

ENGINEERING NOTE PAGE (ENP-1)
PLEASE READ PRIOR TO INSTALLATION

NE0618-002**GREEN YORK HOMES - GRANELLI HOME CORP - LIANA 2 ELEV 2****RESPONSIBILITIES**

THIS DESIGN IS FOR AN INDIVIDUAL BUILDING COMPONENT AND HAS BEEN BASED ON INFORMATION PROVIDED BY THE DESIGN OFFICE OF KOTT LUMBER. THE UNDERSIGNED ENGINEER DISCLAIMS ANY RESPONSIBILITY FOR DAMAGES AS A RESULT OF FAULTY OR INCORRECT INFORMATION, SPECIFICATION AND/OR DESIGNS FURNISHED TO THE ENGINEER. THE UNDERSIGNED ENGINEER IS ONLY RESPONSIBLE FOR THE STRUCTURAL INTEGRITY OF THIS BUILDING COMPONENT FOR THE CONDITIONS AND LOADS SHOWN ON THIS DRAWING. THE STRUCTURAL INTEGRITY OF THE BUILDING AND THE VERIFICATION OF THE DIMENSIONS AND THE DESIGN LOADS USED ARE THE RESPONSIBILITY OF THE BUILDING DESIGNER.

TRUSSES ARE DESIGNED IN CONFORMANCE WITH THE RELEVANT SECTIONS OF THE NATIONAL BUILDING CODE OF CANADA OR THE CANADIAN CODE FOR FARM BUILDINGS, WHICHEVER APPLIES TO THE BUILDING TYPE INDICATED ON THE DRAWING

IT IS THE RESPONSIBILITY OF KOTT LUMBER TO ENSURE THAT TRUSSES ARE MANUFACTURED IN CONFORMANCE WITH THESE DESIGNS AND WITH THE SPECIFICATIONS OUTLINED BELOW. THE UNDERSIGNED ENGINEER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

USE AND OCCUPANCY

- The building is of the type indicated on the drawing

LOADING

- The truss loading intensity and distribution as well as load transfer mechanism is that indicated on the drawing
- No buildings, trees, parapets or other projections higher than the roof for which the trusses are used are located within a distance less than ten (10) times the difference in height, or five metres (16 ft) whichever is greater, unless the drawing indicates that the snow drifting has been taken into account

HANDLING, INSTALLATION AND BRACING

- The trusses must be handled and installed by a qualified professional as per the supplied document titled *Information for Truss Installers* and the BCSI-B1 and BCSI-B3 Summary Sheets
- The compression chords are laterally braced by continuous rigid diaphragm sheathing or as specified on the drawing
- Temporary and permanent bracing must be installed as indicated on the truss drawing and according to the BCSI-B1 and BCSI-B3 Summary Sheets. Bracing for the lateral stability of the truss is to be provided by the building designer
- **It is recommended that a Professional Engineer's advice be obtained for the bracing of trusses spanning more than 12.37m (40'-7")**

SUPPORTS

- The trusses are to be supported at the bearing points indicated and anchored to the supports where considered necessary by the designer of the overall structure
- Bearing sizes shown are the minimum required to prevent crushing of the truss members and do not necessarily take into account stability of the overall building structure
- Elevation of bearings must be carefully checked and shimmed to alignment for solid bearings
- Adequate wood truss bearing is the responsibility of the building designer.

DIMENSIONS

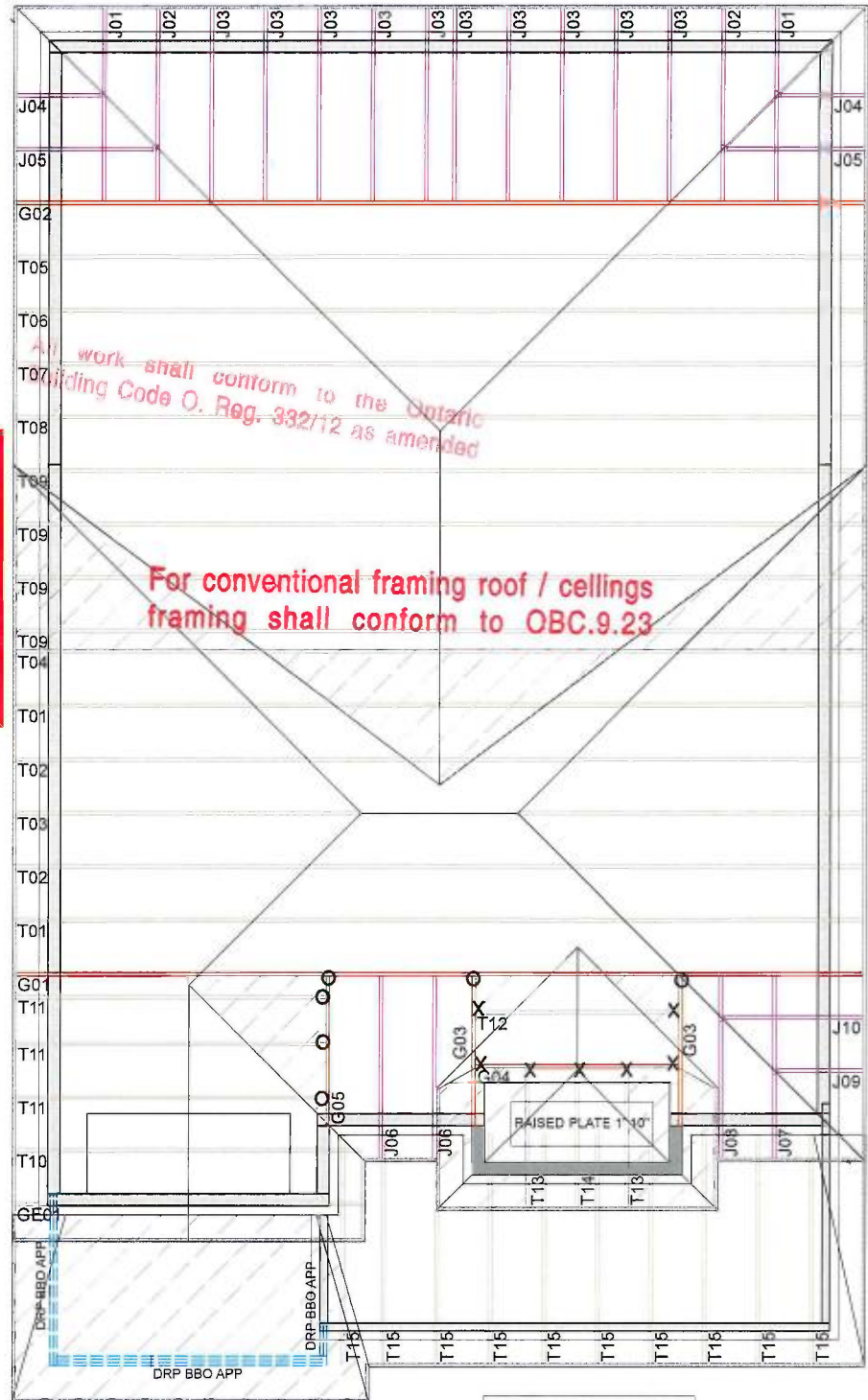
- Geometry of the truss and dimensions indicated on the drawing are identical to those of the installed truss.

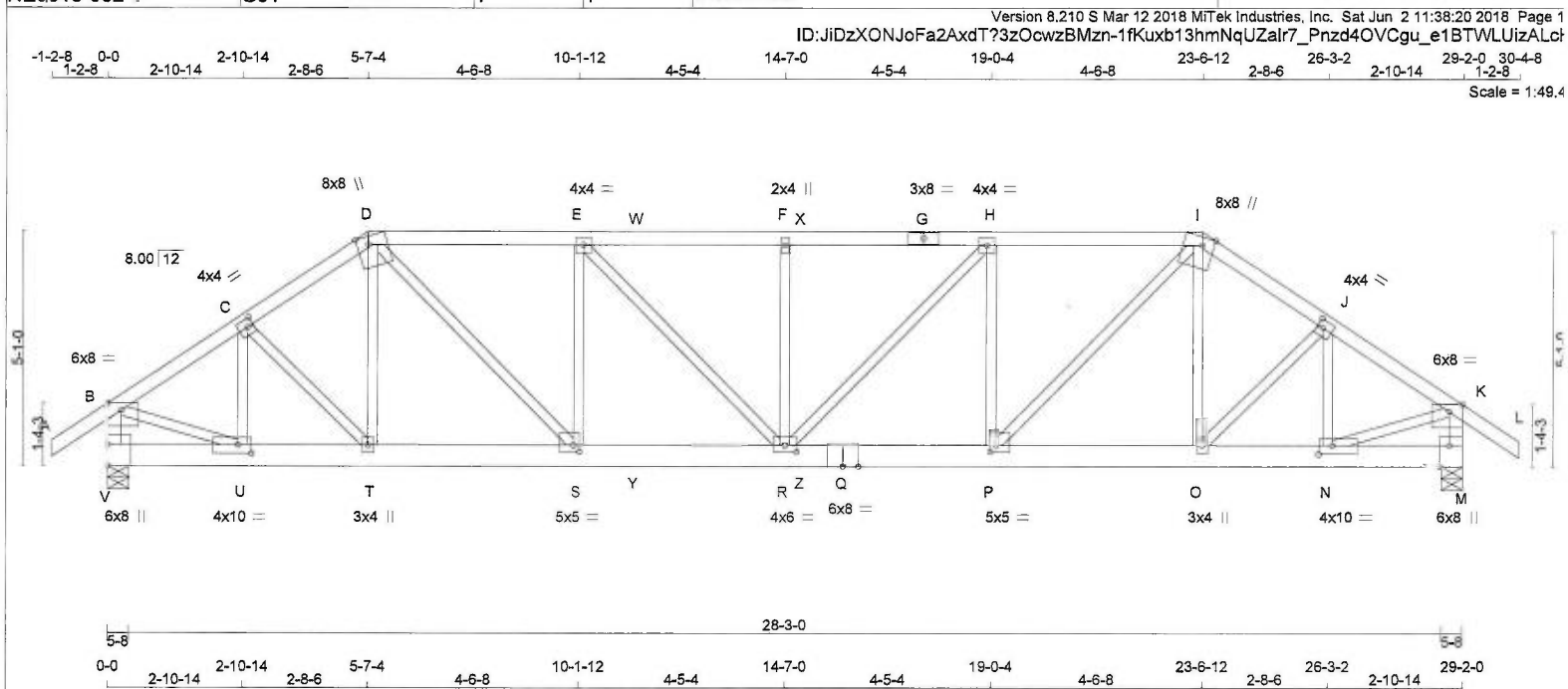
Layout based on plans provided by
Jardin Design Group Inc.
Revision #2 dated May 22, 2018

THIS CERTIFICATION IS FOR THE ENGINEERING REVIEW OF STRUCTURAL COMPONENTS SHOWN ON THIS DRAWING AND IS LIMITED TO THE COMPONENTS SUPPLIED BY KOTT LUMBER FOR THE LOADS AND CONDITIONS SHOWN ON THE CALCULATION PAGE OF EACH COMPONENT. THE PLACEMENT OF THESE COMPONENTS ON THIS LAYOUT IS SHOWN FOR REFERENCE ONLY. ROOF TRUSSES MUST BE INSTALLED ACCORDING TO MANUFACTURER'S GUIDELINES AT 24" C/C UNLESS NOTED OTHERWISE. CONVENTIONAL FRAMING SHOWN ON THE LAYOUT MUST BE DESIGNED AND APPROVED BY THE PROJECT ENGINEER. TEMPORARY AND PERMANENT BRACINGS OF THE ROOF AND BUILDING ARE THE RESPONSIBILITY OF THE PROJECT ENGINEER AND TO CONFORM TO BCSI GUIDELINES.



ENG.JOB:NE0618-002





LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR.				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS										DESIGN CRITERIA									
A - D	2x4	DRY	No.2	SPF	FACTORED		MAXIMUM FACTORED		INPUT		REQRD		*** SPECIAL LOADS ANALYSIS *** GEOMETRY AND/OR BASIC LOADS CHANGED BY USER. LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE SPECIFIED LOADS: TOP CH. LL = 23.3 PSF DL = 3.0 PSF BOT CH. LL = 0.0 PSF DL = 7.3 PSF TOTAL LOAD = 33.6 PSF SPACING = 24.0 IN. C/C LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM GIRDER TYPE: CPrimeHip SIDE SETBACK = 5-7-4 END SETBACK = 5-7-4 END WALL WIDTH = 5-8 CORNER FRAMING TYPE: CONVENTIONAL END JACK TYPE: CONVENTIONAL APPLIED TO FRONT SIDE - ADDTL LOADS BASED ON 55 % OF GSL. LOADS APPLIED TO FIRST 5-7-4 OF SPAN MEASURED FROM THE RIGHT. *** NON STANDARD GIRDER *** ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES. THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010 THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09 - TPIC 2011 (55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL)= L/360 (0.97") CALCULATED VERT. DEFL.(LL) = L/ 999 (0.21") ALLOWABLE DEFL.(TL)= L/360 (0.97") CALCULATED VERT. DEFL.(TL) = L/ 929 (0.38") CSI: TC=0.77/1.00 (E-F:1) , BC=0.98/1.00 (P-R:1) , WB=0.76/1.00 (K-N:1) , SSI=0.38/1.00 (P-R:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00 COMPANION LIVE LOAD FACTOR = 0.50										
B - G	2x4	DRY	No.2	SPF	GROSS REACTION		GROSS REACTION		BRG		BRG												
G - I	2x4	DRY	No.2	SPF	JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX											
I - L	2x4	DRY	No.2	SPF	V	2709	0	2709	0	0	5-8	5-8											
V - B	2x4	DRY	No.2	SPF	M	3183	0	3183	0	0	5-8	5-8											
M - K	2x4	DRY	No.2	SPF	UNFACTORED REACTIONS 1ST LCASE MAX./MIN. COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL V 1903 1320 / 0 0 / 0 0 / 0 0 / 0 583 / 0 0 / 0 M 2238 1542 / 0 0 / 0 0 / 0 0 / 0 696 / 0 0 / 0																		
V - Q	2x6	DRY	No.2	SPF																			
Q - M	2x6	DRY	No.2	SPF																			
ALL WEBS 2x3 DRY No.2 SPF																							
EXCEPT																							
DRY: SEASONED LUMBER.				BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M																			
PLATES (table is in inches) JT TYPE PLATES W LEN Y X B TMVVW-p MT20 6.0 8.0 Edge 3.50 C TMVVW-t MT20 4.0 4.0 2.00 1.50 D TTWW+m MT20 8.0 8.0 Edge 3.00 E TMVVW-t MT20 4.0 4.0 F TMVVW-w MT20 2.0 4.0 G TS-t MT20 3.0 8.0 H TMVVW-t MT20 4.0 4.0 I TTWW+m MT20 8.0 8.0 Edge 3.00 J TMVVW-t MT20 4.0 4.0 2.00 1.50 K TMVVW-p MT20 6.0 8.0 Edge 3.50 M BMV1+t MT20 6.0 8.0 Edge 2.50 N BMVVW-t MT20 4.0 10.0 2.25 3.50 O BMVVW+t MT20 3.0 4.0 P BMVVW-t MT20 5.0 5.0 1.75 1.50 Q BS-t MT20 6.0 8.0 R BMVVW-t MT20 4.0 6.0 1.75 3.00 S BMVVW-t MT20 5.0 5.0 1.75 1.50 T BMVVW-t MT20 3.0 4.0 U BMVVW-t MT20 4.0 10.0 2.25 3.50 V BMV1+t MT20 6.0 8.0 5.50 Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.				BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.44 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED. LOADING TOTAL LOAD CASES: (4)																			
				CHORDS MAX. FACTORED MEMB. FORCE (LBS)										WEBS MAX. FACTORED MEMB. FORCE (LBS)									
				FR-TO										FR-TO									
				A-B 0 / 28										U-C -866 / 0 0.17 (1)									
				B-C -2958 / 0										C-T 0 / 506 0.13 (1)									
C-D -3410 / 0										T-D -172 / 0 0.06 (1)													
D-E -4591 / 0										D-S 0 / 2551 0.63 (1)													
E-W -5133 / 0										S-E -1028 / 0 0.38 (1)													
W-F -5133 / 0										E-R 0 / 790 0.20 (1)													
F-X -5133 / 0										R-F -473 / 0 0.17 (1)													
X-G -5133 / 0										R-H 0 / 827 0.20 (1)													
G-H -5133 / 0										P-H -966 / 0 0.36 (1)													
H-I -4566 / 0										I-P 0 / 1785 0.44 (1)													
I-J -4003 / 0										O-I 0 / 399 0.10 (1)													
J-K -3516 / 0										O-J 0 / 550 0.14 (1)													
K-L 0 / 28										N-J -913 / 0 0.18 (1)													
V-B -2644 / 0										B-U 0 / 2596 0.64 (1)													
M-K -3104 / 0										N-K 0 / 3082 0.76 (1)													
V-U 0 / 0										-18.2 -18.2 0.08 (1) 10.00													
U-T 0 / 2469										-18.2 -18.2 0.40 (1) 10.00													
T-S 0 / 2818										-18.2 -18.2 0.45 (1) 10.00													
S-Y 0 / 4592										-18.2 -18.2 0.72 (1) 10.00													
Y-R 0 / 4592										-32.6 -32.6 0.72 (1) 10.00													
R-Z 0 / 4566										-32.6 -32.6 0.98 (1) 10.00													
Z-Q 0 / 4566										-18.2 -18.2 0.98 (1) 10.00													
Q-P 0 / 4566										-18.2 -18.2 0.98 (1) 10.00													
P-O 0 / 3325										-18.2 -18.2 0.54 (1) 10.00													
O-N 0 / 2932										-32.6 -32.6 0.45 (1) 10.00													
N-M 0 / 0										-32.6 -32.6 0.07 (1) 10.00													
FACTORED CONCENTRATED LOADS (LBS)																							
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE																
I	23-6-12	-297	-297	—	FRONT	VERT	TOTAL																
O	23-7-4	-722	-722	—	FRONT	VERT	TOTAL																
Q	15-10-12	-722	-722	—	FRONT	VERT	TOTAL																
S	10-5-4	-820	-820	—	FRONT	VERT	TOTAL																



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



CONTINUED ON PAGE 2



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 296.6 lbs FACTORED DOWN AT 23-6-12 ON TOP CHORD, AND 819.8 lbs FACTORED DOWN AT 10-5-4, AND 721.6 lbs FACTORED DOWN AT 15-10-12, AND 721.6 lbs FACTORED DOWN AT 23-7-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (R) (INPUT = 0.90)
JSI METAL= 0.89 (Q) (INPUT = 1.00)

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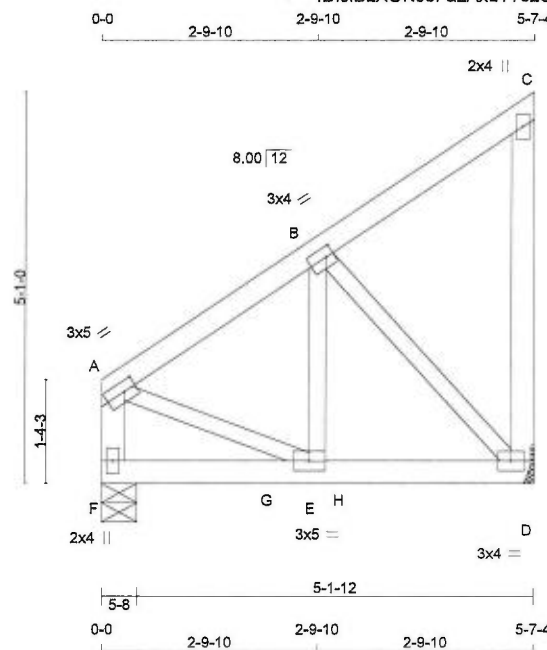
HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 348.5 lbs FACTORED DOWN AT 23-2-0, AND 348.8 lbs FACTORED DOWN AT 6-0-1 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP= 0.89 (U) (INPUT = 0.90)
JSI METAL= 0.96 (Q) (INPUT = 1.00)

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Scale = 1:29.5

TOTAL WEIGHT = 2 X 28 = 55 lb [M]

LUMBER				DESCR.	
CHORDS	SIZE	DRY	LUMBER	SPF	
A - C	2x4	DRY	No.2	SPF	
D - C	2x4	DRY	No.2	SPF	
F - A	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	3.0	5.0	1.50	2.00
B	TMVW-t	MT20	3.0	4.0	1.50	1.25
C	TMV+p	MT20	2.0	4.0		
D	BMVW1-t	MT20	3.0	4.0		
E	BMVW-t	MT20	3.0	5.0		
F	BMV1+p	MT20	2.0	4.0		

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 502.4 lbs FACTORED DOWN AT 2-2-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX
D	722	0	722	0	MECHANICAL	
F	641	0	641	0	5-8	5-8

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT D TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM.LIVE	WIND			
D	507	352 / 0	0 / 0	0 / 0	156 / 0	0 / 0	0 / 0
F	450	312 / 0	0 / 0	0 / 0	138 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS		MEMB.	FACTORED	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)		MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)
FR-TO		FROM TO	FR-TO	FROM TO			
A-B	-552 / 0	-77.4 -77.4 0.10 (1)	6.25	E-B	0 / 535	0.13 (1)	
B-C	-13 / 0	-77.4 -77.4 0.10 (1)	6.25	B-D	-676 / 0	0.19 (1)	
D-C	-85 / 0	0.0 0.0 0.04 (1)	7.81	A-E	0 / 501	0.12 (1)	
F-A	-571 / 0	0.0 0.0 0.06 (1)	7.81				
F-G	0 / 0	-18.2 -18.2 0.24 (1)	10.00				
G-E	0 / 0	-18.2 -18.2 0.24 (1)	10.00				
E-H	0 / 470	-18.2 -18.2 0.33 (1)	10.00				
H-D	0 / 470	-151.7 -151.7 0.33 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
G	2-2-12	-502	-502		FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL = 23.3 PSF
	DL = 3.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.3 PSF
TOTAL LOAD	= 33.6 PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder
START DISTANCE = 3-2-0
START SPAN CARRIED = 7-7-0
END DISTANCE = 5-7-4
END SPAN CARRIED = 7-7-0
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADD'L LOADS BASED ON 55 % OF GSL.
(DEFINED BY USER)

*** NON STANDARD GIRDER ***

ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.10/1.00 (A-B:1), BC=0.33/1.00 (D-E:1), WB=0.19/1.00 (B-D:1), SSI=0.41/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

CONTINUED ON PAGE 2



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NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

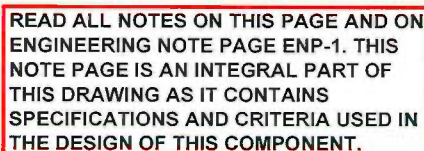
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (B) (INPUT = 0.90)
JSI METAL= 0.18 (A) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON
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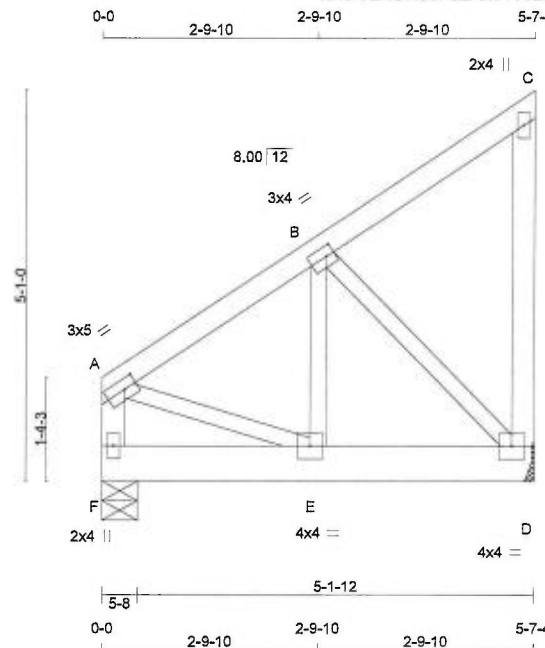
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
NE0618-002	G04	1	1	GREEN YORK HOMES - GRANELLI HOME CORP - LIANA 2 ELEV 2	PAGE 10 OF 40
TRUSS DESC.					

Version 8.210 S Mar 12 2018 © MiTek Industries, Inc. Sat Jun 2 11:38:22 2018 Page 2
ID:JiDzXONJoFa2AxdT?3zOcwzBMzn-_2SeMH2JDNdYktk8zY0tsOiXYI5DM38Jen?RZazALcl

JSI GRIP= 0.78 (A) (INPUT = 0.90)
JSI METAL= 0.12 (A) (INPUT = 1.00)

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ENP-1. THIS
PART OF
IN





Scale = 1:29.5

TOTAL WEIGHT = 31 lb
[M]

LUMBER				DESCR.	
CHORDS	SIZE	DRY	LUMBER	SPF	
A - C	2x4	DRY	No.2	SPF	
D - C	2x4	DRY	No.2	SPF	
F - A	2x4	DRY	No.2	SPF	
F - D	2x6	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
DRY: SEASONED LUMBER.					

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	3.0	5.0	1.50	2.00
B	TMVW-t	MT20	3.0	4.0	1.50	1.25
C	TMV+p	MT20	2.0	4.0		
D	BMVW1-t	MT20	4.0	4.0		
E	BMVW-t	MT20	4.0	4.0		
F	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX
D	820	0	820	0	0	MECHANICAL
F	820	0	820	0	5-8	5-8

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT D TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM.	LIVE			
D	576	400 / 0	0 / 0	0 / 0	0 / 0	177 / 0	0 / 0
F	576	400 / 0	0 / 0	0 / 0	0 / 0	177 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		MAX. UNBRACED LENGTH	WEBS		MAX. UNBRACED LENGTH
	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX (LC)		MAX. FACTORED FORCE (LBS)	MAX (LC)	
FR-TO		FROM	TO					
A-B	-564 / 0	-77.4	-77.4	0.10 (1)	6.25	E-B	0 / 537	0.13 (1)
B-C	-13 / 0	-77.4	-77.4	0.10 (1)	6.25	B-D	-680 / 0	0.18 (1)
D-C	-85 / 0	0.0	0.0	0.03 (1)	7.81	A-E	0 / 506	0.13 (1)
F-A	-566 / 0	0.0	0.0	0.06 (1)	7.81			
F-E	0 / 0	-215.4	-215.4	0.11 (1)	10.00			
E-D	0 / 480	-215.4	-215.4	0.17 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 10-4-8
END DISTANCE = 5-7-4
END SPAN CARRIED = 10-4-8
END WALL WIDTH = 1-8
APPLIED TO BACK SIDE OF BOTTOM CHORD.
- ADDTL LOADS BASED ON 55 % OF GSL.
(DEFINED BY USER)

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.10/1.00 (A-B:1), BC=0.17/1.00 (D-E:1),
WB=0.18/1.00 (B-D:1), SSI=0.23/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



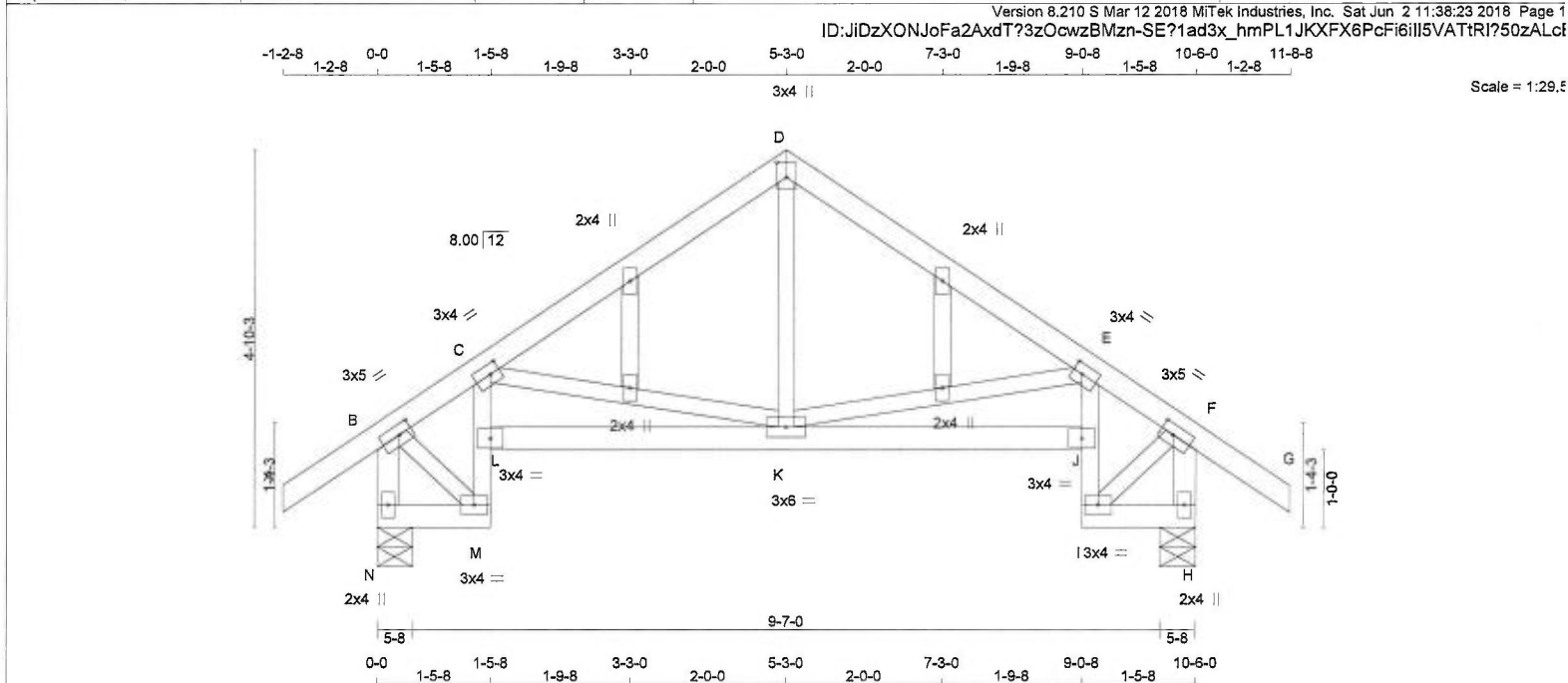
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
NE0618-002.	G05	1	1	GREEN YORK HOMES - GRANELLI HOME CORP - LIANA 2 ELEV 2	PAGE 12 OF 40
TRUSS DESC.					

Version 8.210 S Mar 12 2018 MiTek Industries, Inc. Sat Jun 2 11:38:23 2018 Page 2
ID:JiDzXONJoFa2AxdT?3zOcwzBMzn-SE?1ad3x_hmPL1JKXFX6PcFkyiQy5UXTtRI?50zALcf

JSI GRIP= 0.85 (B) (INPUT = 0.90)
JSI METAL= 0.19 (A) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.





LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER DESCR.
A - D 2x4 DRY No.2 SPF
D - G 2x4 DRY No.2 SPF
N - B 2x4 DRY No.2 SPF
H - F 2x4 DRY No.2 SPF
N - M 2x4 DRY No.2 SPF
M - C 2x3 DRY No.2 SPF
L - J 2x4 DRY No.2 SPF
I - E 2x3 DRY No.2 SPF
I - H 2x4 DRY No.2 SPF
ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
ALL GABLE WEBS 2x3 DRY No.2 SPF
DRY: SEASONED LUMBER.
GABLE STUDS SPACED AT 2'-0" OC.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT		VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
N		602	0	602	0	0	5-8	5-8	
H		602	0	602	0	0	5-8	5-8	

UNFACTORED REACTIONS									
		1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT		COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	
N		420	305 / 0	0 / 0	0 / 0	0 / 0	116 / 0	0 / 0	
H		420	305 / 0	0 / 0	0 / 0	0 / 0	116 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS									
		MAX. FACTORED FORCE		FACTORED VERT. LOAD		MAX. FACTORED FORCE		FACTORED VERT. LOAD	
MEMB.		(LBS)		(PLF)	LC1 MAX	MEMB.		(LBS)	LC1 MAX
FR-TO						FR-TO			
A-B		0 / 28		-77.4	-77.4 0.09 (1)	K-D		0 / 172	0.04 (4)
B-C		-350 / 0		-77.4	-77.4 0.09 (1)	K-E		-182 / 0	0.05 (1)
C-D		-434 / 0		-77.4	-77.4 0.22 (1)	C-K		-182 / 0	0.05 (1)
D-E		-434 / 0		-77.4	-77.4 0.22 (1)	B-M		0 / 328	0.07 (1)
E-F		-350 / 0		-77.4	-77.4 0.09 (1)	I-F		0 / 328	0.07 (1)
F-G		0 / 28		-77.4	-77.4 0.09 (1)				
N-B		-589 / 0		0.0	0.0 0.06 (1)				
H-F		-589 / 0		0.0	0.0 0.06 (1)				
N-M		0 / 0		-18.2	-18.2 0.01 (4)				
M-L		-186 / 0		0.0	0.0 0.70 (1)				
L-C		-158 / 0		0.0	0.0 0.70 (1)				
L-K		0 / 541		-18.2	-18.2 0.13 (1)				
K-J		0 / 541		-18.2	-18.2 0.13 (1)				
I-J		-186 / 0		0.0	0.0 0.70 (1)				
J-E		-158 / 0		0.0	0.0 0.70 (1)				
I-H		0 / 0		-18.2	-18.2 0.01 (4)				

WEBS
MAX. FACTORED FORCE
K-D 0 / 172 0.04 (4)
K-E -182 / 0 0.05 (1)
C-K -182 / 0 0.05 (1)
B-M 0 / 328 0.07 (1)
I-F 0 / 328 0.07 (1)

ALLOWABLE DEFL.(LL)= L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/999 (0.08")

CSI: TC=0.22/1.00 (D-E:1), BC=0.70/1.00 (L-M:1),
WB=0.07/1.00 (F-I:1), SS=0.31/1.00 (C-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

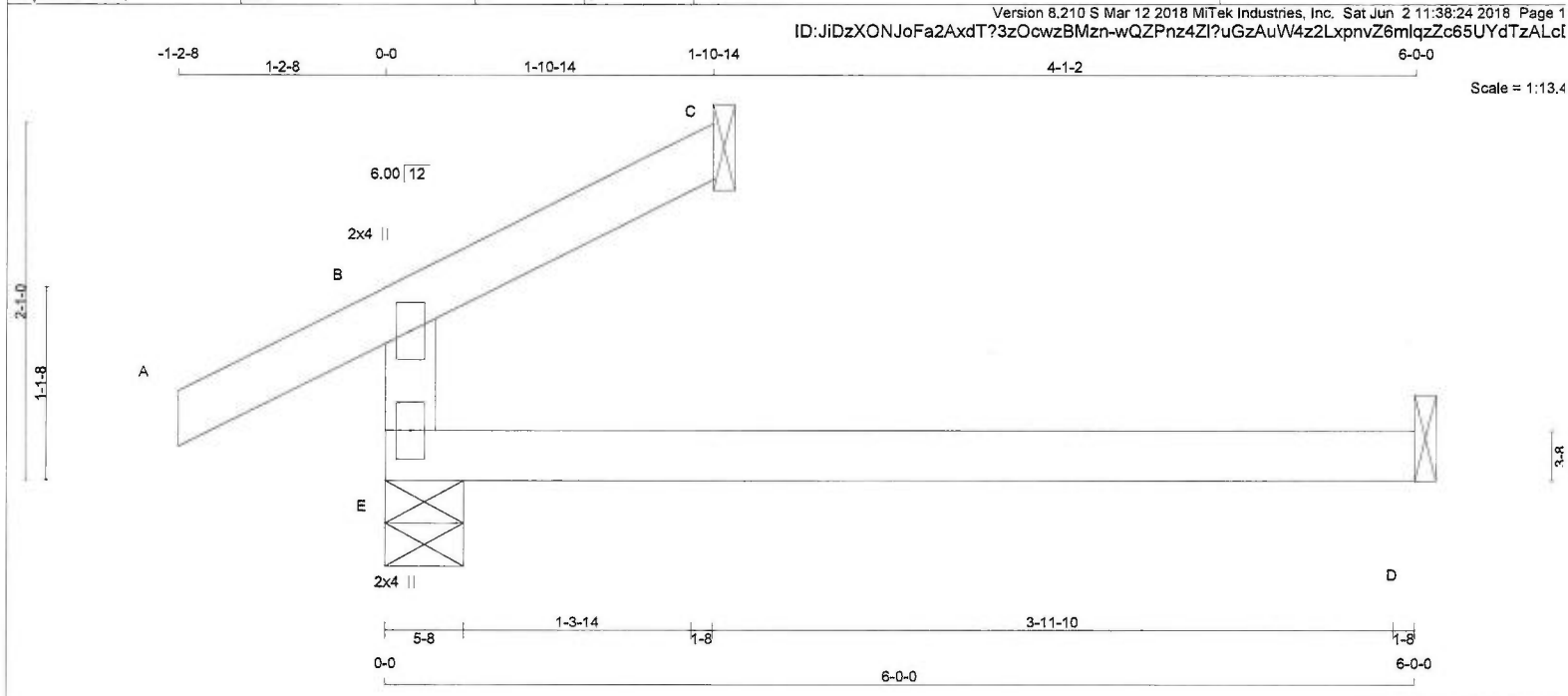
PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.71 (I) (INPUT = 0.90)
JSI METAL= 0.13 (J) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER

N. L. G. A. RULES

CHORDS

SIZE

DRY

LUMBER

DESCR.

SPF

SPF

SPF

E - B

2x4

DRY

No.2

SPF

A - C

2x4

DRY

No.2

SPF

E - D

2x4

DRY

No.2

SPF

DRY: SEASONED LUMBER.

FACTORED

GROSS REACTION

DOWN

HORZ

UPLIFT

BRG

IN-SX

255

0

0

0

5-8

5-8

FACTORED

GROSS REACTION

DOWN

HORZ

UPLIFT

BRG

IN-SX

56

0

0

0

1-8

1-8

FACTORED

GROSS REACTION

DOWN

HORZ

UPLIFT

BRG

IN-SX

45

0

0

0

1-8

1-8

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF

DL = 3.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.3 PSF

TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

PLATES (table is in inches)

JT TYPE

PLATES

W

LEN

Y

X

B

TMV+p

MT20

2.0

4.0

E

BMV1+p

MT20

2.0

4.0

UNFACTORED REACTIONS

1ST LCASE

MAX / MIN

COMPONENT REACTIONS

181

115 / 0

0 / 0

0 / 0

0 / 0

66 / 0

0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MAX. FACTORED

FORCE

VERT. LOAD

LC1

MAX

CS

LC

UNBRAC

MEMB.

MAX. FACTORED

FORCE

MAX

CS

LC

FR-TO

E-B

A-B

B-C

E-D

WEBS

MAX. FACTORED

FORCE

MAX

CS

LC

FR-TO

E-B

A-B

B-C

E-D

LICENSED PROFESSIONAL ENGINEER

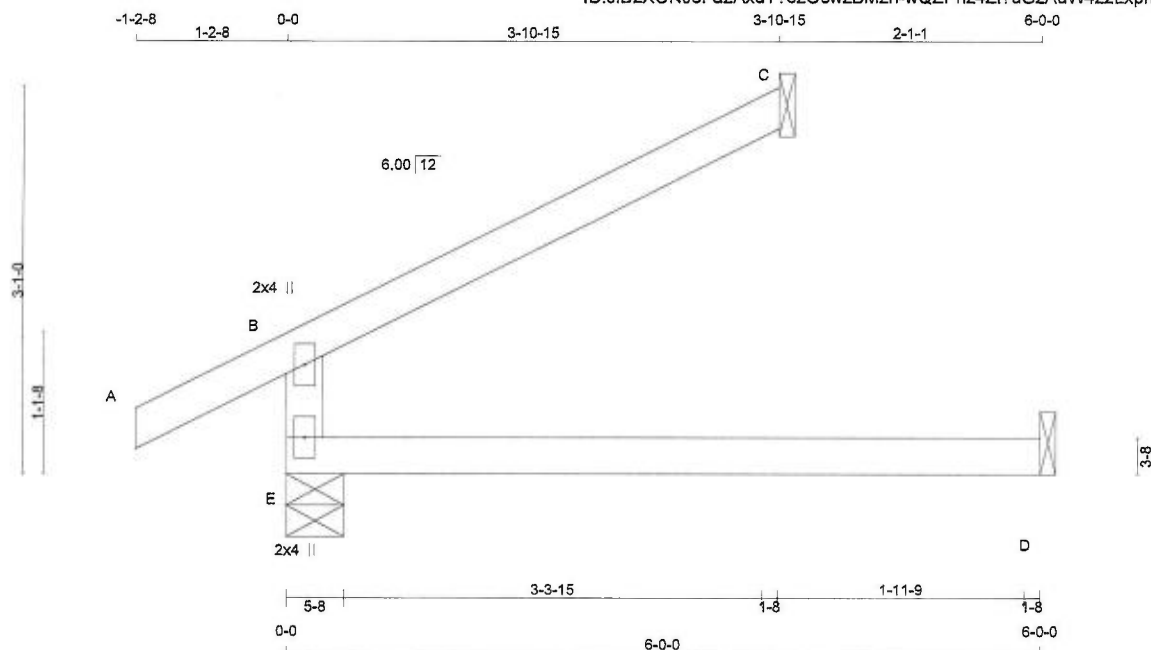
N.A. EL-MASRI

PROVINCIAL OF ONTARIO

Jun 02, 2018

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

KOTT



TOTAL WEIGHT = 2 X 14 = 28 lb
[MT]F

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	DRY	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	2.0	4.0	
E	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX	IN-SX	IN-SX
E	352	0	352	0	5-8	5-8	5-8	5-8
C	114	0	114	0	1-8	1-8	1-8	1-8
D	45	0	50	0	1-8	1-8	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX/MIN. SNOW		LIVE		PERM. LIVE		WIND		DEAD		SOIL	
	COMBINED	SNOW	SNOW	LIVE	LIVE	PERM. LIVE	WIND	DEAD	SOIL	COMBINED	SNOW	SNOW	LIVE	LIVE
E	247	173 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	74 / 0	0 / 0					
C	77	68 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0					
D	36	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0					

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		WEBS		FACTORED	
	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (PLF)	LC1 MAX (PLF)	MAX. UNBRAC LENGTH FR-TO	MEMB. FORCE (LBS)	MAX. FORCE (LBS)	MAX. FORCE (LBS)
E-B	-288 / 0	0.0	0.0	0.11 (4)	7.81			
A-B	0 / 22	-77.4	-77.4	0.09 (1)	10.00			
B-C	-17 / 0	-77.4	-77.4	0.15 (1)	6.25			
E-D	0 / 0	-18.2	-18.2	0.14 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.15/1.00 (B-C:1), BC=0.14/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.13/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

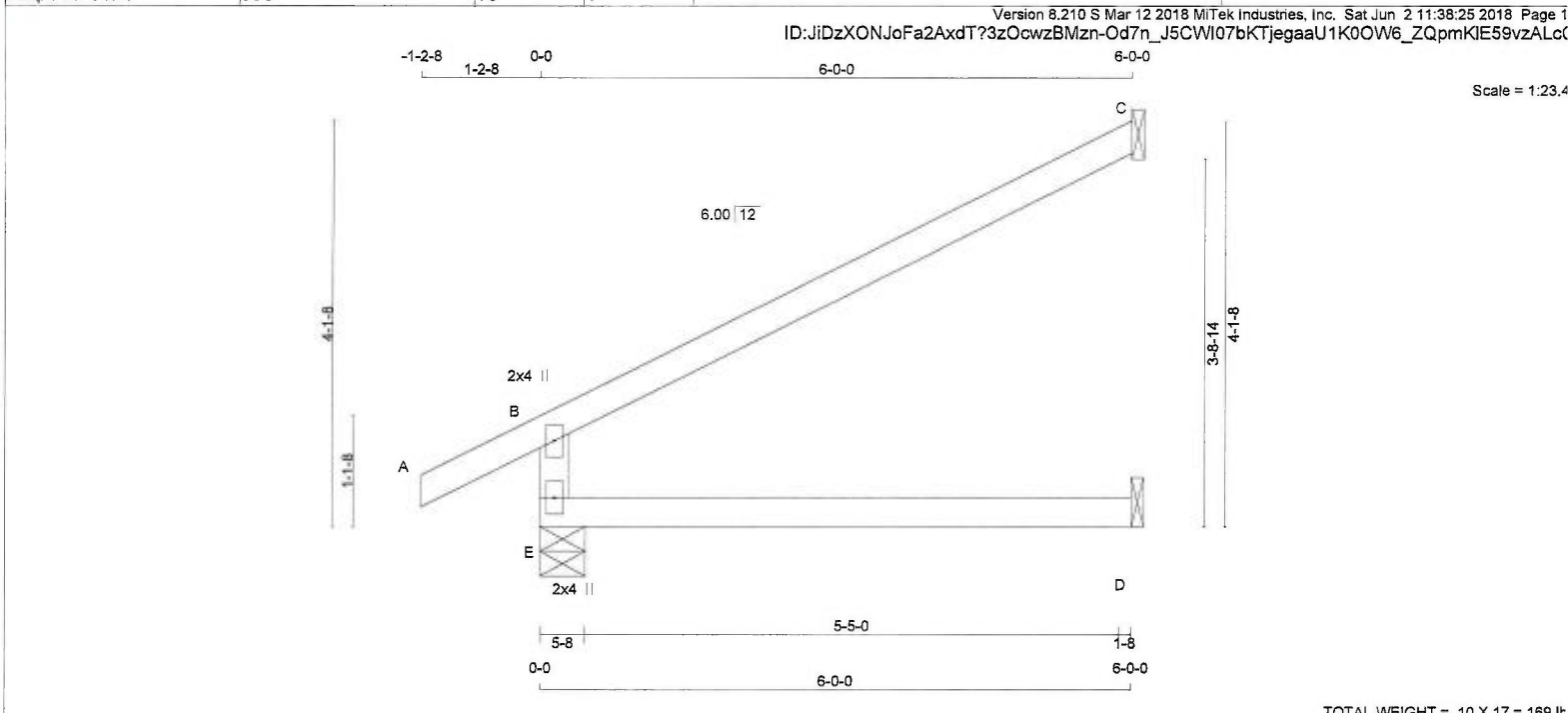
PLATE ROTATION TOL. = 5.0 Deg.

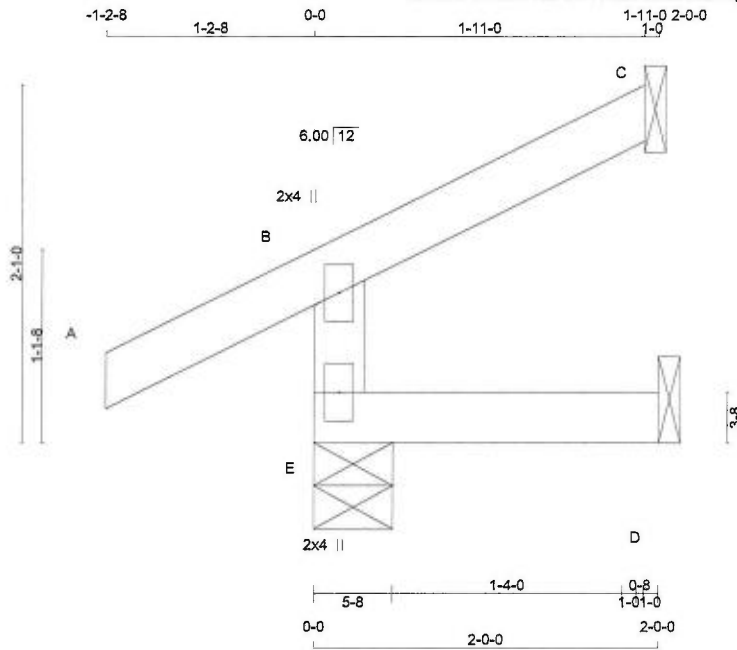
JSI GRIP= 0.17 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.







TOTAL WEIGHT = 2 X 7 = 14 lb
[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
E	211	0	211	0	5-8	5-8
C	56	0	56	0	1-8	1-8
D	16	0	18	0	1-8	1-8

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 23.3 PSF
 DL = 3.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.3 PSF
 TOTAL LOAD = 33.6 PSF

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS							
JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	146	115 / 0	0 / 0	0 / 0	0 / 0	31 / 0	0 / 0
C	38	34 / 0	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	13	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS					WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MAX. UNBRAC LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM	TO		FR-TO		
E-B	-191 / 0	0.0	0.0	0.01 (4)	7.81		
A-B	0 / 22	-77.4	-77.4	0.09 (1)	10.00		
B-C	-8 / 0	-77.4	-77.4	0.05 (1)	10.00		
E-D	0 / 0	-18.2	-18.2	0.02 (4)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, CBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

DESIGN ASSUMPTIONS
 - OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
 ALLOWABLE DEFL.(TL)= L/360 (0.19")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.09/1.00 (A-B:1), BC=0.02/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.07/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

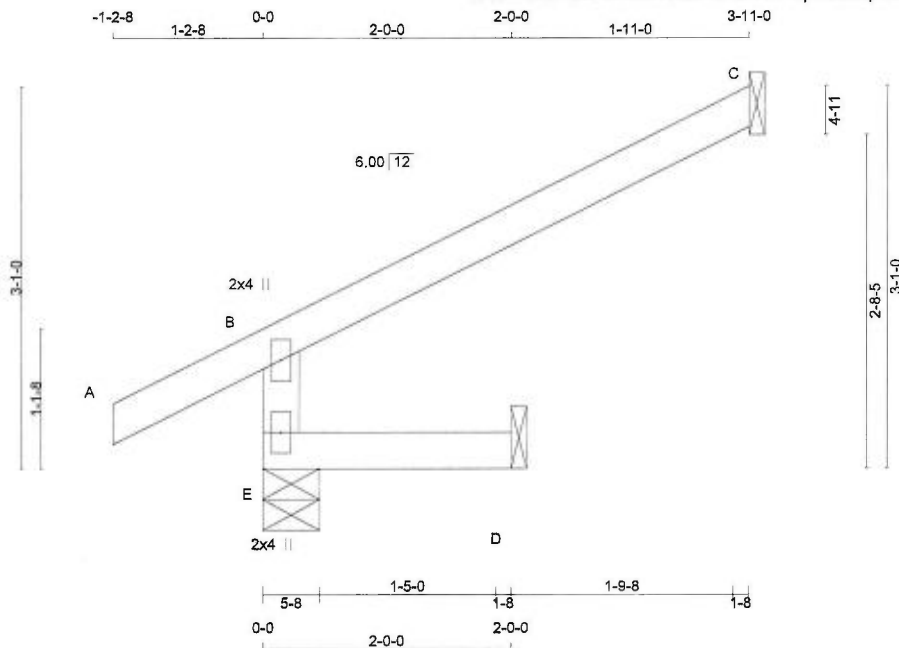
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.11 (B) (INPUT = 0.90)
 JSI METAL= 0.04 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:18.6

TOTAL WEIGHT = 2 X 10 = 19 lb [M]

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	2.0	4.0	
E	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT		VERT	HORZ	DOWN	HORZ	IN-SX		IN-SX	
E	308	0	0	308	0	5-8		5-8	
C	114	0	0	114	0	1-8		1-8	
D	16	0	0	18	0	1-8		1-8	

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS		PERM.LIVE		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	SNOW	LIVE	PERM.LIVE	WIND
E	212	173 / 0	0 / 0	0 / 0	0 / 0	39 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0
C	77	68 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0
D	13	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM	TO	FR-TO			
E-B	-288 / 0	0.0	0.0 0.01 (4)	7.81			
A-B	0 / 22	-77.4	-77.4 0.09 (1)	10.00			
B-C	-17 / 0	-77.4	-77.4 0.20 (1)	6.25			
E-D	0 / 0	-18.2	-18.2 0.02 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.20/1.00 (B-C:1), BC=0.02/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.13/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

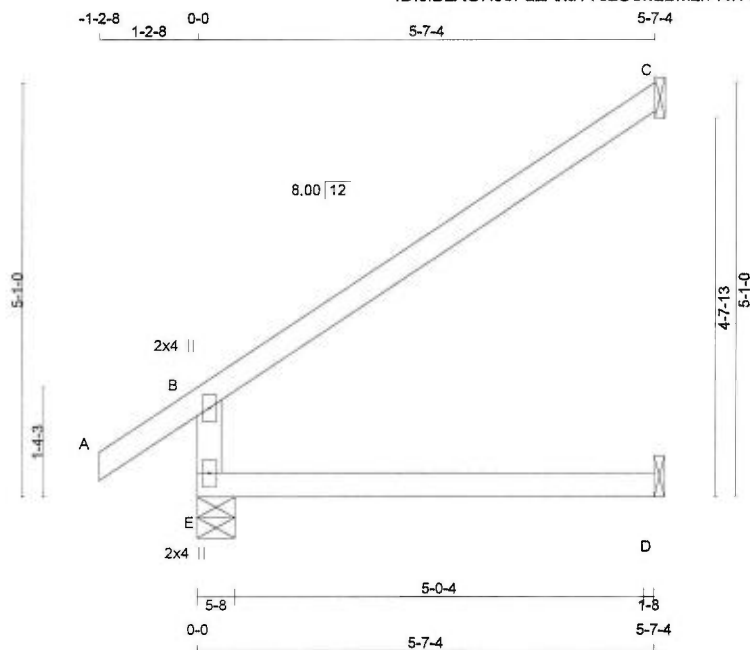
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:28.3

TOTAL WEIGHT = 2 X 17 = 34 lb [M]

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
E	430	0	430	0	5-8	5-8
C	163	0	163	0	1-8	1-8
D	43	0	48	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	299	223 / 0	0 / 0	0 / 0	0 / 0	76 / 0	0 / 0
C	110	98 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0
D	34	0 / 0	0 / 0	0 / 0	0 / 0	34 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS			WEBS		
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	MAX. FACTORED CSI (LC)
FR-TO		FROM	TO	FR-TO		
E-B	-371 / 0	0.0	0.0	0.11 (4)	7.81	
A-B	0 / 28	-77.4	-77.4	0.09 (1)	10.00	
B-C	-30 / 0	-77.4	-77.4	0.41 (1)	6.25	
E-D	0 / 0	-18.2	-18.2	0.12 (4)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.41/1.00 (B-C:1), BC=0.12/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.18/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

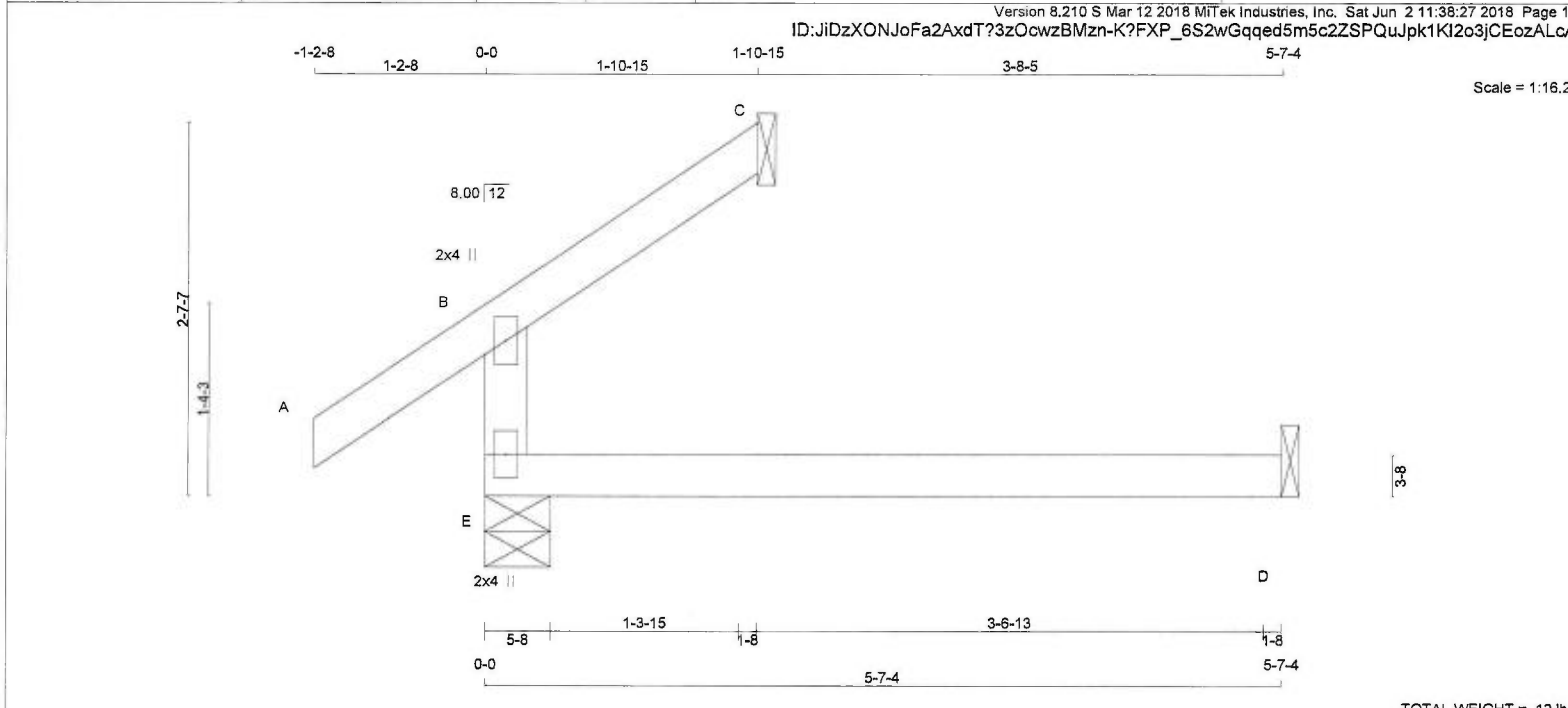
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (B) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER

N. L. G. A. RULES

CHORDS SIZE

LUMBER

DESCR.

SPF

SPF

E - B

2x4

DRY

No.2

SPF

SPF

A - C

2x4

DRY

No.2

SPF

SPF

E - D

2x4

DRY

No.2

SPF

SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
	VERT	HORZ	UPLIFT	IN-SX
E	251	0	0	5-8
C	56	0	0	1-8
D	43	0	0	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	MAX/MIN. LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	178	115 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0
C	38	34 / 0	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	34	0 / 0	0 / 0	0 / 0	0 / 0	34 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	FACTORED	FACTORED	FACTORED	MEMB.	FACTORED	FACTORED	FACTORED
	MAX. FORCE	VERT. LOAD	LC1 MAX	CS1 (LC)		MAX. FORCE	FACTORED	FACTORED
	(LBS)	(PLF)	CS1 (LC)	UNBRAC		(LBS)	CS1 (LC)	CS1 (LC)
		FROM TO	LENGTH	FR-TO				
E-B	-192 / 0	0.0	0.0	0.11 (4)	7.61			
A-B	0 / 28	-77.4	-77.4	0.09 (1)	10.00			
B-C	-10 / 0	-77.4	-77.4	0.05 (1)	10.00			
E-D	0 / 0	-18.2	-18.2	0.12 (4)	10.00			

LICENSED PROFESSIONAL ENGINEER

N.A. EL-MASRI

PROVINCIAL OF ONTARIO

Jun 02, 2018

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF

DL = 3.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.3 PSF

TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014

- CSA 086-09

- TPIC 2011

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")

CALCULATED VERT. DEFL.(LL) = L/999 (0.00")

ALLOWABLE DEFL.(TL)= L/360 (0.19")

CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.11/1.00 (B-E:4), BC=0.12/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.08/1.00 (D-E:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

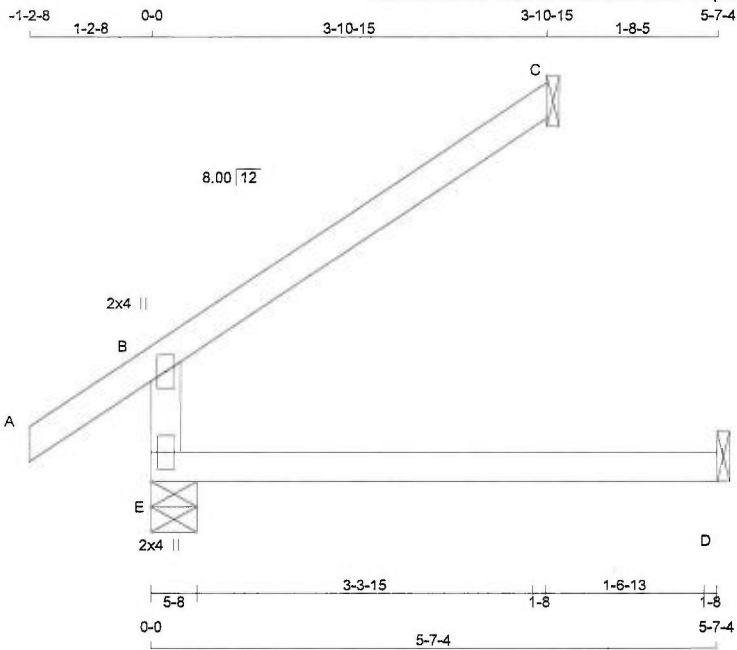
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	818	354	1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.12 (B) (INPUT = 0.90)

JSI METAL= 0.05 (B) (INPUT = 1.00)



Scale = 1:22.5

TOTAL WEIGHT = 15 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	2.0	4.0	
E	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	BRG
E	348	0	0	348	0	0	0	5-8	5-8
C	114	0	0	114	0	0	0	1-8	1-8
D	43	0	0	48	0	0	0	1-8	1-8

SEE MITTEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS							
JT	1ST CASE	MAX/MIN	SNOW	LIVE	PERM.LIVE	WIND	DEAD
E	244	174 / 0	0 / 0	0 / 0	0 / 0	0 / 0	70 / 0
C	77	68 / 0	0 / 0	0 / 0	0 / 0	0 / 0	9 / 0
D	34	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	34 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING							
TOTAL LOAD CASES: (4)							
CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED HORZ. LOAD (PLF)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED HORZ. LOAD (PLF)
E-B	-289 / 0	0.0	0.0	0.11 (4)	7.81		
A-B	0 / 28	-77.4	-77.4	0.09 (1)	10.00		
B-C	-21 / 0	-77.4	-77.4	0.20 (1)	6.25		
E-D	0 / 0	-18.2	-18.2	0.12 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , CBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.20/1.00 (B-C:1) , BC=0.12/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES					
PLATE	GRIP(DRY)	SHEAR	SECTION	(PSI)	(PLI)
	MAX	MIN	MAX	MIN	MAX
MT20	618	354	1667	822	2284

PLATE PLACEMENT TOL. = 0.250 inches

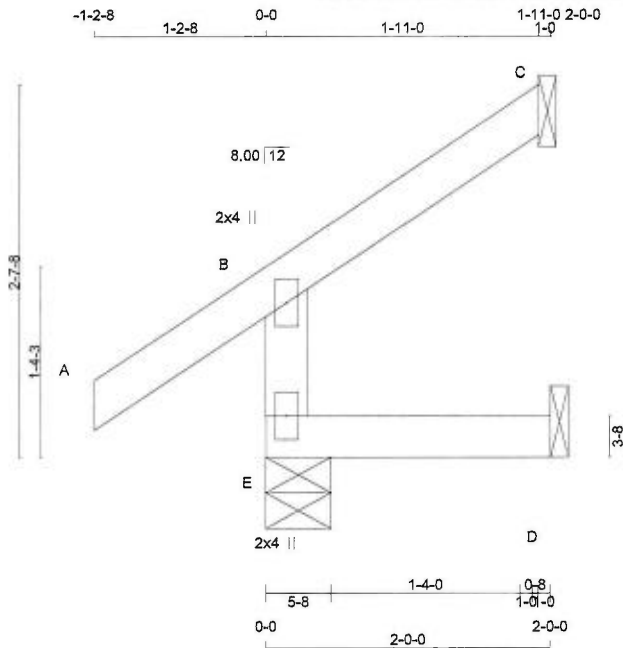
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:16.2

TOTAL WEIGHT = 8 lb
[M]

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	2.0	4.0	
E	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	212	0	212	0	5-8	5-8
C	56	0	56	0	1-8	1-8
D	16	0	18	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. LIVE		PERM.LIVE		WIND	DEAD	SOIL
	SNOW	SNOW	SNOW	SNOW	SNOW	SNOW			
E	148	115 / 0	0 / 0	0 / 0	0 / 0	0 / 0	31 / 0	0 / 0	0 / 0
C	38	34 / 0	0 / 0	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0	0 / 0
D	13	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	C H O R D S		F A C T O R E D		W E B S		F A C T O R E D	
	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (PLF)	LC1 MAX (LC)	MAX. UNBRAC LENGTH FR-TO	MEMB. FR-TO	MAX. FORCE (LBS)	MAX (LC)
FR-TO								
E-B	-192 / 0	0.0	0.0	0.01 (4)	7.81			
A-B	0 / 28	-77.4	-77.4	0.09 (1)	10.00			
B-C	-10 / 0	-77.4	-77.4	0.05 (1)	10.00			
E-D	0 / 0	-18.2	-18.2	0.02 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , CBC 2012 , ABC 2014
- CSA 088-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.09/1.00 (A-B:1) , BC=0.02/1.00 (D-E:4) ,
WB=0.00/1.00 (n/a:0) , SSI=0.06/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

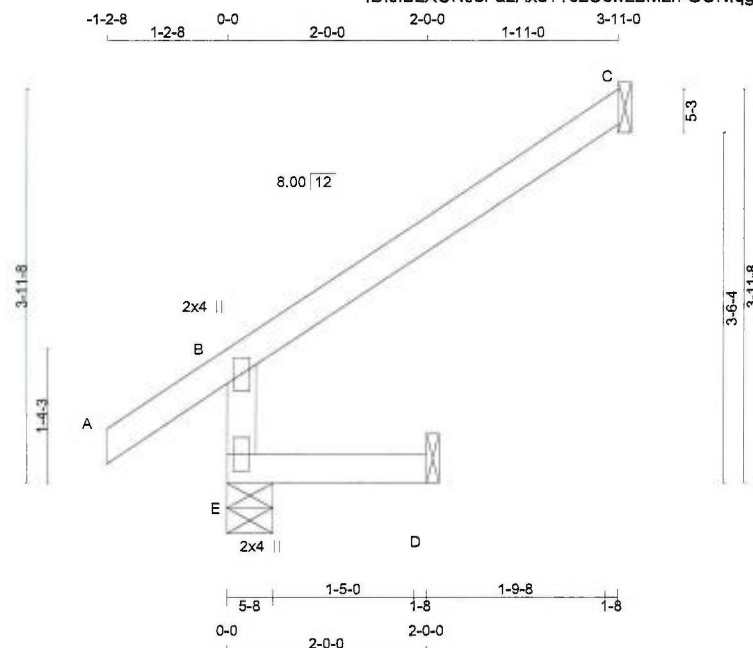
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.12 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:23.1

TOTAL WEIGHT = 11 lb
[M]

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	2.0	4.0	
E	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX	IN-SX	IN-SX
E	309	0	309	0	5-8	5-8	5-8	5-8
C	114	0	114	0	1-8	1-8	1-8	1-8
D	16	0	18	0	1-8	1-8	1-8	1-8

SEE MITTEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
E	212	174 / 0	0 / 0	0 / 0	0 / 0	38 / 0	0 / 0
C	77	68 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0
D	13	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		WEBS		FACTORED	
	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (PLF)	LC1 MAX (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FORCE (LBS)	MAX. CSI (LC)
E-B	-289 / 0	0.0	0.0	0.01 (4)	7.81			
A-B	0 / 28	-77.4	-77.4	0.09 (1)	10.00			
B-C	-21 / 0	-77.4	-77.4	0.20 (1)	6.25			
E-D	0 / 0	-18.2	-18.2	0.02 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.20/1.00 (B-C:1), BC=0.02/1.00 (D-E:4),
WB=0.00/1.00 (n/a:0), SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

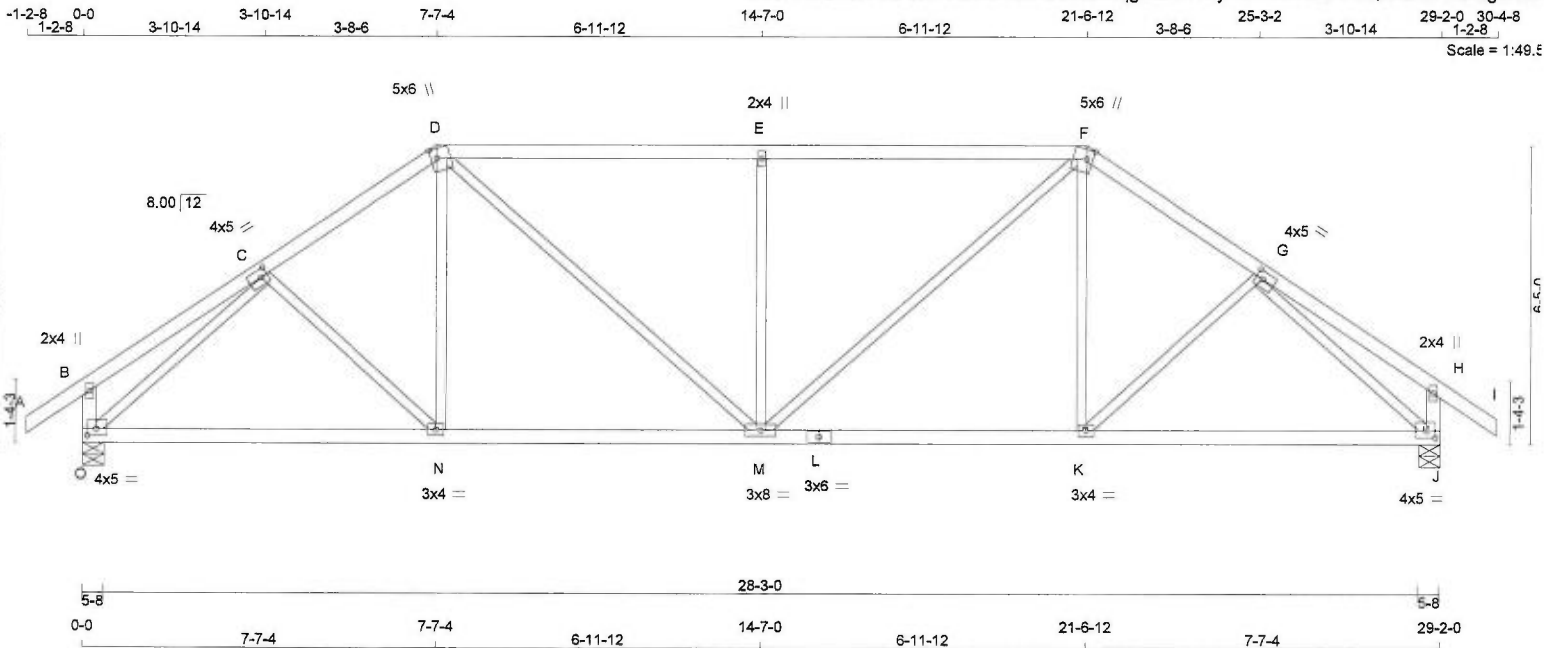
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 2 X 120 = 241 lb
[M]F

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
C - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
O - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
O - L	2x4	DRY	No.2
L - J	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT		VERT	HORZ	UPLIFT	IN-SX
O		1494	0	0	5-8
J		1494	0	0	5-8

UNFACTORED REACTIONS		1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT		COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
O		1047	739 / 0	0 / 0	0 / 0	0 / 0	308 / 0	0 / 0
J		1047	739 / 0	0 / 0	0 / 0	0 / 0	308 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.41 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	FORCE (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORZ. LOAD (LBS)
FR-TO		FROM TO	LENGTH
A-B	0 / 28	-77.4 -77.4 0.09 (1)	10.00
B-C	0 / 17	-77.4 -77.4 0.16 (1)	10.00
C-D	-1580 / 0	-77.4 -77.4 0.16 (1)	5.11
D-E	-1742 / 0	-77.4 -77.4 0.55 (1)	4.41
E-F	-1742 / 0	-77.4 -77.4 0.55 (1)	4.41
F-G	-1580 / 0	-77.4 -77.4 0.16 (1)	5.11
G-H	0 / 17	-77.4 -77.4 0.16 (1)	10.00
H-I	0 / 28	-77.4 -77.4 0.09 (1)	10.00
O-B	-215 / 0	0.0 0.0 0.02 (1)	7.81
J-H	-215 / 0	0.0 0.0 0.02 (1)	7.81
O-N	0 / 1303	-18.2 -18.2 0.35 (1)	10.00
N-M	0 / 1301	-18.2 -18.2 0.35 (4)	10.00
M-L	0 / 1301	-18.2 -18.2 0.35 (4)	10.00
L-K	0 / 1301	-18.2 -18.2 0.35 (4)	10.00
K-J	0 / 1304	-18.2 -18.2 0.35 (1)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/999 (0.14")

CSI: TC=0.55/1.00 (E-F), BC=0.35/1.00 (K-M:4),
WB=0.78/1.00 (G-J:1), SSI=0.26/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

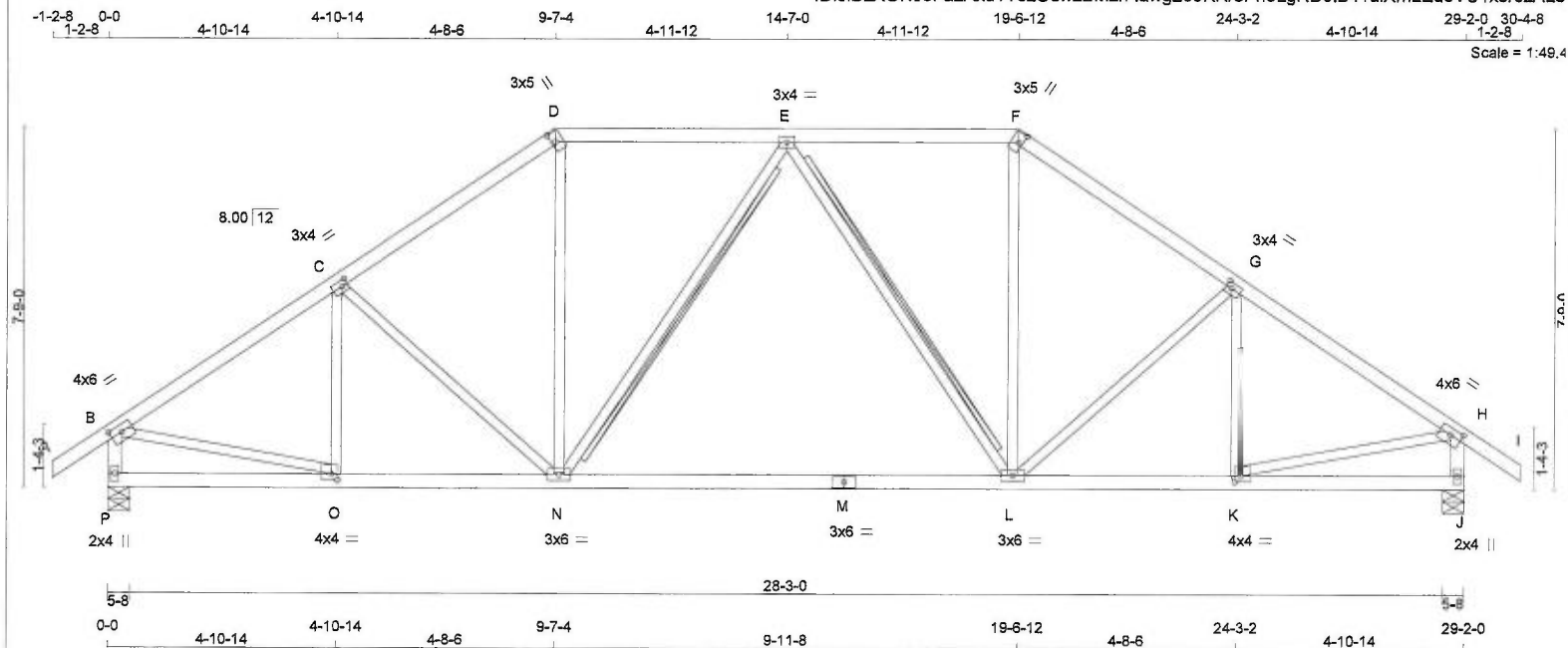
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (G) (INPUT = 0.90)
JSI METAL= 0.53 (C) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	1.75	3.00
C	TMVW-t	MT20	3.0	4.0	1.50	1.50
D	TTW+h	MT20	3.0	5.0	2.50	1.00
E	TMVW-t	MT20	3.0	4.0		
F	TTW+h	MT20	3.0	5.0	2.50	1.00
G	TMVW-t	MT20	3.0	4.0	1.50	1.50
H	TMVW-t	MT20	4.0	6.0	1.75	3.00
J	BMV1+p	MT20	2.0	4.0		
K	BMVW-t	MT20	4.0	4.0	1.50	1.50
L	BMVW-t	MT20	3.0	6.0		
M	BS-t	MT20	3.0	6.0		
N	BMVW-t	MT20	3.0	6.0		
O	BMVW-t	MT20	4.0	4.0	1.50	1.50
P	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	UPLIFT	IN-SX
P	1494	0	0	5-8
J	1494	0	0	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
P	1047	739 / 0	0 / 0	0 / 0	0 / 0	308 / 0	0 / 0	0 / 0
J	1047	739 / 0	0 / 0	0 / 0	0 / 0	308 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.94 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT E-N, E-L

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 28	-77.4	-77.4 0.09 (1)	O-C	-220 / 0	0.07 (1)	
B-C	-1628 / 0	-77.4	-77.4 0.26 (1)	C-N	-208 / 0	0.16 (1)	
C-D	-1492 / 0	-77.4	-77.4 0.25 (1)	N-D	0 / 529	0.12 (1)	
D-E	-1227 / 0	-77.4	-77.4 0.26 (1)	E-N	-280 / 0	0.18 (1)	
E-F	-1227 / 0	-77.4	-77.4 0.26 (1)	F-L	-280 / 0	0.18 (1)	
F-G	-1492 / 0	-77.4	-77.4 0.25 (1)	L-F	0 / 529	0.12 (1)	
G-H	-1629 / 0	-77.4	-77.4 0.26 (1)	H-G	-209 / 0	0.16 (1)	
H-I	0 / 28	-77.4	-77.4 0.09 (1)	I-G	-219 / 0	0.07 (1)	
P-B	-1451 / 0	0.0	0.0 0.15 (1)	B-O	0 / 1404	0.32 (1)	
J-H	-1451 / 0	0.0	0.0 0.15 (1)	K-H	0 / 1405	0.32 (1)	
P-O	0 / 0	-18.2	-18.2 0.11 (4)				
O-N	0 / 1374	-18.2	-18.2 0.38 (1)				
N-M	0 / 1380	-18.2	-18.2 0.38 (1)				
M-L	0 / 1380	-18.2	-18.2 0.38 (1)				
L-K	0 / 1375	-18.2	-18.2 0.38 (1)				
K-J	0 / 0	-18.2	-18.2 0.11 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL) = L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/999 (0.24")

CSI: TC=0.26/1.00 (G-H:1), BC=0.38/1.00 (L-N:1), WB=0.32/1.00 (H-K:1), SSI=0.19/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

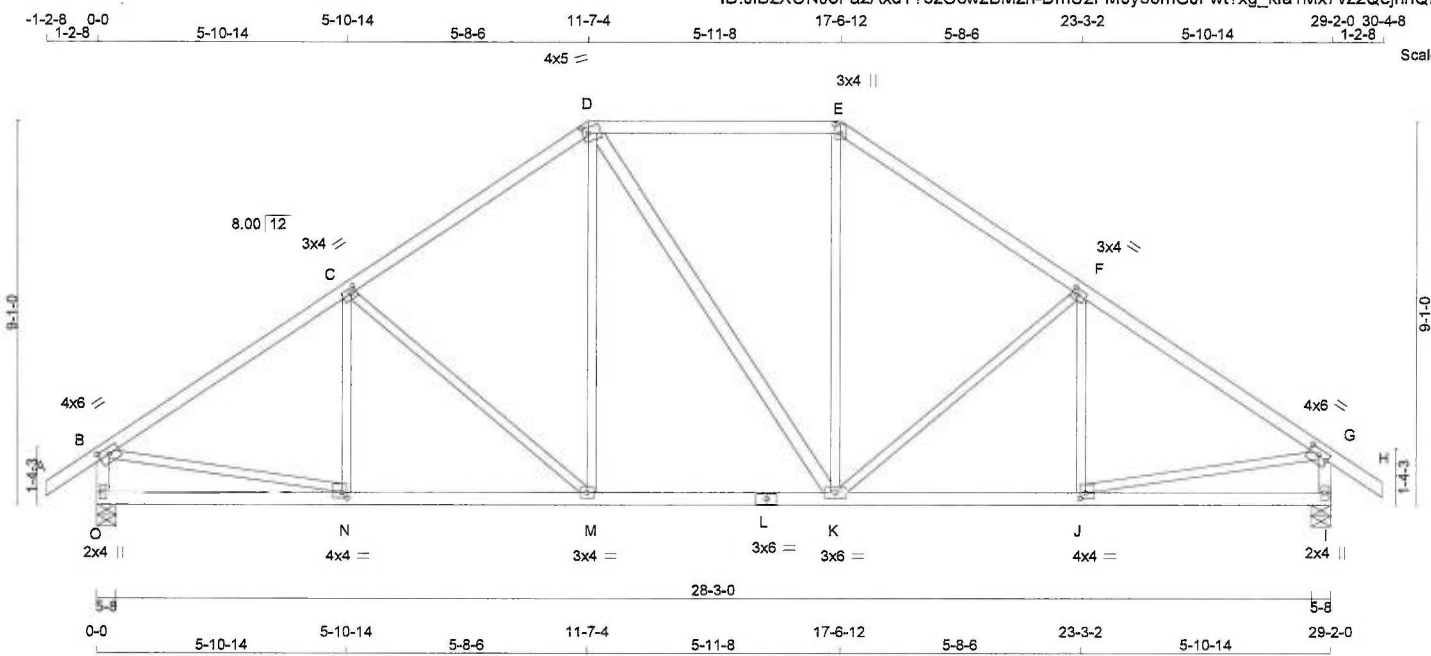
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (D) (INPUT = 0.90)
JSI METAL= 0.54 (M) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - H	2x4	DRY	No.2
O - B	2x4	DRY	No.2
I - G	2x4	DRY	No.2
O - L	2x4	DRY	No.2
L - I	2x4	DRY	No.2
ALL WEBS EXCEPT D - K	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	1.75	3.00
C	TMVW-t	MT20	3.0	4.0	1.50	1.50
D	TTWW-m	MT20	4.0	5.0	1.75	1.50
E	TTW+p	MT20	3.0	4.0	2.50	1.50
F	TMVW-t	MT20	3.0	4.0	1.50	1.50
G	TMVW-t	MT20	4.0	6.0	1.75	3.00
I	BMV1+p	MT20	2.0	4.0		
J	BMVW-t	MT20	4.0	4.0	1.50	1.50
K	BMVW-t	MT20	3.0	6.0		
L	BS-t	MT20	3.0	6.0		
M	BMVW-t	MT20	3.0	4.0		
N	BMVW-t	MT20	4.0	4.0	1.50	1.50
O	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
O	1494	1494	5-8	5-8
I	1494	1494	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX/MIN. SNOW	MAX/MIN. LIVE	MAX/MIN. PERM. LIVE	WIND	DEAD	SOIL
O	1047	739 / 0	0 / 0	0 / 0	0 / 0	308 / 0	0 / 0
I	1047	739 / 0	0 / 0	0 / 0	0 / 0	308 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)
FR-TO		FROM TO		FR-TO		FROM TO	
A-B	0 / 28	-77.4	-77.4 0.09 (1)	N-C	-126 / 62	0.05 (1)	
B-C	-1649 / 0	-77.4	-77.4 0.40 (1)	C-M	-375 / 0	0.44 (1)	
C-D	-1368 / 0	-77.4	-77.4 0.37 (1)	M-D	0 / 347	0.08 (1)	
D-E	-1116 / 0	-77.4	-77.4 0.37 (1)	D-K	0 / 0	0.00 (1)	
E-F	-1369 / 0	-77.4	-77.4 0.37 (1)	K-E	0 / 348	0.08 (1)	
F-G	-1649 / 0	-77.4	-77.4 0.40 (1)	K-F	-374 / 0	0.44 (1)	
G-H	0 / 28	-77.4	-77.4 0.09 (1)	J-F	-127 / 61	0.05 (1)	
O-B	-1449 / 0	0.0	0.0 0.15 (1)	B-N	0 / 1417	0.32 (1)	
I-G	-1449 / 0	0.0	0.0 0.15 (1)	J-G	0 / 1417	0.32 (1)	
O-N	0 / 0	-18.2	-18.2 0.15 (4)				
N-M	0 / 1396	-18.2	-18.2 0.30 (1)				
M-L	0 / 1116	-18.2	-18.2 0.25 (1)				
L-K	0 / 1116	-18.2	-18.2 0.25 (1)				
K-J	0 / 1397	-18.2	-18.2 0.29 (1)				
J-I	0 / 0	-18.2	-18.2 0.15 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CSI: TC=0.40/1.00 (F-G:1), BC=0.30/1.00 (M-N:1), WB=0.44/1.00 (C-M:1), SS=0.18/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618 354 1667 822 2284 1656	

PLATE PLACEMENT TOL. = 0.250 inches

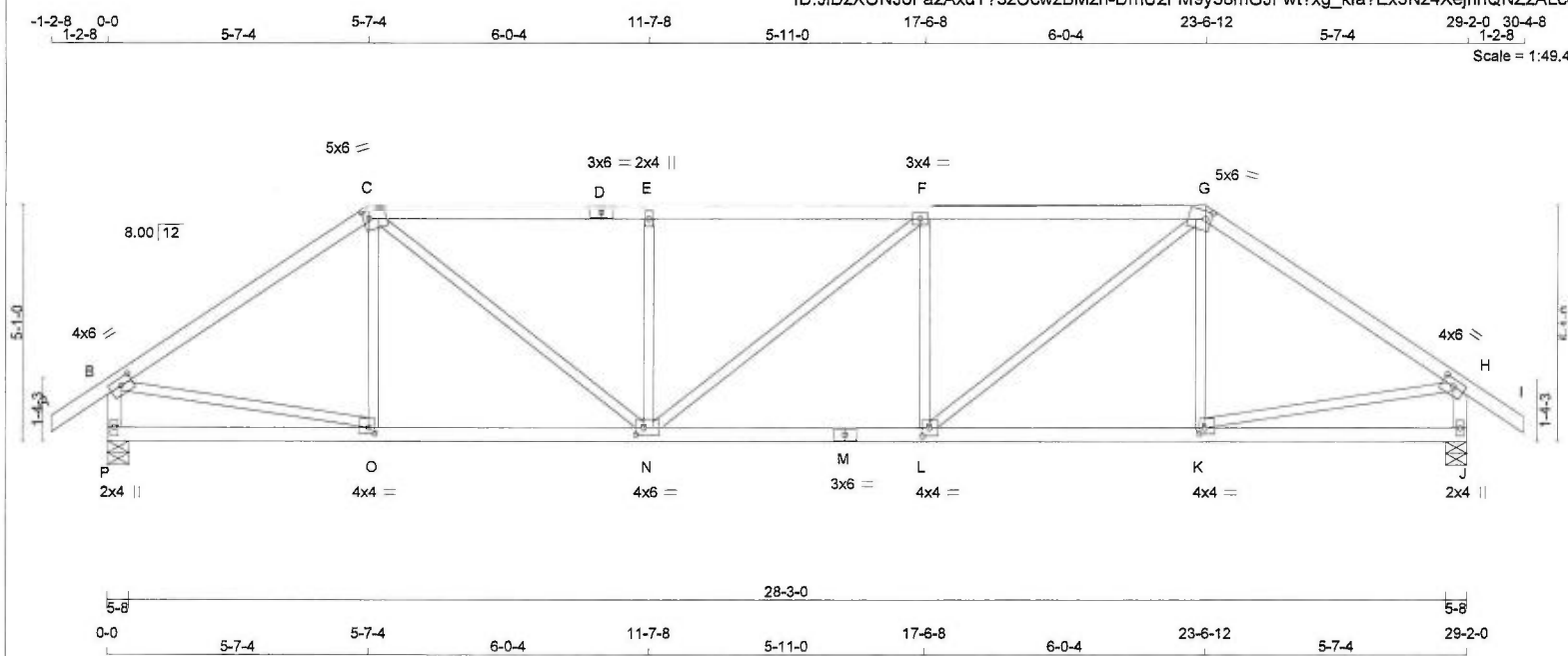
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (E) (INPUT = 0.90)
JSI METAL= 0.52 (J) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER

N. L. G. A. RULES

CHORDS

SIZE

DRY

LUMBER

DESCR.

SPF

ALL WEBS EXCEPT

2x3

DRY

No.2

SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT

TYPE

PLATES

W

LEN

Y

X

B

TMVW-t

MT20

4.0

6.0

1.75

3.00

C

TTWW-m

MT20

5.0

6.0

2.00

1.50

D

TS-t

MT20

3.0

6.0

E

TMVW-w

MT20

2.0

4.0

F

TMVW-t

MT20

3.0

4.0

G

TTWW-m

MT20

5.0

6.0

2.00

1.50

H

TMVW-t

MT20

4.0

6.0

1.75

3.00

J

BMV1+p

MT20

2.0

4.0

K

BMVW-t

MT20

4.0

4.0

1.50

1.50

L

BMVW-t

MT20

4.0

4.0

2.00

1.75

M

BS-t

MT20

3.0

6.0

N

BMVW-t

MT20

4.0

6.0

1.75

2.00

O

BMVW-t

MT20

4.0

4.0

1.50

1.50

P

BMV1+p

MT20

2.0

4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX	IN-SX	IN-SX
P	1494	0	1494	0	0	5-8	5-8	5-8
J	1494	0	1494	0	0	5-8	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE

MAX./MIN. COMPONENT REACTIONS

JT

COMBINED

SNOW

LIVE

PERM.LIVE

WIND

DEAD

SOIL

P

1047

739 / 0

0 / 0

0 / 0

0 / 0

308 / 0

0 / 0

J

1047

739 / 0

0 / 0

0 / 0

0 / 0

308 / 0

0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.18 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM	TO	LENGTH	FR-TO				
A-B	0 / 28	-77.4	-77.4	0.09 (1)	10.00	O-C	-121 / 59	0.05 (1)	
B-C	-1609 / 0	-77.4	-77.4	0.53 (1)	4.60	C-N	0 / 972	0.22 (1)	
C-D	-2097 / 0	-77.4	-77.4	0.49 (1)	4.19	N-E	-499 / 0	0.19 (1)	
D-E	-2097 / 0	-77.4	-77.4	0.49 (1)	4.19	N-F	-3 / 0	0.00 (1)	
E-F	-2096 / 0	-77.4	-77.4	0.50 (1)	4.18	L-F	-500 / 0	0.19 (1)	
F-G	-2098 / 0	-77.4	-77.4	0.50 (1)	4.18	L-G	0 / 974	0.22 (1)	
G-H	-1610 / 0	-77.4	-77.4	0.53 (1)	4.60	K-G	-121 / 59	0.05 (1)	
H-I	0 / 28	-77.4	-77.4	0.09 (1)	10.00	B-O	0 / 1359	0.31 (1)	
P-B	-1453 / 0	0.0	0.0	0.15 (1)	6.78	K-H	0 / 1359	0.31 (1)	
J-H	-1452 / 0	0.0	0.0	0.15 (1)	6.78				
P-O	0 / 0	-18.2	-18.2	0.15 (4)	10.00				
O-N	0 / 1335	-18.2	-18.2	0.30 (1)	10.00				
N-M	0 / 2098	-18.2	-18.2	0.39 (1)	10.00				
M-L	0 / 2098	-18.2	-18.2	0.39 (1)	10.00				
L-K	0 / 1335	-18.2	-18.2	0.30 (1)	10.00				
K-J	0 / 0	-18.2	-18.2	0.15 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF

DL = 3.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.3 PSF

TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014

- CSA 086-09

- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")

CALCULATED VERT. DEFL.(LL) = L/999 (0.10")

ALLOWABLE DEFL.(TL)= L/360 (0.97")

CALCULATED VERT. DEFL.(TL) = L/999 (0.19")

CSI: TC=0.53/1.00 (G-H:1), BC=0.39/1.00 (L-N:1), WB=0.31/1.00 (H-K:1), SSI=0.22/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (N) (INPUT = 0.90)

JSI METAL= 0.57 (M) (INPUT = 1.00)



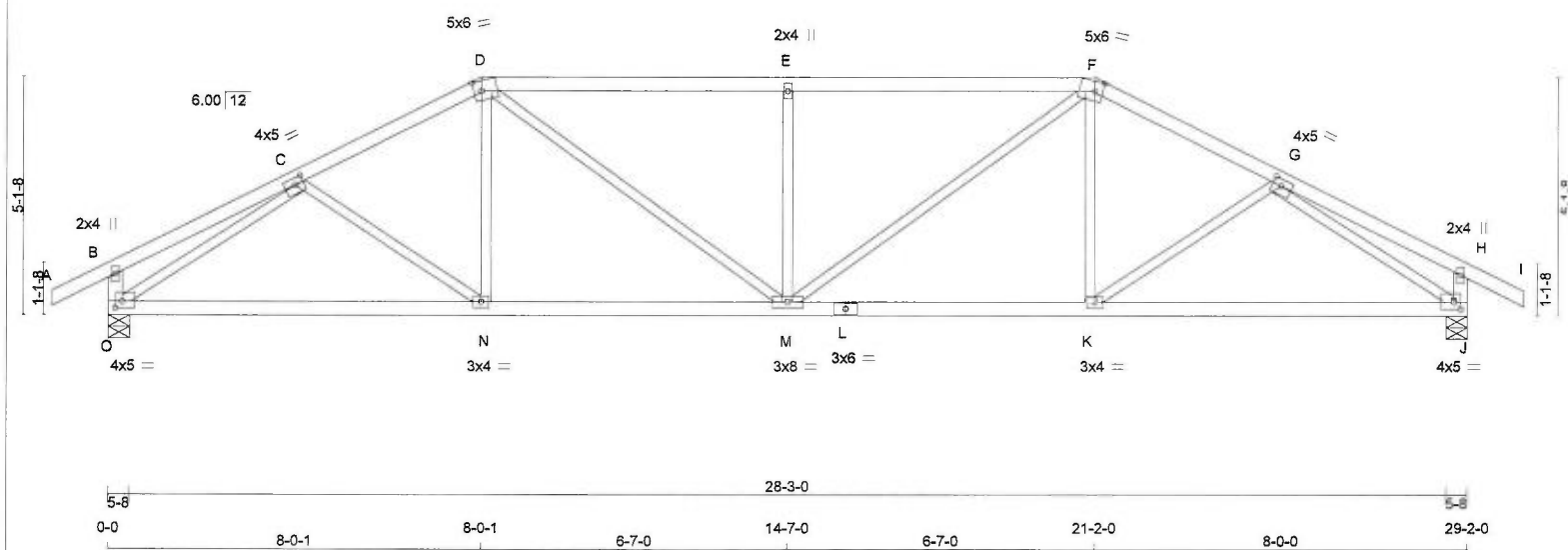
READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



Version 8.210 S Mar 12 2018 MiTek Industries, Inc. Sat Jun 2 11:38:32 2018 Page 1
ID:JiDzXONJoFa2AxdT?3zOcwzBMzn-hz2QSiAbsSu7xPV3YeCDGV6AEKR8iPgoxLQzv?zALc

-1-2-8 0-0 4-1-4 4-1-4 3-10-12 8-0-1 6-7-0 14-7-0 6-7-0 21-2-0 3-10-12 25-0-12 4-1-4 29-2-0 30-4-8 1-2-8

Scale = 1:49.4



LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER DESCR.

A - D 2x4 DRY No.2 SPF
D - F 2x4 DRY No.2 SPF
F - I 2x4 DRY No.2 SPF
O - B 2x4 DRY No.2 SPF
J - H 2x4 DRY No.2 SPF
O - L 2x4 DRY No.2 SPF
L - J 2x4 DRY No.2 SPF
ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	DOWN	IN-SX	IN-SX
O	1493	0	5-8	5-8
J	1493	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
O	1047	739 / 0	0 / 0	0 / 0	0 / 0	308 / 0	0 / 0
J	1047	739 / 0	0 / 0	0 / 0	0 / 0	308 / 0	0 / 0

TOTAL WEIGHT = 112 lb (M)F

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMVW-t	MT20	4.0	5.0	1.75	2.25
D	TTWW-m	MT20	5.0	6.0	2.50	2.00
E	TMV+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.50	2.00
G	TMVW-t	MT20	4.0	5.0	1.75	2.25
H	TMV+p	MT20	2.0	4.0		
J	BMVW1-t	MT20	4.0	5.0	1.75	1.75
K	BMVW-t	MT20	3.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMVW-t	MT20	3.0	8.0		
N	BMVW-t	MT20	3.0	4.0		
O	BMVW1-t	MT20	4.0	5.0	1.75	1.75

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.07 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 22	-77.4 -77.4	0.09 (1)	C-N	-27 / 60	0.02 (4)	
B-C	0 / 14	-77.4 -77.4	0.17 (1)	N-D	0 / 174	0.05 (4)	
C-D	-1914 / 0	-77.4 -77.4	0.18 (1)	D-M	0 / 607	0.14 (1)	
D-E	-2191 / 0	-77.4 -77.4	0.52 (1)	M-E	-625 / 0	0.24 (1)	
E-F	-2191 / 0	-77.4 -77.4	0.52 (1)	M-F	0 / 608	0.14 (1)	
F-G	-1913 / 0	-77.4 -77.4	0.18 (1)	K-F	0 / 174	0.05 (4)	
G-H	0 / 14	-77.4 -77.4	0.17 (1)	K-G	-26 / 60	0.02 (4)	
H-I	0 / 22	-77.4 -77.4	0.09 (1)	O-C	-2103 / 0	0.82 (1)	
O-B	-223 / 0	0.0 0.0	0.02 (1)	G-J	-2102 / 0	0.82 (1)	
J-H	-223 / 0	0.0 0.0	0.02 (1)				
O-N	0 / 1720	-18.2 -18.2	0.42 (1)				
N-M	0 / 1702	-18.2 -18.2	0.42 (1)				
M-L	0 / 1701	-18.2 -18.2	0.42 (1)				
L-K	0 / 1701	-18.2 -18.2	0.42 (1)				
K-J	0 / 1719	-18.2 -18.2	0.42 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/999 (0.19")

CSI: TC=0.52/1.00 (D-E:1), BC=0.42/1.00 (N-O:1), WB=0.82/1.00 (C-O:1), SSI=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

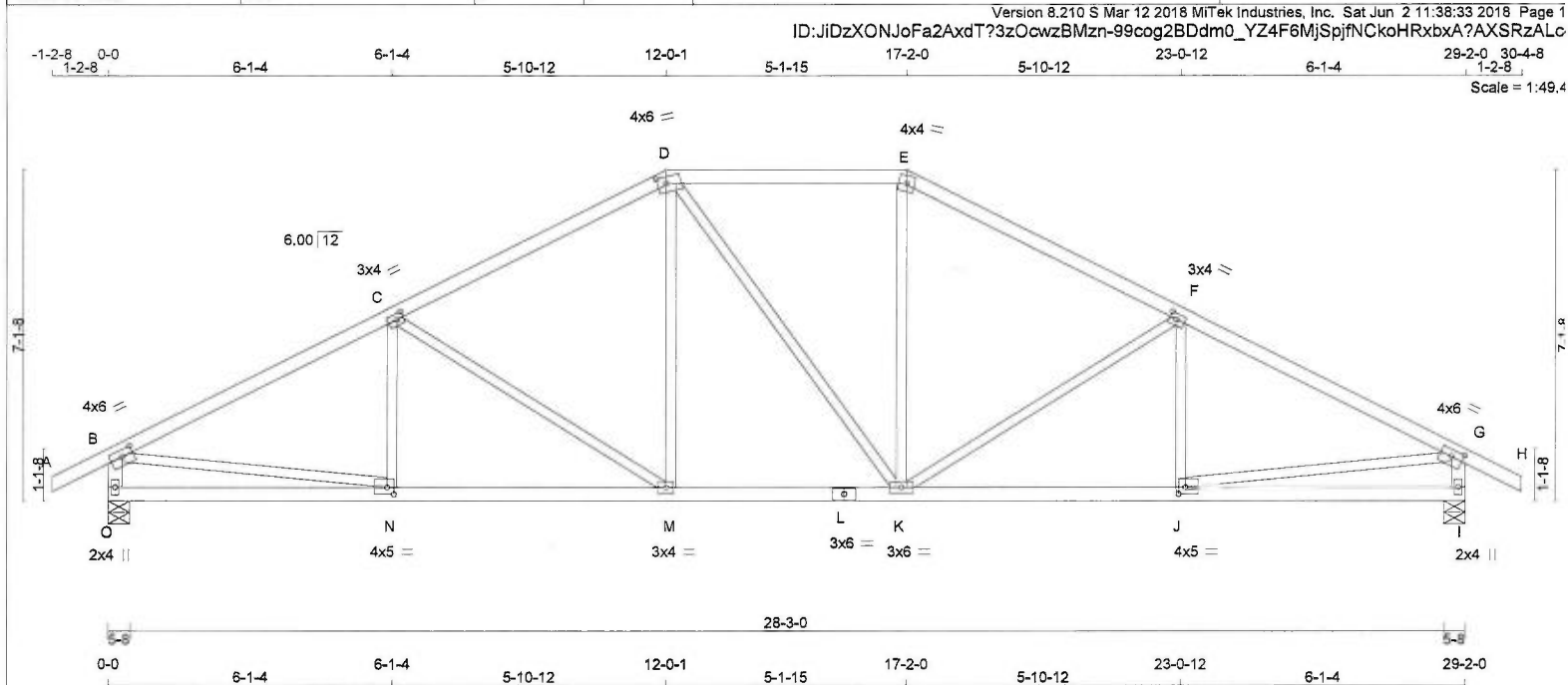
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (F) (INPUT = 0.90)
JSI METAL= 0.62 (C) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - H	2x4	DRY	No.2
O - B	2x4	DRY	No.2
I - G	2x4	DRY	No.2
O - L	2x4	DRY	No.2
L - I	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	1.75	3.00
C	TMVW-t	MT20	3.0	4.0	1.50	1.75
D	TTW-m	MT20	4.0	6.0	1.75	2.25
E	TTW-m	MT20	4.0	4.0		
F	TMVW-t	MT20	3.0	4.0	1.50	1.75
G	TMVW-t	MT20	4.0	6.0	1.75	3.00
I	BMV1+p	MT20	2.0	4.0		
J	BMVW-t	MT20	4.0	5.0	1.75	1.75
K	BMVW-t	MT20	3.0	6.0		
L	BS-t	MT20	3.0	6.0		
M	BMVW-t	MT20	3.0	4.0		
N	BMVW-t	MT20	4.0	5.0	1.75	1.75
O	BMV1+p	MT20	2.0	4.0		



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
O	1493	0	1493	0	5-8	5-8
I	1493	0	1493	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE				
O	1047	739 / 0	0 / 0	0 / 0	0 / 0	308 / 0	0 / 0
I	1047	739 / 0	0 / 0	0 / 0	0 / 0	308 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 MAX (PLF)	MAX. UNBRACED LENGTH FR-TO	MAX. FACTORED FORCE (LBS)
FR-TO	0 / 22	-77.4	-77.4	0.09 (1)
A-B	-2031 / 0	-77.4	-77.4	0.44 (1)
B-C	-1640 / 0	-77.4	-77.4	0.40 (1)
C-D	-1450 / 0	-77.4	-77.4	0.29 (1)
D-E	-1641 / 0	-77.4	-77.4	0.40 (1)
E-F	-2030 / 0	-77.4	-77.4	0.44 (1)
F-G	0 / 22	-77.4	-77.4	0.09 (1)
G-H	-1446 / 0	0.0	0.0	0.15 (1)
O-B	-1445 / 0	0.0	0.0	0.15 (1)
I-G	0 / 0	-18.2	-18.2	0.16 (4)
O-N	0 / 1838	-18.2	-18.2	0.37 (1)
N-M	0 / 1449	-18.2	-18.2	0.29 (1)
M-L	0 / 1449	-18.2	-18.2	0.29 (1)
L-K	0 / 1837	-18.2	-18.2	0.36 (1)
K-J	0 / 0	-18.2	-18.2	0.16 (4)
J-I				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/999 (0.16")

CSI: TC=0.44/1.00 (B-C:1), BC=0.37/1.00 (M-N:1), WB=0.46/1.00 (C-M:1), SSI=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

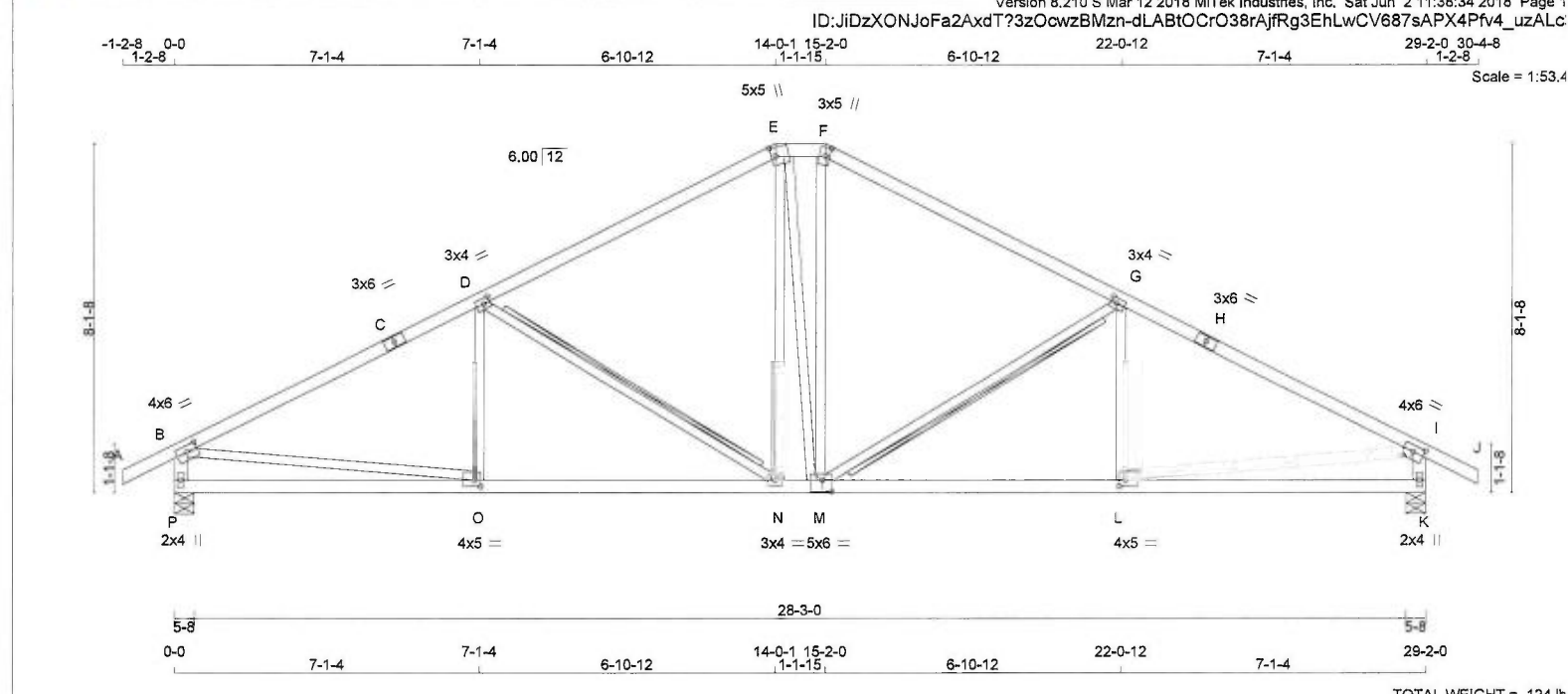
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.55 (N) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. SPF				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER										DESIGN CRITERIA			
A - C 2x4 DRY No.2 SPF				BEARINGS										SPECIFIED LOADS:			
C - E 2x4 DRY No.2 SPF				FACTORED MAXIMUM FACTORED INPUT REQD										TOP CH. LL = 23.3 PSF			
E - F 2x4 DRY No.2 SPF				GROSS REACTION GROSS REACTION BRG BRG										DL = 3.0 PSF			
F - H 2x4 DRY No.2 SPF				JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX										BOT CH. LL = 0.0 PSF			
H - J 2x4 DRY No.2 SPF				P 1493 0 1493 0 0 5-8 5-8										DL = 7.3 PSF			
P - B 2x4 DRY No.2 SPF				K 1493 0 1493 0 0 5-8 5-8										TOTAL LOAD = 33.6 PSF			
K - I 2x4 DRY No.2 SPF				UNFACTORED REACTIONS										SPACING = 24.0 IN. C/C			
P - M 2x4 DRY No.2 SPF				1ST LCASE MAX./MIN. COMPONENT REACTIONS										LOADING IN FLAT SECTION BASED ON A			
M - K 2x4 DRY No.2 SPF				JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL										SLOPE OF 2.00/12 MINIMUM			
ALL WEBS 2x3 DRY No.2 SPF				P 1047 739 / 0 0 / 0 0 / 0 0 / 0 308 / 0 0 / 0										THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010			
EXCEPT				K 1047 739 / 0 0 / 0 0 / 0 0 / 0 308 / 0 0 / 0										THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012, BCBC 2012, ABC 2014 - CSA 086-09 - TPIC 2011			
DRY: SEASONED LUMBER.				BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, K										(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD			
				BRACING										ALLOWABLE DEFL.(LL)= L/360 (0.97")			
				TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.10 FT.										CALCULATED VERT. DEFL.(LL) = L / 999 (0.09")			
				MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.										ALLOWABLE DEFL.(TL)= L/360 (0.97")			
				ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.										CALCULATED VERT. DEFL.(TL) = L / 999 (0.19")			
				2x4 DRY SPF No.2 T-BRACE AT D-N, G-M										CSI: TC=0.62/1.00 (B-D:1), BC=0.41/1.00 (N-O:1),			
				FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.										WB=0.42/1.00 (B-O:1), SSI=0.24/1.00 (B-D:1)			
				END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW										DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10			
				LOADING										COMP=1.10 SHEAR=1.10 TENS= 1.10			
				TOTAL LOAD CASES: (4)										COMPANION LIVE LOAD FACTOR = 0.50			
				CHORDS										WEBS			
				MAX. FACTORED FACTORED										MAX. FACTORED			
				MEMB. FORCE VERT. LOAD LC1 MAX										MEMB. FORCE MAX			
				(LBS) (PLF) CSI (LC)										(LBS) MAX			
				UNBRAC										CSI (LC)			
				LENGTH FR-TO													
				FR-TO										FR-TO			
				A-B 0 / 22 -77.4 -77.4 0.09 (1) 10.00 C-D -65 / 103 0.04 (4)													
				B-C -2027 / 0 -77.4 -77.4 0.62 (1) 4.10 D-N -638 / 0 0.33 (1)													
				C-D -2027 / 0 -77.4 -77.4 0.62 (1) 4.10 N-E 0 / 378 0.08 (1)													
				D-E -1481 / 0 -77.4 -77.4 0.55 (1) 4.72 E-M 0 / 27 0.01 (1)													
				E-F -1306 / 0 -77.4 -77.4 0.03 (1) 5.64 M-F 0 / 406 0.09 (1)													
				F-G -1484 / 0 -77.4 -77.4 0.55 (1) 4.72 M-G -631 / 0 0.33 (1)													
				G-H -2024 / 0 -77.4 -77.4 0.62 (1) 4.10 L-G -71 / 101 0.03 (4)													
				H-I -2024 / 0 -77.4 -77.4 0.62 (1) 4.10 B-O 0 / 1850 0.42 (1)													
				I-J 0 / 22 -77.4 -77.4 0.09 (1) 10.00 L-I 0 / 1848 0.42 (1)													
				P-B -1440 / 0 0.0 0.0 0.15 (1) 6.81													
				K-I -1439 / 0 0.0 0.0 0.15 (1) 6.81													
				P-O 0 / 0 -18.2 -18.2 0.22 (4) 10.00													
				O-N 0 / 1838 -18.2 -18.2 0.41 (1) 10.00													
				N-M 0 / 1302 -18.2 -18.2 0.31 (1) 10.00													
				M-L 0 / 1836 -18.2 -18.2 0.40 (1) 10.00													
				L-K 0 / 0 -18.2 -18.2 0.22 (4) 10.00													

LICENSED PROFESSIONAL ENGINEER

N.A. EL-MASRI

PROVINCIAL OF ONTARIO

Jun 02, 2018

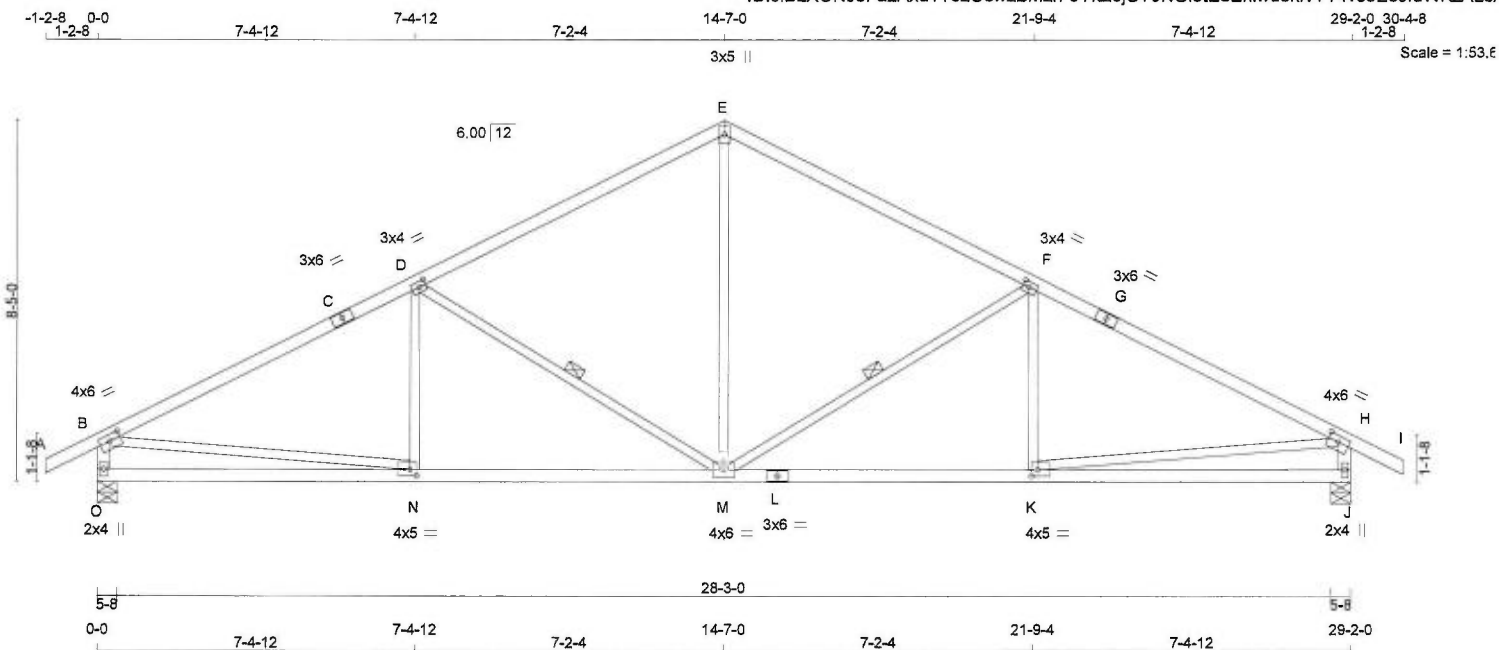
READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

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TOTAL WEIGHT = 4 X 113 = 454 lb

LUMBER		N. L. G. A. RULES		LUMBER		DESCR.	
CHORDS	SIZE						
A - C	2x4	DRY	No.2			SPF	
C - E	2x4	DRY	No.2			SPF	
E - G	2x4	DRY	No.2			SPF	
G - I	2x4	DRY	No.2			SPF	
O - B	2x4	DRY	No.2			SPF	
J - H	2x4	DRY	No.2			SPF	
O - L	2x4	DRY	No.2			SPF	
L - J	2x4	DRY	No.2			SPF	

ALL WEBS 2x3 DRY No.2

DRY; SEASONED LUMBER.

PLATES (table is in inches)					
JL	TYPE	PLATES	W	LEN	Y X
B	TMWV-t	MT20	4.0	6.0	1.75 3.00
C	TS-4	MT20	3.0	6.0	
D	TMWV-t	MT20	3.0	4.0	1.50 1.75
E	TTW+p	MT20	3.0	5.0	
F	TMWV-t	MT20	3.0	4.0	1.50 1.75
G	TS-1	MT20	3.0	6.0	
H	TMWV-t	MT20	4.0	6.0	1.75 3.00
J	BMV1+p	MT20	2.0	4.0	
K	BMWV-t	MT20	4.0	5.0	1.75 1.75
L	LS-4	MT20	3.0	6.0	
M	BMWVWV-t	MT20	4.0	6.0	
N	BMWV-t	MT20	4.0	5.0	1.75 1.75
O	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BOILING BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
O	1493	0	1493	0	0	5-8	5-8
J	1493	0	1493	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	1047	739 / 0	0 / 0	0 / 0	0 / 0	308 / 0	0 / 0
O	1047	739 / 0	0 / 0	0 / 0	0 / 0	308 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.01 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1- 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF D-M, F-M. DBS = 20-0-0 . CBF = 84 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		LENGTH	FR-TO			
A-B	0 / 22	-77.4	-77.4	0.09 (1)	10.00	N-D	-56 / 108	0.04 (4)
B-C	-2020 / 0	-77.4	-77.4	0.68 (1)	4.01	D-M	-674 / 0	0.33 (1)
C-D	-2020 / 0	-77.4	-77.4	0.68 (1)	4.01	M-E	0 / 845	0.19 (1)
D-E	-1443 / 0	-77.4	-77.4	0.60 (1)	4.69	M-F	-673 / 0	0.33 (1)
E-F	-1443 / 0	-77.4	-77.4	0.59 (1)	4.69	K-F	-57 / 108	0.04 (4)
F-G	-2019 / 0	-77.4	-77.4	0.68 (1)	4.02	B-N	0 / 1845	0.42 (1)
G-H	-2019 / 0	-77.4	-77.4	0.68 (1)	4.02	K-H	0 / 1844	0.41 (1)
H-I	0 / 22	-77.4	-77.4	0.09 (1)	10.00			
O-B	-1437 / 0	0.0	0.0	0.14 (1)	6.81			
J-H	-1437 / 0	0.0	0.0	0.14 (1)	6.81			
O-N	0 / 0	-18.2	-18.2	0.23 (4)	10.00			
N-M	0 / 1834	-18.2	-18.2	0.41 (1)	10.00			
M-L	0 / 1833	-18.2	-18.2	0.41 (1)	10.00			
L-K	0 / 1833	-18.2	-18.2	0.41 (1)	10.00			
K-J	0 / 0	-18.2	-18.2	0.23 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL	=	23.3	PSF
		DL	=	3.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.3	PSF
TOTAL LOAD				=	33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.68/1.00 (B-D:1), BC=0.41/1.00 (M-N:1),
WB=0.42/1.00 (B-N:1), SSI=0.25/1.00 (B-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)	(PLI)	(PSI)	(PLI)	(PSI)	(PLI)
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

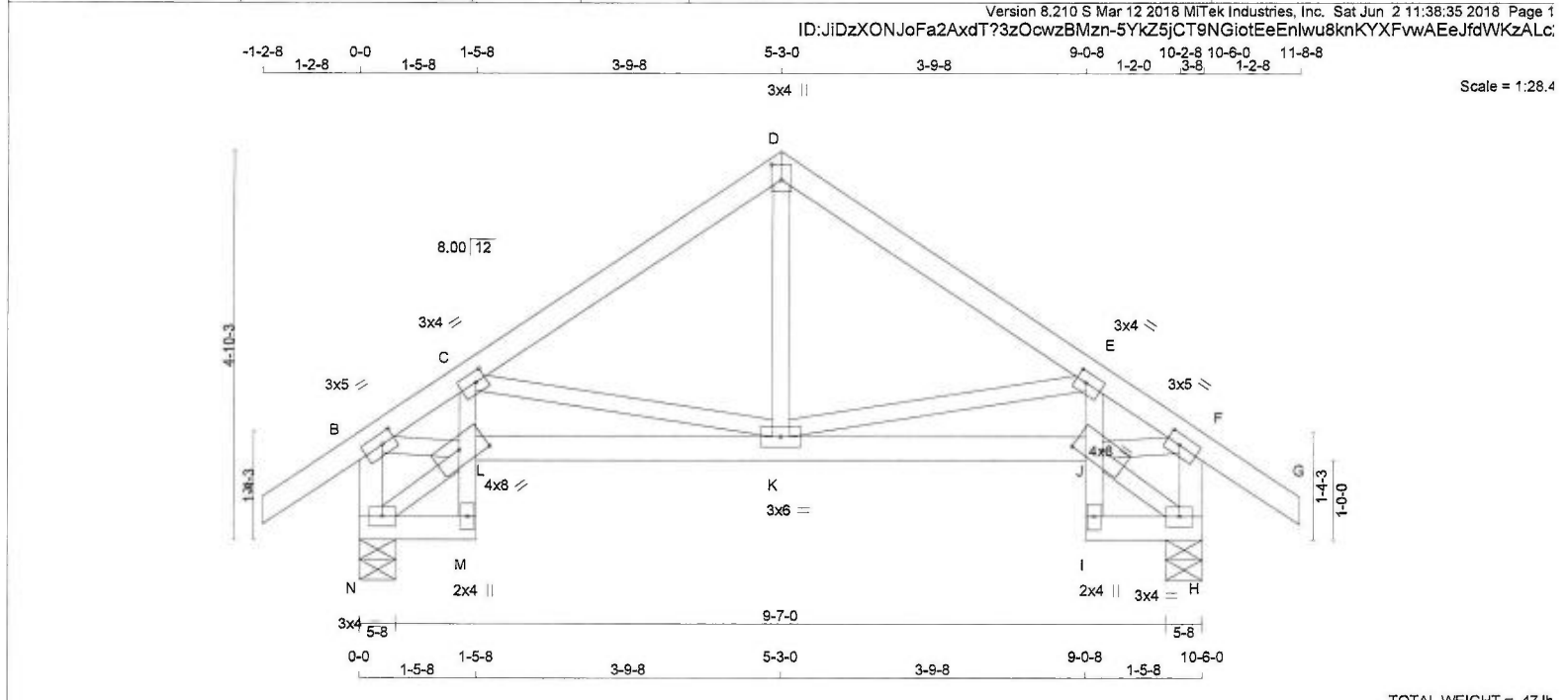
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.55 (N) (INPUT = 1.00)



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TOTAL WEIGHT = 47 LB									
[M]F									
DESIGN CRITERIA									
SPECIFIED LOADS:									
TOP CH. LL = 23.3 PSF									
DL = 3.0 PSF									
BOT CH. LL = 0.0 PSF									
DL = 7.3 PSF									
TOTAL LOAD = 33.6 PSF									
SPACING = 24.0 IN. C/C									
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010									
THIS DESIGN COMPLIES WITH:									
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014									
- CSA 088-09									
- TPIC 2011									
DESIGN ASSUMPTIONS									
-OVERHANG NOT TO BE ALTERED OR CUT OFF.									
(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD									
ALLOWABLE DEFL.(LL)= L/360 (0.35")									
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")									
ALLOWABLE DEFL.(TL)= L/360 (0.35")									
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")									
CSI: TC=0.14/1.00 (D-E:1) , BC=0.14/1.00 (K-L:1) , WB=0.14/1.00 (B-L:1) , SSI=0.11/1.00 (D-E:1)									
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10									
COMP=1.10 SHEAR=1.10 TENS= 1.10									
COMPANION LIVE LOAD FACTOR = 0.50									
AUTOSOLVE HEELS OFF									
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .									
NAIL VALUES									
PLATE GRIP(DRY) SHEAR SECTION									
(PSI) (PLI) (PLI)									
MAX MIN MAX MIN MAX MIN									
MT20 618 354 1667 822 2284 1656									
PLATE PLACEMENT TOL. = 0.250 inches									
PLATE ROTATION TOL. = 5.0 Deg.									
JSI GRIP= 0.86 (L) (INPUT = 0.90)									
JSI METAL= 0.24 (F) (INPUT = 1.00)									

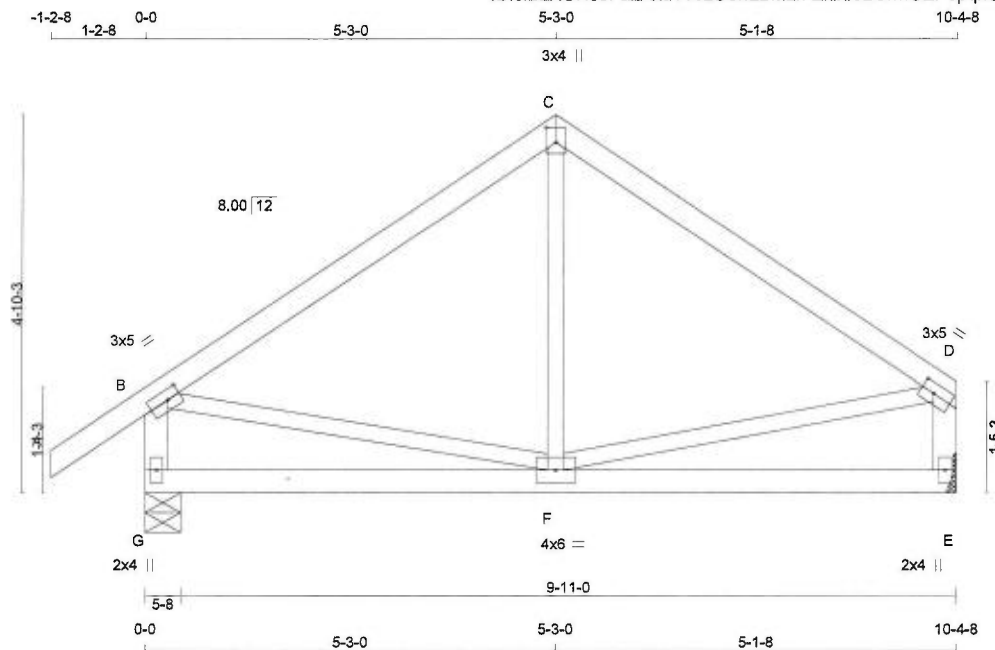
LUMBER									
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Version 8.210 S Mar 12 2018 MiTek Industries, Inc. Sat Jun 2 11:38:36 2018 Page 1
ID:JiDzXONJoFa2AxdT?3zOcwzBMzn-ZkIxI3D5whOZP0pqnUG9RLHw0ysWeOTNsZOB2mzALc



Scale = 1:29.5

TOTAL WEIGHT = 3 X 42 = 125 lb
[M]F

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
G - B	2x4	DRY	No.2
E - D	2x4	DRY	No.2
G - E	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	3.0	5.0	1.50	2.00
C	TTVW+p	MT20	3.0	4.0	2.25	1.50
D	TMVW-t	MT20	3.0	5.0	1.50	2.00
E	BMV1+p	MT20	2.0	4.0		
F	BMVWW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX
G	596	0	596	0	5-8	5-8
E	496	0	496	0	MECHANICAL	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT E TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	SNOW	MAX/MIN. LIVE	PERM. LIVE	WIND	DEAD	SOIL
G	416	302 / 0	0 / 0	0 / 0	0 / 0	115 / 0	0 / 0
E	348	242 / 0	0 / 0	0 / 0	0 / 0	107 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-77.4 -77.4	0.09 (1)	10.00	F-C	-6 / 90	0.03 (4)
B-C	-356 / 0	-77.4 -77.4	0.27 (1)	6.25	B-F	0 / 302	0.07 (1)
C-D	-356 / 0	-77.4 -77.4	0.26 (1)	6.25	F-D	0 / 303	0.07 (1)
G-B	-559 / 0	0.0 0.0	0.06 (1)	7.81			
E-D	-460 / 0	0.0 0.0	0.05 (1)	7.81			
G-F	0 / 0	-18.2 -18.2	0.14 (4)	10.00			
F-E	0 / 0	-18.2 -18.2	0.14 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL LOAD		=	33.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.27/1.00 (B-C:1), BC=0.14/1.00 (F-G:4), WB=0.07/1.00 (D-F:1), SSI=0.13/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

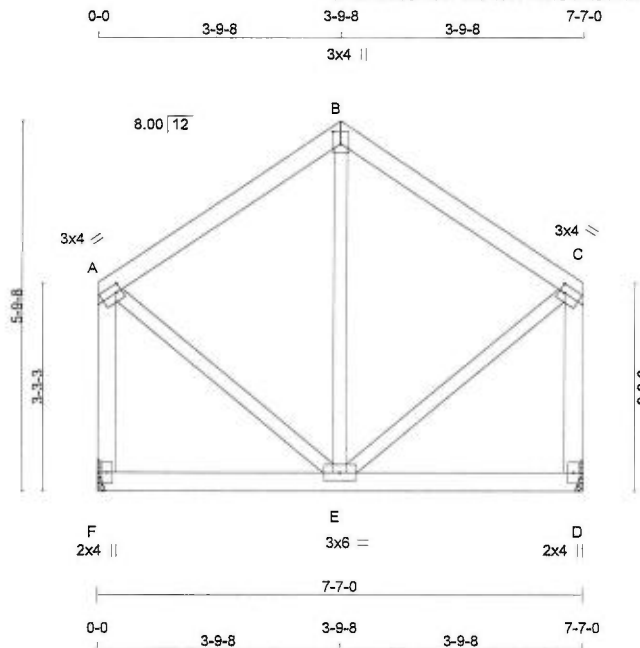
JSI GRIP= 0.51 (B) (INPUT = 0.90)
JSI METAL= 0.16 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



Scale = 1:36.0



TOTAL WEIGHT = 37 lb
(M)F

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - B	2x4	DRY	No.2
B - C	2x4	DRY	No.2
F - A	2x4	DRY	No.2
D - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	3.0	4.0	1.50	1.00
B	TTW+p	MT20	3.0	4.0	2.25	1.50
C	TMVW-t	MT20	3.0	4.0	1.50	1.00
D	BMV1+p	MT20	2.0	4.0		
E	BMVWV-t	MT20	3.0	6.0		
F	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
GROSS REACTION		GROSS REACTION		BRG		BRG	
JT	VERT	DOWN	HORZ	IN-SX		IN-SX	
F	363	0	363	0	0	MECHANICAL	
D	363	0	363	0	0	MECHANICAL	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT F, D TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN	COMPONENT REACTIONS				
F	255	177 / 0	LIVE	PERM. LIVE	WIND	DEAD	SOIL
D	255	177 / 0	0 / 0	0 / 0	0 / 0	78 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX LC1 (LC)	
FR-TO		FROM TO		FR-TO			
A-B	-157 / 0	-77.4 -77.4	0.14 (1)	6.25	E-B	-119 / 27	0.06 (1)
B-C	-157 / 0	-77.4 -77.4	0.14 (1)	6.25	A-E	0 / 166	0.04 (1)
F-A	-336 / 0	0.0 0.0	0.06 (1)	7.81	E-C	0 / 166	0.04 (1)
D-C	-336 / 0	0.0 0.0	0.06 (1)	7.81			
F-E	0 / 0	-18.2 -18.2	0.07 (4)	10.00			
E-D	0 / 0	-18.2 -18.2	0.07 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.25")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.25")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.14/1.00 (B-C:1), BC=0.07/1.00 (D-E:4), WB=0.06/1.00 (B-E:1), SSI=0.10/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

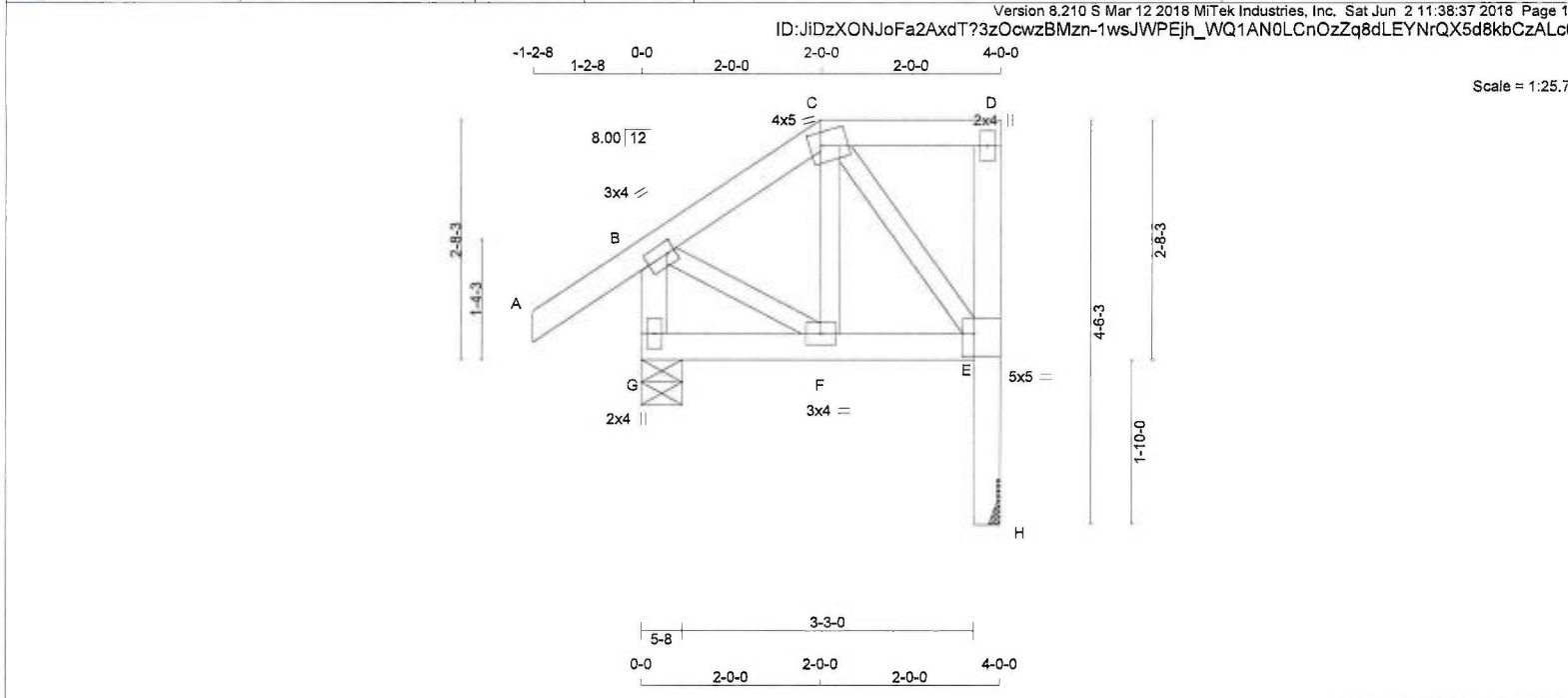
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.53 (A) (INPUT = 0.90)
JSI METAL= 0.09 (C) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



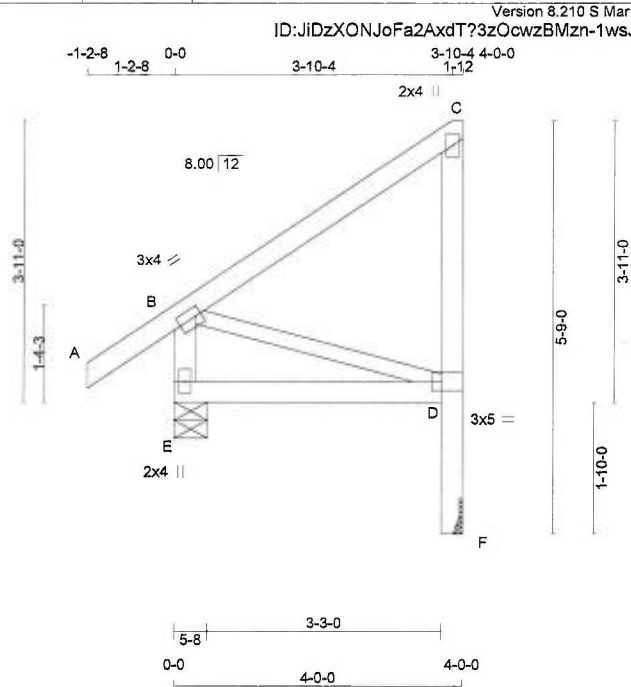


LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR.					DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER										DESIGN CRITERIA														
A - C 2x4 DRY No.2 SPF					BEARINGS										SPECIFIED LOADS:														
C - D 2x4 DRY No.2 SPF					FACTORED MAXIMUM FACTORED INPUT REQD										TOP CH. LL = 23.3 PSF														
H - D 2x4 DRY No.2 SPF					GROSS REACTION GROSS REACTION BRG BRG										DL = 3.0 PSF														
G - B 2x4 DRY No.2 SPF					JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX										BOT CH. LL = 0.0 PSF														
G - E 2x4 DRY No.2 SPF					H 191 0 191 0 0 0 MECHANICAL										DL = 7.3 PSF														
					G 291 0 291 0 0 5-8 5-8										TOTAL LOAD = 33.6 PSF														
ALL WEBS 2x3 DRY No.2 SPF					A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT H TO RESIST THE MAX FACTORED REACTIONS.																								
DRY: SEASONED LUMBER.																													
UNFACTORED REACTIONS																													
1ST LCASE MAX./MIN. COMPONENT REACTIONS																													
JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL																													
H 134 93 / 0 0 / 0 0 / 0 0 / 0 41 / 0 0 / 0																													
G 202 153 / 0 0 / 0 0 / 0 0 / 0 49 / 0 0 / 0																													
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G																													
BRACING																													
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.																													
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.																													
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.																													
LOADING																													
TOTAL LOAD CASES: (4)																													
CHORDS WEBS																													
MAX. FACTORED FACTORED MAX. FACTORED																													
MEMB. FORCE VERT. LOAD LC1 MAX. MAX. MEMB. FORCE MAX.																													
(LBS) (PLF) CSI (LC) UNBRAC LENGTH FR-TO (LBS) CSI (LC)																													
FR-TO FROM TO																													
A- B 0 / 28 -77.4 -77.4 0.09 (1) 10.00 F- C 0 / 36 0.01 (4)																													
B- C -101 / 0 -77.4 -77.4 0.05 (1) 6.25 C- E -129 / 0 0.02 (1)																													
C- D 0 / 0 -77.4 -77.4 0.05 (1) 10.00 B- F 0 / 93 0.02 (1)																													
H- E -191 / 0 0.0 0.0 0.02 (1) 7.81																													
E- D -77 / 0 0.0 0.0 0.01 (1) 7.81																													
G- B -274 / 0 0.0 0.0 0.03 (1) 7.81																													
G- F 0 / 0 -18.2 -18.2 0.02 (4) 10.00																													
F- E 0 / 83 -18.2 -18.2 0.02 (4) 10.00																													
																													
READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.																													
																													



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 SPECIFICATIONS AND CRITERIA USED IN
 THE DESIGN OF THIS COMPONENT.





Scale: 3/8"=1'

TOTAL WEIGHT = 22 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
F - C	2x4	DRY	No.2	SPF
E - B	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-1	MT20	3.0	4.0	1.50	1.00
C	TMV+p	MT20	2.0	4.0		
D	BMVW-1	MT20	3.0	5.0	1.50	1.50
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
F	191	0	191	0	MECHANICAL	
E	291	0	291	0	5-8	5-8

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT F TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
		SNOW	LIVE			
F	134	93 / 0	0 / 0	0 / 0	41 / 0	0 / 0
E	202	153 / 0	0 / 0	0 / 0	49 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)		
FR-TO		FROM TO			FR-TO				
A-B	0 / 28	-77.4	-77.4	0.09 (1)	10.00	B-D	0 / 0	0.00 (1)	
B-C	0 / 0	-77.4	-77.4	0.21 (1)	10.00				
F-D	-191 / 0	0.0	0.0	0.02 (1)	7.81				
D-C	-155 / 0	0.0	0.0	0.04 (1)	7.81				
E-B	-254 / 0	0.0	0.0	0.03 (1)	7.81				
E-D	0 / 0	-18.2	-18.2	0.09 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.21/1.00 (B-C:1), BC=0.09/1.00 (D-E:4), WB=0.00/1.00 (B-D:1), SSI=0.10/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
MT20	618	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

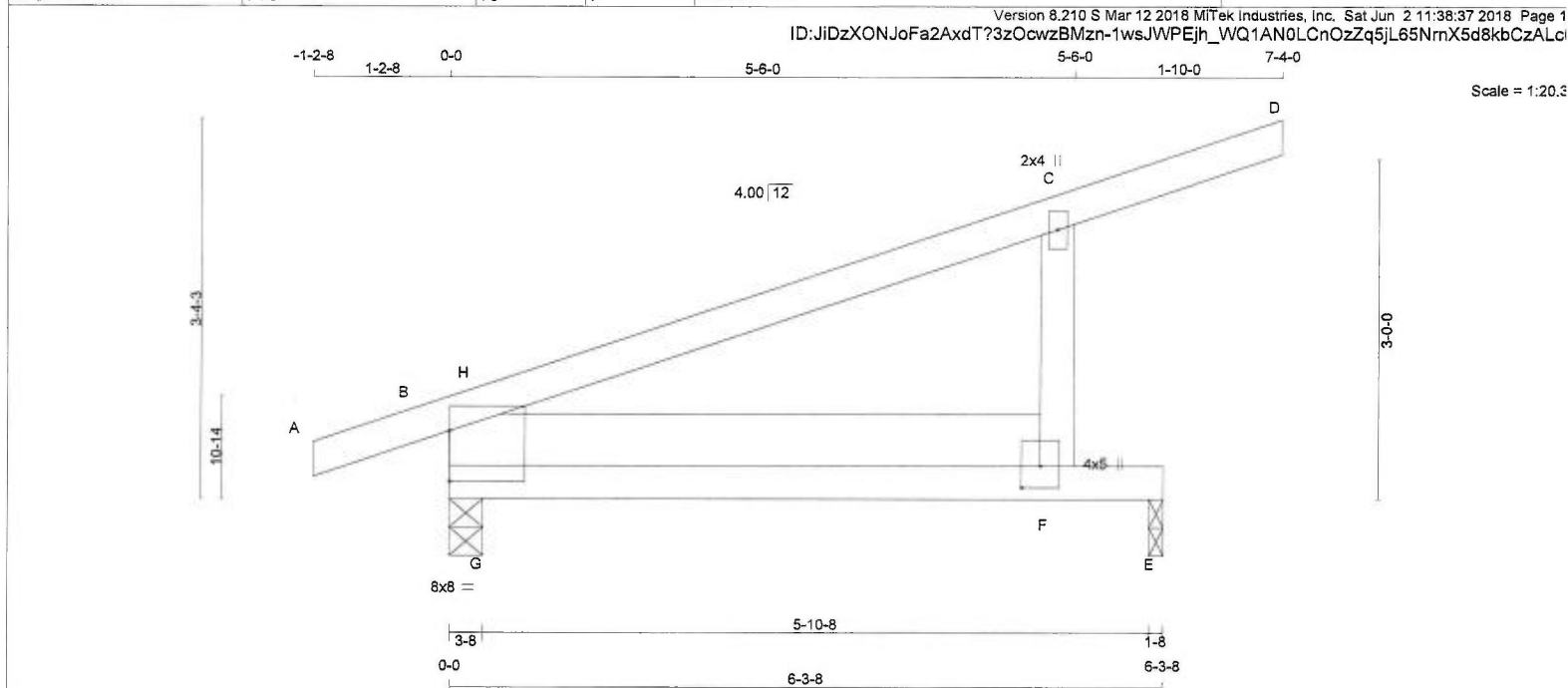
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 10 X 29 = 293 lb

LUMBER

N.L.G.A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
F - C	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF

REINFORCING MEMBERS

HW1	2x6	DRY	No.2	SPF
HW1	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
B	412	0	412	0	3-8	3-8
E	370	0	370	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM.LIVE	WIND			
B	287	214 / 0	0 / 0	0 / 0	0 / 0	73 / 0	0 / 0
E	259	188 / 0	0 / 0	0 / 0	0 / 0	70 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.90 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	UNBRACED LENGTH FR-TO
FR-TO		FROM TO		FR-TO			
A-B	-7 / 0	-77.4 -77.4	0.08 (1)	10.00	F-H	-672 / 0	0.15 (1)
B-H	-982 / 0	-77.4 -77.4	0.28 (1)	5.90	G-H	0 / 631	0.00 (1)
H-C	0 / 8	-77.4 -77.4	0.27 (1)	10.00			
C-D	-23 / 0	-77.4 -77.4	0.19 (1)	6.25			
F-C	-314 / 0	0.0 0.0	0.04 (1)	7.81			
B-G	0 / 646	-18.2 -18.2	0.33 (1)	10.00			
G-F	0 / 646	-18.2 -18.2	0.57 (1)	10.00			
F-E	0 / 0	-18.2 -18.2	0.46 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.21")
CALCULATED VERT. DEFL.(LL) = L/ 867 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.21")
CALCULATED VERT. DEFL.(TL) = L/ 461 (0.16")

CSI: TC=0.28/1.00 (B-H:1), BC=0.57/1.00 (F-G:1), WB=0.15/1.00 (F-H:1), SS=0.38/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.53 (F) (INPUT = 0.90)
JSI METAL= 0.21 (F) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



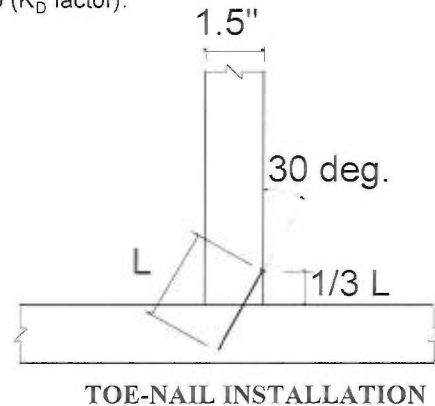
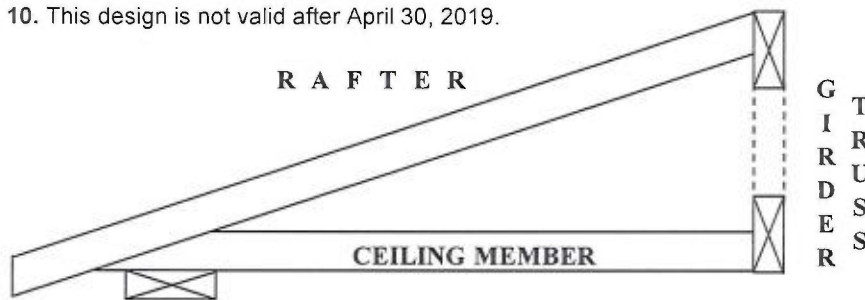
BEARING ANCHORAGE BY TOE-NAILS FOR LATERAL CAPACITY

B37579H1

NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL LATERAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	132	147
	3.25	0.144	132	147
	3.50	0.160	159	177

NOTES:

1. Rafter and ceiling members may be anchored to top and bottom chords of girder truss by toe-nailing rafter and ceiling members to girder chords provided the reaction does not exceed the lateral capacities in the table. Hangers (specified by others) are required for reactions higher than the maximum toe-nail capacity. Reactions are based on factored loads.
2. Toe nail capacities shown in the table are for one toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-09, section 10.9.4.1.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in tables below.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately $1/3$ the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See next page for nailing on bearing plate).
7. For loads due to wind the nail lateral capacity in this table may be multiplied by 1.15 (K_D factor).
8. Lumber must be dry ($< 19\%$ moisture content) at the time of nail installation.
9. Nail values in this table comply with CSA O86-09, section 10.9.4
10. This design is not valid after April 30, 2019.



Nail type	Common wire	Common spiral	Common wire
Nail dia. (in)	0.160	0.152	0.144
	(3.5" nail)		(3" and 3.25" nail)
LUMBER SIZE	MAXIMUM NUMBER OF TOE-NAILS		
2X4 SPF	2	2	3
2X4 D. Fir	2	2	2
2X6 SPF	4	4	4
2X6 D. Fir	3	3	3



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April 26, 2017

BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING

B37579H2

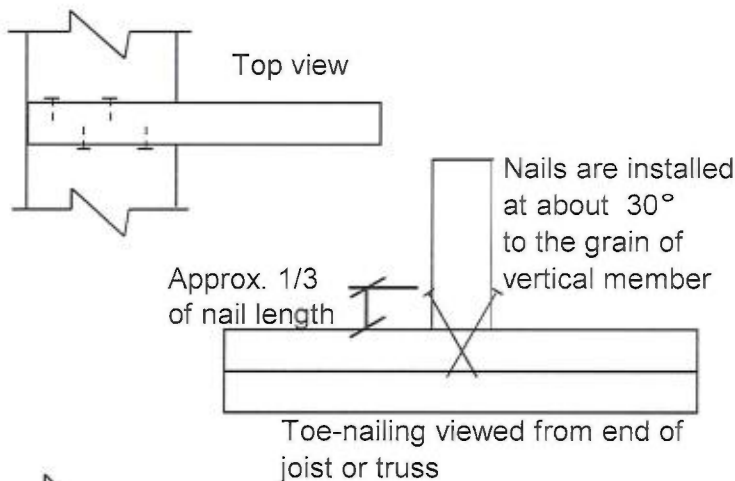
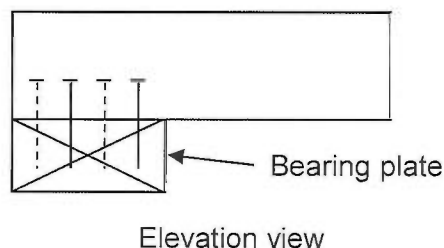
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	30	42
	3.25	0.144	32	45
	3.50	0.160	38	52

Note: If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

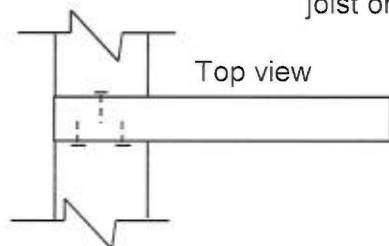
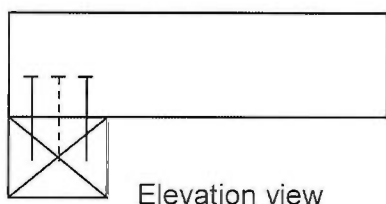
NOTES:

1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to wind or earthquake load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for one toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-09, section 10.9.5.2.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37579H1).
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-09, section 10.9.5
9. This design is not valid after April 30, 2019

Toe-nailing on 2x6 Bearing Plate



Toe-nailing on 2x4 Bearing Plate



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