

ENGINEERING NOTE PAGE (ENP-1)
PLEASE READ PRIOR TO INSTALLATION

NE0618-006
GREEN YORK HOMES - GRANELLI
HOME CORP - LIANA 3 ELEV 2

RESPONSIBILITIES

THIS DESIGN IS FOR AN INDIVIDUAL BUILDING COMPONENT AND HAS BEEN BASED ON INFORMATION PROVIDED BY THE DESIGN OFFICE OF KOTT LUMBER. THE UNDERSIGNED ENGINEER DISCLAIMS ANY RESPONSIBILITY FOR DAMAGES AS A RESULT OF FAULTY OR INCORRECT INFORMATION, SPECIFICATION AND/OR DESIGNS FURNISHED TO THE ENGINEER. THE UNDERSIGNED ENGINEER IS ONLY RESPONSIBLE FOR THE STRUCTURAL INTEGRITY OF THIS BUILDING COMPONENT FOR THE CONDITIONS AND LOADS SHOWN ON THIS DRAWING. THE STRUCTURAL INTEGRITY OF THE BUILDING AND THE VERIFICATION OF THE DIMENSIONS AND THE DESIGN LOADS USED ARE THE RESPONSIBILITY OF THE BUILDING DESIGNER.

TRUSSES ARE DESIGNED IN CONFORMANCE WITH THE RELEVANT SECTIONS OF THE NATIONAL BUILDING CODE OF CANADA OR THE CANADIAN CODE FOR FARM BUILDINGS, WHICHEVER APPLIES TO THE BUILDING TYPE INDICATED ON THE DRAWING

IT IS THE RESPONSIBILITY OF KOTT LUMBER TO ENSURE THAT TRUSSES ARE MANUFACTURED IN CONFORMANCE WITH THESE DESIGNS AND WITH THE SPECIFICATIONS OUTLINED BELOW. THE UNDERSIGNED ENGINEER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

USE AND OCCUPANCY

- The building is of the type indicated on the drawing

All work shall conform to the Ontario Building Code O. Reg. 332/12 as amended

LOADING

- The truss loading intensity and distribution as well as load transfer mechanism is that indicated on the drawing
- No buildings, trees, parapets or other projections higher than the roof for which the trusses are used are located within a distance less than ten (10) times the difference in height, or five metres (16 ft) whichever is greater, unless the drawing indicates that the snow drifting has been taken into account

HANDLING, INSTALLATION AND BRACING

- The trusses must be handled and installed by a qualified professional as per the supplied document titled *Information for Truss Installers* and the BCSI-B1 and BCSI-B3 Summary Sheets
- The compression chords are laterally braced by continuous rigid diaphragm sheathing or as specified on the drawing
- Temporary and permanent bracing must be installed as indicated on the truss drawing and according to the BCSI-B1 and BCSI-B3 Summary Sheets. Bracing for the lateral stability of the truss is to be provided by the building designer
- **It is recommended that a Professional Engineer's advice be obtained for the bracing of trusses spanning more than 12.37m (40'-7")**

SUPPORTS

- The trusses are to be supported at the bearing points indicated and anchored to the supports where considered necessary by the designer of the overall structure
- Bearing sizes shown are the minimum required to prevent crushing of the truss members and do not necessarily take into account stability of the overall building structure
- Elevation of bearings must be carefully checked and shimmed to alignment for solid bearings
- Adequate wood truss bearing is the responsibility of the building designer.

DIMENSIONS

- Geometry of the truss and dimensions indicated on the drawing are identical to those of the installed truss.

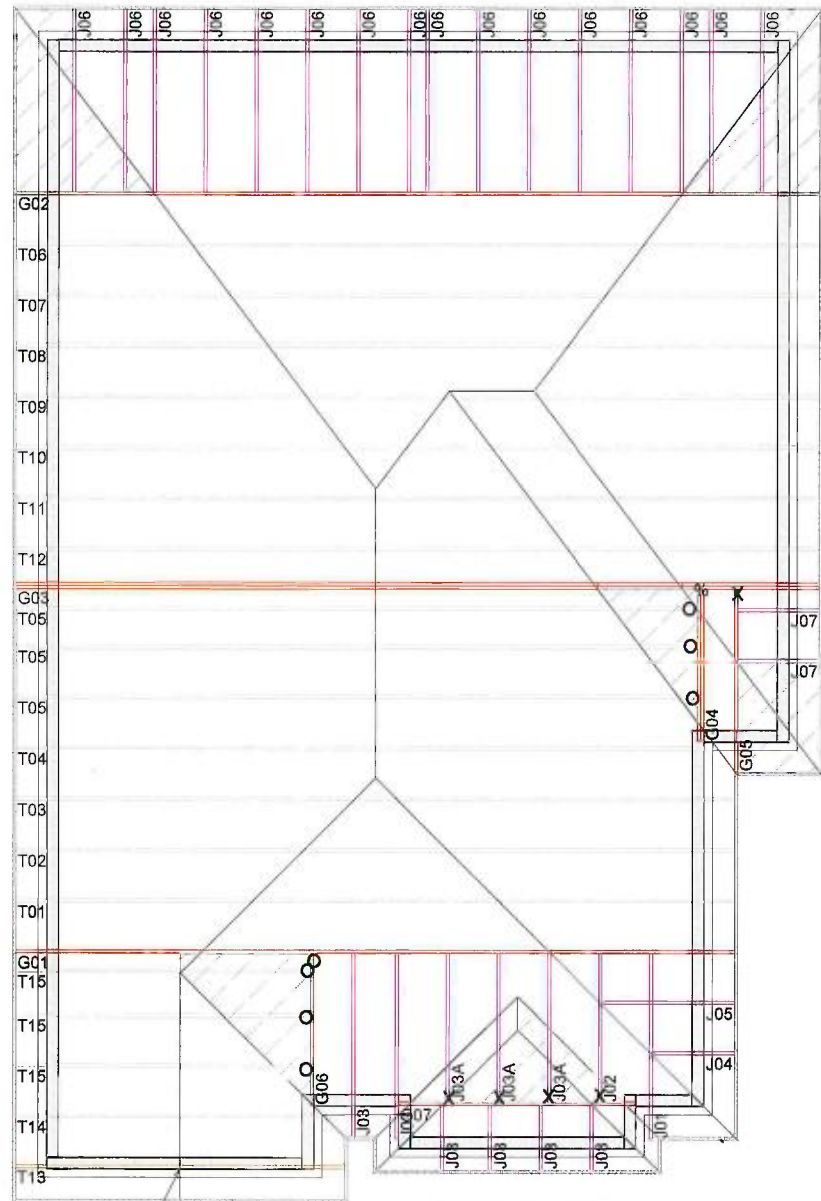
Layout based on plans provided by
Jardin Design Group Inc.
Revision #2 dated May 22, 2018

THIS CERTIFICATION IS FOR THE ENGINEERING REVIEW OF STRUCTURAL COMPONENTS SHOWN ON THIS DRAWING AND IS LIMITED TO THE COMPONENTS SUPPLIED BY KOTT LUMBER FOR THE LOADS AND CONDITIONS SHOWN ON THE CALCULATION PAGE OF EACH COMPONENT. THE PLACEMENT OF THESE COMPONENTS ON THIS LAYOUT IS SHOWN FOR REFERENCE ONLY. ROOF TRUSSES MUST BE INSTALLED ACCORDING TO MANUFACTURER'S GUIDELINES AT 24" C/C UNLESS NOTED OTHERWISE. CONVENTIONAL FRAMING SHOWN ON THE LAYOUT MUST BE DESIGNED AND APPROVED BY THE PROJECT ENGINEER. TEMPORARY AND PERMANENT BRACINGS OF THE ROOF AND BUILDING ARE THE RESPONSIBILITY OF THE PROJECT ENGINEER AND TO CONFORM TO BCSI GUIDELINES.



ENG.JOB:NE0618-006

All work shall conform to the Ontario
Building Code O. Reg. 332/12 as amended



Only front gable truss designed at
10/12 pitch

LOWER ROOF TO BE
CRF AS PER PLAN

For conventional framing roof / ceilings
framing shall conform to OBC.9.23

19-447156 RR.

ROOF TRUSS LAYOUT



CONVENTIONAL
FRAMING BY OTHERS

HANGER LEGEND

X - LUS24
- HGUS26

O - LJS26DS
% - HGUS26-2

DESIGN CRITERIA
SPECIFIED LOADS
TOP CH. LL = 23.3 psf
DL = 3.0 psf
BOT CH. LL = 0.0 psf
DL = 7.0 psf
TOTAL LOAD = 33.3 psf

SPACING @ 24" CENTERS.
THIS TRUSS SYSTEM IS DESIGN FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

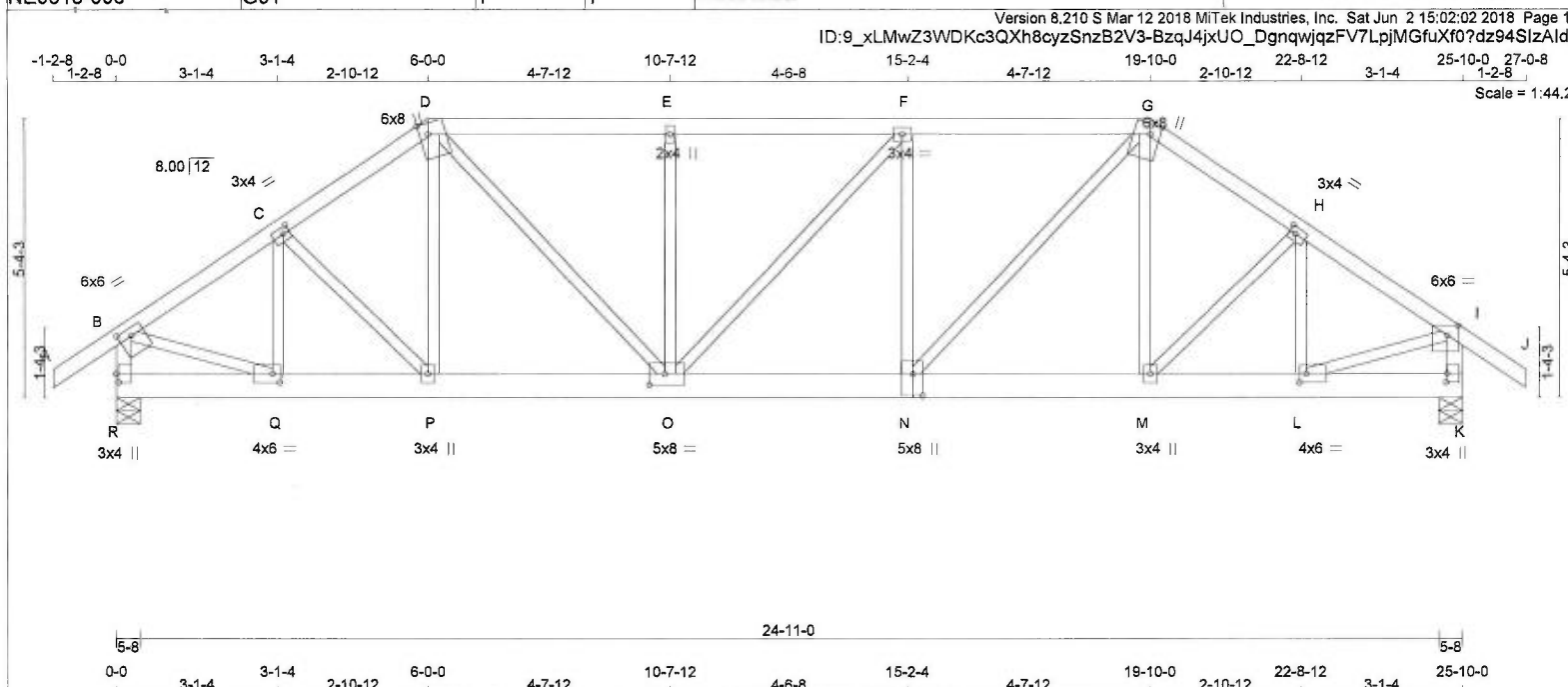
THIS DESIGN COMPLIES WITH PART 9 OF OBC 2012,
BCBC 2012, ABC 2012, CSA 086-09, TPIC 2011

ALLOWABLE DEFLECTION OF EACH COMPONENT = L/360

Model:	LIANA 3 ELE 2
Customer:	GREEN YORK HOMES
Project:	GRANELLI HOME
Location:	BRAMPTON, ON
Drawn By:	DM
Date:	5/31/2018

SIZE AND LOCATION OF CONVENTIONAL FRAMING IS APPROXIMATE. ALL AREAS
MAY NOT BE SHOWN. REFER TO ARCHITECTURAL PLANS FOR DETAILS.

ALL CONVENTIONAL FRAMING TO CONFORM WITH PART 9 OF THE O.B.C RAFTER THAT
CROSS MEET OVER TRUSSES TO BE 2X4 SPF @ 24" O/C WITH 2X4 VERT POST TO THE
TRUSS BELOW EA CROSS PT. VERT POS. THAN 6' TO HAVE A LATERAL BRACING SO THAT
THE DISTANCE BETEN PT & VERT ROWS OF BRACING DOES NOT EXCEED 6'



LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER										DESIGN CRITERIA			
N. L. G. A. RULES				BEARINGS										*** SPECIAL LOADS ANALYSIS ***			
CHORDS SIZE LUMBER DESCR.				FACTORED GROSS REACTION MAXIMUM FACTORED INPUT REQD										GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.			
A - D 2x4 DRY No.2 SPF				JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX										LOADS WERE DERIVED FROM USER INPUT			
D - G 2x4 DRY No.2 SPF				R 2248 0 2248 0 0 5-8 5-8										NO FURTHER MODIFICATIONS WERE MADE			
G - J 2x4 DRY No.2 SPF				K 2499 0 2499 0 0 5-8 5-8										SPECIFIED LOADS:			
R - B 2x4 DRY No.2 SPF				UNFACTORED REACTIONS										TOP CH. LL = 23.3 PSF			
K - I 2x4 DRY No.2 SPF				1ST LCASE MAX./MIN. COMPONENT REACTIONS										DL = 3.0 PSF			
R - N 2x6 DRY No.2 SPF				JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL										BOT CH. LL = 0.0 PSF			
N - K 2x6 DRY No.2 SPF				R 1576 1110 / 0 0 / 0 0 / 0 466 / 0 0 / 0										DL = 7.3 PSF			
ALL WEBS 2x3 DRY No.2 SPF				K 1755 1218 / 0 0 / 0 0 / 0 538 / 0 0 / 0										TOTAL LOAD = 33.6 PSF			
EXCEPT				BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K										SPACING = 24.0 IN. C/C			
DRY: SEASONED LUMBER.				BRACING										LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM			
PLATES (table is in inches)				TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.87 FT.										GIRDER TYPE: CPrimeHip			
JT TYPE PLATES W LEN Y X				MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.										SIDE SETBACK = 6-0-0			
B TMVW-t MT20 6.0 6.0 1.75 3.00				ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.										END SETBACK = 6-0-0			
C TMWW-t MT20 3.0 4.0 1.50 1.50				LOADING										END WALL WIDTH = 5-8			
D TTTWW+m MT20 6.0 8.0 Edge 2.00				TOTAL LOAD CASES: (4)										CORNER FRAMING TYPE: CONVENTIONAL			
E TMW+w MT20 2.0 4.0				CHORDS										END JACK TYPE: CONVENTIONAL			
F TMWW-t MT20 3.0 4.0				MAX. FACTORED										APPLIED TO FRONT SIDE			
G TTTWW+m MT20 6.0 8.0 2.25 2.25				MEMB. FORCE VERT. LOAD LC1 MAX. UNBRAC										- ADDT'L LOADS BASED ON 55 % OF GSL.			
H TMWW-t MT20 3.0 4.0 1.50 1.50				WEBS										LOADS APPLIED TO FIRST 15-0-0 OF SPAN			
I TMVV-p MT20 6.0 6.0 2.25 2.50				MAX. FACTORED										MEASURED FROM THE RIGHT.			
K BMV1+p MT20 3.0 4.0 2.00 0.25				MEMB. FORCE MAX. UNBRAC										*** NON STANDARD GIRDER ***			
L BMWW-t MT20 4.0 6.0 2.00 1.50				FR-TO FROM TO LENGTH FR-TO										ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.			
M BMWW+t MT20 3.0 4.0				A-B 0 / 28 -77.4 -77.4 0.10 (1) 10.00 Q-C -653 / 0 0.13 (1)										THIS TRUSS IS DESIGNED FOR RESIDENTIAL			
N BSWW+t MT20 5.0 8.0 5.00 2.25				B-C -2456 / 0 -77.4 -77.4 0.19 (1) 4.21 C-P 0 / 311 0.08 (1)										OR SMALL BUILDING REQUIREMENTS OF			
O BMWW-t MT20 5.0 8.0 2.50 3.50				C-D -2749 / 0 -77.4 -77.4 0.21 (1) 3.99 P-D -59 / 30 0.02 (1)										PART 9, NBCC 2010			
P BMWW+t MT20 3.0 4.0				D-E -3530 / 0 -77.4 -77.4 0.51 (1) 3.26 D-O 0 / 1841 0.46 (1)										THIS DESIGN COMPLIES WITH:			
Q BMWW-t MT20 4.0 6.0 2.00 1.75				E-F -3530 / 0 -145.9 -145.9 0.74 (1) 2.87 O-E -509 / 0 0.21 (1)										- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014			
R BMV1+p MT20 3.0 4.0 2.00 0.50				F-G -3457 / 0 -145.9 -145.9 0.74 (1) 2.92 O-F 0 / 108 0.03 (1)										- CSA 086-09			
				G-H -3038 / 0 -77.4 -77.4 0.22 (1) 3.80 N-F -838 / 0 0.34 (1)										- TPIC 2011			
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.				H-I -2737 / 0 -77.4 -77.4 0.21 (1) 4.00 N-G 0 / 1383 0.34 (1)										(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD			
				I-J 0 / 28 -77.4 -77.4 0.10 (1) 10.00 M-G -42 / 113 0.04 (4)										ALLOWABLE DEFL.(LL)= L/360 (0.86")			
HANGERS NOTES				R-B -2191 / 0 0.0 0.0 0.24 (1) 5.69 M-H 0 / 319 0.08 (1)										CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")			
1)				K-I -2420 / 0 0.0 0.0 0.27 (1) 5.44 L-H -662 / 0 0.14 (1)													
				R-Q 0 / 0 -18.2 -18.2 0.06 (1) 10.00 B-Q 0 / 2145 0.53 (1)													
				Q-P 0 / 2053 -18.2 -18.2 0.33 (1) 10.00 L-I 0 / 2390 0.59 (1)													
				P-O 0 / 2271 -18.2 -18.2 0.36 (1) 10.00													
				O-N 0 / 3457 -34.4 -34.4 0.54 (1) 10.00													
				N-M 0 / 2511 -34.4 -34.4 0.37 (1) 10.00													
				M-L 0 / 2287 -34.4 -34.4 0.35 (1) 10.00													
				L-K 0 / 0 -34.4 -34.4 0.07 (1) 10.00													

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 348.1 lbs FACTORED DOWN AT 19-10-0 ON TOP CHORD, AND 854.0 lbs FACTORED DOWN AT 10-5-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667 822 2284 1656

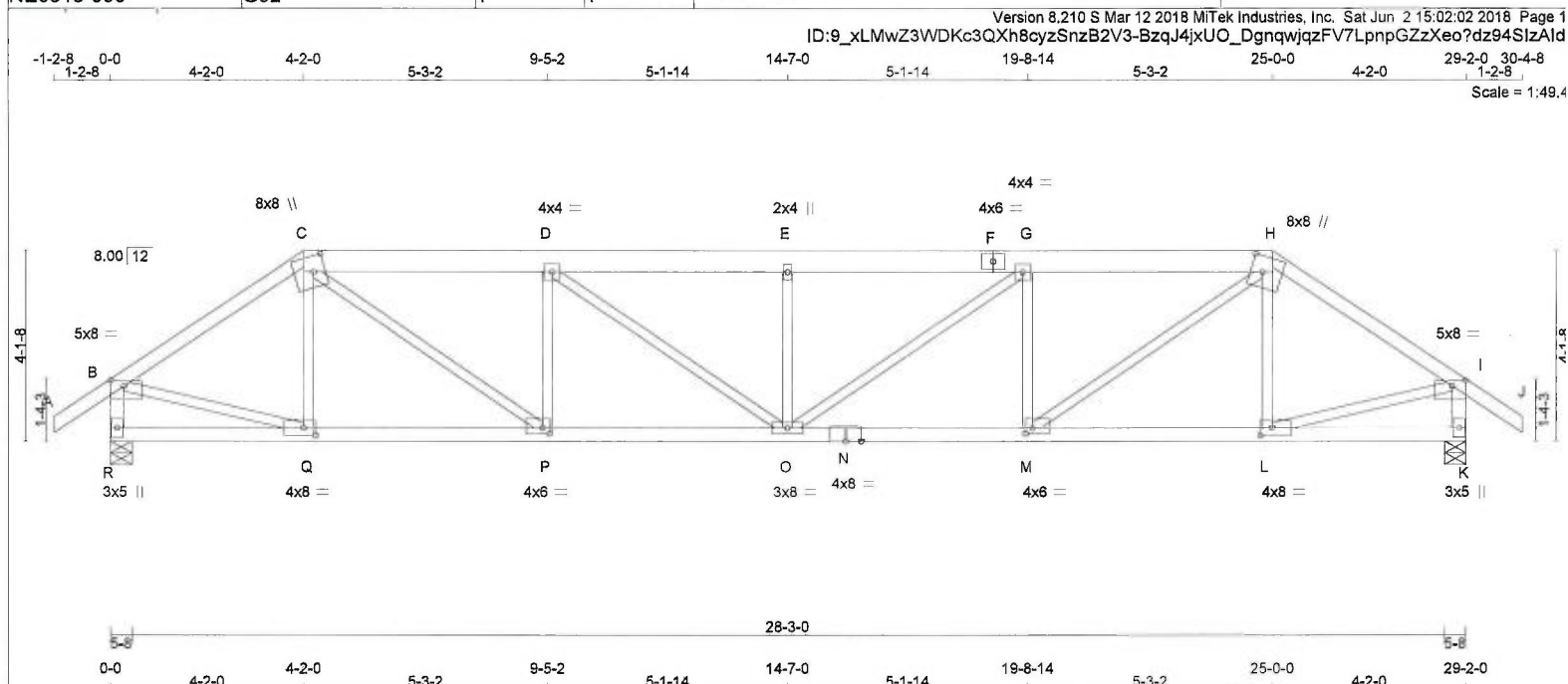
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (O) (INPUT = 0.90)
JSI METAL= 0.64 (B) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
R - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVV-p	MT20	5.0	8.0	1.50	3.50
C	TTWW+m	MT20	8.0	8.0	4.00	3.00
D	TMWW-t	MT20	4.0	4.0		
E	TMW-w	MT20	2.0	4.0		
F	TS-t	MT20	4.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW+m	MT20	8.0	8.0	4.00	3.00
I	TMVV-p	MT20	5.0	8.0	1.50	3.50
K	BMV1+p	MT20	3.0	5.0		
L	BMWW-t	MT20	4.0	8.0	2.00	3.00
M	BMWW-t	MT20	4.0	6.0	1.50	1.75
N	BS-t	MT20	4.0	8.0		
O	BMWW-t	MT20	3.0	8.0		
P	BMWW-t	MT20	4.0	6.0	1.50	1.75
Q	BMWW-t	MT20	4.0	8.0	2.00	3.00
R	BMV1+p	MT20	3.0	5.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 241.8 lbs FACTORED DOWN AT 25-0-0, AND 241.8 lbs FACTORED DOWN AT 4-2-0 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	IN-SX	IN-SX
R	2685	0	5-8	5-8
K	2685	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX/MIN	COMPONENT REACTIONS	DEAD	SOIL
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND
R	1885	1314 / 0	0 / 0	0 / 0	571 / 0
K	1885	1314 / 0	0 / 0	0 / 0	571 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.44 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 MAX (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LC1 MAX (LC)	UNBRACED LENGTH
FR-TO				FR-TO			
A-B	0 / 28	-77.4	-77.4 0.10 (1)	Q-C	-449 / 14	0.11 (1)	10.00
B-C	-3151 / 0	-77.4	-77.4 0.46 (1)	C-P	0 / 2539	0.63 (1)	3.52
C-D	-4680 / 0	-145.9	-145.9 0.43 (1)	P-D	-1309 / 0	0.33 (1)	3.63
D-E	-5292 / 0	-145.9	-145.9 0.43 (1)	D-O	0 / 757	0.19 (1)	3.44
E-F	-5292 / 0	-145.9	-145.9 0.43 (1)	O-E	-711 / 0	0.18 (1)	3.44
F-G	-5292 / 0	-145.9	-145.9 0.43 (1)	O-G	0 / 757	0.19 (1)	3.44
G-H	-4680 / 0	-145.9	-145.9 0.43 (1)	M-G	-1309 / 0	0.33 (1)	3.63
H-I	-3151 / 0	-77.4	-77.4 0.46 (1)	M-H	0 / 2539	0.63 (1)	3.52
I-J	0 / 28	-77.4	-77.4 0.10 (1)	L-H	-449 / 14	0.11 (1)	10.00
R-B	-2628 / 0	0.0	0.0 0.30 (1)	B-Q	0 / 2704	0.67 (1)	5.23
K-I	-2628 / 0	0.0	0.0 0.30 (1)	L-I	0 / 2704	0.67 (1)	5.23
R-Q	0 / 0	-34.4	-34.4 0.19 (4)				10.00
Q-P	0 / 2617	-34.4	-34.4 0.57 (1)				10.00
P-O	0 / 4680	-34.4	-34.4 0.92 (1)				10.00
O-N	0 / 4680	-34.4	-34.4 0.92 (1)				10.00
N-M	0 / 4680	-34.4	-34.4 0.92 (1)				10.00
M-L	0 / 2617	-34.4	-34.4 0.57 (1)				10.00
L-K	0 / 0	-34.4	-34.4 0.19 (4)				10.00

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	4-2-0	-242	-242	--	FRONT	VERT	TOTAL
H	25-0-0	-242	-242	--	FRONT	VERT	TOTAL

TOTAL WEIGHT = 128 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF

DL = 3.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.3 PSF

TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip

SIDE SETBACK = 4-2-0

END SETBACK = 6-0-0

END WALL WIDTH = 5-8

CORNER FRAMING TYPE: CONVENTIONAL

END JACK TYPE: CONVENTIONAL

APPLIED TO FRONT SIDE

- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014

- CSA 086-09

- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.97")

CALCULATED VERT. DEFL.(LL) = L/999 (0.23")

ALLOWABLE DEFL.(TL) = L/360 (0.97")

CALCULATED VERT. DEFL.(TL) = L/851 (0.41")

CSI: TC=0.46/1.00 (H-I:1), BC=0.92/1.00 (M-C:1), WB=0.67/1.00 (L-L:1), SS=0.29/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1667	822	2284	1556

CONTINUE ON PAGE 2



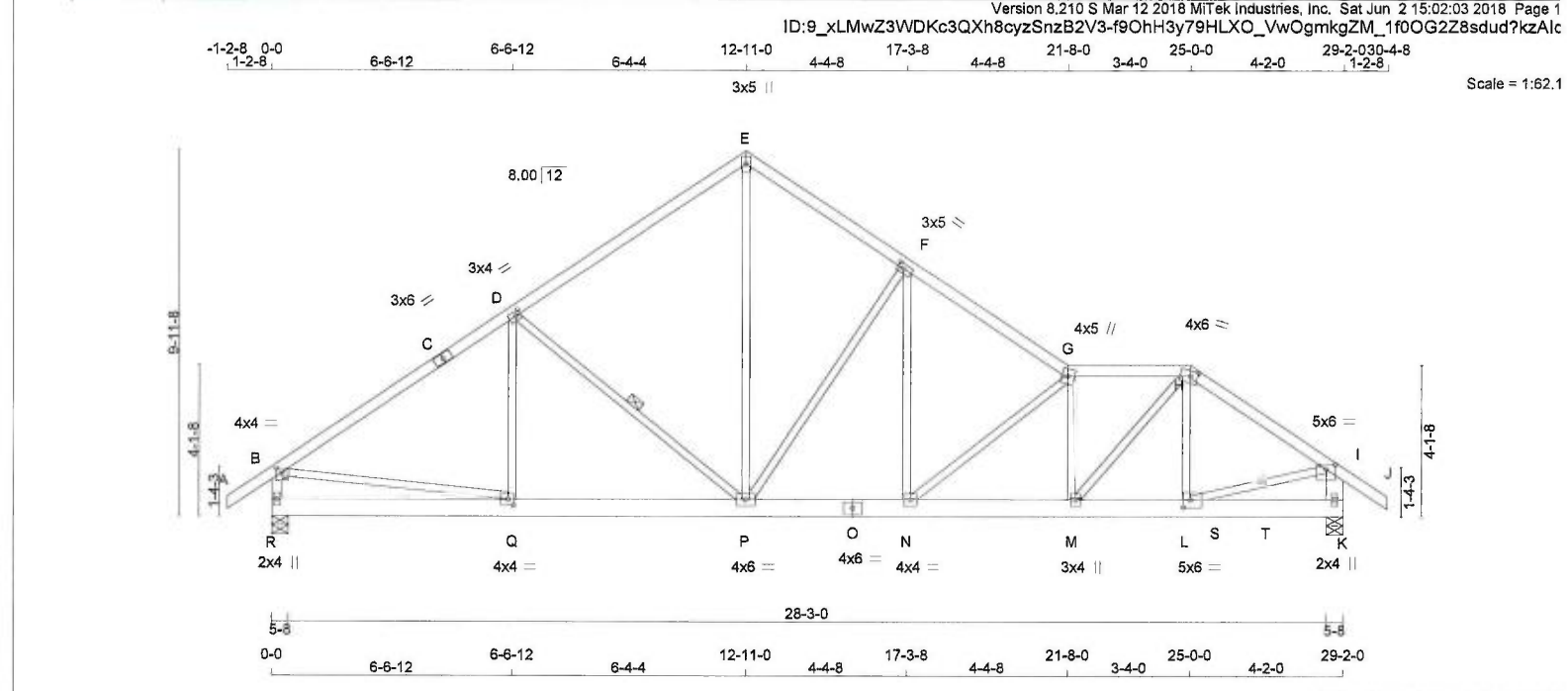
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN YORK HOMES - GRANELLI HOME CORP - LIANA 3 ELEV 2	DRWG NO.
NE0618-006	G02	1	1	TRUSS DESC.		PAGE 6 OF 42

Version 8.210 S Mar 12 2018 MiTek Industries, Inc. Sat Jun 2 15:02:03 2018 Page 2
ID:9_xLMwZ3WDKc3QXh8cyzSnzB2V3-f9OhH3y79HLXO_VwOgmkgZMyZfvCG528sdud?kzAlc

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP= 0.90 (Q) (INPUT = 0.90)
JSI METAL= 0.90 (N) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.





8/1/2018 10:00 AM 2 X 149 = 297 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2 SPF
C - E	2x4	DRY	No.2 SPF
E - G	2x4	DRY	No.2 SPF
G - H	2x4	DRY	No.2 SPF
H - J	2x4	DRY	No.2 SPF
R - B	2x4	DRY	No.2 SPF
K - I	2x6	DRY	No.2 SPF
R - O	2x6	DRY	No.2 SPF
O - K	2x6	DRY	No.2 SPF
ALL WEBS EXCEPT	2x3	DRY	No.2 SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C	12	TOP
C-E	12	TOP
E-G	12	TOP
G-H	12	TOP
H-J	12	TOP
R-B	12	TOP
K-I	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
R-O	12	TOP
O-K	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG		
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX		
R	1744	0	1744	0	0	5-8	5-8		
K	3460	0	3460	0	0	5-8	5-8		
UNFACTORED REACTIONS									
JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS						
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
R	1223	861 / 0	0 / 0	0 / 0	0 / 0	362 / 0	0 / 0		
K	2429	1697 / 0	0 / 0	0 / 0	0 / 0	732 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.68 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF D-P. DBS = 20-0-0, CBF = 53 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) TO EACH PLY USING (0.122"x3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS			WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 MAX (PLF)	MAX. MEMB. FORCE (LBS)	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO	FR-TO		
A-B	0 / 28	-77.4 -77.4 0.05 (1)	Q-D	-134 / 72	0.03 (1)
B-C	-2032 / 0	-77.4 -77.4 0.36 (1)	D-P	-421 / 0	0.10 (1)
C-D	-2032 / 0	-77.4 -77.4 0.36 (1)	P-E	0 / 1545	0.19 (1)
D-E	-1711 / 0	-77.4 -77.4 0.34 (1)	P-F	-1377 / 0	0.89 (1)
E-F	-1699 / 0	-77.4 -77.4 0.15 (1)	N-F	0 / 1232	0.15 (1)
F-G	-2563 / 0	-77.4 -77.4 0.16 (1)	N-G	-1740 / 0	0.50 (1)
G-H	-3462 / 0	-77.4 -77.4 0.12 (1)	M-G	-558 / 0	0.07 (1)
H-I	-3798 / 0	-77.4 -77.4 0.20 (1)	M-H	0 / 404	0.05 (1)
I-J	0 / 28	-77.4 -77.4 0.05 (1)	L-H	0 / 1519	0.19 (1)
R-B	-1687 / 0	0.0 0.0 0.09 (1)	B-Q	0 / 1736	0.21 (1)
K-I	-3072 / 0	0.0 0.0 0.11 (1)	L-I	0 / 3231	0.40 (1)
R-Q	0 / 0	-18.2 -18.2 0.04 (4)			
Q-P	0 / 1718	-18.2 -18.2 0.14 (1)			
P-O	0 / 2154	-18.2 -18.2 0.17 (1)			
O-N	0 / 2154	-18.2 -18.2 0.17 (1)			
N-M	0 / 3477	-18.2 -18.2 0.30 (1)			
M-L	0 / 3197	-18.2 -18.2 0.46 (1)			
L-S	0 / 0	-18.2 -18.2 0.32 (1)			
S-T	0 / 0	-18.2 -18.2 0.32 (1)			
T-K	0 / 0	-18.2 -18.2 0.32 (1)			

FACTORED CONCENTRATED LOADS (LBS)							
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
S	25-8-8	-1946	-1946	-	FRONT	VERT	TOTAL
T	27-1-4	-270	-270	-	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/999 (0.12")

CSI: TC=0.36/1.00 (B-D:1), BC=0.46/1.00 (L-M:1), WB=0.89/1.00 (F-P:1), SSI=0.63/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

CONTINUED ON PAGE 2

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	4.0	1.25	2.00
C	TS-t	MT20	3.0	6.0		
D	TMVW-t	MT20	3.0	4.0	1.50	1.50
E	TTW+p	MT20	3.0	5.0		
F	TMVW-t	MT20	3.0	5.0	1.50	2.00
G	TTW+m	MT20	4.0	5.0		
H	TTW-m	MT20	4.0	6.0	1.75	2.50
I	TMVW-p	MT20	5.0	6.0	1.50	3.00
K	BMV1+p	MT20	2.0	4.0		
L	BMVW-t	MT20	5.0	6.0	2.50	2.25
M	BMVW+t	MT20	3.0	4.0		
N	BMVW-t	MT20	4.0	4.0		
O	BS-t	MT20	4.0	6.0		
P	BMVW-t	MT20	4.0	6.0		
Q	BMVW-t	MT20	4.0	4.0	2.00	1.75
R	BMV1+p	MT20	2.0	4.0		

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (G) (INPUT = 0.90)

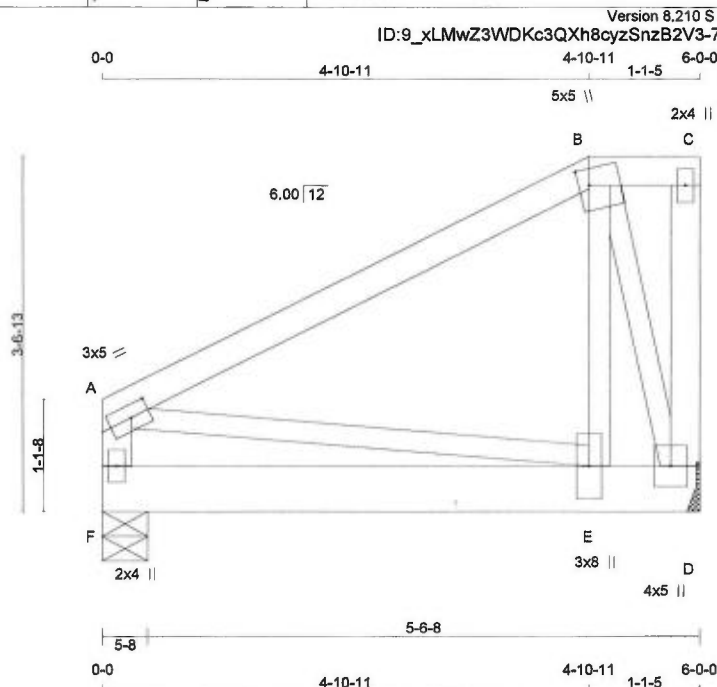
JSI METAL= 0.40 (L) (INPUT = 1.00)

HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1945.7 lbs FACTORED DOWN AT 25'-8"-8, AND 270.4 lbs FACTORED DOWN AT 27'-1"-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

READ ALL NOTES ON THIS PAGE AND ON
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THE DESIGN OF THIS COMPONENT.





Scale = 1:23.2

TOTAL WEIGHT = $2 \times 31 = 61 \text{ lb}$

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY No.2	SPF
B - C	2x4	DRY No.2	SPF
D - C	2x4	DRY No.2	SPF
F - A	2x4	DRY No.2	SPF
F - D	2x6	DRY No.2	SPF

ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
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DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT
SEPARATELY THEN FASTENED TOGETHER AS
FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A- B 1	12	TOP
B- C 1	12	TOP
C- D 1	12	TOP
F- A 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F- D 2	9	SIDE (276.4)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	3.0	5.0	1.50	2.25
B	TTWW+m	MT20	5.0	5.0	2.00	1.25
C	TMV+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
D	1945	0	1945	0	0	MECHANICAL	
F	1945	0	1945	0	0	5-8	5-8

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT D TO RESIST THE MAX. FACTORED REACTIONS.

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	1367	947 / 0	0 / 0	0 / 0	0 / 0	419 / 0	0 / 0
F	1367	947 / 0	0 / 0	0 / 0	0 / 0	419 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX LC1 CSI (LC)		
FR-TO		FROM	TO		FR-TO				
A-B	-932 / 0	-77.4	-77.4	0.18 (1)	6.25	E-B	0 / 2841		
B-C	0 / 0	-77.4	-77.4	0.01 (1)	10.00	B-D	-2811 / 0		
D-C	-43 / 0	0.0	0.0	0.00 (1)	7.81	A-E	0 / 842		
F-A	-729 / 0	0.0	0.0	0.04 (1)	7.81				
F-E	0 / 0	-571.0	-571.0	0.48 (1)	10.00				
E-D	0 / 926	-571.0	-571.0	0.45 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.3	PSF
TOTAL LOAD =		33.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CStdGirder

START DISTANCE = 0-0
START SPAN CARRIED = 25-7-0
END DISTANCE = 6-0-0
END SPAN CARRIED = 25-7-0
END WALL WIDTH = 5-8
APPLIED TO BACK SIDE OF BOTTOM CHORD.
- ADD'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= $L/360$ (0.20")
 CALCULATED VERT. DEFL.(LL) = $L/999$ (0.05")
 ALLOWABLE DEFL.(TL)= $L/360$ (0.20")
 CALCULATED VERT. DEFL.(TL) = $L/822$ (0.09")

CSI: TC=0.18/1.00 (A-B:1), BC=0.48/1.00 (E-F:1),
WB=0.35/1.00 (B-E:1), SSI=0.54/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

CONTINUED ON PAGE 2



**READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
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SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.**



PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
D	BMVW1+p	MT20	4.0	5.0		
E	BMWW+t	MT20	3.0	8.0		
F	BMV1+p	MT20	2.0	4.0		

PLATE PLACEMENT TOL. = 0.250 inches

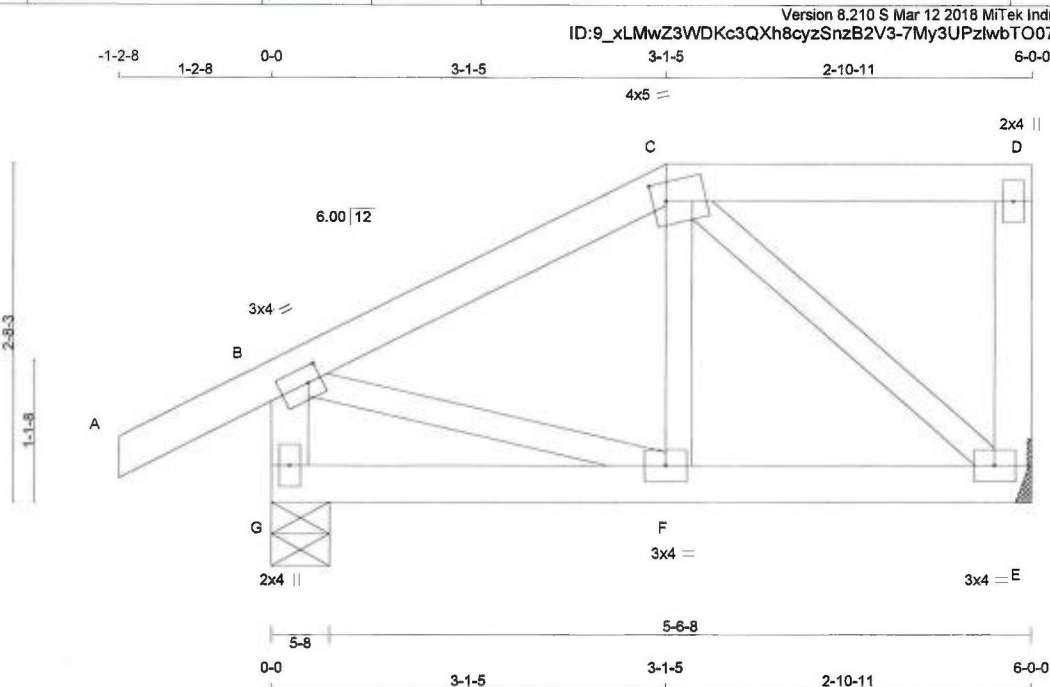
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (E) (INPUT = 0.90)

JSI METAL= 0.39 (B) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
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TOTAL WEIGHT = 26 lb [M]

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-I	MT20	3.0	4.0	1.50	1.25
C	TTVW-m	MT20	4.0	5.0	1.75	1.25
D	TMV+p	MT20	2.0	4.0		
E	BMVW1-t	MT20	3.0	4.0		
F	BMVW-I	MT20	3.0	4.0		
G	BMV1+p	MT20	2.0	4.0		

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 20.0 lbs FACTORED DOWN AT 3-1-5 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX		
E	272	0	272	0	0	MECHANICAL	5-8	5-8	
G	383	0	383	0	0	5-8	5-8		

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT E TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	190	134 / 0	0 / 0	0 / 0	0 / 0	56 / 0	0 / 0
G	266	201 / 0	0 / 0	0 / 0	0 / 0	65 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	UNBRACED LENGTH
FR-TO		FROM	TO	FR-TO			
A-B	0 / 22	-77.4	-77.4 0.10 (1)	10.00	F-C	0 / 52	0.02 (4)
B-C	-209 / 0	-77.4	-77.4 0.14 (1)	6.25	C-E	-243 / 0	0.06 (1)
C-D	0 / 0	-68.5	-68.5 0.11 (1)	10.00	B-F	0 / 193	0.05 (1)
E-D	-99 / 0	0.0	0.0 0.01 (1)	7.81			
G-B	-360 / 0	0.0	0.0 0.04 (1)	7.81			
G-F	0 / 0	-16.2	-16.2 0.04 (4)	10.00			
F-E	0 / 187	-16.2	-16.2 0.06 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	3-1-5	-20	-20	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 3-1-5
RIGHT SETBACK = 0-0
END SETBACK = 2-0-0
END WALL WIDTH = 5-8
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDTL LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.14/1.00 (B-C:1), BC=0.06/1.00 (E-F:4), WB=0.06/1.00 (C-E:1), SSI=0.09/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

CONTINUED ON PAGE 2



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



PLATE PLACEMENT TOL. = 0.250 inches

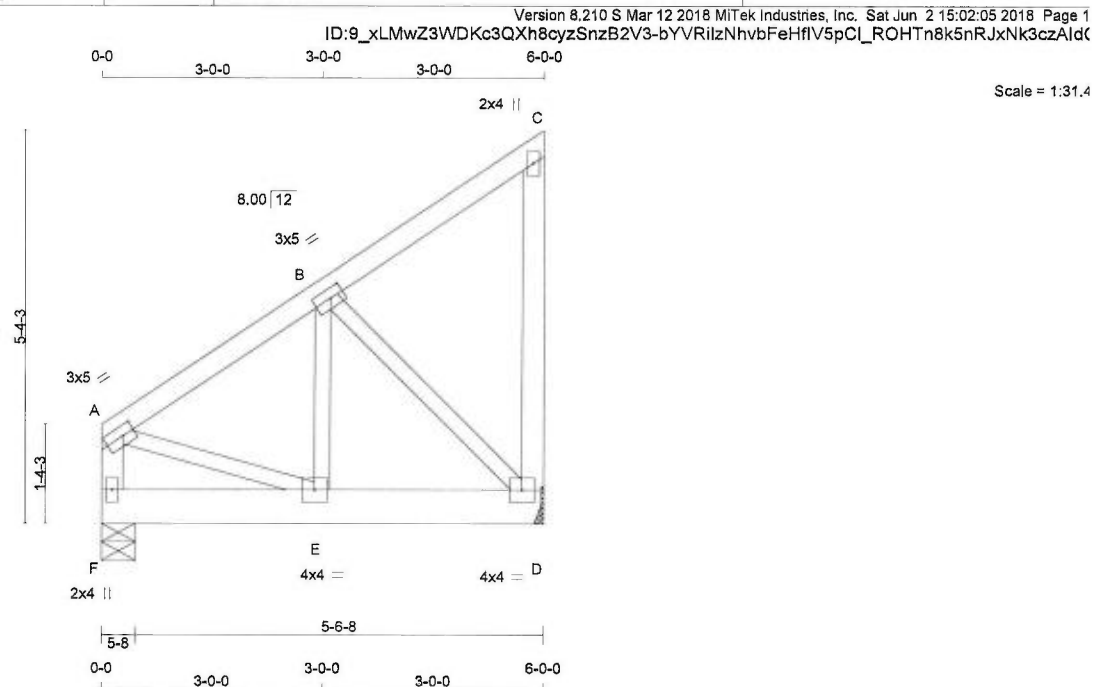
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.49 (B) (INPUT = 0.90)

JSI METAL= 0.12 (B) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 33 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
D - C	2x4	DRY No.2	SPF
F - A	2x4	DRY No.2	SPF
F - D	2x6	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
D	855	0	855	0	5-8	5-8
F	855	0	855	0	5-8	5-8

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT D TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
D	600	416 / 0	0 / 0	0 / 0	0 / 0	184 / 0	0 / 0
F	600	416 / 0	0 / 0	0 / 0	0 / 0	184 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS					
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED		MAX CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)			
		VERT. LOAD (PLF)	LC1								
FR-TO		FROM	TO			FR-TO					
A-B	-605 / 0	-77.4	-77.4	0.12 (1)	6.25	E-B	0 / 565	0.14 (1)			
B-C	-14 / 0	-77.4	-77.4	0.12 (1)	6.25	B-D	-721 / 0	0.21 (1)			
D-C	-90 / 0	0.0	0.0	0.04 (1)	7.81	A-E	0 / 540	0.13 (1)			
F-A	-596 / 0	0.0	0.0	0.07 (1)	7.81						
F-E	0 / 0	-207.5	-207.5	0.12 (1)	10.00						
E-D	0 / 515	-207.5	-207.5	0.19 (1)	10.00						

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 10-4-8
END DISTANCE = 6-0-0
END SPAN CARRIED = 10-4-8
APPLIED TO BACK SIDE OF BOTTOM CHORD.
- ADD'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.12/1.00 (A-B:1), BC=0.19/1.00 (D-E:1), WB=0.21/1.00 (B-D:1), SSI=0.24/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2



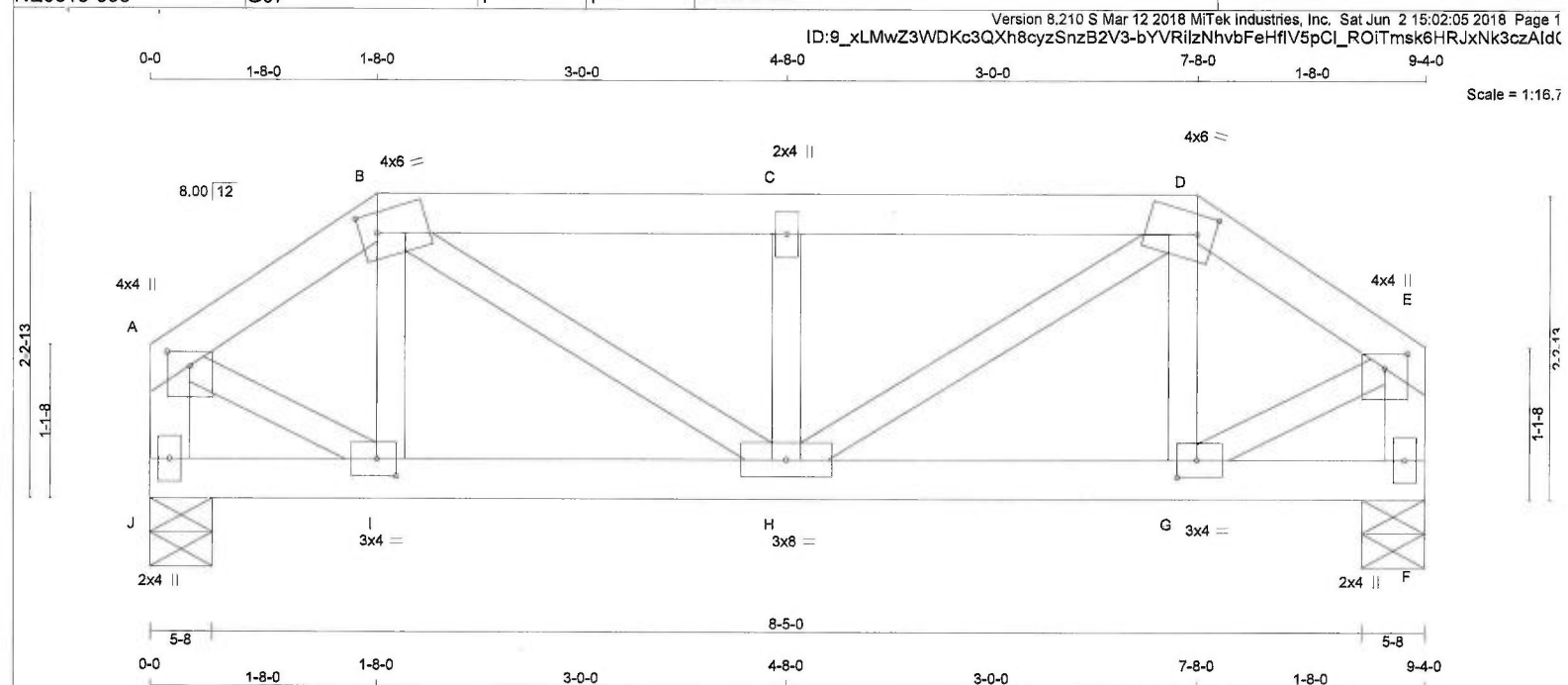
READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
NE0618-006	G06	1	1	GREEN YORK HOMES - GRANELLI HOME CORP - LIANA 3 ELEV 2	PAGE 14 OF 42
Version 8.210 S Mar 12 2018 MiTek Industries, Inc. Sat Jun 2 15:02:05 2018 Page 2					
ID:9_xLMwZ3WDKc3QXh8cyzSnzB2V3-bYYVRilzNhvbFeHflV5pCI_ROHTn8k5nRjXNk3czAldC					
TRUSS DESC.					

JSI GRIP= 0.85 (B) (INPUT = 0.90)
JSI METAL= 0.20 (A) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.





LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2 SPF
B - D	2x4	DRY	No.2 SPF
D - E	2x4	DRY	No.2 SPF
J - A	2x4	DRY	No.2 SPF
F - E	2x4	DRY	No.2 SPF
J - F	2x4	DRY	No.2 SPF

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
B	TTWW+m	MT20	4.0	6.0	1.75	1.50
C	TMW+w	MT20	2.0	4.0		
D	TTWW+m	MT20	4.0	6.0	1.75	1.50
E	TMVW+p	MT20	4.0	4.0	1.25	2.00
F	BMV1+p	MT20	2.0	4.0		
G	BMWW-t	MT20	3.0	4.0	1.50	1.75
H	BMWW-t	MT20	3.0	8.0		
I	BMWW-t	MT20	3.0	4.0	1.50	1.75
J	BMV1+p	MT20	2.0	4.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 10.7 lbs FACTORED DOWN AT 7-8-0, AND 10.7 lbs FACTORED DOWN AT 1-8-0 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
J	853	0	853	0
F	853	0	853	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN.	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
J	598	418 / 0	0 / 0
F	598	418 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.98 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 MAX. CSI (LC)	UNBRACED LENGTH
FR-TO				FR-TO			
A-B	-775 / 0	-77.4	-77.4 0.05 (1)	I-B	-5 / 32	0.01 (4)	6.25
B-C	-1066 / 0	-68.5	-68.5 0.10 (1)	B-H	0 / 505	0.12 (1)	5.98
C-D	-1066 / 0	-68.5	-68.5 0.10 (1)	H-C	-230 / 0	0.04 (1)	5.98
D-E	-776 / 0	-77.4	-77.4 0.05 (1)	H-D	0 / 504	0.12 (1)	6.25
J-A	-790 / 0	0.0	0.0 0.09 (1)	G-D	-4 / 32	0.01 (4)	7.81
F-E	-790 / 0	0.0	0.0 0.09 (1)	A-I	0 / 705	0.17 (1)	7.81
J-I	0 / 0	-108.8	-108.8 0.09 (1)	G-E	0 / 706	0.17 (1)	10.00
I-H	0 / 642	-108.8	-108.8 0.20 (1)				10.00
H-G	0 / 642	-108.8	-108.8 0.20 (1)				10.00
G-F	0 / 0	-108.8	-108.8 0.09 (1)				10.00

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
B	1-8-0	-11	-11	—	FRONT	VERT	TOTAL
D	7-8-0	-11	-11	—	FRONT	VERT	TOTAL

TOTAL WEIGHT = 36 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	PSF
LL	23.3	PSF
BOT CH.	LL	PSF
LL	0.0	PSF
LL	7.3	PSF
TOTAL LOAD	33.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip

SIDE SETBACK = 1-8-0

END SETBACK = 2-0-0

END WALL WIDTH = 5-8

CORNER FRAMING TYPE: CONVENTIONAL

END JACK TYPE: CONVENTIONAL

APPLIED TO FRONT SIDE

- ADDTL LOADS BASED ON 55 % OF GSL.

GIRDER TYPE: CStdGirder

START DISTANCE = 0-0

START SPAN CARRIED = 5-10-8

END DISTANCE = 9-4-0

END SPAN CARRIED = 5-10-8

END WALL WIDTH = 0-0

APPLIED TO BACK SIDE OF BOTTOM CHORD.

- ADDTL LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.31")

CALCULATED VERT. DEFL.(LL) = L/999 (0.02")

ALLOWABLE DEFL.(TL)= L/360 (0.31")

CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.10/1.00 (B-C:1), BC=0.20/1.00 (G-H:1), WB=0.17/1.00 (E-G:1), SSI=0.14/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00

COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

CONTINUED ON PAGE 2



TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	618	354	1667
		822	2284
			1656

PLATE PLACEMENT TOL. = 0.250 inches

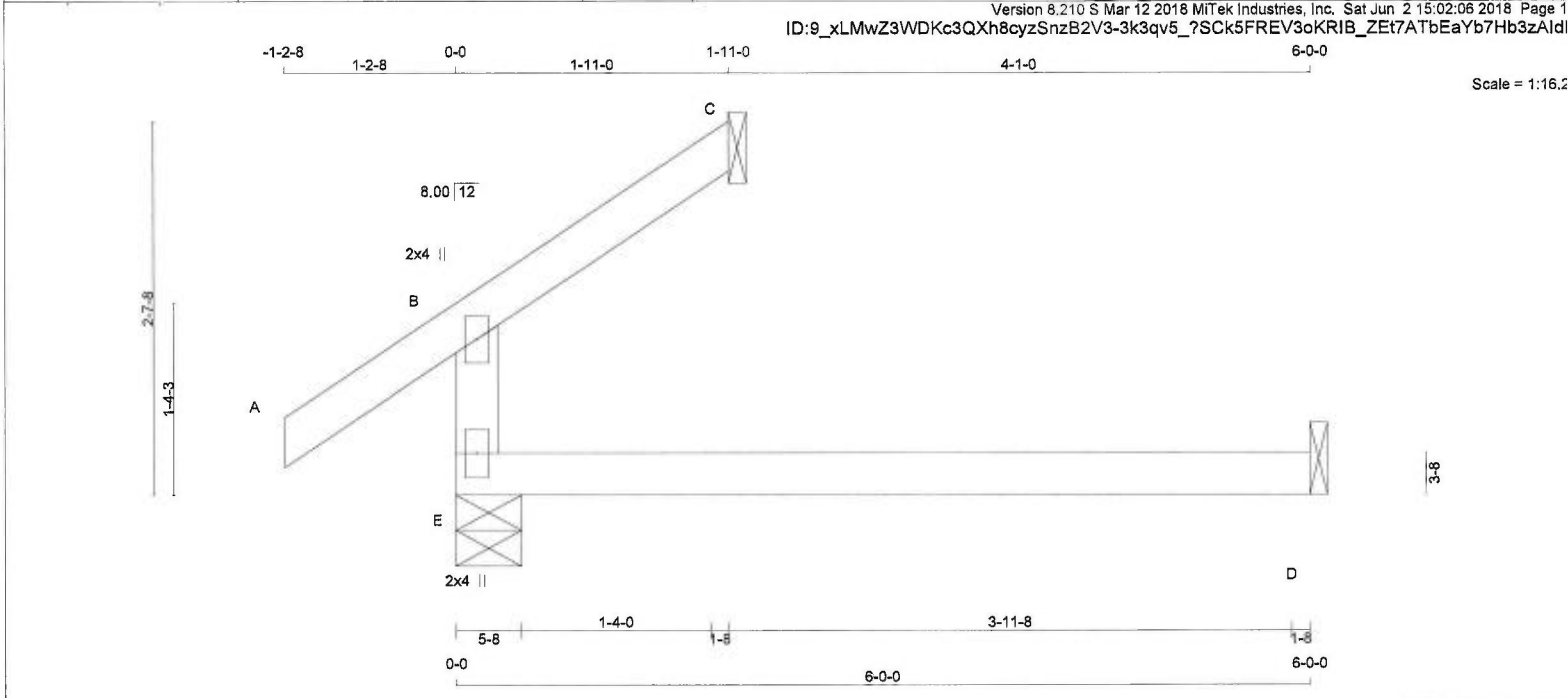
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (D) (INPUT = 0.90)

JSI METAL= 0.24 (I) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER
E - B 2x4 DRY No.2
A - C 2x4 DRY No.2
E - D 2x4 DRY No.2

DESCR.
SPF
SPF
SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)
JT TYPE PLATES W LEN Y X
B TMV+p MT20 2.0 4.0
E BMV1+p MT20 2.0 4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG	
	VERT	HORZ	DOWN	HORZ		IN-SX	IN-SX
E	256	0	256	0	0	5-8	5-8
C	56	0	56	0	0	1-8	1-8
D	46	0	51	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C , D
UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS				WIND	DEAD	SOIL
		SNOW	LIVE	PERM.LIVE				
E	181	115 / 0	0 / 0	0 / 0	0 / 0	66 / 0	0 / 0	
C	38	34 / 0	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0	
D	37	0 / 0	0 / 0	0 / 0	0 / 0	37 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E , C
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.
LOADING
TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		WEBS		FACTORED	
	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FORCE (LBS)	MAX
E-B	-192 / 0	0.0	0.0	0.11 (4)	7.81			
A-B	0 / 28	-77.4	-77.4	0.09 (1)	10.00			
B-C	-10 / 0	-77.4	-77.4	0.04 (1)	10.00			
E-D	0 / 0	-18.2	-18.2	0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.11/1.00 (B-E/4) , BC=0.13/1.00 (D-E/4) , WB=0.00/1.00 (n/a:0) , SSI=0.09/1.00 (D-E/4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

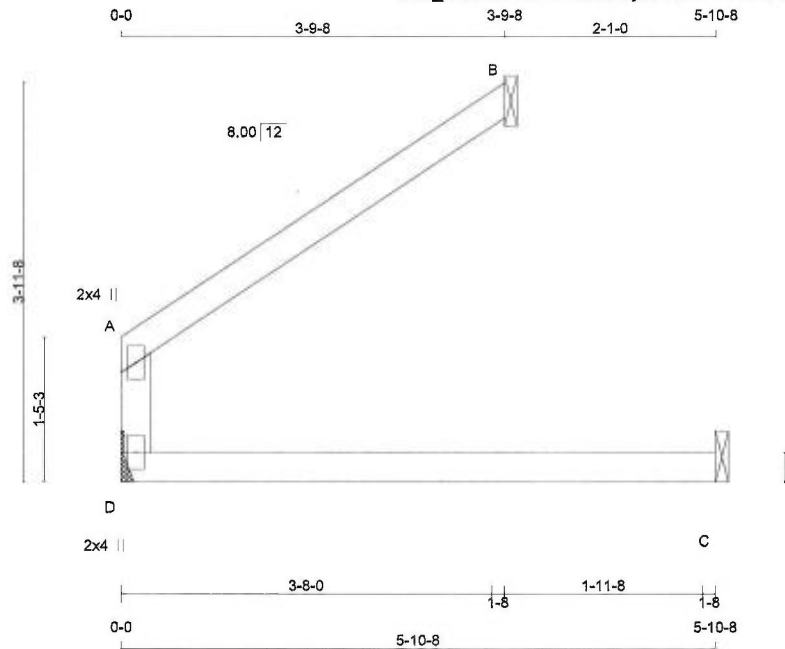
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.12 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Scale = 1:22.5



TOTAL WEIGHT = 13 lb [M]F

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
D - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0		
D	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ
D	201	0	201	0
B	143	0	143	0
C	56	0	56	0

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT D TO RESIST THE MAX FACTORED REACTIONS.

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) B, C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. SNOW	MIN. LIVE	PERM. LIVE	WIND	DEAD	SOIL
D	143	91 / 0	0 / 0	0 / 0	0 / 0	52 / 0	0 / 0
B	98	81 / 0	0 / 0	0 / 0	0 / 0	18 / 0	0 / 0
C	44	5 / 0	0 / 0	0 / 0	0 / 0	39 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED HORIZ. LOAD (LC)	UNBRACED LENGTH
FR-TO						
D-A		-150 / 0	0.0	0.0	0.06 (4)	7.81
A-B		-2 / 5	-77.4	-77.4	0.17 (1)	10.00
D-C		0 / 0	-18.2	-18.2	0.15 (4)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.06")

CSI: TC=0.17/1.00 (A-B:1), BC=0.15/1.00 (C-D:4), WB=0.00/1.00 (n/a:0), SSI=0.10/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

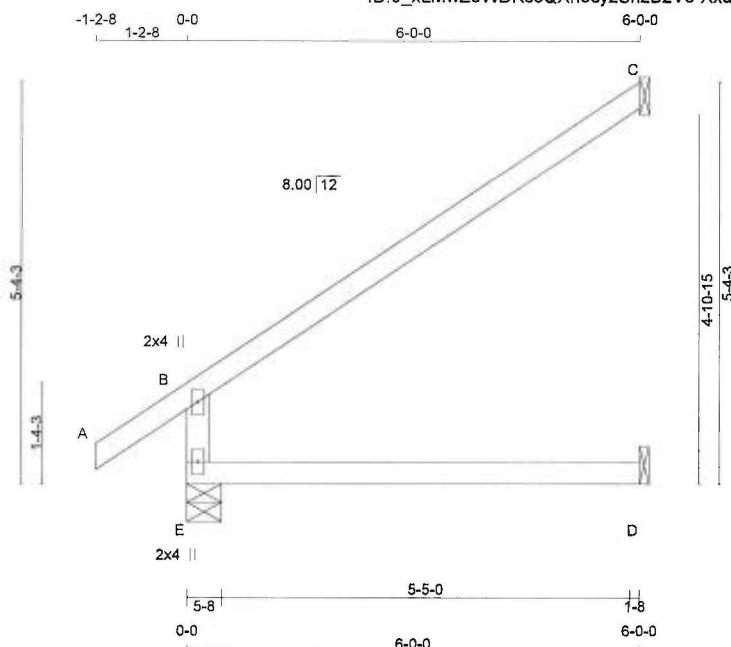
JSI GRIP= 0.10 (A) (INPUT = 0.90)
JSI METAL= 0.04 (A) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



Scale = 1:30.0



TOTAL WEIGHT = 2 X 18 = 36 lb [M/JF]

LUMBER					DESCR.
N. L. G. A. RULES	CHORDS	SIZE	LUMBER		
E - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	2.0	4.0	
E	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	454	0	454	0	5-8	5-8
C	174	0	174	0	1-8	1-8
D	46	0	51	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LOASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
E	316	235 / 0	0 / 0	0 / 0	0 / 0	81 / 0	0 / 0
C	118	105 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0
D	37	0 / 0	0 / 0	0 / 0	0 / 0	37 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		WEBS		FACTORED	
	MAX. FORCE (LBS)	VERT. (PLF)	LOAD LC1	MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FORCE (LBS)	MAX. CSI (LC)
E-B	-390 / 0	0.0	0.0	0.11 (4)	7.81			
A-B	0 / 28	-77.4	-77.4	0.09 (1)	10.00			
B-C	-32 / 0	-77.4	-77.4	0.36 (1)	6.25			
E-D	0 / 0	-18.2	-18.2	0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
	DL = 3.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.3	PSF
TOTAL LOAD	= 33.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.36/1.00 (B-C:1), BC=0.13/1.00 (D-E:4),
WB=0.00/1.00 (n/a:0), SS=0.19/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

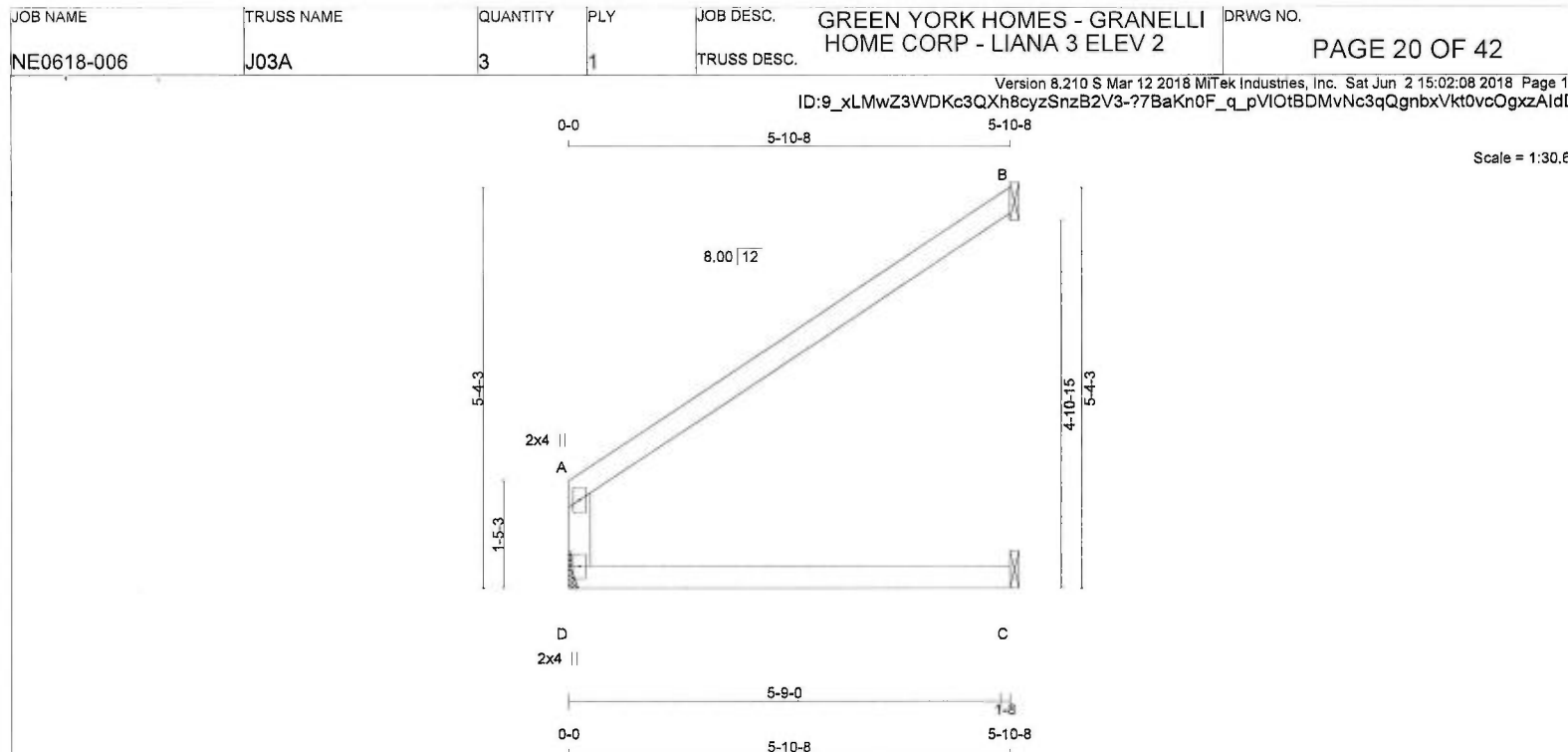
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (B) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
D - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0		
D	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
D	281	0	281	0	MECHANICAL	
B	207	0	207	0	1-8	1-8
C	73	0	73	0	1-8	1-8

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT D TO RESIST THE MAX FACTORED REACTIONS.

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) B, C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	197	137 / 0	0 / 0	0 / 0	0 / 0	61 / 0	0 / 0
B	142	122 / 0	0 / 0	0 / 0	0 / 0	20 / 0	0 / 0
C	56	15 / 0	0 / 0	0 / 0	0 / 0	41 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. FACTORED FORCE (LBS)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO	CSI (LC)		FR-TO		CSI (LC)
D- A	-247 / 0	0.0	0.0	0.14 (1)	7.81		
A- B	-11 / 2	-77.4	-77.4	0.39 (1)	6.25		
D- C	0 / 0	-18.2	-18.2	0.20 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/662 (0.11")

CSI: TC=0.39/1.00 (A-B:1), BC=0.20/1.00 (C-D:1), WB=0.00/1.00 (n/a:0), SSI=0.16/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

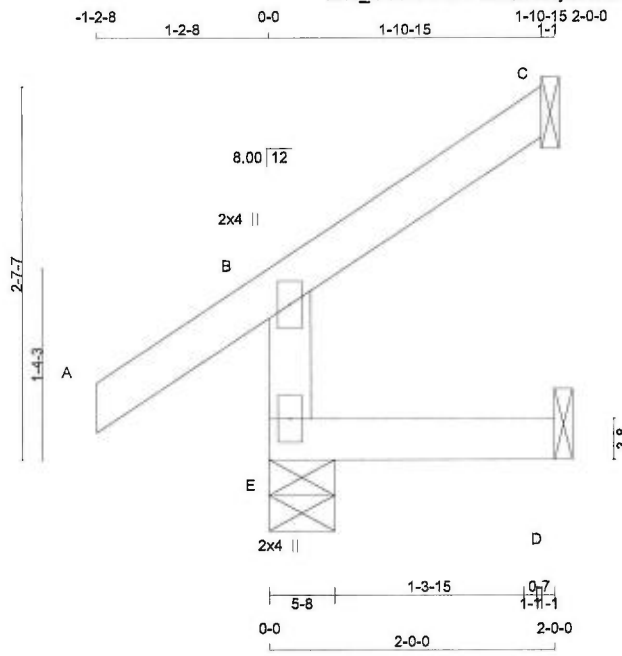
JSI GRIP= 0.16 (A) (INPUT = 0.90)

JSI METAL= 0.06 (A) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:16.2

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	2.0	4.0
E	BMV1+p	MT20	2.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	212	0	212	0	5-8	5-8
C	56	0	56	0	1-8	1-8
D	16	0	18	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

JT	UNFACTORED REACTIONS		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	1ST LCASE	SNOW	LIVE	PERM. LIVE			
E	146	115 / 0	0 / 0	0 / 0	0 / 0	31 / 0	0 / 0
C	38	34 / 0	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	13	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

MEMB.	CHORDS		FACTORED		WEBS		FACTORED	
	MAX. FORCE (LBS)	VERT. (PLF)	LOAD LC1	MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FORCE (LBS)	MAX. CSI (LC)
E-B	-192 / 0	0.0	0.0	0.01 (4)	7.81			
A-B	0 / 28	-77.4	-77.4	0.09 (1)	10.00			
B-C	-10 / 0	-77.4	-77.4	0.05 (1)	10.00			
E-D	0 / 0	-18.2	-18.2	0.02 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 8 lb [M]

DESIGN CRITERIA		
SPECIFIED LOADS:		
TOP CH.	LL = 23.3	PSF
	DL = 3.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.3	PSF
TOTAL LOAD	= 33.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.09/1.00 (A-B:1), BC=0.02/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.06/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY)	SHEAR	SECTION	(PSI)	(PLI)
	MAX	MIN	MAX	MIN	MAX
MT20	618	354	1667	822	2284

PLATE PLACEMENT TOL. = 0.250 inches

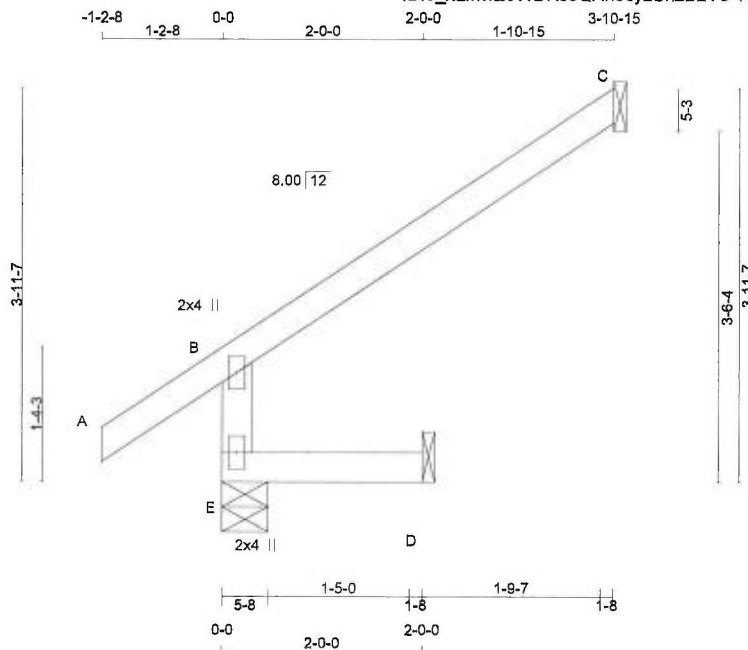
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.12 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 11 lb
 [M]

LUMBER				DESCR.	
N. L. G. A. RULES	SIZE	LUMBER		SPF	
CHORDS				SPF	
E - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	2.0	4.0	
E	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
E	309	0	309	0	5-8	5-8
C	114	0	114	0	1-8	1-8
D	16	0	16	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
		COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
E	212	174 / 0	0 / 0	0 / 0	0 / 0	0 / 0	38 / 0
C	77	68 / 0	0 / 0	0 / 0	0 / 0	0 / 0	9 / 0
D	13	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	FR-TO
FR-TO							
E-B	-289 / 0	0.0	0.0 (4)	7.81			
A-B	0 / 28	-77.4	-77.4 (1)	10.00			
B-C	-21 / 0	-77.4	-77.4 (1)	6.25			
E-D	0 / 0	-18.2	-18.2 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 23.3 PSF
 DL = 3.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.3 PSF
 TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 088-09
 - TPIC 2011

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
 ALLOWABLE DEFL.(TL)= L/360 (0.19")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.20/1.00 (B-C:1), BC=0.02/1.00 (D-E:4),
 WB=0.00/1.00 (n/a:0), SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY)	SHEAR	SECTION		
(PSI)	(PLI)	(PLI)	(PSI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284

PLATE PLACEMENT TOL. = 0.250 inches

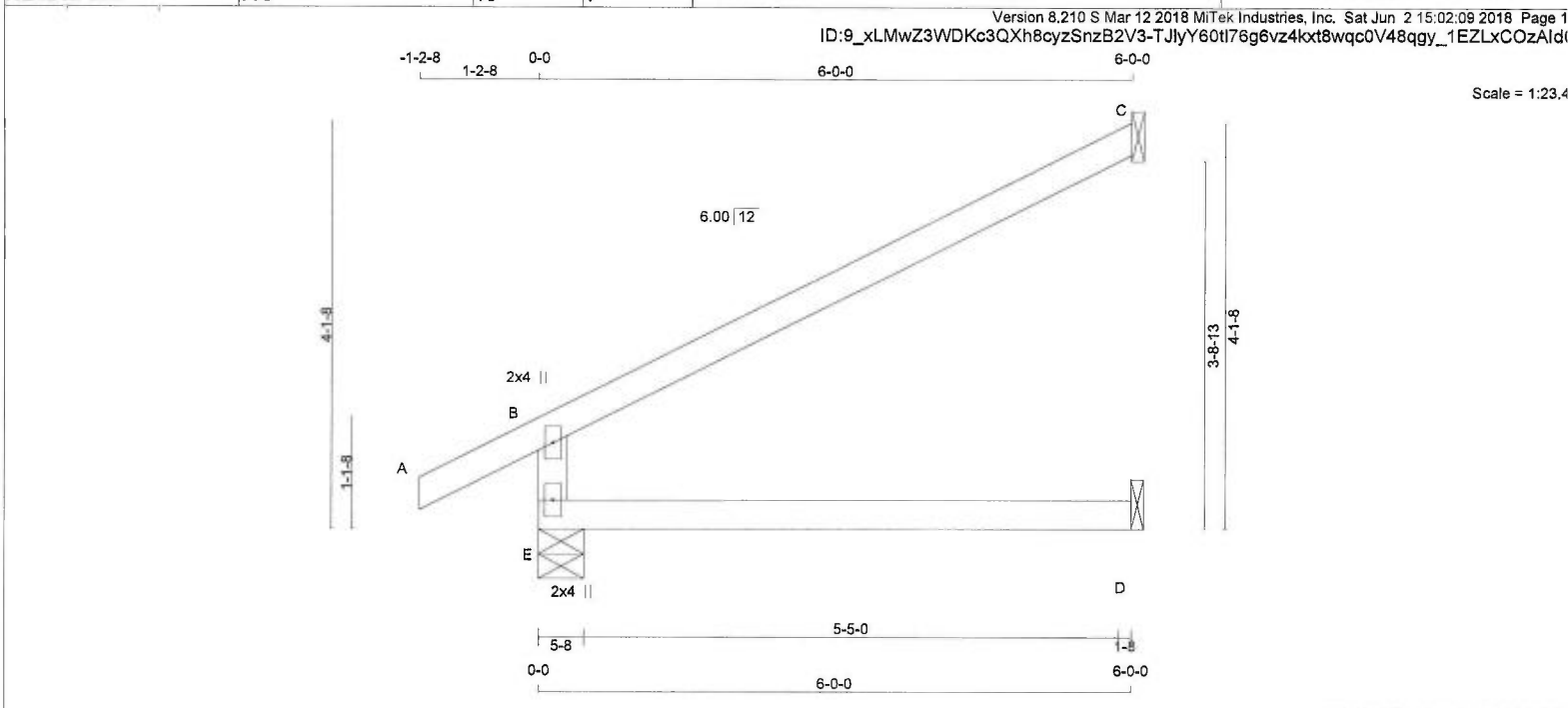
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (B) (INPUT = 0.90)
 JSI METAL= 0.07 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL HEIGHT = 16 X 17 = 271 IN
[M]F

LUMBER					
N. L. G. A. RULES					
CHORDS	SIZE	LUMBER		DESCR.	
E - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER							
BEARINGS							
	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	453	0	453	0	0	5-8	5-8
C	174	0	174	0	0	1-8	1-8
D	45	0	50	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS							
	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	316	234 / 0	0 / 0	0 / 0	0 / 0	82 / 0	0 / 0
C	118	105 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING							
TOTAL LOAD CASES: (4)							
CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX (LC)
FR-TO		FROM	TO		FR-TO		
E-B	-389 / 0	0.0	0.0	0.11 (4)	7.81		
A-B	0 / 22	-77.4	-77.4	0.09 (1)	10.00		
B-C	-26 / 0	-77.4	-77.4	0.36 (1)	6.25		
E-D	0 / 0	-18.2	-18.2	0.14 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.36/1.00 (B-C:1), BC=0.14/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

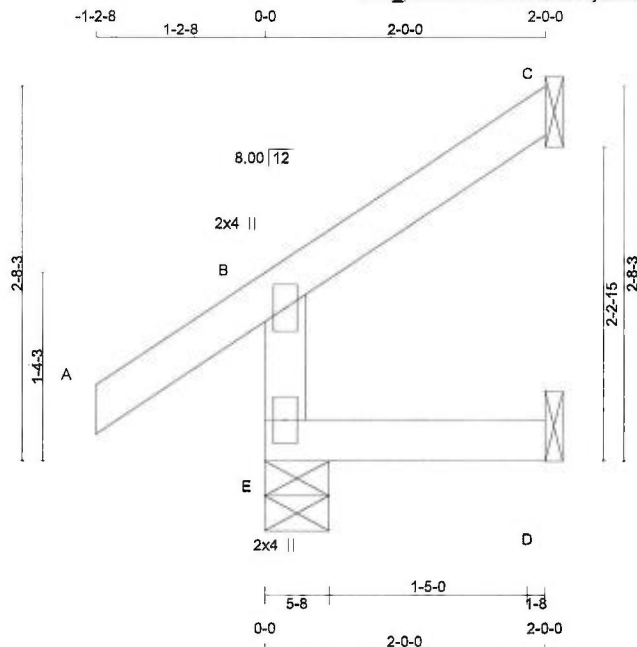
JSI GRIP= 0.22 (B) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



READ ALL NOTES ON THIS PAGE AND ON
 ENGINEERING NOTE PAGE ENP-1. THIS
 NOTE PAGE IS AN INTEGRAL PART OF
 THIS DRAWING AS IT CONTAINS
 SPECIFICATIONS AND CRITERIA USED IN
 THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 2 X 8 = 16 lb

LUMBER				
N. L. G. A. RULES	SIZE	DRY	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	2.0	4.0
E	BMV1+p	MT20	2.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
E	216	0	216	0	5-8	5-8
C	58	0	58	0	1-8	1-8
D	16	0	18	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
E	149	118 / 0	0 / 0	0 / 0	0 / 0	31 / 0	0 / 0
C	40	35 / 0	0 / 0	0 / 0	0 / 0	5 / 0	0 / 0
D	13	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED UNBRACED LENGTH (FT)	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO		
E-B	-196 / 0	0.0 0.0 0.01 (4)	7.81	
A-B	0 / 28	-77.4 -77.4 0.09 (1)	10.00	
B-C	-10 / 0	-77.4 -77.4 0.05 (1)	6.25	
E-D	0 / 0	-18.2 -18.2 0.02 (4)	10.00	

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.09/1.00 (A-B:1), BC=0.02/1.00 (D-E:4),
WB=0.00/1.00 (n/a:0), SSI=0.06/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES				
PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	MAX MIN	MAX MIN
MT20	618	354	1667	822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

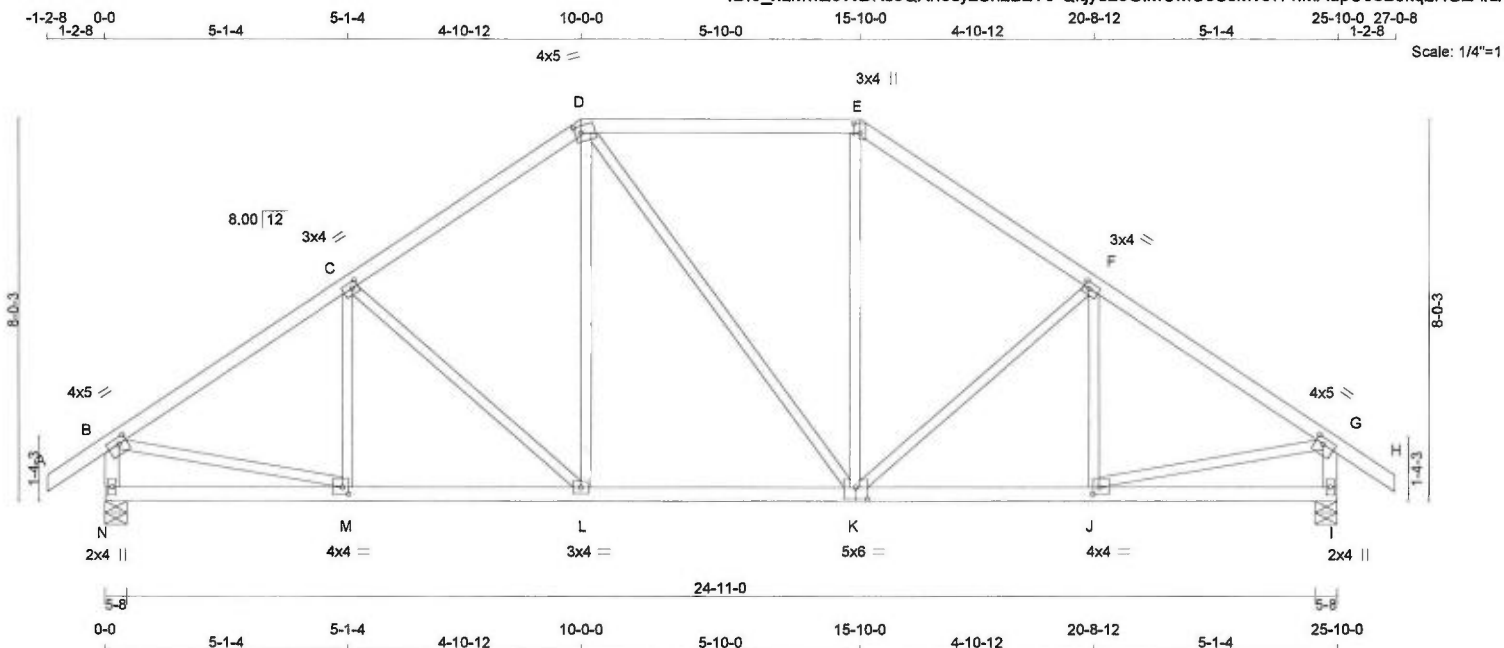
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 114 lb
[M]F

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-I	MT20	4.0	5.0	1.50	2.00
C	TMWW-I	MT20	3.0	4.0	1.50	1.50
D	TTWW-m	MT20	4.0	5.0	1.75	1.50
E	TTWW-p	MT20	3.0	4.0	2.50	1.50
F	TMWW-t	MT20	3.0	4.0	1.50	1.50
G	TMWW-t	MT20	4.0	5.0	1.50	2.00
I	BMV1-p	MT20	2.0	4.0		
J	BMWW-I	MT20	4.0	4.0	1.75	1.50
K	BSWWW-I	MT20	5.0	6.0	3.00	3.00
L	BMWW-I	MT20	3.0	4.0		
M	BMWW-t	MT20	4.0	4.0	1.75	1.50
N	BMV1-p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
N	1335	0	1335	0
I	1335	0	1335	0

UNFACTORED REACTIONS

JT	1ST CASE	MAX./MIN. SNOW	MAX./MIN. LIVE	MAX./MIN. PERM. LIVE	MAX./MIN. WIND	MAX./MIN. DEAD	MAX./MIN. SOIL
N	935	662 / 0	0 / 0	0 / 0	0 / 0	274 / 0	0 / 0
I	935	662 / 0	0 / 0	0 / 0	0 / 0	274 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.19 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
FR-TO					FR-TO			
A-B	0 / 28	-77.4	-77.4	0.09 (1)	10.00	M-C	-144 / 41	0.05 (1)
B-C	-1419 / 0	-77.4	-77.4	0.28 (1)	5.19	C-L	-291 / 0	0.24 (1)
C-D	-1211 / 0	-77.4	-77.4	0.27 (1)	5.52	L-D	0 / 291	0.07 (1)
D-E	-988 / 0	-77.4	-77.4	0.35 (1)	5.82	D-K	0 / 0	0.00 (1)
E-F	-1211 / 0	-77.4	-77.4	0.26 (1)	5.52	K-E	0 / 291	0.07 (1)
F-G	-1418 / 0	-77.4	-77.4	0.28 (1)	5.19	K-F	-289 / 0	0.24 (1)
G-H	0 / 28	-77.4	-77.4	0.09 (1)	10.00	J-F	-145 / 40	0.05 (1)
H-I	-1295 / 0	0.0	0.0	0.13 (1)	7.10	B-M	0 / 1226	0.28 (1)
I-G	-1295 / 0	0.0	0.0	0.13 (1)	7.10	J-G	0 / 1225	0.28 (1)
N-M	0 / 0	-18.2	-18.2	0.10 (4)	10.00			
M-L	0 / 1202	-18.2	-18.2	0.25 (1)	10.00			
L-K	0 / 988	-18.2	-18.2	0.22 (1)	10.00			
K-J	0 / 1201	-18.2	-18.2	0.25 (1)	10.00			
J-I	0 / 0	-18.2	-18.2	0.10 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL LOAD		=	33.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.86")
CALCULATED VERT. DEFL.(LL)= L/999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.86")
CALCULATED VERT. DEFL.(TL)= L/999 (0.09")

CSI: TC=0.35/1.00 (D-E:1), BC=0.25/1.00 (J-K:1), WB=0.28/1.00 (B-M:1), SSI=0.18/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

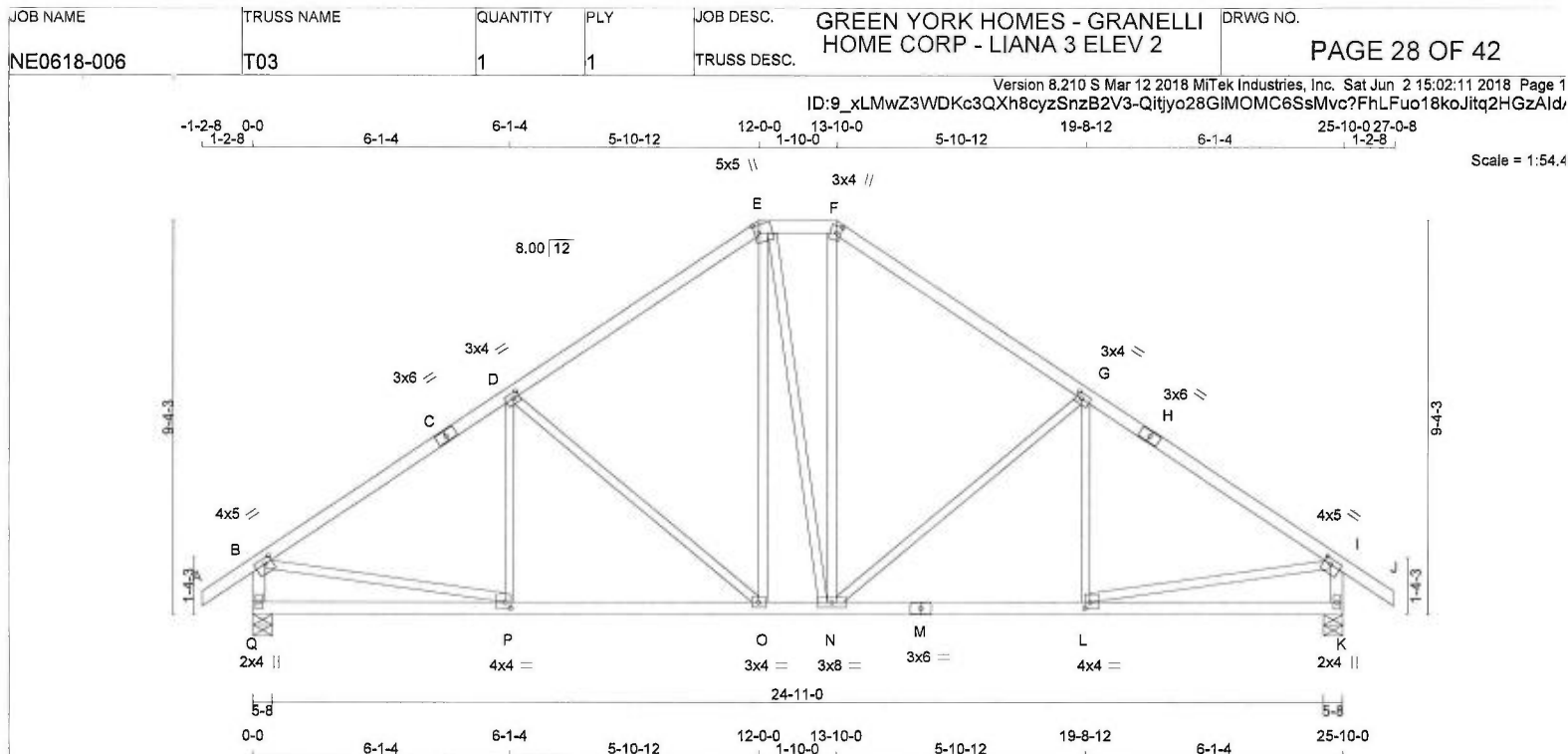
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (B) (INPUT = 0.90)
JSI METAL= 0.47 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER				N. L. G. A. RULES	
CHORDS	SIZE	LUMBER	DESCR.	SPF	SPF
A - C	2x4	DRY	No.2	SPF	SPF
C - E	2x4	DRY	No.2	SPF	SPF
E - F	2x4	DRY	No.2	SPF	SPF
F - H	2x4	DRY	No.2	SPF	SPF
H - J	2x4	DRY	No.2	SPF	SPF
Q - B	2x4	DRY	No.2	SPF	SPF
K - I	2x4	DRY	No.2	SPF	SPF
Q - M	2x4	DRY	No.2	SPF	SPF
M - K	2x4	DRY	No.2	SPF	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-t	MT20	4.0	5.0	1.50 2.00
C	TS-t	MT20	3.0	6.0	
D	TMVW-t	MT20	3.0	4.0	1.50 1.50
E	TTW+m	MT20	5.0	5.0	2.25 1.25
F	TTW+m	MT20	3.0	4.0	2.00 1.25
G	TMVW-t	MT20	3.0	4.0	1.50 1.50
H	TS-t	MT20	3.0	6.0	
I	TMVW-t	MT20	4.0	5.0	1.50 2.00
K	BMV1+p	MT20	2.0	4.0	
L	BMVW-t	MT20	4.0	4.0	1.75 1.50
M	BS-t	MT20	3.0	6.0	
N	BMVW-t	MT20	3.0	8.0	
O	BMVW-t	MT20	3.0	4.0	
P	BMVW-t	MT20	4.0	4.0	1.75 1.50
Q	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
		GROSS REACTION		GROSS REACTION		BRG		BRG	
JT		VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
Q		1335	0	1335	0	0	5-8	5-8	
K		1335	0	1335	0	0	5-8	5-8	

UNFACTORED REACTIONS		1ST LCASE		MAX./MIN. COMPONENT REACTIONS	
JT		COMBINED	SNOW	LIVE	PERM.LIVE
Q		935	662 / 0	0 / 0	0 / 0
K		935	662 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.02 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	MAX. FACTORED (LC)
FR-TO		FROM	TO	FR-TO			
A-B	0 / 28	-77.4	-77.4 0.09 (1)	P-D	-76 / 84	10.00	0.03 (1)
B-C	-1422 / 0	-77.4	-77.4 0.41 (1)	D-O	-435 / 0	5.02	0.56 (1)
C-D	-1422 / 0	-77.4	-77.4 0.41 (1)	O-E	0 / 334	5.02	0.08 (1)
D-E	-1087 / 0	-77.4	-77.4 0.38 (1)	E-N	0 / 111	5.58	0.00 (1)
E-F	-863 / 0	-77.4	-77.4 0.04 (1)	N-F	0 / 346	6.25	0.08 (1)
F-G	-1089 / 0	-77.4	-77.4 0.38 (1)	N-G	-431 / 0	5.58	0.55 (1)
G-H	-1420 / 0	-77.4	-77.4 0.41 (1)	L-G	-80 / 82	5.02	0.04 (1)
H-I	-1420 / 0	-77.4	-77.4 0.41 (1)	B-P	0 / 1226	5.02	0.28 (1)
I-J	0 / 28	-77.4	-77.4 0.09 (1)	L-I	0 / 1224	10.00	0.28 (1)
Q-B	-1290 / 0	0.0	0.0 0.13 (1)			7.10	
K-I	-1289 / 0	0.0	0.0 0.13 (1)			7.10	
Q-P	0 / 0	-18.2	-18.2 0.17 (4)			10.00	
P-O	0 / 1209	-18.2	-18.2 0.28 (1)			10.00	
O-N	0 / 881	-18.2	-18.2 0.20 (1)			10.00	
N-M	0 / 1207	-18.2	-18.2 0.28 (1)			10.00	
M-L	0 / 1207	-18.2	-18.2 0.28 (1)			10.00	
L-K	0 / 0	-18.2	-18.2 0.17 (4)			10.00	

TOTAL WEIGHT = 121 lb [M]F

DESIGN CRITERIA
SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C
LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010
THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.86")
CALCULATED VERT. DEFL.(TL) = L/999 (0.10")

CSI: TC=0.41/1.00 (B-D:1), BC=0.28/1.00 (O-P:1), WB=0.56/1.00 (D-O:1), SS=0.19/1.00 (B-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

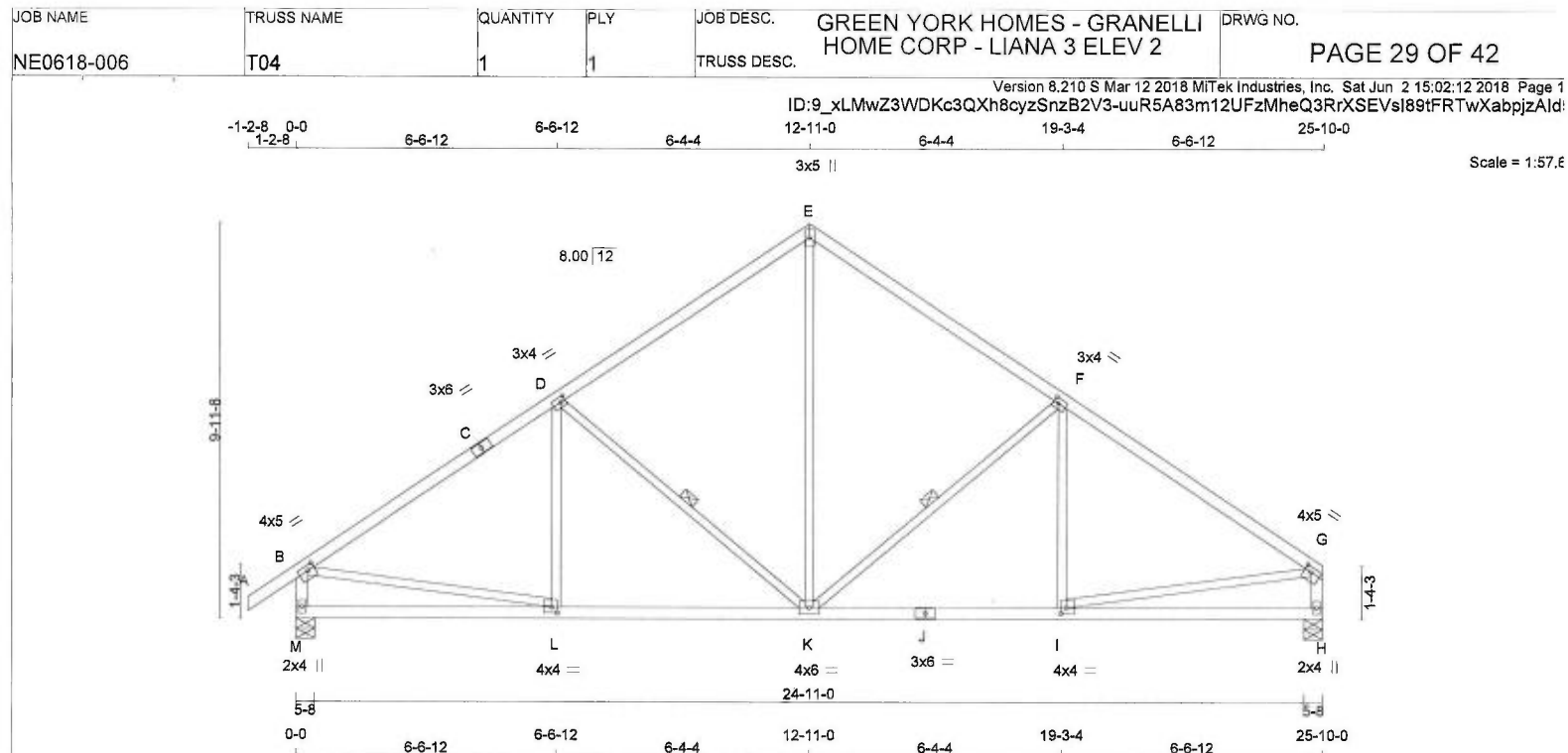
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (F) (INPUT = 0.90)
JSI METAL= 0.48 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 108 lb
[M][F]

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	5.0	1.50	2.00
C	TS-t	MT20	3.0	6.0		
D	TMVW-t	MT20	3.0	4.0	1.50	1.50
E	TTW+p	MT20	3.0	5.0		
F	TMVW-t	MT20	3.0	4.0	1.50	1.50
G	TMVW-t	MT20	4.0	5.0	1.50	2.00
H	BMV1+p	MT20	2.0	4.0		
I	BMVW-t	MT20	4.0	4.0	1.75	1.50
J	BS-t	MT20	3.0	6.0		
K	BMVW-t	MT20	4.0	6.0		
L	BMVW-t	MT20	4.0	4.0	1.75	1.50
M	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
M	1335	0	1335	0
H	1235	0	1235	0

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	935	662 / 0	0 / 0	0 / 0	0 / 0	274 / 0	0 / 0
H	868	602 / 0	0 / 0	0 / 0	0 / 0	266 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.92 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF D-K, F-K, DBS = 20-0-0, CBF = 61 LBS.

DBS = DIAGONAL BRACE SPACING (MAX), CBF = CUMULATIVE BRACING FORCE (PER BRACE), FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	CS (LC)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	CS (LC)
FR-TO						FR-TO				
A-B	0 / 28	-77.4	-77.4	0.09 (1)	10.00	L-D	-57 / 94	0.03 (4)		
B-C	-1415 / 0	-77.4	-77.4	0.48 (1)	4.92	D-K	-488 / 0	0.23 (1)		
C-D	-1415 / 0	-77.4	-77.4	0.48 (1)	4.92	K-E	0 / 743	0.17 (1)		
D-E	-1036 / 0	-77.4	-77.4	0.44 (1)	5.59	K-F	-487 / 0	0.23 (1)		
E-F	-1036 / 0	-77.4	-77.4	0.44 (1)	5.59	I-F	-58 / 94	0.03 (4)		
F-G	-1414 / 0	-77.4	-77.4	0.48 (1)	4.92	B-L	0 / 1219	0.27 (1)		
M-B	-1285 / 0	0.0	0.0	0.13 (1)	7.11	I-G	0 / 1219	0.27 (1)		
H-G	-1186 / 0	0.0	0.0	0.12 (1)	7.33					
M-L	0 / 0	-18.2	-18.2	0.18 (4)	10.00					
L-K	0 / 1205	-18.2	-18.2	0.29 (1)	10.00					
K-J	0 / 1204	-18.2	-18.2	0.29 (1)	10.00					
J-I	0 / 1204	-18.2	-18.2	0.29 (1)	10.00					
I-H	0 / 0	-18.2	-18.2	0.18 (4)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL LOAD	=	33.6	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.86")
CALCULATED VERT. DEFL.(TL) = L/999 (0.10")

CSI: TC=0.48/1.00 (B-D:1), BC=0.29/1.00 (K-L:1), WB=0.27/1.00 (B-L:1), SSI=0.20/1.00 (B-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

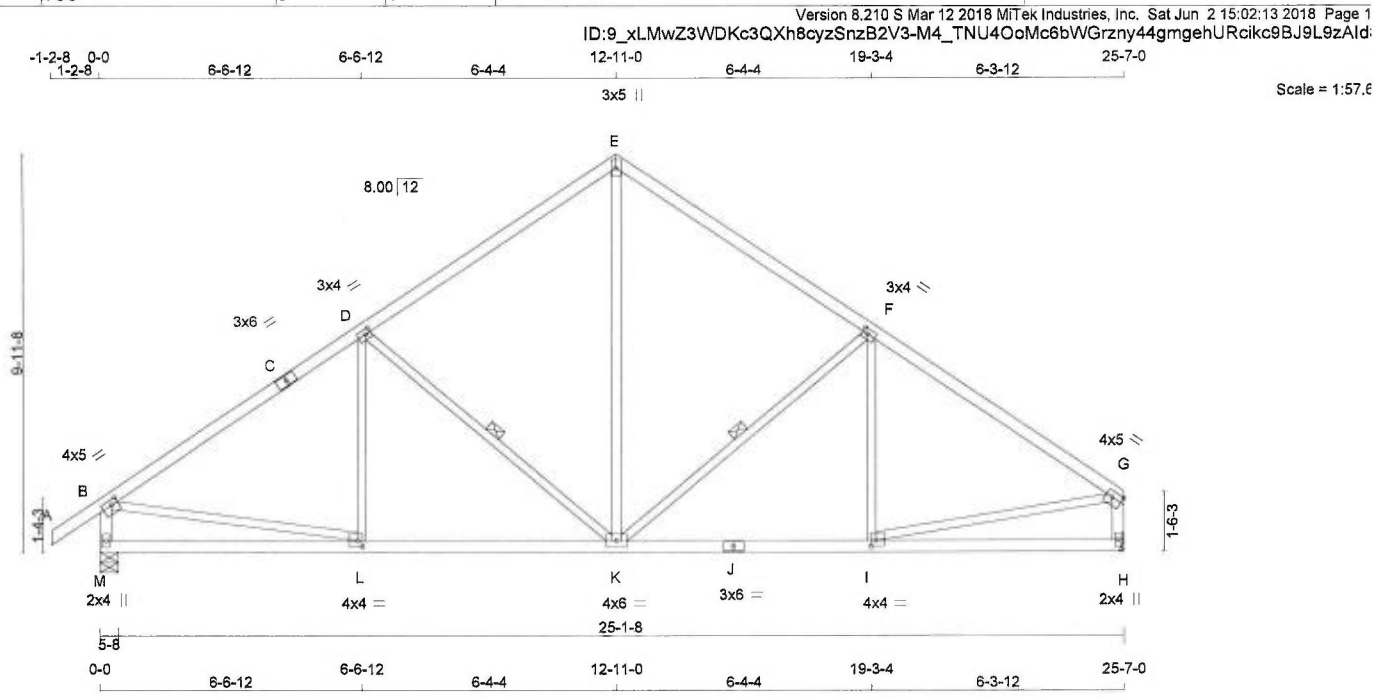
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (G) (INPUT = 0.90)
JSI METAL= 0.48 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER

N. L. G. A. RULES

CHORDS

SIZE

LUMBER

DESCR.

A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT

TYPE

PLATES

W

LEN

Y

X

B	TMVW-t	MT20	4.0	5.0	1.50	2.00
C	TS-t	MT20	3.0	6.0		
D	TMVW-t	MT20	3.0	4.0	1.50	1.50
E	TTW+p	MT20	3.0	5.0		
F	TMVW-t	MT20	3.0	4.0	1.50	1.50
G	TMVW-t	MT20	4.0	5.0	1.75	Edge
H	BMV1+p	MT20	2.0	4.0		
I	BMVW-t	MT20	4.0	4.0	1.75	1.50
J	BS-t	MT20	3.0	6.0		
K	BMVW-t	MT20	4.0	6.0		
L	BMVW-t	MT20	4.0	4.0	1.75	1.50
M	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED

GROSS REACTION

MAXIMUM FACTORED

GROSS REACTION

INPUT

REQRD

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
M	1323	0	1323	0	0	5-8	5-8
H	1223	0	1223	0	0	MECHANICAL	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT H TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

1ST LCASE

MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	927	656 / 0	0 / 0	0 / 0	0 / 0	271 / 0	0 / 0
H	859	596 / 0	0 / 0	0 / 0	0 / 0	264 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.94 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF D-K, F-K. DBS = 20-0-0, CBF = 61 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. UNBRAC. LENGTH (LC)	MEMB.	FACTORED FORCE (LBS)	MAX. UNBRAC. LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-77.4 -77.4	0.09 (1)	L-D	-55 / 94	0.03 (4)	
B-C	-1397 / 0	-77.4 -77.4	0.48 (1)	D-K	-490 / 0	0.23 (1)	
C-D	-1397 / 0	-77.4 -77.4	0.48 (1)	K-E	0 / 719	0.16 (1)	
D-E	-1016 / 0	-77.4 -77.4	0.44 (1)	K-F	-448 / 0	0.21 (1)	
E-F	-1015 / 0	-77.4 -77.4	0.43 (1)	I-F	-91 / 81	0.05 (1)	
F-G	-1359 / 0	-77.4 -77.4	0.45 (1)	B-L	0 / 1205	0.27 (1)	
M-B	-1273 / 0	0.0 0.0	0.13 (1)	I-G	0 / 1179	0.27 (1)	
H-G	-1176 / 0	0.0 0.0	0.12 (1)				
M-L	0 / 0	-18.2 -18.2	0.18 (4)				
L-K	0 / 1190	-18.2 -18.2	0.29 (1)				
K-J	0 / 1158	-18.2 -18.2	0.28 (1)				
J-I	0 / 1158	-18.2 -18.2	0.28 (1)				
I-H	0 / 0	-18.2 -18.2	0.18 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF

DL = 3.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.3 PSF

TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014

- CSA 086-09

- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.85")

CALCULATED VERT. DEFL.(LL) = L/999 (0.05")

ALLOWABLE DEFL.(TL)= L/360 (0.85")

CALCULATED VERT. DEFL.(TL) = L/999 (0.10")

CSI: TC=0.48/1.00 (B-D:1), BC=0.29/1.00 (K-L:1), WB=0.27/1.00 (B-L:1), SS=0.20/1.00 (B-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (G) (INPUT = 0.90)

JSI METAL= 0.48 (B) (INPUT = 1.00)

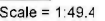
LICENSED PROFESSIONAL ENGINEER

N.A. EL-MASRI

Jun 02, 2018

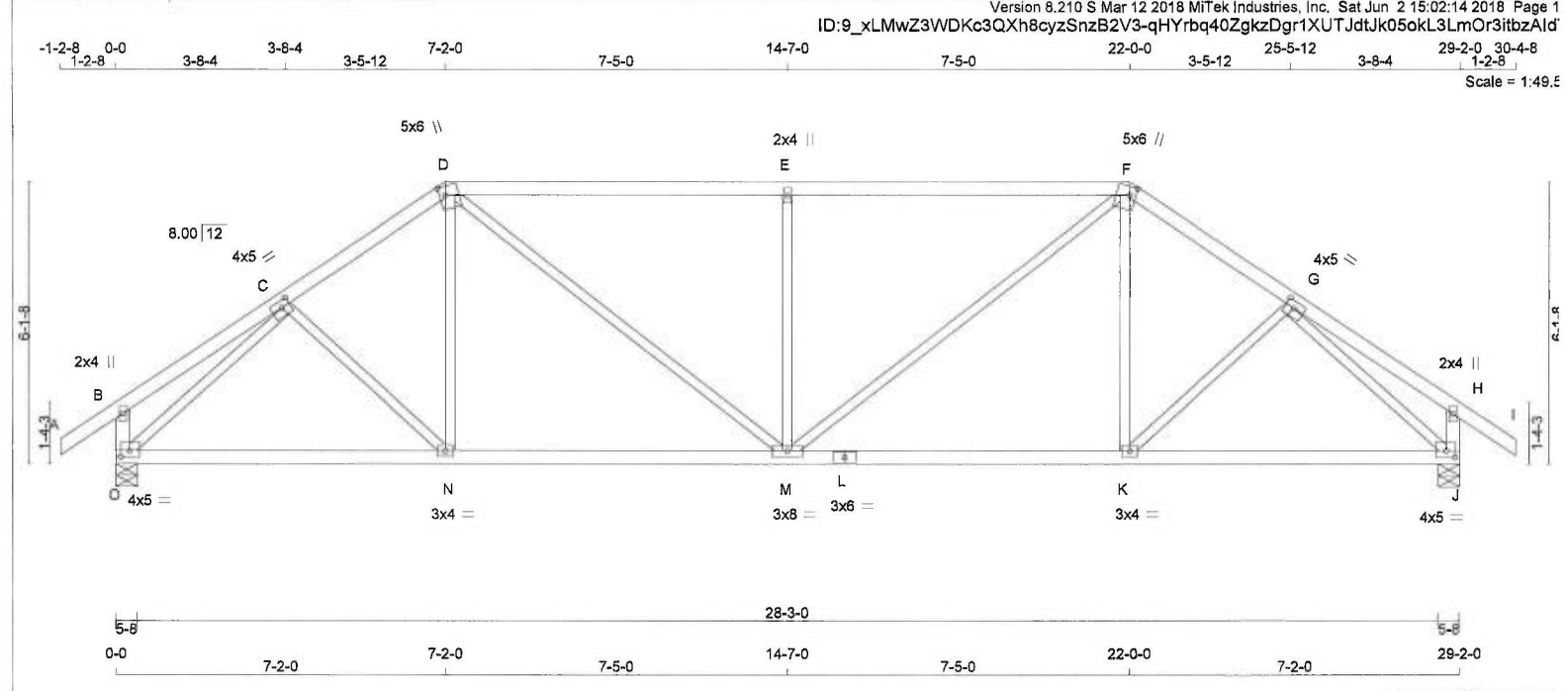
READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

KOTT



M

KOTT



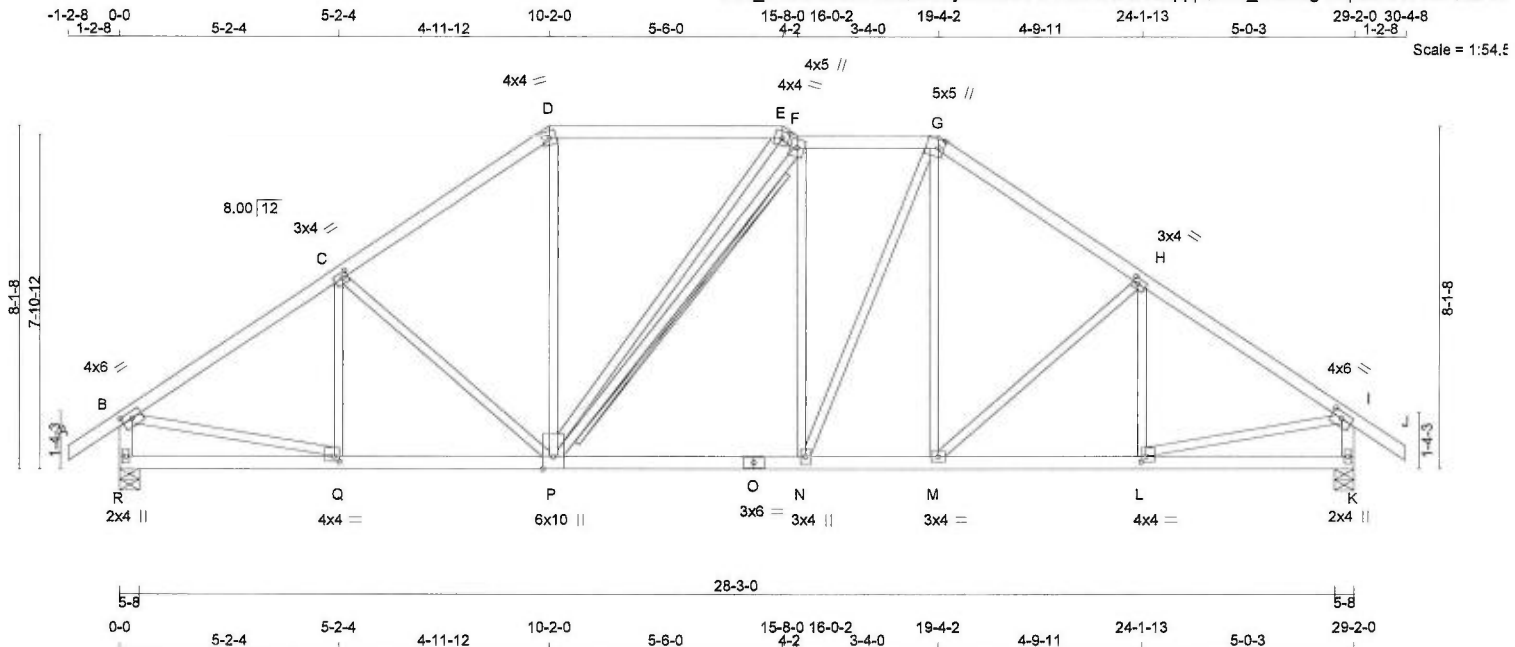
LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. SPF				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER										TOTAL WEIGHT = 119 lb			
				BEARINGS				DESIGN CRITERIA									
				FACTORED				SPECIFIED LOADS:									
				GROSS REACTION				TOP CH. LL = 23.3 PSF									
A - D 2x4 DRY No.2				DOWN				DL = 3.0 PSF									
D - F 2x4 DRY No.2				UP				BOT CH. LL = 0.0 PSF									
F - I 2x4 DRY No.2				BRG				DL = 7.3 PSF									
O - B 2x4 DRY No.2				IN-SX				TOTAL LOAD = 33.6 PSF									
J - H 2x4 DRY No.2				PERM. LIVE				SPACING = 24.0 IN. C/C									
O - L 2x4 DRY No.2				WIND				LOADING IN FLAT SECTION BASED ON A									
L - J 2x4 DRY No.2				DEAD				SLOPE OF 2.00/12 MINIMUM									
ALL WEBS 2x3 DRY No.2				SOIL				THIS TRUSS IS DESIGNED FOR RESIDENTIAL									
EXCEPT								OR SMALL BUILDING REQUIREMENTS OF									
DRY: SEASONED LUMBER.								PART 9, NBCC 2010									
								THIS DESIGN COMPLIES WITH:									
								- PART 9 OF OBC 2012, BCBC 2012, ABC 2014									
								- CSA 086-09									
								- TPIC 2011									
								(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F.									
								RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED									
								ROOF LIVE LOAD									
								ALLOWABLE DEFL.(LL)= L/360 (0.97")									
								CALCULATED VERT. DEFL.(LL)= L/ 999 (0.08")									
								ALLOWABLE DEFL.(TL)= L/360 (0.97")									
								CALCULATED VERT. DEFL.(TL)= L/ 999 (0.15")									
								CSI: TC=0.95/1.00 (D-E:1), BC=0.35/1.00 (K-M:1),									
								WB=0.70/1.00 (G-J:1), SSI=0.28/1.00 (D-E:1)									
								DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10									
								COMP=1.10 SHEAR=1.10 TENS= 1.10									
								COMPANION LIVE LOAD FACTOR = 0.50									
								TRUSS PLATE MANUFACTURER IS NOT									
								RESPONSIBLE FOR QUALITY CONTROL IN									
								THE TRUSS MANUFACTURING PLANT.									
								NAIL VALUES									
								PLATE GRIP(DRY) SHEAR SECTION									
								(PSI) (PLI) (PLI)									
								MAX MIN MAX MIN MAX MIN									
								MT20 618 354 1667 822 2284 1656									
								PLATE PLACEMENT TOL. = 0.250 inches									
								PLATE ROTATION TOL. = 5.0 Deg.									
								JSI GRIP= 0.89 (C) (INPUT = 0.90)									
								JSI METAL= 0.53 (C) (INPUT = 1.00)									

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
		GROSS REACTION		GROSS REACTION		BRG		BRG	
JT		VERT	HORZ	DOWN		HORZ	UPLIFT	IN-SX	IN-SX
O		1494	0	1494		0	0	5-8	5-8
J		1494	0	1494		0	0	5-8	5-8
UNFACTORED REACTIONS									
		1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT		COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
O		1047	739 / 0	0 / 0	0 / 0	0 / 0	308 / 0	0 / 0	
J		1047	739 / 0	0 / 0	0 / 0	0 / 0	308 / 0	0 / 0	
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J									
BRACING									
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.60 FT.									
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.									
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.									
LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MEMB.		MAX. FACTORED	FACTORED	MAX	MAX. FACTORED	MEMB.	FORCE	MAX	
		FORCE	VERT. LOAD	LC1	UNBRAC		(LBS)	CSI (LC)	
		(LBS)	(PLF)	CSI (LC)	LENGTH				
FR-TO			FROM	TO	FR-TO				
A- B		0 / 28	-77.4	-77.4	0.09 (1)	10.00	C- N	0 / 59	0.02 (4)
B- C		0 / 16	-77.4	-77.4	0.14 (1)	10.00	N- D	0 / 134	0.04 (4)
C- D		-1595 / 0	-77.4	-77.4	0.18 (1)	5.06	D- M	0 / 672	0.15 (1)
D- E		-1842 / 0	-77.4	-77.4	0.95 (1)	3.60	M- E	-706 / 0	0.42 (1)
E- F		-1842 / 0	-77.4	-77.4	0.95 (1)	3.60	M- F	0 / 672	0.15 (1)
F- G		-1595 / 0	-77.4	-77.4	0.18 (1)	5.06	K- F	0 / 134	0.04 (4)
G- H		0 / 16	-77.4	-77.4	0.14 (1)	10.00	K- G	0 / 59	0.02 (4)
H- I		0 / 28	-77.4	-77.4	0.09 (1)	10.00	O- C	-1785 / 0	0.70 (1)
O- B		-209 / 0	0.0	0.0	0.02 (1)	7.81	G- J	-1785 / 0	0.70 (1)
J- H		-209 / 0	0.0	0.0	0.02 (1)	7.81			
O- N		0 / 1292	-18.2	-18.2	0.34 (1)	10.00			
N- M		0 / 1314	-18.2	-18.2	0.35 (1)	10.00			
M- L		0 / 1314	-18.2	-18.2	0.35 (1)	10.00			
L- K		0 / 1314	-18.2	-18.2	0.35 (1)	10.00			
K- J		0 / 1292	-18.2	-18.2	0.34 (1)	10.00			

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.







TOTAL WEIGHT = 142 lb

LUMBER		LUMBER		DESCR	
N L G A	RULES				
CHORDS	SIZE				
A - D	2x4	DRY	No.2		SPF
D - E	2x4	DRY	No.2		SPF
E - F	2x4	DRY	No.2		SPF
F - G	2x4	DRY	No.2		SPF
G - J	2x4	DRY	No.2		SPF
R - B	2x4	DRY	No.2		SPF
K - I	2x4	DRY	No.2		SPF
R - O	2x4	DRY	No.2		SPF
O - K	2x4	DRY	No.2		SPF

ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
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DRY: SEASONED LUMBER.

PLATES (table is in inches)							
JT	TYPE	PLATES	W	LEN	Y	X	
B	TMWw-t	MT20	4.0	6.0	1.75	3.00	
C	TMWw-t	MT20	3.0	4.0	1.50	1.50	
D	TTW-m	MT20	4.0	4.0			
E	TTW-m	MT20	4.0	4.0	2.00	1.75	
F	TTWW+m	MT20	4.0	5.0			
G	TTWW+m	MT20	5.0	5.0	2.50	1.50	
H	TMWw-t	MT20	3.0	4.0	1.50	1.50	
I	TMWw-t	MT20	4.0	6.0	1.75	3.00	
K	BMW1+p	MT20	2.0	4.0			
L	BMWw-t	MT20	4.0	4.0	1.50	1.50	
M	BMWw-t	MT20	3.0	4.0			
N	BMWw-t	MT20	3.0	4.0			
O	BS-t	MT20	3.0	6.0			
P	BMWwWw+m	MT20	6.0	10.0	Edge		
Q	BMWw-t	MT20	4.0	4.0	1.50	1.50	
R	BMW1+p	MT20	2.0	4.0			

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
R	1494	0	1494	0	0	5-8	5-8
K	1494	0	1494	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1047	739 / 0	0 / 0	0 / 0	0 / 0	308 / 0	0 / 0
K	1047	739 / 0	0 / 0	0 / 0	0 / 0	308 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT F-P

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)		
FR-TO		FROM TO			FR-TO				
A-B	0 / 28	-77.4	-77.4	0.09 (1)	10.00	Q-C	-178 / 31 0.06 (1)		
B-C	-1640 / 0	-77.4	-77.4	0.30 (1)	4.89	C-P	-268 / 0 0.23 (1)		
C-D	-1452 / 0	-77.4	-77.4	0.28 (1)	5.13	P-D	0 / 435 0.10 (1)		
D-E	-1190 / 0	-77.4	-77.4	0.32 (1)	5.48	P-F	-677 / 0 0.48 (1)		
E-F	-1107 / 0	-77.4	-77.4	0.01 (1)	6.02	N-F	-217 / 0 0.26 (1)		
F-G	-1327 / 0	-77.4	-77.4	0.13 (1)	5.50	N-G	0 / 324 0.07 (1)		
G-H	-1466 / 0	-77.4	-77.4	0.27 (1)	5.14	M-G	0 / 234 0.05 (1)		
H-I	-1638 / 0	-77.4	-77.4	0.28 (1)	4.91	M-H	-250 / 0 0.20 (1)		
I-J	0 / 28	-77.4	-77.4	0.09 (1)	10.00	L-H	-183 / 33 0.06 (1)		
R-B	-1453 / 0	0.0	0.0	0.15 (1)	6.78	B-Q	0 / 1413 0.32 (1)		
K-I	-1455 / 0	0.0	0.0	0.15 (1)	6.77	L-I	0 / 1412 0.32 (1)		
						P-E	0 / 472 0.11 (1)		
R-Q	0 / 0	-18.2	-18.2	0.10 (4)	10.00				
Q-P	0 / 1386	-18.2	-18.2	0.29 (1)	10.00				
P-O	0 / 1330	-18.2	-18.2	0.28 (1)	10.00				
O-N	0 / 1330	-18.2	-18.2	0.28 (1)	10.00				
N-M	0 / 1200	-18.2	-18.2	0.23 (1)	10.00				
M-L	0 / 1383	-18.2	-18.2	0.27 (1)	10.00				
L-K	0 / 0	-18.2	-18.2	0.11 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.3	PSF
TOTAL LOAD	=	33.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A
SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.13")

CSI: TC=0.32/1.00 (D-E:1), BC=0.29/1.00 (P-Q:1),
WB=0.49/1.00 (F-P:1), SSI=0.17/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES					
PLATE	GRIP(DRY)		SHEAR		SECTION
	(PSI)		(PLI)		(PLI)
	MAX	MIN	MAX	MIN	MAX MIN
MT20	618	354	1667	822	2284 1656

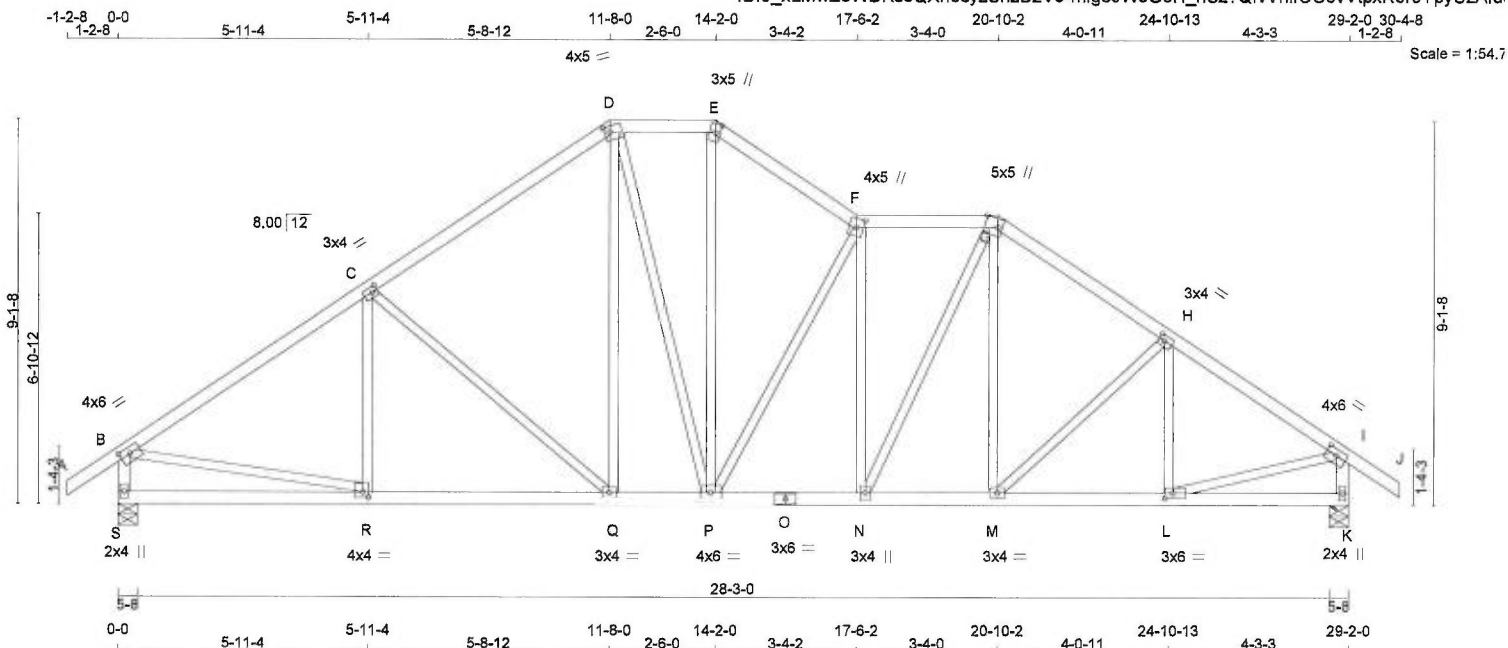
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (L) (INPUT = 0.90)
JSI METAL= 0.52 (Q) (INPUT = 1.00)

**READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.**





TOTAL WEIGHT = 146 lb

N. L. G. A. RULES				
CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
S - O	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

J	T	TYPE	PLATES	W	LEN	Y	X
B	T	TMWw-t	MT20	4.0	6.0	1.75	3.00
C	T	TMWw-t	MT20	3.0	4.0	1.50	1.50
D	T	TTWw-m	MT20	4.0	5.0	1.75	1.50
E	T	TTW+m	MT20	3.0	5.0	2.50	1.25
F	T	TTWw+m	MT20	4.0	5.0	2.50	1.75
G	T	TTWw+m	MT20	5.0	5.0	Edge	3.75
H	T	TMWw-t	MT20	3.0	4.0	1.50	1.50
I	T	TMWw-t	MT20	4.0	6.0	1.75	3.00
K	B	BMV1+p	MT20	2.0	4.0		
L	B	BMWw-t	MT20	3.0	6.0	1.50	2.25
M	B	BMWw-t	MT20	3.0	4.0		
N	B	BMWw+t	MT20	3.0	4.0		
O	B	BS-	MT20	3.0	6.0		
P	B	BMWwWw-t	MT20	4.0	6.0		
Q	B	BMWw-t	MT20	3.0	4.0		
R	B	BMWw-t	MT20	4.0	4.0	1.50	1.50
S	B	BMV1+p	MT20	2.0	4.0		



BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
S	1494	0	1494	0	0	5-8	5-8
K	1494	0	1494	0	0	5-8	5-8

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
S	1047	739 / 0	0 / 0	0 / 0	0 / 0	308 / 0	0 / 0
K	1047	739 / 0	0 / 0	0 / 0	0 / 0	308 / 0	0 / 0

BRACING

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

TOTAL LOAD CASES: (4)

C H O R D S					W E B S				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	LC1 MAX. PLT (LF)	MAX. UNBRAC. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)		
FR-TO		FROM	TO	LENGTH	FR-TO				
A-B	0 / 28	-77.4	-77.4	0.09 (1)	4.05	R-C	-120 / 67	0.05 (1)	
B-C	-1850 / 0	-77.4	-77.4	0.40 (1)	10.70	C-Q	-389 / 0	0.46 (1)	
C-D	-1362 / 0	-77.4	-77.4	0.37 (1)	5.15	Q-D	0 / 325	0.07 (1)	
D-E	-1152 / 0	-77.4	-77.4	0.08 (1)	5.86	D-P	0 / 158	0.04 (1)	
E-F	-1377 / 0	-77.4	-77.4	0.13 (1)	5.42	P-E	0 / 538	0.12 (1)	
F-G	-1475 / 0	-77.4	-77.4	0.13 (1)	5.27	P-F	-722 / 0	0.79 (1)	
G-H	-1538 / 0	-77.4	-77.4	0.19 (1)	5.13	N-F	-382 / 0	0.31 (1)	
H-I	-1610 / 0	-77.4	-77.4	0.20 (1)	5.04	N-G	0 / 482	0.11 (1)	
I-J	0 / 28	-77.4	-77.4	0.09 (1)	10.00	M-G	0 / 164	0.04 (1)	
S-B	-1449 / 0	0.0	0.0	0.15 (1)	6.79	M-H	-131 / 0	0.07 (1)	
K-I	-1459 / 0	0.0	0.0	0.15 (1)	6.77	L-H	-248 / 0	0.07 (1)	
						B-R	0 / 1418	0.32 (1)	
						L-I	0 / 1395	0.31 (1)	
S-R	0 / 0	-18.2	-18.2	0.16 (4)	10.00				
R-Q	0 / 1398	-18.2	-18.2	0.30 (1)	10.00				
Q-P	0 / 1111	-18.2	-18.2	0.22 (1)	10.00				
P-O	0 / 1481	-18.2	-18.2	0.27 (1)	10.00				
O-N	0 / 1481	-18.2	-18.2	0.27 (1)	10.00				
N-M	0 / 1263	-18.2	-18.2	0.23 (1)	10.00				
M-L	0 / 1356	-18.2	-18.2	0.25 (1)	10.00				
L-K	0 / 0	-18.2	-18.2	0.07 (4)	10.00				

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.3	PSF
TOTAL LOAD	=	33.6	PSF

LOADING IN ALL FLAT SECTIONS BASED ON A
SLOPE OF 2.00/12 MINIMUM

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIQ 2011

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/999 (0.12")

CSI: TC=0.40/1.00 (B-C:1), BC=0.30/1.00 (Q-R:1),
WB=0.79/1.00 (E-P:1), SSI=0.18/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

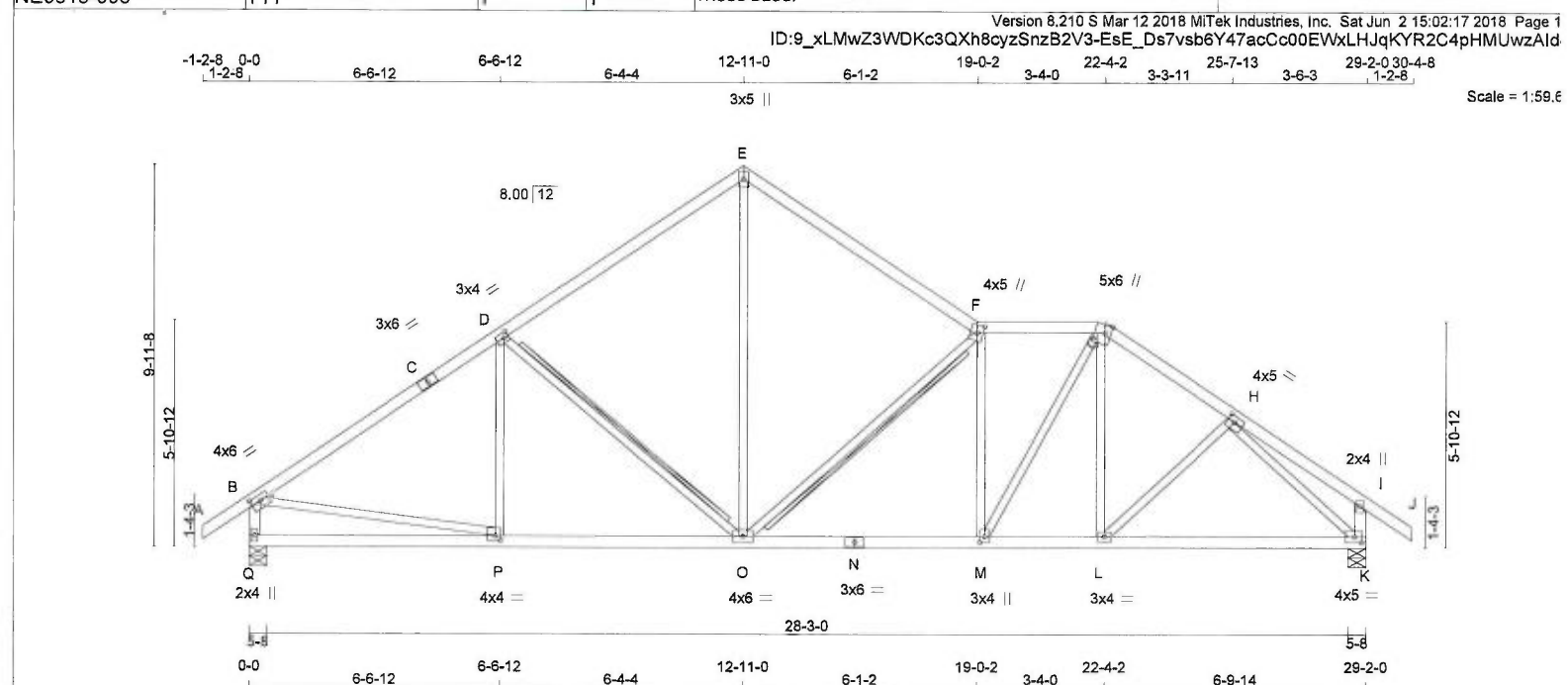
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (G) (INPUT = 0.90)
JSI METAL= 0.52 (R) (INPUT = 1.00)



LUMBER

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
G - J	2x4	DRY	No.2
J - B	2x4	DRY	No.2
K - I	2x4	DRY	No.2
Q - N	2x4	DRY	No.2
N - K	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

SPF

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	4.0	6.0	1.75	3.00
C	TS-t	MT20	3.0	6.0		
D	TMWW-t	MT20	3.0	4.0	1.50	1.50
E	TTWW-p	MT20	3.0	5.0		
F	TTWW+m	MT20	4.0	5.0	2.50	1.75
G	TTWW+m	MT20	5.0	6.0	Edge	1.75
H	TMWW-t	MT20	4.0	5.0	1.75	2.25
I	TMV-p	MT20	2.0	4.0		
K	BMVW-t	MT20	4.0	5.0	1.75	2.25
L	BMWW-t	MT20	3.0	4.0		
M	BMWW-t	MT20	3.0	4.0	1.75	1.50
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	6.0		
P	BMWW-t	MT20	4.0	4.0	1.50	1.50
Q	BMVW-t	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT VERT	1494	0	5-8
Q VERT	1494	0	5-8

UNFACTORED REACTIONS							
	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1047	739 / 0	0 / 0	0 / 0	0 / 0	308 / 0	0 / 0
K	1047	739 / 0	0 / 0	0 / 0	0 / 0	308 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.62 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT D-O, F-O

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS1 (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-77.4 -77.4	0.09 (1)	10.00	P-D	-90 / 82	0.05 (1)
B-C	-1646 / 0	-77.4 -77.4	0.50 (1)	4.62	D-O	-459 / 0	0.24 (1)
C-D	-1646 / 0	-77.4 -77.4	0.50 (1)	4.62	O-E	0 / 974	0.22 (1)
D-E	-1293 / 0	-77.4 -77.4	0.46 (1)	5.12	E-F	-822 / 0	0.43 (1)
E-F	-1258 / 0	-77.4 -77.4	0.41 (1)	5.26	F-G	-505 / 0	0.27 (1)
F-G	-1644 / 0	-77.4 -77.4	0.14 (1)	5.05	G-H	0 / 653	0.15 (1)
G-H	-1599 / 0	-77.4 -77.4	0.13 (1)	5.12	H-I	0 / 88	0.03 (4)
H-I	0 / 15	-77.4 -77.4	0.12 (1)	10.00	I-J	0 / 64	0.02 (4)
I-J	0 / 28	-77.4 -77.4	0.09 (1)	10.00	J-K	0 / 1414	0.32 (1)
J-K	-1444 / 0	0.0 0.0	0.15 (1)	6.80	K-L	-1777 / 0	0.64 (1)
K-L	-205 / 0	0.0 0.0	0.02 (1)	7.81			
Q-P	0 / 0	-18.2 -18.2	0.18 (4)	10.00			
P-O	0 / 1397	-18.2 -18.2	0.31 (1)	10.00			
O-N	0 / 1653	-18.2 -18.2	0.35 (1)	10.00			
N-M	0 / 1653	-18.2 -18.2	0.35 (1)	10.00			
M-L	0 / 1317	-18.2 -18.2	0.31 (1)	10.00			
L-K	0 / 1277	-18.2 -18.2	0.31 (1)	10.00			

TOTAL WEIGHT = 129 lb [M]F

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL LOAD		=	33.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL)= L/999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL)= L/999 (0.16")

CSI: TC=0.50/1.00 (B-D:1), BC=0.35/1.00 (M-O:1), WB=0.64/1.00 (H-K:1), SSI=0.20/1.00 (B-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

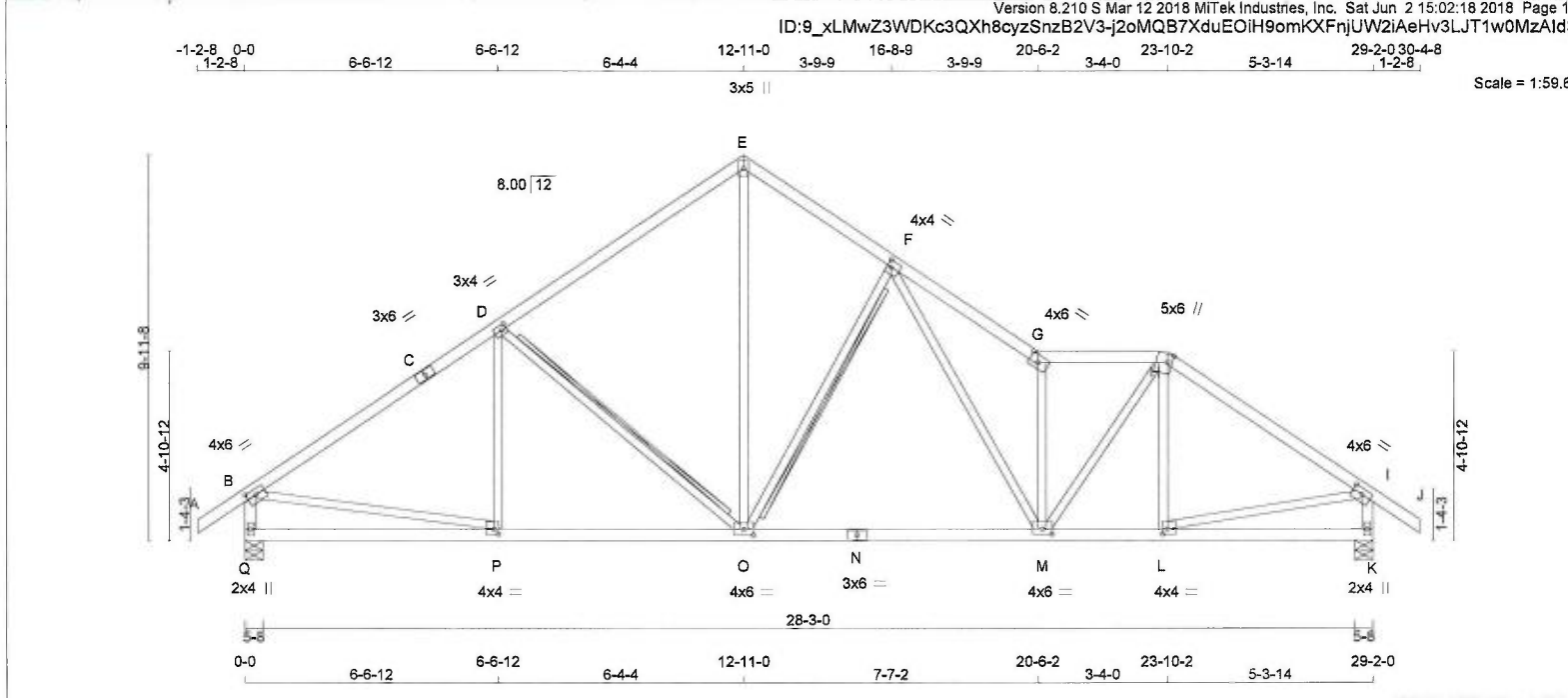
PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.53 (H) (INPUT = 1.00)



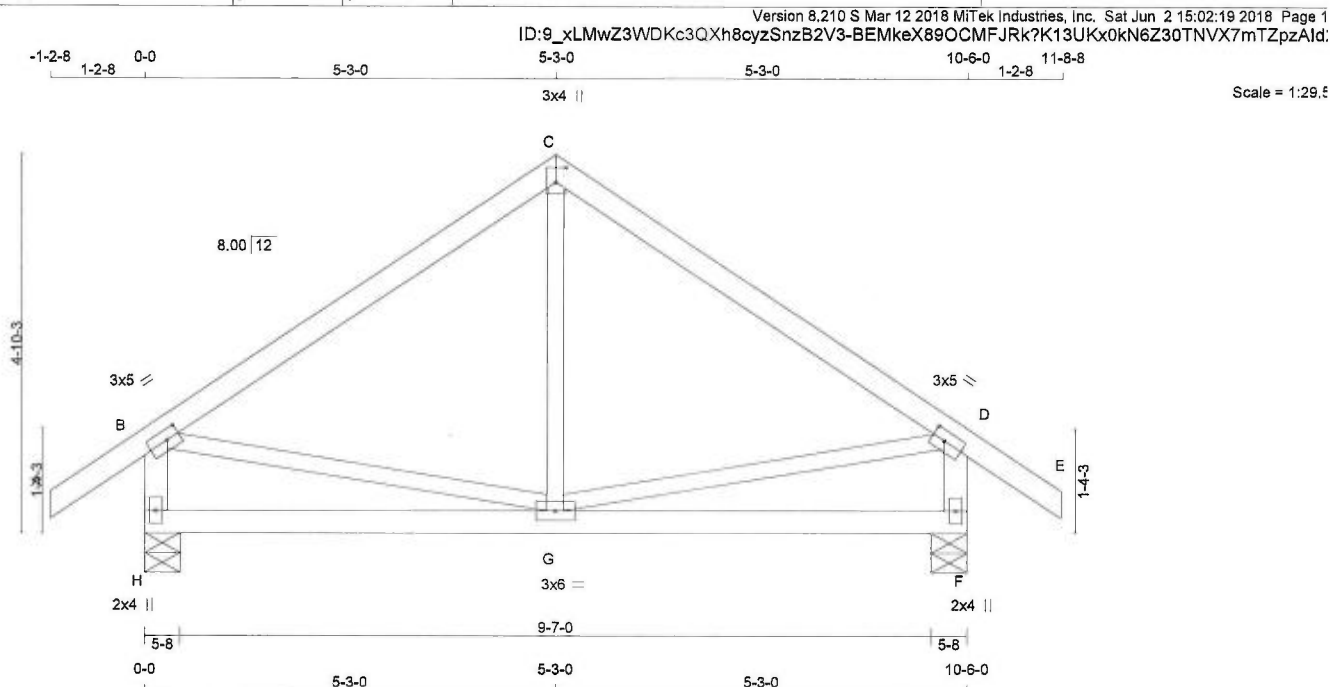


LUMBER N. L. G. A. RULES CHORDS SIZE A - C 2x4 DRY No.2 C - E 2x4 DRY No.2 E - G 2x4 DRY No.2 G - H 2x4 DRY No.2 H - J 2x4 DRY No.2 Q - B 2x4 DRY No.2 K - I 2x4 DRY No.2 Q - N 2x4 DRY No.2 N - K 2x4 DRY No.2 ALL WEBS 2x3 DRY No.2 EXCEPT DRY: SEASONED LUMBER.			DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 23.3 PSF DL = 3.0 PSF BOT CH. LL = 0.0 PSF DL = 7.3 PSF TOTAL LOAD = 33.6 PSF SPACING = 24.0 IN. C/C LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12 MINIMUM THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010 THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012, BCBC 2012, ABC 2014 - CSA 086-09 - TPIC 2011 (55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL)= L/360 (0.97") CALCULATED VERT. DEFL.(LL) = L/999 (0.10") ALLOWABLE DEFL.(TL)= L/360 (0.97") CALCULATED VERT. DEFL.(TL) = L/999 (0.23") CSI: TC=0.50/1.00 (B-D:1), BC=0.35/1.00 (M-O:1), WB=0.52/1.00 (G-M:1), SSI=0.20/1.00 (B-D:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10 COMPANION LIVE LOAD FACTOR = 0.50 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT . NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 618 354 1667 822 2284 1656 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.89 (M) (INPUT = 0.90) JSI METAL= 0.55 (G) (INPUT = 1.00)		
PLATES (table is in inches) JT TYPE PLATES W LEN Y X B TMVW-t MT20 4.0 6.0 1.75 3.00 C TS-t MT20 3.0 6.0 D TMWW-t MT20 3.0 4.0 1.50 1.50 E TTW+p MT20 3.0 5.0 F TMWW-t MT20 4.0 4.0 2.00 1.25 G TTW-h MT20 4.0 6.0 2.25 3.00 H TTWW+m MT20 5.0 6.0 2.50 1.50 I TMVW-t MT20 4.0 6.0 1.75 3.00 K BMV1+p MT20 2.0 4.0 L BMWW-t MT20 4.0 4.0 1.50 1.50 M BMWWW-t MT20 4.0 6.0 1.50 3.00 N BS-t MT20 3.0 6.0 O BMWWW-t MT20 4.0 6.0 1.75 3.00 P BMWW-t MT20 4.0 4.0 1.50 1.50 Q BMV1+p MT20 2.0 4.0			BEARINGS FACTORED GROSS REACTION JT VERT HORZ Q 1494 0 K 1494 0 UNFACTORED REACTIONS 1ST LCASE MAX/MIN. COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL Q 1047 739 / 0 0 / 0 0 / 0 0 / 0 308 / 0 0 / 0 K 1047 739 / 0 0 / 0 0 / 0 0 / 0 308 / 0 0 / 0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.41 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. 2x4 DRY SPF No.2 T-BRACE AT D-O, F-O FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH. END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW LOADING TOTAL LOAD CASES: (4)		
CHORDS MEMB. MAX. FACTORED FORCE (LBS) FACTORED VERT. LOAD (PLF) MAX. UNBRACED LENGTH (LC) MAX. FACTORED FORCE (LBS) MAX. UNBRACED LENGTH (LC) FR-TO FROM TO A-B 0 / 28 -77.4 -77.4 0.09 (1) 10.00 P-D -96 / 77 0.05 (1) B-C -1645 / 0 -77.4 -77.4 0.50 (1) 4.63 D-O -454 / 0 0.24 (1) C-D -1645 / 0 -77.4 -77.4 0.50 (1) 4.63 O-E 0 / 1105 0.25 (1) D-E -1296 / 0 -77.4 -77.4 0.46 (1) 5.12 O-F -761 / 0 0.37 (1) E-F -1282 / 0 -77.4 -77.4 0.16 (1) 5.53 F-M 0 / 968 0.22 (1) F-G -2230 / 0 -77.4 -77.4 0.21 (1) 4.41 M-G -1483 / 0 0.52 (1) G-H -1842 / 0 -77.4 -77.4 0.15 (1) 4.82 M-H 0 / 888 0.20 (1) H-I -1606 / 0 -77.4 -77.4 0.33 (1) 4.89 L-H -168 / 10 0.06 (1) I-J 0 / 28 -77.4 -77.4 0.09 (1) 10.00 B-P 0 / 1413 0.32 (1) Q-B -1443 / 0 0.0 0.0 0.15 (1) 6.81 L-I 0 / 1358 0.31 (1) K-I -1451 / 0 0.0 0.0 0.15 (1) 6.79 Q-P 0 / 0 -18.2 -18.2 0.17 (4) 10.00 P-O 0 / 1396 -18.2 -18.2 0.35 (1) 10.00 O-N 0 / 1409 -18.2 -18.2 0.35 (1) 10.00 N-M 0 / 1409 -18.2 -18.2 0.35 (1) 10.00 M-L 0 / 1331 -18.2 -18.2 0.27 (1) 10.00 L-K 0 / 0 -18.2 -18.2 0.12 (4) 10.00			WEBS MEMB. MAX. FACTORED FORCE (LBS) MAX. FACTORED FORCE (LBS) MAX. UNBRACED LENGTH (LC) FR-TO FROM TO A-B 0 / 28 -77.4 -77.4 0.09 (1) 10.00 P-D -96 / 77 0.05 (1) B-C -1645 / 0 -77.4 -77.4 0.50 (1) 4.63 D-O -454 / 0 0.24 (1) C-D -1645 / 0 -77.4 -77.4 0.50 (1) 4.63 O-E 0 / 1105 0.25 (1) D-E -1296 / 0 -77.4 -77.4 0.46 (1) 5.12 O-F -761 / 0 0.37 (1) E-F -1282 / 0 -77.4 -77.4 0.16 (1) 5.53 F-M 0 / 968 0.22 (1) F-G -2230 / 0 -77.4 -77.4 0.21 (1) 4.41 M-G -1483 / 0 0.52 (1) G-H -1842 / 0 -77.4 -77.4 0.15 (1) 4.82 M-H 0 / 888 0.20 (1) H-I -1606 / 0 -77.4 -77.4 0.33 (1) 4.89 L-H -168 / 10 0.06 (1) I-J 0 / 28 -77.4 -77.4 0.09 (1) 10.00 B-P 0 / 1413 0.32 (1) Q-B -1443 / 0 0.0 0.0 0.15 (1) 6.81 L-I 0 / 1358 0.31 (1) K-I -1451 / 0 0.0 0.0 0.15 (1) 6.79 Q-P 0 / 0 -18.2 -18.2 0.17 (4) 10.00 P-O 0 / 1396 -18.2 -18.2 0.35 (1) 10.00 O-N 0 / 1409 -18.2 -18.2 0.35 (1) 10.00 N-M 0 / 1409 -18.2 -18.2 0.35 (1) 10.00 M-L 0 / 1331 -18.2 -18.2 0.27 (1) 10.00 L-K 0 / 0 -18.2 -18.2 0.12 (4) 10.00		



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 44 lb [M][F]

LUMBER				DESCR.
N. L. G. A. RULES	SIZE	LUMBER		
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVV+	MT20	3.0	5.0	1.50	2.00
C	TTW+	MT20	3.0	4.0	2.25	1.50
D	TMVV+	MT20	3.0	5.0	1.50	2.00
F	BMV1+	MT20	2.0	4.0		
G	BMVV+	MT20	3.0	6.0		
H	BMV1+	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
H	602	0	602	0	5-8	5-8
F	602	0	602	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN	COMPONENT REACTIONS				
H	420	305 / 0	0 / 0	0 / 0	0 / 0	116 / 0	0 / 0
F	420	305 / 0	0 / 0	0 / 0	0 / 0	116 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 MAX CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX LC1 MAX CSI (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 28	-77.4	-77.4 0.09 (1)	10.00	G-C	0 / 93	0.03 (4)
B-C	-365 / 0	-77.4	-77.4 0.27 (1)	6.25	B-G	0 / 310	0.07 (1)
C-D	-365 / 0	-77.4	-77.4 0.27 (1)	6.25	G-D	0 / 310	0.07 (1)
D-E	0 / 28	-77.4	-77.4 0.09 (1)	10.00			
H-B	-565 / 0	0.0	0.0 0.06 (1)	7.81			
F-D	-565 / 0	0.0	0.0 0.06 (1)	7.81			
H-G	0 / 0	-18.2	-18.2 0.14 (4)	10.00			
G-F	0 / 0	-18.2	-18.2 0.14 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:		
TOP CH.	LL	23.3 PSF
	DL	3.0 PSF
BOT CH.	LL	0.0 PSF
	DL	7.3 PSF
TOTAL LOAD		33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.27/1.00 (C-D:1), BC=0.14/1.00 (F-G:4), WB=0.07/1.00 (B-G:1), SSI=0.13/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	618	354	1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

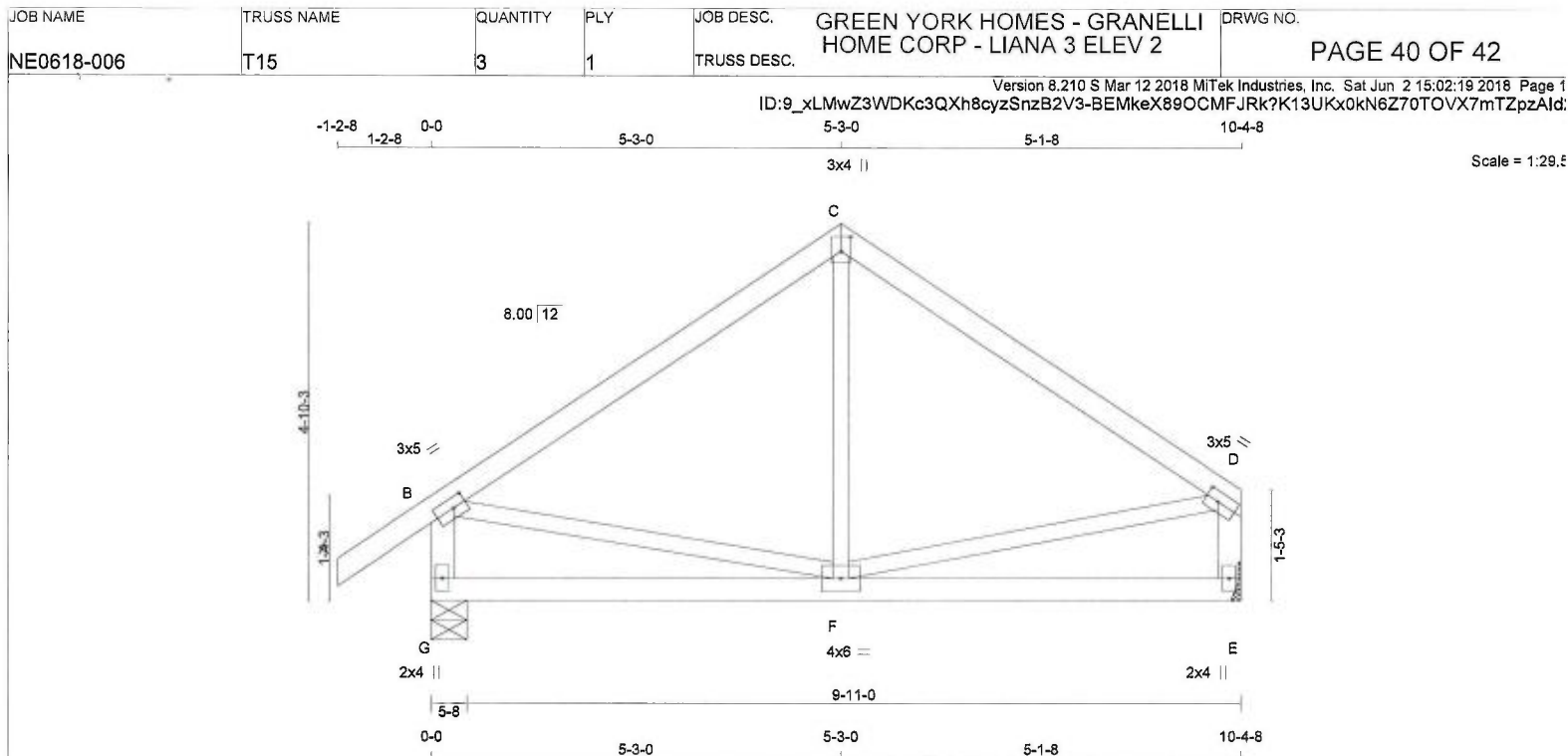
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.68 (G) (INPUT = 0.90)
JSI METAL= 0.16 (D) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 3 X 42 = 125 lb
[M][F]

LUMBER					N. L. G. A. RULES	
CHORDS	SIZE		LUMBER		DESCR.	
A - C	2x4	DRY	No.2		SPF	
C - D	2x4	DRY	No.2		SPF	
G - B	2x4	DRY	No.2		SPF	
E - D	2x4	DRY	No.2		SPF	
G - E	2x4	DRY	No.2		SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2		SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	3.0	5.0	1.50	2.00
C	TTW+p	MT20	3.0	4.0	2.25	1.50
D	TMVW-t	MT20	3.0	5.0	1.50	2.00
E	BMV1+p	MT20	2.0	4.0		
F	BMVWW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
G	596	0	596	0
E	496	0	496	0
			5-8	5-8
			MECHANICAL	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT E TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	416	302 / 0	0 / 0	0 / 0	0 / 0	115 / 0	0 / 0
E	348	242 / 0	0 / 0	0 / 0	0 / 0	107 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX LC1 (LC)
FR-TO		FROM TO			FR-TO		
A- B	0 / 28	-77.4 -77.4	0.09 (1)	10.00	F- C	-6 / 90	0.03 (4)
B- C	-357 / 0	-77.4 -77.4	0.27 (1)	6.25	B- F	0 / 302	0.07 (1)
C- D	-357 / 0	-77.4 -77.4	0.26 (1)	6.25	F- D	0 / 304	0.07 (1)
G- B	-559 / 0	0.0 0.0	0.06 (1)	7.81			
E- D	-460 / 0	0.0 0.0	0.05 (1)	7.81			
G- F	0 / 0	-18.2 -18.2	0.14 (4)	10.00			
F- E	0 / 0	-18.2 -18.2	0.14 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL LOAD	=	33.6	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.27/1.00 (B-C:1), BC=0.14/1.00 (E-F:4), WB=0.07/1.00 (D-F:1), SSI=0.13/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.51 (B) (INPUT = 0.90)
JSI METAL= 0.16 (B) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



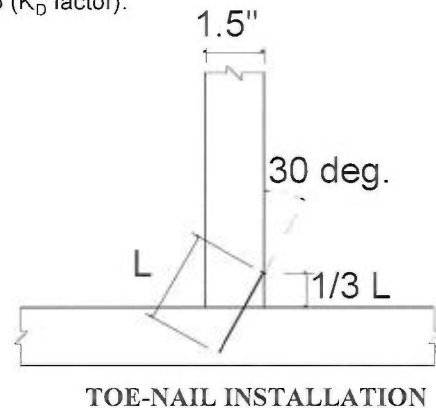
BEARING ANCHORAGE BY TOE-NAILS FOR LATERAL CAPACITY

B37579H1

NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL LATERAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	132	147
	3.25	0.144	132	147
	3.50	0.160	159	177

NOTES:

1. Rafter and ceiling members may be anchored to top and bottom chords of girder truss by toe-nailing rafter and ceiling members to girder chords provided the reaction does not exceed the lateral capacities in the table. Hangers (specified by others) are required for reactions higher than the maximum toe-nail capacity. Reactions are based on factored loads.
2. Toe nail capacities shown in the table are for one toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-09, section 10.9.4.1.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in tables below.
5. Nail values in table are based on the following relative lumber densities: G = 0.42 (SPF), G = 0.49 (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See next page for nailing on bearing plate).
7. For loads due to wind the nail lateral capacity in this table may be multiplied by 1.15 (K_D factor).
8. Lumber must be dry (< 19% moisture content) at the time of nail installation.
9. Nail values in this table comply with CSA O86-09, section 10.9.4
10. This design is not valid after April 30, 2019.



Nail type	Common wire	Common spiral	Common wire
Nail dia. (in)	0.160	0.152	0.144
	(3.5" nail)		(3" and 3.25" nail)
LUMBER SIZE	MAXIMUM NUMBER OF TOE-NAILS		
2X4 SPF	2	2	3
2X4 D. Fir	2	2	2
2X6 SPF	4	4	4
2X6 D. Fir	3	3	3



MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

PEO
Certificate No. 10889485



April 26, 2017

BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING

B37579H2

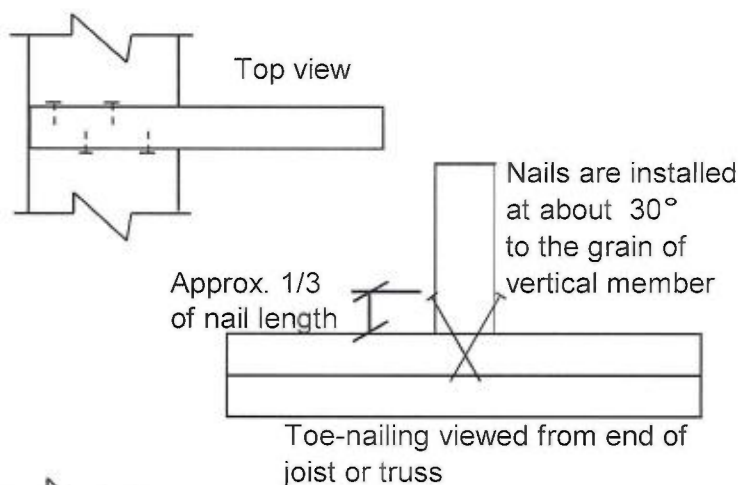
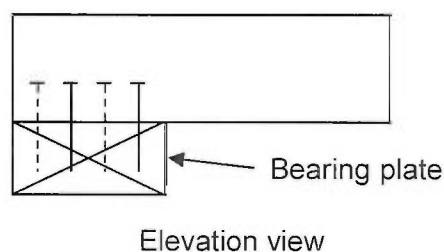
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	30	42
	3.25	0.144	32	45
	3.50	0.160	38	52

Note: If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

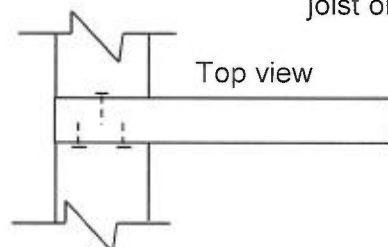
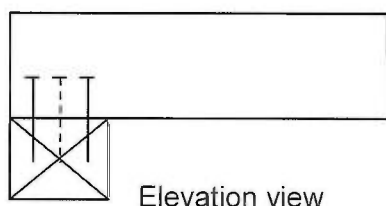
NOTES:

1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to **wind or earthquake** load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-09, section 10.9.5.2.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37579H1).
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-09, section 10.9.5
9. This design is not valid after April 30, 2019

Toe-nailing on 2x6 Bearing Plate



Toe-nailing on 2x4 Bearing Plate



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April 26, 2017