

F-GREEN YORK HOMES-GRANELLI HOME CORP-LOT-15

Ground Floor LVL/LSL Pcs Length 2 10-0-0 Label Description Width Depth Plies Qty F12 1.75 2.0E-3000Fb LVL Layout Name F10 Forex 1.75 11.875 8-0-0 2 2.0E-3000Fb LVL LOT 15 (AMELIA 12 EL- 2) 1.75 11.875 8-0-0 Design Method 2.0E-3000Fb LVL LSD 1.75 11.875 F8 6-0-0 Forex 2.0E-3000Fb LVL Description Joist **GREEN YORK HOMES GRANELLI HOMES PROJECT** Label Description Width Depth Qty Plies Pcs |Length BRAMPTON,ON 2.5 11.875 F15 LPI 20Plus 2 18-0-0 Created F20 LPI 20Plus 2.5 11.875 1 12-0-0 F19 LPI 20Plus May 31, 2018 2.5 11.875 1 10-0-0 2.5 11.875 F13 LPI 20Plus 4-0-0 2 Builder J1 LPI 20Plus 2.5 11.875 14 12-0-0 Sales Rep 2.5 11.875 J10 LPI 20Plus 12 10-0-0 J7 LPI 20Plus 2.5 11.875 5 6-0-0 Designer J5 NJ60H 2.5 11.875 5 20-0-0 J4 NJ60H 2.5 11.875 22 18-0-0 Shipping 3 16-0-0 J3 NJ60H 2.5 11.875 J2 NJ60H 2.5 11.875 1 10-0-0 Builder's Project Rim Board Pcs Length Label Description Width Depth Qty Plies **Kott Lumber Company** Norbord Rimboard 1.125 11.875 14 Anderson Blvd Plus 1.125 X Stouffville, Ontario 11.875 Canada Hanger L4A 7X4 Beam/Girder Supported 905-642-4400 Member Label Pcs Description Skew Slope fasteners fasteners **Ground Floor** H1 2 HUS1.81/10 30 16d Design Method LSD H2 1 HUS1.81/10 Building Code NBCC 2010 / OBC НЗ 31 LF2511 12 10dx1 1/2 1 #8x1 1/4WS Blocking Floor Plies Pcs Length Label Width Depth Description Qty Loads BLK2 LPI 20 Plus 2.5 11.875 LinFt Varies 17-0-0 Live BLK1 NJ60H 2.5 11.875 LinFt Varies 11-0-0 15 **Deflection Joist** 480 LL Span L/ TL Span L/ 360 Framer to verify dimensions on the architectural drawings. LL Cant 2L/ 480 Double joist only require filler/backer ply when supporting another member using a face-mounted hanger. TL Cant 2L/ 360 Install 2x4 blocking @ 24" o/c under parallel non-loadbearing walls. **Deflection Girder** Install single-ply flush window header along inside face of rimboard/rimjoist LL Span L/ 360 Refer to Nascor specifier guide for installation details. TL Span L/ 240 Squash blocks recommended to be installed at end bearing on all first level joists which support loading from above exceeding two levels floor or roof. LL Cant 2L/ 480 Load transfer blocks to be installed under all point loads, TL Cant 2L/ 360 It shall be the framer's responsibility that floor joists and beams Decking are fastened as per the hanger manufacturer's standards. Deck OSB Refer to Multiple Member Connection Detail to ply to ply nailing or bolting Thickness 3/4" Fastener Nailed & Glued

Rim parallel to joists: 1-1/8" rimboard with 2"x4" block (1/16" longer than rim depth) @ 16" o/c. All other components and structural elements supporting the floor system such as beams, walls, columns and foundation walls and footings including anchorage of components and bracing for lateral stability are the responsibility of others.

Hatch area represents ceramic tiled floor with an additional dead load

The framing shown on this layout may be deviate from the architectural drawings. Project Engineer to review and approve the deviation prior

M-2057 LOT 15



Point Load Support Load from Above Norbord Rimboard Plus 1.125 X 11.875 LPI 20Plus 11.875 NJ60H 11.875

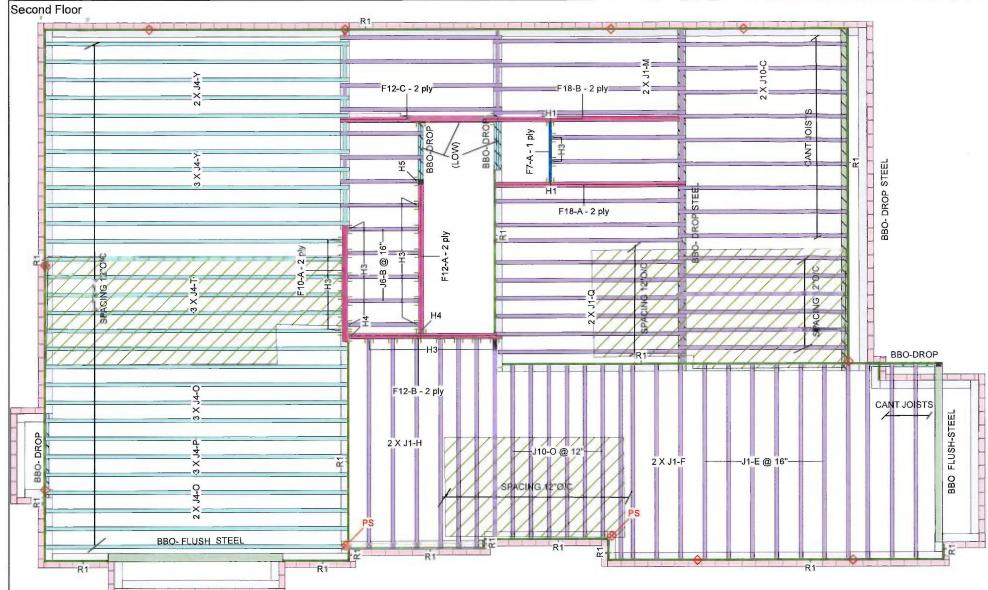
Forex 2.0E-3000Fb LVL 1.75 X 11.875 1.75 X 7.5 (Dropped) 5,25 X 10,25 (Dropped)

Engineered floor joists shall be installed accordance with the supplier's layout and

Vibration

19-447 155 - RR.

Ground Floor



Architectural Drawing Info

JARDIN DESIGN GROUP 64 JARDIN DR, SUITE 3A VAUGHAN, ON L4K 3P3 Project # 17-55 Model: LOT-15,AMELIA 12 Date: DEC 21,2018

JOISTS SPACING 16"O/C UNLESS NOTED OTHERWISE

- 1. OBC 2012 O.Reg 332/12 as amended
- 2. Nascor CCMC 13535-R
- 3. LVL CCMC -12904-R
- 4. CAN/CSA-O86-09
- 5. CCMC -12787-R APA PR-L310(C)

This certification is to confirm that:

- 1. The loads used in the calculation of the attached approved components conform to the floor assembly shown on this layout.
- 2. The floor joists comply with the KOTT span table for the loads and spacing shown on this layout. The floor system must be assembled in accordance to the KOTT Specifier Guide. Multi-ply members must be attached together as per the included multiple member connection detail. All other components and structural elements supporting the floor system such as beams, walls, columns and foundation walls and footings including anchorage of components and bracing for

lateral stability are the responsibility of others.





										IM0119-019
Second										
LVL/LS	_		) A # 111			01	Dr.	- D	1 0.	NASCOR
	Descr	iption	Width		pth	Qty	Plies	Pcs	Length	IMASCUR
F18	Forex	000Fb LVL	1.75	11.8	3/5	2	2	4	12-0-0	
F12	Forex	000Fb LVL	1.75	11.8	875	3	2	6	10-0-0	Layout Name LOT 15 (AMELIA 12 EL- 2)
F10	Forex 2.0E-3	000Fb LVL	1.75	11.8	875	1	2	2	8-0-0	Design Method LSD
F7	Forex 2.0E-3	000Fb LVL	1.75	11.8	875			1	4-0-0	Description
I Joist										GREEN YORK HOMES
Label	Descr		Width	-	pth	Qty	Plies	Pcs	Length	GRANELLI HOMES PROJECT BRAMPTON,ON
J1	LPI 20		2.5	-	B75			34	12-0-0	Created
J10	LPI 20		2.5	_	875			25	10-0-0	
J8	LPI 20		2.5		875			2	8-0-0	May 31, 2018
J7	LPI 20		2.5	-	875			4	6-0-0	Builder
J6	LPI 20		2.5	_	875			5 29	4-0-0 18-0-0	Sales Rep
J4	NJ60H		2.5	11.	875			29	18-0-0	Designer
Rim Bo	Descr	intion	Width	Do	pth	Qty	Plies	Pcs	Length	SB
R1		d Rimboard	1.125	_	875	Giy	1 1105	16	12	Shipping
'``	Plus 1.		1.120		0,0			,,,		11 0
	11.875									Project
Hange	r									Builder's Project
						В	eam/Girde		pported	Kott Lumber Company
	В.	In	- 10	L	01-		f t		ember	14 Anderson Blvd
Label	Pcs	Descriptio		kew	Slo	pe i	fasteners		steners	Stouffville, Ontario
H1 H3	2 26	HUS1.81/1 LF2511	0				30 16d 12 10d	_	0 16d x1 1/4WS	Canada
H4	20	HGUS410				_	46 16d		6 16d	L4A 7X4
H5	1	HUC312				_	16 16d		0dx1 1/2	905-642-4400
H6	2	Unknown			-	_	10 100	-	OUX! IIL	Second Floor
	_	Hanger								Design Method LSD
NOTES										Building Code NBCC 2010 / OBC 2012
		erify dimension							- 1	Floor
		only require a a face-mo			wne	n suppo	orting anothi	er		Loads
		locking @ 24			rallel	non-loa	dbearing wa	alls.	- 1	Live 40
4. Insta	all single	-ply flush wir	ndow hea	der a	long i	nside fa	ce of rimbo	ard/rimjo	ist	Dead 15
		cor specifier								Deflection Joist
		ks recomme								LL Span L/ 480
		r blocks to b							·	TL Span L/ 360
	It shall be the framer's responsibility that floor joists and beams							- 1	LL Cant 2L/ 480	
are f	astened	as per the h	anger ma	nufac	turer'	s standa	ards.			TL Cant 2L/ 360
Refer to	Multiple	Member Co	nnection	Deta	il to n	lv to nlv	nailing or b	oltina		Deflection Girder
requiren						, ۲۰,		9		LL Span L/ 360
				-,,						TL Span L/ 240
		oists: 1-1/8"   6" longer tha			16"	alc				LL Cant 2L/ 480
		nents and st					1			TL Cant 2L/ 360
		euch as has					•		- 1	Jan 20

the floor system such as beams, walls, columns and foundation walls and footings including anchorage of components and bracing for lateral stability are the responsibility of others.

Hatch area represents ceramic tiled floor with an additional dead load of 5 PSF.

The framing shown on this layout may be deviate from the architectural drawings. Project Engineer to review and approve the deviation prior to construction

Legend Point Load Support Load from Above Wall Norbord Rimboard Plus 1.125 X 11.875 LPI 20Plus 11.875 NJ60H 11.875 Forex 2.0E-3000Fb LVL 1.75 X 11.875 1.5 X 7.5 (Dropped) 7///// 1.5 X 9.5 (Dropped) 1///// 1.75 X 9.5 (Dropped) 11/1// 5 X 10.25 (Dropped) 5 X 10.25 5.25 X 10.25

M-2057 LOT 15

Decking

Thickness

Fastener

Vibration

OSB

5/8"

Nailed & Glued

Gypsum 1/2"

Deck



# **Engineering Note Page (ENP-2)**

REVISION 2018-10-17

M-2057 LOT 15

# Please read all notes prior to installation of the component

# **DESIGN INFORMATION**

This building component is certified as an individual component for the loads and conditions shown on the calculation and drawing page.

The responsibility of the undersigned engineer is <u>only</u> limited to the calculation of this building component for the loads and conditions shown on this drawing.

The responsibility of the undersigned is limited to the verification of the structural capacity of the floor joists and LVL beams based on placement as shown on the layout. The loads applied are limited to the gravity effects of the specified loads. The structural integrity of the building and the effect of wind, uplift, seismic, lateral or other forces, calculation of adequate support and anchorage of components, as well as the dimensions and design loads used to calculate components are the responsibility of the overall building designer.

Floor joists and OSB rim board are designed to carry uniformly distributed loads only. Point loads should be transferred through the floor cavity with transfer blocks. Structural elements such as walls, posts, connectors, and transfer blocks are the responsibility of the overall building designer.

The undersigned engineer disclaims any responsibility for damages as a result of being furnished faulty or incorrect information, specifications and/or designs.

Installation of floor joists is to be carried out in accordance with the current edition of the manufacturer's literature available at <a href="http://www.kottgroup.com">http://www.kottgroup.com</a>.

# CODE

This building component is designed in accordance with the National Building Code of Canada, the Ontario Building Code, CCMC and Canadian Standards Association guidelines.

### COMPONENT

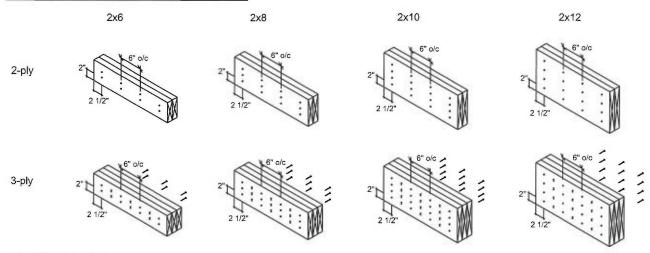
- 1. The building component used in construction must be the same as indicated on the drawings.
- 2. The building component must be installed and assembled as per specification shown on the drawing and in accordance with the manufacturer's assembly and installation.
- 3. Members consisting of multiple plies must be connected as per the document "Multi-ply Connection Details".
- 4. Pass-thru transfer block framing is required at all point loads over bearings.

# HANDLING AND INSTALLATION

Do not drill any hole, cut or notch a certified building component without a written preauthorization.



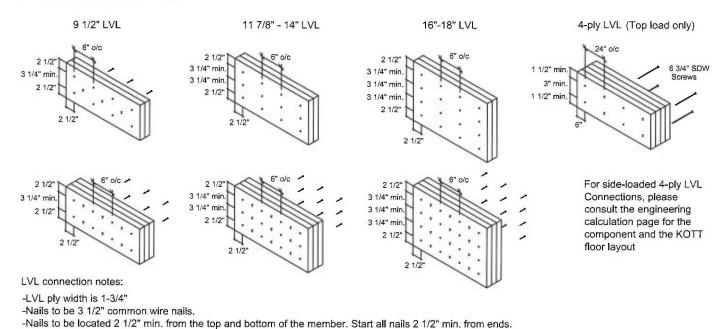
# **Conventional Connections**



Conventional connection notes:

- -Nails to be 3" long wire nails.
- -Nails to be located 2" min. from the top and bottom of the member. Start all nails 2 1/2" min. from ends.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail driven from the opposite side.

# LVL Connections



# Multiple Member Connections

-Minimum 3 1/4" spacing between rows.

All connections are for uniformly distributed loads.

For multi-ply connections of I-joists, refer to Manufacturer's Installation Guide

- "X" represents nail or screw driven from the opposite side.

-Number of rows and spacing as per details shown, unless noted otherwise.



KOTT Inc. 3228 Moodie Drive Ottawa, ON K2H 7V1 613-838-2775



Client Project:

Address:

Date: 1/3/2019

Designer: SB

Job Name: LOT 15 (AMELIA 12 EL- 2)

Project #:

Forex 2.0E-3000Fb LVL

1.750" X 11.875"

Floor (Residential)

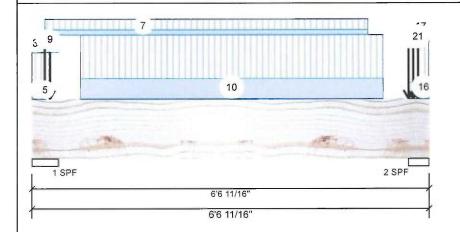
NBCC 2010 / OBC 2012

LSD

No Not Checked

Not Checked

2-Ply - PASSED Level: Ground Floor

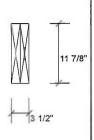


Application:

Vibration:

Design Method:

**Building Code:** Load Sharing:



### Member Information

Туре:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	
Floor Live:	40 PSF
Dead:	15 PSF

# Unfactored Reactions UNPATTERNED Ib (Unlift)

Umacto	red Reactions	DIVITALILIE	ALD ID (Ohin	•)	
Brg	Live	Dead	Snow	Wind	
1	3723	1780	0	0	
2	2558	1233	0	0	
1					
4					

# **Bearings and Factored Reactions**

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	5.250"	69%	2225 / 5585	7809	L	1.25D+1.5L
2 - SPF	4.063"	61%	1542 / 3836	5378	L	1.25D+1.5L

**Analysis Results** 

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	3714 ft-lb	3'3 3/4"	34261 ft-lb	0.108 (11%)	1.25D+1.5L	L
Unbraced	3714 ft-lb	3'3 3/4"	32638 ft-lb	0.114 (11%)	1.25D+1.5L	L
Shear	2285 lb	5'3 1/2"	11596 lb	0.197 (20%)	1.25D+1.5L	L
Perm Defl in.	0.008 (L/8835)	3'3 7/8"	0.197 (L/360)	0.040 (4%)	D	Uniform
LL Defl inch	0.016 (L/4425)	3'3 7/8"	0.197 (L/360)	0.080 (8%)	L	L
TL Defl inch	0.024 (L/2948)	3'3 7/8"	0.295 (L/240)	0.080 (8%)	D+L	L

### Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.

7 Lateral slenderness ratio based on full section width.

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



January 04, 2019

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
2	Part. Uniform	0-0-0 to 0-2-9		Тор	107 PLF	248 PLF	0 PLF	0 PLF	J4
3	Part. Uniform	0-0-0 to 0-2-9		Тор	80 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
4	Point	0-2-9		Тор	1073 lb	2359 lb	0 Pass	-Thru Pr	aming Squasif Block is
5	Part. Uniform	0-2-9 to 0-2-9		Тор	107 PLF	248 PLF			I point loads over bearings
6	Part. Uniform	0-2-9 to 0-5-4		Тор	80 PLF	0 PLF	0 PLF Refe	o PLF	Wall Self Weight

Continued on page 2...

Detail for ply to ply nailing or bolting requirements Manufacturer Info

Forex APA: PR-L318

Kott Lumber Company 14 Anderson Blvd, Ontario Canada L4A 7X4 905-642-4400

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

### Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

### Handling & Installation

- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code

- paprovals

  Damaged Beams must not be used

  Design assumes top edge is laterally restrained

  Provide lateral support at bearing points to avoid

  lateral displacement and rotation



This design is valid until 10/18/2021

6. For flat roofs provide proper drainage to prevent

Page 2 of 2



Client:

Project: Address:

1/3/2019 Date:

Designer: SB

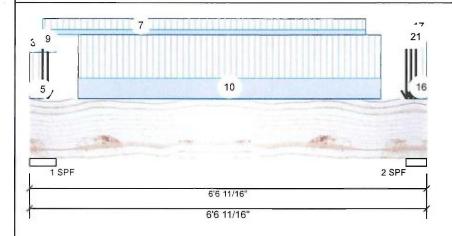
Job Name: LOT 15 (AMELIA 12 EL- 2)

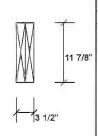
Project #:

Forex 2.0E-3000Fb LVL F10-B

1.750" X 11.875"

2-Ply - PASSED Level: Ground Floor





Continued fro	om page 1				Toront 1				
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
7	Part. Uniform	0-2-9 to 5-6-9		Near Face	39 PLF	81 PLF	0 PLF	0 PLF	
9	Point	0-3-9		Far Face	101 lb	229 lb	0 lb	0 lb	J4
10	Part. Uniform	0-9-9 to 5-9-9		Far Face	157 PLF	330 PLF	0 PLF	0 PLF	
16	Part. Uniform	6-2-10 to 6-6-11		Тор	126 PLF	335 PLF	0 PLF	0 PLF	J4
17	Part. Uniform	6-2-10 to 6-6-11		Тор	80 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
18	Point	6-2-10		Тор	1 lb	3 lb	0 lb	0 lb	
19	Point	6-2-10		Near Face	41 lb	83 lb	0 lb	0 lb	F8
20	Point	6-3-9		Far Face	96 lb	203 lb	0 lb	0 lb	J4
21	Point	6-4-8		Тор	518 lb	1155 lb	0 lb	0 lb	F10 F10
	Self Weight				10 PLF				



Pass-Thru Framing Squash Block is required at all point loads over bearings

**Refer to Multiple Member Connection** Detail for ply to ply nailing or bolting requirements

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

Handling & Installation

1. IVL beams must not be cut or drilled

2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

3. Damaged Beams must not be used

4. Design assumes top edge is laterally restrained

5. Provide lateral support at bearing points to avoid lateral displacement and rotation

5. For flat roofs provide proper drainage to prevent

READ ALL NOTES ON THIS PAGE AND ON THE

IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE

Manufacturer Info

Forex APA: PR-L318









Client: Project:

Address:

Date: 1/3/2019

Designer: SB

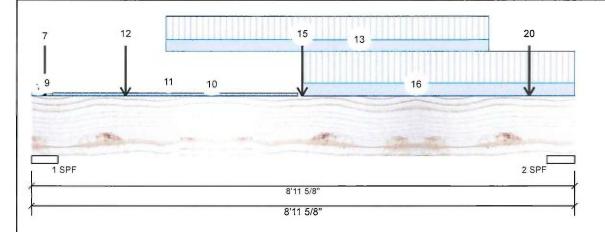
Job Name: LOT 15 (AMELIA 12 EL- 2)

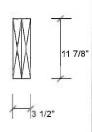
Project #:

Forex 2.0E-3000Fb LVL

1.750" X 11.875"

2-Ply - PASSED Level: Ground Floor





### Member Information

Type:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	

40 PSF

15 PSF

Application: Design Method: Building Code:

Floor (Residential) LSD

NBCC 2010 / OBC 2012 No

Load Sharing: Deck:

Not Checked Vibration: Not Checked

# Unfactored Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	Wind
1	1330	706	0	0
2	1958	931	0	0

### **Bearings and Factored Reactions**

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	5.250"	25%	883 / 1995	2878	L	1.25D+1.5L
2 - SPF	5.500"	35%	1164 / 2937	4101	L	1.25D+1.5L

## **Analysis Results**

Floor Live:

Dead:

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	7293 ft-lb	4'8 7/16"	34261 ft-lb	0.213 (21%)	1.25D+1.5L	L
Unbraced	7293 ft-lb	4'8 7/16"	31134 ft-lb	0.234 (23%)	1.25D+1.5L	L
Shear	3333 lb	7'7"	11596 lb	0.287 (29%)	1.25D+1.5L	L
Perm Defl in.	0.025 (L/3941)	4'6 7/8"	0.273 (L/360)	0.090 (9%)	D	Uniform
LL Defl inch	0.051 (L/1944)	4'7 5/16"	0.273 (L/360)	0.190 (19%)	L	L
TL Defl inch	0.076 (L/1302)	4'7 3/16"	0.410 (L/240)	0.180 (18%)	D+L	L

### Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3), Assumed point load size: beam width X 4.5.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.

7 Lateral slenderness ratio based on full section width.

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 0-4-2	(Span)0-7-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 0-4-2	(Span)0-4-12	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	0-2-10		Тор	3 lb	7 lb	o Ib Pa	ss-Thru F	Framing Squash Block is all point loads over bearings
4	Point	0-2-10		Тор	2 lb	dl 0	0 lb	0 lb	Wall Self Weight

Top

46 lb

Continued on page 2...

requirements Manufacturer Info

Kott Lumber Company 14 Anderson Blvd, Ontario Canada L4A 7X4 905-642-4400

olb Refer to Multiple Member Connection

Detail for ply to ply nailing or bolting

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design cnteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Point

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

LVL beams must not be cut or drilled
Refer to manufacturer's product information
regarding installation requirements, multi-ply
fastening details, beam strength values, and code

0-2-10

paprovals

Damaged Beams must not be used

Design assumes top edge is laterally restrained

Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent

113 lb







Designer: SB

Page 2 of 2

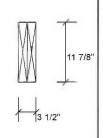
IM0119-019

Job Name: LOT 15 (AMELIA 12 EL- 2) Project #:

1.750" X 11.875" 2-Ply - PASSED Level: Ground Floor Forex 2.0E-3000Fb LVL F12-D

Address:

20 12 15 13 11 9 10 16 1 SPF 2 SPF 8'11 5/8' 8'11 5/8'



Continued	from page 1								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
6	Point	0-2-10		Тор	12 lb	28 lb	0 lb	0 lb	J4
7	Point	0-2-10		Тор	36 lb	0 lb	0 lb	0 lb	Wall Self Weight
8	Part, Uniform	0-2-12 to 0-4-2		Тор	2 PLF	0 PLF	0 PLF	0 PLF	
9	Part, Uniform	0-2-12 to 0-4-2		Тор	1 PLF	0 PLF	0 PLF	0 PLF	
10	Tie-In	0-4-2 to 4-4-13	(Span)0-10-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
11	Part. Uniform	0-4-2 to 4-4-13		Тор	2 PLF	0 PLF	0 PLF	0 PLF	
12	Point	1-6-10		Near Face	124 lb	250 lb	0 lb	0 lb	J1
13	Part. Uniform	2-2-10 to 7-6-10		Near Face	98 PLF	201 PLF	0 PLF	0 PLF	
15	Point	4-5-11		Far Face	154 lb	281 lb	0 lb	0 lb	F9
16	Part. Uniform	4-5-13 to 8-11-10		Тор	110 PLF	270 PLF	0 PLF	0 PLF	
20	Point	8-2-10		Near Face	122 lb	250 lb	0 lb	0 lb	J10
	Self Weight				10 PLF				



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

### Lumber

- Dry service conditions, unless noted otherwise
   LVL not to be treated with fire retardant or corrosive
- chemicals
- Handling & Installation
- andling & Installation
  LVL beams must not be cut or drilled
  Refer to manufacturer's product information
  regarding installation requirements, multi-ply
  fastening details, beam strength values, and code
  approvals
  Damaged Beams must not be used
  Design assumes top adge is laterally restrained
  Provide lateral support at bearing points to avoid
  lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info







Client: Project: Address: Date: Design

Designer: SB

Job Name: LOT 15 (AMELIA 12 EL- 2)

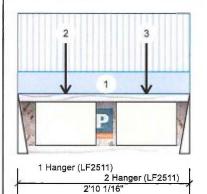
Project #:

F13-A LPI 20Plus

11.875" - PASSED

Level: Ground Floor

1/3/2019



2'10 1/16"

11 7/8"

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			AIII Q	

I	Туре:	Girder	Application:	Floor (Residential)
ı	Plies:	1	Design Method:	LSD
I	Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
١	Deflection LL:	360	Load Sharing:	No
I	Deflection TL:	240	Deck:	Not Checked
I	Importance:	Normal	Vibration:	Not Checked
I	General Load			
I	Floor Live:	40 PSF		
I	Dead:	15 PSF		
1				

# Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind	
1	236	89	0	0	
2	244	92	0	0	

## Analysis Results

	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
l	Moment	307 ft-lb	1'1 11/16"	6250 ft-lb	0.049 (5%)	1.25D+1.5L	L
ı	Shear	475 lb	2'8 13/16"	2345 lb	0.202 (20%)	1.25D+1.5L	L
	Perm Defl in.	0.001 (L/24100)	1'3 3/4"	0.088 (L/360)	0.010 (1%)	D	Uniform
ı	LL Defl inch	0.003 (L/9050)	1'3 3/4"	0.088 (L/360)	0.040 (4%)	L	L
	TL Defl inch	0.005 (L/6580)	1'3 3/4"	0.132 (L/240)	0.040 (4%)	D+L	L

# Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.	
1 - Hanger	2.000"	29%	111 / 355	466	L	1.25D+1.5L	
2 - Hanger	2.000"	30%	115 / 366	481	L	1.25D+1.5L	

# **Design Notes**

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.001", Long Term = 0.002"
- 3 Fill all hanger nailing holes.
- 4 See manufacture installation guide note E4 for installation details
- 5 Girders are designed to be supported on the bottom edge only.
- 6 Top flange braced at bearings.

7 Bottom flange braced at bearings

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January 04, 2019

I	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
	1	Tie-In	0-0-0 to 2-10-1	(Span)1-4-9	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	2	Point	0-9-8		Far Face	77 lb	205 lb	0 lb	0 lb	J10
	3	Point	2-1-8		Far Face	74 lb	197 lb	0 lb	0 lb	J10
ı								Day	- The C	in- Carre

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT. Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

### Notes

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

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This design is valid until

### Manufacturer Info

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Client: Project: Address:

Date: 1/3/2019

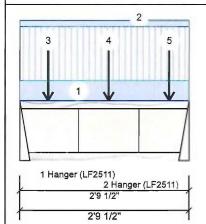
Designer: SB Job Name: LOT 15 (AMELIA 12 EL- 2)

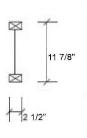
Project #:

LPI 20Plus

11.875" - PASSED

Level: Ground Floor





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Туре:	Girder	Application:	Floor (Residential)
Plies:	1	Design Method:	LSD
Moisture Condition	: Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

### Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind	
1	404	192	0	0	
2	430	205	0	0	
1					

## **Analysis Results**

Γ	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
l	Moment	595 ft-lb	1'5 5/8"	6250 ft-lb	0.095 (10%)	1.25D+1.5L	L
l	Shear	896 lb	2'8 1/4"	2345 lb	0.382 (38%)	1.25D+1.5L	L
	Perm Defl in.	0.003 (L/10568)	1'5 5/8"	0.086 (L/360)	0.030 (3%)	D	Uniform
l	LL Defl inch	0.006 (L/5035)	1'5 5/8"	0.086 (L/360)	0.070 (7%)	L	L
	TL Defl inch	0.009 (L/3410)	1'5 5/8"	0.129 (L/240)	0.070 (7%)	D+L	L

# **Bearings and Factored Reactions**

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.	_
1 - Hanger	2.000"	53%	240 / 606	846	L	1.25D+1.5L	
2 - Hanger	2.000"	57%	256 / 645	901	L	1.25D+1.5L	

### **Design Notes**

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.003", Long Term = 0.004"
- 3 Fill all hanger nailing holes.
- 4 See manufacture installation guide note E4 for installation details
- 5 Girders are designed to be supported on the bottom edge only.
- 6 Top flange braced at bearings.
- 7 Bottom flange braced at bearings

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January 04, 2019

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 2-9-8	(Span)1-3-7	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Part. Uniform	0-0-0 to 2-9-8		Тор	3 PLF	0 PLF	0 PLF	0 PLF	
3	Point	0-5-10		Near Face	113 lb	239 lb	0 lb	0 lb	J3
4	Point	1-5-10		Near Face	145 lb	305 lb	0 lb Pa	ss-Thru F Juired at	Framing Squash Block is all point loads over bearings
5	Point	2-5-10		Near Face	104 lb	218 lb	0 lb	0 lb	J3
							Da	Com do Biller	Hinle Mambay Connection

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed. product listed

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Wind

0

0

Ld. Comb. 1.25D+1.5L 1.25D+1.5L

Page 1 of 1



Client: Project: Address: Date: 1/3/2019

Designer: S B

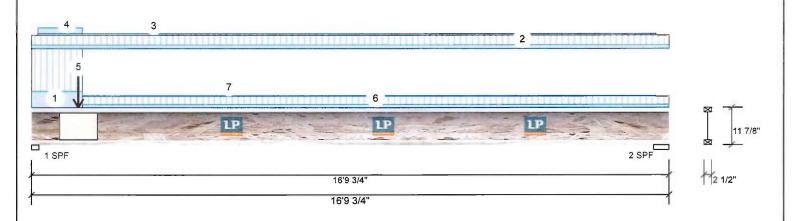
Job Name: LOT 15 (AMELIA 12 EL- 2)

Project #:

LPI 20Plus

11.875" - PASSED

Level: Ground Floor



Member Infor	mation			Unfactore	d Reaction	ns Ul	NPATTERNI	ED lb (	<b>Uplift</b> )
Туре:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snov	W
Plies:	1	Design Method:	LSD	1	647		313		0
Moisture Condition	n: Dry	Building Code:	NBCC 2010 / OBC 2012	2	242		118		0
Deflection LL:	360	Load Sharing:	No						
Deflection TL:	240	Deck:	Not Checked						
Importance:	Normal	Vibration:	Not Checked						
General Load								777	
Floor Live:	40 PSF			Bearings a	and Factor	ed R	eactions		
Dead:	15 PSF			Bearing L	ength (	Cap.	React D/L lb	Total	Ld. Case
				1-SPF 2	.375"	83%	391 / 971	1362	L
_				2-SPF 4	.875"	28%	148 / 363	511	L

Analysis Result	Αn	alv	sis	Resu	Its
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Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2308 ft-lb	7'1 5/8"	6250 ft-lb	0.369 (37%)	1.25D+1.5L	L
Shear	1342 lb	1 5/8"	2345 lb	0.572 (57%)	1.25D+1.5L	L
Perm Defl in.	0.093 (L/2110)	7'11 3/8"	0.544 (L/360)	0.170 (17%)	D	Uniform
LL Defl inch	0.189 (L/1035)	7'11 5/16"	0.544 (L/360)	0,350 (35%)	L	L
TL Defl inch	0.282 (L/695)	7'11 5/16"	0.817 (L/240)	0.350 (35%)	D+L	L

# **Design Notes**

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.093", Long Term = 0.139"
- 3 See manufacture installation guide note E4 for installation details
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top flange must be laterally braced at a maximum of 6'10" o.c.

6 Bottom flange braced at bearings.



January 04, 2019

ID	Load Type Lo	ocation Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In 0-0-0 f	to 1-4-2 (Span)3-0-	О Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In 0-0-0 to	16-9-12 (Span)0-8-	2 Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Part. Uniform 0-1-14 to	16-5-10	Тор	2 PLF	0 PLF	0 PLF	0 PLF	
4	Part. Uniform 0-2-1	to 1-4-2	Тор	8 PLF	0 PLF	0 PLF	0 PLF	
5	Point	1-2-14	Far Face	192 lb	404 lb	0 lb Pas	ss-Thrub F	raming Squash Block is
6	Tie-In 1-4-2 to	16-9-12 (Span)0-6-	14 Top	15 PSF	40 PSF	0 PSF req	uired at a	all point loads over bearings
7	Part. Uniform 1-4-2 to	16-5-9	Тор	1 PLF	0 PLF			tiple Member Connection y to ply nailing or bolting

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

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This design is valid until 10/31/2020

### Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325 www.lpcorp.com CCMC: 12412-R APA: PR-L238C

requirements





IM0119-019

Wind

n

0

Ld. Comb.

1.25D+1.5L

1.25D+1.5L

Page 1 of 1

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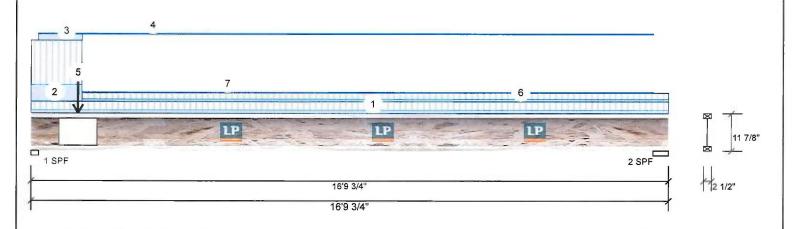
Project: Address: Designer: SB

Job Name: LOT 15 (AMELIA 12 EL- 2)

Project #:

F15-B LPI 20Plus 11.875" - PASSED

Level: Ground Floor



Member Inforn	nation			Unfactor	ed React	ions Ul	NPATTERN	ED lb (	(Uplift)
Туре:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snov	N
Plies:	1	Design Method:	LSD	1	634		302		0
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012	2	202		95		0
Deflection LL:	360	Load Sharing:	No	1 -					
Deflection TL:	240	Deck:	Not Checked						
Importance:	Normal	Vibration:	Not Checked						
General Load									
Floor Live:	40 PSF			Bearings	and Fact	ored R	eactions		
Dead:	15 PSF			Bearing	Length	Cap.	React D/L lb	Total	Ld. Case
				1 - SPF :	2.375"	81%	378 / 951	1329	L
				2 - SPF -	4.875"	23%	119 / 302	421	L

Ana	lysis	Resu	lts
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Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1986 ft-lb	6'8 3/4"	6250 ft-lb	0.318 (32%)	1.25D+1.5L	L
Shear	1310 lb	1 5/8"	2345 lb	0.559 (56%)	1.25D+1.5L	L
Perm Defl in.	0.078 (L/2505)	7'10 1/4"	0.544 (L/360)	0.140 (14%)	D	Uniform
LL Defl inch	0.164 (L/1193)	7'10 1/4"	0.544 (L/360)	0.300 (30%)	L	L
TL Defl inch	0.243 (L/808)	7'10 1/4"	0.817 (L/240)	0.300 (30%)	D+L	L

# **Design Notes**

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.078", Long Term = 0.117"
- 3 See manufacture installation guide note E4 for installation details
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top flange must be laterally braced at a maximum of 7'4" o.c.

6 Bottom flange braced at bearings.

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January 04, 2019

	ID	Load Type	Location	Trib Width	Side	Dead	Live
	1	Tie-In	0-0-0 to 16-9-12	(Span)0-6-14	Тор	15 PSF	40 PSF
	2	Tie-In	0-0-0 to 1-4-2	(Span)3-0-0	Тор	15 PSF	40 PSF
	3	Part, Uniform	0-2-4 to 1-4-2		Тор	8 PLF	0 PLF
	4	Part. Uniform	0-2-4 to 16-5-0		Тор	1 PLF	0 PLF
	5	Point	1-2-14		Near Face	205 lb	430 lb
	6	Tie-In	1-4-2 to 16-9-12	(Span)0-5-2	Тор	15 PSF	40 PSF
I	7	Part. Uniform	1-4-2 to 16-5-1		Тор	1 PLF	0 PLF
1							

0 lb Pass-Thrub Framing Squash Block is 0 PSF required at all point loads over bearings

Comments

<sup>0</sup> PLF Refer <del>to Multiple Member Connection</del> Detail for ply to ply nailing or bolting requirements

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

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IN THE DESIGN OF THIS COMPONENT.

READ ALL NOTES ON THIS PAGE AND ON THE

ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT

CONTAINS SPECIFICATIONS AND CRITERIA USED

This design is valid until 10/31/2020

### Manufacturer Info

Snow

0 PSF

0 PSF 0 PLF

0 PLF

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www.lpcorp.com CCMC: 12412-R APA: PR-L238C

Wind

0 PSF

0 PSF

0 PLF

0 PLF





Address:

F-GREEN YORK HOMES-GRANELLI HOME CORP-LOT-15 1/3/2019

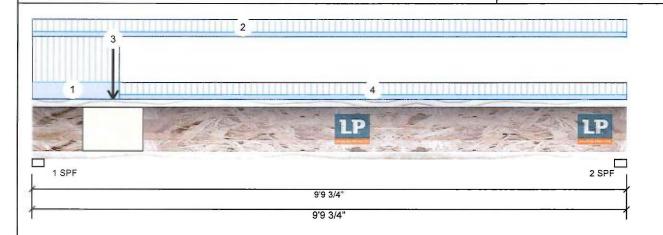
Designer: SB

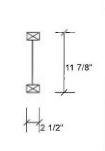
Job Name: LOT 15 (AMELIA 12 EL- 2)

Project #:

11.875" - PASSED LPI 20Plus

Level: Ground Floor





IM0119-019

Page 1 of 1

Type:	Girder	Application:	Floor (Residential)
Plies:	1	Design Method:	LSD
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load		100	
Floor Live:	40 PSF		
Dead:	15 PSF		

Live		Dead	Snov	w	Wind	Т
437		164		0	0	
198		74		0	0	
	الممسمة	Onnations.				
is and Fac	torea i	Reactions				
Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.	
2.375"	53%	205 / 656	861	L	1.25D+1.5L	
	437 198 <b>3s and Fac</b> 3 Length	Length Cap.	437 164 198 74  gs and Factored Reactions Length Cap. React D/L lb	437 164 198 74  gs and Factored Reactions 19 Length Cap. React D/L lb Total	437 164 0 198 74 0  gs and Factored Reactions g Length Cap. React D/L lb Total Ld. Case	437 164 0 0 198 74 0 0  gs and Factored Reactions 19 Length Cap. React D/L lb Total Ld. Case Ld. Comb.

**Unfactored Reactions UNPATTERNED lb (Uplift)** 

### **Analysis Results**

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1108 ft-lb	3'10 7/16"	6250 ft-lb	0.177 (18%)	1.25D+1.5L	L
Shear	840 lb	1 5/8"	2345 lb	0.358 (36%)	1.25D+1.5L	L
Perm Defl in.	0.015 (L/7673)	4'6 1/2"	0.318 (L/360)	0.050 (5%)	D	Uniform
LL Defl inch	0.040 (L/2882)	4'6 9/16"	0.318 (L/360)	0.120 (12%)	L	L
TL Defl inch	0.055 (L/2095)	4'6 9/16"	0.477 (L/240)	0.110 (11%)	D+L	L

# **Design Notes**

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.015", Long Term = 0.022"
- 3 See manufacture installation guide note E4 for installation details
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top flange must be laterally braced at a maximum of 9'3" o.c.

6 Bottom flange braced at bearings.

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January 04, 2019

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-5-4	(Span)3-0-9	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 9-9-12	(Span)0-10-3	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	1-4-0		Far Face	92 lb	244 lb	0 lb	0 lb	F13
4	Tie-In	1-5-4 to 9-9-12	(Span)0-9-13	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2, THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Pass-Thru Framing Squash Block is required at all point loads over bearings

**Refer to Multiple Member Connection** Detail for ply to ply nailing or bolting requirements

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

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Client:

Project:

F-GREEN YORK HOMES-GRANELLI HOME CORP-LOT-15 Date: 1/3/2019

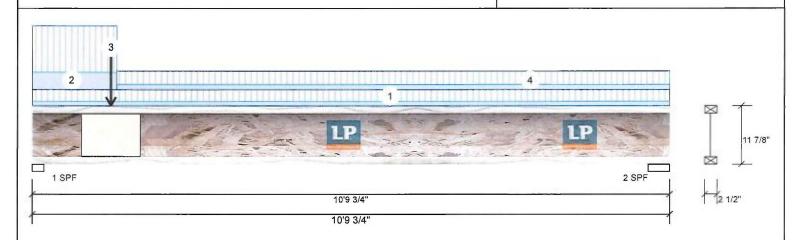
Designer: SB

Job Name: LOT 15 (AMELIA 12 EL- 2)

Project #:

11.875" - PASSED LPI 20Plus

Level: Ground Floor



Member Inforn	nation			Unfacto	red React	ions L	INPATTERN	ED lb (	(Uplift)	
Туре:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snov	W	)
Plies:	1	Design Method:	LSD	1	445		167		0	
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012	2	214		80		0	
Deflection LL:	360	Load Sharing:	No							
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal	Vibration:	Not Checked							
General Load										_
Floor Live:	40 PSF			Bearing:	s and Fact	ored	Reactions			
Dead:	15 PSF			Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	
				1 - SPF	2.375"	54%	209 / 667	876	L	
				2 - SPF	4.375"	23%	100 / 321	421	L	

Analysis Result	Results	:	vsis	alv	n	F
-----------------	---------	---	------	-----	---	---

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1226 ft-lb	4'4 13/16"	6250 ft-lb	0.196 (20%)	1.25D+1.5L	L
Shear	855 lb	1 5/8"	2345 lb	0.365 (36%)	1.25D+1.5L	Ļ
Perm Defl in.	0.019 (L/6615)	5'	0.346 (L/360)	0.050 (5%)	D	Uniform
LL Defl inch	0.050 (L/2484)	5'	0.346 (L/360)	0.140 (14%)	L	L
TL Defl inch	0.069 (L/1806)	5'	0.519 (L/240)	0.130 (13%)	D+L	L

## **Design Notes**

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.019", Long Term = 0.028"
- 3 See manufacture installation guide note E4 for installation details
- 4 Girders are designed to be supported on the bottom edge only. 5 Top flange must be laterally braced at a maximum of 8'11" o.c.
- 6 Bottom flange braced at bearings.

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IM0119-019

Wind 0 0

Ld. Comb. 1,25D+1.5L 1.25D+1.5L

Page 1 of 1

January 04, 2019

ĺ	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
١	1	Tie-In	0-0-0 to 10-9-12	(Span)0-9-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
١	2	Tie-In	0-0-0 to 1-5-4	(Span)3-0-9	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
١	3	Point	1-4-0	12	Near Face	89 lb	236 lb	0 lb	0 lb	F13
١	4	Tie-In	1-5-4 to 10-9-12	(Span)	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
I				0-10-12				Pa	ss-Thru F	raming Squ

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

uash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

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This design is valid until 10/31/2020

### Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325 www.lpcorp.com CCMC: 12412-R APA: PR-L238C







Client:

Project: Address: Date: 1/3/2019

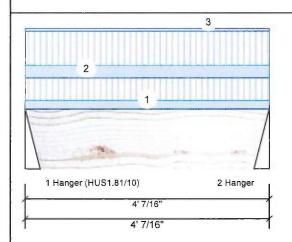
Designer: S B

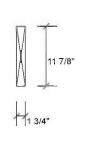
Job Name: LOT 15 (AMELIA 12 EL- 2)

Forex 2.0E-3000Fb LVL

1.750" X 11.875" - PASSED

Level: Ground Floor





### **Member Information**

"	Weiliber Illioilliation								
	Туре:	Girder							
	Plies:	1							
	Moisture Condition:	Dry							
	Deflection LL:	360							
	Deflection TL:	240							
	Importance:	Normal							
	General Load								
	Floor Live:	40 PSF							
	Dead:	15 PSF							

Application: Design Method:

**Building Code:** Load Sharing:

Floor (Residential) LSD NBCC 2010 / OBC 2012

No

Not Checked Not Checked Vibration:

# **Unfactored Reactions UNPATTERNED lb (Uplift)**

Brg	Live	Dead	Snow	Wind	
1	135	66	0	0	
2	135	66	0	0	

# **Bearings and Factored Reactions**

Hanger

Live

40 PSF

40 PLF

0 PLF

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.	
1 -	3.000"	7%	83 / 202	285	L	1.25D+1.5L	
Hanger							
2 -	3.000"	7%	83 / 202	285	L	1,25D+1.5L	

### **Analysis Results**

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	236 ft-lb	2' 3/16"	17130 ft-lb	0.014 (1%)	1.25D+1.5L	L
Unbraced	236 ft-lb	2' 3/16"	12143 ft-lb	0.019 (2%)	1.25D+1.5L	L
Shear	119 lb	1'2 1/8"	5798 lb	0.020 (2%)	1.25D+1.5L	L
Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
LL Defl inch	0.001 (L/37489)	2' 1/4"	0.122 (L/360)	0.010 (1%)	L	L
TL Defl inch	0.002 (L/25138)	2' 1/4"	0.183 (L/240)	0.010 (1%)	D+L	L

## **Design Notes**

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.
- 4 Bottom braced at bearings.

ENGINEER I.MATIJEVIC 100528832 NOVINCE OF ONTARIO

January 04, 2019

ID	Load Type	Location	Trib Width	Side	Dead
1	Tie-In	0-0-0 to 4-0-7	(Span)1-4-1	Тор	15 PSF
2	Part. Uniform	0-0-0 to 4-0-7		Тор	15 PLF
3	Part. Uniform	0-0-0 to 4-0-7		Тор	3 PLF
	Self Weight				5 PLF

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Pass-Thru Framing Squash Block is required at all point loads over bearings

Comments

**Refer to Multiple Member Connection** Detail for ply to ply nailing or bolting requirements

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

### Handling & Installation

- and ling & Installation
  LVL beams must not be cut or drilled
  Refer to manufacturer's product information
  regarding installation requirements, multi-ply
  fastening details, beam strength values, and code
  approvals
  Damaged Beams must not be used
  Design assumes top adge is laterally restrained
  Provide lateral support at bearing points to avoid
  lateral displacement and rotation

This design is valid until 10/18/2021

6. For flat roofs provide proper drainage to prevent

Manufacturer Info

Forex

Snow

0 PSF

0 PLF

0 PLF

Wind

0 PSF

0 PLF

0 PLF



Client:

Project: Address:

1/3/2019 Date:

Designer: SB

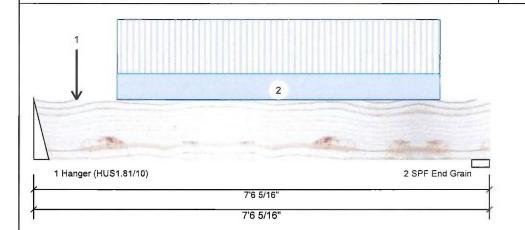
Job Name: LOT 15 (AMELIA 12 EL- 2)

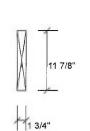
Project #:

Forex 2.0E-3000Fb LVL

1.750" X 11.875" - PASSED

Level: Ground Floor





Member	mormation
Type:	Girder

Туре:	Girder
Plies:	1
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	

40 PSF

15 PSF

Application: Floor (Residential) Design Method: LSD NBCC 2010 / OBC 2012 **Building Code:** 

Load Sharing: Nο Deck: Not Checked

Vibration: Not Checked

**Analysis Results** 

Floor Live:

Dead:

Ī	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
ı	Moment	1087 ft-lb	3'8 13/16"	17130 ft-lb	0.063 (6%)	1.25D+1.5L	L
ı	Unbraced	1087 ft-lb	3'8 13/16"	6326 ft-lb	0.172 (17%)	1.25D+1.5L	L
ı	Shear	676 lb	1'2 1/8"	5798 lb	0.117 (12%)	1.25D+1.5L	L
l	Perm Defl in.	0.007 (L/13076)	3'8 7/8"	0.237 (L/360)	0.030 (3%)	D	Uniform
ı	LL Defl inch	0.012 (L/7091)	3'8 7/8"	0.237 (L/360)	0.050 (5%)	L	L
ĺ	TL Defl inch	0.019 (L/4598)	3'8 7/8"	0.355 (L/240)	0.050 (5%)	D+L	L

## **Design Notes**

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.
- 4 Bottom braced at bearings.

Load Type

Point

2 Part. Uniform 1-4-8 to 6-8-8 Far Face **39 PLF** 81 PLF Self Weight 5 PLF

Location

0-8-8

Trib Width

Side

Far Face

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

# Unfactored Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	Wind
1	281	154	0	0
2	241	134	0	0

# **Bearings and Factored Reactions**

-						
Bearing Length	Cap. Re	eact D/L lb	Total	Ld. Case	Ld. Comb.	
1 - 3.000" Hanger	16%	192 / 422	614	L	1.25D+1.5L	
2 - SPE 3.500"	12%	168 / 361	529	L	1 25D+1 5I	

End Grain

Live

90 lb

Snow

0 PLF

0 lb



January 04, 2019

Pass-Thru Framing Squash Block is required at all point loads over bearings

**Refer to Multiple Member Connection** Detail for ply to ply nailing or bolting requirements

ID

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. Lumber

- Dry service conditions, unless noted otherwise
   LVL not to be treated with fire retardant or corrosive
- chemicals
- Handling & Installation
- andling & Installation
  LVL beams must not be cut or drilled
  Refer to manufacturer's product information
  regarding installation requirements, multi-ply
  fastening details, beam strength values, and code
  approvals
  Damaged Beams must not be used
  Design assumes top adge is laterally restrained
  Provide lateral support at beaming points to avoid
  lateral displacement and rotation

Dead

44 lb

6. For flat roofs provide proper drainage to prevent

Manufacturer Info

Forex

Wir

0 lb

0 PLF

J7

Kott Lumber Company 14 Anderson Blvd, Ontario L4A 7X4 905-642-4400





Client:

Project: Address:

Date: 1/3/2019

Designer: S B

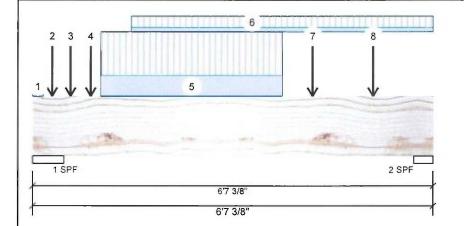
Job Name: LOT 15 (AMELIA 12 EL- 2)

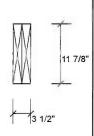
Project #:

Forex 2.0E-3000Fb LVL

1.750" X 11.875"

2-Ply - PASSED Level: Second Floor





Mem	nor	nto	rma	TIME

•	Weinber Information					
	Туре:	Girder				
	Plies:	2				
	Moisture Condition:	Dry				
	Deflection LL:	360				
	Deflection TL:	240				
	Importance:	Normal				
	General Load					
	Floor Live:	40 PSF				
	Dead:	15 PSF				

### Application: Floor (Residential)

Design Method:	LSD
<b>Building Code:</b>	NBCC 2010 / OBC 2012
Load Sharing:	No

Deck: Not Checked Not Checked Vibration:

# Unfactored Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	Wind	
1	2149	981	0	0	
2	1155	518	0	0	

# **Bearings and Factored Reactions**

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	6.188"	33%	1227 / 3223	4450	Ļ	1.25D+1.5L
2-SPF	3.813"	29%	648 / 1732	2380	L	1.25D+1.5L

**Analysis Results** 

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	3687 ft-lb	3'4 7/8"	34261 ft-lb	0.108 (11%)	1.25D+1.5L	L
Unbraced	3687 ft-lb	3'4 7/8"	32638 ft-lb	0.113 (11%)	1.25D+1.5L	L
Shear	2653 lb	1'5 5/16"	11596 lb	0.229 (23%)	1.25D+1.5L	L
Perm Defl in.	0.008 (L/9333)	3'4 13/16"	0.197 (L/360)	0.040 (4%)	D	Uniform
LL Defl inch	0.016 (L/4353)	3'4 15/16"	0.197 (L/360)	0.080 (8%)	L	L
TL Defl inch	0.024 (L/2968)	3'4 7/8"	0.295 (L/240)	0.080 (8%)	D+L	L

### Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on full section width.

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



January 04, 2019

ı	O Lateral Sienue	mess ratio based on rul	Section width.							
I	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
I	1	Tie-In	0-0-0 to 0-2-2	(Span)1-4-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
I	2	Point	0-3-14		Near Face	367 lb	841 lb	0 lb	0 lb	F12
I	3	Point	0-7-8		Far Face	145 lb	326 lb	0 lb	0 lb	J4
I	4	Point	0-11-8		Near Face	29 lb	78 lb	0 lb req	uired at a	raming Squash Block is all point loads over bearings
I	5	Part. Uniform	1-1-8 to 4-1-8		Far Face	155 PLF	326 PLF	0 PLF	0 PLF	
	6 Continued on page	Part. Uniform 2	1-7-8 to 6-7-6		Near Face	32 PLF	85 PLF	Det		tiple Member Connection y to ply nailing or bolting

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, It is the responsibility of the customer and/or the contractor to ensure the component autiability of the intended application, and to verify the dimensions and loads.

### Lumber

- Dry service conditions, unless noted otherwise
   LVL not to be treated with fire retardant or corrosive
- chemicals Handling & Installation
  - LVL beams must not be cut or drilled
    Refer to manufacturer's product information
    regarding installation requirements, multi-ply
    fastening details, beam strength values, and code

  - paprovals

    Damaged Beams must not be used

    Design assumes top edge is laterally restrained

    Provide lateral support at bearing points to avoid lateral displacement and rotation
- For flat roofs provide proper drainage to prevent ponding

Manufacturer Info Forex

Kott Lumber Company 14 Anderson Blvd, Ontario Canada L4A 7X4 905-642-4400



isDesign™

Project:

Address:

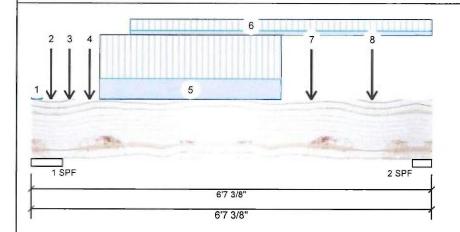
Designer: SB

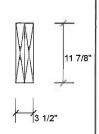
Job Name: LOT 15 (AMELIA 12 EL- 2)

Forex 2.0E-3000Fb LVL

1.750" X 11.875"

2-Ply - PASSED Level: Second Floor





Continued	from page 1					-			
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
7	Point	4-7-8		Far Face	147 lb	326 lb	0 lb	0 lb	J4
8	Point	5-7-8		Far Face	122 lb	326 lb	d1 0	0 lb	J4
	Self Weight				10 PLF				



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Pass-Thru Framing Squash Block is required at all point loads over bearings

**Refer to Multiple Member Connection** Detail for ply to ply nailing or bolting requirements

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the dimensions and loads.

- Dry service conditions, unless noted otherwise
   LVL not to be treated with fire retardant or corrosive
- chemicals
- Handling & Installation
- andling & Installation
  LVL beams must not be cut or drilled
  Refer to manufacturer's product information
  regarding installation requirements, multi-ply
  fastening details, beam strength values, and code
  approvals
  Damaged Beams must not be used
  Design assumes top edge is laterally restrained
  Provide lateral support at bearing points to avoid
  lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

This design is valid until 10/18/2021

Manufacturer Info

1467 ft-lb 1.25D+1.5L L 4'4 1/8" 34261 ft-lb 0.043 (4%) Moment Unbraced 1467 ft-lb 4'4 1/8" 31209 ft-lb 0.047 (5%) 1.25D+1.5L L Shear 878 lb 1'3 1/8" 11596 lb 0.076 (8%) 1.25D+1.5L L Perm Defl in. 0.005 4'3 15/16" 0.270 (L/360) 0.020 (2%) D Uniform (L/19415) LL Defl inch 0.010 (L/9404) 4'3 15/16" 0.270 (L/360) 0.040 (4%) L TL Defl inch 0.015 (L/6335) 4'3 15/16" 0.405 (L/240) 0.040 (4%) D+L



1 Fill all hanger nailing holes.

- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top braced at bearings.

**Design Notes** 

- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on full section width.

ID Load Type Trib Width Live Location Side Dead Wind Comments Snow 0-5-14 Far Face 29 lb 78 lb di 0 0 lb J6 2 Part. Uniform 1-1-14 to 8-7-3 Far Face **32 PLF** 86 PLF 0 PLF 0 PLF

> Self Weight 10 PLF

> > READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Pass-Thru Framing Squash Block is required at all point loads over bearings

**Refer to Multiple Member Connection** Detail for ply to ply nailing or bolting requirements

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

### Handling & Installation

- LVL beams must not be cut or drilled
  Refer to manufacturer's product information
  regarding installation requirements, multi-ply
  fastening details, beam strength values, and code
  approvals
- payering dapprovals parmaged Beams must not be used Design assumes top edge is laterally restrained Provide lateral support at bearing points to lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent

Manufacturer Info Forex

Kott Lumber Company 14 Anderson Blvd, Ontario Canada **L4A 7X4** 905-642-4400



/ Lateral Sit	siluciness rado paseu o	it full section width.				
ID	Load Type	Location	Trib Width	Side	Dead	Live
1	Tie-In	0-0-0 to 3-11-8	(Span)0-7-10	Тор	15 PSF	40 PSF
2	Tie-In	0-0-0 to 1-2-4	(Span)0-4-7 to 0-1-12	Тор	15 PSF	40 PSF
3	Point	1-2-4		Near Face	106 lb	283 lb
4	Part. Uniform	1-10-4 to 4-6-4		Near Face	86 PLF	228 PLF
5	Point	4-1-4		Far Face	172 lb	352 lb
Continued on p	page 2					

0 lb required at all point loads over bearings

PLF Refer to Multiple Member Connection O lb Detail for bly to ply nailing or bolting requirements

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to venify the dimensions and loads.

- Dry service conditions, unless noted otherwise
   LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply regarding installation requirements, making fastening details, beam strength values, and code

- approvals
  Damaged Beams must not be used
  Design assumes top edge is laterally restrained
  Provide lateral support at bearing points to avoid
  lateral displacement and rotation

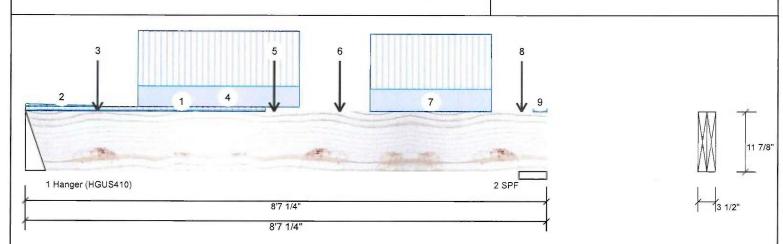
For flat roofs provide proper drainage to prevent ponding

Manufacturer Info APA: PR-L318

Kott Lumber Company 14 Anderson Blvd, Ontario Canada L4A 7X4 905-642-4400







Continued f	from page 1								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
6	Point	5-2-4		Near Face	101 lb	267 lb	0 lb	0 lb	J1
7	Part. Uniform	5-8-4 to 7-8-4		Near Face	92 PLF	228 PLF	0 PLF	0 PLF	
8	Point	8-2-4		Near Face	7 lb	17 lb	0 lb	0 lb	J1
9	Tie-In	8-4-8 to 8-7-4	(Span) 0-10-14	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	Self Weight				10 PLF				



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Pass-Thru Framing Squash Block is required at all point loads over bearings

**Refer to Multiple Member Connection** Detail for ply to ply nailing or bolting requirements

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

### Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

# Handling & Installation

- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

- approvals
  Damaged Beams must not be used
  Design assumes top edge is laterally restrained
  Provide lateral support at bearing points to avoid
  lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex

Kott Lumber Company 14 Anderson Blvd, Ontario Canada L4A 7X4 905-642-4400





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Client: Project: Address:

Designer:

SB

Job Name: LOT 15 (AMELIA 12 EL- 2)

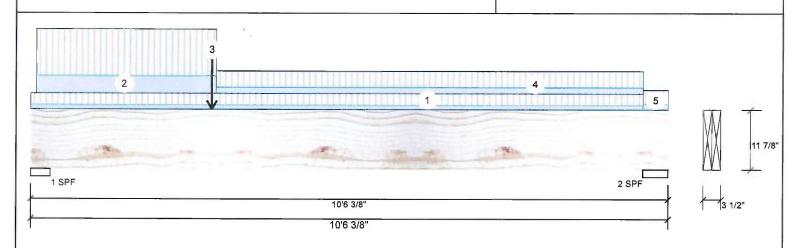
Project #

Forex 2.0E-3000Fb LVL

1.750" X 11.875"

2-Ply - PASSED

Level: Second Floor



Member Info	rmation			Unfactore	ed Reac	tions U	NPATTERN	ED lb (Uplift)	
Type:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snow	Wind
Plies:	2	Design Method:	LSD	1	828		377	0	0
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	426		217	0	0
Deflection LL:	360	Load Sharing:	No						
Deflection TL:	240	Deck:	Not Checked						
Importance:	Normal	Vibration:	Not Checked						
General Load									
Floor Live:	40 PSF			Bearings	and Fac	tored F	Reactions		
Dead:	15 PSF			Bearing 1	Length	Cap.	React D/L lb	Total Ld. Case	Ld. Comb.
				1 - SPF 3	3.875"	21%	471 / 1242	1713 L	1.25D+1.5L
	T.			2-SPF 5	5.000"	8%	271 / 640	911 L	1.25D+1.5L

**Analysis Results** 

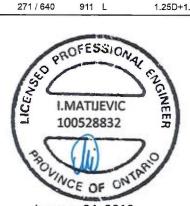
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	3866 ft-lb	2'11 15/16"	34261 ft-lb	0.113 (11%)	1.25D+1.5L	L
Unbraced	3866 ft-lb	2'11 15/16"	29686 ft-lb	0.130 (13%)	1.25D+1.5L	L
Shear	1486 lb	1'3"	11596 lb	0.128 (13%)	1.25D+1.5L	L
Perm Defl in.	0.016 (L/7482)	4'9 1/16"	0.331 (L/360)	0.050 (5%)	D	Uniform
LL Defl inch	0.034 (L/3491)	4'8 1/8"	0.331 (L/360)	0.100 (10%)	L	L
TL Defl inch	0.050 (L/2380)	4'8 7/16"	0.496 (L/240)	0.100 (10%)	D+L	L

### **Design Notes**

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.

6 Lateral slenderness ratio based on full section width

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



January 04, 2019

- C Lutolu,	ordinactinece tand basea	on fall scotlon width.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 10-1-6	(Span) 0-10-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-1-2 to 3-0-13	(Span)3-7-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	2-11-15		Far Face	270 lb	660 lb	0 lb	0 lb	F7
4	Tie-In	3-0-13 to 10-1-6	(Span)1-3-9	Тор	15 PSF	40 PSF	0 PSF	0.PSF	raming Squash Block is
5	Tie-In	10-1-6 to 10-6-6	(Span)1-1-9	Тор	15 PSF	40 PSF	0 PSF rec	uired Sit a	all point loads over bearings
	Self Weight				10 PLF				

**Refer to Multiple Member Connection** Detail for ply to ply nailing or bolting

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

# Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

### chemicals

### Handling & Installation

- LVL beams must not be cut or drilled
  Refer to manufacturer's product information
  regarding installation requirements, multi-ply
  fastening details, beam strength values, and code

- approvals
  Damaged Beams must not be used
  Design assumes top edge is laterally restrained
  Provide lateral support at bearing points to avoid
  lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer info

APA: PR-L318

Kott Lumber Company 14 Anderson Blvd, Ontario Canada L4A 7X4 905-642-4400





ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

required at all point loads over bearings

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6. For flat roofs provide proper

requiremen Manufacturer Info APA: PR-L318

Kott Lumber Company 14 Anderson Blvd, Ontario Canada L4A 7X4 905-642-4400





Client:

Project: Address: Date: 1/3/2019

Designer: SB

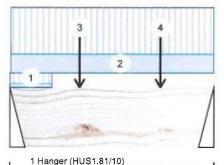
Job Name: LOT 15 (AMELIA 12 EL- 2)

Project #:

Forex 2.0E-3000Fb LVL

1.750" X 11.875" - PASSED

Level: Second Floor



2 Hanger (HUS1.81/10) 3'4 7/16' 3'4 7/16"

iviemper Intori	nation		
Type:	Girder	Application:	Floor (Residential)
Plies:	1	Design Method:	LSD
Moisture Condition	: Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

Brg	Live	Dead	Snow	Wind
1	660	270	0	0
2	650	266	0	0
2	000	200	v	v

### **Analysis Results** Analysis Actual Location Allowed Comb. Case Capacity 0.054 (5%) 1.25D+1.5L L 923 ft-lb 1'7 3/16" 17130 ft-lb Moment Unbraced 923 ft-lb 1'7 3/16" 13790 ft-lb 0.067 (7%) 1.25D+1.5L L 0.114 (11%) 1.25D+1.5L L Shear 662 lb 2'2 5/16" 5798 lb Perm Defl in. 0.002 1'7 3/4" 0.100 (L/360) 0.020 (2%) D Uniform (L/21560) LL Defl inch 0.004 (L/8783) 1'7 11/16" 0.100 (L/360) 0.040 (4%) L TL Defl inch 0.006 (L/6241) 1'7 11/16" 0.150 (L/240) 0.040 (4%) D+L L

**Bearings and Factored Reactions** Cap. React D/L lb Bearing Length Total Ld. Case Ld. Comb. 1.25D+1.5L 34% 338 / 990 1328 L 3.000" Hanger

3.000" 34% 333 / 975 1307 L 1.25D+1.5L Hanger

**Design Notes** 

1 Fill all hanger nailing holes.

when Information

- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.
- 4 Bottom braced at bearings

PROFESSIONAL ENGINEER I.MATUEVIC 100528832 OVINCE OF ONTARY January 04, 2019

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Winc	1.
1	Tie-In	0-0-0 to 0-8-5	(Span)3-1-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Part. Uniform	0-0-3 to 3-4-7		Тор	110 PLF	270 PLF	0 PLF	0 PLF	
3	Point	1-1-13		Near Face	71 lb	190 lb	0 lb	0 lb	J8
4	Point	2-5-13		Near Face	64 lb	171 lb	0 lb	0 lb	J8
	Self Weight				5 PLF	Pass-Thru	Framing Sq	uash Blo	ck is

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Pass-Thru Framing Squash Block is required at all point loads over bearings

**Refer to Multiple Member Connection** Detail for ply to ply nailing or bolting requirements

Manufacturer Info

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# Lumber

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2 -

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