

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1648 ft-lb	9' 1/2"	6250 ft-lb	0.264 (26%)	1.25D+1.5L	L
Shear	1078 lb	13'3 7/8"	2345 lb	0.460 (46%)	1.25D+1.5L	L
Perm Defl in.	0.036 (L/4358)	7'4 1/2"	0.434 (L/360)	0.080 (8%)	D	Uniform
LL Defl inch	0.096 (L/1633)	7'4 1/2"	0.434 (L/360)	0.220 (22%)	L	L
TL Defl inch	0.132 (L/1188)	7'4 1/2"	0.652 (L/240)	0.200 (20%)	D+L	L

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.036", Long Term = 0.054"
- 3 See manufacture installation guide note E4 for installation details
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top flange must be laterally braced at a maximum of 7'11" o.c.

6 Bottom flange braced at bearings.

Analysis	Actual	Location	Allowed	Capacity	Comp.	Case
Moment	1648 ft-lb	9' 1/2"	6250 ft-lb	0.264 (26%)	1.25D+1.5L	L
Shear	1078 lb	13'3 7/8"	2345 lb	0.460 (46%)	1.25D+1.5L	L
Perm Defl in.	0.036 (L/4358)	7'4 1/2"	0.434 (L/360)	0.080 (8%)	D	Uniform
LL Defl inch	0.096 (L/1633)	7'4 1/2"	0.434 (L/360)	0.220 (22%)	L	L
TL Defl inch	0.132 (L/1188)	7'4 1/2"	0.652 (L/240)	0.200 (20%)	D+L	L
Design Notes						

T.L. WISE 100083566 100083566 WCE OF ONTP

December 17, 2018

								Pagg-Th	ru Framing	S
4	Point	11-9-6		Near Face	137 lb	366 lb	0 lb	0 lb	F12	
3	Tie-In	11-8-2 to 13-5-8	(Span)3-2-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
2	Tie-In	0-0-0 to 11-8-2	(Span)0-6-6	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
1	Tie-In	0-0-0 to 13-5-8	(Span)0-6-12	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments	

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

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This design is valid until 10/31/2020

Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325 www.lpcorp.com CCMC: 12412-R APA: PR-L238C Kott Lumber Company 14 Anderson Blvd, Ontario Canada





Client:

GREEN YORK HOMES

Project: Address:

Date:

12/14/2018

RCO

Designer: Job Name: LOT 1 (CELESTIAL 2 EL.1) Page 1 of 1

1.25D+1.5L

Project #:

2 - SPF 2.375"

78%

303 / 968

1271 L

T.L. WISE 100083566

100083566

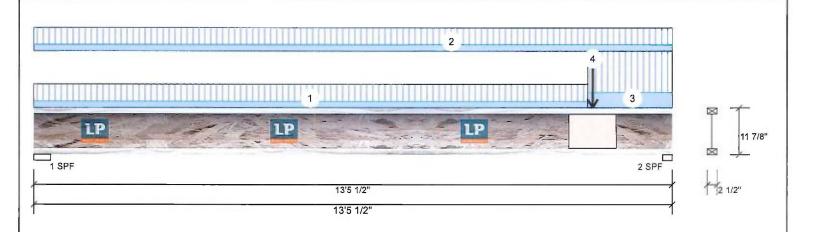
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December 17, 2018

LPI 20Plus

11.875" - PASSED

Level: Ground Floor



Member Info	rmation			Unfacto	red Reac	tions U	NPATTERN	ED lb (Uplift)	
Туре:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snov	N	Wind
Plies:	1	Design Method:	LSD	1	398		149		0	0
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	646		242		0	0
Deflection LL:	360	Load Sharing:	No							
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal	Vibration:	Not Checked							
General Load										
Floor Live:	40 PSF			Bearing	s and Fac	tored I	Reactions			
Dead:	15 PSF			Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.
				1 - SPF	4.250"	43%	186 / 596	783	L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2693 ft-lb	7'5 7/16"	6250 ft-lb	0.431 (43%)	1.25D+1.5L	L
Shear	1247 lb	13'3 7/8"	2345 lb	0.532 (53%)	1.25D+1.5L	L
Perm Defl in.	0.060 (L/2618)	7' 3/8"	0.434 (L/360)	0.140 (14%)	D	Uniform
LL Defl inch	0.159 (L/982)	7' 3/8"	0.434 (L/360)	0.370 (37%)	L	L
TL Defl inch	0.219 (L/714)	7' 3/8"	0.652 (L/240)	0.340 (34%)	D+L	L

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.060", Long Term = 0.090"
- 3 See manufacture installation guide note E4 for installation details
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top flange must be laterally braced at a maximum of 6'4" o.c.

6 Bottom	flange braced at bearing	S.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 11-8-2	(Span)1-4-2	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 13-5-8	(Span)1-3-14	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Tie-In	11-8-2 to 13-5-8	(Span)3-2-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
4	Point	11-9-6		Far Face	97 lb	259 lb	0.lb	0.lb	E12

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

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This design is valid until 10/31/2020



Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325 www.lpcorp.com CCMC: 12412-R APA: PR-L238C





Client:

GREEN YORK HOMES

Project:

Address:

Date: 12/14/2018

RCO Designer:

Job Name: LOT 1 (CELESTIAL 2 EL.1)

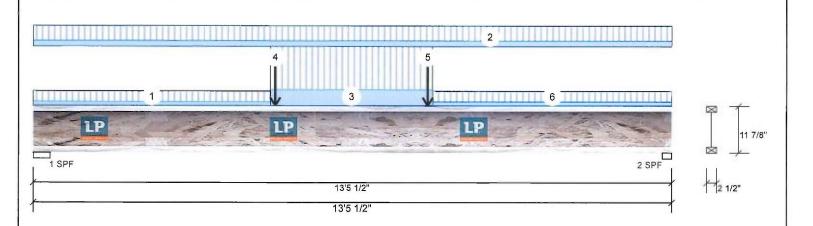
Project #:

2 - SPF 2.375"

LPI 20Plus

11.875" - PASSED

Level: Ground Floor



Member Inform	nation			Unfactored Reactions UNPATTERNED Ib (Uplift)					
Туре:	Girder	Application:	Floor (Residential)	Brg	Live	9	Dead	Snov	N
Plies:	1	Design Method:	LSD	1	288		108		0
Moisture Condition	: Dry	Building Code:	NBCC 2010 / OBC 2012	2	281		105		0
Deflection LL:	360	Load Sharing:	No	_					
Deflection TL:	240	Deck:	Not Checked						
Importance:	Normal	Vibration:	Not Checked						
General Load									
Floor Live:	40 PSF			Bearings	and Fac	tored l	Reactions		
Dead:	15 PSF			Bearing	Length	Cap.	React D/L lb	Total	Ld. Case
				1 - SPF	4.250"	31%	135 / 432	567	L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2362 ft-lb	6'9 13/16"	6250 ft-lb	0.378 (38%)	1.25D+1.5L	L
Shear	556 lb	3 1/2"	2345 lb	0.237 (24%)	1.25D+1.5L	L
Perm Defl in.	0,050 (L/3138)	6'9 5/8"	0.434 (L/360)	0.110 (11%)	D	Uniform
LL Defl inch	0.133 (L/1172)	6'9 5/8"	0.434 (L/360)	0.310 (31%)	L	L
TL Defl inch	0.183 (L/853)	6'9 5/8"	0.652 (L/240)	0.280 (28%)	D+L	L

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.050", Long Term = 0.075"
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top flange must be laterally braced at a maximum of 6'9" o.c.

5 Bottom flange braced at bearings

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T.L. WISE 100083566
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1 3 KILL 18 18 18 18 18 18 18 18 18 18 18 18 18
Documber 17 2019
December 17, 2018

131 / 422

Wind

0 PSF 0 PSF 0 PSF 0 lb F11

Comments

Wind 0 0

Ld. Comb. 1.25D+1.5L

1.25D+1.5L

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Page 1 of 1

1	o bottom hang	ge braced at bearings.						
ı	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow
ı	1	Tie-In	0-0-0 to 5-0-2	(Span)0-4-14	Тор	15 PSF	40 PSF	0 PSF
I	2	Tie-In	0-0-0 to 13-5-8	(Span)0-6-12	Тор	15 PSF	40 PSF	0 PSF
	3	Tie-In	5-0-2 to 8-5-2	(Span)1-7-7	Тор	15 PSF	40 PSF	0 PSF
I	4	Point	5-1-6		Far Face	42 lb	112 lb	0 lb
1	5	Point	8-3-14		Far Face	42 lb	114 lb	0 lb
ı	6	Tie-In	8-5-2 to 13-5-8	(Span)0-4-14	Тор	15 PSF	40 PSF	0 PSF

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

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This design is valid until 10/31/2020

Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325 www.lpcorp.com CCMC: 12412-R APA: PR-L238C





Client:

Project: Address: **GREEN YORK HOMES**

12/14/2018 Date:

Designer: RCO

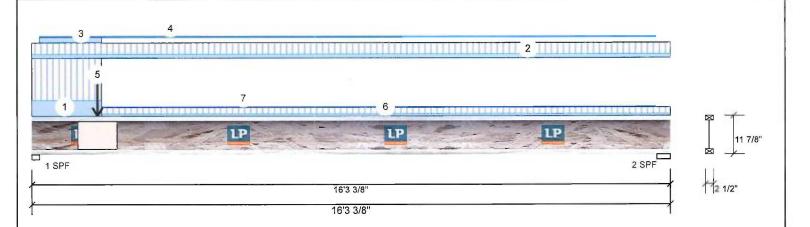
Job Name: LOT 1 (CELESTIAL 2 EL.1)

Project #:

LPI 20Plus

11.875" - PASSED

Level: Ground Floor



Unfactored Reactions UNPATTERNED lb (Uplift) Member Information Application: Floor (Residential) Live Dead Snow Type: Plies: Design Method: LSD 715 347 0 1 Moisture Condition: Dry Building Code: NBCC 2010 / OBC 2012 0 268 129 2 Deflection LL: 360 Load Sharing: No Deflection TL: 240 Deck: Not Checked Vibration: Not Checked Importance: Normal General Load Bearings and Factored Reactions 40 PSF Floor Live: 15 PSF Cap. React D/L lb Total Ld. Case Ld. Comb. Dead: Bearing Length 1 - SPF 2.375" 92% 434 / 1072 1506 L 2 - SPF 4.125" 31% 162 / 401 563 L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2674 ft-lb	6'3"	6250 ft-lb	0.428 (43%)	1.25D+1.5L	L
Shear	1484 lb	1 5/8"	2345 lb	0.633 (63%)	1.25D+1.5L	L
Perm Defl in.	0.101 (L/1881)	7'6 7/8"	0.529 (L/360)	0.190 (19%)	D	Uniform
LL Defl inch	0.208 (L/917)	7'6 7/8"	0.529 (L/360)	0.390 (39%)	L	L
TL Defl inch	0.309 (L/616)	7'6 7/8"	0.793 (L/240)	0.390 (39%)	D+L	L

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.101", Long Term = 0.152"
- 3 See manufacture installation guide note E4 for installation details
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top flange must be laterally braced at a maximum of 6'5" o.c.

6 Bottom flange braced at bearings

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T.L. WISE 100083566
Sul 180
December 17 2018

Wind

0

1.25D+1.5L

1.25D+1.5L

Page 1 of 1

December 17, 2018

O DOLLOTTI	iange braceu at bearing	J							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-9-6	(Span)3-2-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 16-3-6	(Span)0-10-3	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Part. Uniform	0-2-6 to 1-9-6		Тор	8 PLF	0 PLF	0 PLF	0 PLF	
4	Part. Uniform	0-2-6 to 15-10-15		Тор	2 PLF	0 PLF	0 PLF	0 PLF	
5	Point	1-8-2		Far Face	219 lb	451 lb	0 lb		ry Framing Squash Block is I at all point loads over bearings
6	Tie-In	1-9-6 to 16-3-6	(Span)0-5-13	Тор	15 PSF	40 PSF	0 PSF	0 PSF	at all politioaus over bearings
7	Part. Uniform	1-9-6 to 15-10-15		Тор	1 PLF	0 PLF	0 PLF		Multiple Member Connection

ember Connection Detail for ply to ply nailing or bolting

requirements

Notes

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

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This design is valid until 10/31/2020

Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325 www.lpcorp.com CCMC: 12412-R APA: PR-L238C Kott Lumber Company 14 Anderson Blvd, Ontario Canada L4A 7X4 905-642-4400





Client: Project:

Address:

GREEN YORK HOMES

Date:

12/14/2018

Designer: RCO

Job Name: LOT 1 (CELESTIAL 2 EL.1)

2 - SPF 4.125"

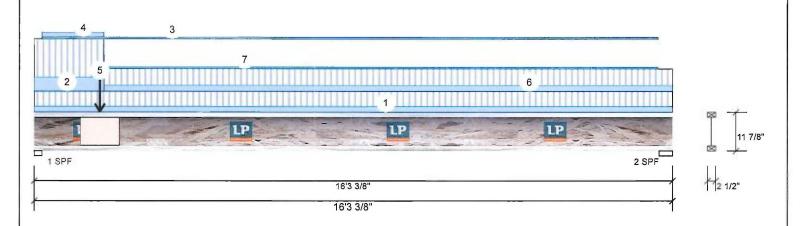
54%

284 / 706

LPI 20Plus

11.875" - PASSED

Level: Ground Floor



Member Information Unfactored Reactions UNPATTERNED Ib (Uplift) Type: Girden Application: Floor (Residential) Brg Live Dead Snow Plies: Design Method: LSD 765 372 0 1 Moisture Condition: Dry Building Code: NBCC 2010 / OBC 2012 471 227 0 2 Deflection LL: 360 Load Sharing: Deflection TL: 240 Not Checked Importance: Vibration: Not Checked Normal General Load Floor Live: 40 PSF Bearings and Factored Reactions Dead: 15 PSF Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1 - SPF 2.375" 99% 465 / 1147 1612 L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	4104 ft-lb	7'5 1/2"	6250 ft-lb	0.657 (66%)	1.25D+1.5L	L
Shear	1588 lb	1 5/8"	2345 lb	0.677 (68%)	1.25D+1.5L	L
Perm Defl in.	0.155 (L/1227)	7'10 5/16"	0.529 (L/360)	0.290 (29%)	D	Uniform
LL Defl inch	0.318 (L/598)	7'10 5/16"	0.529 (L/360)	0.600 (60%)	L	L
TL Defl inch	0.473 (L/402)	7'10 5/16"	0,793 (L/240)	0.600 (60%)	D+L	L

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.155", Long Term = 0,233"
- 3 See manufacture installation guide note E4 for installation details
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top flange must be laterally braced at a maximum of 5'1" o.c.

6 Bottom flange braced at bearings.



990 L

Wind

0

0

1.25D+1.5L

1.25D+1.5L

. - - - - . . . -

Page 1 of 1

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind Comments
1	Tie-In	0-0-0 to 16-3-6	(Span)1-3-5	Тор	15 PSF	40 PSF	0 PSF	0 PSF
2	Tie-In	0-0-0 to 1-9-6	(Span)3-2-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF
3	Part, Uniform	0-2-6 to 15-11-0		Тор	3 PLF	0 PLF	0 PLF	0 PLF
4	Part. Uniform	0-2-6 to 1-9-6		Тор	8 PLF	0 PLF	0 PLF	0 PLF
5	Point	1-8-2		Near Face	147 lb	302 lb	0 lb	Pass Thru Framing Squash Block is
6	Tie-In	1-9-6 to 16-3-6	(Span)1-4-11	Тор	15 PSF	40 PSF	0 PSF	required at all point loads over bearings 0 PSF
7	Part. Uniform	1-9-6 to 15-11-0		Тор	3 PLF	0 PLF	0 PLF	Refer to Multiple Member Connection Detail for ply to ply nailing or bolting

Notes

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This design is valid until 10/31/2020

Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325 www.lpcorp.com CCMC: 12412-R APA: PR-L238C

requirements





Client:

GREEN YORK HOMES

Project: Address:

Date: 12/14/2018

RCO

Designer:

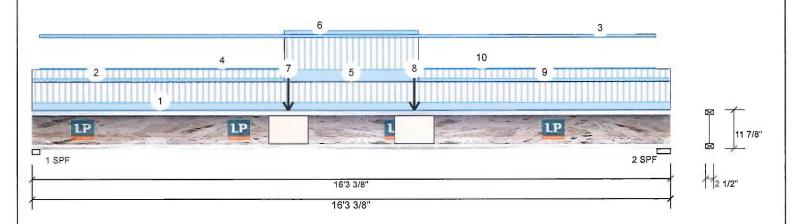
Job Name: LOT 1 (CELESTIAL 2 EL.1)

Project #:

LPI 20Plus 11.875" - PASSED

. .. _ _ _ _ _ _ _ _ _ _ _ _ _

Level: Ground Floor



Member Information Type: Plies: Moisture Condition: Dry Deflection LL: 360 Deflection TL: 240 Importance: Normal General Load 40 PSF Floor Live: 15 PSF

Application: Design Method: Building Code:

Deck:

Vibration:

Floor (Residential) LSD NBCC 2010 / OBC 2012

Load Sharing: No

Not Checked Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	408	206	0	0
2	416	209	0	0

Bearings and Factored Reactions

Bearing Length	Cap. Rea	act D/L lb	Total	Ld. Case	Ld. Comb.	
1 - SPF 2.375"	53%	257 / 612	869	L	1.25D+1.5L	
2 - SPF 4.125"	48%	261 / 624	885	L	1,25D+1.5L	

Analysis Results

Dead:

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	4338 ft-lb	8' 5/8"	6250 ft-lb	0.694 (69%)	1.25D+1.5L	L
Shear	868 lb	16'	2345 lb	0.370 (37%)	1.25D+1.5L	L
Perm Defl in.	0.159 (L/1194)	8' 7/8"	0.529 (L/360)	0.300 (30%)	D	Uniform
LL Defl inch	0.316 (L/603)	8' 15/16"	0.529 (L/360)	0.600 (60%)	L	L
TL Defl inch	0.475 (L/400)	8' 15/16"	0.793 (L/240)	0.600 (60%)	D+L	L

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.159", Long Term = 0.239"
- 3 See manufacture installation guide note E4 for installation details
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top flange must be laterally braced at a maximum of 4'11" o.c.

6 Bottom flange braced at bearings.



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Page 1 of 2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments	
1	Tie-In	0-0-0 to 16-3-6	(Span)1-0-9	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
2	Tie-In	0-0-0 to 6-5-6	(Span)0-5-7	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
3	Part. Uniform	0-2-6 to 15-10-15		Тор	3 PLF	0 PLF	0 PLF	0 PLF		
4	Part. Uniform	0-2-6 to 6-5-6		Тор	1 PLF	0 PLF	0 PLF	0 PLF		
5	Tie-In	6-5-6 to 9-10-6	(Span)1-7-7	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
6	Part. Uniform	6-5-6 to 9-10-6		Тор	4 PLF	0 PLF	0 PLF	0 PLF		
7	Point	6-6-10		Near Face	65 lb	129 lb	0 lb	0 lb	F11	

Continued on page 2...

Notes

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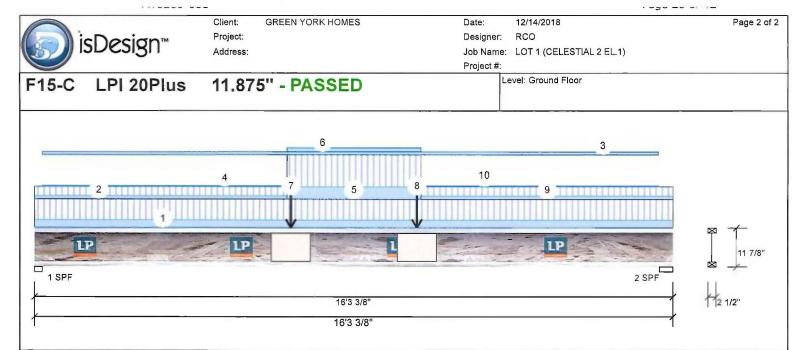
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Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325 www.lpcorp.com CCMC: 12412-R APA: PR-L238C Kott Lumber Company 14 Anderson Blvd, Ontario L4A 7X4 905-642-4400



This design is valid until 10/31/2020



ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
8	Point	9-9-2		Near Face	63 lb	127 lb	0 lb	0 lb	F11
9	Tie-In	9-10-6 to 16-3-6	(Span)0-5-7	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
10	Part. Uniform	9-10-6 to 15-10-14		Тор	1 PLF	0 PLF	0 PLF	0 PLF	

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

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Manufacturer Info

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This design is valid until

Client: Project:

Address:

GREEN YORK HOMES

12/14/2018 Date:

RCO

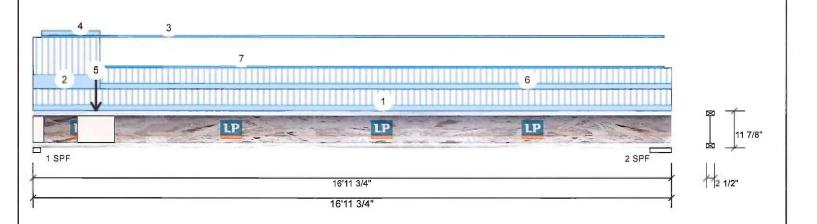
Designer: Job Name: LOT 1 (CELESTIAL 2 EL.1)

Project #:

11.875" - PASSED LPI 20Plus

Level: Ground Floor

Unfactored Reactions UNPATTERNED lb (Uplift)



Type:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snov	N	Wind
Plies:	1	Design Method:	LSD	1	802		390		0	0
Moisture Condition	n: Dry	Building Code:	NBCC 2010 / OBC 2012	2	497		241		0	0
Deflection LL:	360	Load Sharing:	No							
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal	Vibration:	Not Checked							
General Load				-						
Floor Live:	40 PSF			Bearings	and Fact	ored F	Reactions			
Dead:	15 PSF			Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.
				1 - SPF	2.375"	79%	488 / 1203	1690	L	1.25D+1.5L
				2 - SPF	6.875"	57%	301 / 745	1046	L	1.25D+1.5L

Analysis Results

Member Information

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	4358 ft-lb	7'8"	6250 ft-lb	0.697 (70%)	1.25D+1.5L	L
Shear	1666 lb	1 5/8"	2345 lb	0.710 (71%)	1.25D+1.5L	L
Perm Defl in.	0.174 (L/1129)	8'1 1/16"	0,544 (L/360)	0.320 (32%)	D	Uniform
LL Defl inch	0.356 (L/551)	8'1 1/16"	0.544 (L/360)	0.650 (65%)	L	L
TL Defl inch	0.530 (L/370)	8'1 1/16"	0.817 (L/240)	0.650 (65%)	D+L	L

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.174", Long Term = 0.260"
- 3 See manufacture installation guide note E4 for installation details
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top flange must be laterally braced at a maximum of 4'10" o.c.
- 6 Bottom flange braced at bearings.
- 7 Web stiffeners required at Bearing 1

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Page 1 of 1

December 17, 2018

L	/ VVED Stilleliels	required at bearing	1.e							
I	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
l	1	Tie-In	0-0-0 to 16-11-12	(Span)1-4-5	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
l	2	Tie-In	0-0-0 to 1-9-6	(Span)3-2-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
l	3	Part. Uniform	0-2-10 to 16-9-6		Тор	3 PLF	0 PLF	0 PLF	0 PLF	
ł	4	Part. Uniform	0-2-12 to 1-9-6		Тор	8 PLF	0 PLF	0 PLF		ru Framing Squash Block is
I	5	Point	1-8-2		Near Face	159 lb	325 lb	0 lb	required	at all point loads over bearings
l	6	Tie-In	1-9-6 to 16-11-12	(Span)1-3-11	Тор	15 PSF	40 PSF	0 PSF	Referto	Multiple Member Connection
	7	Part. Uniform	1-9-6 to 16-9-6		Тор	3 PLF	0 PLF	0 PLF		r ply to ply nailing or bolting

Notes

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This design is valid until 10/31/2020

Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325 www.lpcorp.com CCMC: 12412-R APA: PR-L238C





Client:

GREEN YORK HOMES

Project:

Address:

Date: 12/14/2018

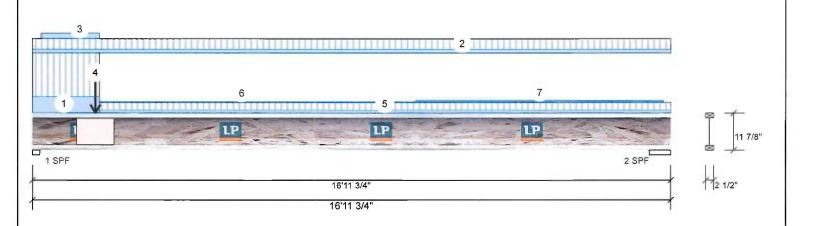
Designer: RCO

Job Name: LOT 1 (CELESTIAL 2 EL.1)

LPI 20Plus

11.875" - PASSED

Level: Ground Floor



Unfactored Reactions UNPATTERNED Ib (Uplift) Member Information Type: Girder Application: Floor (Residential) Live Dead Snow Plies: Design Method: LSD 719 335 0 1 Moisture Condition: Dry **Building Code:** NBCC 2010 / OBC 2012 278 129 0 2 Deflection LL: 360 Load Sharing: No Deflection TL: Not Checked 240 Deck: Importance: Normal Vibration: Not Checked General Load Floor Live: 40 PSF **Bearings and Factored Reactions** Dead: 15 PSF Cap. React D/L lb Total Ld. Case Ld. Comb. Bearing Length 1 - SPF 2.375" 91% 418 / 1078 1496 L 2 - SPF 6.875" 32% 161 / 417 578 L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case	
Moment	2709 ft-lb	6'6 1/4"	6250 ft-lb	0.433 (43%)	1.25D+1.5L	L	
Shear	1475 lb	1 5/8"	2345 lb	0.629 (63%)	1.25D+1.5L	L	
Perm Defl in.	0.103 (L/1907)	7'9 7/8"	0.544 (L/360)	0.190 (19%)	D	Uniform	
LL Defl inch	0.227 (L/864)	7'9 7/8"	0.544 (L/360)	0.420 (42%)	L	L	
TL Defl inch	0.330 (L/594)	7'9 7/8"	0.817 (L/240)	0.400 (40%)	D+L	L	

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.103", Long Term = 0.154"
- 3 See manufacture installation guide note E4 for installation details
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top flange must be laterally braced at a maximum of 6'4" o.c.

6 Bottom flange braced at bearings.

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December 17, 2018

Wind

0

0

1.25D+1.5L

1.25D+1.5L

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Page 1 of 1

December 17, 2018

O DOLLOIII II	ange bracea at bearing	, J.						
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind
1	Tie-In	0-0-0 to 1-9-6	(Span)3-2-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF
2	Tie-In	0-0-0 to 16-11-12	(Span)0-9-3	Тор	15 PSF	40 PSF	0 PSF	0 PSF
3	Part, Uniform	0-2-13 to 1-9-6		Тор	8 PLF	0 PLF	0 PLF	0 PLF
4	Point	1-8-2		Far Face	218 lb	450 lb	0 lb	0 lb
5	Tie-In	1-9-6 to 16-11-12	(Span)0-6-13	Тор	15 PSF	40 PSF	0 PSF	Pass Th
6	Part, Uniform	1-9-6 to 16-9-6		Тор	1 PLF	0 PLF	0 PLF	required 0 PLF
7	Part. Uniform	10-2-6 to 16-9-6		Тор	2 PLF	0 PLF	0 PLF	Referto

hru Framing Squash Block is d at all point loads over bearings

Comments

o Multiple Member Connection Detail for ply to ply nailing or bolting requirements

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This design is valid until 10/31/2020

Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325 www.lpcorp.com CCMC: 12412-R APA: PR-L238C Kott Lumber Company 14 Anderson Blvd, Ontario Canada





Client: Project:

Address:

GREEN YORK HOMES

Date: 12/14/2018

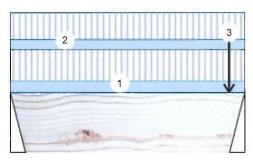
Designer: RCO

Job Name: LOT 1 (CELESTIAL 2 EL.1)

Forex 2.0E-3000Fb LVL

1.750" X 11.875" - PASSED

Level: Ground Floor



1 Hanger (HUS1.81/10) 2 Hanger (HUS1.81/10)

3'10 1/2"

3'10 1/2"



Page 1 of 1

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м	em	ber	Info	rmation

Туре:	Girder
Plies:	1
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	

Application: Design Method: Building Code:

Deck:

Vibration:

Load Sharing:

Floor (Residential) NBCC 2010 / OBC 2012

No

Not Checked Not Checked

Unfactored Reactions UNPATTERNED Ib (Uplift)

Live	Dead	Snow	Wind
62	33	0	0
580	266	0	0
	62	62 33	62 33 0

Floor Live:

Dead:

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case	
Moment	133 ft-lb	2'3 13/16"	17130 ft-lb	0.008 (1%)	1.25D+1.5L	L	
Unbraced	133 ft-lb	2'3 13/16"	12574 ft-lb	0.011 (1%)	1.25D+1.5L	L	
Shear	101 lb	2'8 3/8"	5798 lb	0.017 (2%)	1.25D+1.5L	L	
Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)			
LL Defl inch	0.001 (L/66968)	2'1 7/8"	0.117 (L/360)	0.010 (1%)	L	L	
TL Defl inch	0.001	2'1 3/4"	0.175 (L/240)	0.010 (1%)	D+L	L	

Analysis Results

40 PSF

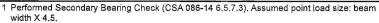
15 PSF

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Moment	133 ft-lb	2'3 13/16"	17130 ft-lb	0.008 (1%)	1.25D+1.5L	L
Unbraced	133 ft-lb	2'3 13/16"	12574 ft-lb	0.011 (1%)	1.25D+1.5L	L
Shear	101 lb	2'8 3/8"	5798 lb	0.017 (2%)	1,25D+1,5L	L
Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
LL Defl inch	0.001 (L/66968)	2'1 7/8"	0.117 (L/360)	0.010 (1%)	L	L
TL Defl inch	0.001 (L/44165)	2'1 3/4"	0.175 (L/240)	0.010 (1%)	D+L	L

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.	
1 - Hanger	3.000"	3%	42 / 94	136	L	1.25D+1.5L	
2 - Hanger	3.000"	64%	332 / 870	1202	L	1.25D+1.5L	





- 2 Fill all hanger nailing holes.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.

Load Type

Self Weight

Tie-In

Tie-In

Point



Location	Trib Width	Side	Dead	Live	Snow	
0-0-0 to 3-10-8	(Span)0-8-9	Тор	15 PSF	40 PSF	0 PSF	
0-0-0 to 3-10-8	(Span)0-7-7	Тор	15 PSF	40 PSF	0 PSF	
3-7-6		Top	242 lb	539 lb	0 lb	
			E D. E			

Pass-Thru Framing Squash Block is

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

required at all point loads over bearings

Comments

ID

1 2

3

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

- Handling & Installation
- LVL beams must not be out or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
- Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- For flat roofs provide proper drainage to prevent ponding

Manufacturer Info Forex APA: PR-L318

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT. This design is va

Kott Lumber Company 14 Anderson Blvd, Ontario Canada L4A 7X4 905-642-4400



Wind

0 PSF

0 PSF

Client: Project:

Address:

GREEN YORK HOMES

Date: 12/14/2018

RCO Designer:

Job Name: LOT 1 (CELESTIAL 2 EL.1)

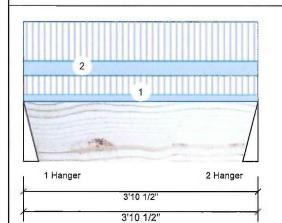
Project #:

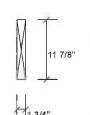
Forex 2.0E-3000Fb LVL

1.750" X 11.875" - PASSED

Brg

Level: Ground Floor





Wind

Page 1 of 1

Member Inforn	nation	
Туре:	Girder	Application:
Plies:	1	Design Method:
Moisture Condition:	Dry	Building Code:
Deflection LL:	360	Load Sharing:
Deflection TL:	240	Deck:
Importance:	Normal	Vibration:
General Load		577 = 1
Floor Live:	40 PSF	
Dead:	15 PSF	

Application:	Floor (Residential)
Design Method:	LSD
Building Code:	NBCC 2010 / OBC 2012
Load Sharing:	No
Deck:	Not Checked
Vibration:	Not Checked

1	113		32		Ų	U
2	115		52		0	0
Bearing	s and Fac	tored	Reactions			
Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
4	2.000	00/	00 / 470	000	1	4 050 14 51

Snow

Unfactored Reactions UNPATTERNED lb (Uplift)

Dead

Live

115

Analysis Res	sults					
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	188 ft-lb	1'11 1/4"	17130 ft-lb	0.011 (1%)	1.25D+1.5L	L
Unbraced	188 ft-lb	1'11 1/4"	12574 ft-lb	0.015 (1%)	1.25D+1.5L	L.
Shear	93 lb	1'2 1/8"	5798 lb	0.016 (2%)	1.25D+1.5L	L
Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
LL Defl inch	0.001 (L/45885)	1'11 1/4"	0.117 (L/360)	0.010 (1%)	L	L
TL Defl inch	0.001 (L/31535)	1'11 1/4"	0.175 (L/240)	0.010 (1%)	D+L	L

3.000" 1.25D+1.5L Hanger 3.000" 1.25D+1.5L 2 -6% 66 / 173 238 L Hanger

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.
- 4 Bottom braced at bearings.

T.L. WISE 100083566 100083566 INCE OF ONT December 17, 2018

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 3-10-8	(Span)0-11-11	Тор	15 P\$F	40 PSF	0 PSF	0 PSF	
2	Part, Uniform	0-0-0 to 3-10-8		Тор	15 PLF	40 PLF	0 PLF	0 PLF	
	Self Weight				5 PLF				

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

LIVL beams must not be cut or drilled.
Refer to manufacturer's product information requirements, multi-ply fastening details, beam strength values, and code approvals
Damaged Beams must not be used.
Design assumes top edge is laterally restrained.
Provide lateral support at beaming points to avoid lateral displacement and rotation.

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex APA: PR-L318

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Kott Lumber Company 14 Anderson Blvd, Ontario Canada L4A 7X4 905-642-4400



This design

Client: Project:

Address:

GREEN YORK HOMES

Date:

12/14/2018

RCO

Job Name: LOT 1 (CELESTIAL 2 EL.1)

Project #:

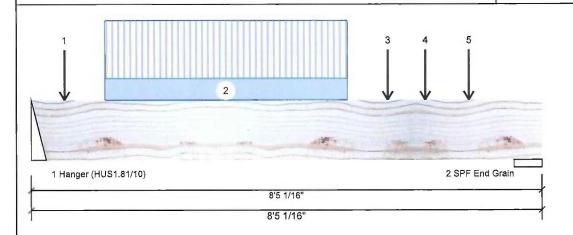
Designer:

Forex 2.0E-3000Fb LVL

. . . _ _ _ _ _ _ _ _ _ _ _ _

1.750" X 11.875" - PASSED

Level: Ground Floor



Page 1 of 1

Member Information

Type:	Girder
Plies:	1
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	
Floor Live:	40 PSF
Dead:	15 PSF

Application: Design Method:

Building Code:

Load Sharing:

Floor (Residential) LSD

NBCC 2010 / OBC 2012 No

Deck: Vibration:

Not Checked Not Checked

Unfactored Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	Wind
1	404	181	0	0
2	704	324	0	0

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2193 ft-lb	5'10 5/8"	17130 ft-lb	0.128 (13%)	1.25D+1.5L	L
Unbraced	2193 ft-lb	5'10 5/8"	5736 ft-lb	0.382 (38%)	1.25D+1.5L	L
Shear	1452 lb	7' 7/16"	5798 lb	0.250 (25%)	1.25D+1.5L	L
Perm Defl in.	0.013 (L/7131)	4'5 9/16"	0.261 (L/360)	0.050 (5%)	D	Uniform
LL Defl inch	0.029 (L/3217)	4'5 7/16"	0.261 (L/360)	0.110 (11%)	L	L
TL Defl inch	0.042 (L/2217)	4'5 1/2"	0.392 (L/240)	0.110 (11%)	D+L	L

Bearings and Factored Reactions

Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - Hanger	3.000"	21%	227 / 607	833	L	1.25D+1.5L
2 - SPF End Grain	5.500"	20%	405 / 1055	1460	L	1.25D+1.5L

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.
- 4 Bottom braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind
1	Point	0-6-10		Far Face	30 lb	79 lb	0 lb	0 lb
2	Part. Uniform	1-2-10 to 5-2-10		Far Face	29 PLF	77 PLF	0 PLF	0 PLF
3	Point	5-10-10		Far Face	28 lb	76 lb	0 lb	0 lb
4	Point	6-6-0		Far Face	266 lb	580 lb	0 lb	0 lb
5	Point	7-2-10		Far Face	25 lb	65 lb	0 lb	Pass-Th
	Self Weight				5 PLF			required



S-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

LVI. beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

approvers

Damaged Beams must not be used
Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is va

Manufacturer Info Forex APA: PR-L318

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



Project:

Address:

GREEN YORK HOMES

Date:

12/14/2018

Page 1 of 2

Designer: RCO

Job Name: LOT 1 (CELESTIAL 2 EL.1)

Project #:

Forex 2.0E-3000Fb LVL

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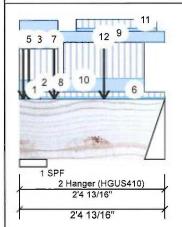
1.750" X 11.875"

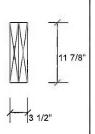
2-Ply - PASSED

Bra

1

Level: Ground Floor





Wind

0

0

1.25D+1.5L

Member Inform	ation
Туре:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	
Floor Live:	40 PSF
Dead:	15 PSF

Application: Floor (Residential) Design Method: LSD **Building Code:** NBCC 2010 / OBC 2012 Load Sharing: No Deck: Not Checked Not Checked Vibration:

Bearings	and Fa	actored I	Reactions				
Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.	
1 - SPF	5.375"	52%	1509 / 4187	5695	L	1.25D+1.5L	

Snow

0

0

Analysis Results Comb. Analysis Actual Location Allowed Capacity Case 375 ft-lb 1'4 3/4" 34261 ft-lb 0.011 (1%) 1.25D+1.5L L Moment 375 ft-lb 1'4 3/4" 34261 ft-lb 0.011 (1%) 1.25D+1.5L L Unbraced 582 lb 1'4 1/2" 11596 lb 0.050 (5%) 1.25D+1.5L L Shear Perm Defl in, 0.000 (L/999) 0 999.000 (L/0) 0.000 (0%) 1'4 3/4" 0.058 (L/360) 0.010 (1%) L LL Defl inch 0.001 (L/35461) TL Defl inch 0.001 1'4 3/4" 0.087 (L/240) 0.010 (1%) D+L

4.000" 648 L 6% 199 / 449 Hanger TI WISE THE

Unfactored Reactions UNPATTERNED Ib (Uplift)

Dead

1207

159

Live

2791

299

Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Fill all hanger nailing holes.

(L/24245)

- 3 Girders are designed to be supported on the bottom edge only.
- 4 Multiple plies must be fastened together as per manufacturer's details.
- 5 Top loads must be supported equally by all plies.
- 6 Top braced at bearings.
- 7 Bottom braced at bearings.
- 8 Lateral slenderness ratio based on full section width

	mood ratio bassa on n	an occion wiath.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 0-4-2	(Span)0-6-7	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Part. Uniform	0-0-0 to 0-7-10		Тор	33 PLF	88 PLF	0 PLF	0 PLF	J5
3	Part. Uniform	0-0-0 to 0-7-10		Тор	80 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight

This desi

Continued on page 2...

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Handling & Installation
- Handling & Installation

 1. LVL beams must not be out or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

- 6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info Forex APA: PR-L318

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Kott Lumber Company 14 Anderson Blvd, Ontario Canada L4A 7X4 905-642-4400

100083566

NCE OF ON

December 17, 2018





Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Version 18.40.162 Powered by iStruct™

Client: Project:

Address:

GREEN YORK HOMES

12/14/2018

Page 2 of 2

. ~ 5 ~ ~

Designer: RCO

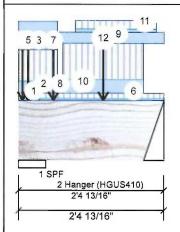
Job Name: LOT 1 (CELESTIAL 2 EL,1)

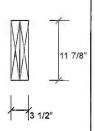
Project #:

Forex 2.0E-3000Fb LVL

1.750" X 11.875" 2-Ply - PASSED

Level: Ground Floor





.Continued from	page 1								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
4	Point	0-0-12		Far Face	55 lb	147 lb	0 lb	0 lb	J4
5	Point	0-1-14		Тор	901 lb	2190 lb	0 lb	0 lb	F9 F9
6	Tie-In	0-4-2 to 2-4-13	(Span)0-8-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
7	Point	0-6-14		Far Face	84 lb	225 lb	0 lb	0 lb	F13
8	Part. Uniform	0-7-10 to 0-8-12		Тор	13 PLF	34 PLF	0 PLF	0 PLF	J5
9	Part. Uniform	0-7-10 to 2-4-13		Тор	31 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
10	Part, Uniform	0-8-12 to 2-0-12		Тор	38 PLF	101 PLF	0 PLF	0 PLF	J5
11	Part. Uniform	0-11-4 to 2-3-4		Тор	9 PLF	25 PLF	0 PLF	0 PLF	J1
12	Point	1-4-12		Far Face	100 lb	267 lb	0 lb	dl 0	J5
	Self Weight				10 PLF				

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the inlended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- andling & Installation:
 LVL beams must not be cult or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening defails, beam strength values, and code
 approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info Forex APA: PR-L318

Kott Lumber Company 14 Anderson Blvd, Ontario Canada L4A 7X4 905-642-4400

This design is valid until 7/10/2021



Client: Project:

Address:

GREEN YORK HOMES

Date:

Project #:

12/14/2018

RCO

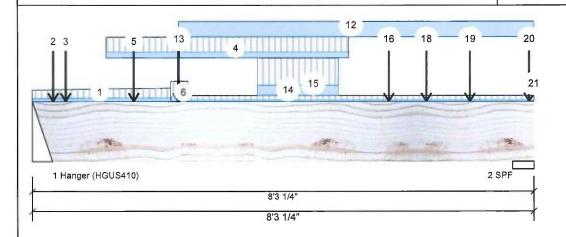
Designer:

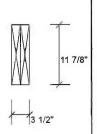
Job Name: LOT 1 (CELESTIAL 2 EL.1)

1.750" X 11.875" Forex 2.0E-3000Fb LVL

2-Ply - PASSED

Level: Ground Floor





Page 1 of 2

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Member Information						
Туре:	Girder					
Plies:	2					
Moisture Condition:	Dry					
Deflection LL:	360					
Deflection TL:	240					
Importance:	Normal					
General Load						
Floor Live:	40 PSF					

15 PSF

Application: Design Method: **Building Code:**

Deck:

Vibration:

Floor (Residential) LSD NBCC 2010 / OBC 2012

Load Sharing: No

Not Checked Not Checked

l	Jnfactored	Reactions	UNPAT	TERNED	lb	(Uplift)	
-							-

Brg	Live	Dead	Snow	Wind	
1	1093	723	0	0	
2	992	790	0	0	

Bearings and Factored Reactions

Bearing Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - 4.000"	24%	903 / 1640	2543	L	1.25D+1.5L
Hanger					
2 - SPF 4.192"	27%	988 / 1487	2475	L	1.25D+1.5L

Analysis Results

Dead:

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	4881 ft-lb	3'9 7/8"	34261 ft-lb	0.142 (14%)	1.25D+1.5L	L
Unbraced	4881 ft-lb	3'9 7/8"	31493 ft-lb	0.155 (15%)	1.25D+1.5L	L
Shear	2408 lb	1'3 1/8"	11596 lb	0.208 (21%)	1.25D+1.5L	L
Perm Defl in.	0.021 (L/4444)	4' 11/16"	0.257 (L/360)	0.080 (8%)	D	Uniform
LL Defl inch	0.028 (L/3322)	4' 1/8"	0.257 (L/360)	0.110 (11%)	L	L
TL Defl inch	0.049 (L/1901)	4' 5/16"	0.386 (L/240)	0.130 (13%)	D+L	L

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on full section width.

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December 17, 2018

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments	Ī
1	Tie-In	0-0-0 to 2-3-7	(Span)2-2-7 to 2-6-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
2	Point	0-4-2		Far Face	15 lb	39 lb	0 lb	dl 0	J1	
3	Point	0-6-10		Near Face	30 lb	79 lb	0 lb	0 lb	J1	
4	Part. Uniform	1-2-10 to 5-2-10		Near Face	29 PLF	77 PLF	0 PLF	0 PLF		
5	Point	1-8-2		Far Face	17 lb	44 lb	0 lb	0 lb	J1	

Continued on page 2...

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- and ling & Installation
 LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and oode
 approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at beaming points to avoid
 lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info Forex APA: PR-L318

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED This design is IN THE DESIGN OF THIS COMPONENT.





Client: Project:

Address:

GREEN YORK HOMES

Date: Designer:

12/14/2018

Job Name: LOT 1 (CELESTIAL 2 EL.1)

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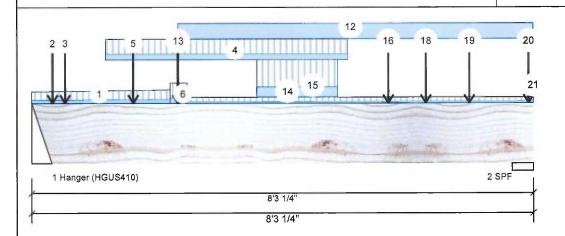
Project #:

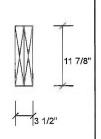
Forex 2.0E-3000Fb LVL

1.750" X 11.875"

2-Ply - PASSED

Level: Ground Floor





Page 2 of 2

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.Continued fr									2
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
6	Tie-In	2-3-7 to 2-6-13	(Span)3-9-13	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
7	Point	2-4-15		Тор	79 lb	211 lb	0 lb	0 lb	J5
8	Point	2-4-15		Тор	20 lb	52 lb	0 lb	0 lb	J1
9	Point	2-4-15		Тор	2 lb	5 lb	0 lb	0 lb	J5
10	Point	2-4-15		Тор	86 lb	0 lb	0 lb	0 lb	Wall Self Weight
11	Point	2-4-15		Тор	12 lb	0 lb	0 lb	0 lb	Wall Self Weight
12	Part, Uniform	2-4-15 to 8-3-3		Тор	80 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
13	Point	2-5-1		Far Face	159 lb	299 lb	0 lb	0 lb	F6
14	Tie-In	2-6-13 to 8-3-4	(Span)1-1-13	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
15	Part. Uniform	3-8-10 to 5-0-10		Тор	53 PLF	140 PLF	0 PLF	0 PLF	J3
16	Point	5-10-10		Near Face	28 lb	76 lb	0 lb	0 lb	J1
17	Point	6-6-0		Near Face	33 lb	62 lb	0 lb	0 lb	F2
18	Point	6-6-0		Тор	141 lb	322 lb	0 lb	0 lb	F10 F10
19	Point	7-2-10		Near Face	25 lb	65 lb	0 lb	0 lb	J1
20	Point	8-2-5		Near Face	34 lb	75 lb	0 lb	0 lb	F2
21	Part. Uniform	8-3-3 to 8-3-4		Тор	40 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
	Self Weight				10 PLF				

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design cntern and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the inhended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

landling & Installation.

LVL beams must not be cut or drilled.
Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals.

Damaged Beams must not be used.
Design assumes top adge is laterally restrained.
Provide lateral support at bearing points to avoid lateral displacement and rotation.

For flat roofs provide proper drainage to prevent ponding

This design is valid until 7/10/2021

Forex APA: PR-L318

Manufacturer Info





Client: Project:

Address:

GREEN YORK HOMES

12/14/2018

Designer: RCO

Job Name: LOT 1 (CELESTIAL 2 EL.1)

Page 1 of 2

Project #:

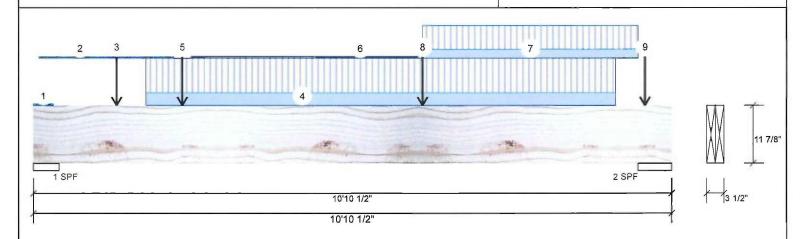
Date:

Forex 2.0E-3000Fb LVL

1.750" X 11.875"

2-Ply - PASSED

Level: Ground Floor



nation			Unfacto	red React	tions U	INPATTERNI	ED Ib (Up	olift)	
Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snow		Wind
2	Design Method:	LSD	1	3015		1438	0		0
Dry	Building Code:	NBCC 2010 / OBC 2012	2	3156		1322	0		0
360	Load Sharing:	No	1.5						
240	Deck:	Not Checked							
Normal	Vibration:	Not Checked							
40 PSF			Bearings	s and Fac	tored	Reactions			
15 PSF			Bearing	Length	Сар.	React D/L lb	Total Lo	d. Case	Ld. Comb.
			1 - SPF	5.250"	56%	1797 / 4523	6320 L		1.25D+1.5L
			2-SPF	6.875"	43%	1652 / 4734	6386 L		1.25D+1.5L
	Girder 2 Dry 360 240 Normal	Girder Application: Design Method: Building Code: Load Sharing: Deck: Normal Vibration: 40 PSF 15 PSF	Girder Application: Floor (Residential) 2 Design Method: LSD Building Code: NBCC 2010 / OBC 2012 Load Sharing: No Deck: Not Checked Normal Vibration: Not Checked	Application: Floor (Residential) Brg	Application: Floor (Residential) Brg Live	Application: Floor (Residential) Brg Live	Application: Floor (Residential) Brg Live Dead	Application: Floor (Residential) Brg Live Dead Snow	Application: Floor (Residential) Brg Live Dead Snow

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	16147 ft-lb	5'6 1/8"	34261 ft-lb	0.471 (47%)	1.25D+1.5L	L
Unbraced	16147 ft-lb	5'6 1/8"	29618 ft-lb	0.545 (55%)	1.25D+1.5L	L
Shear	6696 lb	1'4 3/8"	11596 lb	0.577 (58%)	1.25D+1.5L	L
Perm Defl in.	0.075 (L/1589)	5'3 11/16"	0.333 (L/360)	0.230 (23%)	D	Uniform
LL Defl inch	0.170 (L/704)	5'4 11/16"	0.333 (L/360)	0.510 (51%)	L	L
TL Defl inch	0.246 (L/488)	5'4 3/8"	0.499 (L/240)	0.490 (49%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on full section width.

T.L. WISE 100083566	
T.L. WISE 100083566	
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December 17, 2018

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 0-4-2	(Span)0-7-10	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-1-2 to 2-4-13	(Span)0-5-14	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	1-5-1		Near Face	151 lb	402 lb	0 (b	0 lb	J8
4	Part. Uniform	1-11-1 to 9-11-1		Near Face	148 PLF	395 PLF	0 PLF	0 PLF	
5	Point	2-6-9		Far Face	723 lb	1093 lb	0 lb	0 lb	F7
6	Tie-In	2-8-5 to 6-6-12	(Span)0-8-6	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

Continued on page 2...

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the Intended application, and to venfy the dimensions and loads.

Lumber

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info Forex APA: PR-L318

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Kott Lumber Company 14 Anderson Blvd, Ontario Canada L4A 7X4 905-642-4400



This design i





Client: Project:

Address:

GREEN YORK HOMES

Date:

12/14/2018

RCO

Page 2 of 2

Project #:

Designer:

Forex 2.0E-3000Fb LVL

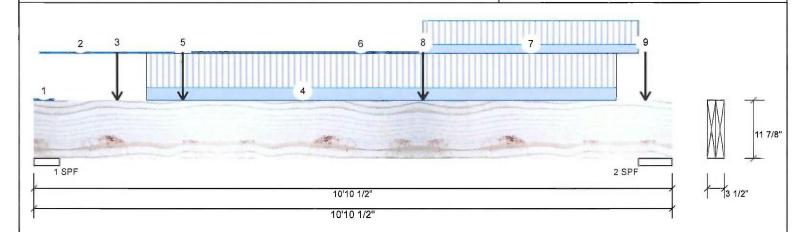
Self Weight

1.750" X 11.875"

2-Ply - PASSED

Level: Ground Floor

Job Name: LOT 1 (CELESTIAL 2 EL.1)



10 PLF

ı	Continued from p	page 1								
	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
	7	Part. Uniform	6-7-10 to 10-3-10		Тор	101 PLF	270 PLF	0 PLF	0 PLF	
	8	Point	6-7-10		Far Face	181 lb	404 lb	0 lb	0 lb	F5
	9	Point	10-5-1		Near Face	16 lb	42 lb	0 lb	0 lb	J8

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to venify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

landling & Installation.
LVL beams must not be out or drilled.
Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals.
Damaged Beams must not be used.
Design assumes top edge is laterally restrained.
Provide lateral support at beaming points to avoid lateral displacement and rotation.

For flat roofs provide proper drainage to prevent ponding

This design is valid until 7/10/2021

Forex

APA: PR-L318

Manufacturer Info



Client: Project:

Address:

GREEN YORK HOMES

Date:

12/14/2018

RCO Designer:

Job Name: LOT 1 (CELESTIAL 2 EL.1)

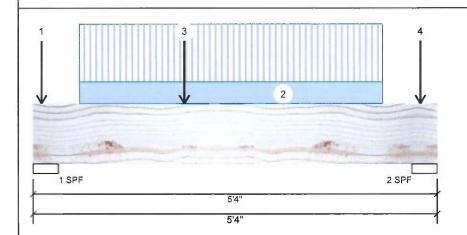
Project #:

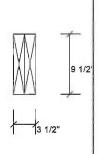
Forex 2.0E-3000Fb LVL

1.750" X 9.500"

2-Ply - PASSED

Level: Second Floor





Page 1 of 1

١	Welliber Milotillation								
	Туре:	Girder							
	Plies:	2							
	Moisture Condition:	Dry							
	Deflection LL:	360							
	Deflection TL:	240							
	Importance:	Normal							
	General Load								
	Floor Live:	40 PSF							

15 PSF

Mambar Information

Application: Design Method:

Building Code: Load Sharing:

Deck:

Vibration:

Floor (Residential) LSD

NBCC 2010 / OBC 2012

No

Not Checked Not Checked

Brg	Live	Dead	Snow	VVind
1	2317	939	0	0
2	1592	645	0	0

Unfactored Reactions UNPATTERNED lb (Uplift)

Bearings and Factored Reactions Cap. React D/L lb Total Ld. Case Ld. Comb. Bearing Length 1 - SPF 4.000" 1173 / 3476 4649 L 1.25D+1.5L 54% 2 - SPF 4.000" 37% 806 / 2388 3194 L 1.25D+1.5L

Wind

0 lb

Analysis Results

Dead:

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	6963 ft-lb	2'	22724 ft-lb	0.306 (31%)	1.25D+1.5L	L
Unbraced	6963 ft-lb	2'	22724 ft-lb	0.306 (31%)	1.25D+1.5L	L
Shear	4094 lb	1' 3/4"	9277 lb	0.441 (44%)	1.25D+1.5L	L
Perm Defl in.	0.014 (L/4012)	2'3 9/16"	0.160 (L/360)	0.090 (9%)	D	Uniform
LL Defl inch	0.035 (L/1625)	2'3 9/16"	0.160 (L/360)	0.220 (22%)	L	L
TL Defl inch	0.050 (L/1157)	2'3 9/16"	0.240 (L/240)	0.210 (21%)	D+L	L

Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on full section width

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WOLE OF ONTARIO
December 17, 2018

D	Load Type	Location	Trib Width	Side	Dead	Live	Snow
1	Point	0-1-6		Тор	50 lb	133 lb	0 lb
2	Part. Uniform	0-7-6 to 4-7-6		Тор	120 PLF	320 PLF	0 PLF
3	Point	2-0-0		Тор	975 lb	2394 lb	0 lb
4	Point	5-1-6		Тор	38 lb	102 lb	0 lb
	Self Weight				8 PLF		

0 PLF PassoThrup Framing Squash Block is required at all point loads over bearings

Comments

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- andling & Installation
 LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
 approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Forex APA: PR-L318

Manufacturer Info

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED This design is IN THE DESIGN OF THIS COMPONENT.





Client: Project:

Address:

GREEN YORK HOMES

12/14/2018 Date:

Designer: RCO

Job Name: LOT 1 (CELESTIAL 2 EL.1)

Project #:

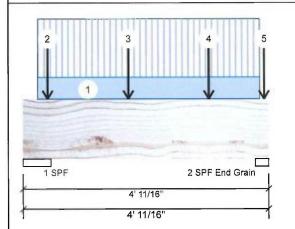
Forex 2.0E-3000Fb LVL

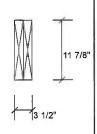
1.750" X 11.875"

2-Ply - PASSED Level: Second Floor

1

2





Wind

0

0

Page 1 of 1

Member Information							
Туре:	Girder	Application:	Floor (Residential)				
Plies:	2	Design Method:	LSD				
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012				
Deflection LL:	360	Load Sharing:	No				
Deflection TL:	240	Deck:	Not Checked				
Importance:	Normal	Vibration:	Not Checked				
General Load							
Floor Live:	40 PSF						
Dead:	15 PSF						

Live

322

539

Unfactored Reactions UNPATTERNED Ib (Uplift)

Dead

141

242

Bearing	Bearings and Factored Reactions										
Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.					
1 - SPF	5.500"	6%	176 / 483	660	L	1.25D+1.5L					
2 - SPF End Grain	2.632"	16%	302 / 808	1110	L	1.25D+1.5L					

Snow

0

T.L. WISE

100083566

NCE OF ON

December 17, 2018

Analysis Results

	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case	
	Moment	673 ft-lb	1'8 7/8"	34261 ft-lb	0.020 (2%)	1.25D+1.5L	L	
	Unbraced	673 ft-lb	1'8 7/8"	34261 ft-lb	0.020 (2%)	1.25D+1.5L	L	
ı	Shear	530 lb	2'10 15/16"	11596 lb	0.046 (5%)	1.25D+1.5L	L	
l	Perm Defl in.	0.001 (L/60821)	2' 1/2"	0.117 (L/360)	0,010 (1%)	D	Uniform	
	LL Defl inch	0.002 (L/25507)	2' 1/4"	0.117 (L/360)	0.010 (1%)	L	L	
	TL Defl inch	0.002 (L/17971)	2' 3/8"	0.175 (L/240)	0.010 (1%)	D+L	L	

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on full section width.

	O Editoral Startas	IIICOO IGGO DGOOG	off full books, within						_
ı	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	
-	1	Part, Uniform	0-2-14 to 3-10-15		Тор	30 PLF	80 PLF	0 PLF	
2	2	Point	0-4-14		Near Face	3 lb	8 lb	0 lb	
3	3	Point	1-8-14		Near Face	74 lb	198 lb	0 lb	
2	4	Point	3-0-14		Near Face	63 lb	167 lb	0 lb	
	5	Point	3-11-13		Near Face	94 lb	194 lb	0 lb	
		Self Weight				10 PLF			

Olb J3 Pass-Thru Framing Squash Block is required at all point loads over bearings

Comments

Wind

0 PLF

Manufacturer Info

Forex APA: PR-L318

0 lb

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- chemicals Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code 1.
- nasterning details, beam strength values, and code approvals.

 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid lateral displacement and rotation.

READ ALL NOTES ON THIS PAGE AND ON THE

For flat roofs provide proper drainage to prevent ponding

ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Kott Lumber Company 14 Anderson Blvd, Ontario L4A 7X4





Version 18.40.162 Powered by iStruct™

Client: Project:

Address:

GREEN YORK HOMES

12/14/2018

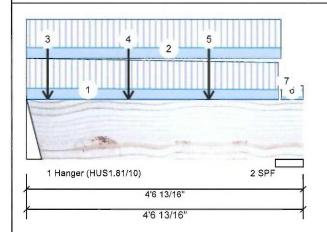
RCO Designer:

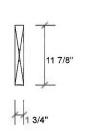
Job Name: LOT 1 (CELESTIAL 2 EL.1)

Project #:

Date:

Forex 2.0E-3000Fb LVL 1.750" X 11.875" - PASSED Level: Second Floor





Page 1 of 2

Member Inforn	ember Information				Unfactored Reactions UNPATTERNED Ib (Uplift)					
Туре:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snov	ν	Wind
Plies:	1	Design Method:	LSD	1	351		142		0	0
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012	2	303		125	9	0	0
Deflection LL:	360	Load Sharing:	No	1 -						
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal	Vibration:	Not Checked	1						
General Load										
Floor Live:	40 PSF			Bearing	s and Fac	tored	Reactions			
Dead:	15 PSF			Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.
				1 - Hanger	3.000"	18%	178 / 527	705	L	1.25D+1.5L
Analysis Result	Analysis Results				5.500"	10%	156 / 454	610	L	1.25D+1.5L

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	632 ft-lb	2'1 1/8"	17130 ft-lb	0.037 (4%)	1.25D+1.5L	L
Unbraced	632 ft-lb	2'1 1/8"	11283 ft-lb	0.056 (6%)	1.25D+1.5L	L
Shear	433 lb	1'2 1/8"	5798 lb	0.075 (7%)	1.25D+1.5L	L
Perm Defl in.	0.001 (L/32806)	2'1 13/16"	0.133 (L/360)	0.010 (1%)	D	Uniform
LL Defl inch	0.004 (L/13295)	2'1 3/4"	0.133 (L/360)	0.030 (3%)	L	L
TL Defl inch	0.005 (L/9461)	2'1 3/4"	0.199 (L/240)	0.030 (3%)	D+L	L



December 17, 2018

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.
- 4 Bottom braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments	
1	Tie-In	0-0-0 to 4-1-14	(Span)2-11-6 to 2-7-14	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
2	Tie-In	0-0-0 to 4-2-7	(Span)2-9-10	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
3	Point	0-4-3		Near Face	17 lb	45 lb	0 lb	dl 0	J1	
4	Point	1-8-3		Near Face	26 lb	69 lb	0 lb	0 lb	J1	
5	Point	3-0-3		Near Face	24 lb	64 lb	0 lb	0 lb	J1	
6	Tie-In	4-2-7 to 4-6-13	(Span)0-7-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF		

Continued on page 2...

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

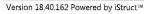
- Handling & Installation
- andling & Installation
 LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam sirength values, and code
 approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at beaming points to avoid
 lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info Forex APA: PR-L318

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED This design IN THE DESIGN OF THIS COMPONENT.







Client: Project:

Address:

GREEN YORK HOMES

Date: 12/14/2018

RCO

Page 2 of 2

Designer:

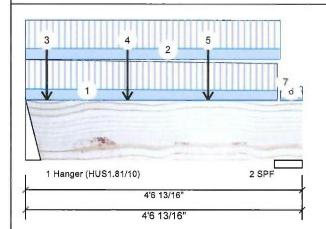
Job Name: LOT 1 (CELESTIAL 2 EL.1)

Project #:

Forex 2.0E-3000Fb LVL

1.750" X 11.875" - PASSED

Level: Second Floor



..Continued from page 1

ID Load Type 7

Tie-In

4-2-7 to 4-6-13 (Span)0-4-5

Location Trib Width

Side Top

Live Dead 15 PSF 40 PSF

Snow 0 PSF Wind Comments

0 PSF

Self Weight

5 PLF

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

1.

- Handling & Installation
- and ling & Installation
 LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
 approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

Manufacturer Info

 For flat roofs provide proper drainage to prevent punding Forex APA: PR-L318

Kott Lumber Company 14 Anderson Blvd, Ontario Canada L4A 7X4 905-642-4400

This design is valid until 7/10/2021

Client: Project:

Address:

GREEN YORK HOMES

12/14/2018

Designer: RCO

Job Name: LOT 1 (CELESTIAL 2 EL.1)

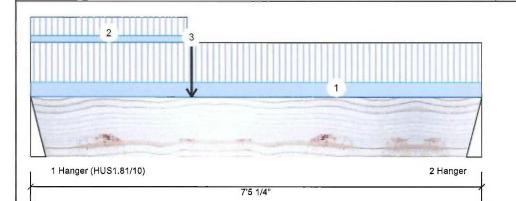
Project #:

Date:

Forex 2.0E-3000Fb LVL

1.750" X 11.875" - PASSED

Level: Second Floor





Page 1 of 1

Member	Information
Туре:	Girder

Plies: Moisture Condition: Dry Deflection LL: 360 Deflection TL: 240 Importance:

Normal General Load 40 PSF 15 PSF Application: Design Method:

7'5 1/4'

Building Code: Load Sharing:

NBCC 2010 / OBC 2012 Nο

Floor (Residential)

Deck: Not Checked Not Checked Vibration:

LSD

Unfactored Reactions UNPATTERNED lb (Uplift) Dead Snow

Live	Dead	Snow	Wind
314	142	0	0
194	94	0	0
	314	314 142	314 142 0

Analysis Results

Floor Live:

Dead:

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1399 ft-lb	2'7 7/8"	17130 ft-lb	0.082 (8%)	1.25D+1.5L	L
Unbraced	1399 ft-lb	2'7 7/8"	6365 ft-lb	0.220 (22%)	1.25D+1.5L	L
Shear	581 lb	1'2 1/8"	5798 lb	0.100 (10%)	1.25D+1.5L	L
Perm Defl in.	0.006 (L/13727)	3'2 7/8"	0.235 (L/360)	0.030 (3%)	D	Uniform
LL Defl inch	0.014 (L/6157)	3'2 3/16"	0.235 (L/360)	0.060 (6%)	L	L
TL Defl inch	0.020 (L/4251)	3'2 7/16"	0.353 (L/240)	0.060 (6%)	D+L	L

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - Hanger	3.000"	17%	178 / 472	650	L	1.25D+1.5L
2 - Hanger	3.000"	10%	118 / 291	408	L	1.25D+1.5L

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.

4 Bottom braced at bearings.

3'2 7/16" 0.353 (L/240) 0.060 (6%) D+L

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December 17, 2018

L	. = - 114111 #144									
I	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
	1	Tie-In	0-0-0 to 7-5-4	(Span) 0-10-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
ĺ	2	Tie-In	0-0-0 to 2-7-0	(Span)0-5-1	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
l	3	Point	2-7-14		Near Face	142 lb	351 lb	0 lb	0 lb	F3
١		Self Weight				5 PLF			D. T.	

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design enterial and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the Intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

andling & Installation
LVL beams must not be out or drilled
Refer to manufacturer's product information
regarding installation requirements, multi-ply
fastening details, beam strength values, and code
approvals
Damaged Beams must not be used
Design assumes top edge is laterally restrained
Provide lateral support at beaming points to avoid
lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex APA: PR-L318

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Kott Lumber Company 14 Anderson Blvd, Ontario Canada L4A 7X4



This desi



Client: Project:

Address:

GREEN YORK HOMES

Date: Designer:

12/14/2018

RCO

Project #:

Forex 2.0E-3000Fb LVL

1.750" X 11.875"

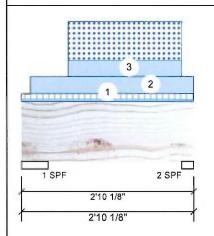
2-Ply - PASSED

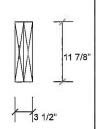
1

2

Level: Second Floor

Job Name: LOT 1 (CELESTIAL 2 EL.1)





Wind

0

0

Page 1 of 1

Plies: Moisture Condition:	2	Design Method:	LSD
Moisture Condition:	_		
	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
mportance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

Bearings and Factored Reactions												
Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.						
1 - SPF	5.250"	4%	207 / 188	394	L	1.25D+1.5S +0.5L						
2-SPF	2.375"	9%	214 / 242	457	L	1.25D+1.5S +0.5L						

Unfactored Reactions UNPATTERNED Ib (Uplift)

Dead

165

172

Snow

114

152

Live

33

28

Analysis Location Allowed Capacity Comb. 0.008 (1%) 262 ft-lb 1'6 13/16" 33233 ft-lb 1.25D+1.5S L Moment +0.5L 262 ft-lb 1'6 13/16" 33233 ft-lb 0.008 (1%) 1,25D+1,5S L Unbraced +0.5L 0.007 (1%) 1.25D+1.5S L 81 lb 1'4 3/8" 11248 lb Shear +0.5L Perm Defl in. 0.000 (L/999) 0 999,000 (L/0) 0.000 (0%) LL Defl inch 0.000 (L/999) 0 999.000 (L/0) 0.000 (0%) TL Defl inch 0.001 1'6 3/4" 0.117 (L/240) 0.010 (1%) D+S+0.5L L (L/38547)



Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on full section width.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow
1	Tie-In	0-0-0 to 2-10-2	(Span)1-0-13	Тор	15 PSF	40 PSF	0 PSF
2	Part. Uniform	0-1-12 to 2-10-2		Тор	64 PLF	0 PLF	0 PLF
3	Part. Uniform	0-9-4 to 2-7-12		Тор	61 PLF	0 PLF	142 PLF
	Self Weight				10 PLF		

0 PSF PasseThruvFalaseimgeSquash Block is required at all point loads over bearings

Comments

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to centure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- chemicals
- andling & Installation

 LVL beams must not be cut or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvels
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained.
 Provide lateral support at beaming points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent

Manufacturer Info Forex APA: PR-L318

Wind

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Kott Lumber Company 14 Anderson Blvd, Ontario L4A 7X4 905-642-4400



This design is



Client: Project:

Address:

GREEN YORK HOMES

Date: 12/14/2018

Designer: RCO

Job Name: LOT 1 (CELESTIAL 2 EL.1)

Project #:

Forex 2.0E-3000Fb LVL

1.750" X 11.875"

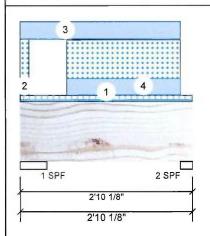
2-Ply - PASSED

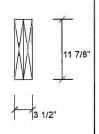
Brg

1

2

Level: Second Floor





Wind

0

0

Page 1 of 1

Member Inforn	Member Information							
Туре:	Girder	Application:	Floor (Residential)					
Plies:	2	Design Method:	LSD					
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012					
Deflection LL:	360	Load Sharing:	No					
Deflection TL:	240	Deck:	Not Checked					
Importance:	Normal	Vibration:	Not Checked					
General Load								
Floor Live:	40 PSF							
Dead:	15 PSF							

Bearings	s and Fac	tored R	eactions			
Bearing			React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	5.250"	4%	226 / 215	441	L	1.25D+1.5S +0.5L
2 - SPF	2.375"	9%	195 / 239	434	L	1,25D+1,5\$ +0,5L

Unfactored Reactions UNPATTERNED Ib (Uplift)

Dead

181

156

Snow

135

152

Live

25

21

Analysis Results **Analysis** Actual Location Allowed Capacity Comb. Case Moment 258 ft-lb 1'6 13/16" 33575 ft-lb 0.008 (1%) 1.25D+1.5S L +0.5L Unbraced 258 ft-lb 1'6 13/16" 33575 ft-lb 0.008 (1%) 1.25D+1.5S L +0.5L 0.007 (1%) 1.25D+1.5S L Shear 80 lb 1'4 3/8" 11364 lb +0.51Perm Defl in. 0.000 (L/999) 0 999.000 (L/0) 0.000 (0%) LL Defl inch 0.000 (L/999) 0 999.000 (L/0) 0.000 (0%) TL Defl inch 0.001 1'6 3/4" 0.117 (L/240) 0.010 (1%) D+S+0.5L L (L/39192)



Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on full section width.

ı	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow
ı	1	Tie-In	0-0-0 to 2-10-2	(Span)0-9-11	Тор	15 PSF	40 PSF	0 PSF
ı	2	Part. Uniform	0-0-0 to 0-1-12		Тор	62 PLF	0 PLF	144 PLF
	3	Part. Uniform	0-0-0 to 2-7-12		Тор	64 PLF	0 PLF	0 PLF
	4	Part. Uniform	0-9-4 to 2-7-12		Тор	61 PLF	0 PLF	142 PLF
ı		Self Weight				10 PLF		
١								

PasspThru Framing Squash Block is required at all point loads over bearings

Comments

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- I. IVIL beams must not be cut or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
 Damaged Beams must not be used.
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

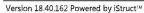
This design is

Manufacturer Info Forex APA: PR-L318

Wind

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. - - - . - - . . -GREEN YORK HOMES 12/14/2018 Client: Date: Page 1 of 2 RCO Project: Designer: isDesign Address: Job Name: LOT 1 (CELESTIAL 2 EL.1) Project #: Level: Second Floor 2-Ply - PASSED Forex 2.0E-3000Fb LVL 1.750" X 11.875" 16 11 3 4 2 11 7/8" 1 LVL 2 SPF 10'10 1/2" 10'10 1/2" Member Information Unfactored Reactions UNPATTERNED Ib (Uplift) Type: Girder Application: Floor (Residential) Live Dead Snow Wind Plies: 2 Design Method: LSD 1 2394 975 0 0 NBCC 2010 / OBC 2012 Moisture Condition: Dry **Building Code:** 2 2576 1100 0 Deflection LL: 360 Load Sharing: Νo Deflection TL: 240 Deck: Not Checked Importance: Vibration: Not Checked Normal General Load Bearings and Factored Reactions Floor Live: 40 PSF Cap. React D/L lb Dead: 15 PSF Bearing Length Total Ld. Case Ld. Comb. 1219 / 3591 4810 L 1.25D+1.5L 1 - LVL 3.500" 53% 2 - SPF 5.500" 1.25D+1.5L 44% 1375 / 3864 5239 L **Analysis Results** Analysis Actual Location Allowed Capacity Comb. T.L. WISE Moment 13922 ft-lb 5'3 7/8" 34261 ft-lb 0.406 (41%) 1.25D+1.5L L Unbraced 13922 ft-lb 5'3 7/8" 29373 ft-lb 0.474 (47%) 1.25D+1.5L L Shear 5403 lb 1'2 5/8" 11596 lb 0.466 (47%) 1.25D+1.5L L Perm Defl in. 0.062 (L/1983) 5'4 1/8" 0.342 (L/360) 0.180 (18%) D Uniform LL Defl inch 0.150 (L/820) 5'3 7/8" 0.342 (L/360) 0.440 (44%) L 100083566 TL Defl inch 0.212 (L/580) 5'3 7/8" 0.512 (L/240) 0.410 (41%) D+L **Design Notes** 1 Girders are designed to be supported on the bottom edge only. 2 Multiple plies must be fastened together as per manufacturer's details. 3 Top loads must be supported equally by all plies. December 17, 2018 4 Top braced at bearings, 5 Bottom braced at bearings. 6 Lateral slenderness ratio based on full section width

6 Lateral	sienderness ratio based	on full section wiath.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 0-2-6	(Span) 0-10-10	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-2-15 to 1-6-6	(Span)3-5-15 to 3-6-3	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	1-3-14		Near Face	169 lb	451 lb	0 lb	0 lb	J8
4	Point	1-6-6		Far Face	31 lb	83 lb	0 lb	0 lb	J1
5	Part, Uniform	1-11-14 to 4-7-14		Near Face	140 PLF	373 PLF	0 PLF	0 PLF	

Continued on page 2...

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Lumber

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

- LVL beams must not be cut or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
- Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

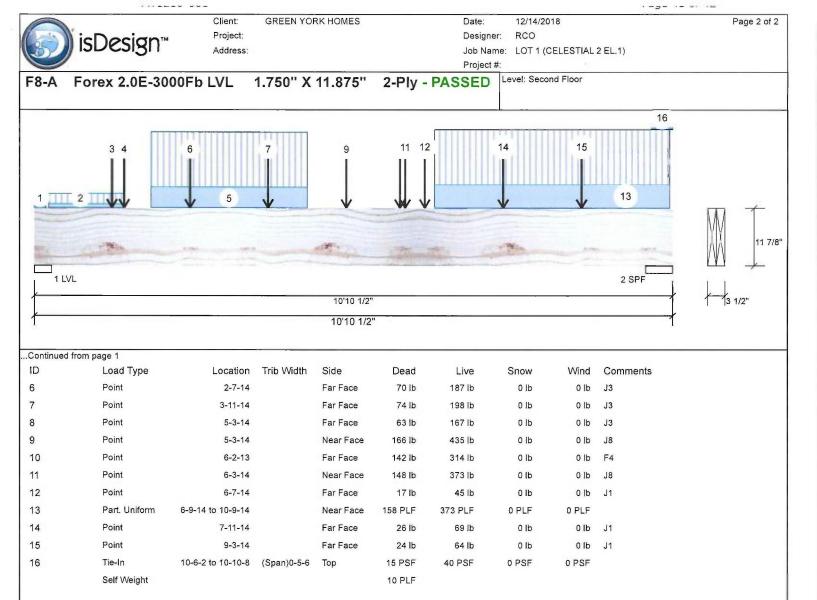
For flat roofs provide proper drainage to prevent ponding

Manufacturer Info Forex APA: PR-L318

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Kott Lumber Company 14 Anderson Blvd, Ontario Canada 14A 7X4

Version 18.40.162 Powered by iStruct™



Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

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Handling & Installation

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For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex APA: PR-L318 Kott Lumber Company 14 Anderson Blvd, Ontario Canada L4A 7X4 905-642-4400

This design is valid until 7/10/2021

-5- .- -1 GREEN YORK HOMES 12/14/2018 Page 1 of 1 Date: Client: Designer: RCO Project: isDesign Job Name: LOT 1 (CELESTIAL 2 EL.1) Address: Project #: Level: Second Floor 1.750" X 11.875" 2-Ply - PASSED Forex 2.0E-3000Fb LVL 3 2 1 2 SPF 1 Hanger (HGUS410) 16' 3/4' 16' 3/4 Unfactored Reactions UNPATTERNED Ib (Uplift) Member Information Wind Type: Girder Application: Floor (Residential) Live Dead Snow Plies: Design Method: LSD 1 2070 856 0 0 NBCC 2010 / OBC 2012 Moisture Condition: Dry **Building Code:** 2 2190 901 0 0 Deflection LL: 360 Load Sharing: No Deflection TL: 240 Not Checked Deck: Not Checked Importance: Normal Vibration: General Load Bearings and Factored Reactions Floor Live: 40 PSF Dead: 15 PSF Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 4.000" 40% 1070 / 3106 4175 L 1.25D+1.5L Hanger 1.25D+1.5L **Analysis Results** 2 - SPF 4.000" 51% 1126 / 3285 4411 L Location Allowed Analysis Actual Capacity Comb. T.L. WISE 16529 ft-lb 8' 1/4" 34261 ft-lb 0.482 (48%) 1.25D+1.5L L Moment Unbraced 16529 ft-lb 8' 1/4" 23053 ft-lb 0.717 (72%) 1.25D+1.5L L Shear 4147 lb 14'9 5/8" 11596 lb 0.358 (36%) 1.25D+1.5L L Perm Defl in. 0.159 (L/1171) 8' 5/16" 0.517 (L/360) 0.310 (31%) D Uniform LL Defl inch 0.387 (L/482) 8' 5/16" 0.517 (L/360) 0.750 (75%) L 1 100083566 TL Defl inch 0.546 (L/341) 8' 5/16" 0.776 (L/240) 0.700 (70%) D+L **Design Notes** 1 Fill all hanger nailing holes. 2 Girders are designed to be supported on the bottom edge only. 3 Multiple plies must be fastened together as per manufacturer's details. December 17, 2018 4 Top loads must be supported equally by all plies. 5 Top braced at bearings. 6 Bottom braced at bearings. 7 Lateral slenderness ratio based on full section width. ID Trib Width Live Wind Comments Load Type Location Side Dead Snow 40 PSF 0 PSF 0 PSF Tie-In 0-0-0 to 16-0-2 (Span)0-6-5 15 PSF Top 0-5-10 to 15-1-10 99 PLF 263 PLF 0 PLF 0 PLF 2 Part. Uniform Far Face Point 3 15-9-10 Far Face 88 lb 234 lb Pass-Thru Framing Squash Block is Self Weight 10 PLF required at all point loads over bearings Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements Manufacturer Info Kott Lumber Company Notes chemicals For flat roofs provide proper drainage to prevent ponding 14 Anderson Blvd, Ontario Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteries and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. Handling & Installation Forex APA: PR-L318

Lumber

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Canada L4A 7X4



