

ENGINEERING NOTE PAGE (ENP-1)
PLEASE READ PRIOR TO INSTALLATION

NE0518-226
GREEN YORK HOMES-
AMELIA 3 EL 2

RESPONSIBILITIES

THIS DESIGN IS FOR AN INDIVIDUAL BUILDING COMPONENT AND HAS BEEN BASED ON INFORMATION PROVIDED BY THE DESIGN OFFICE OF KOTT LUMBER. THE UNDERSIGNED ENGINEER DISCLAIMS ANY RESPONSIBILITY FOR DAMAGES AS A RESULT OF FAULTY OR INCORRECT INFORMATION, SPECIFICATION AND/OR DESIGNS FURNISHED TO THE ENGINEER. THE UNDERSIGNED ENGINEER IS ONLY RESPONSIBLE FOR THE STRUCTURAL INTEGRITY OF THIS BUILDING COMPONENT FOR THE CONDITIONS AND LOADS SHOWN ON THIS DRAWING. THE STRUCTURAL INTEGRITY OF THE BUILDING AND THE VERIFICATION OF THE DIMENSIONS AND THE DESIGN LOADS USED ARE THE RESPONSIBILITY OF THE BUILDING DESIGNER.

TRUSSES ARE DESIGNED IN CONFORMANCE WITH THE RELEVANT SECTIONS OF THE NATIONAL BUILDING CODE OF CANADA OR THE CANADIAN CODE FOR FARM BUILDINGS, WHICHEVER APPLIES TO THE BUILDING TYPE INDICATED ON THE DRAWING

IT IS THE RESPONSIBILITY OF KOTT LUMBER TO ENSURE THAT TRUSSES ARE MANUFACTURED IN CONFORMANCE WITH THESE DESIGNS AND WITH THE SPECIFICATIONS OUTLINED BELOW. THE UNDERSIGNED ENGINEER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

USE AND OCCUPANCY

- The building is of the type indicated on the drawing

LOADING

- The truss loading intensity and distribution as well as load transfer mechanism is that indicated on the drawing
- No buildings, trees, parapets or other projections higher than the roof for which the trusses are used are located within a distance less than ten (10) times the difference in height, or five metres (16 ft) whichever is greater, unless the drawing indicates that the snow drifting has been taken into account

HANDLING, INSTALLATION AND BRACING

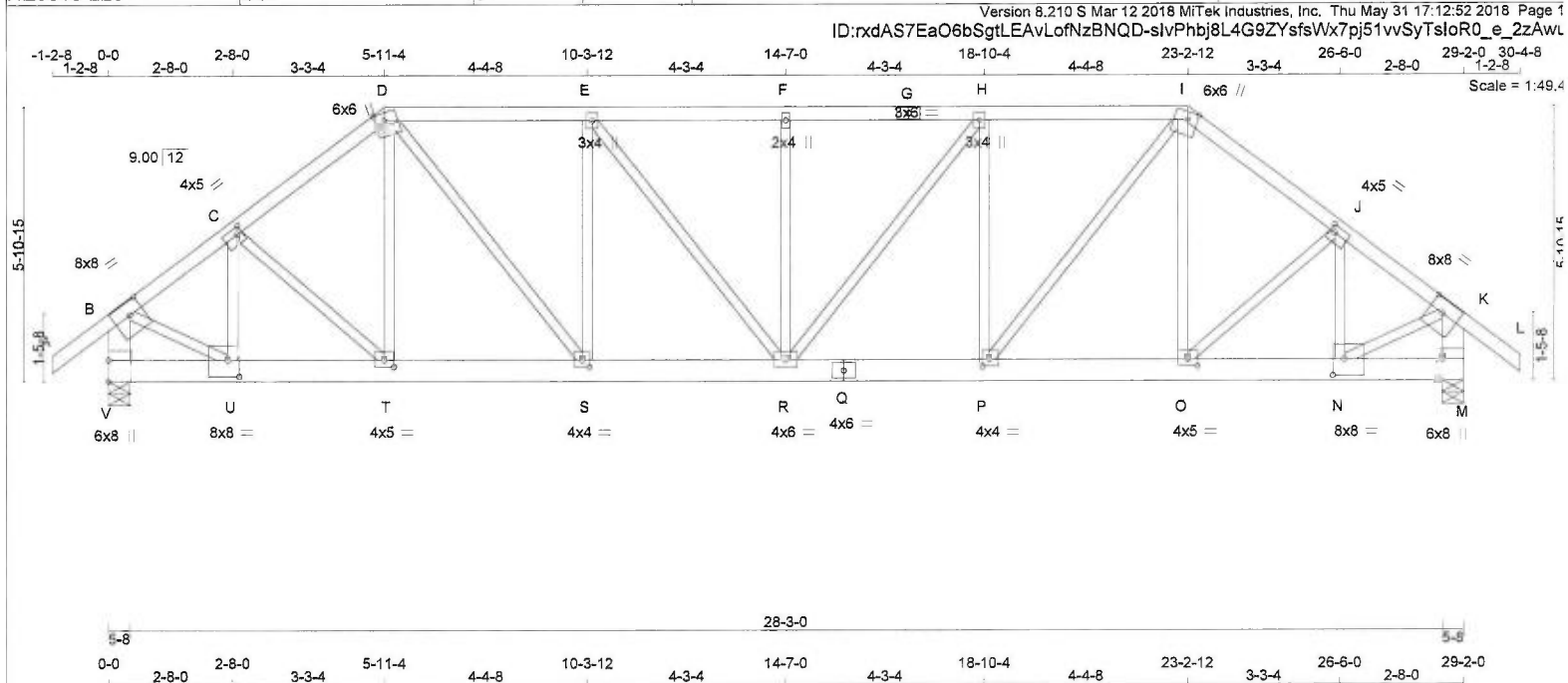
- The trusses must be handled and installed by a qualified professional as per the supplied document titled *Information for Truss Installers* and the BCSI-B1 and BCSI-B3 Summary Sheets
- The compression chords are laterally braced by continuous rigid diaphragm sheathing or as specified on the drawing
- Temporary and permanent bracing must be installed as indicated on the truss drawing and according to the BCSI-B1 and BCSI-B3 Summary Sheets. Bracing for the lateral stability of the truss is to be provided by the building designer
- **It is recommended that a Professional Engineer's advice be obtained for the bracing of trusses spanning more than 12.37m (40'-7")**

SUPPORTS

- The trusses are to be supported at the bearing points indicated and anchored to the supports where considered necessary by the designer of the overall structure
- Bearing sizes shown are the minimum required to prevent crushing of the truss members and do not necessarily take into account stability of the overall building structure
- Elevation of bearings must be carefully checked and shimmed to alignment for solid bearings
- Adequate wood truss bearing is the responsibility of the building designer.

DIMENSIONS

- Geometry of the truss and dimensions indicated on the drawing are identical to those of the installed truss.



TOTAL WEIGHT = 154 lb
[M]

N. L. G. A. RULES				
CHORDS	SIZE		LUMBER	DESCR
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
V - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
V - Q	2x6	DRY	No.2	SPF
Q - M	2x6	DRY	No.2	SPF

ALL WEBS 2x3 DRY
EXCEPT

DRY: SEASONED LUMBER.

J	T	TYPE	PLATES	W	LEN	Y	X
B	T	TMVW-t	MT20	8.0	8.0	3.25	3.50
C	D	TMVW-t	MT20	4.0	5.0	1.75	1.25
D	D	TTVW+m	MT20	6.0	6.0	2.00	2.50
E	F	TMVW+t	MT20	3.0	4.0		
F	F	TMVW+w	MT20	2.0	4.0		
G	F	TS-t	MT20	3.0	6.0		
H	I	TMVW+t	MT20	3.0	4.0		
I	I	TTVW+m	MT20	6.0	6.0	2.00	2.50
J	K	TMVW-t	MT20	4.0	5.0	1.75	1.25
K	M	TMVW-t	MT20	8.0	8.0	3.25	3.50
M	N	BMV1+t	MT20	6.0	8.0	Edge	0.50
N	N	BMVW-t	MT20	8.0	8.0	4.25	3.00
O	P	BMVW-t	MT20	4.0	5.0	1.75	2.50
P	P	BMVW-t	MT20	4.0	4.0	1.75	1.75
Q	R	BS-t	MT20	4.0	6.0		
R	R	BMVWVW-t	MT20	4.0	6.0		
S	T	BMVW-t	MT20	4.0	4.0	1.75	1.75
T	T	BMVW-t	MT20	4.0	5.0	1.75	2.50
U	U	BMVW-t	MT20	8.0	8.0	4.25	3.00
V	V	BMV1+t	MT20	6.0	8.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

BEARINGS

	FACTORED		MAXIMUM FACTORED			INPUT	REQRD
	GROSS	REACTION	GROSS	REACTION	UPLIFT	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ		IN-SX	IN-SX
V	4235	0	4235	0	0	5-8	5-8
M	1749	0	1749	0	0	5-8	5-8

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
V	2970	2090 / 0	0 / 0	0 / 0	0 / 0	880 / 0	0 / 0
M	1226	870 / 0	0 / 0	0 / 0	0 / 0	355 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.19 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

TOTAL LOAD CASES: (4)

C H O R D S					W E B S				
MAX. FACTORED		FACTORED			MAX. FACTORED				
MEMB.	FORCE	VERT. LOAD	LC1	MAX.	MAX.	MEMB.	FORCE	MAX.	
	(LBS)	(PLF)	CSI (LC)	UNBRAC			(LBS)	CSI (LC)	
FR-TO		FROM TO		LENGTH	FR-TO				
A-B	0 / 30	-77.4	-77.4	0.10 (1)	10.00	U-C	0 / 1454	0.36 (1)	
B-C	-4165 / 0	-77.4	-77.4	0.32 (1)	3.19	C-T	-1404 / 0	0.49 (1)	
C-D	-2886 / 0	-77.4	-77.4	0.26 (1)	3.85	T-D	0 / 1120	0.28 (1)	
D-E	-2599 / 0	-77.4	-77.4	0.35 (1)	3.94	D-S	0 / 452	0.11 (1)	
E-F	-2532 / 0	-77.4	-77.4	0.34 (1)	3.99	S-E	-284 / 0	0.15 (1)	
F-G	-2532 / 0	-77.4	-77.4	0.34 (1)	3.99	E-R	-110 / 0	0.10 (1)	
G-H	-2532 / 0	-77.4	-77.4	0.34 (1)	3.99	R-F	-303 / 0	0.16 (1)	
H-I	-2189 / 0	-77.4	-77.4	0.31 (1)	4.27	R-H	0 / 562	0.14 (1)	
I-J	-1869 / 0	-77.4	-77.4	0.17 (1)	4.74	H-P	-817 / 0	0.42 (1)	
J-K	-1661 / 0	-77.4	-77.4	0.15 (1)	4.97	P-I	0 / 1141	0.28 (1)	
K-L	0 / 30	-77.4	-77.4	0.10 (1)	10.00	O-I	-25 / 44	0.02 (4)	
V-B	-4004 / 0	0.0	0.0	0.29 (1)	5.31	O-J	0 / 186	0.05 (1)	
M-K	-1709 / 0	0.0	0.0	0.12 (1)	7.55	N-J	-516 / 0	0.11 (1)	
						B-U	0 / 3587	0.89 (1)	
V-U	0 / 0	-100.0	-100.0	0.20 (1)	10.00	N-K	0 / 1442	0.35 (1)	
U-T	0 / 3339	-17.5	-17.5	0.62 (1)	10.00				
T-S	0 / 2318	-17.5	-17.5	0.35 (1)	10.00				
S-R	0 / 2599	-17.5	-17.5	0.38 (1)	10.00				
R-Q	0 / 2189	-17.5	-17.5	0.33 (1)	10.00				
Q-P	0 / 2189	-17.5	-17.5	0.33 (1)	10.00				
P-O	0 / 1480	-17.5	-17.5	0.22 (1)	10.00				
O-N	0 / 1342	-17.5	-17.5	0.21 (1)	10.00				
N-M	0 / 0	-17.5	-17.5	0.03 (1)	10.00				

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
U	2-8-0	-2797	-2797	—	FRONT	VERT	TOTAL



**READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.**

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED
BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

TOP	CH.	LL =	23.3	PSF
		DL =	3.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.0	PSF
TOTAL LOAD		=	33.3	PSF

100

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 5-11-4
END DISTANCE = 2-8-0
END SPAN CARRIED = 5-11-4
END WALL WIDTH = 5-8
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADD'L LOADS BASED ON 55 % OF GSL.

ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , CBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= $L/360$ (0.97")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.10")
ALLOWABLE DEFL.(TL)= $L/360$ (0.97")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.17")

CSI: TC=0.35/1.00 (D-E:1), BC=0.62/1.00 (T-U:1),
WB=0.89/1.00 (B-U:1), SSI=0.17/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

CONTINUED ON PAGE 2



HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 2797.1 lbs FACTORED DOWN AT 2-8-0 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

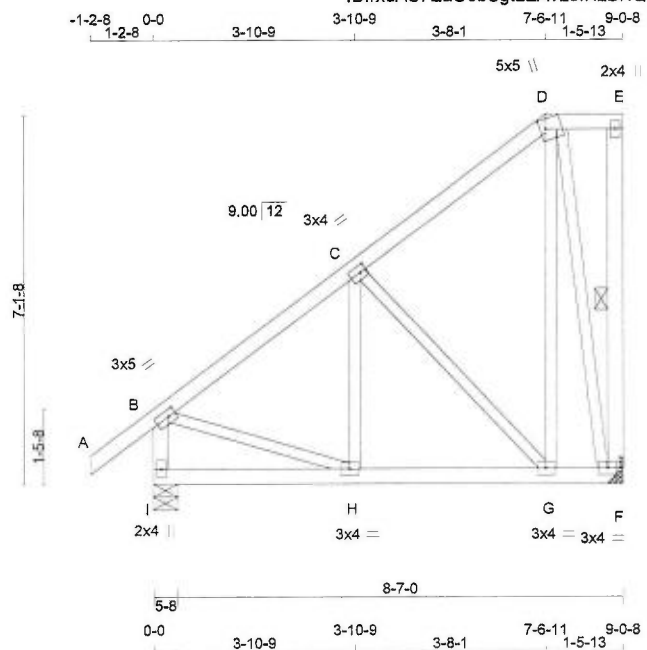
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (D) (INPUT = 0.90)
JSI METAL= 0.80 (K) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:44.5

TOTAL WEIGHT = 54 lb
[M]

LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER		
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
F - E	2x4	DRY	No.2	SPF	
I - B	2x4	DRY	No.2	SPF	
I - F	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVV-t	MT20	3.0	5.0	1.50	1.75
C	TMWW-t	MT20	3.0	4.0	1.50	1.50
D	TTVV+m	MT20	5.0	5.0	2.25	1.25
E	TMV+p	MT20	2.0	4.0		
F	BMVV1-t	MT20	3.0	4.0		
G	BMWW-t	MT20	3.0	4.0		
H	BMWW-t	MT20	3.0	4.0		
I	BMV1+p	MT20	2.0	4.0		

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 432.4 lbs FACTORED DOWN AT 7-6-11 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT		VERT	HORZ	UPLIFT	
F		951	0	0	MECHANICAL
I		677	0	0	5-8

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT F TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS		1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
F	664	483 / 0	0 / 0	0 / 0	0 / 0	181 / 0	0 / 0	
I	478	319 / 0	0 / 0	0 / 0	0 / 0	159 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF E-F. DBS = 20-0-0. CBF = 13 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING				TOTAL LOAD CASES: (4)			
CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX	MEMB.	FORCE (LBS)	MAX	LC1 (LC)
FR-TO		FROM TO	CSI (LC)	FR-TO		UNBRAC	LENGTH
A-B	0 / 30	-77.4 -77.4	0.10 (1)	H-C	0 / 127	0.05 (4)	
B-C	-473 / 0	-77.4 -77.4	0.20 (1)	C-G	-325 / 0	0.17 (1)	
C-D	-238 / 0	-77.4 -77.4	0.20 (1)	G-D	0 / 299	0.07 (1)	
D-E	0 / 0	-144.6 -144.6	0.06 (1)	D-F	-834 / 0	0.77 (1)	
F-E	-107 / 0	0.0 0.0	0.03 (1)	B-H	0 / 411	0.10 (1)	
I-B	-628 / 0	0.0 0.0	0.07 (1)				
I-H	0 / 0	-32.7 -32.7	0.14 (4)				
H-G	0 / 395	-32.7 -32.7	0.17 (4)				
G-F	0 / 178	-32.7 -32.7	0.05 (4)				

FACTORED CONCENTRATED LOADS (LBS)				FACE	DIR.	TYPE
JT	LOC.	LC1	MAX- MAX+	FRONT	VERT	TOTAL
D	7-6-11	-432	-432			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 7-6-11
RIGHT SETBACK = 0-0
END SETBACK = 5-11-4
END WALL WIDTH = 5-8
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDTL LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.30")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.30")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.20/1.00 (B-C:1), BC=0.17/1.00 (G-H:4), WB=0.77/1.00 (D-F:1), SSI=0.13/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

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PLATE PLACEMENT TOL. = 0.250 inches

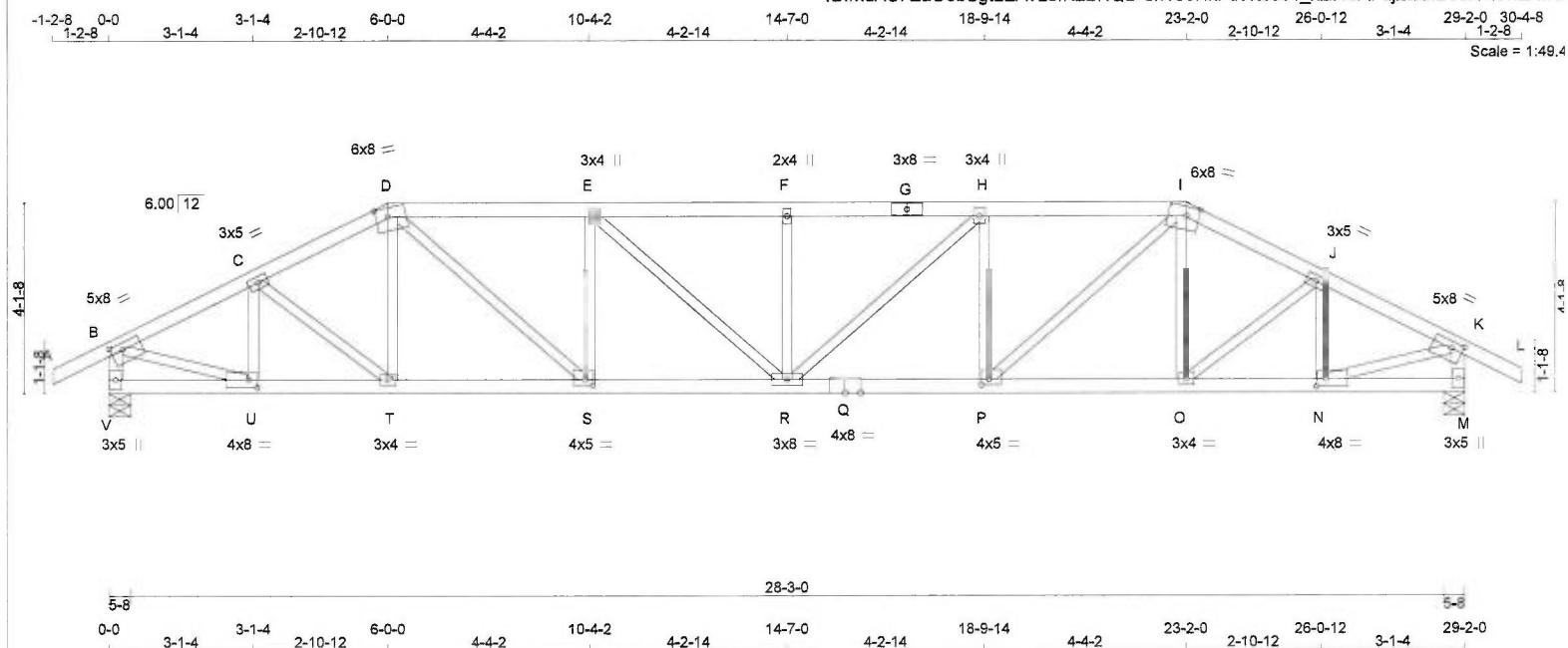
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (F) (INPUT = 0.90)
JSI METAL= 0.19 (B) (INPUT = 1.00)



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LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - L	2x4	DRY	No.2
V - B	2x4	DRY	No.2
M - K	2x4	DRY	No.2
V - Q	2x4	DRY	No.2
Q - M	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	1.75	3.00
C	TMVW-t	MT20	3.0	5.0		
D	TTVW-m	MT20	6.0	8.0	2.25	3.25
E	TMVW-t	MT20	3.0	4.0		
F	TMVW-w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	8.0		
H	TMVW-t	MT20	3.0	4.0		
I	TTVW-m	MT20	6.0	8.0	2.25	3.25
J	TMVW-t	MT20	3.0	5.0		
K	TMVW-t	MT20	5.0	8.0	1.75	3.00
M	BMV1+p	MT20	3.0	5.0		
N	BMVW-t	MT20	4.0	8.0	2.00	2.25
O	BMVW-t	MT20	3.0	4.0		
P	BMVW-t	MT20	4.0	5.0	1.50	2.00
Q	BS-t	MT20	4.0	8.0		
R	BMVW-t	MT20	3.0	8.0		
S	BMVW-t	MT20	4.0	5.0	1.50	2.00
T	BMVW-t	MT20	3.0	4.0		
U	BMVW-t	MT20	4.0	8.0	2.00	2.25
V	BMV1+p	MT20	3.0	5.0		

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
V	2644	0	2644	0	5-8	5-8
M	2644	0	2644	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
V	1855	1302 / 0	0 / 0	0 / 0	0 / 0	553 / 0	0 / 0
M	1855	1302 / 0	0 / 0	0 / 0	0 / 0	553 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.15 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS			WEBS		
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED HORZ. LOAD (LC1)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORZ. LOAD (LC1)	MAX. FACTORED HORZ. LOAD (LC1)
FR-TO						
A-B	0 / 22	-77.4	-77.4 0.10 (1)	10.00	U-C	-748 / 0 0.13 (1)
B-C	-3483 / 0	-77.4	-77.4 0.28 (1)	3.51	C-T	0 / 473 0.12 (1)
C-D	-3915 / 0	-77.4	-77.4 0.30 (1)	3.30	T-D	-147 / 76 0.04 (1)
D-E	-4840 / 0	-145.9	-145.9 0.83 (1)	2.36	D-S	0 / 1802 0.45 (1)
E-F	-5231 / 0	-145.9	-145.9 0.89 (1)	2.15	S-E	-1054 / 0 0.29 (1)
F-G	-5231 / 0	-145.9	-145.9 0.89 (1)	2.15	E-R	0 / 527 0.13 (1)
G-H	-5231 / 0	-145.9	-145.9 0.89 (1)	2.15	R-F	-570 / 0 0.15 (1)
H-I	-4840 / 0	-145.9	-145.9 0.83 (1)	2.36	R-H	0 / 527 0.13 (1)
I-J	-3915 / 0	-77.4	-77.4 0.30 (1)	3.30	P-H	-1054 / 0 0.29 (1)
J-K	-3483 / 0	-77.4	-77.4 0.28 (1)	3.51	P-I	0 / 1802 0.45 (1)
K-L	0 / 22	-77.4	-77.4 0.10 (1)	10.00	O-I	-147 / 76 0.04 (1)
V-B	-2586 / 0	0.0	0.0 0.29 (1)	5.28	O-J	0 / 473 0.12 (1)
M-K	-2586 / 0	0.0	0.0 0.29 (1)	5.28	N-J	-748 / 0 0.13 (1)
V-U	0 / 0	-33.0	-33.0 0.08 (4)	10.00	B-U	0 / 3226 0.80 (1)
U-T	0 / 3120	-33.0	-33.0 0.62 (1)	10.00	N-K	0 / 3226 0.80 (1)
T-S	0 / 3488	-33.0	-33.0 0.68 (1)	10.00		
S-R	0 / 4840	-33.0	-33.0 0.93 (1)	10.00		
R-Q	0 / 4840	-33.0	-33.0 0.93 (1)	10.00		
Q-P	0 / 4840	-33.0	-33.0 0.93 (1)	10.00		
P-O	0 / 3488	-33.0	-33.0 0.68 (1)	10.00		
O-N	0 / 3120	-33.0	-33.0 0.62 (1)	10.00		
N-M	0 / 0	-33.0	-33.0 0.08 (4)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
D	8-0-0	-348	-348	—	FRONT	VERT	TOTAL
I	23-2-0	-348	-348	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF

DL = 3.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.0 PSF

TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip

SIDE SETBACK = 6-0-0

END SETBACK = 6-0-0

END WALL WIDTH = 5-8

CORNER FRAMING TYPE: CONVENTIONAL

END JACK TYPE: CONVENTIONAL

APPLIED TO FRONT SIDE

- ADD'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, CBC 2012, ABC 2014

- CSA 086-09

- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.97")

CALCULATED VERT. DEFL.(LL) = L/999 (0.27")

ALLOWABLE DEFL.(TL) = L/360 (0.97")

CALCULATED VERT. DEFL.(TL) = L/739 (0.47")

CSI: TC=0.89/1.00 (F-H:1), BC=0.93/1.00 (P-R:1), WB=0.80/1.00 (K-N:1), SSI=0.32/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	618	354	1667 822 2284 1656

CONTINUED ON PAGE 2



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN YORK HOMES-AMELIA 3 EL 2	DRWG NO.
NE0518-226	G03	1	1	TRUSS DESC.		PAGE 8 OF 40

Version 8.210 S Mar 12 2018 MiTek Industries, Inc. Thu May 31 17:12:54 2018 Page 2
ID: rxdAS7EaO6bSgtLEAvLofNzBNQD-oh196HkPtiWtos01_xzbv8AFuj2axnB5uKTl3wzAwu

HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 348.1 lbs FACTORED DOWN AT 23-2-0, AND 348.1 lbs FACTORED DOWN AT 6-0-0 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

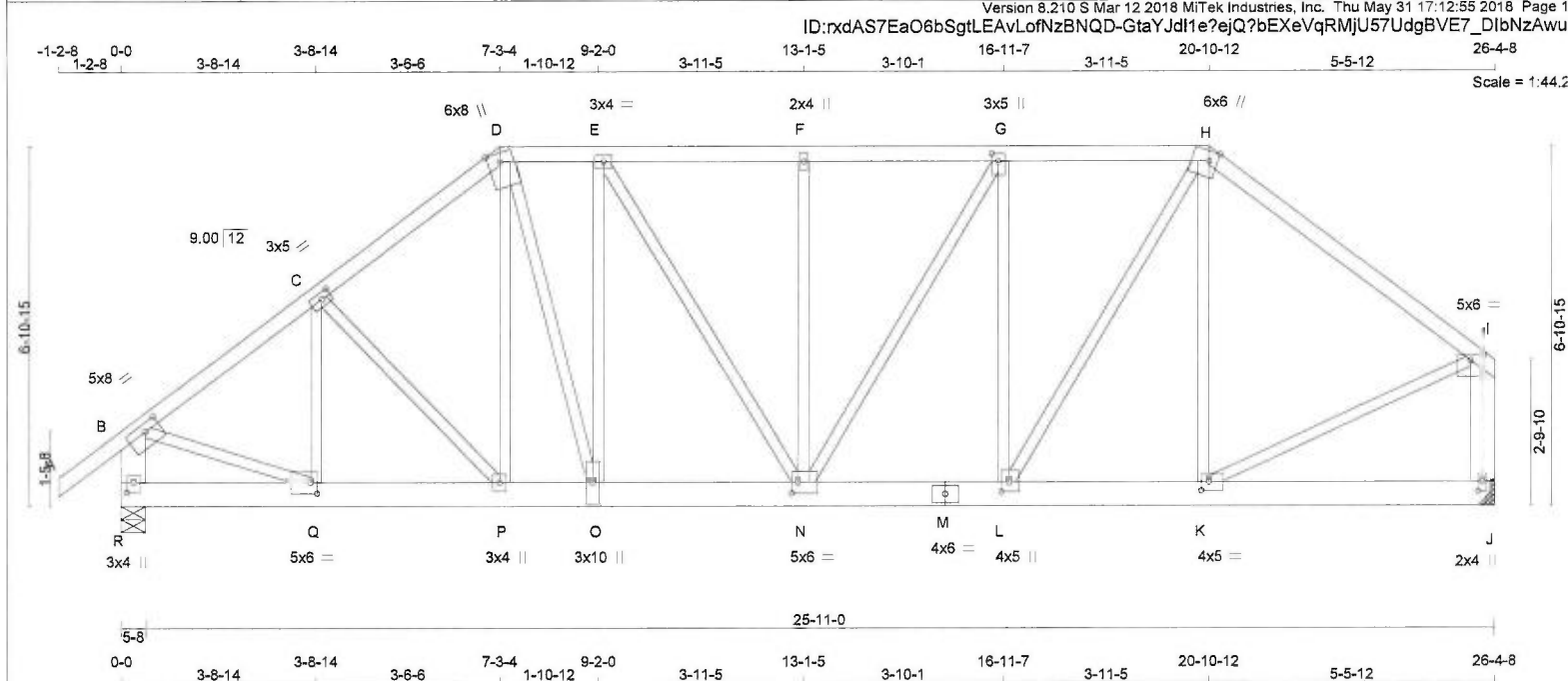
JSI GRIP= 0.90 (I) (INPUT = 0.90)

JSI METAL= 0.94 (Q) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - H	2x4	DRY	No.2
H - I	2x4	DRY	No.2
R - B	2x6	DRY	No.2
J - I	2x6	DRY	No.2
R - M	2x6	DRY	No.2
M - J	2x6	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	1.75	3.50
C	TMVW-t	MT20	3.0	5.0	1.50	2.25
D	TTVW+m	MT20	6.0	8.0	Edge	2.75
E	TMVW-t	MT20	3.0	4.0		
F	TMVW-w	MT20	2.0	4.0		
G	TMVW+t	MT20	3.0	5.0	1.75	1.50
H	TTVW+m	MT20	6.0	6.0	Edge	2.00
I	TMVW-p	MT20	5.0	6.0	1.25	3.00
J	BMV1+p	MT20	2.0	4.0	2.25	1.00
K	BMVW-t	MT20	4.0	5.0	2.00	1.75
L	BMVW+t	MT20	4.0	5.0	2.25	1.75
M	BS-t	MT20	4.0	6.0		
N	BMVW+t	MT20	5.0	6.0	2.50	1.50
O	BMVW+t	MT20	3.0	10.0		
P	BMVW+t	MT20	3.0	4.0		
Q	BMVW-t	MT20	5.0	6.0	2.50	1.50
R	BMV1+p	MT20	3.0	4.0	2.25	1.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
1)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	UPLIFT	IN-SX
R	2815	0	2815	0
J	2030	0	2030	0

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT J TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED							
R	1973	1393 / 0	0 / 0	0 / 0	0 / 0	0 / 0	580 / 0	0 / 0
J	1425	997 / 0	0 / 0	0 / 0	0 / 0	0 / 0	428 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.52 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 30	-77.4 -77.4	0.10 (1)	10.00	Q-C	-720 / 0	0.19 (1)
B-C	-3065 / 0	-77.4 -77.4	0.33 (1)	3.70	C-P	0 / 295	0.07 (1)
C-D	-3362 / 0	-77.4 -77.4	0.35 (1)	3.52	P-D	0 / 102	0.03 (1)
D-E	-3172 / 0	-77.4 -77.4	0.19 (1)	3.75	D-O	0 / 1796	0.44 (1)
E-F	-2859 / 0	-77.4 -77.4	0.27 (1)	3.86	O-E	0 / 314	0.08 (1)
F-G	-2859 / 0	-77.4 -77.4	0.29 (1)	3.83	E-N	-605 / 0	0.69 (1)
G-H	-2298 / 0	-77.4 -77.4	0.25 (1)	4.25	N-F	-310 / 0	0.24 (1)
H-I	-1887 / 0	-77.4 -77.4	0.61 (1)	4.21	N-G	0 / 1106	0.27 (1)
I-R	-2756 / 0	0.0 0.0	0.20 (1)	6.25	L-G	-1276 / 0	0.99 (1)
J-I	-1993 / 0	0.0 0.0	0.20 (1)	7.11	L-H	0 / 1545	0.38 (1)
					K-H	-534 / 0	0.41 (1)
R-Q	0 / 0	-17.5 -17.5	0.07 (1)	10.00	B-Q	0 / 2561	0.63 (1)
Q-P	0 / 2466	-17.5 -17.5	0.43 (1)	10.00	K-I	0 / 1640	0.41 (1)
P-O	0 / 2670	-17.5 -17.5	0.50 (1)	10.00			
O-N	0 / 3172	-17.5 -17.5	0.56 (1)	10.00			
N-M	0 / 2299	-17.5 -17.5	0.32 (1)	10.00			
M-L	0 / 2299	-17.5 -17.5	0.32 (1)	10.00			
L-K	0 / 1499	-17.5 -17.5	0.24 (1)	10.00			
K-J	0 / 0	-17.5 -17.5	0.06 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
O	9-2-0	-2243	-2243	-	FRONT	VERT	TOTAL

TOTAL WEIGHT = 151 lb

DESIGN CRITERIA
*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9. NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.88")
CALCULATED VERT. DEFL.(LL)= L/999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.88")
CALCULATED VERT. DEFL.(TL)= L/999 (0.18")

CSI: TC=0.61/1.00 (H-I:1) BC=0.56/1.00 (N-O:1) WB=0.99/1.00 (G-L:1) SSI=0.15/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	822	2284	1656

CONTINUED ON PAGE 2



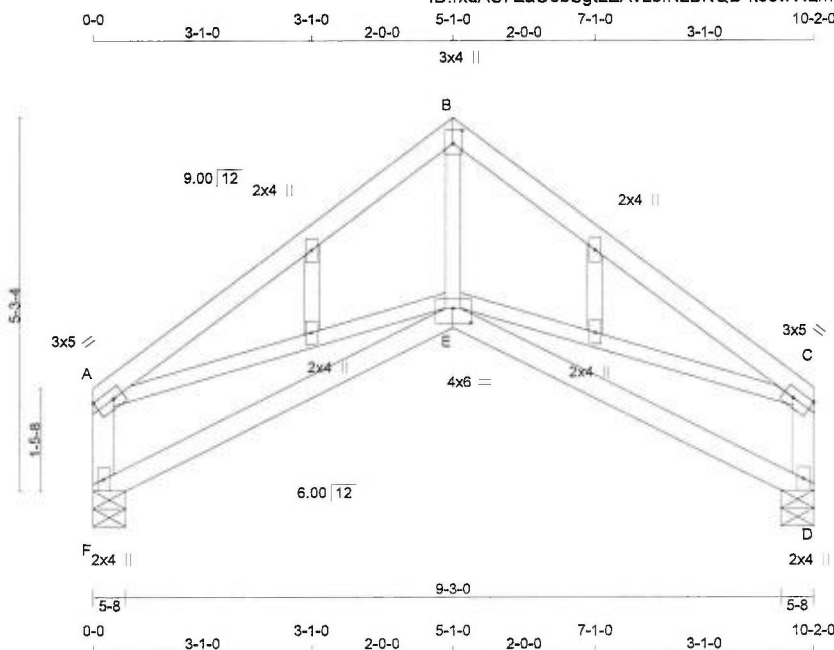
HANGERS NOTES
 1) SPECIAL HANGER(S) OR CONNECTION(S)
 REQUIRED TO SUPPORT CONCENTRATED
 LOAD(S) 2242.8 lbs FACTORED DOWN AT 9-2-0
 ON BOTTOM CHORD. DESIGN FOR
 UNSPECIFIED CONNECTION(S) IS DELEGATED
 TO THE BUILDING DESIGNER.

PLATE PLACEMENT TOL. = 0.250 inches
 PLATE ROTATION TOL. = 5.0 Deg.
 JSI GRIP= 0.90 (C) (INPUT = 0.90)
 JSI METAL= 0.62 (Q) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON
 ENGINEERING NOTE PAGE ENP-1. THIS
 NOTE PAGE IS AN INTEGRAL PART OF
 THIS DRAWING AS IT CONTAINS
 SPECIFICATIONS AND CRITERIA USED IN
 THE DESIGN OF THIS COMPONENT.





Scale = 1:32.5

TOTAL WEIGHT = 43 lb [M]F

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - B	2x4	DRY	No.2
B - C	2x4	DRY	No.2
F - A	2x4	DRY	No.2
D - C	2x4	DRY	No.2
F - E	2x4	DRY	No.2
E - D	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 EXCEPT

ALL GABLE WEBS 2x3 DRY No.2 DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	3.0	5.0	1.50	Edge
B	TTW+p	MT20	3.0	4.0	2.25	1.50
C	TMVW-t	MT20	3.0	5.0	1.50	Edge
D	BMV1+p	MT20	2.0	4.0		
E	BBVWV-p	MT20	4.0	6.0	2.50	3.00
F	BMV1+p	MT20	2.0	4.0		
G, H, I, J	NP+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
F	482	0	482	0
D	482	0	482	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN.	COMPONENT REACTIONS
F	COMBINED	SNOW	LIVE
JT	338	237 / 0	0 / 0
D	338	237 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	WEBS
	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO	VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)
A-B	-586 / 0	-77.4 -77.4 0.26 (1) 6.25
B-C	-586 / 0	-77.4 -77.4 0.26 (1) 6.25
F-A	-438 / 0	0.0 0.0 0.05 (1) 7.81
D-C	-438 / 0	0.0 0.0 0.05 (1) 7.81
F-E	0 / 0	-17.5 -17.5 0.13 (4) 10.00
E-D	0 / 0	-17.5 -17.5 0.13 (4) 10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 23.3 PSF
	DL	= 3.0 PSF
BOT CH.	LL	= 0.0 PSF
	DL	= 7.0 PSF
TOTAL LOAD		= 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.26/1.00 (A-B:1), BC=0.13/1.00 (D-E:4), WB=0.11/1.00 (A-E:1), SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	618	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

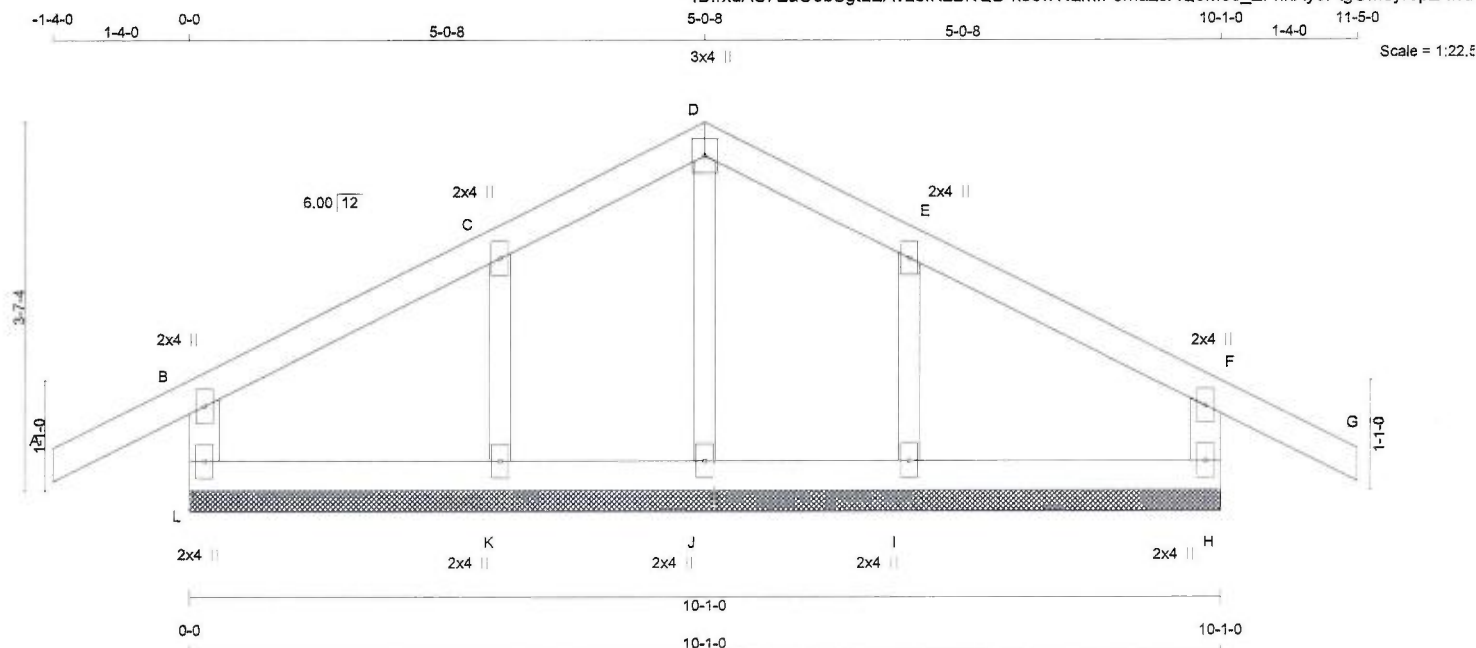
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (E) (INPUT = 0.90)
JSI METAL= 0.21 (A) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 35 lb
IM

LUMBER

N. L. G. A. RULES			
CHORDS	SIZE		LUMBER
L - B	2x4	DRY	No.2
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
H - F	2x4	DRY	No.2
L - H	2x4	DRY	No.2

ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS			
	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

GABLE STUDS SPACED AT 2'-0" OC.

PLATES (table is in inches)

IT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMW+w	MT20	2.0	4.0		
D	TTV+p	MT20	3.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TMV+p	MT20	2.0	4.0		
H	BMV1+p	MT20	2.0	4.0		
I, J, K						
L	BMW1+w	MT20	2.0	4.0		
I	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

C H O R D S				W E B S				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO			FR-TO			
L-B	-240 / 0	0.0	0.0	0.02 (4)	7.81	J-D	-122 / 0	0.03 (1)
A-B	0 / 24	-77.4	-77.4	0.11 (1)	10.00	K-C	-197 / 0	0.03 (1)
B-C	-26 / 0	-77.4	-77.4	0.09 (1)	6.25	I-E	-197 / 0	0.03 (1)
C-D	-21 / 0	-77.4	-77.4	0.06 (1)	6.25			
D-E	-21 / 0	-77.4	-77.4	0.06 (1)	6.25			
E-F	-26 / 0	-77.4	-77.4	0.09 (1)	6.25			
F-G	0 / 24	-77.4	-77.4	0.11 (1)	10.00			
H-F	-240 / 0	0.0	0.0	0.02 (4)	7.81			
L-K	0 / 20	-17.5	-17.5	0.03 (4)	10.00			
K-J	0 / 14	-17.5	-17.5	0.03 (4)	10.00			
J-I	0 / 14	-17.5	-17.5	0.03 (4)	10.00			
I-H	0 / 20	-17.5	-17.5	0.03 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCG 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , CBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11/1.00 (F-G:1) , BC=0.03/1.00 (H-I:4)
WB=0.03/1.00 (E-I:1) , SSI=0.09/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL = 0.250 inches

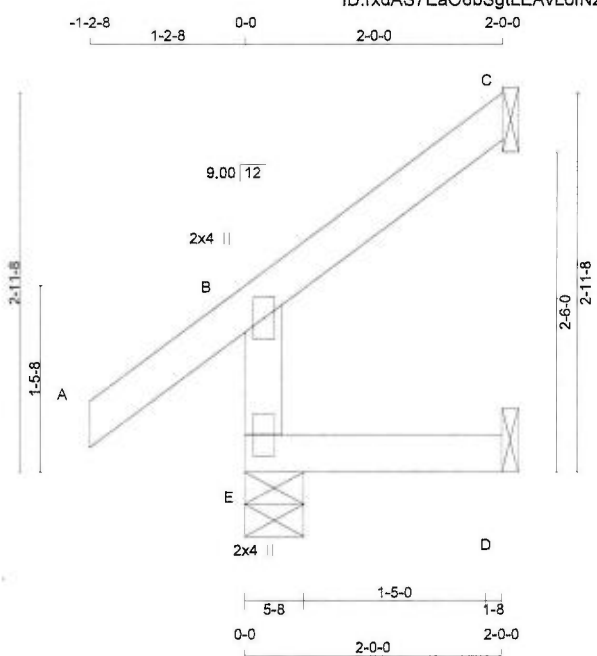
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (B) (INPUT = 0.90)
JSI METAL= 0.06 (F) (INPUT = 1.00)



**READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.**





Scale = 1:17.5

LUMBER

N. L. G. A. RULES

CHORDS

SIZE

DRY

LUMBER

DESCR.

SPF

SPF

SPF

E - B

2x4

DRY

No.2

SPF

A - C

2x4

DRY

No.2

SPF

E - D

2x4

DRY

No.2

SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	UP
E	216	0	216	0
C	59	0	59	0
D	16	0	16	0

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	149	118 / 0	0 / 0	0 / 0	0 / 0	0 / 0	31 / 0	0 / 0
C	40	35 / 0	0 / 0	0 / 0	0 / 0	0 / 0	5 / 0	0 / 0
D	13	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
 ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED HORIZ. LOAD (PLF)	MAX. FACTORED UNBRACED LENGTH (FT)
FR-TO					
E-B	-196 / 0	0.0	0.0	0.01 (4)	7.81
A-B	0 / 30	-77.4	-77.4	0.09 (1)	10.00
B-C	-11 / 0	-77.4	-77.4	0.05 (1)	6.25
E-D	0 / 0	-17.5	-17.5	0.02 (4)	10.00

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD	=	33.3	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, CBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

DESIGN ASSUMPTIONS
 - OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
 ALLOWABLE DEFL.(TL)= L/360 (0.19")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.09/1.00 (A-B:1), BC=0.02/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.06/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX MIN
MT20	618	354	1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

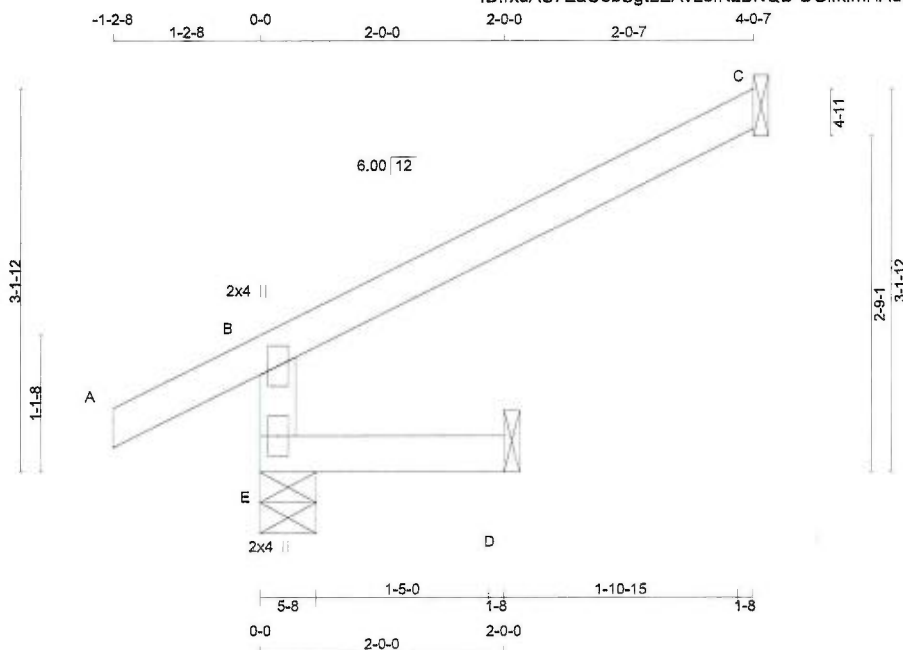
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (B) (INPUT = 0.90)
 JSI METAL= 0.05 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:18.75

TOTAL WEIGHT = 2 X 10 = 20 lb [M]

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

DESCR.
SPF
SPF
SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
E	313	0	313	0	5-8	5-8
C	117	0	117	0	1-8	1-8
D	16	0	17	0	1-8	1-8

SEE MITTEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	215	177 / 0	0 / 0	0 / 0	0 / 0	38 / 0	0 / 0
C	80	71 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0
D	12	0 / 0	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		WEBS		FACTORED	
	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX	MAX. UNBRACED LENGTH	MEMB.	MAX. FORCE (LBS)	MAX
FR-TO								
E-B	-294 / 0	0.0	0.0	0.01 (4)	7.81			
A-B	0 / 22	-77.4	-77.4	0.09 (1)	10.00			
B-C	-17 / 0	-77.4	-77.4	0.21 (1)	6.25			
E-D	0 / 0	-17.5	-17.5	0.02 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.21/1.00 (B-C-1), BC=0.02/1.00 (D-E-4), WB=0.00/1.00 (n/a:0), SSI=0.14/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

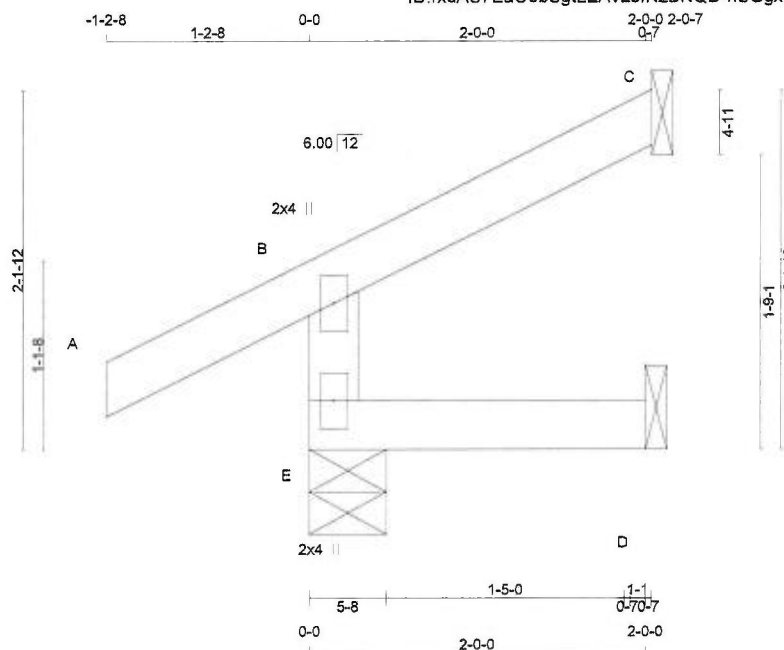
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:13.7

TOTAL WEIGHT = 2 X 7 = 15 lb [M]

LUMBER
 N. L. G. A. RULES
 CHORDS SIZE LUMBER
 E - B 2x4 DRY No.2
 A - C 2x4 DRY No.2
 E - D 2x4 DRY No.2
 DRY: SEASONED LUMBER.

DESCR.
 SPF
 SPF
 SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT		REQRD	
	VERT	HORZ	DOWN	HORZ	BRG	IN-SX	BRG	IN-SX
E	216	0	216	0	5-8	5-8	5-8	5-8
C	59	0	59	0	1-8	1-8	1-8	1-8
D	16	0	17	0	1-8	1-8	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
E	149	118 / 0	0 / 0	0 / 0	0 / 0	31 / 0	0 / 0
C	40	36 / 0	0 / 0	0 / 0	0 / 0	5 / 0	0 / 0
D	12	0 / 0	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED UNBRACED LENGTH (LC)	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO		
E-B	-197 / 0	0.0 0.0 0.01 (4)	7.81	
A-B	0 / 22	-77.4 -77.4 0.09 (1)	10.00	
B-C	-9 / 0	-77.4 -77.4 0.05 (1)	10.00	
E-D	0 / 0	-17.5 -17.5 0.02 (4)	10.00	

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 23.3 PSF
 DL = 3.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

DESIGN ASSUMPTIONS
 -OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
 ALLOWABLE DEFL.(TL) = L/360 (0.19")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.09/1.00 (A-B:1), BC=0.02/1.00 (D-E:4),
 WB=0.00/1.00 (n/a:0), SSI=0.07/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION
 (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

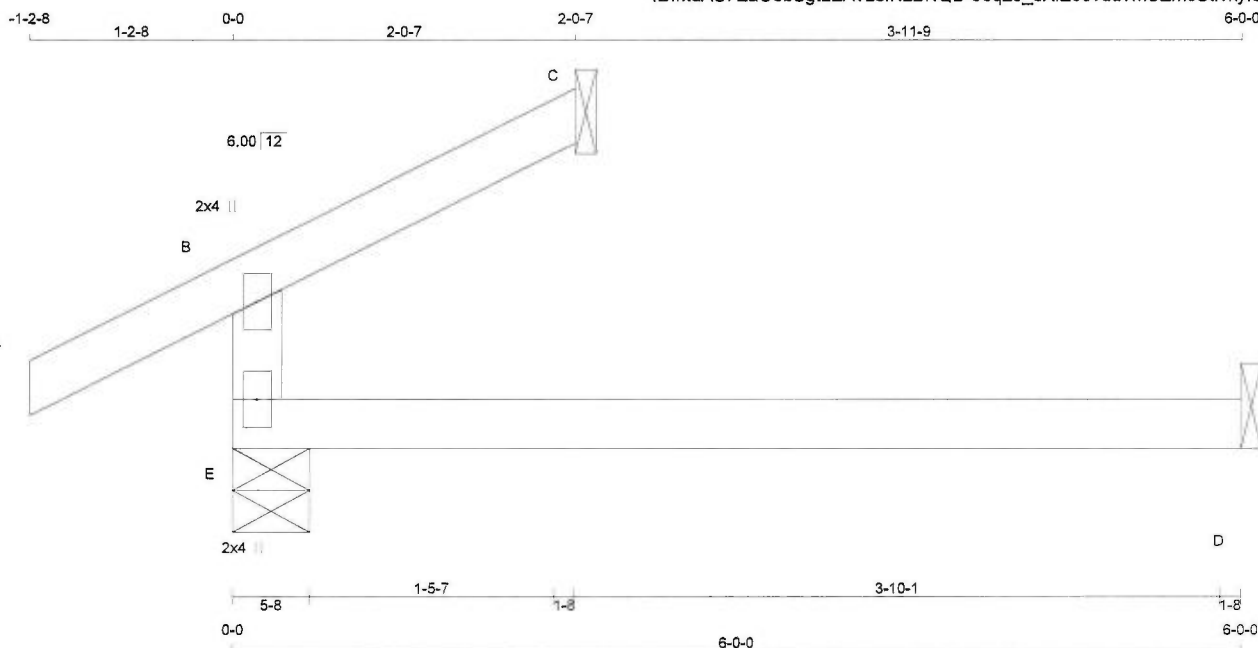
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.11 (B) (INPUT = 0.90)
 JSI METAL= 0.04 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:13.7

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	2.0	4.0	
E	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
E	258	0	258	0	0	5-8	5-8	
C	59	0	59	0	0	1-8	1-8	
D	43	0	48	0	0	1-8	1-8	

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	183	118 / 0	0 / 0	0 / 0	0 / 0	65 / 0	0 / 0
C	40	36 / 0	0 / 0	0 / 0	0 / 0	5 / 0	0 / 0
D	35	0 / 0	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. VERT. LOAD (LC1)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. VERT. LOAD (LC1)	UNBRACED LENGTH FR-TO
FR-TO				FR-TO			
E-B	-197 / 0	0.0	0.0 0.11 (4)	7.81			
A-B	0 / 22	-77.4	-77.4 0.09 (1)	10.00			
B-C	-9 / 0	-77.4	-77.4 0.04 (1)	10.00			
E-D	0 / 0	-17.5	-17.5 0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.11/1.00 (B-E:4), BC=0.13/1.00 (D-E:4),
WB=0.00/1.00 (n/a:0), SSI=0.08/1.00 (D-E:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY)	SHEAR	SECTION	(PSI)	(PLI)
	MAX	MIN	MAX	MIN	MAX
MT20	618	354	1667	822	2284

PLATE PLACEMENT TOL. = 0.250 inches

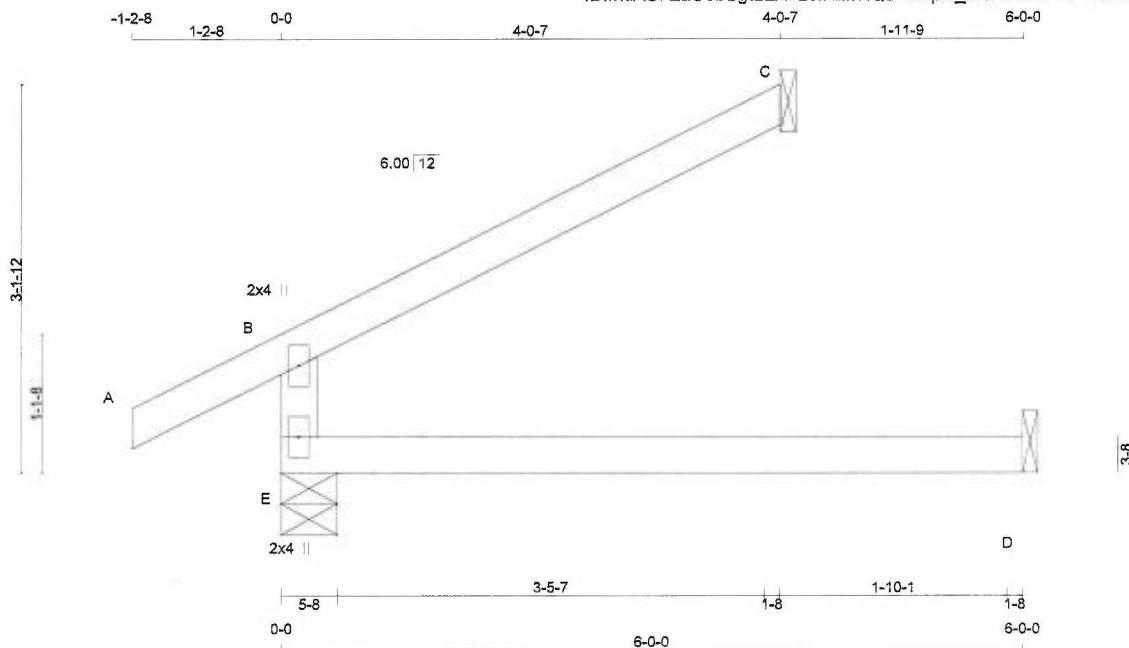
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.11 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	2.0	4.0
E	BMV1+p	MT20	2.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	BRG
E	355	0	0	355	0	0	0	5-8	5-8
C	117	0	0	117	0	0	0	1-8	1-8
D	43	0	0	48	0	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	249	177 / 0	0 / 0	0 / 0	0 / 0	72 / 0	0 / 0
C	80	71 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0
D	35	0 / 0	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	MEMB.	FORCE (LBS)	MAX (LC)	UNBRAC LENGTH FR-TO
FR-TO							
E-B	-294 / 0	0.0	0.0 0.11 (4)	7.81			
A-B	0 / 22	-77.4	-77.4 0.09 (1)	10.00			
B-C	-17 / 0	-77.4	-77.4 0.16 (1)	6.25			
E-D	0 / 0	-17.5	-17.5 0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.I. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.16/1.00 (B-C:1), BC=0.13/1.00 (D-E:4),
WB=0.00/1.00 (n/a:0), SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

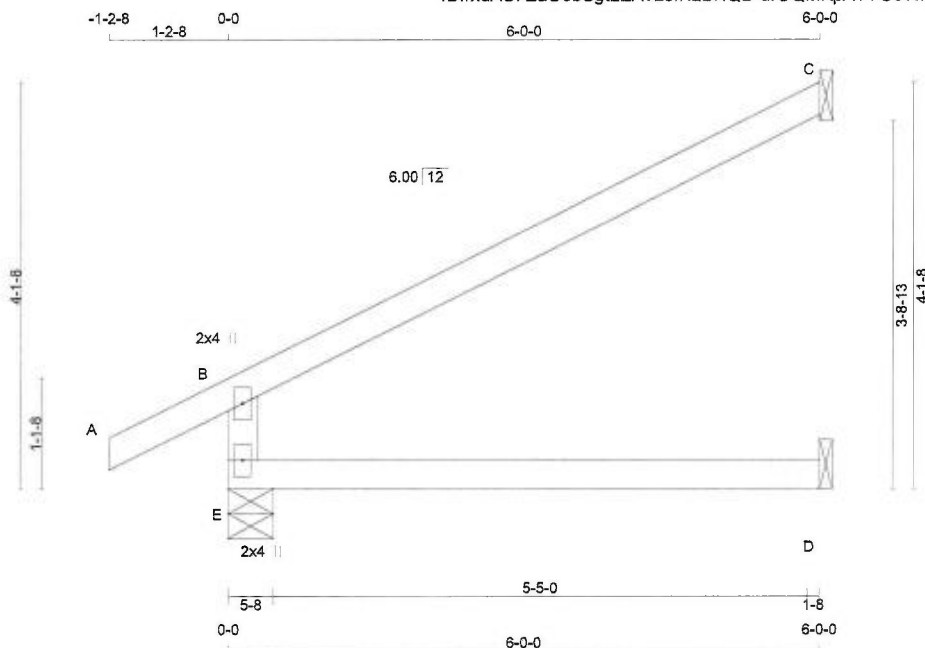
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:23.4

TOTAL WEIGHT = 10 X 17 = 169 lb
 [M]F

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

DESCR.
 SPF
 SPF
 SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
E	450	0	450	0	5-8	5-8
C	174	0	174	0	1-8	1-8
D	43	0	48	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE	MAX./MIN. COMPONENT REACTIONS				
		COMBINED	SNOW	LIVE	PERM. LIVE	WIND
E	314	234 / 0	0 / 0	0 / 0	0 / 0	80 / 0
C	118	105 / 0	0 / 0	0 / 0	0 / 0	14 / 0
D	35	0 / 0	0 / 0	0 / 0	0 / 0	35 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED HORZ. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORZ. LOAD (LC)	
FR-TO		FROM TO		FR-TO			
E-B	-389 / 0	0.0 0.0 0.11 (4)	7.81				
A-B	0 / 22	-77.4 -77.4 0.09 (1)	10.00				
B-C	-26 / 0	-77.4 -77.4 0.36 (1)	6.25				
E-D	0 / 0	-17.5 -17.5 0.13 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3 PSF
DL = 3.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.0 PSF	
TOTAL LOAD	= 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
 ALLOWABLE DEFL.(TL) = L/360 (0.20")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.36/1.00 (B-C:1), BC=0.13/1.00 (D-E:4),
 WB=0.00/1.00 (n/a:0), SSI=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

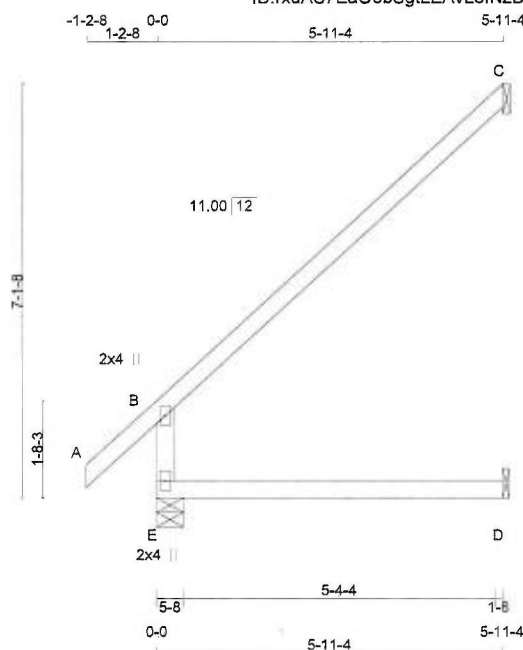
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (B) (INPUT = 0.90)
 JSI METAL= 0.09 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:39.6

TOTAL WEIGHT = 4 X 20 = 78 lb [M]F

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT		VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
E		448	0	448	0	0	5-8	5-8	
C		172	0	172	0	0	1-8	1-8	
D		44	0	50	0	0	1-8	1-8	

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	312	234 / 0	0 / 0	0 / 0	0 / 0	78 / 0	0 / 0
C	117	104 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0
D	35	0 / 0	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM	TO	FR-TO			
E-B	-388 / 0	0.0	0.0 0.10 (4)	7.81			
A-B	0 / 34	-77.4	-77.4 0.10 (1)	10.00			
B-C	-39 / 0	-77.4	-77.4 0.36 (1)	6.25			
E-D	0 / 0	-17.5	-17.5 0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9. NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.36/1.00 (B-C:1), BC=0.13/1.00 (D-E:4),
WB=0.00/1.00 (n/a:0), SSI=0.17/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

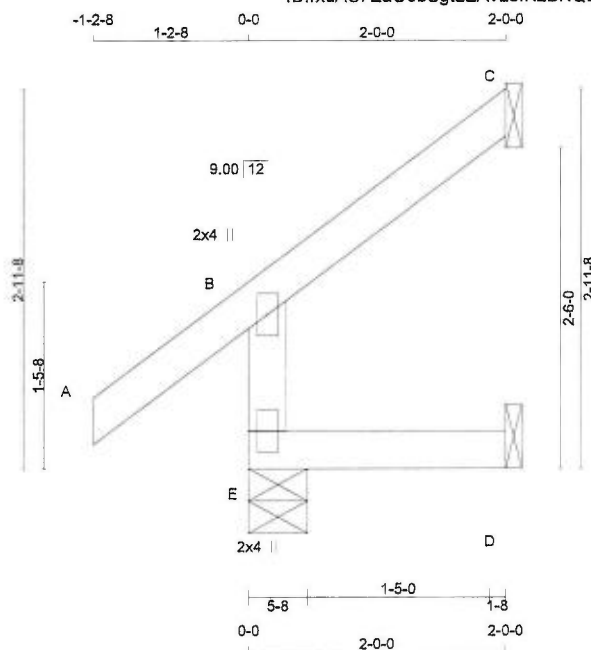
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.29 (B) (INPUT = 0.90)
JSI METAL= 0.11 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 3 X 8 = 25 lb [M]

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	216	0	216	0	5-8	5-8
C	59	0	59	0	1-8	1-8
D	16	0	16	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LOASE		MAX./MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	149	118 / 0	0 / 0	0 / 0	0 / 0	31 / 0	0 / 0
C	40	35 / 0	0 / 0	0 / 0	0 / 0	5 / 0	0 / 0
D	13	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS		WEBS			
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MAX. UNBRAC LENGTH FR-TO	MEMB. FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO		FROM TO				
E-B	-196 / 0	0.0	0.0	0.01 (4)	7.81	
A-B	0 / 30	-77.4	-77.4	0.09 (1)	10.00	
B-C	-11 / 0	-77.4	-77.4	0.05 (1)	6.25	
E-D	0 / 0	-17.5	-17.5	0.02 (4)	10.00	

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
DL =	3.0	PSF	
BOT CH.	LL =	0.0	PSF
DL =	7.0	PSF	
TOTAL LOAD =	33.3	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
 ALLOWABLE DEFL.(TL) = L/360 (0.19")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.09/1.00 (A-B:1), BC=0.02/1.00 (D-E:4),
 WB=0.00/1.00 (n/a:0), SSI=0.06/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667
		822	2284
			1656

PLATE PLACEMENT TOL. = 0.250 inches

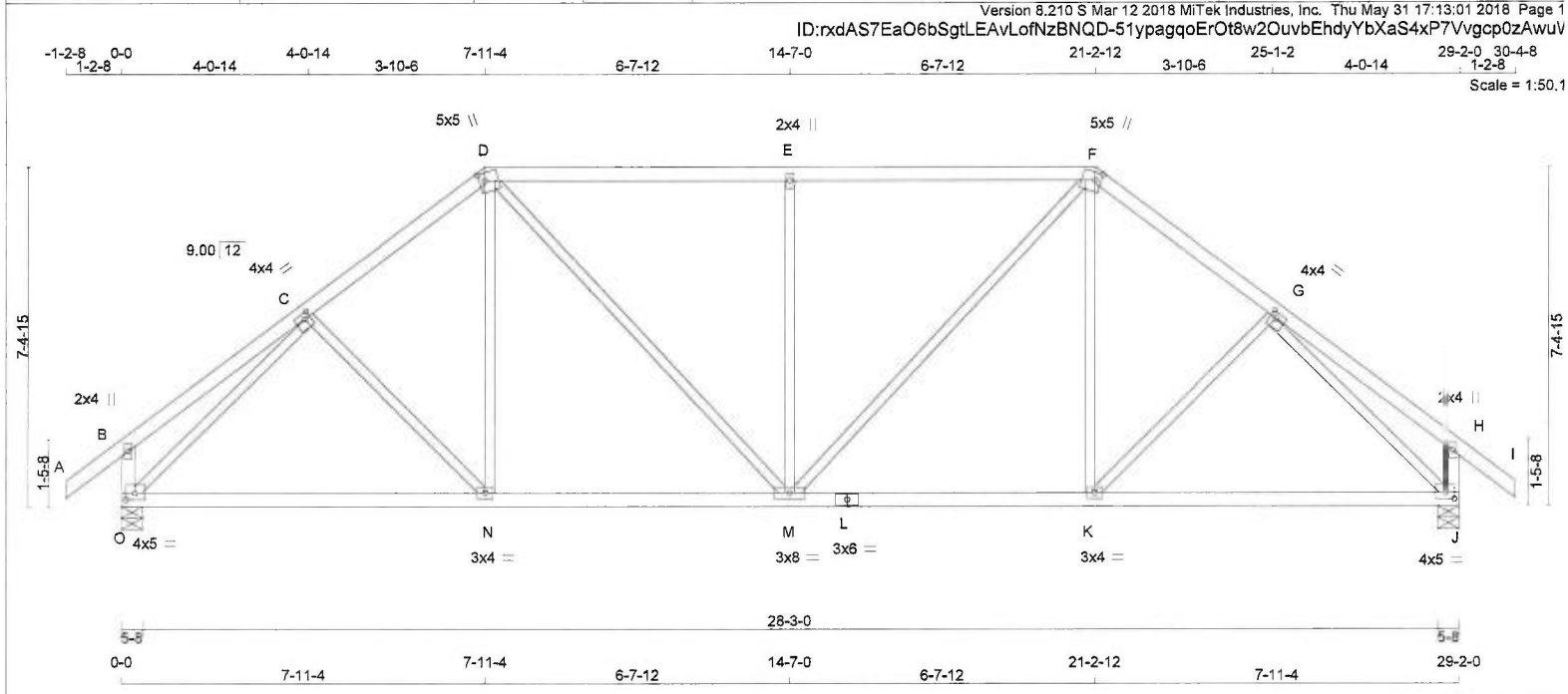
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (B) (INPUT = 0.90)
 JSI METAL= 0.05 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
O - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
O - L	2x4	DRY	No.2
L - J	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMVW-t	MT20	4.0	4.0	1.50	1.50
D	TTVW+m	MT20	5.0	5.0	2.00	1.50
E	TMVW+w	MT20	2.0	4.0		
F	TTVW+m	MT20	5.0	5.0	2.00	1.50
G	TMVW-t	MT20	4.0	4.0	1.50	1.50
H	TMV+p	MT20	2.0	4.0		
J	BMVW1-t	MT20	4.0	5.0	1.75	2.25
K	BMVW-t	MT20	3.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMVW-t	MT20	3.0	8.0		
N	BMVW-t	MT20	3.0	4.0		
O	BMVW1-t	MT20	4.0	5.0	1.75	2.25



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	UPLIFT	IN-SX
O	1484	0	1484	0
J	1484	0	1484	0

UNFACTORED REACTIONS

	1ST LCASE	MAX/MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW
O	1039	740 / 0
J	1039	740 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.81' FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 30	-77.4 -77.4 0.09 (1)	10.00	C-N	-37 / 41	0.02 (1)	
B-C	0 / 20	-77.4 -77.4 0.19 (1)	10.00	N-D	0 / 178	0.05 (4)	
C-D	-1447 / 0	-77.4 -77.4 0.17 (1)	5.28	D-M	0 / 495	0.11 (1)	
D-E	-1480 / 0	-77.4 -77.4 0.48 (1)	4.81	M-E	-631 / 0	0.52 (1)	
E-F	-1480 / 0	-77.4 -77.4 0.48 (1)	4.81	M-F	0 / 495	0.11 (1)	
F-G	-1447 / 0	-77.4 -77.4 0.17 (1)	5.28	K-F	0 / 178	0.05 (4)	
G-H	0 / 20	-77.4 -77.4 0.19 (1)	10.00	K-G	-37 / 41	0.02 (1)	
H-I	0 / 30	-77.4 -77.4 0.09 (1)	10.00	O-C	-1678 / 0	0.90 (1)	
O-B	-219 / 0	0.0 0.0 0.02 (1)	7.81	G-J	-1678 / 0	0.90 (1)	
J-H	-219 / 0	0.0 0.0 0.02 (1)	7.81				
O-N	0 / 1165	-17.5 -17.5 0.34 (4)	10.00				
N-M	0 / 1141	-17.5 -17.5 0.34 (4)	10.00				
M-L	0 / 1141	-17.5 -17.5 0.34 (4)	10.00				
L-K	0 / 1141	-17.5 -17.5 0.34 (4)	10.00				
K-J	0 / 1165	-17.5 -17.5 0.34 (4)	10.00				

TOTAL WEIGHT = 127 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.48/1.00 (D-E:1), BC=0.34/1.00 (K-M:4), WB=0.90/1.00 (G-J:1), SS=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

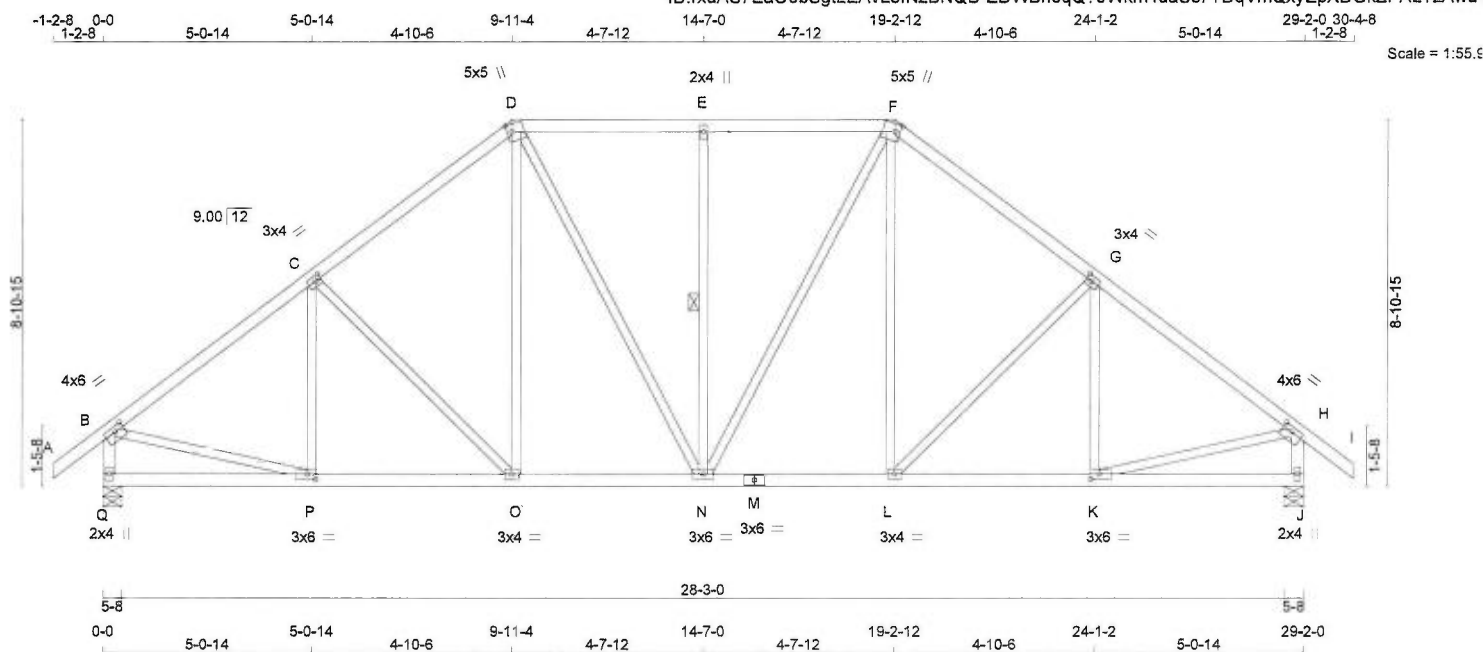
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (G) (INPUT = 0.90)
JSI METAL= 0.62 (C) (INPUT = 1.00)





LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
Q - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
Q - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVV-t	MT20	4.0	6.0	1.50	2.75
C	TMVV-t	MT20	3.0	4.0	1.50	1.50
D	TTVV+m	MT20	5.0	5.0	2.25	1.50
E	TMV-w	MT20	2.0	4.0		
F	TTVV+m	MT20	5.0	5.0	2.25	1.50
G	TMVV-t	MT20	3.0	4.0	1.50	1.50
H	TMVV-t	MT20	4.0	6.0	1.50	2.75
J	BMV1+p	MT20	2.0	4.0		
K	BMVV-t	MT20	3.0	6.0	1.50	2.50
L	BMVV-t	MT20	3.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMVV-t	MT20	3.0	6.0		
O	BMVV-t	MT20	3.0	4.0		
P	BMVV-t	MT20	3.0	6.0	1.50	2.50
Q	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED		MAXIMUM FACTORED		INPUT	REQRD
	GROSS REACTION		GROSS REACTION		BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX
Q	1484	0	1484	0	5-8	5-8
J	1484	0	1484	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
Q	1039	740 / 0	0 / 0	0 / 0	0 / 0	299 / 0	0 / 0
J	1039	740 / 0	0 / 0	0 / 0	0 / 0	299 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.07 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N. DBS = 20-0-0. CBF = 54 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO	CSI (LC)	FR-TO			
A-B	0 / 30	-77.4 -77.4	0.08 (1)	P-C	-178 / 31	0.07 (1)	
B-C	-1508 / 0	-77.4 -77.4	0.28 (1)	C-O	-244 / 0	0.23 (1)	
C-D	-1349 / 0	-77.4 -77.4	0.27 (1)	O-D	0 / 256	0.06 (1)	
D-E	-1196 / 0	-77.4 -77.4	0.22 (1)	D-N	0 / 291	0.07 (1)	
E-F	-1196 / 0	-77.4 -77.4	0.22 (1)	N-E	-434 / 0	0.22 (1)	
F-G	-1349 / 0	-77.4 -77.4	0.27 (1)	N-F	0 / 291	0.07 (1)	
G-H	-1508 / 0	-77.4 -77.4	0.28 (1)	L-F	0 / 256	0.06 (1)	
H-I	0 / 30	-77.4 -77.4	0.09 (1)	L-G	-244 / 0	0.23 (1)	
Q-B	-1446 / 0	0.0 0.0	0.15 (1)	K-G	-178 / 31	0.07 (1)	
J-H	-1446 / 0	0.0 0.0	0.15 (1)	B-P	0 / 1258	0.28 (1)	
				K-H	0 / 1258	0.28 (1)	
Q-P	0 / 0	-17.5 -17.5	0.10 (4)				
P-O	0 / 1228	-17.5 -17.5	0.24 (1)				
O-N	0 / 1056	-17.5 -17.5	0.21 (1)				
N-M	0 / 1056	-17.5 -17.5	0.21 (1)				
M-L	0 / 1056	-17.5 -17.5	0.21 (1)				
L-K	0 / 1228	-17.5 -17.5	0.24 (1)				
K-J	0 / 0	-17.5 -17.5	0.10 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/999 (0.09")

CSI: TC=0.28/1.00 (G-H:1), BC=0.24/1.00 (O-P:1), WB=0.28/1.00 (B-P:1), SSI=0.17/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

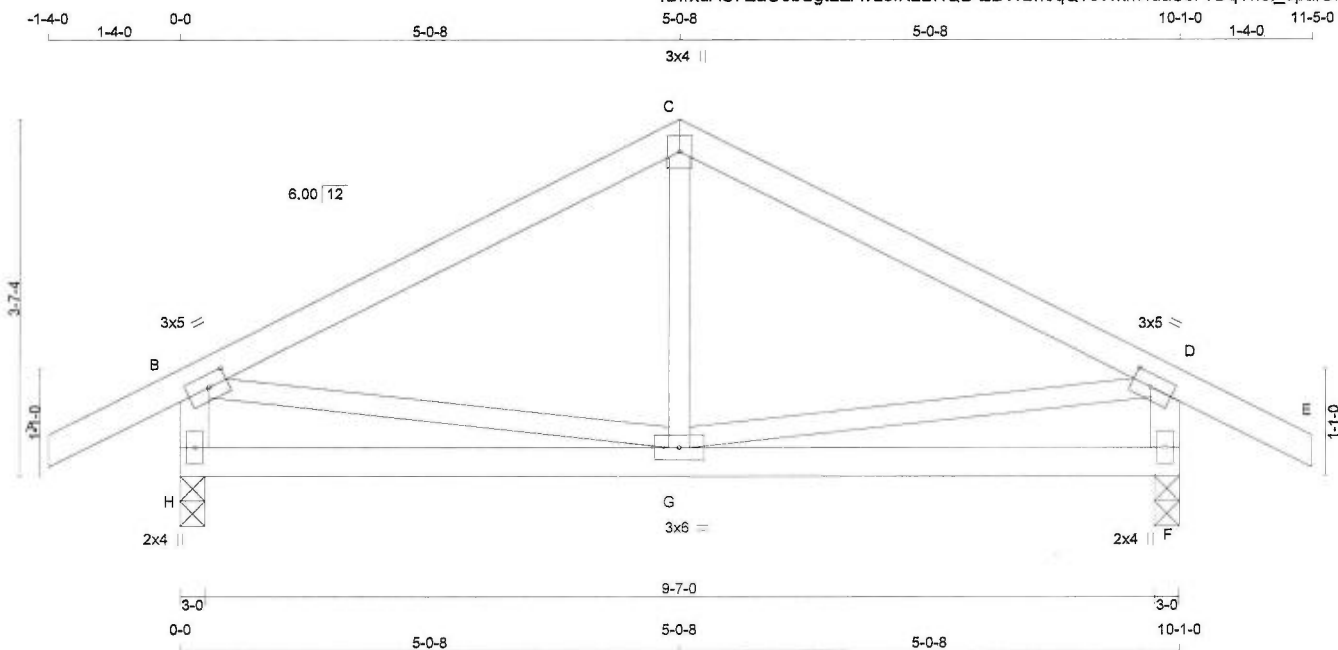
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (N) (INPUT = 0.90)
JSI METAL= 0.42 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 40 lb
 [M]F

LUMBER
 N. L. G. A. RULES
 CHORDS SIZE LUMBER DESCR.

A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	3.0	5.0	1.50	2.25
C	TTW+p	MT20	3.0	4.0		
D	TMVW-t	MT20	3.0	5.0	1.50	2.25
F	BMV1+p	MT20	2.0	4.0		
G	BMVWV-t	MT20	3.0	6.0		
H	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
H	586	0	586	0	3-0	3-0
F	586	0	586	0	3-0	3-0

UNFACTORED REACTIONS

JT	COMBINED	1ST LCASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
		SNOW	LIVE	PERM. LIVE				
H	409	300 / 0	0 / 0	0 / 0	0 / 0	109 / 0	0 / 0	0 / 0
F	409	300 / 0	0 / 0	0 / 0	0 / 0	109 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)		
FR-TO		FROM TO			FR-TO				
A-B	0 / 24	-77.4	-77.4 0.11 (1)	10.00	G-C	-10 / 84	0.03 (4)		
B-C	-425 / 0	-77.4	-77.4 0.25 (1)	6.25	B-G	0 / 385	0.09 (1)		
C-D	-425 / 0	-77.4	-77.4 0.25 (1)	6.25	G-D	0 / 385	0.09 (1)		
D-E	0 / 24	-77.4	-77.4 0.11 (1)	10.00					
H-B	-552 / 0	0.0	0.0 0.06 (1)	7.81					
F-D	-552 / 0	0.0	0.0 0.06 (1)	7.81					
H-G	0 / 0	-17.5	-17.5 0.12 (4)	10.00					
G-F	0 / 0	-17.5	-17.5 0.12 (4)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 23.3 PSF
 DL = 3.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.34")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
 ALLOWABLE DEFL.(TL) = L/360 (0.34")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.25/1.00 (B-C:1), BC=0.12/1.00 (G-H:4), WB=0.09/1.00 (D-G:1), SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION
 (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

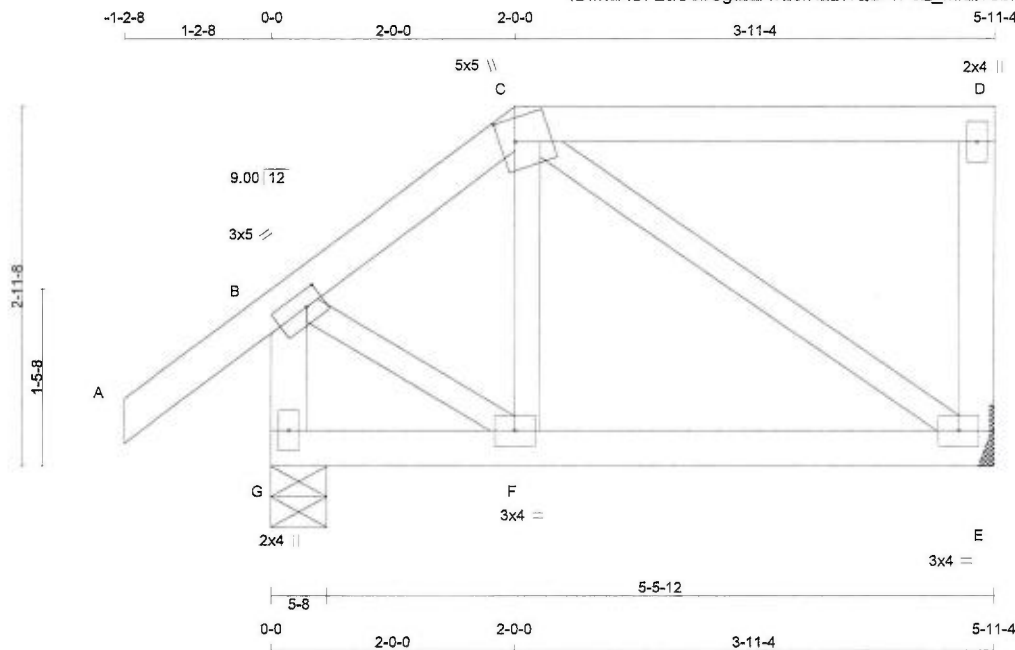
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (G) (INPUT = 0.90)
 JSI METAL= 0.17 (D) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:18.5

TOTAL WEIGHT = 27 lb [M]

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
E - D	2x4	DRY	No.2
G - B	2x4	DRY	No.2
G - E	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	3.0	5.0	1.50	1.75
C	TTVW+m	MT20	5.0	5.0	2.25	1.50
D	TMV+p	MT20	2.0	4.0		
E	BMVW1-t	MT20	3.0	4.0		
F	BMVW1-t	MT20	3.0	4.0		
G	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT REQRD BRG
E	282 0	282 0	MECHANICAL
G	382 0	382 0	5-8

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT E TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX/MIN. SNOW	MAX/MIN. LIVE	MAX/MIN. PERM. LIVE	MAX/MIN. WIND	MAX/MIN. DEAD	MAX/MIN. SOIL
E	198	138 / 0	0 / 0	0 / 0	0 / 0	59 / 0	0 / 0
G	266	199 / 0	0 / 0	0 / 0	0 / 0	67 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 30	-77.4 -77.4	0.09 (1)	10.00	F-C	-15 / 49	0.02 (4)
B-C	-189 / 0	-77.4 -77.4	0.05 (1)	6.25	C-E	-181 / 0	0.06 (1)
C-D	0 / 0	-77.4 -77.4	0.20 (1)	10.00	B-F	0 / 171	0.04 (1)
E-D	-152 / 0	0.0 0.0	0.02 (1)	7.81			
G-B	-373 / 0	0.0 0.0	0.04 (1)	7.81			
G-F	0 / 0	-17.5 -17.5	0.05 (4)	10.00			
F-E	0 / 150	-17.5 -17.5	0.07 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.20/1.00 (C-D:1), BC=0.07/1.00 (E-F:4),
WB=0.06/1.00 (C-E:1), SSI=0.12/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

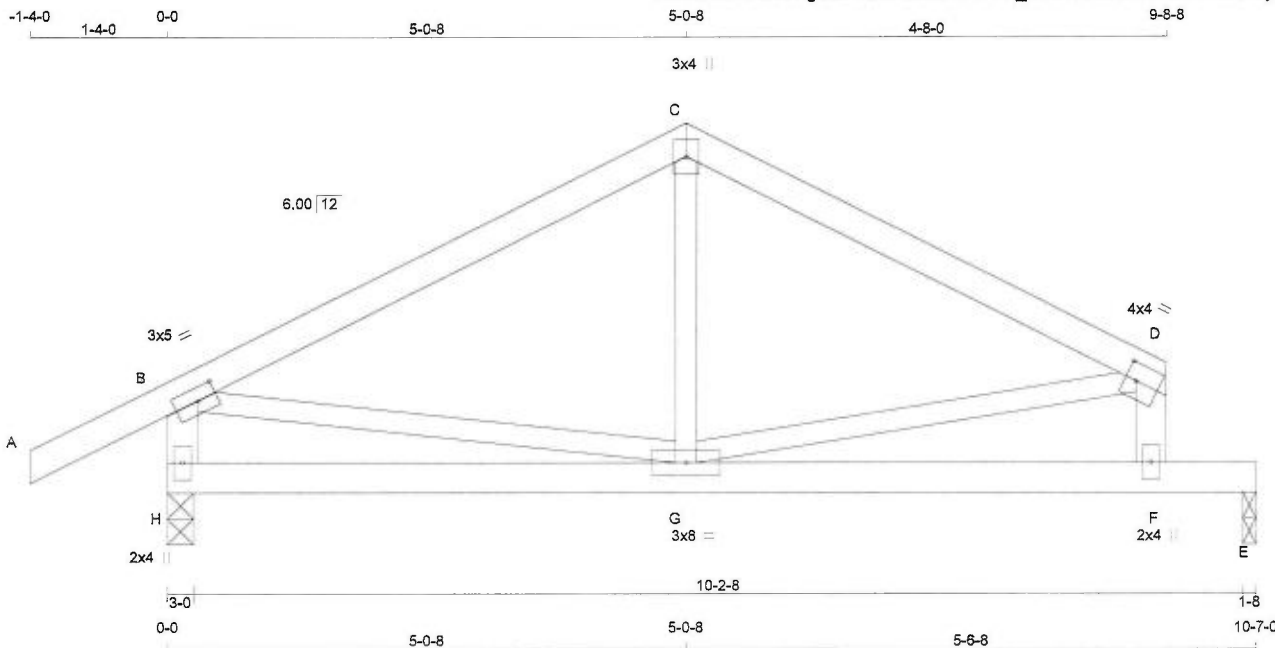
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.31 (B) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:22.4

TOTAL WEIGHT = 2 X 38 = 76 lb [M]

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVV-t	MT20	3.0	5.0	1.50	2.25
C	TTW+p	MT20	3.0	4.0		
D	TMVV-t	MT20	4.0	4.0	2.00	1.25
F	BMV+p	MT20	2.0	4.0		
G	BMVVV-t	MT20	3.0	8.0		
H	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	610	0	610	0	0	3-0	3-0
E	502	0	502	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
H	426	312 / 0	0 / 0	0 / 0	0 / 0	114 / 0	0 / 0
E	352	246 / 0	0 / 0	0 / 0	0 / 0	106 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM	TO			FR-TO			
A-B	0 / 24	-77.4	-77.4	0.11 (1)	10.00	G-C	0 / 103	0.04 (4)	
B-C	-497 / 0	-77.4	-77.4	0.35 (1)	6.25	B-G	0 / 450	0.10 (1)	
C-D	-497 / 0	-77.4	-77.4	0.30 (1)	6.25	G-D	0 / 454	0.10 (1)	
H-B	-594 / 0	0.0	0.0	0.06 (1)	7.81				
F-D	-494 / 0	0.0	0.0	0.05 (1)	7.81				
H-G	0 / 0	-17.5	-17.5	0.19 (1)	10.00				
G-F	0 / 0	-17.5	-17.5	0.55 (1)	10.00				
F-E	0 / 0	-94.9	-94.9	0.55 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.35")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.11")
ALLOWABLE DEFL.(TL) = $L/360$ (0.35")
CALCULATED VERT. DEFL.(TL) = $L/631$ (0.20")

CSI: TC=0.35/1.00 (B-C:1), BC=0.55/1.00 (F-G:1), WB=0.10/1.00 (D-G:1), SSI=0.39/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

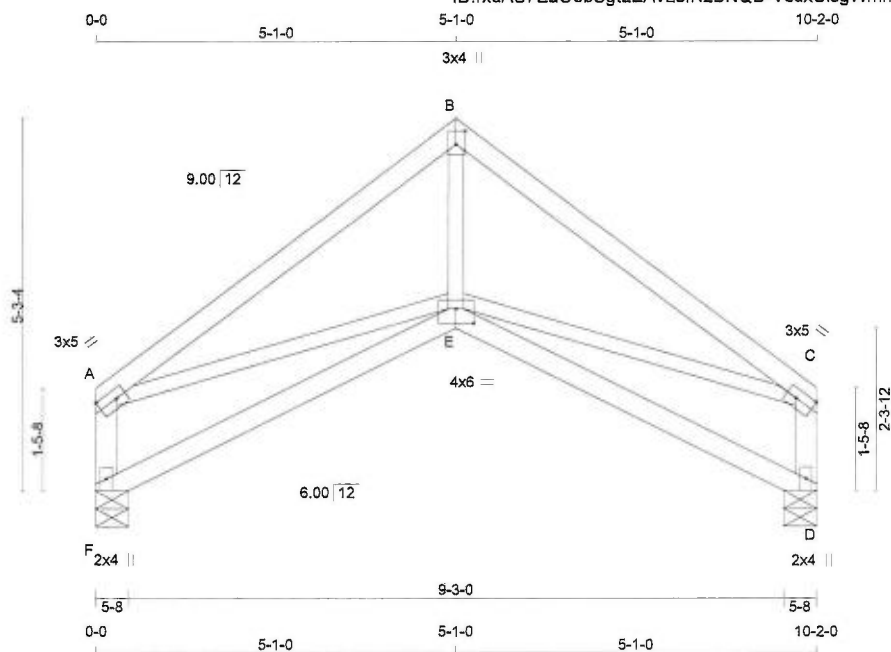
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (D) (INPUT = 0.90)
JSI METAL= 0.22 (D) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:32.5

TOTAL WEIGHT = 2 X 41 = 81 lb [M/F]

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF	
B - C	2x4	DRY	No.2	SPF	
F - A	2x4	DRY	No.2	SPF	
D - C	2x4	DRY	No.2	SPF	
F - E	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	3.0	5.0	1.50	Edge
B	TTVW+p	MT20	3.0	4.0	2.25	1.50
C	TMVW-t	MT20	3.0	5.0	1.50	Edge
D	BMV1+p	MT20	2.0	4.0		
E	BBWWVW-p	MT20	4.0	6.0	2.50	3.00
F	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX		
F	482	0	482	0	0	5-8	5-8		
D	482	0	482	0	0	5-8	5-8		

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN.	COMPONENT REACTIONS						
F	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
JT	338	237 / 0	0 / 0	0 / 0	0 / 0	102 / 0	0 / 0		
D	338	237 / 0	0 / 0	0 / 0	0 / 0	102 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	-586 / 0	E-B	0 / 310
B-C	-586 / 0	A-E	0 / 482
F-A	-438 / 0	E-C	0 / 482
D-C	-438 / 0		
F-E	0 / 0		
E-D	0 / 0		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.26/1.00 (A-B:1) , BC=0.13/1.00 (D-E:4) , WB=0.11/1.00 (A-E:1) , SS=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

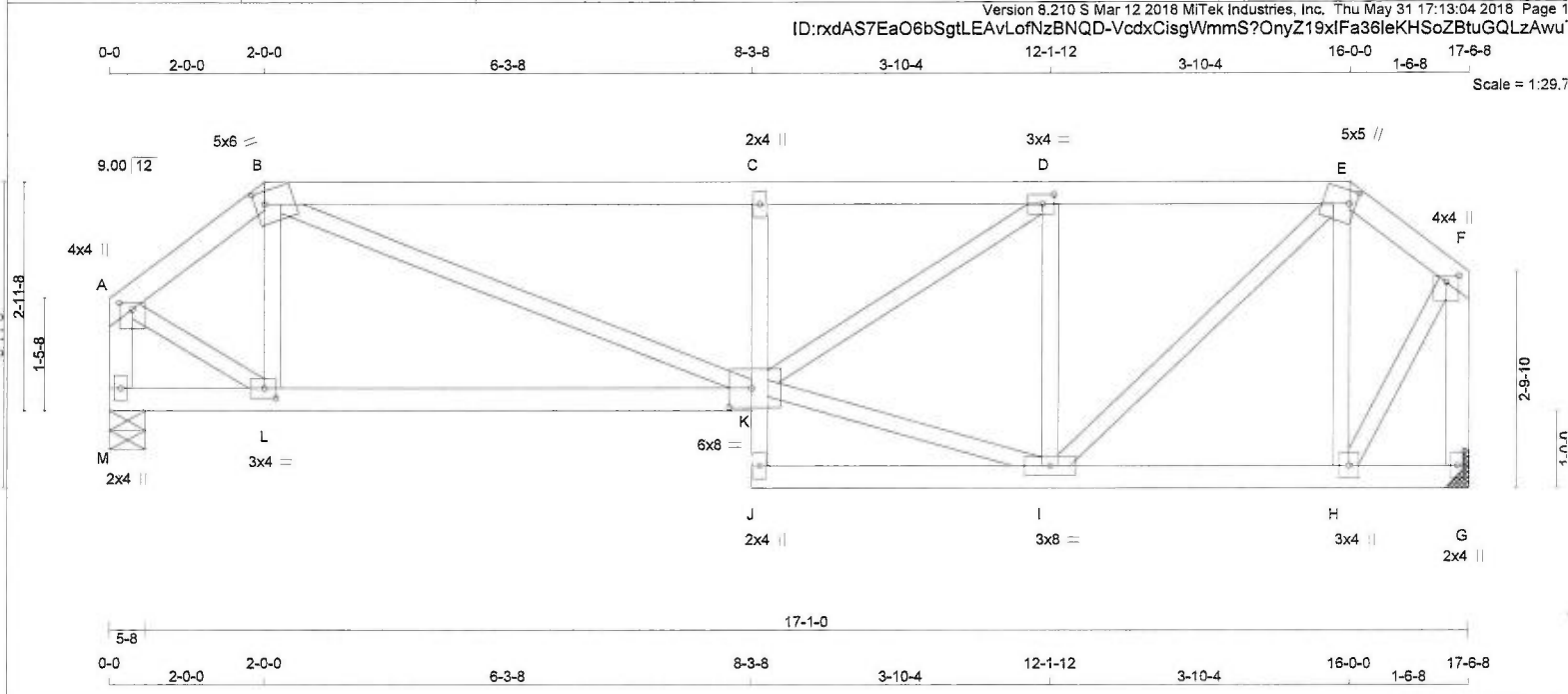
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (E) (INPUT = 0.90)
JSI METAL= 0.21 (A) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2
B - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
M - A	2x4	DRY	No.2
G - F	2x4	DRY	No.2
M - K	2x4	DRY	No.2
J - C	2x3	DRY	No.2
J - G	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVV+p	MT20	4.0	4.0	1.00	2.00
B	TTVV-m	MT20	5.0	6.0	2.00	1.50
C	TMV+p	MT20	2.0	4.0		
D	TMVV+t	MT20	3.0	4.0	1.50	1.75
E	TTVV+m	MT20	5.0	5.0	2.00	1.00
F	TMVV+p	MT20	4.0	4.0	1.00	2.00
G	BMV1+p	MT20	2.0	4.0		
H	BMVV+t	MT20	3.0	4.0		
I	BMVVW-t	MT20	3.0	8.0		
J	BMV+p	MT20	2.0	4.0		
K	BVMVVW-t	MT20	6.0	8.0	3.00	3.50
L	BMVVW-t	MT20	3.0	4.0	1.50	1.75
M	BMV1+p	MT20	2.0	4.0		



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	UPLIFT	IN-SX
M	832	0	832	0
G	832	0	832	0

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT G TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. SNOW	MIN. LIVE	PERM. LIVE	WIND	DEAD	SOIL
M	584	408 / 0	0 / 0	0 / 0	0 / 0	175 / 0	0 / 0
G	584	408 / 0	0 / 0	0 / 0	0 / 0	175 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.86 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH FR-TO	MAX. FACTORED CSI (LC)
FR-TO				FR-TO			
A-B	-745 / 0	-77.4	-77.4 0.06 (1)	6.25	L-B	-202 / 25	0.04 (1)
B-C	-1464 / 0	-77.4	-77.4 0.46 (1)	4.86	B-K	0 / 951	0.21 (1)
C-D	-1456 / 0	-77.4	-77.4 0.40 (1)	4.91	K-I	0 / 879	0.20 (1)
D-E	-871 / 0	-77.4	-77.4 0.17 (1)	6.25	K-D	0 / 718	0.16 (1)
E-F	-427 / 0	-77.4	-77.4 0.03 (1)	6.25	I-D	-671 / 0	0.16 (1)
M-A	-846 / 0	0.0	0.0 0.09 (1)	7.81	I-E	0 / 750	0.17 (1)
G-F	-828 / 0	0.0	0.0 0.12 (1)	7.81	H-E	-453 / 0	0.11 (1)
					A-L	0 / 673	0.15 (1)
					H-F	0 / 613	0.14 (1)
M-L	0 / 0	-17.5	-17.5 0.16 (4)	10.00			
L-K	0 / 585	-17.5	-17.5 0.20 (4)	10.00			
J-K	0 / 29	0.0	0.0 0.06 (1)	10.00			
K-C	-482 / 0	0.0	0.0 0.07 (1)	7.81			
J-I	0 / 21	-17.5	-17.5 0.06 (4)	10.00			
I-H	0 / 326	-17.5	-17.5 0.08 (4)	10.00			
H-G	0 / 0	-17.5	-17.5 0.03 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	PSF
LL	23.3	PSF
DL	3.0	PSF
BOT CH.	LL	PSF
LL	0.0	PSF
DL	7.0	PSF
TOTAL LOAD	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.58")

CALCULATED VERT. DEFL.(LL) = $L/999$ (0.05")

ALLOWABLE DEFL.(TL) = $L/360$ (0.58")

CALCULATED VERT. DEFL.(TL) = $L/999$ (0.10")

CSI: TC=0.46/1.00 (B-C:1), BC=0.20/1.00 (K-L:4), WB=0.21/1.00 (B-K:1), SS=0.23/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822
	2284	1656

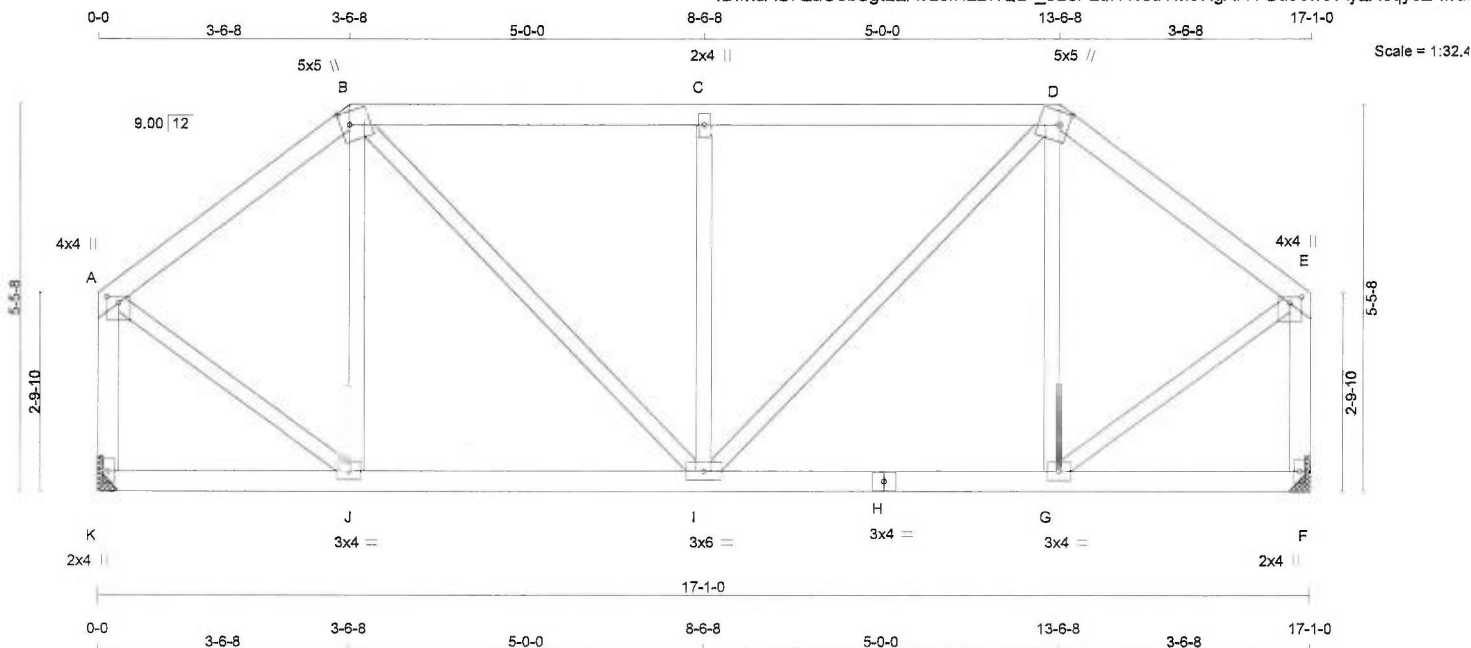
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (D) (INPUT = 0.90)

JSI METAL= 0.25 (B) (INPUT = 1.00)





TOTAL WEIGHT = 76 lb [M]

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - B	2x4	DRY	No.2
B - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
K - A	2x4	DRY	No.2
F - E	2x4	DRY	No.2
K - H	2x4	DRY	No.2
H - F	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TTWW+m	MT20	5.0	5.0	2.25	1.50
C	TMW+w	MT20	2.0	4.0		
D	TTWW+m	MT20	5.0	5.0	2.25	1.50
E	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	2.0	4.0		
G	BMVW-t	MT20	3.0	4.0		
H	BS-t	MT20	3.0	4.0		
I	BMVW-t	MT20	3.0	6.0		
J	BMVW-t	MT20	3.0	4.0		
K	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	DOWN	HORZ	UPLIFT
K	810	0	810	0
F	810	0	810	0

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT K, F TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN	COMPONENT REACTIONS
K	COMBINED	SNOW	LIVE PERM.LIVE WIND DEAD SOIL
K	569	398 / 0	0 / 0 0 / 0 171 / 0 0 / 0
F	569	398 / 0	0 / 0 0 / 0 171 / 0 0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX UNBRAC LENGTH	MEMB.	WEBS MAX. FACTORED FORCE (LBS)	MAX
FR-TO		FROM TO			FR-TO		
A-B	-569 / 0	-77.4 -77.4	0.17 (1)	6.25	J-B	-223 / 0	0.10 (1)
B-C	-720 / 0	-77.4 -77.4	0.33 (1)	6.25	B-I	0 / 388	0.09 (1)
C-D	-720 / 0	-77.4 -77.4	0.33 (1)	6.25	I-C	-474 / 0	0.21 (1)
D-E	-569 / 0	-77.4 -77.4	0.17 (1)	6.25	I-D	0 / 388	0.09 (1)
K-A	-787 / 0	0.0 0.0	0.12 (1)	7.81	G-D	-223 / 0	0.10 (1)
F-E	-787 / 0	0.0 0.0	0.12 (1)	7.81	A-J	0 / 548	0.12 (1)
					G-E	0 / 548	0.12 (1)
K-J	0 / 0	-17.5 -17.5	0.07 (4)	10.00			
J-I	0 / 449	-17.5 -17.5	0.12 (1)	10.00			
I-H	0 / 449	-17.5 -17.5	0.12 (1)	10.00			
H-G	0 / 449	-17.5 -17.5	0.12 (1)	10.00			
G-F	0 / 0	-17.5 -17.5	0.07 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.57")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.57")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.33/1.00 (B-C:1), BC=0.12/1.00 (I-J:1), WB=0.21/1.00 (C-I:1), SSI=0.19/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

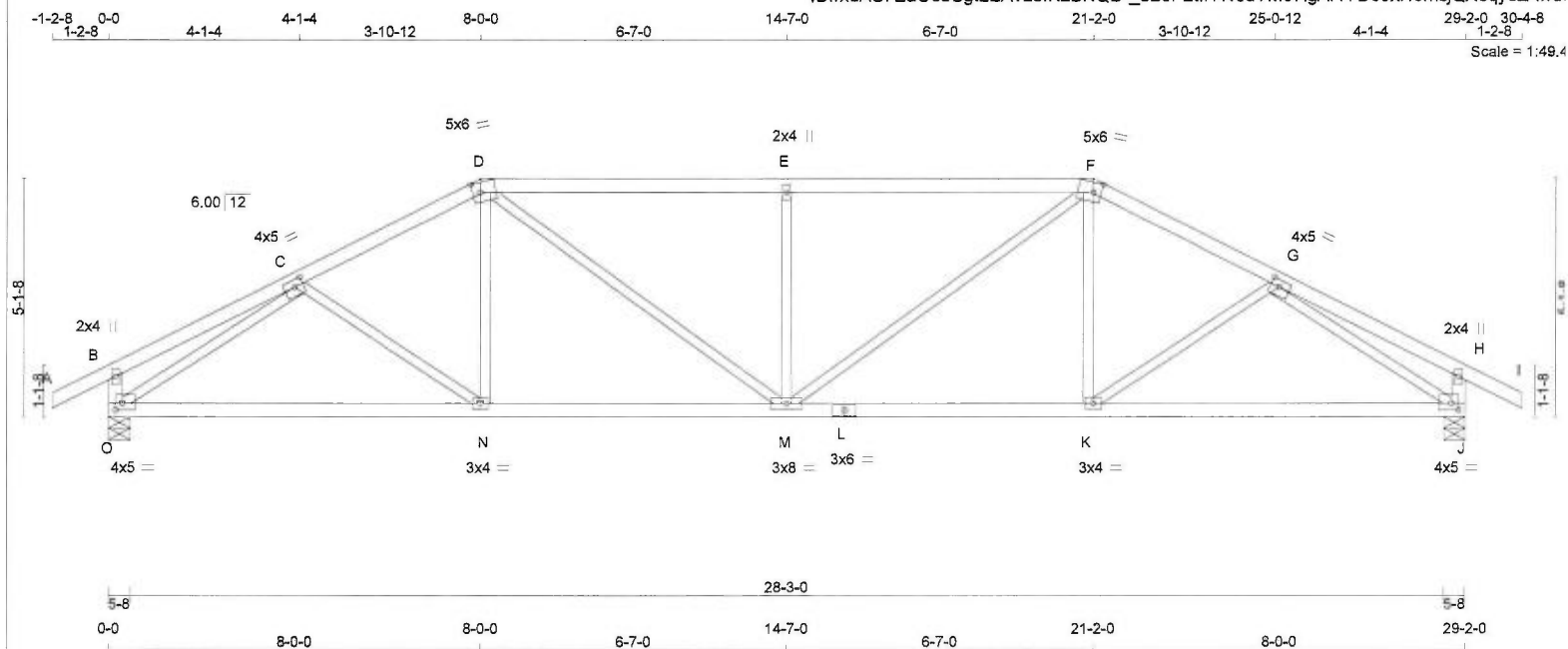
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.17 (E) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 112 lb
[M]/F

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWV-t	MT20	4.0	5.0	1.75	2.25
D	TTWW-m	MT20	5.0	6.0	2.50	2.00
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.50	2.00
G	TMWV-t	MT20	4.0	5.0	1.75	2.25
H	TMV+p	MT20	2.0	4.0		
J	BMWV1-t	MT20	4.0	5.0	1.75	1.75
K	BMWV-t	MT20	3.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWVW-t	MT20	3.0	8.0		
N	BMWV-t	MT20	3.0	4.0		
O	BMWV1-t	MT20	4.0	5.0	1.75	1.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ
JT				
O	1482	0	1482	0
J	1482	0	1482	0

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
O	1038	739 / 0	0 / 0	0 / 0	0 / 0	299 / 0	0 / 0
J	1038	739 / 0	0 / 0	0 / 0	0 / 0	299 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.09 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 22	-77.4 -77.4	0.09 (1)	10.00	C-N	-29 / 57	0.02 (4)
B-C	0 / 14	-77.4 -77.4	0.17 (1)	10.00	N-D	0 / 169	0.05 (4)
C-D	-1898 / 0	-77.4 -77.4	0.18 (1)	4.74	D-M	0 / 605	0.14 (1)
D-E	-2175 / 0	-77.4 -77.4	0.52 (1)	4.09	M-E	-626 / 0	0.24 (1)
E-F	-2175 / 0	-77.4 -77.4	0.52 (1)	4.09	M-F	0 / 605	0.14 (1)
F-G	-1898 / 0	-77.4 -77.4	0.18 (1)	4.74	K-F	0 / 169	0.05 (4)
G-H	0 / 14	-77.4 -77.4	0.17 (1)	10.00	K-G	-29 / 57	0.02 (4)
H-I	0 / 22	-77.4 -77.4	0.09 (1)	10.00	O-C	-2088 / 0	0.82 (1)
O-B	-223 / 0	0.0 0.0	0.02 (1)	7.81	G-J	-2088 / 0	0.82 (1)
J-H	-223 / 0	0.0 0.0	0.02 (1)	7.81			
O-N	0 / 1708	-17.5 -17.5	0.41 (1)	10.00			
N-M	0 / 1688	-17.5 -17.5	0.41 (1)	10.00			
M-L	0 / 1688	-17.5 -17.5	0.41 (1)	10.00			
L-K	0 / 1688	-17.5 -17.5	0.41 (1)	10.00			
K-J	0 / 1708	-17.5 -17.5	0.41 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL) = L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/999 (0.19")

CSI: TC=0.52/1.00 (D-E:1), BC=0.41/1.00 (J-K:1), WB=0.82/1.00 (G-J:1), SSI=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

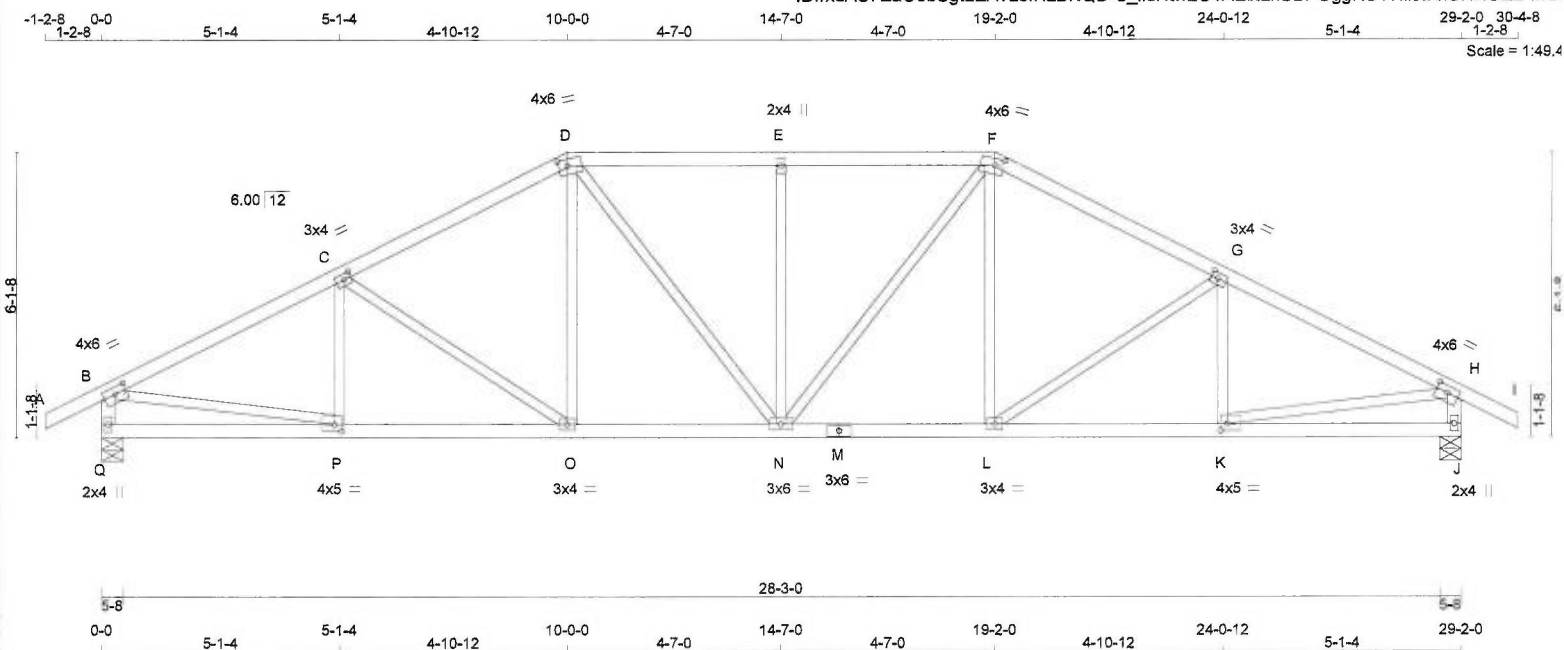
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (F) (INPUT = 0.90)
JSI METAL= 0.61 (C) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 121 lb
[M]F

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
Q - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
Q - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-t	MT20	4.0	6.0	1.75 3.00
C	TMVW-t	MT20	3.0	4.0	1.50 1.75
D	TTWW-m	MT20	4.0	6.0	1.75 2.25
E	TMVW-w	MT20	2.0	4.0	
F	TTWW-m	MT20	4.0	6.0	1.75 2.25
G	TMVW-t	MT20	3.0	4.0	1.50 1.75
H	TMVW-t	MT20	4.0	6.0	1.75 3.00
J	BMV1+p	MT20	2.0	4.0	
K	BMVW-t	MT20	4.0	5.0	1.75 1.75
L	BMVW-t	MT20	3.0	4.0	
M	BS-t	MT20	3.0	6.0	
N	BMVW-t	MT20	3.0	6.0	
O	BMVW-t	MT20	3.0	4.0	
P	BMVW-t	MT20	4.0	5.0	1.75 1.75
Q	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG
Q	1482 0	1482 0	5-8
J	1482 0	1482 0	5-8

UNFACTORED REACTIONS			
JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	REQRD
Q	1038 739 / 0	0 / 0	299 / 0
J	1038 739 / 0	0 / 0	299 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.52 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

CHORDS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1) (LC)
FR-TO		FROM TO	LENGTH FR-TO
A-B	0 / 22	-77.4 -77.4	0.09 (1) 10.00
B-C	-1995 / 0	-77.4 -77.4	0.30 (1) 4.52
C-D	-1769 / 0	-77.4 -77.4	0.28 (1) 4.76
D-E	-1765 / 0	-77.4 -77.4	0.24 (1) 4.80
E-F	-1765 / 0	-77.4 -77.4	0.24 (1) 4.80
F-G	-1769 / 0	-77.4 -77.4	0.28 (1) 4.76
G-H	-1995 / 0	-77.4 -77.4	0.30 (1) 4.52
H-I	0 / 22	-77.4 -77.4	0.09 (1) 10.00
Q-B	-1442 / 0	0.0 0.0	0.15 (1) 6.81
J-H	-1442 / 0	0.0 0.0	0.15 (1) 6.81
Q-P	0 / 0	-17.5 -17.5	0.10 (4) 10.00
P-O	0 / 1802	-17.5 -17.5	0.34 (1) 10.00
O-N	0 / 1567	-17.5 -17.5	0.30 (1) 10.00
N-M	0 / 1567	-17.5 -17.5	0.30 (1) 10.00
M-L	0 / 1567	-17.5 -17.5	0.30 (1) 10.00
L-K	0 / 1802	-17.5 -17.5	0.34 (1) 10.00
K-J	0 / 0	-17.5 -17.5	0.10 (4) 10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/999 (0.09")
ALLOWABLE DEFL.(TL) = L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/999 (0.16")

CSI: TC=0.30/1.00 (G-H:1), BC=0.34/1.00 (K-L:1),
WE=0.41/1.00 (H-K:1), SSI=0.17/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

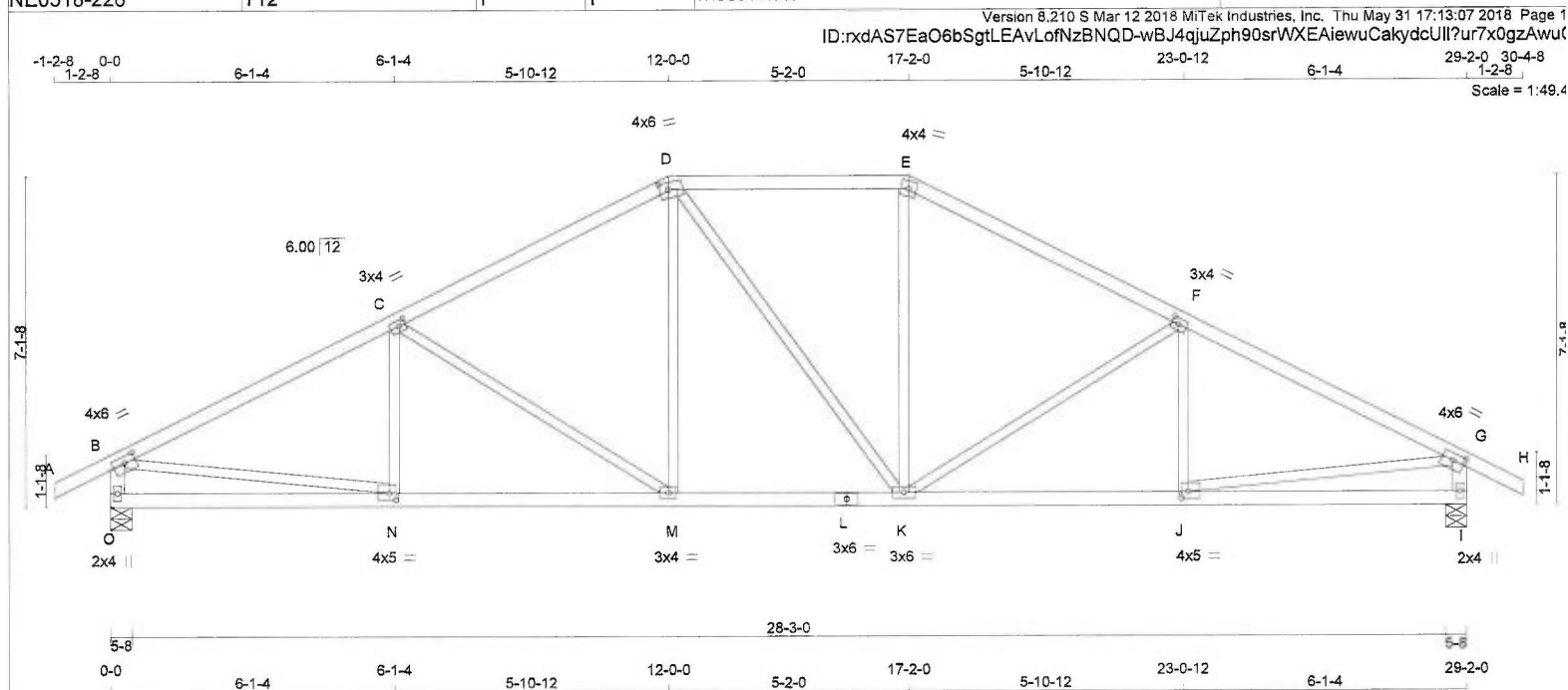
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.54 (K) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 118 lb [M]F

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	1.75	3.00
C	TMVW-t	MT20	3.0	4.0	1.50	1.75
D	TTWV-m	MT20	4.0	6.0	1.75	2.25
E	TTW-m	MT20	4.0	4.0		
F	TMVW-t	MT20	3.0	4.0	1.50	1.75
G	TMVW-t	MT20	4.0	6.0	1.75	3.00
I	BMV1+p	MT20	2.0	4.0		
J	BMVW-t	MT20	4.0	5.0	1.75	1.75
K	BMVW-t	MT20	3.0	6.0		
L	BS-t	MT20	3.0	6.0		
M	BMVW-t	MT20	3.0	4.0		
N	BMVW-t	MT20	4.0	5.0	1.75	1.75
O	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
O	1482	0	1482	0
I	1482	0	1482	0

UNFACTORED REACTIONS

	1ST LCASE	MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
O	1038	739 / 0	0 / 0	0 / 0	0 / 0	299 / 0	0 / 0
I	1038	739 / 0	0 / 0	0 / 0	0 / 0	299 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.34 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	FACTORED MAX. (LC)
FR-TO		FROM	TO	FR-TO			
A-B	0 / 22	-77.4	-77.4 0.09 (1)	10.00	N-C	-128 / 64	0.03 (1)
B-C	-2015 / 0	-77.4	-77.4 0.44 (1)	4.34	C-M	-465 / 0	0.46 (1)
C-D	-1628 / 0	-77.4	-77.4 0.40 (1)	4.77	M-D	0 / 346	0.08 (1)
D-E	-1439 / 0	-77.4	-77.4 0.29 (1)	5.13	D-K	0 / 0	0.00 (1)
E-F	-1628 / 0	-77.4	-77.4 0.40 (1)	4.77	K-E	0 / 347	0.08 (1)
F-G	-2015 / 0	-77.4	-77.4 0.44 (1)	4.34	K-F	-464 / 0	0.46 (1)
G-H	0 / 22	-77.4	-77.4 0.09 (1)	10.00	J-F	-129 / 63	0.03 (1)
O-B	-1437 / 0	0.0	0.0 0.14 (1)	6.81	B-N	0 / 1840	0.41 (1)
I-G	-1436 / 0	0.0	0.0 0.14 (1)	6.81	J-G	0 / 1840	0.41 (1)
O-N	0 / 0	-17.5	-17.5 0.15 (4)	10.00			
N-M	0 / 1824	-17.5	-17.5 0.36 (1)	10.00			
M-L	0 / 1438	-17.5	-17.5 0.29 (1)	10.00			
L-K	0 / 1438	-17.5	-17.5 0.29 (1)	10.00			
K-J	0 / 1823	-17.5	-17.5 0.36 (1)	10.00			
J-I	0 / 0	-17.5	-17.5 0.15 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.97")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.09")
ALLOWABLE DEFL.(TL) = $L/360$ (0.97")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.16")

CSI: TC=0.44/1.00 (B-C:1), BC=0.36/1.00 (M-N:1), WB=0.46/1.00 (C-M:1), SSI=0.20/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

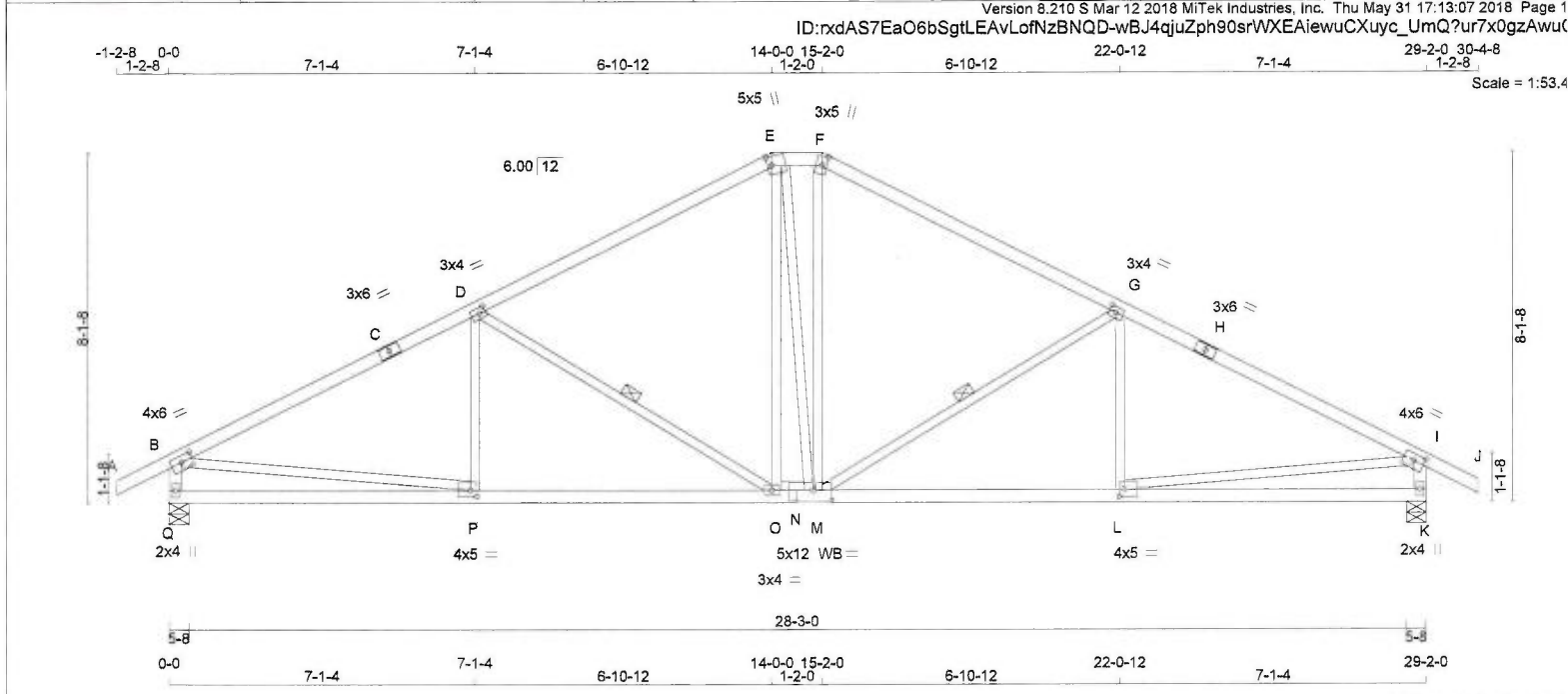
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.55 (N) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Version 8.210 S Mar 12 2018 MiTek Industries, Inc. Thu May 31 17:13:07 2018 Page 1
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N. L. G. A. RULES				BUILDING DESIGNER							DESIGN CRITERIA				
CHORDS	SIZE	LUMBER	DESCR.	BEARINGS							SPECIFIED LOADS:				
A - C	2x4	DRY	No.2	SPF	FACTORED		MAXIMUM FACTORED		INPUT	REQRD	TOP CH. LL = 23.3 PSF				
C - E	2x4	DRY	No.2	SPF	GROSS REACTION		GROSS REACTION		BRG		DL = 3.0 PSF				
E - F	2x4	DRY	No.2	SPF	JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	BOT CH. LL = 0.0 PSF		
F - H	2x4	DRY	No.2	SPF	Q	1482	0	1482	0	0	5-8	5-8	DL = 7.0 PSF		
H - J	2x4	DRY	No.2	SPF	K	1482	0	1482	0	0	5-8	5-8	TOTAL LOAD = 33.3 PSF		
Q - B	2x4	DRY	No.2	SPF	UNFACTORED REACTIONS									SPACING = 24.0 IN. C/C	
K - I	2x4	DRY	No.2	SPF											
Q - N	2x4	DRY	No.2	SPF	1ST LCASE		MAX./MIN. COMPONENT REACTIONS								
N - K	2x4	DRY	No.2	SPF	JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL			
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	Q	1038	739 / 0	0 / 0	0 / 0	0 / 0	299 / 0	0 / 0			
					K	1038	739 / 0	0 / 0	0 / 0	0 / 0	299 / 0	0 / 0			
LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM															

ALL WEBS 2x3 DRY No.2 SPF TOTAL WEIGHT = 125 lb [M]F

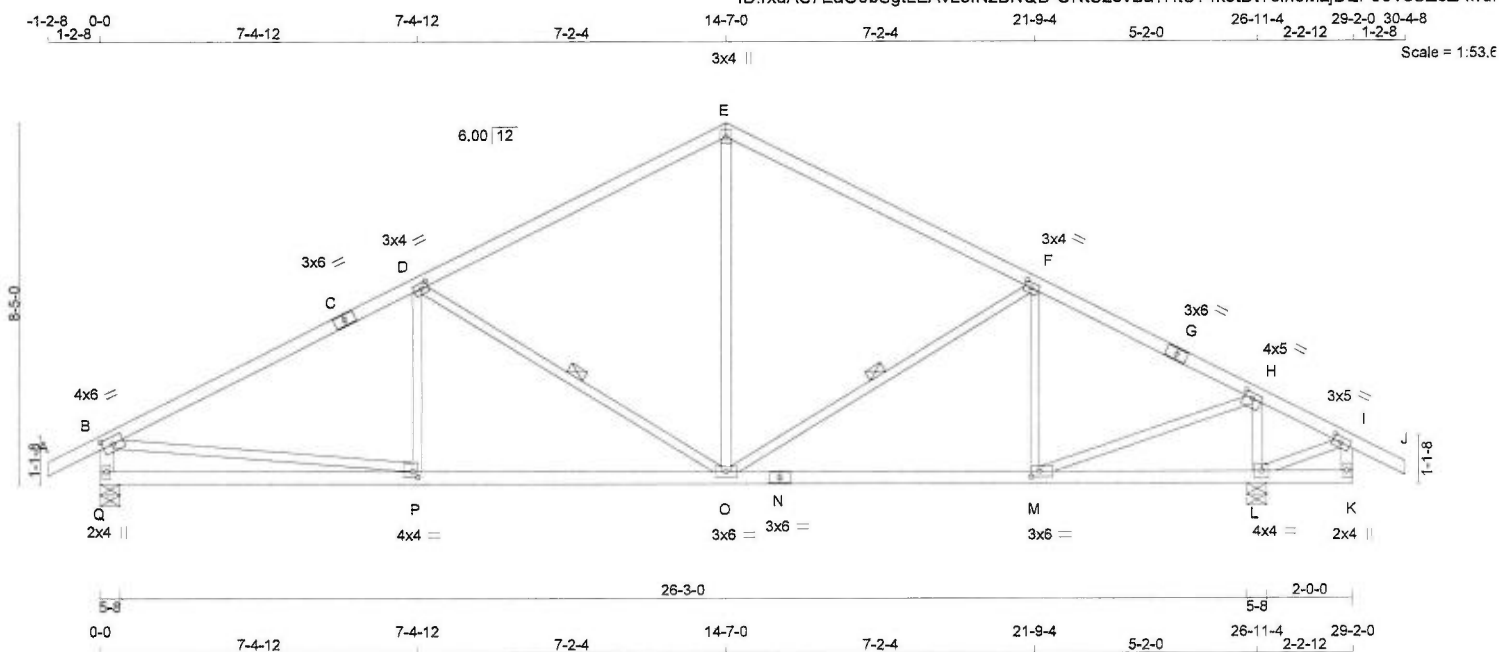
EXCEPT DRY: SEASONED LUMBER.

PLATES (table is in inches)				BRACING				LOADING			
JT	TYPE	PLATES	W	LEN	Y	X		TOTAL LOAD CASES: (4)			
B	TMVW-t	MT20	4.0	6.0	1.75	3.00		CHORDS			
C	TS-t	MT20	3.0	6.0				MAX. FACTORED			
D	TMWW-t	MT20	3.0	4.0	1.50	1.75		FACTORED			
E	TTWW+m	MT20	5.0	5.0	2.50	1.25		VERT. LOAD LC1 MAX			
F	TTW+m	MT20	3.0	5.0	2.50	1.25		FROM TO			
G	TMWW-t	MT20	3.0	4.0	1.50	1.75		MAX. UNBRAC			
H	TS-t	MT20	3.0	6.0				LENGTH FR-TO			
I	TMVW-t	MT20	4.0	6.0	1.75	3.00		MEMB. FORCE			
K	BMV1+p	MT20	2.0	4.0				MAX. FACTORED			
L	BMWW-t	MT20	4.0	5.0	2.00	1.50		VERT. LOAD LC1 MAX			
M								FROM TO			
N	BSWWW-I	MT20	5.0	12.0	3.00	5.00		MEMB. FORCE			
O	BMWW-t	MT20	3.0	4.0				MAX. FACTORED			
P	BMWW-t	MT20	4.0	5.0	1.75	1.75		VERT. LOAD LC1 MAX			
Q	BMV1+p	MT20	2.0	4.0				FROM TO			

WB - INDICATES BLOCKING REQUIRED

CHORDS				WEBS			
MEMB.	FORCE (LBS)	FACTORED (PLF)	CSI (LC)	MEMB.	FORCE (LBS)	FACTORED (PLF)	CSI (LC)
FR-TO				FR-TO			
A-B	0 / 22	-77.4	-77.4 0.09 (1)	P-D	-70 / 98	0.03 (4)	
B-C	-2011 / 0	-77.4	-77.4 0.62 (1)	D-O	-633 / 0	0.29 (1)	
C-D	-2011 / 0	-77.4	-77.4 0.62 (1)	O-E	0 / 373	0.08 (1)	
D-E	-1470 / 0	-77.4	-77.4 0.54 (1)	E-M	0 / 27	0.01 (1)	
E-F	-1296 / 0	-77.4	-77.4 0.03 (1)	M-F	0 / 401	0.09 (1)	
F-G	-1473 / 0	-77.4	-77.4 0.55 (1)	M-G	-627 / 0	0.29 (1)	
G-H	-2009 / 0	-77.4	-77.4 0.62 (1)	L-G	-75 / 96	0.03 (4)	
H-I	-2009 / 0	-77.4	-77.4 0.62 (1)	B-P	0 / 1836	0.41 (1)	
I-J	0 / 22	-77.4	-77.4 0.09 (1)	L-I	0 / 1835	0.41 (1)	
Q-B	-1431 / 0	0.0	0.0 0.14 (1)				
K-I	-1430 / 0	0.0	0.0 0.14 (1)				
Q-P	0 / 0	-17.5	-17.5 0.22 (4)				
P-O	0 / 1824	-17.5	-17.5 0.40 (1)				
O-N	0 / 1292	-17.5	-17.5 0.30 (1)				
N-M	0 / 1292	-17.5	-17.5 0.30 (1)				
M-L	0 / 1823	-17.5	-17.5 0.39 (1)				
L-K	0 / 0	-17.5	-17.5 0.21 (4)				

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



TOTAL WEIGHT = 8 X 115 = 919 lb [M]/F

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - J	2x4	DRY	No.2
J - B	2x4	DRY	No.2
K - I	2x4	DRY	No.2
Q - N	2x4	DRY	No.2
N - K	2x4	DRY	No.2

ALL WEBS EXCEPT			
2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT VERT	DOWN	UP	IN-SX
Q 1359	0	0	5-8
L 1605	0	0	5-8

UNFACTORED REACTIONS							
1ST LCASE	MAX/MIN.	COMPONENT	REACTIONS				
JT COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	
Q 952	677 / 0	0 / 0	0 / 0	0 / 0	275 / 0	0 / 0	
L 1124	800 / 0	0 / 0	0 / 0	0 / 0	324 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF D-O, F-O. DBS = 20-0-0. CBF = 86 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 22	-77.4 -77.4	0.09 (1)	10.00	P-D	-40 / 107	0.04 (4)
B-C	-1780 / 0	-77.4 -77.4	0.65 (1)	4.25	D-O	-891 / 0	0.34 (1)
C-D	-1780 / 0	-77.4 -77.4	0.65 (1)	4.25	O-E	0 / 598	0.13 (1)
D-E	-1186 / 0	-77.4 -77.4	0.58 (1)	5.07	O-F	-187 / 0	0.09 (1)
E-F	-1178 / 0	-77.4 -77.4	0.51 (1)	5.21	M-F	-406 / 0	0.14 (1)
F-G	-1308 / 0	-77.4 -77.4	0.42 (1)	5.15	M-H	0 / 1448	0.33 (1)
G-H	-1308 / 0	-77.4 -77.4	0.42 (1)	5.15	L-H	-1453 / 0	0.22 (1)
H-I	0 / 201	-77.4 -77.4	0.14 (1)	10.00	B-P	0 / 1629	0.37 (1)
I-J	0 / 22	-77.4 -77.4	0.09 (1)	10.00	L-I	-176 / 0	0.03 (1)
Q-B	-1305 / 0	0.0 0.0	0.13 (1)	7.06			
K-I	-5 / 7	0.0 0.0	0.00 (4)	7.81			
Q-P	0 / 0	-17.5 -17.5	0.22 (4)	10.00			
P-O	0 / 1619	-17.5 -17.5	0.37 (1)	10.00			
O-N	0 / 1192	-17.5 -17.5	0.30 (1)	10.00			
N-M	0 / 1192	-17.5 -17.5	0.30 (1)	10.00			
M-L	-165 / 0	-17.5 -17.5	0.11 (4)	6.25			
L-K	0 / 0	-17.5 -17.5	0.07 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.90")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (0.90")
CALCULATED VERT. DEFL.(TL) = L/999 (0.14")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL) = L/120 (0.22")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/120 (0.22")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.85/1.00 (B-D:1), BC=0.37/1.00 (O-P:1), WB=0.37/1.00 (B-P:1), SS=0.25/1.00 (B-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

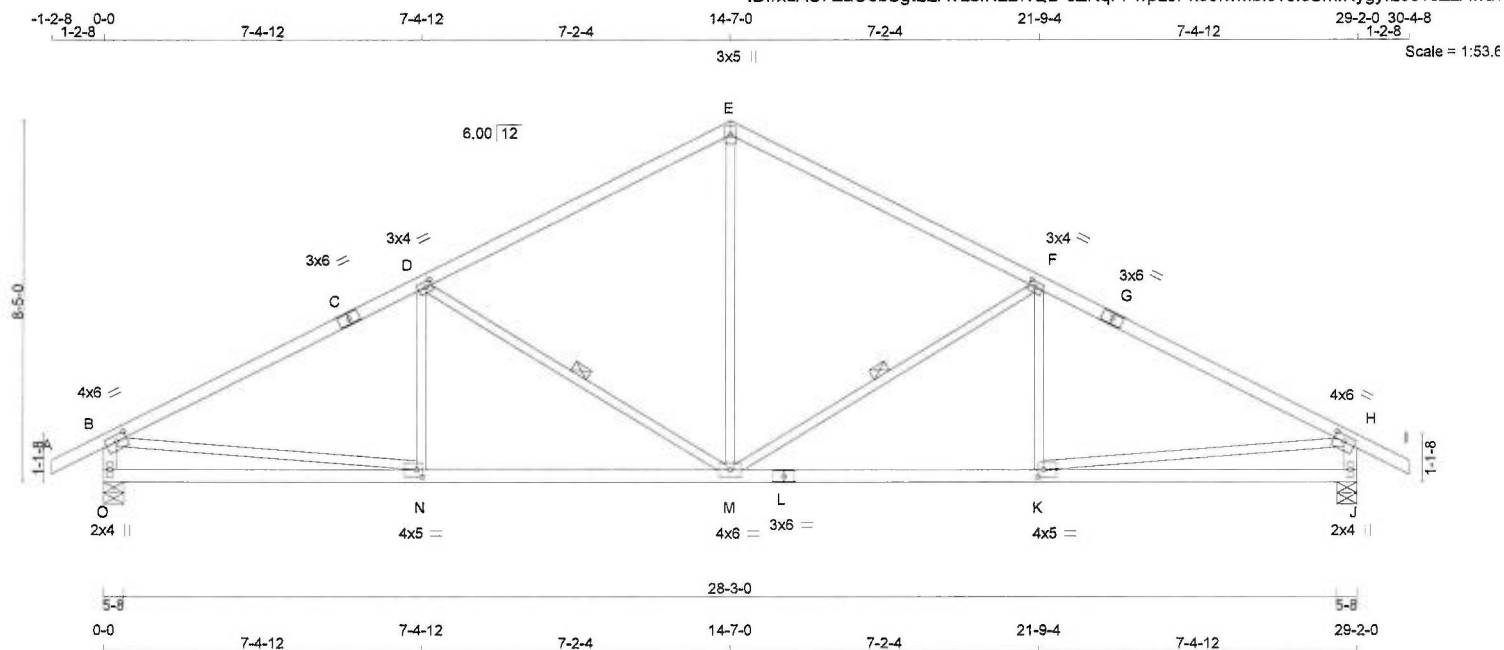
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (M) (INPUT = 0.90)
JSI METAL= 0.61 (P) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = $5 \times 113 = 567 \text{ lb}$
[M][F]

ESTIMATED
N. L. G. A.

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - E	2x4	DRY No.2	SPF
E - G	2x4	DRY No.2	SPF
G - I	2x4	DRY No.2	SPF
O - B	2x4	DRY No.2	SPF
J - H	2x4	DRY No.2	SPF
O - L	2x4	DRY No.2	SPF
L - J	2x4	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

JT TYPE	PLATES
1	1
2	2
3	3
4	4
5	5
6	6
7	7
8	8
9	9
10	10
11	11
12	12
13	13
14	14
15	15
16	16
17	17
18	18
19	19
20	20
21	21
22	22
23	23
24	24
25	25
26	26
27	27
28	28
29	29
30	30
31	31
32	32
33	33
34	34
35	35
36	36
37	37
38	38
39	39
40	40
41	41
42	42
43	43
44	44
45	45
46	46
47	47
48	48
49	49
50	50
51	51
52	52
53	53
54	54
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56	56
57	57
58	58
59	59
60	60
61	61
62	62
63	63
64	64
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66	66
67	67
68	68
69	69
70	70
71	71
72	72
73	73
74	74
75	75
76	76
77	77
78	78
79	79
80	80
81	81
82	82
83	83
84	84
85	85
86	86
87	87
88	88
89	89
90	90
91	91
92	92
93	93
94	94
95	95
96	96
97	97
98	98
99	99
100	100

C	TMVW-t	MT20	4.0	6.0	1.75	3.00
B	TS-t	MT20	3.0	6.0		
D	TMVW-t	MT20	3.0	4.0	1.50	1.75
E	TTW-t	MT20	3.0	5.0		
F	TMVW-t	MT20	3.0	4.0	1.50	1.75
G	TS-t	MT20	3.0	6.0		
H	TMVW-t	MT20	4.0	6.0	1.75	3.00
J	BMV1+p	MT20	2.0	4.0		
K	BMVW-t	MT20	4.0	5.0	2.00	1.50
L	BS-t	MT20	3.0	6.0		
M	BMVW-t	MT20	4.0	6.0		
N	BMVW-t	MT20	4.0	5.0	2.00	1.50
O	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
O	1482	0	1482	0	0	5-8	5-8
J	1482	0	1482	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS					SOIL
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	
O	1038	739 / 0	0 / 0	0 / 0	0 / 0	299 / 0	0 / 0
J	1038	739 / 0	0 / 0	0 / 0	0 / 0	299 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.03 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 -1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF F-M, D-M. DBS = 20-0-0 . CBF = 84 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED		WEBS	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 22	-77.4	-77.4	0.09 (1)	10.00	M-E	0 / 835 0.19 (1)
B-C	-2004 / 0	-77.4	-77.4	0.68 (1)	4.03	M-F	-669 / 0 0.33 (1)
C-D	-2004 / 0	-77.4	-77.4	0.68 (1)	4.03	K-F	-61 / 103 0.04 (4)
D-E	-1431 / 0	-77.4	-77.4	0.59 (1)	4.70	D-M	-669 / 0 0.33 (1)
E-F	-1431 / 0	-77.4	-77.4	0.59 (1)	4.70	N-D	-61 / 103 0.04 (4)
F-G	-2004 / 0	-77.4	-77.4	0.68 (1)	4.03	B-N	0 / 1830 0.41 (1)
G-H	-2004 / 0	-77.4	-77.4	0.68 (1)	4.03	K-H	0 / 1830 0.41 (1)
H-I	0 / 22	-77.4	-77.4	0.09 (1)	10.00		
O-B	-1428 / 0	0.0	0.0	0.14 (1)	6.83		
J-H	-1428 / 0	0.0	0.0	0.14 (1)	6.83		
O-N	0 / 0	-17.5	-17.5	0.22 (4)	10.00		
N-M	0 / 1819	-17.5	-17.5	0.40 (1)	10.00		
M-L	0 / 1819	-17.5	-17.5	0.40 (1)	10.00		
L-K	0 / 1819	-17.5	-17.5	0.40 (1)	10.00		
K-J	0 / 0	-17.5	-17.5	0.22 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.68/1.00 (F-H:1), BC=0.40/1.00 (K-M:1),
WB=0.41/1.00 (H-K:1), SSI=0.25/1.00 (F-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

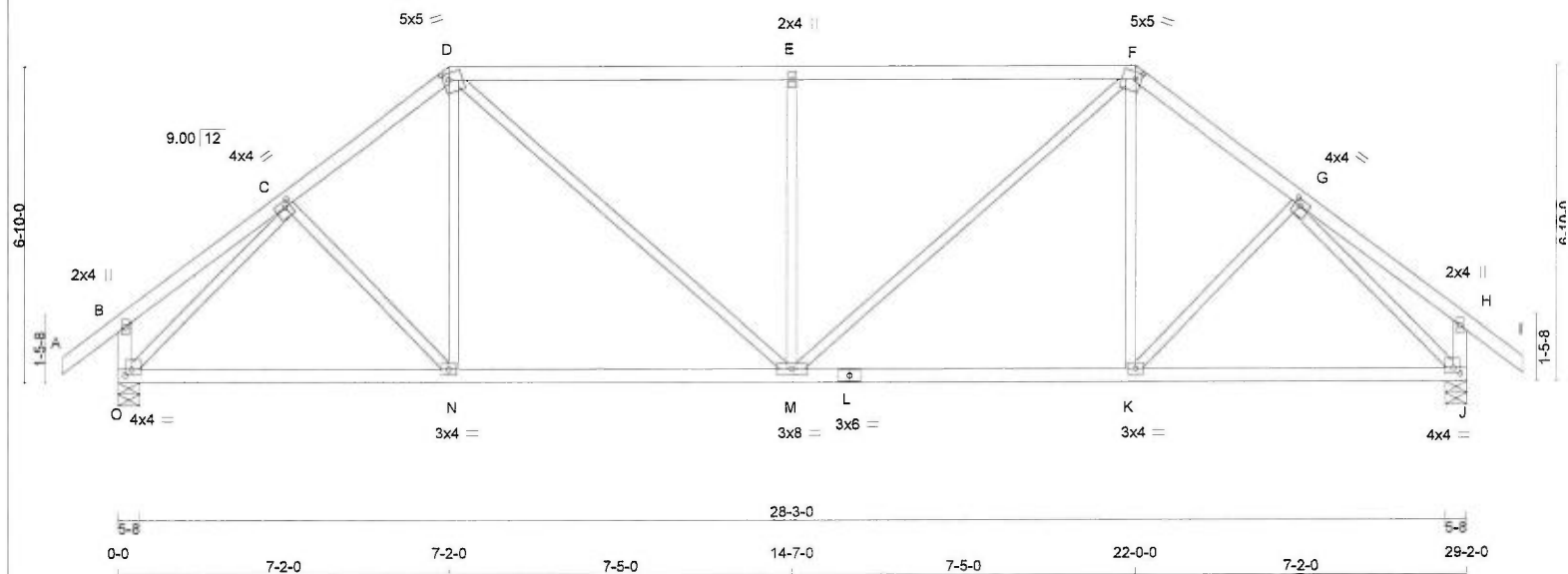
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (N) (INPUT = 0.90)
JSI METAL= 0.55 (K) (INPUT = 1.00)



**READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.**





LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
O - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
O - L	2x4	DRY	No.2
L - J	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMVW-t	MT20	4.0	4.0	1.50	1.50
D	TMVW-m	MT20	5.0	5.0	1.75	1.50
E	TMVW-w	MT20	2.0	4.0		
F	TMVW-m	MT20	5.0	5.0	1.75	1.50
G	TMVW-t	MT20	4.0	4.0	1.50	1.50
H	TMV+p	MT20	2.0	4.0		
J	BMVW-t	MT20	4.0	4.0	1.50	1.75
K	BMVW-t	MT20	3.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMVW-t	MT20	3.0	8.0		
N	BMVW-t	MT20	3.0	4.0		
O	BMVW-t	MT20	4.0	4.0	1.50	1.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	DOWN	UPLIFT	IN-SX
O	1484	0	1484	0
J	1484	0	1484	0

UNFACTORED REACTIONS

	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
JT	COMBINED	SNOW	LIVE	PERM. LIVE	
O	1039	740 / 0	0 / 0	0 / 0	299 / 0
J	1039	740 / 0	0 / 0	0 / 0	299 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.82 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 30	-77.4	-77.4 0.09 (1)	10.00	C-N	0 / 52	0.02 (4)
B-C	0 / 18	-77.4	-77.4 0.15 (1)	10.00	N-D	0 / 133	0.04 (4)
C-D	-1471 / 0	-77.4	-77.4 0.18 (1)	5.22	D-M	0 / 624	0.14 (1)
D-E	-1630 / 0	-77.4	-77.4 0.92 (1)	3.82	M-E	-706 / 0	0.55 (1)
E-F	-1630 / 0	-77.4	-77.4 0.92 (1)	3.82	M-F	0 / 624	0.14 (1)
F-G	-1471 / 0	-77.4	-77.4 0.18 (1)	5.22	K-F	0 / 133	0.04 (4)
G-H	0 / 18	-77.4	-77.4 0.15 (1)	10.00	K-G	0 / 52	0.02 (4)
H-I	0 / 30	-77.4	-77.4 0.09 (1)	10.00	O-C	-1678 / 0	0.73 (1)
O-B	-209 / 0	0.0	0.0 0.02 (1)	7.81	G-J	-1678 / 0	0.73 (1)
J-H	-209 / 0	0.0	0.0 0.02 (1)	7.81			
O-N	0 / 1147	-17.5	-17.5 0.31 (1)	10.00			
N-M	0 / 1162	-17.5	-17.5 0.32 (4)	10.00			
M-L	0 / 1162	-17.5	-17.5 0.32 (4)	10.00			
L-K	0 / 1162	-17.5	-17.5 0.32 (4)	10.00			
K-J	0 / 1147	-17.5	-17.5 0.31 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/999 (0.13")

CSI: TC=0.92/1.00 (D-E:1), BC=0.32/1.00 (K-M:4), WB=0.73/1.00 (G-J:1), SSI=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

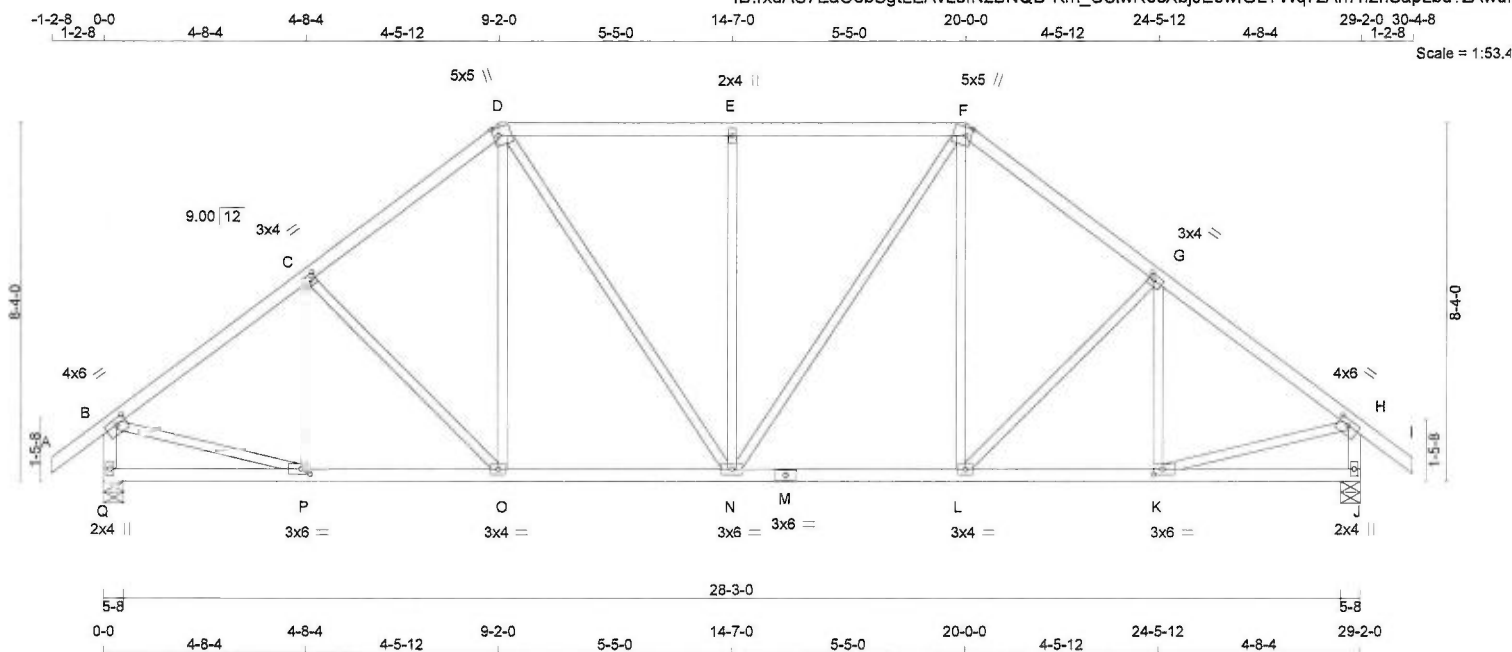
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (J) (INPUT = 0.90)
JSI METAL= 0.62 (C) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:53.4

TOTAL WEIGHT = 137 lb
[M]F

LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	DOWN	IN-SX	IN-SX
Q	1484	0	5-8	5-8
J	1484	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	1039	740 / 0	0 / 0	0 / 0	0 / 0	299 / 0	0 / 0
J	1039	740 / 0	0 / 0	0 / 0	0 / 0	299 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.13 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)
FR-TO			FROM TO		FR-TO			
A-B	0 / 30	-77.4	-77.4	0.09 (1)	10.00	P-C	-209 / 13	0.08 (1)
B-C	-1498 / 0	-77.4	-77.4	0.24 (1)	5.13	C-O	-188 / 0	0.15 (1)
C-D	-1386 / 0	-77.4	-77.4	0.23 (1)	5.29	O-D	0 / 222	0.05 (1)
D-E	-1293 / 0	-77.4	-77.4	0.31 (1)	5.32	D-N	0 / 365	0.08 (1)
E-F	-1293 / 0	-77.4	-77.4	0.31 (1)	5.32	N-E	-511 / 0	0.70 (1)
F-G	-1386 / 0	-77.4	-77.4	0.23 (1)	5.29	N-F	0 / 365	0.08 (1)
G-H	-1498 / 0	-77.4	-77.4	0.24 (1)	5.13	L-F	0 / 222	0.05 (1)
H-I	0 / 30	-77.4	-77.4	0.09 (1)	10.00	L-G	-188 / 0	0.15 (1)
Q-B	-1448 / 0	0.0	0.0	0.15 (1)	6.79	K-G	-209 / 13	0.08 (1)
J-H	-1448 / 0	0.0	0.0	0.15 (1)	6.79	B-P	0 / 1253	0.28 (1)
						K-H	0 / 1253	0.28 (1)
Q-P	0 / 0	-17.5	-17.5	0.08 (4)	10.00			
P-O	0 / 1218	-17.5	-17.5	0.24 (1)	10.00			
O-N	0 / 1089	-17.5	-17.5	0.23 (1)	10.00			
N-M	0 / 1089	-17.5	-17.5	0.23 (1)	10.00			
M-L	0 / 1089	-17.5	-17.5	0.23 (1)	10.00			
L-K	0 / 1218	-17.5	-17.5	0.24 (1)	10.00			
K-J	0 / 0	-17.5	-17.5	0.08 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/999 (0.10")

CSI: TC=0.31/1.00 (D-E:1), BC=0.24/1.00 (O-P:1),
WB=0.70/1.00 (E-N:1), SSI=0.20/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (N) (INPUT = 0.90)
JSI METAL= 0.42 (B) (INPUT = 1.00)

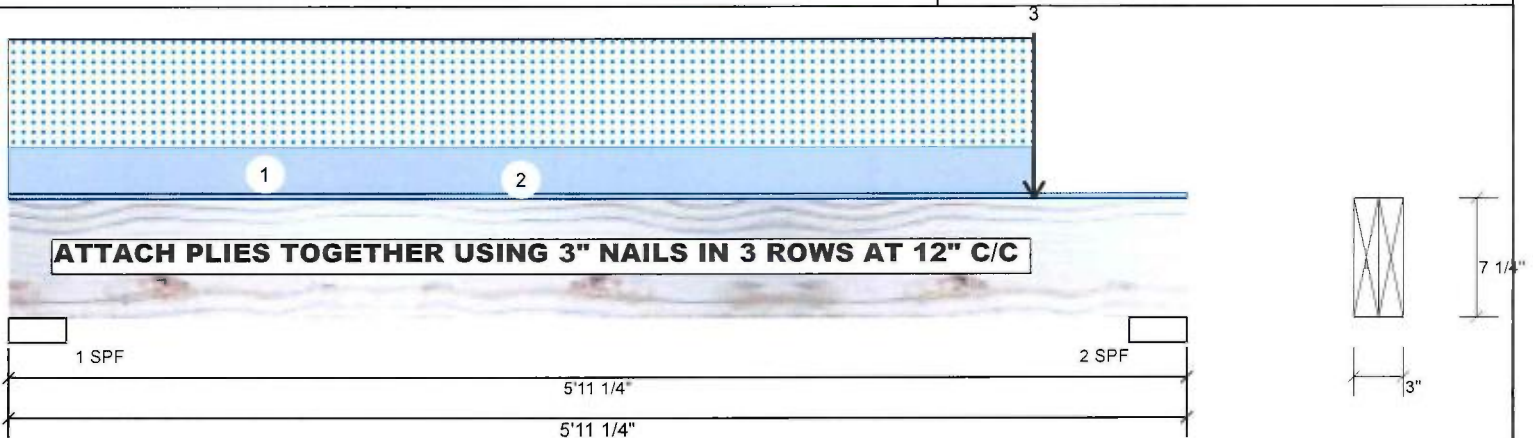


READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



B2 S-P-F #2 2.000" X 8.000" 2-Ply - PASSED

Level: Level



Member Information

Type:	Girder	Application:	Roof (Residential)
Plies:	2	Slope:	0/12
Moisture Condition:	Dry	Design Method:	LSD
Deflection LL:	480	Building Code:	NBCC 2010 / OBC 2012
Deflection TL:	240	Load Sharing:	No
Importance:	Normal	Deck:	Not Checked
		Vibration:	Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	0	331	699	0
2	0	611	1352	0

Bearings and Factored Reactions

Bearing	Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
1 - SPF	3.500"	23% 414 / 1048	1462 L	1.25D+1.5S
2 - SPF	3.500"	43% 764 / 2028	2792 L	1.25D+1.5S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2169 ft-lb	3' 4 15/16"	4047 ft-lb	0.536 (54%)	1.25D+1.5S	L
Unbraced	2169 ft-lb	3' 4 15/16"	4024 ft-lb	0.539 (54%)	1.25D+1.5S	L
Shear	2546 lb	5' 1 1/4"	3406 lb	0.747 (75%)	1.25D+1.5S	L
LL Defl inch	0.044 (L/1493)	3' 15/16"	0.137 (L/480)	0.320 (32%)	S	L
TL Defl inch	0.065 (L/1019)	3' 15/16"	0.274 (L/240)	0.240 (24%)	D+S	L

Design Notes

- Girders are designed to be supported on the bottom edge only.
- Multiple plies must be fastened together as per manufacturer's details.
- Top loads must be supported equally by all plies.
- Top braced at bearings.
- Bottom braced at bearings.
- Lateral slenderness ratio based on full section width.



ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Part. Uniform	0-0-0 to 5-2-0		Top	88 PLF	0 PLF	204 PLF	0 PLF	
2	Uniform			Top	10 PLF	0 PLF	0 PLF	0 PLF	
3	Point	5-2-0		Top	428 lb	0 lb	997 lb	0 lb	

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Manufacturer Info

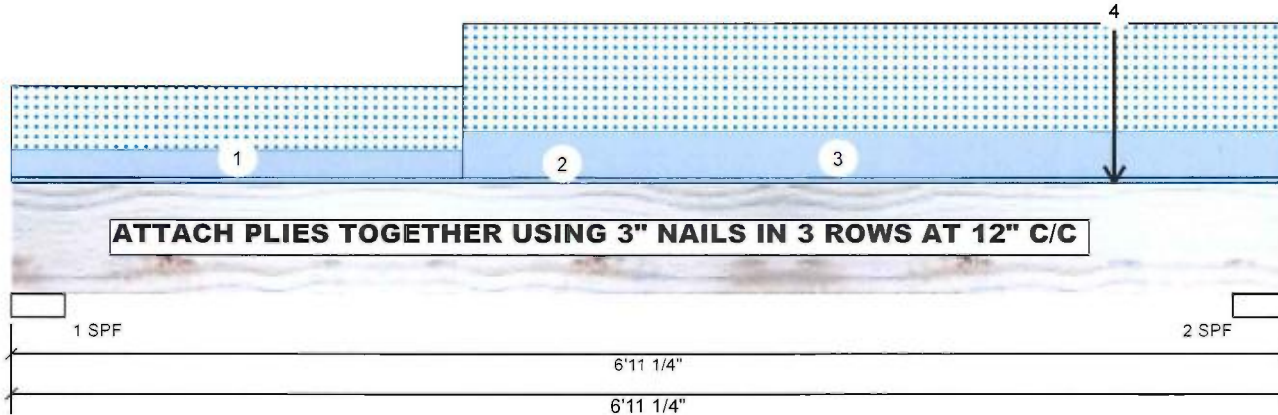


Kott Group
14 Anderson Blvd., On
L4A7X4
905-642-4400



B1 S-P-F #2 2.000" X 8.000" 2-Ply - PASSED

Level: Level



Member Information

Type:	Girder	Application:	Roof (Residential)
Plies:	2	Slope:	0/12
Moisture Condition:	Dry	Design Method:	LSD
Deflection LL:	480	Building Code:	NBCC 2010 / OBC 2012
Deflection TL:	240	Load Sharing:	No
Importance:	Normal	Deck:	Not Checked
		Vibration:	Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	0	280	578	0
2	0	481	1093	0

Bearings and Factored Reactions

Bearing	Length	Cap. React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	3.500"	19%	350 / 868	1218 L	1.25D+1.5S
2 - SPF	3.500"	35%	601 / 1639	2240 L	1.25D+1.5S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2369 ft-lb	3'10 7/16"	4047 ft-lb	0.585 (59%)	1.25D+1.5S	L
Unbraced	2369 ft-lb	3'10 7/16"	4015 ft-lb	0.590 (59%)	1.25D+1.5S	L
Shear	1890 lb	6'1 1/4"	3406 lb	0.555 (55%)	1.25D+1.5S	L
LL Defl inch	0.066 (L/1182)	3'7"	0.162 (L/480)	0.410 (41%)	S	L
TL Defl inch	0.096 (L/807)	3'6 15/16"	0.324 (L/240)	0.300 (30%)	D+S	L

Design Notes

- Girders are designed to be supported on the bottom edge only.
- Multiple plies must be fastened together as per manufacturer's details.
- Top loads must be supported equally by all plies.
- Top braced at bearings.
- Bottom braced at bearings.
- Lateral slenderness ratio based on full section width.



ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Part. Uniform	0-0-0 to 2-5-8		Top	51 PLF	0 PLF	119 PLF	0 PLF	
2	Uniform			Top	10 PLF	0 PLF	0 PLF	0 PLF	
3	Part. Uniform	2-5-8 to 6-11-4		Top	86 PLF	0 PLF	200 PLF	0 PLF	
4	Point	6-0-0		Top	181 lb	0 lb	483 lb	0 lb	

**READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
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Manufacturer Info



Kott Group
14 Anderson Blvd., On
L4A7X4
905-642-4400

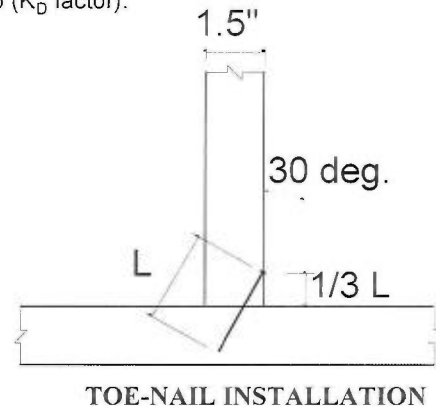
BEARING ANCHORAGE BY TOE-NAILS FOR LATERAL CAPACITY

B37579H1

NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL LATERAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	132	147
	3.25	0.144	132	147
	3.50	0.160	159	177

NOTES:

1. Rafter and ceiling members may be anchored to top and bottom chords of girder truss by toe-nailing rafter and ceiling members to girder chords provided the reaction does not exceed the lateral capacities in the table. Hangers (specified by others) are required for reactions higher than the maximum toe-nail capacity. Reactions are based on factored loads.
2. Toe nail capacities shown in the table are for one toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-09, section 10.9.4.1.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in tables below.
5. Nail values in table are based on the following relative lumber densities: G = 0.42 (SPF), G = 0.49 (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See next page for nailing on bearing plate).
7. For loads due to wind the nail lateral capacity in this table may be multiplied by 1.15 (K_D factor).
8. Lumber must be dry (< 19% moisture content) at the time of nail installation.
9. Nail values in this table comply with CSA O86-09, section 10.9.4
10. This design is not valid after April 30, 2019.



Nail type	Common wire	Common spiral	Common wire
Nail dia. (in)	0.160	0.152	0.144
	(3.5" nail)		(3" and 3.25" nail)
LUMBER SIZE	MAXIMUM NUMBER OF TOE-NAILS		
2X4 SPF	2	2	3
2X4 D. Fir	2	2	2
2X6 SPF	4	4	4
2X6 D. Fir	3	3	3



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PEO
Certificate No. 10889485



April 26, 2017

BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING

B37579H2

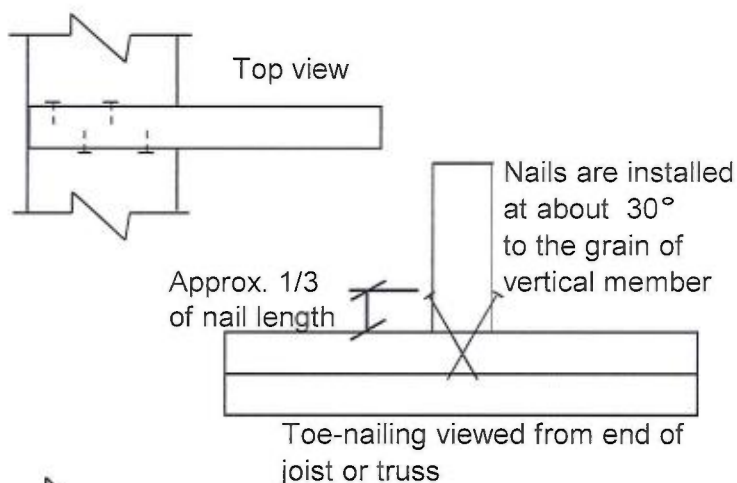
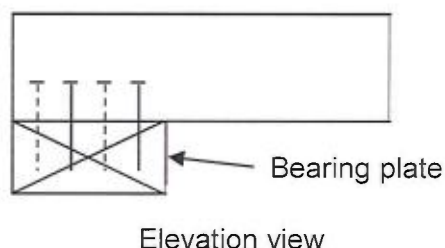
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	30	42
	3.25	0.144	32	45
	3.50	0.160	38	52

Note: If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

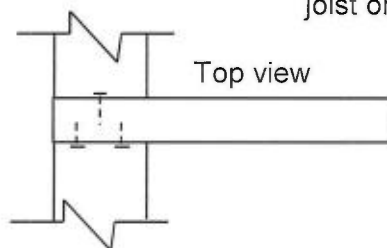
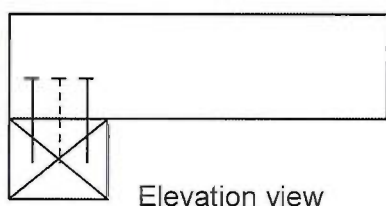
NOTES:

1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to wind or earthquake load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for one toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-09, section 10.9.5.2.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities: G = 0.42(SPF), G = 0.49(D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37579H1).
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-09, section 10.9.5
9. This design is not valid after April 30, 2019

Toe-nailing on 2x6 Bearing Plate



Toe-nailing on 2x4 Bearing Plate



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April 26, 2017