Engineered most joists small be motioned in accordance with the supplier's layout and specifications forming part of the permit drawings. This certification is to confirm that:

1. The loads used in the calculation of the attached approved components conform to the floor assembly shown on this layout.

2. The floor joists comply with the KOTT span table for the loads and spacing shown on this layout.

The floor system must be assembled in accordance to the KOTT Specifier Guide. Multi-ply members must be attached together as per the included multiple member connection detail.

All other components and structural elements supporting the floor system such as beams, walls, columns and foundation walls and footings including anchorage of components and bracing for lateral stability are the responsibility of others.



1. OBC 2012 O.Reg 332/12 as amended

2. Nascor CCMC - 13535-R

JOISTS SPACING 16"O/C

NOTED OTHERWISE

Architectural Drawing Info

64 JARDIN DR, SUITE 3A

VAUGHAN, ON, L4K 3P3

Project # 17-55

UNLESS

Date: AUG 30 2018

JARDIN DESIGN GROUP INC

Model: LOT-6 (AMELIA - EL-1)

3. LVL CCMC -12904-R

4. CAN/CSA-O86-09

5. CCMC -12787-R APA PR-L310(C)

J7 NJ60H 2.5 11.875 Rim Board Label Description Width Depth Qty | Plies | 1.125 11.875 Norbord Rimboard 16 Plus 1.125 X Hanger Beam/Girder Skew Slope label Pcs Description fasteners Unknown H1 1 Hanger H2 2 HUS1.81/10 30 10dx1 1/2 НЗ 29 LF2511 12 10d 1 HUS1.81/10 H5 Blocking Label Description Width | Depth | Qty Plies BLK1 LPI 20 Plus 2.5 11.875 LinFt BLK2 NJH 2.5 11.875 LinFt NOTES: Framer to verify dimensions on the architectural drawings. 2. Double joist only require filler/backer ply when supporting another member using a face-mounted hanger.

are fastened as per the hanger manufacturer's standards.

Rim parallel to joists: 1-1/8" rimboard with All other components and structural elements supporting

Hatch area represents ceramic tiled floor with an additional dead load

The framing shown on this layout may be deviate from the architectural drawings, Project Engineer to review and approve the deviation prior to construction.

Legend Point Load Support Load from Above Wall LPI 20Plus 11.875 NJ60H 11.875 5.25 X 10.25 (Dropped)



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This layout is to be used as an installation guide only. It is meant to be used in conjunction with the architectural and structural drawings, not to replace them

SB

Shipping

Project

Canada

1 4A 7X4

Floor

oads

Dead

905-642-4400

Ground Floor

esign Method

Deflection Joist

LL Span L/

TL Span L/

LL Cant 2L/

TL Cant 2L/

LL Span L/

TL Span L/

LL Cant 2L/

TL Cant 2L/

Decking

Thickness

Fastener

Vibration

M-2057

Deck

Deflection Girder

Builder's Project

14 Anderson Blvd

Stouffville, Ontario

Kott Lumber Company

Building Code NBCC 2010 / OBC

LSD

2012

40

15

480

360

480

360

360

240

480

360

OSB

3/4"

Nailed & Glued

IM1218-096

LVL/LSL Label Description Width Depth Qty Plies Pcs Length 1.75 11.875 12-0-0 F9 Forex 2.0E-3000Fb LVL Layout Name F8 Forex 1.75 11.875 10-0-0 LOT-6 (AMELIA 3 EL-1 _5BEDRM) 2.0E-3000Fb LVL F11 1.75 Forex 11.875 2 8-0-0 Design Method 2.0E-3000Fb LVL LSD F7 Forex 1.75 11.875 8-0-0 Description 2.0E-3000Fb LVL **GREEN YORK HOMES** F10 1.75 11.875 2 6-0-0 Forex 2.0E-3000Fb LVL GRANELLI HOMES PROJECT BRAMPTON,ON F6 Forex 1.75 11.875 4-0-0 2.0E-3000Fb LVL Created May 29, 2018 Builder

Ground Floor

Joist Label Description Width Depth Pcs Length F15 LPI 20Plus 2.5 11.875 16-0-0 6 F14 LPI 20Plus 2.5 11.875 4-0-0 Sales Rep 28 16-0-0 Designer 2.5 11.875 J1 LPI 20Plus J6 LPI 20Plus 2.5 11.875 20 14-0-0 J5 LPI 20Plus 2.5 11.875 4 12-0-0 J4 LPI 20Plus 2.5 11.875 10-0-0 2.5 11.875 J3 LPI 20Plus 8-0-0 7 J2 LPI 20Plus 2.5 11.875 6-0-0 3 16-0-0

Pcs Length

Supported Member fasteners 10 16d 1 #8x1 1/4WS

Pcs Length Varies 34-0-0 Varies 11-0-0

Install 2x4 blocking @ 24" o/c under parallel non-loadbearing walls. Install single-ply flush window header along inside face of rimboard/rimjoist

Refer to Nascor specifier guide for installation details.

Squash blocks recommended to be installed at end bearing on all first level joists which support loading from above exceeding two levels floor or roof.

Load transfer blocks to be installed under all point loads. It shall be the framer's responsibility that floor joists and beams

Refer to Multiple Member Connection Detail to ply to ply nailing or bolting requirements.

2"x4" block (1/16" longer than rim depth) @ 16" o/c. the floor system such as beams, walls, columns and foundation walls and footings including anchorage of components and bracing for lateral stability are the responsibility of others.

Norbord Rimboard Plus 1.125 X 11.875 Forex 2.0E-3000Fb LVL 1.75 X 11.875

19-447141.00.00KK

F16-A - 1 ply

PROFESSION

I.MATIJEVIC

100528832

NCE OF ONT

December 18, 2018

KOTT

OTB

Beam/Girder Supported Member Label Pcs Description Skew Slope fasteners fasteners 2 HUS1.81/10 H2 30 16d 10 16d НЗ 26 LF2511 12 10d 1 #8x1 1/4WS H4 1 HGUS410 46 16d 16 16d NOTES:

- Framer to verify dimensions on the architectural drawings.

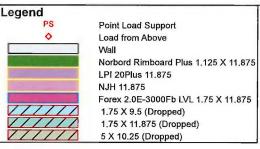
- Load transfer blocks to be installed under all point loads. It shall be the framer's responsibility that floor joists and beams

Refer to Multiple Member Connection Detail to ply to ply nailing or bolting

Rim parallel to joists: 1-1/8" rimboard with 2"x4" block (1/16" longer than rim depth) @ 16" o/c. the floor system such as beams, walls, columns and foundation walls and footings including anchorage of components and bracing for lateral stability are the

Hatch area represents ceramic tiled floor with an additional dead load

The framing shown on this layout may be deviate from the architectural drawings. Project Engineer to review and approve the deviation prior to construction.



Label Description Width Depth Qty Plies Pcs Length 1.75 11.875 2.0E-3000Fb LVL 1.75 11.875 2 2 2.0E-3000Fb LVL 11.875 1.75 4 2.0E-3000Fb LVL 1.75 11.875 2.0E-3000Fb LVL 1.75 11.875 2.0E-3000Fb LVL Label Description Width Depth Qty Plies Pcs Length J8 LPI 20Plus 2.5 11.875 18 20-0-0 J1 LPI 20Plus 2.5 11.875 50 16-0-0 J6 LPI 20Plus 2.5 | 11.875 14 14-0-0 J5 LPI 20Plus 2.5 11.875 5 12-0-0 J3 LPI 20Plus 2.5 11.875 F16 LPI 20Plus 2.5 11.875 2.5 11.875 2 Label Description Width Depth Qty Plies Pcs | Length Norbord Rimboard 1.125 11.875 17 Plus 1.125 X Label Description Width Depth Qty Plies Pcs Length BLK1 LPI 20 Plus 2.5 11.875 LinFt Varies 34-0-0

2. Double joist only require filler/backer ply when supporting another member using a face-mounted hanger.

Install 2x4 blocking @ 24" o/c under parallel non-loadbearing walls. Install single-ply flush window header along inside face of rimboard/rimjoist

Refer to Nascor specifier guide for installation details.

Squash blocks recommended to be installed at end bearing on all first level joists which support loading from above exceeding two levels floor or roof.

are fastened as per the hanger manufacturer's standards.

All other components and structural elements supporting

Legend	
PS	Point Load Support
♦	Load from Above
	Wall
	Norbord Rimboard Plus 1.125 X
	LPI 20Plus 11.875
	NJH 11.875
	Forex 2.0E-3000Fb LVL 1.75 X
/////	1.75 X 9.5 (Dropped)
//////	1.75 X 11.875 (Dropped)
/////	5 X 10.25 (Dropped)

Second Floor

Architectural Drawing Info

JARDIN DESIGN GROUP INC 64 JARDIN DR. SUITE 3A VAUGHAN, ON, L4K 3P3

Model: LOT-6 (AMELIA - EL-1) Date: AUG 30 2018

JOISTS SPACING 16"O/C NOTED OTHERWISE

1. OBC 2012 O.Reg 332/12 as amended

2. Nascor CCMC - 13535-R

3. LVL CCMC -12904-R 4. CAN/CSA-O86-09

5. CCMC -12787-R APA PR-L310(C)

Version 18.80.219 Powered by iStruct™

This layout is to be used as an installation guide only. It is meant to be used in conjunction with the architectural and structural drawings, not to replace them

This certification is to confirm that:

multiple member connection detail.

spacing shown on this layout.

responsibility of others.

conform to the floor assembly shown on this layout.

1. The loads used in the calculation of the attached approved components

The floor system must be assembled in accordance to the KOTT Specifier

Guide. Multi-ply members must be attached together as per the included

All other components and structural elements supporting the floor system

such as beams, walls, columns and foundation walls and footings including

2. The floor joists comply with the KOTT span table for the loads and

anchorage of components and bracing for lateral stability are the

16-0-0 Layout Name 12-0-0

LOT-6 (AMELIA 3 EL-1 5BEDRM)

8-0-0 Design Method

Description **GREEN YORK HOMES GRANELLI HOMES PROJECT** BRAMPTON,ON

Created May 29, 2018

8-0-0

4-0-0

Builder Sales Rep 8-0-0 Designer

20-0-0 SB 4-0-0 Shipping Project

> Builder's Project **Kott Lumber Company**

14 Anderson Blvd Stouffville, Ontario Canada L4A 7X4

905-642-4400

Second Floor Design Method LSD Building Code NBCC 2010 / OBC 2012 Floor

Loads Live Dead Deflection Joist LL Span L/

40

15

480 TL Span L/ 360 LL Cant 2L/ 480 TL Cant 2L/ 360 Deflection Girder 360

LL Span L/ TL Span L/ 240 LL Cant 2L/ 480 TL Cant 2L/ 360 Decking Deck OSB

Thickness 5/8" Fastener Nailed & Glued Vibration

Gypsum 1/2"

Ceiling:

M-2057

Engineering Note Page (ENP-2) M-2057 LOT 6

REVISION 2018-10-17

Please read all notes prior to installation of the component

DESIGN INFORMATION

This building component is certified as an individual component for the loads and conditions shown on the calculation and drawing page.

The responsibility of the undersigned engineer is only limited to the calculation of this building component for the loads and conditions shown on this drawing.

The responsibility of the undersigned is limited to the verification of the structural capacity of the floor joists and LVL beams based on placement as shown on the layout. The loads applied are limited to the gravity effects of the specified loads. The structural integrity of the building and the effect of wind, uplift, seismic, lateral or other forces, calculation of adequate support and anchorage of components, as well as the dimensions and design loads used to calculate components are the responsibility of the overall building designer.

Floor joists and OSB rim board are designed to carry uniformly distributed loads only. Point loads should be transferred through the floor cavity with transfer blocks. Structural elements such as walls, posts, connectors, and transfer blocks are the responsibility of the overall building designer.

The undersigned engineer disclaims any responsibility for damages as a result of being furnished faulty or incorrect information, specifications and/or designs.

Installation of floor joists is to be carried out in accordance with the current edition of the manufacturer's literature available at http://www.kottgroup.com.

CODE

This building component is designed in accordance with the National Building Code of Canada, the Ontario Building Code, CCMC and Canadian Standards Association guidelines.

COMPONENT

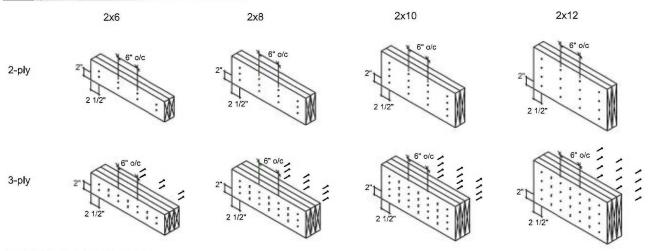
- 1. The building component used in construction must be the same as indicated on the drawings.
- 2. The building component must be installed and assembled as per specification shown on the drawing and in accordance with the manufacturer's assembly and installation.
- 3. Members consisting of multiple plies must be connected as per the document "Multi-ply Connection Details".
- 4. Pass-thru transfer block framing is required at all point loads over bearings.

HANDLING AND INSTALLATION

Do not drill any hole, cut or notch a certified building component without a written preauthorization.



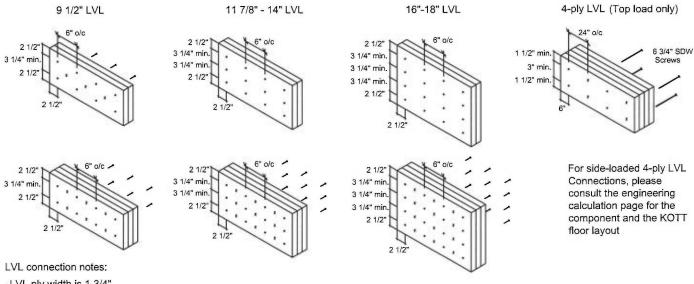
Conventional Connections



Conventional connection notes:

- -Nails to be 3" long wire nails.
- -Nails to be located 2" min. from the top and bottom of the member. Start all nails 2 1/2" min. from ends.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail driven from the opposite side.

LVL Connections



- -LVL ply width is 1-3/4"
- -Nails to be 3 1/2" common wire nails.
- -Nails to be located 2 1/2" min. from the top and bottom of the member. Start all nails 2 1/2" min. from ends.
- -Minimum 3 1/4" spacing between rows.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail or screw driven from the opposite side.

Multiple Member Connections

All connections are for uniformly distributed loads.

For multi-ply connections of I-joists, refer to Manufacturer's Installation Guide



KOTT Inc. 3228 Moodie Drive Ottawa, ON K2H 7V1 613-838-2775



Client:

Project: Address: Date: 12/17/2018

Designer: SB

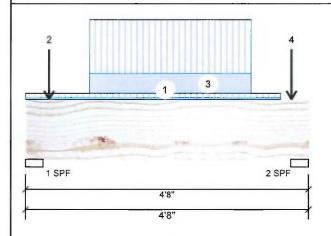
Job Name: LOT-6 (AMELIA 3 EL-1 _5BEDRM)

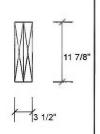
Project #:

Forex 2.0E-3000Fb LVL

1.750" X 11.875"

2-Ply - PASSED Level: Ground Floor





Member III	ioimation
Туре:	Girder
Dline:	2

Moisture Condition: Dry Deflection LL: 360 Deflection TL: 240 Importance: Normal

General Load Floor Live: 40 PSF Dead: 15 P\$F

Application: Design Method:

Building Code:

Load Sharing:

Deck:

Vibration:

Floor (Residential) LSD NBCC 2010 / OBC 2012

No Not Checked Not Checked

Unfactored Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	Wind	
1	733	298	0	0	
2	757	307	0	0	
1					
11					

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.	
1 - SPF	3.500"	20%	372 / 1100	1472	L	1.25D+1.5L	
2-SPF	3.500"	20%	384 / 1136	1520	L	1.25D+1.5L	

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1240 ft-lb	2'3 3/4"	34261 ft-lb	0.036 (4%)	1.25D+1.5L	L
Unbraced	1240 ft-lb	2'3 3/4"	34261 ft-lb	0.036 (4%)	1.25D+1.5L	L
Shear	1461 lb	1'2 5/8"	11596 lb	0.126 (13%)	1.25D+1.5L	L
Perm Defl in.	0.002 (L/33315)	2'3 7/8"	0.140 (L/360)	0.010 (1%)	D	Uniform
LL Defl inch	0.004 (L/13707)	2'3 13/16"	0.140 (L/360)	0.030 (3%)	L	L
TL Defl inch	0.005 (L/9712)	2'3 13/16"	0.210 (L/240)	0.020 (2%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on full section width.

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LICEA	I.MATIJEVIC 100528832	ENGINEER
12/2	DVINCE OF CHILD	

December 18, 2018

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow
1	Tie-In	0-0-0 to 4-2-8	(Span)1-2-0	Тор	15 PSF	40 PSF	0 PSF
2	Point	0-4-10		Near Face	122 lb	326 lb	0 lb
3	Part, Uniform	1-0-10 to 3-8-10		Near Face	106 PLF	281 PLF	0 PLF
4	Point	4-4-10		Near Face	119 lb	317 lb	0 lb
	Self Weight REA	D ALL NOTES ON TH	S PAGE AND O	N THE	10 PLF		

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

ID

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

IN THE DESIGN OF THIS COMPONENT. Handling & Installation

andting & Installation
LVL beams must not be cut or drilled
Refer to manufacturer's product information
regarding installation requirements, multi-ply
fastiening details, beam strength values, and code
approvals
Damaged Beams must not be used
Design assumes top adge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED

For flat roofs provide proper drainage to prevent ponding

This design is valid until 10/18/2021

Manufacturer Info Forex

Kott Lumber Company 14 Anderson Blvd, Ontario Canada L4A 7X4 905-642-4400

Comments

Wind

0 PSF 0 lb







Client:

Project: Address:

12/17/2018 Date:

Designer: SB

Job Name: LOT-6 (AMELIA 3 EL-1 _5BEDRM)

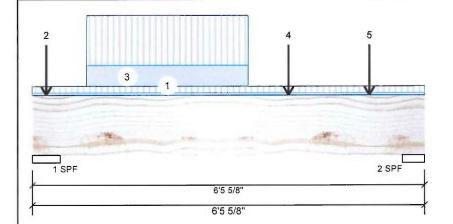
Project #:

Forex 2.0E-3000Fb LVL

1.750" X 11.875"

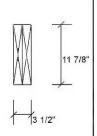
2-Ply - PASSED

Level: Ground Floor



Deck:

Vibration:



Member Information

Туре:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	
Floor Live:	40 PSF
Dead:	15 PSF

Floor (Residential) Application: Design Method: LSD NBCC 2010 / OBC 2012 Building Code: Load Sharing:

No Not Checked Not Checked

Unfactored Reactions UNPATTERNED Ib (Uplift)

rg	Live	Dead	Snow	Wind
1	837	383	0	0
2	823	385	0	0

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.	
1 - SPF	5.500"	15%	479 / 1256	1735	L	1.25D+1.5L	
2 SDE	4 375"	18%	482 / 1234	1716	1	1 25D+1 5I	

Analysis Results

							_
I	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
l	Moment	2763 ft-lb	3'2 11/16"	34261 ft-lb	0.081 (8%)	1.25D+1.5L	L
I	Unbraced	2763 ft-lb	3'2 11/16"	32711 ft-lb	0.084 (8%)	1.25D+1.5L	L
I	Shear	1902 lb	5'2 1/8"	11596 lb	0.164 (16%)	1.25D+1.5L	L
	Perm Defl in.	0.005 (L/12838)	3'3 1/4"	0.192 (L/360)	0.030 (3%)	D	Uniform
ĺ	LL Defl inch	0.012 (L/5833)	3'3 1/16"	0.192 (L/360)	0.060 (6%)	L	L
I	TL Defl inch	0.017 (L/4011)	3'3 1/8"	0.289 (L/240)	0.060 (6%)	D+L	L

Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



December 18, 2018

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow
1	Part. Uniform	0-0-0 to 6-5-10		Тор	15 PLF	40 PLF	0 PLF
2	Point	0-2-12		Тор	1 lb	0 lb	0 lb
3	Part. Uniform	0-10-12 to 3-6-12		Near Face	129 PLF	305 PLF	0 PLF
4	Point	4-2-12		Near Face	171 lb	387 lb	0 lb
5	Point	5-6-12		Near Face	94 lb	201 lb	0 lb
	Self Weight				10 PLF		

Wind Comments

0 PLF

0 lb Wall Self Weight

O PLESS-Thru Framing Squash Block is equired at all point loads over bearings 0 lb

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

- Handling & Installation
- andling & installation
 LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
 approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

- 6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 10/18/2021

Manufacturer Info Forex







Client:

Project: Address:

12/17/2018 Date:

SB Designer:

Job Name: LOT-6 (AMELIA 3 EL-1 _5BEDRM)

Project #:

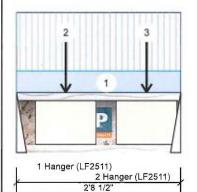
Brg

2

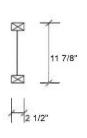
LPI 20Plus

11.875" - PASSED

Level: Ground Floor



2'8 1/2"



Wind

0

0

Member Information						
Type:	Girder	Application:	Floor (Residentia			
Plies:	1	Design Method:	LSD			
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OB			
Deflection LL:	360	Load Sharing:	No			
Deflection TL:	240	Deck:	Not Checked			
Importance:	Normal	Vibration:	Not Checked			
0						

BC 2012 General Load Floor Live: 40 PSF Dead: 15 PSF

Unfactored Reactions UNPATTERNED Ib (Uplift) Snow

Dead

109

123

Live

291

329

Bearings and Factored Reactions									
Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.			
1 - Hanger	2.000"	36%	136 / 436	573	L	1.25D+1.5L			
2 - Hanger	2.000"	41%	154 / 493	647	L	1.25D+1.5L			

Analysis Results

Г	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
	Moment	389 ft-lb	9 3/4"	6250 ft-lb	0.062 (6%)	1.25D+1.5L	L
	Shear	642 lb	2'7 1/4"	2345 lb	0.274 (27%)	1.25D+1.5L	L
	Perm Defl in.	0.002 (L/19148)	11"	0.083 (L/360)	0.020 (2%)	D	Uniform
	LL Defl inch	0.004 (L/7173)	11"	0.083 (L/360)	0.050 (5%)	L	L
	TL Defl inch	0.006 (L/5218)	11"	0.125 (L/240)	0.050 (5%)	D+L	L

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.002", Long Term = 0.002"
- 3 Fill all hanger nailing holes.
- 4 See manufacture installation guide note E4 for installation details
- 5 Girders are designed to be supported on the bottom edge only.
- 6 Top flange braced at bearings.
- 7 Bottom flange braced at bearings



0

0

December 18, 2018

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 2-8-8	(Span)1-3-7	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	0-9-12		Near Face	109 lb	291 lb	0 lb	0 lb	J6
3	Point	2-1-12		Near Face	97 lb	259 lb	0 lb	0 lb	J6

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

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This design is valid until 10/31/2020

Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325 www.lpcorp.com CCMC: 12412-R APA: PR-L238C Kott Lumber Company 14 Anderson Blvd, Ontario 905-642-4400







Client:

Project: Address: Date: 12/17/2018

Designer: SB

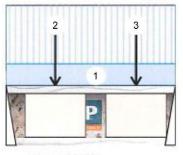
Job Name: LOT-6 (AMELIA 3 EL-1 _5BEDRM)

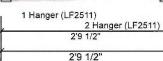
Project #:

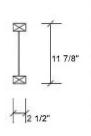
LPI 20Plus

11.875" - PASSED

Level: Ground Floor







Wind 0

Member Information						
Type:	Girder	Application:	Floor (Residential)			
Plies:	1	Design Method:	LSD			
Moisture Condition	n: Dry	Building Code:	NBCC 2010 / OBC 2012			
Deflection LL:	360	Load Sharing:	No			
Deflection TL:	240	Deck:	Not Checked			
Importance:	Normal	Vibration:	Not Checked			
General Load						
Floor Live:	40 PSF					
Dead:	15 PSF					

Unfacto	red Reaction	S UNPATTER	NED lb (Uplift)
Brg	Live	Dead	Snow
1	304	114	0
2	328	123	0
Bearing	s and Factore	d Reactions	

Analysis Res	Analysis Results								
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case			
Moment	407 ft-lb	9 3/4"	6250 ft-lb	0.065 (7%)	1.25D+1.5L	L			
Shear	641 lb	2'8 1/4"	2345 lb	0.273 (27%)	1.25D+1.5L	L			
Perm Defl in.	0.002 (L/18458)	1'1 3/8"	0.086 (L/360)	0.020 (2%)	D	Uniform			
LL Defl inch	0.004 (L/6920)	1'1 3/8"	0.086 (L/360)	0.050 (5%)	L	L			
TL Defl inch	0.006 (L/5033)	1'1 3/8"	0.129 (L/240)	0.050 (5%)	D+L	L			

Bearing	Bearings and Factored Reactions								
Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.			
1 - Hanger	2.000"	38%	142 / 455	597	L	1.25D+1.5L			
2 - Hanger	2.000"	41%	154 / 492	646	L	1.25D+1.5L			



December 18, 2018

_								
1	Provide	restraint	at	supports	to	ensure	lateral	stability

- 2 Dead Load Deflection: Instant = 0.002", Long Term = 0.003"
- 3 Fill all hanger nailing holes.

Design Notes

- 4 See manufacture installation guide note E4 for installation details
- 5 Girders are designed to be supported on the bottom edge only.
- 6 Top flange braced at bearings.
- 7 Bottom flange braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 2-9-8	(Span)1-3-7	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	0-9-12		Near Face	109 lb	291 lb	0 lb	0 lb	J6
3	Point	2-1-12		Near Face	101 lb	269 lb	0 lb	0 lb	J6

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

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This design is valid until 10/31/2020

Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325 www.lpcorp.com CCMC: 12412-R APA: PR-L238C







Client:

Project: Address:

12/17/2018 Date:

SB Designer:

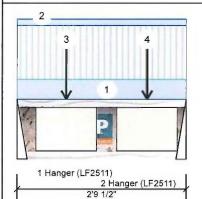
Job Name: LOT-6 (AMELIA 3 EL-1 _5BEDRM)

Project #:

LPI 20Plus

11.875" - PASSED

Level: Ground Floor



2'9 1/2"

11 7/8"

Wind

0

			-
Mer	nber	Info	rmation

Γ	Туре:	Girder	Application:	Floor (Residential)
l	Plies:	1	Design Method:	LSD
l	Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
l	Deflection LL:	360	Load Sharing:	No
l	Deflection TL:	240	Deck:	Not Checked
l	Importance:	Normal	Vibration:	Not Checked
l	General Load			
l	Floor Live:	40 PSF		
l	Dead:	15 PSF		

Unfactored Reactions UNPATTERNED Ib (Uplift)

Dead

148

Paprings	and Easteres	Donations			_
2	328	161	0	0	

Analysis Results

1							
Γ	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
l	Moment	436 ft-lb	9 3/4"	6250 ft-lb	0.070 (7%)	1.25D+1.5L	L
l	Shear	689 lb	2'8 1/4"	2345 lb	0.294 (29%)	1.25D+1.5L	L
	Perm Defl in.	0.002 (L/14174)	1'1 9/16"	0.086 (L/360)	0.030 (3%)	D	Uniform
l	LL Defl inch	0.004 (L/6920)	1'1 3/8"	0.086 (L/360)	0.050 (5%)	L	L
l	TL Defl inch	0.007 (L/4650)	1'1 7/16"	0.129 (L/240)	0.050 (5%)	D+L	L

Live

304

Brg

1

Dearings	anura	ctorear	(Cactions				
Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.	
1 - Hanger	2.000"	40%	185 / 455	640	L	1.25D+1.5L	
2 - Hanger	2.000"	44%	202 / 492	694	L	1.25D+1.5L	

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.002", Long Term = 0.003"
- 3 Fill all hanger nailing holes.
- 4 See manufacture installation guide note E4 for installation details
- 5 Girders are designed to be supported on the bottom edge only.
- 6 Top flange braced at bearings.

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT

CONTAINS SPECIFICATIONS AND CRITERIA USED



Snow

0

December 18, 2018

Comments

/ Bottom fi	lange braced at bearings.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind
1	Tie-In	0-0-0 to 2-9-8	(Span)1-3-7	Тор	15 PSF	40 PSF	0 PSF	0 PSF
2	Part. Uniform	0-0-0 to 2-9-8		Тор	3 PLF	0 PLF	0 PLF	0 PLF
3	Point	0-9-12		Far Face	141 lb	291 lb	0 lb	0 lb
4	Point	2-1-12		Far Face	133 lb	269 lb	0 lb	PALL

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

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IN THE DESIGN OF THIS COMPONENT.

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Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325 www.lpcorp.com CCMC: 12412-R APA: PR-L238C

Manufacturer Info

0 PLF 0 lb J6









Client: Project:

Address:

12/17/2018 Date:

Designer:

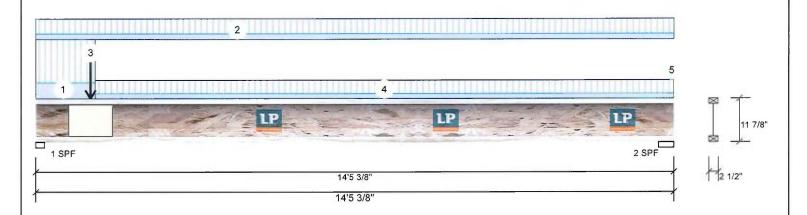
Job Name: LOT-6 (AMELIA 3 EL-1 _5BEDRM)

Project #:

LPI 20Plus

11.875" - PASSED

Level: Ground Floor



ti	ion							Unfacto	red React	ions U	INPATTERN	ED lb	(Uplift)	
Sirc	der			Application	on: F	loor (Resident	ial)	Brg	Live	_	Dead	Sno	w	Wind
				Design M	lethod: L	SD		1	594		223		0	0
у	y			Building (Code: N	BCC 2010 / O	BC 2012	2	305		114		0	0
60	0			Load Sha	aring: N	0								
40	0			Deck:	N	ot Checked								
lor	rmal			Vibration:	. N	ot Checked								
										_				
0 F	PSF							Bearing	s and Fact	ored	Reactions			
5 F	PSF							Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
								1 - SPF	2.375"	71%	278 / 891	1169	L	1.25D+1.5L
								2 - SPF	4.125"	33%	143 / 457	600	L	1.25D+1.5L
										-				
ıl		Lo	ocation	Allowed	Capacity	Comb.	Case	577				100	SERVICE STREET	
t-It	lb		6'6"	6250 ft-lb	0.355 (35%) 1.25D+1.5L	L				0	HOLES	SIONA	
b			1 5/8"	2345 lb	0.490 (49%) 1.25D+1.5L	L				10	_	- V	c.
(L/	/3014)	6'1	11 1/16"	0.468 (L/360)	0.120 (12%) D	Uniform				18 C			3
(L/	/1130)	6'1	11 1/16"	0.468 (L/360)	0.320 (32%) L	L				CE	MATH	EVIC	ENGINEE
(L/	/822)	6'1	11 1/16"	0.702 (L/240)	0.290 (29%) D+L	L				12	.IVIATU	LVIC	EER
(L/	./822)	6'1	11 1/16"	0.702 (L/240)	0.290 (29%) D+L	L	1				3 1	100528	100528832

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2218 ft-lb	6'6"	6250 ft-lb	0.355 (35%)	1.25D+1.5L	L
Shear	1148 lb	1 5/8"	2345 lb	0.490 (49%)	1.25D+1.5L	L
Perm Defl in.	0.056 (L/3014)	6'11 1/16"	0.468 (L/360)	0.120 (12%)	D	Uniform
LL Defl inch	0.149 (L/1130)	6'11 1/16"	0.468 (L/360)	0.320 (32%)	L	L
TL Defl inch	0.205 (L/822)	6'11 1/16"	0.702 (L/240)	0.290 (29%)	D+L	L

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.056", Long Term = 0.084"
- 3 See manufacture installation guide note E4 for installation details
- 4 Girders are designed to be supported on the bottom edge only.

5 Top flange must be laterally braced at a maximum of 7' o.c.	
6 Bottom flange braced at bearings.	

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-4-2	(Span)2-11-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 14-5-6	(Span)1-0-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	1-2-14		Far Face	109 lb	291 lb	0 lb	0 lb	F14
4	Tie-In	1-4-2 to 14-5-6	(Span)0-11-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
5	Part. Uniform	14-4-3 to 14-5-6		Тор	2 PLF	0 PLF	0 PLF		s-Thru Framing Squash Block is iired at all point loads over bearings

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325 www.lpcorp.com CCMC: 12412-R APA: PR-L238C Kott Lumber Company 14 Anderson Blvd, Ontario Canada 905-642-4400

Refer to Multiple Member Connection

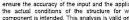
Detail for ply to ply nailing or bolting

requirements

340 VINCE OF ONTARIO

December 18, 2018





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This design is valid until 10/31/2020





Client:

Project: Address:

12/17/2018 Date:

SB Designer:

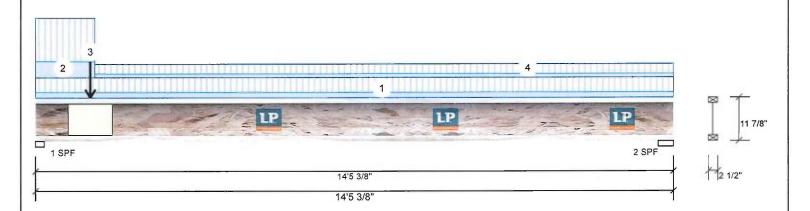
Job Name: LOT-6 (AMELIA 3 EL-1 _5BEDRM)

Project #

LPI 20Plus

11,875" - PASSED

Level: Ground Floor



Member Info	rmation						Unfacto	red React	ions U	NPATTERN	IED lb	(Uplift)	
Туре:	Girder		Applicatio	n: F	loor (Resident	ial)	Brg	Live		Dead	Sno	w	Wind
Plies:	1		Design M	ethod: L	SD		1	600		225		0	0
Moisture Conditio	on: Dry		Building C	Code: N	BCC 2010 / C	BC 2012	2	271		102		0	0
Deflection LL:	360		Load Sha	ring: N	o								
Deflection TL:	240		Deck:	N	ot Checked								
Importance:	Normal		Vibration:	N	ot Checked								
General Load													
Floor Live:	40 PSF						Bearing	s and Fact	ored F	Reactions			
Dead:	15 PSF						Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
							1 - SPF	2.375"	72%	281 / 899	1180	L	1.25D+1.5L
							2-SPF	4.125"	29%	127 / 407	534	L	1.25D+1.5L
Analysis Resu	lts							-					
Analysis A	ctual	Location	Allowed	Capacity	Comb.	Case					FESS	0	
Moment 20	028 ft-lb	6'3 5/8"	6250 ft-lb	0.325 (32%) 1.25D+1.5L	L				PAC	, 2007	ONAL	
Shear 11	159 lb	1 5/8"	2345 lb	0.494 (49%) 1.25D+1.5L	L				10		1.0	
Perm Defl in. 0.	.051 (L/3291)	6'10 7/16"	0.468 (L/360)	0.110 (11%) D	Uniform				15		7 6	1
LL Defl inch 0.	.137 (L/1233)	6'10 7/16"	0.468 (L/360)	0.290 (29%) L	L			- 1	LICENSE W.I 100	ATIJEV	IC CAC	2
TL Defl inch 0.	.188 (L/897)	6'10 7/16"	0.702 (L/240)	0.270 (27%) D+L	L				100	52883		20
Docian Notes							7		- 1	1	-		~/

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2028 ft-lb	6'3 5/8"	6250 ft-lb	0.325 (32%)	1.25D+1.5L	L
Shear	1159 lb	1 5/8"	2345 lb	0.494 (49%)	1.25D+1.5L	L
Perm Defl in.	0.051 (L/3291)	6'10 7/16"	0.468 (L/360)	0.110 (11%)	D	Uniform
LL Defl inch	0.137 (L/1233)	6'10 7/16"	0.468 (L/360)	0.290 (29%)	L	L
TL Defl inch	0.188 (L/897)	6'10 7/16"	0.702 (L/240)	0.270 (27%)	D+L	L

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.051", Long Term = 0.077"
- 3 See manufacture installation guide note E4 for installation details
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top flange must be laterally braced at a maximum of 7'3" o.c.

6 Battom flance broad at bearings

	o bottom narige	braceu at bearings.								
Ī	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
	1	Tie-In	0-0-0 to 14-5-6	(Span)1-0-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	2	Tie-In	0-0-0 to 1-4-2	(Span)2-11-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	3	Point	1-2-14		Near Face	123 lb	329 lb	0 lb	0 lb	F14
	4	Tie-In	1-4-2 to 14-5-6	(Span)0-8-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT

CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Notes

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This design is valid until 10/31/2020

Manufacturer Info

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Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection

Detail for ply to ply nailing or bolting

SAOVINCE OF ONTARIO

December 18, 2018

requirements







Client:

Project: Address:

12/17/2018 Date:

Designer:

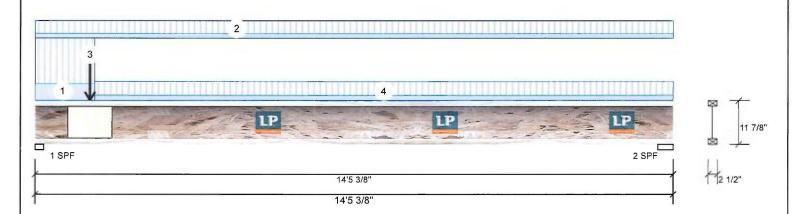
Job Name: LOT-6 (AMELIA 3 EL-1 _5BEDRM)

Project #:

LPI 20Plus

11.875" - PASSED

Level: Ground Floor



nation			Unfactore	d React	ions UN	PATTERNI	ED lb (l	Uplift)	
Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snow	1	Wind
1	Design Method:	LSD	1	584		219	C)	0
Dry	Building Code:	NBCC 2010 / OBC 2012	2	281		106	C)	0
360	Load Sharing:	No	1.55						
240	Deck:	Not Checked							
Normal	Vibration:	Not Checked							
40 PSF			Bearings a	and Fact	ored Re	actions			
15 PSF			Bearing L	.ength	Cap. R	leact D/L lb	Total	Ld. Case	Ld. Comb.
			1 - SPF 2	.375"	70%	274 / 876	1150	L	1.25D+1.5L
			2 - SPF 4	.125"	30%	132 / 422	554	L	1.25D+1.5L
	Girder 1 Dry 360 240 Normal	Girder Application: Design Method: Building Code: Load Sharing: Deck: Vibration: 40 PSF 15 PSF	Girder Application: Floor (Residential) 1 Design Method: LSD Dry Building Code: NBCC 2010 / OBC 2012 360 Load Sharing: No 240 Deck: Not Checked Normal Vibration: Not Checked 40 PSF 15 PSF	Application: Floor (Residential) Brg	Application: Floor (Residential) Brg Live	Application: Floor (Residential) Brg Live	Application: Floor (Residential) Brg Live Dead	Application: Floor (Residential) Brg Live Dead Snow	Application: Floor (Residential) Brg Live Dead Snow

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2075 ft-lb	6'4 7/8"	6250 ft-lb	0.332 (33%)	1.25D+1.5L	L
Shear	1130 lb	1 5/8"	2345 lb	0.482 (48%)	1.25D+1.5L	L
Perm Defl in.	0.052 (L/3218)	6'10 3/4"	0.468 (L/360)	0.110 (11%)	D	Uniform
LL Defl inch	0.140 (L/1207)	6'10 3/4"	0.468 (L/360)	0.300 (30%)	L	L
TL Defl inch	0.192 (L/878)	6'10 3/4"	0.702 (L/240)	0.270 (27%)	D+L	L

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.052", Long Term = 0.078"
- 3 See manufacture installation guide note E4 for installation details
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top flange must be laterally braced at a maximum of 7'2" o.c.

o Bollo	in nange braceu at bearing:	5.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-4-2	(Span)3-0-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 14-5-6	(Span)0-10-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	1-2-14		Far Face	114 lb	304 lb	o Ib	0 lb	F14
4	Tie-In	1-4-2 to 14-5-6	(Span)0-11-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

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This design is valid until 10/31/2020



December 18, 2018

Pass-Thru Framing Squash Block is

required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Manufacturer Info

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Client: Project: Address:

12/17/2018 Date:

Designer: SB

Job Name: LOT-6 (AMELIA 3 EL-1 _5BEDRM)

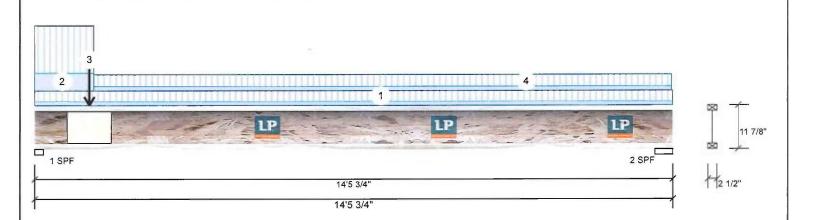
Unfactored Reactions UNPATTERNED In (Unlift)

Project #:

LPI 20Plus

11.875" - PASSED

Level: Ground Floor



Member Illioi	rmation			offiactored Reactions ONPATTERNED ID (Opint)							
Туре:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snov	N	Wind	
Plies:	1	Design Method:	LSD	1	562		211		0	0	
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	236		88		0	0	
Deflection LL:	360	Load Sharing:	No	150							
Deflection TL:	240	Deck:	Not Checked								
Importance:	Normal	Vibration:	Not Checked								
General Load				_							
Floor Live:	40 PSF			Bearings	and Fac	tored I	Reactions				
Dead:	15 PSF			Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.	
				1 - SPF	2.375"	68%	264 / 844	1107	L	1.25D+1.5L	
				2 - SPF	4.875"	25%	111 / 354	464	L	1.25D+1.5L	

Analysis Results

Member Information

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1783 ft-lb	6'1 11/16"	6250 ft-lb	0.285 (29%)	1.25D+1.5L	L
Shear	1088 lb	1 5/8"	2345 lb	0.464 (46%)	1.25D+1.5L	L
Perm Defl in.	0.045 (L/3744)	6'9 13/16"	0.467 (L/360)	0.100 (10%)	D	Uniform
LL Defl inch	0.120 (L/1404)	6'9 13/16"	0.467 (L/360)	0.260 (26%)	L	L
TL Defl inch	0.165 (L/1021)	6'9 13/16"	0.700 (L/240)	0.240 (24%)	D+L	L

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.045", Long Term = 0.067"
- 3 See manufacture installation guide note E4 for installation details
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top flange must be laterally braced at a maximum of 7'8" o.c.

6 BC	ottom nange praced at bearings.				1.0				
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 14-5-12	(Span)0-8-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 1-4-2	(Span)3-0-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	1-2-14		Near Face	123 lb	328 lb	0 lb	0 lb	F14
4	Tie-In	1-4-2 to 14-5-6	(Span)0-9-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

This design is valid until 10/31/2020



December 18, 2018

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting

requirements

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the receiver little of

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Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325

www.lpcorp.com CCMC: 12412-R APA: PR-L238C







Client:

Project: Address: Date: 12/17/2018

Designer: SB

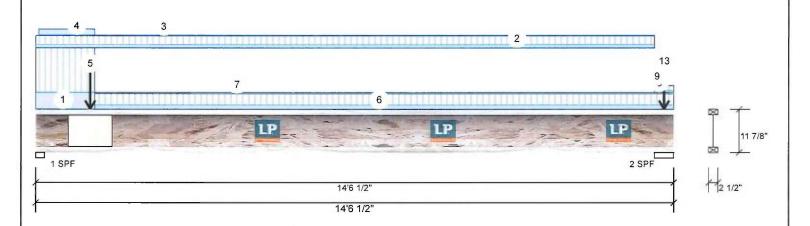
Job Name: LOT-6 (AMELIA 3 EL-1_5BEDRM)

Project #

F15-E LPI 20Plus

11.875" - PASSED

Level: Ground Floor



Member Information Unfactored Reactions UNPATTERNED Ib (Uplift) Type: Girder Floor (Residential) Brg Live Dead Snow Wind Application: Plies: 1 Design Method: LSD 551 270 0 0 Moisture Condition: Dry NBCC 2010 / OBC 2012 Building Code: 2 561 280 0 Deflection LL: 360 Load Sharing: No Deflection TL: 240 Deck: Not Checked Importance: Normal Vibration: Not Checked General Load **Bearings and Factored Reactions** Floor Live: 40 PSF Cap. React D/L lb Dead: 15 PSF Bearing Length Total Ld. Case Ld. Comb. 1 - SPF 2.375" 71% 338 / 827 1.25D+1.5L 1164 L 2 - SPF 5.250" 65% 350 / 842 1191 L 1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1834 ft-lb	6'1 1/8"	6250 ft-lb	0.293 (29%)	1.25D+1.5L	L
Shear	1144 lb	1 5/8"	2345 lb	0.488 (49%)	1.25D+1.5L	L
Perm Defl in.	0.056 (L/2990)	6'9 3/4"	0.468 (L/360)	0.120 (12%)	D	Uniform
LL Defl inch	0.115 (L/1461)	6'9 3/4"	0.468 (L/360)	0.250 (25%)	L	L
TL Defl inch	0.172 (L/982)	6'9 3/4"	0.702 (L/240)	0.240 (24%)	D+L	L

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Applied loads over end bearings and loads exceeding 250 lbs over intermediate bearings must be transferred directly to the support by rim board, blocking, squash blocks, or other device.
- 3 Dead Load Deflection: Instant = 0.056", Long Term = 0.084"
- 4 See manufacture installation guide note E4 for installation details
- 5 Girders are designed to be supported on the bottom edge only.
- 6 Top flange must be laterally braced at a maximum of 7'7" o.c.

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



December 18, 2018

7 Bottom	flange braced at bearings	3.	-			51.52		
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind Comments
1	Tie-In	0-0-0 to 1-4-2	(Span)3-0-0 to 3-0-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF
2	Tie-In	0-0-0 to 14-1-4	(Span)0-7-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF
3	Part. Uniform	0-0-10 to 14-1-4		Тор	1 PLF	0 PLF	0 PLF	O Bass-Thru Framing Squash Block is required at all point loads over bearings
4	Part, Uniform	0-0-12 to 1-4-2		Тор	8 PLF	0 PLF	0 PLF	o PLF
5 continued or	Point n page 2	1-2-14		Far Face	161 lb	328 lb	0 lb	Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

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Manufacturer Info

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Client:

Project: Address: Date: 12/17/2018

Designer: S B

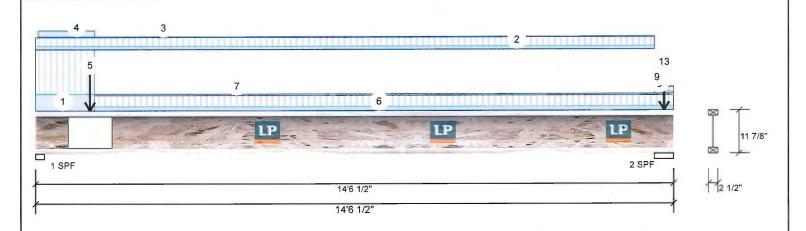
Job Name: LOT-6 (AMELIA 3 EL-1 _5BEDRM)

Project #:

F15-E LPI 20Plus

11.875" - PASSED

Level: Ground Floor



Continued f	rom page 1								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
6	Tie-In	1-4-2 to 14-6-8	(Span)0-9-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
7	Part. Uniform	1-4-2 to 14-5-1		Тор	2 PLF	0 PLF	0 PLF	0 PLF	
8	Tie-In	14-1-4 to 14-6-8	(Span)0-4-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
9	Part, Uniform	14-1-4 to 14-5-1		Тор	1 PLF	0 PLF	0 PLF	0 PLF	
10	Point	14-3-14		Тор	50 lb	134 lb	0 lb	0 lb	J6
	Bearing Length	0-1-8							
11	Point	14-3-14		Тор	31 lb	80 lb	0 lb	0 lb	J1
	Bearing Length	0-1-8							
12	Point	14-3-14		Тор	47 lb	124 lb	0 lb	0 lb	J6
	Bearing Length	0-1-8							
13	Point	14-3-14		Тор	43 lb	0 lb	0 lb	0 lb	Wall Self Weight
	Bearing Length	0-1-8							



Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this raport. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

IN THE DESIGN OF THIS COMPONENT.

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED

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Client:

Project: Address:

12/17/2018 Date:

SB Designer:

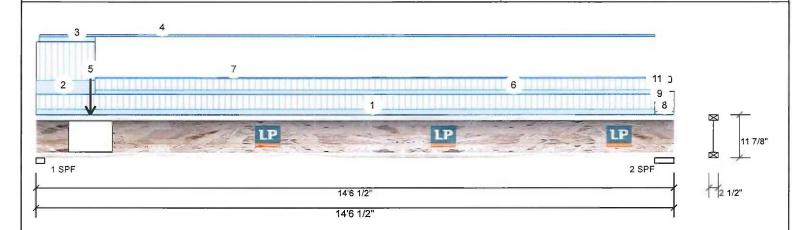
Job Name: LOT-6 (AMELIA 3 EL-1 _5BEDRM)

Project #:

LPI 20Plus

11.875" - PASSED

Level: Ground Floor



Floor (Residential)

Not Checked

Not Checked

NBCC 2010 / OBC 201

LSD

No

Member	Information
Type:	Girder
m	

Plies: Moisture Condition: Dry Deflection LL: 360 Deflection TI · 240 Importance: Normal General Load

Floor Live: 40 PSF Dead: 15 PSF

Unfactored Reactions UNPATTERNED lb (Uplift)

	Brg	Live	Dead	Snow	Wind
	1	632	311	0	0
12	2	327	162	0	0
	1				

Bearings and Factored Reactions

Bearing Le	ength (Cap. Rea	ct D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF 2.	375"	82%	389 / 948	1336	L	1.25D+1.5L
2 - SPF 5.	250"	38%	202 / 491	694	I	1 25D+1 5I

Analysis Results

Analysis Actual Location Allowed Capacity Comb. Case Moment 2576 ft-lb 6'6 5/16" 6250 ft-lb 0.412 (41%) 1.25D+1.5L L 1313 lb 1.5/8" 2345 lb 0.560 (56%) 1.25D+1.5L L Shear Perm Defl in. 0.079 (L/2119) 6'11 3/16" 0.468 (L/360) 0.170 (17%) D Uniform LL Defl inch 0.161 (L/1047) 6'11 3/16" 0.468 (L/360) 0.340 (34%) L TL Defl inch 0.240 (L/701) 6'11 3/16" 0.702 (L/240) 0.340 (34%) D+L

Application:

Design Method:

Building Code:

Load Sharing:

Deck:

Vibration:

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.079", Long Term = 0.119"
- 3 See manufacture installation guide note E4 for installation details
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top flange must be laterally braced at a maximum of 6'6" o.c.

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



December 18, 2018

6 Bottom II	ange praced at bearing	S,						
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind Comments
1	Tie-In	0-0-0 to 14-1-4	(Span)1-2-0 to 1-2-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF
2	Tie-In	0-0-0 to 1-4-2	(Span)3-0-0 to 3-0-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF
3	Part. Uniform	0-0-14 to 1-4-2		Тор	8 PLF	0 PLF	0 PLF	0 PLF
4	Part. Uniform	0-0-15 to 14-1-4		Тор	3 PLF	0 PLF	0 PLF	⁰ Pass-Thru Fram
5	Point	1-2-14		Near Face	148 lb	304 lb	0 lb	required at all po
6	Tie-In	1-4-2 to 14-1-4	(Span)0-11-0 to 0-11-0	Тор	15 PSF	40 PSF	0 PSF	Refer to Multiple

Continued on page 2...

hru Framing Squash Block is d at all point loads over bearings

Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

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Manufacturer Info

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Client:

Project: Address:

12/17/2018 Date:

Designer: SB

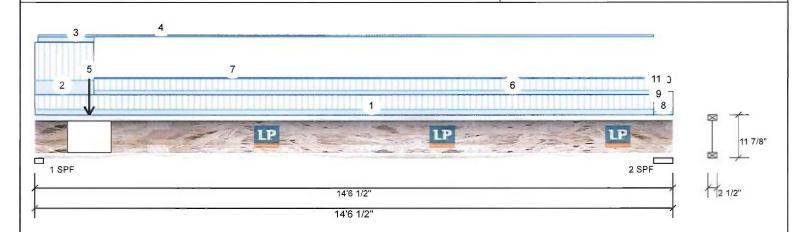
Job Name: LOT-6 (AMELIA 3 EL-1 _5BEDRM)

Project #:

LPI 20Plus

11.875" - PASSED

Level: Ground Floor



Continued fro	m page 1								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
7	Part. Uniform	1-4-2 to 14-1-4		Тор	2 PLF	0 PLF	0 PLF	0 PLF	
8	Tie-In	14-1-4 to 14-6-8	(Span)0-8-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
9	Tie-In	14-1-4 to 14-6-8	(Span)0-8-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
10	Part, Uniform	14-1-4 to 14-5-5		Тор	2 PLF	0 PLF	0 PLF	0 PLF	
11	Part. Uniform	14-1-4 to 14-5-4		Тор	2 PLF	0 PLF	0 PLF	0 PLF	



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2, THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

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Client:

Project: Address: Date: 12/17/2018

Designer: SB

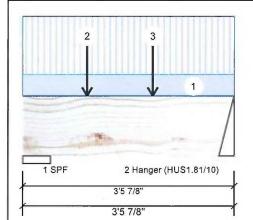
Job Name: LOT-6 (AMELIA 3 EL-1 _5BEDRM)

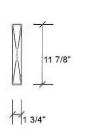
Project #

Forex 2.0E-3000Fb LVL

1.750" X 11.875" - PASSED

Level: Ground Floor





Туре:	Girder	Application:	Floor (Residential)
Plies:	1	Design Method:	LSD
Moisture Condition	: Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

Unfactored Reactions UNPATTERNED Ib (Uplift) Brg Live Dead Snow Wind 360 170 0 0 1 259 122 0 0 2

Bearings and Factored Reactions Cap. React D/L lb Bearing Length Total I.d. Case Ld. Comb. 1 - SPF 5.500" 213 / 540 753 L 1.25D+1.5L 13% 14% 152 / 389 541 L 1.25D+1.5L Hanger

Analysis Results Analysis Actual Location Allowed Capacity Comb. 485 ft-lb 0.028 (3%) 1.25D+1.5L L Moment 1'11 1/4" 17130 ft-lb Unbraced 485 ft-lb 1'11 1/4" 13987 ft-lb 0.035 (3%) 1.25D+1.5L L Shear 526 lb 1'4 5/8" 5798 lb 0.091 (9%) 1.25D+1.5L L Perm Defl in. 0.001 1'10 7/16" 0.097 (L/360) 0.010 (1%) D Uniform (L/36212) LL Defl inch 0.002 1'10 5/16" 0.097 (L/360) 0.020 (2%) L L (L/17367) TL Defl inch 0.003 1'10 5/16" 0.145 (L/240) 0.020 (2%) D+L L (L/11738)



December 18, 2018

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.

4 BOROTT E	naced at bearings.								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Part. Uniform	0-0-0 to 3-5-14		Тор	30 PLF	80 PLF	0 PLF	0 PLF	
2	Point	1-0-12		Far Face	102 lb	206 lb	0 lb	0 lb	J4
3	Point	2-1-12		Far Face	69 lb	134 lb	0 lb	0 lb	J2
	Self Weight				5 PLF				Thru Framing Squash Block is ed at all point loads over bearings

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

chemicals

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- andling & Installation
 LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
 approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent



This design is valid until 10/18/2021

Manufacturer Info

Forex APA: PR-L318







Client:

Project: Address:

12/17/2018 Date:

Designer:

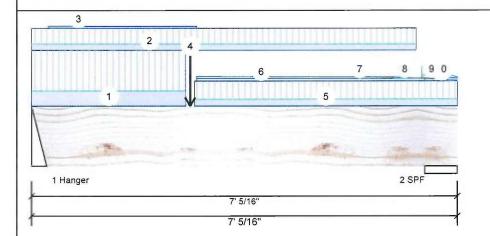
Job Name: LOT-6 (AMELIA 3 EL-1 _5BEDRM)

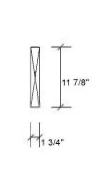
Level: Ground Floor

Project #:

Forex 2.0E-3000Fb LVL

1,750" X 11,875" - PASSED





Wind

0

Member Information					
Туре:	Girder				
Plies:	1				
Moisture Condition:	Dry				
Deflection LL:	360				
Deflection TL:	240				
Importance:	Normal				
General Load					

40 PSF 15 PSF

Floor (Residential) Application: LSD Design Method: NBCC 2010 / OBC 2012 Building Code: Load Sharing: Deck: Not Checked Vibration: Not Checked

Brg 1 2

Unfactored Reactions UNPATTERNED Ib (Uplift) Live Dead Snow 410 199 0 295 156

Bearings and Factored Reactions

9%

2 - SPF 6.438"

Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 3.000" 22% 248 / 615 864 L 1.25D+1.5L Hanger

Analysis Results

Floor Live:

Dead:

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1478 ft-lb	2'7 7/16"	17130 ft-lb	0.086 (9%)	1.25D+1.5L	L
Unbraced	1478 ft-lb	2'7 7/16"	7067 ft-lb	0.209 (21%)	1.25D+1.5L	L
Shear	649 lb	1'2 1/8"	5798 lb	0.112 (11%)	1.25D+1.5L	L
Perm Defl in.	0.006 (L/11753)	3' 11/16"	0.212 (L/360)	0.030 (3%)	D	Uniform
LL Defl inch	0.013 (L/5910)	3' 1/8"	0.212 (L/360)	0.060 (6%)	L	L
TL Defl inch	0.019 (L/3933)	3' 5/16"	0.318 (L/240)	0.060 (6%)	D+L	L

195 / 442 637 L

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.
- 4 Bottom braced at bearings.

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



December 18, 2018

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind Comments
1	Tie-In	0-0-0 to 2-6-9	(Span)3-1-13	Тор	15 PSF	40 PSF	0 PSF	0 PSF
2	Tie-In	0-0-0 to 6-4-2	(Span)1-3-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF
3	Part. Uniform	0-3-4 to 2-8-10		Тор	3 PLF	0 PLF	0 PLF	0 PLF
4	Point	2-7-7		Far Face	122 lb	259 lb	0 lb	0 lb F6
5	Tie-In	2-8-5 to 7-0-5	(Span)1-5-0	Тор	15 PSF	40 PSF	0 PSF	oPass-Thru Framing Squash Block is
6	Part. Uniform	2-8-10 to 6-5-14		Тор	4 PLF	0 PLF	0 PLF	required at all point loads over bearings
7	Tapered Start	2-8-10		Тор	3 PLF	0 PLF	0 PLF	Refer to Multiple Member Connection
Continued on	page 2							Detail for ply to ply nailing or bolting

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation

chemicals

- LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
- approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding



This design is valid until 10/18/2021

Manufacturer Info

requirements

Forex APA: PR-L318







Client:

Project: Address:

12/17/2018 Date:

Designer: SB

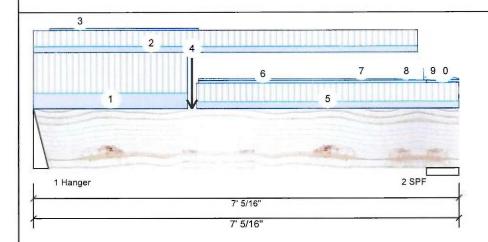
Job Name: LOT-6 (AMÉLIA 3 EL-1 _5BEDRM)

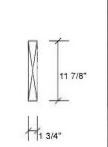
Project #:

Forex 2.0E-3000Fb LVL F7-B

1.750" X 11.875" - PASSED

Level: Ground Floor





Side Dead Live Snow Wind Comments
2 PLF 0 PLF 0 PLF 0 PLF
Top 2 PLF 0 PLF 0 PLF 0 PLF
OPLF OPLF OPLF
Top 15 PSF 40 PSF 0 PSF 0 PSF
Top 4 PLF 0 PLF 0 PLF 0 PLF
1 PLF 0 PLF 0 PLF 0 PLF
5 PLF
Top



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to venfy the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

andling & Installation
LVL beams must not be cut or drilled
Refer to manufacturer's product information
regarding installation requirements, multi-ply
fastening details, beam strength values, and code
approvals
Damaged Beams must not be used
Design assumes top adge is laterally restrained
Provide lateral support at beaming points to avoid
lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding



This design is valid until 10/18/2021

Manufacturer Info

Forex APA: PR-L318







Client: Project:

Address:

12/17/2018 Date:

SB Designer:

Job Name: LOT-6 (AMELIA 3 EL-1 _5BEDRM)

Level: Ground Floor

Project #:

Forex 2.0E-3000Fb LVL

1.750" X 11.875" - PASSED

2 3 6 8 4 1 SPF End Grain 2 Hanger (HUS1.81/10) 9' 9/16'

Member Information

Type: Girder Plies: 1 Moisture Condition: Dry Deflection LL: 360 Load Sharing: Deflection TL: 240 Deck: Importance: Normal Vibration: General Load Floor Live: 40 PSF

15 PSF

Application: Floor (Residential) Design Method: Building Code: NBCC 2010 / OBC 2012

9' 9/16'

No Not Checked Not Checked Unfactored Reactions UNPATTERNED lb (Uplift) Brg Live Dead Snow

1	1200	535	0	0
2	1290	544	0	0

Bearings and Factored Reactions

67%

Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1 - SPF 3.500" 54% 668 / 1800 2468 L 1.25D+1.5L End Grain

680 / 1936

Analysis Results

Dead:

	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
	Moment	5061 ft-lb	5'4 3/4"	17130 ft-lb	0.295 (30%)	1.25D+1.5L	L
	Unbraced	5061 ft-lb	5'4 3/4"	5210 ft-lb	0.971 (97%)	1.25D+1.5L	L
	Shear	2425 lb	7'10 7/16"	5798 lb	0.418 (42%)	1.25D+1.5L	L
	Perm Defl in.	0.035 (L/2951)	4'8 1/8"	0.288 (L/360)	0.120 (12%)	D	Uniform
	LL Defl inch	0.081 (L/1283)	4'8 9/16"	0.288 (L/360)	0.280 (28%)	L	L
i	TL Defl inch	0.116 (L/894)	4'8 7/16"	0.432 (L/240)	0.270 (27%)	D+L	L

Hanger

2 -

3.000"

1.25D+1.5L

Wind

Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Fill all hanger nailing holes.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top braced at bearings.

5 Bottom braced at bearings

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



2616 L

December 18, 2018

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 9-0-9	(Span)3-11-7 to 3-11-7	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	0-1-12		Тор	110 lb	248 lb	0 lb	0 lb	C4
3	Point	1-4-12		Far Face	77 lb	158 lb	0 lb	0 lb	J3
4	Part. Uniform	2-0-12 to 4-8-12		Far Face	59 PLF	123 PLF	0 PLF	0 PLF	
6	Point	5-4-12		Far Face	126 lb	296 lb	0 lb	0 lb	J3
7	Point	6-8-12		Far Face	156 lb	393 lb	0 lb	0 lb	J3

Continued on page 2...

Pass-Thru Framing Squash Block is required at all point loads over bearings

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply regarding installation requirements, multi-p fastening details, beam strength values, and con
- amaged Beams must not be used
- Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation



equirements

This design is valid until 10/18/2021

Manufacturer Info

Kett Lumber Company

Keter to Multiple Member Confidence Blvd, Ontario

905-642-4400





Client:

Project: Address: Date: 12/17/2018

Designer: SB

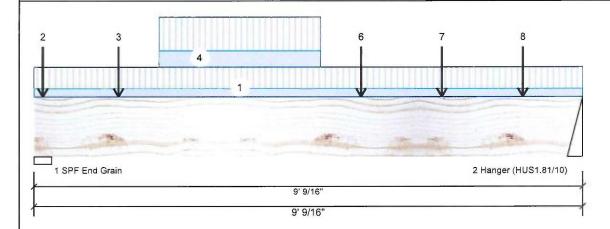
Job Name: LOT-6 (AMELIA 3 EL-1 _5BEDRM)

Project #:

Forex 2.0E-3000Fb LVL

1.750" X 11.875" - PASSED

Level: Ground Floor



Side

Far Face

.Continued from page 1

ID Load Type 8 Point

8-0-12

Location Trib Width

Dead 141 lb

Live 352 lb Snow 0 lb Wind Comments

0 lb

Self Weight 5 PLF

> PROFESSIONAL CONTRACTOR LIMATE SHOWINGE OF ONTERIO December 18, 2018

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

IN THE DESIGN OF THIS COMPONENT.

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ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE

IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVI beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
 approvalls
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding



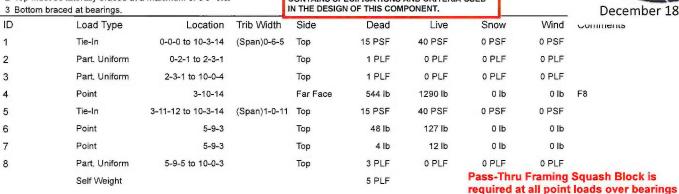
This design is valid until 10/18/2021

Manufacturer Info

Forex APA: PR-L318







Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design cnterna and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply regarding installation requirements, multi-ply fastening details, beam strength values, and code
- smaged Beams must not be used Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prever

This design is valid until 10/18/2021

Manufacturer Info

etail for ply to ply nailing o quirements

Kott Lumber Company bolting 905-642-4400





Version 18.80.219 Powered by iStruct™



Client:

Project: Address:

12/17/2018 Date:

SB Designer:

Job Name: LOT-6 (AMELIA 3 EL-1 _5BEDRM)

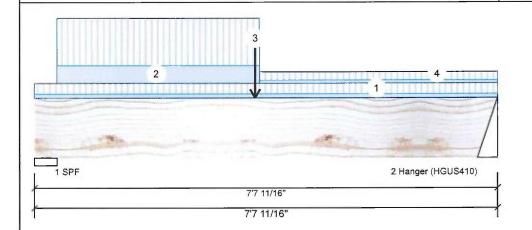
Project #:

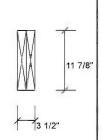
Forex 2.0E-3000Fb LVL

1.750" X 11.875"

2-Ply - PASSED

Level: Second Floor





Wind

0

0

Туре:	Girder	Application:	Floor (Residential)
Plies:	2	Design Method:	LSD
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

Unfactor	red Reactions	UNPATTER	NED lb (Uplift)	
Brg	Live	Dead	Snow	

224

179

489

372

Bearing	s and Fac	tored R	eactions				
Bearing	Length	Cap. F	React D/L lb	Total	Ld. Case	Ld. Comb.	
1-SPF	4.467"	11%	280 / 733	1013	L	1.25D+1.5L	
2	4.000"	8%	224 / 558	782	1	1 25D+1 5I	

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2402 ft-lb	3'7 11/16"	34261 ft-lb	0.070 (7%)	1.25D+1.5L	L
Unbraced	2402 ft-lb	3'7 11/16"	31940 ft-lb	0.075 (8%)	1.25D+1.5L	L
Shear	838 lb	1'3 9/16"	11596 lb	0.072 (7%)	1.25D+1.5L	L
Perm Defl in.	0.006 (L/15375)	3'7 3/4"	0.235 (L/360)	0.020 (2%)	D	Uniform
LL Defl inch	0.012 (L/6820)	3'7 3/4"	0.235 (L/360)	0.050 (5%)	L	L
TL Defl inch	0.018 (L/4725)	3'7 3/4"	0.353 (L/240)	0.050 (5%)	D+L	L

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on full section width.

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

2

Hanger



0

0

December 18, 2018

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 7-7-11	(Span)0-9-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-4-7 to 3-8-9	(Span)3-4-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	3-7-11		Near Face	187 lb	477 lb	0 lb	0 lb	F6
4	Tie-In	3-8-9 to 7-7-11	(Span)0-7-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	Self Weight				10 PLF				ng Squash Block is int loads over bearings

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design enterin and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- chemicals

Handling & Installation

- andling & Installation
 LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
 approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at beaming points to avoid
 lateral displacement and rotation



This design is valid until 10/18/2021

Refer to Multiple Member Connection

Detail for ply to ply nailing authoriting propany
14 Anderson Blvd, Ontario requirements

APA: PR-L318

Canada 905-642-4400





IM1218-096

Page 1 of 2



Client:

Project: Address: Date: 12/17/2018

Designer: SB

Job Name: LOT-6 (AMELIA 3 EL-1 _5BEDRM)

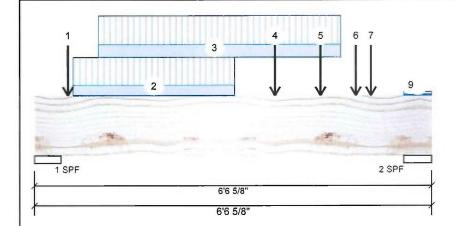
Project #:

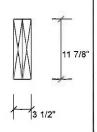
Forex 2.0E-3000Fb LVL

1.750" X 11.875"

2-Ply - PASSED

Level: Second Floor





Wind

Ld. Comb.

1.25D+1.5L

1.25D+1.5L

Member Information

Type:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	
Floor Live:	40 PSF
Dead:	15 PSF

Floor (Residential) Application: Design Method: LSD NBCC 2010 / OBC 2012 **Building Code:**

Load Sharing: No Deck: Not Checked Vibration: Not Checked

Unfactored Reactions UNPATTERNED Ib (Uplift) Dead

Live

Bearing Length

1 - SPF 5.250"

2 - SPF 5.500"

Postina	s and Factored	Doctions		
2	1474	664	0	0
1	1641	717	0	0

896 / 2461

830 / 2211

Cap. React D/L lb

30%

26%

Snow

Analysis Results

Г							
	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
	Moment	4870 ft-lb	3'3 5/8"	34261 ft-lb	0.142 (14%)	1.25D+1.5L	L
	Unbraced	4870 ft-lb	3'3 5/8"	32706 ft-lb	0.149 (15%)	1.25D+1.5L	L
	Shear	3587 lb	5'2"	11596 lb	0.309 (31%)	1.25D+1.5L	L
	Perm Defl in.	0.009 (L/7426)	3'3 5/8"	0.193 (L/360)	0.050 (5%)	D	Uniform
	LL Defl inch	0.021 (L/3252)	3'3 7/16"	0.193 (L/360)	0.110 (11%)	L	L
	TL Defl inch	0.031 (L/2262)	3'3 1/2"	0.289 (L/240)	0.110 (11%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on full section width.

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Total Ld. Case

3357 L

3042 L

December 18, 2018

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Point	0-6-10		Far Face	113 lb	256 lb	0 lb	0 lb	J1
2	Part. Uniform	0-7-10 to 3-3-10		Near Face	105 PLF	280 PLF	0 PLF	0 PLF	
3	Part. Uniform	1-0-10 to 5-0-10		Far Face	127 PLF	286 PLF	0 PLF	0 PLF	
4	Point	3-11-10		Near Face	111 lb	296 lb	0 lb	0 lb	J1
5	Point	4-8-10		Near Face	179 lb	372 lb	0 lb	0 lb	F11
6	Point	5-3-10		Near Face	25 lb	67 lb	0 lb	0 lb	J9

Continued on page 2...

Pass-Thru Framing Squash Block is required at all point loads over bearings

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to venfy the dimensions and loads.

Lumber

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- chemicals

Handling & Installation

- andling & Installation
 LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strongth values, and code
 approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation



This design is valid until 10/18/2021

Reference Multiple Member Competition pany 14 Anderson Blvd, Ontario 484 direments

L4A 7X4 905-642-4400







Client:

Project: Address:

12/17/2018 Date:

Designer: SB

Job Name: LOT-6 (AMELIA 3 EL-1 _5BEDRM)

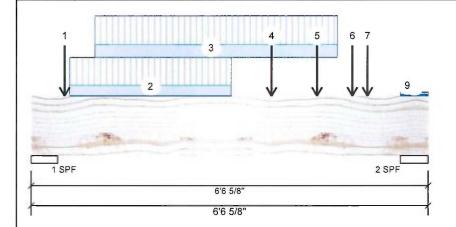
Project #:

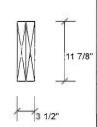
Forex 2.0E-3000Fb LVL

1.750" X 11.875"

2-Ply - PASSED

Level: Second Floor





..Continued from page 1

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
7	Point	5-6-10		Far Face	98 lb	221 lb	0 lb	0 lb	J1
8	Tie-In	6-1-2 to 6-6-10	(Span)0-3-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
9	Tie-In	6-1-2 to 6-6-10	(Span)1-0-1	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	Self Weight				10 PLF				



Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

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READ ALL NOTES ON THIS PAGE AND ON THE

ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED

chemicals

- andling & Installation.

 LVL beams must not be cut or drilled.
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals.

 Damaged Beams must not be used.
 Design assumes top edge is laterally restrained.
 Provide lateral support at bearing points to avoid lateral displacement and rotation.

6. For flat roofs provide proper drainage to prevent



This design is valid until 10/18/2021

Manufacturer Info

Forex APA: PR-L318 Kott Lumber Company 14 Anderson Blvd, Ontario 905-642-4400







ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 10-3-12	(Span)0-11-5	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Part, Uniform	0-2-5 to 10-3-12		Тор	1 PLF	0 PLF	0 PLF	0 PLF	
	Self Weight				10 PLF				aming Squash Block is I point loads over bearings

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- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosis.
- chemicals Handling & Installation

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Provide lateral support at bearing points to avoid
lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent



This design is valid until 10/18/2021

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Manufacturer Info Forex APA: PR-L318

Kott Lumber Company 14 Anderson Blvd, Ontario 905-642-4400







Project: Address:

12/17/2018 Date:

Designer:

Job Name: LOT-6 (AMELIA 3 EL-1 _5BEDRM)

Project #:

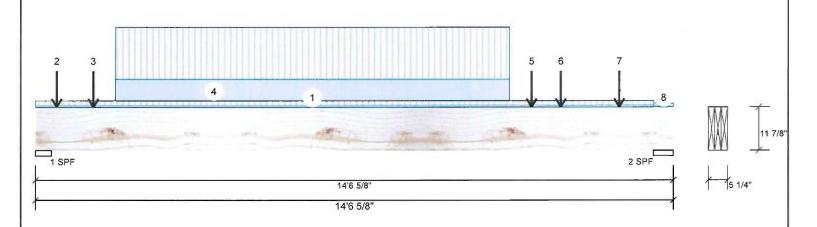
Forex 2.0E-3000Fb LVL

1.750" X 11.875"

3-Ply - PASSED

Level: Second Floor

Unfactored Reactions UNPATTERNED Ib (Uplift)



Type:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind
Plies:	3	Design Method:	LSD	1	2133	983	0	0
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	2054	923	0	0
Deflection LL:	360	Load Sharing:	Yes					
Deflection TL:	240	Deck:	Not Checked					
Importance:	Normal	Vibration:	Not Checked					
General Load								
Floor Live:	40 PSF			Bearings a	and Fac	tored Reactions		
Dead:	15 PSF			Bearing L	.ength	Cap. React D/L lb	Total Ld. Cas	e Ld. Comb.
				1 - SPF 4	.375"	31% 1228 / 3200	4428 L	1.25D+1.5L
2002020				2 - SPF 5	5.500"	24% 1154 / 3081	4234 L	1.25D+1.5L

Analysis Results

Member Information

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	15054 ft-lb	7'2 13/16"	53447 ft-lb	0.282 (28%)	1.25D+1.5L	L
Unbraced	15054 ft-lb	7'2 13/16"	50470 ft-lb	0.298 (30%)	1.25D+1.5L	L
Shear	4341 lb	1'3 1/2"	17394 lb	0.250 (25%)	1.25D+1.5L	L
Perm Defl in.	0.084 (L/1972)	7'2 5/8"	0.462 (L/360)	0.180 (18%)	D	Uniform
LL Defl inch	0.185 (L/899)	7'2 7/8"	0.462 (L/360)	0.400 (40%)	L	L
TL Defl inch	0.269 (L/618)	7'2 13/16"	0.693 (L/240)	0.390 (39%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.

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December 18, 2018

-										
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments	
1	Tie-In	0-0-0 to 14-1-4	(Span)1-3-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
2	Point	0-5-13		Far Face	80 lb	190 lb	0 lb	0 lb	J1	
3	Point	1-3-13		Far Face	107 lb	255 lb	0 lb	0 lb	J1	
4	Part. Uniform	1-9-13 to 10-9-13		Far Face	116 PLF	278 PLF	0 PLF	0 PLF		
5	Point	11-3-13		Far Face	89 lb	232 lb	0 lb	0 lb	J1	
6	Point	11-11-13		Far Face	104 lb	278 lb	0 lb	0 lb	J1	

Continued on page 2...

Pass-Thru Framing Squash Block is required at all point loads over bearings

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the Intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- andling & installation
 LVL beams must not be out or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
 approvals
 Damaged Beams must not be used
 Design assumes top adge is laterally restrained
 Provide lateral support at beaning points to avoid
 lateral displacement and rotation

Manufacturer Info efer to Multiple Member Con etail for ply to ply nailing or quirements

Kott Lumber Company

1166 troops Blvd, Ontario
Canada

120 troops 905-642-4400



This design is valid until 10/18/2021



Client:

Project: Address:

12/17/2018 Date:

Designer: SB

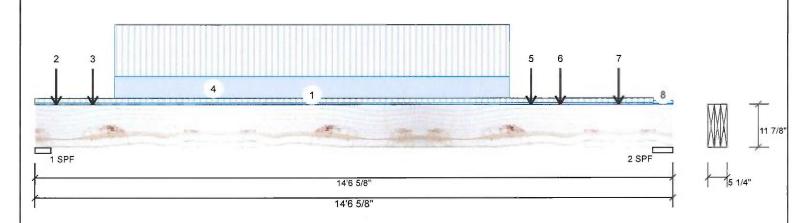
Job Name: LOT-6 (AMELIA 3 EL-1_5BEDRM)

Project #:

Forex 2.0E-3000Fb LVL

1.750" X 11.875"

3-Ply - PASSED Level: Second Floor



..Continued from page 1

D	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
7	Point	13-3-13		Far Face	139 lb	371 lb	0 lb	0 lb	J1
8	Tie-In	14-1-4 to 14-6-10	(Span)0-8-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
l	Self Weight				14 PLF				



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Lumber

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- andling & installation
 LVI, beams must not be cut or drilled
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 fastening details, beam strength values, and code
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 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding



This design is valid until 10/18/2021

Manufacturer Info

Forex APA: PR-L318







Client: Project:

Address:

Date: 12/17/2018

Designer: S B

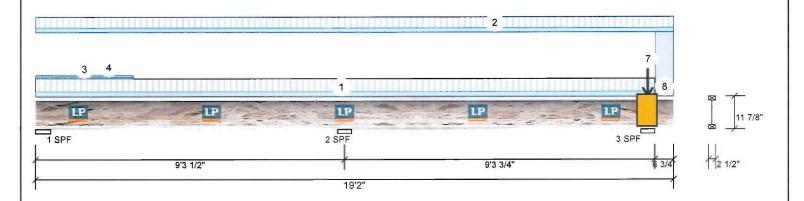
Job Name: LOT-6 (AMELIA 3 EL-1 _5BEDRM)

Project #:

F16-A LPI 20Plus

11.875" - PASSED

Level: Second Floor



Member Information Girder Floor (Residential) Type: Application: Plies: 1 Design Method: LSD NBCC 2010 / OBC 2012 Moisture Condition: Dry **Building Code:** Deflection LL: 360 Load Sharing: No Deflection TL: 240 Deck: Not Checked Importance: Normal Vibration: Not Checked General Load Floor Live: 40 PSF Dead: 15 PSF

Ana	lysis	Results	
Δ III α	I y SIS	I/C3ult3	

L								
ľ	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case	
l	Neg Moment	-577 ft-lb	9'3 1/2"	6250 ft-lb	0.092 (9%)	1.25D+1.5L	LL_	
l	Pos Moment	431 ft-lb	4'1 3/4"	5500 ft-lb	0.078 (8%)	1.25D+1.5L	L	
l	Shear	656 lb	18'7 1/4"	1853 lb	0.354 (35%)	1.25D+1.5S	L	
	Perm Defl in.	0.005 (L/21427)	4'5 3/8"	0.297 (L/360)	0.020 (2%)	D	Uniform	
l	LL Defl inch	0.015 (L/6927)	4'8 1/2"	0.297 (L/360)	0.050 (5%)	L+0.5S	L_L	
l	TL Defl inch	0.020 (L/5239)	4'7 3/4"	0.445 (L/240)	0.050 (5%)	D+L+0.5S	L_L	
	LL Cant	-0.002 (2L/5898)	Rt Cant	0.200 (2L/480)	0.011 (1%)	L	_L_	
١	TL Cant	0.002 (2L/5512)	Rt Cant	0.300 (2L/360)	0.008 (1%)	D+L+0.5S	L_L	

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Applied loads over end bearings and loads exceeding 250 lbs over intermediate bearings must be transferred directly to the support by rim board, blocking, squash blocks, or other device.
- 3 Dead Load Deflection: Instant = 0.005", Long Term = 0.007"
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top flange braced at bearings.
- 6 Bottom flange braced at bearings.

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Unfactored Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	Wind
1	116	54	1	0
2	322	118	0 (-4)	0
3	331	338	201	0

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	5.500"	14%	67 / 193	260	L	1.25D+1.5L
2-SPF	5.000"	16%	149 / 502	652	LL_	1.25D+1.5L
3 - SPF	5.000"	34%	421 / 617	1038	_LL	1.25D+1.5L +0.5S



December 18, 2018

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended, This analysis is valid only for the product listed.

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This design is valid until 10/31/2020

Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325 www.lpcorp.com CCMC: 12412-R APA: PR-L238C







Client: Project:

Address:

Date: 12/17/2018

Designer: SB

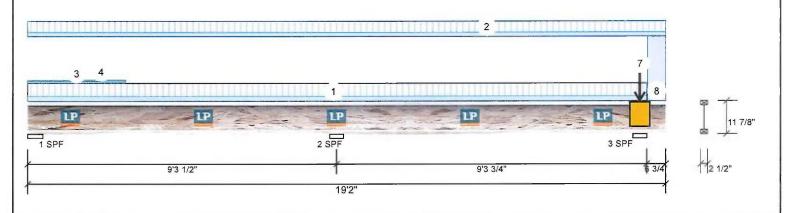
Job Name: LOT-6 (AMELIA 3 EL-1 _5BEDRM)

Project #:

F16-A LPI 20Plus

11.875" - PASSED

Level: Second Floor



ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 18-7-3	(Span)0-9-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 19-2-0	(Span)0-8-5	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Part. Uniform	0-0-0 to 2-11-3		Тор	2 PLF	0 PLF	0 PLF	0 PLF	
4	Part. Uniform	0-0-0 to 2-11-4		Тор	2 PLF	0 PLF	0 PLF	0 PLF	
5	Point	18-4-12		Тор	199 lb	204 lb	197 lb	0 lb	F2 F2
	Bearing Length	0-1-8							
6	Point	18-4-12		Тор	7 lb	0 lb	0 lb	0 lb	Wall Self Weight
	Bearing Length	0-1-8							
7	Point	18-4-12		Тор	36 lb	0 lb	0 lb	0 lb	Wall Self Weight
	Bearing Length	0-1-8							
8	Part, Uniform	18-7-8 to 19-2-0		Тор	80 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight



Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

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IN THE DESIGN OF THIS COMPONENT.

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2, THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT

CONTAINS SPECIFICATIONS AND CRITERIA USED

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This design is valid until

Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325 www.lpcorp.com CCMC: 12412-R APA: PR-L238C







Client: Project:

Address:

12/17/2018 Date:

Designer: SB

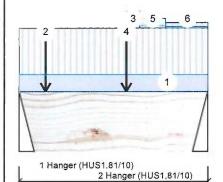
Job Name: LOT-6 (AMELIA 3 EL-1 _5BEDRM)

Project #:

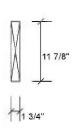
Forex 2.0E-3000Fb LVL

1.750" X 11.875" - PASSED

Level: Second Floor



3'1 3/8' 3'1 3/8"



wember	Intor	matior

Type:	Girder
Plies:	1
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	
Floor Live:	40 PSF
Dead:	15 PSF

Floor (Residential) Application: Design Method: LSD

NBCC 2010 / OBC 2012 **Building Code:** Load Sharing: No Deck:

Not Checked Not Checked Vibration:

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind	
1	477	187	0	0	
2	449	176	0	0	

Analysis Results

l	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
l	Moment	568 ft-lb	1'8 1/8"	17130 ft-lb	0.033 (3%)	1.25D+1.5L	L
l	Unbraced	568 ft-lb	1'8 1/8"	14337 ft-lb	0.040 (4%)	1.25D+1.5L	L
l	Shear	386 lb	1'2 1/8"	5798 lb	0.067 (7%)	1.25D+1.5L	L
	Perm Defl in.	0.001 (L/35583)	1'7 9/16"	0.091 (L/360)	0.010 (1%)	D	Uniform
	LL Defl inch	0.002 (L/13914)	1'7 9/16"	0.091 (L/360)	0.030 (3%)	L	L
	TL Defl inch	0.003 (L/10003)	1'7 9/16"	0.137 (L/240)	0.020 (2%)	D+L	L

Bearings and Factored Reactions

Bearing	Length	Cap. I	React D/L lb	Total	Ld. Case	Ld. Comb.	
1 -	3.000"	24%	233 / 716	949	L	1.25D+1.5L	
Hanger							
2 -	3.000"	23%	220 / 674	894	L	1.25D+1.5L	

Hanger



December 18, 2018

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.
- 4 Bottom braced at bearings

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

, Doctoin i	bracea at bearings.		Plant 1						
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Part. Uniform	0-0-0 to 3-1-6		Тор	90 PLF	240 PLF	0 PLF	0 PLF	
2	Point	0-5-4		Far Face	28 lb	74 lb	0 lb	0 lb	J9
3	Tie-In	1-9-4 to 1-11-14	(Span)3-1-11 to 3-1-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
4	Point	1-9-4		Far Face	23 lb	61 lb	0 lb	0 lb	J9
5	Tie-In	1-11-14 to 2-5-0	(Span)1-9-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
6	Tie-In	2-5-0 to 3-1-6	(Span)1-0-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	Self Weight				5 PLF			_	Squash Block is loads over bearing

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- andling & Installation
 LVL beams must not be out or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
 approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at beaming points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding



This design is valid until 10/18/2021

Manufacturer Info

Kott Lumber Company
14 Anderson Blvd, Ontario etail for ply to ply nailing o 905-642-4400 equirements







Client:

Project: Address:

12/17/2018 Date:

Designer:

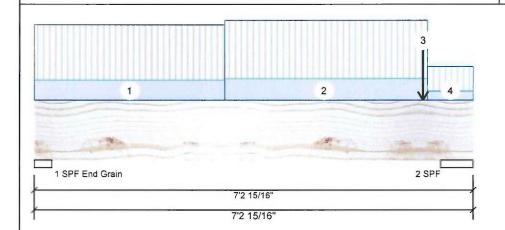
Job Name: LOT-6 (AMELIA 3 EL-1 _5BEDRM)

Project #:

Forex 2.0E-3000Fb LVL

1.750" X 11.875" - PASSED

Level: Second Floor





Member Information

Ī	Type:	Girder	Application:	Floor (Residential)
	Plies:	1	Design Method:	LSD
	Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
	Deflection LL:	360	Load Sharing:	No
	Deflection TL:	240	Deck:	Not Checked
	Importance:	Normal	Vibration:	Not Checked
	General Load			
	Floor Live:	40 PSF		
	Dead:	15 PSF		

Unfactored Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	Wind	
1	248	110	0	0	
2	643	266	0	0	

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	884 ft-lb	3'10 9/16"	17130 ft-lb	0.052 (5%)	1.25D+1.5L	L
Unbraced	884 ft-lb	3'10 9/16"	6876 ft-lb	0.129 (13%)	1.25D+1.5L	L
Shear	1154 lb	5'9 3/8"	5798 lb	0.199 (20%)	1.25D+1.5L	L
Perm Defl in.	0.004 (L/19225)	3'7 3/4"	0.218 (L/360)	0.020 (2%)	D	Uniform
LL Defl inch	0.009 (L/8390)	3'7 15/16"	0.218 (L/360)	0.040 (4%)	L	L
TL Defl inch	0.013 (L/5841)	3'7 7/8"	0.327 (L/240)	0.040 (4%)	D+L	L

Bearings and Factored Reactions								
Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.		
1 - SPF End Grain	3.500"	11%	138 / 372	510	L	1.25D+1.5L		
2 - SPF	6.438"	19%	333 / 065	1208	1	1 250±1 51		

Design Notes

ID

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Top braced at bearings.
- 3 Bottom braced at bearings.

Load Type

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE

IS AN INTEGRAL PART OF THIS DRAWING AS IT

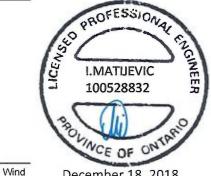
CONTAINS SPECIFICATIONS AND CRITERIA USED

1	Tie-In	0-0-0 to 3-1-12	(Span)3-1-13	Тор	15 PSF
2	Tie-In	3-1-12 to 6-5-14	(Span)3-4-0	Тор	15 PSF
3	Point	6-5-0		Far Face	176 lb
4	Tie-In	6-5-14 to 7-2-15	(Span)1-5-0	Тор	15 PSF
	Self Weight				5 PLF

Location

Trib Width

Side



December 18, 2018

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

0 PSF

0 PSF

0 PSF

0 lb F6

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

IN THE DESIGN OF THIS COMPONENT.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- andling & Installation
 LVL beams must not be out or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
 approvals
 Damaged Beams must not be used
 Design assumes top adge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Dead

Live

40 PSF

40 PSF

449 lb

40 PSF



This design is valid until 10/18/2021

Manufacturer Info

Forex APA: PR-L318

Snow

0 PSF

0 PSF

0 PSF

0 lb



