Layout Name

Description

Created May 30, 2018 Builder

Sales Rep

Designer RCO Shipping Project

Canada

L4A 7X4 905-642-4400 Job Path

Builder's Project

14 Anderson Blvd

Stouffville, Ontario

Kott Lumber Company

D:\Users\rochavillo\WORK FROM HOME\GREEN YORK HOMES

INDIGO 1 (ELEV.1).isl

Ground Floor

Design Method

Deflection Joist

LL Span L/

TL Span L/

LL Cant 2L/

TL Cant 2L/

Deflection Girder

Floor

Loads

Live

Dead

\GRANELLI HOME CORP\MODELS

VINDIGO 1 VINDIGO 1 ELEV.1\FLOOF

Building Code NBCC 2010 / OBC

LSD

2012

40

15

480

360

480

360

360

240

480

360

OSB

3/4"

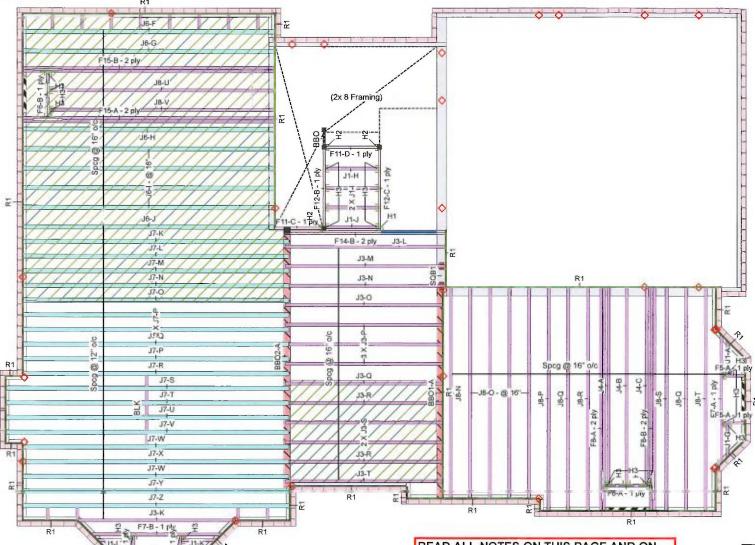
LSD

INDIGO 1 (ELEV.1) Design Method

GRANELLI HOME CORP. BRAMPTON, ONT.

GREEN YORK HOMES

Ground Floor



BUILDING DIVISION REVIEWED MAR 14 2013

MARY FRI

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS





JARDIN DESIGN GROUP INC. 64 Jardin Dr., Suite 3A, Vaughan, ON Date: Rev.2; May 22,2018 Project No: 17-55 Model: Indigo 1

Legend



Load from Above Wall Wall Opening Norbord Rimboard Plus 1.125 X 11.875 NJ60U 11.875 NJH 11 875 Forex 2.0E-3000Fb LVL 1.75 X 11.875

- 1. OBC 2012 O.Reg 332/12 as amended
- 2. Nascor CCMC 13535-R
- 3. LVL CCMC -14056-R
- 4. CAN/CSA-O86-09
- 5. CCMC -12787-R APA PR-L310(C)

Engineered floor joists shall be installed in accordance with the supplier's layout a specifications forming part of the permit drawin

All work shall conform to the (Building Code O. Reg. 332/12 as am

	F11	Forex 2.0E-3000Fb LVL	1.75	11.875			2	4-0-0			
	I Joist (Flush)										
	Label	Description	Width	Depth	Qty	Plies	Pcs	Length			
and	J7	NJ60U	3,5	11.875			19	20-0-0			
ngs.	J6	NJ60U	3.5	11.875			8	18-0-0			
	F15	NJH	2.5	11.875	2	2	4	18-0-0			
	F8	NJH	2.5	11.875	2	2	4	16-0-0			
	F7	NJH	2.5	11.875			2	10-0-0			
Onta	F6	NJH	2.5	11.875			2	4-0-0			
	F5	NJH	2.5	11.875			4	2-0-0			
neng	ed 18	NJH	2.5	11.875			- 13	16-0-0			
	J4	NJH	2.5	11.875			3	14-0-0			
	J3	NJH	2.5	11.875			14	12-0-0			
	J1	NJH	2.5	11.875	- 8		8	4-0-0			
	Rim Bo	ard									
	Label	Description	Width	Depth	Qty	Plies	Pcs	Length			
	R1	Norbord Rimboard Plus 1.125 X 11.875	1.125	11.875			15	12			
1	Blockin	g		,							

Width Depth

1.75 11.875

11.875

1.75

Qty

Plies

Plies

Pcs Length

Varies | 16-0-0

Pcs Length

2

12-0-0

6-0-0

BLK1 NJH langer

Label Description

Ground Floor LVL/LSL (Flush) Label Description

F12 Forex

Forex

2.0E-3000Fb LVL

2.0E-3000Fb LVL

					Beam/Girder	Supported Member
Label	Pcs	Description	Skew	Slope	fasteners	fasteners
H1	1	HUS1.81/10			30 16d	10 16d
H2	3	L\$90				
Н3	25	LT251188			4 10dx1 1/2	2 10dx1 1/2

Width | Depth | Qty

2.5 11.875 LinFt

NOTES:

- . Framer to verify dimensions on the architectural drawings.
- 2. Double joist only require filler/backer ply when supporting another member using a face-mounted hanger.
- 3. Install 2x4 blocking @ 24" o/c under parallel non-load bearing walls.
- . Install single-ply flush window header along inside face of rimboard/rimioist.
- . Refer to Nascor specifier guide for installation works.
- 6. Squash blocks recommended to be installed at end bearing on all first level joists which support loading from above exceeding two levels floor or roof.
- Load transfer blocks to be installed under all point loads.
- B. It shall be the framer's responsibility that floor joists and beams are fastened as per the hanger manufacturer's standards.

Refer to Multiple Member Connection Detail to ply to ply nailing or bolting requirements.

Rim parallel to joists: 1-1/8" rimboard with 2"x 4" block (1/16" longer than rim depth @ 16" o/c). All other components and structural elements supporting the floor system such as beams, walls, columns, and foundation walls and footings including anchorage of components and bracing for lateral stability are the responsibility of Others.

Hatch area represents ceramic tiled floor with an additional dead load

The framing shown on this layout may deviate from the architectural and structural drawings. Project Engineer to review and approve the deviation prior

LL Span L/ TL Span L/ LL Cant 2L/ TL Cant 2L/ Decking Deck Thickness Fastener Nailed & Glued Vibration

M- 2057 19-447257.000.00.18. LOT 27



EWP Studio Simpson Strong-Tie® Component Solutions™

THIS CERTIFICATION IS TO CONFIRM THAT:

1. THE LOADS USED IN THE CALCULATION OF THE ATTACHED

APPROVED COMPONENTS CONFORM TO THE FLOOR ASSEMBLY

2. THE FLOOR JOISTS COMPLY WITH THE NASCOR SPAN TABLE

THE FLOOR SYSTEM MUST BE ASSEMBLED IN ACCORDANCE TO

THE NASCOR SPECIFIER GUIDE, MULTI-PLY MEMBERS MUST BE ATTACHED TOGETHER AS PER THE INCLUDED MULTIPLE

FOR THE LOADS AND SPACING SHOWN ON THIS LAYOUT.

ALL OTHER COMPONENTS AND STRUCTURAL ELEMENTS

COLUMNS AND FOUNDATION WALLS AND FOOTINGS

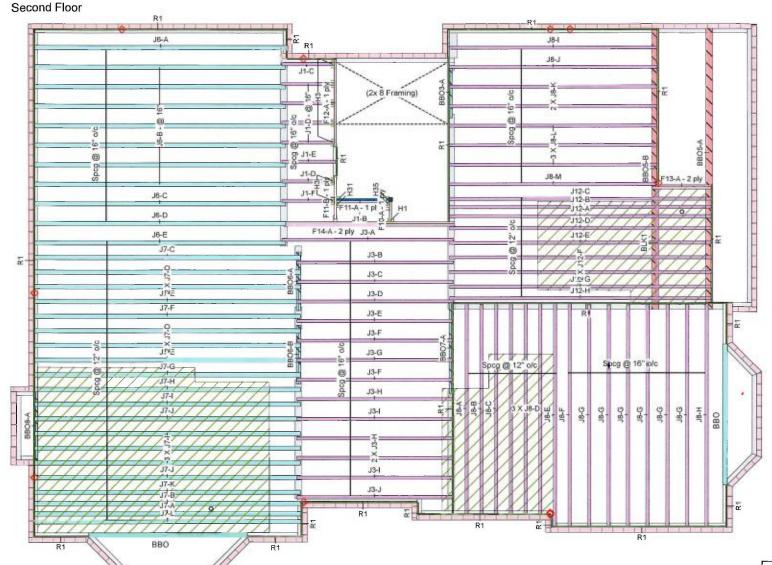
SUPPORTING THE FLOOR SYSTEM SUCH AS BEAMS, WALLS,

INCLUDING ANCHORAGE OF COMPONENTS AND BRACING FOR LATERAL STABILITY ARE THE RESPONSIBILITY OF OTHERS.

SHOWN ON THIS LAYOUT.

MEMBER CONNECTION DETAIL.







THIS CERTIFICATION IS TO CONFIRM THAT:

- 1. THE LOADS USED IN THE CALCULATION OF THE ATTACHED APPROVED COMPONENTS CONFORM TO THE FLOOR ASSEMBLY SHOWN ON THIS LAYOUT.
- 2. THE FLOOR JOISTS COMPLY WITH THE NASCOR SPAN TABLE FOR THE LOADS AND SPACING SHOWN ON THIS LAYOUT.

THE FLOOR SYSTEM MUST BE ASSEMBLED IN ACCORDANCE TO THE NASCOR SPECIFIER GUIDE. MULTI-PLY MEMBERS MUST BE ATTACHED TOGETHER AS PER THE INCLUDED MULTIPLE MEMBER CONNECTION DETAIL.

ALL OTHER COMPONENTS AND STRUCTURAL ELEMENTS SUPPORTING THE FLOOR SYSTEM SUCH AS BEAMS, WALLS, COLUMNS AND FOUNDATION WALLS AND FOOTINGS INCLUDING ANCHORAGE OF COMPONENTS AND BRACING FOR LATERAL STABILITY ARE THE RESPONSIBILITY OF OTHERS.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS**

ARCHITECTURAL DRAWINGS:

JARDIN DESIGN GROUP INC. 64 Jardin Dr., Suite 3A, Vaughan, ON Date: Rev.2; May 22,2018 Project No: 17-55 Model: Indigo 1

Legend



Load from Above Norbord Rimboard Plus 1.125 X 11.875 NJ60U 11.875 NJH 11,875 Forex 2.0E-3000Fb LVL 1.75 X 11.875

- 1. OBC 2012 O.Reg 332/12 as amended
- 2. Nascor CCMC 13535-R
- 3. LVL CCMC -14056-R
- 4. CAN/CSA-086-09
- 5. CCMC -12787-R APA PR-L310(C)

Second								_
	L (Flush)		I = I				1	
Label	Description	Width	Depth	Qty	Plies	Pcs	Length	1
F14	Forex 2.0E-3000Fb LVL	1.75	11.875	1	2	2	12-0-0	L
F13	Forex 2.0E-3000Fb LVL	1.75	11.875	1	2	2	6-0-0	La
F12	Forex 2.0E-3000Fb LVL	1.75	11.875			1	6-0-0	P
F11	Forex 2.0E-3000Fb LVL	1.75	11.875			2	4-0-0	D.
F10	Forex 2.0E-3000Fb LVL	1.75	11.875			1	2-0-0	E
I Joist (Flush)							C
Label	Description	Width	Depth	Qty	Plies	Pcs	Length	1
J7	NJ60U	3.5	11.875			20	20-0-0	В
J6	NJ60U	3.5	11.875			11	18-0-0	
J12	NJH	2.5	11.875			9	18-0-0	_
J8	NJH	2.5	11.875			23	16-0-0	S
J3	NJH	2.5	11.875			14	12-0-0	
J1	NJH	2.5	11.875			9	4-0-0	D
Rim Bo	ard							1 6
Label	Description	Width	Depth	Qty	Plies	Pcs	Length	s
R1	Norbord Rimboard Plus 1.125 X	1.125	11.875			16	12	Р
	11.875							В
Blockin								
Label	Description	Width	Depth	Qty	Plies	Pcs	Length	,
BLK1	NJH	2.5	11.875	LinFt		Varies	7-0-0	
Hanger								1

Skew Slope

NOTES:

H1

НЗ

H5

- . Framer to verify dimensions on the architectural drawings.
- 2. Double joist only require filler/backer ply when supporting another member using a face-mounted hanger.

Pcs Description

2 HUS1.81/10

1 HUCQ1.81/9-

9 LT251188

SDS

- 3. Install 2x4 blocking @ 24" o/c under parallel non-load bearing walls.
- I. Install single-ply flush window header along inside face of
- . Refer to Nascor specifier guide for installation works.
- . Squash blocks recommended to be installed at end bearing on all first level joists which support loading from above exceeding two levels floor or roof.
- . Load transfer blocks to be installed under all point loads.
- . It shall be the framer's responsibility that floor joists and beams are fastened as per the hanger manufacturer's standards.

Refer to Multiple Member Connection Detail to ply to ply nailing or

Rim parallel to joists: 1-1/8" rimboard with 2"x 4" block (1/16" longer than rim depth @ 16" o/c). All other components and structural elements supporting the floor system such as beams, walls, columns, and foundation walls and footings including anchorage of components and bracing for lateral stability are the responsibility of Others.

Hatch area represents ceramic tiled floor with an addtional dead load

The framing shown on this layout may deviate from the architectural and structural drawings. Project Engineer to review and approve the deviation prior to construction.

							ACCOUNT WHEN THE PARTY OF THE P
)							MIACC
ion	Width	Depth	Qty	Plies	Pcs	Length	NASC
0Fb LVL	1.75	11.875	1	2	2	12-0-0	
0Fb LVL	1.75	11.875	1	2	2	6-0-0	Layout Name INDIGO 1 (ELEV.1)
0Fb LVL	1.75	11.875			1	6-0-0	Design Method
0Fb LVL	1.75	11.875			2	4-0-0	Description
0Fb LVL	1.75	11.875			1	2-0-0	GRANELLI HOME CORF BRAMPTON, ONT.
							Created
ion	Width	Depth	Qty	Plies	Pcs	Length	May 30, 2018
	3.5	11.875			20	20-0-0	Builder
	3.5	11.875			11	18-0-0	GREEN YORK HOMES
	2.5	11.875			9	18-0-0	
	2.5	11.875			23	16-0-0	Sales Rep
	2.5	11.875			14	12-0-0	RM _
	2.5	11.875			9	4-0-0	Designer
							RCO
ion	Width	Depth	Qty	Plies	Pcs	Length	Shipping
Rimboard 5 X	1.125	11.875			16	12	Project
							Builder's Project
							Kott Lumber Com
ion	Width	Depth	Qty	Plies	Pcs	Length	14 Anderson Blvd
	2.5	11.875	LinFt		Varies	7-0-0	Stouffville, Ontario
							Giodifyino, Officialio

Beam/Girder

fasteners

30 16d

4 10dx1 1/2

Supported

Member

10 16d

npany

Canada L4A 7X4

905-642-4400 Job Path

2 10dx1 1/2 D:\Users\rochavillo\WORK FROM HOME\GREEN YORK HOMES \GRANELLI HOME CORP\MODELS VINDIGO 1VINDIGO 1 ELEV.1VFLOOF VINDIGO 1 (ELEV.1).isl

Second Floor

Design Method LSD Building Code NBCC 2010 / OBC 2012

15

480

360

480

360

360

240

480

360

Floor Loads Live Dead Deflection Joist LL Span L/ TL Span L/

LL Cant 2L/ TL Cant 2L/ Deflection Girder LL Span L/

TL Span L/ LL Cant 2L/ TL Cant 2L/

Decking Deck **Thickness**

OSB 5/8" Fastener Nailed & Glued

Vibration Gypsum 1/2" Ceiling:

M- 2057



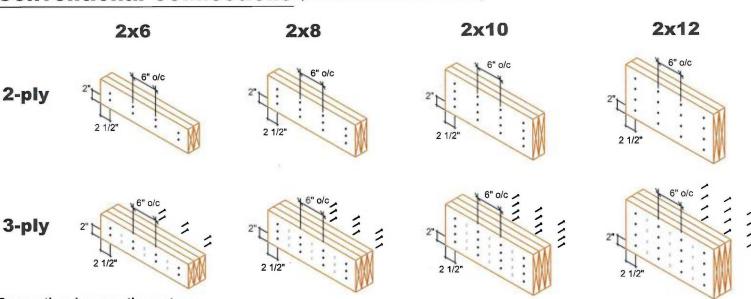
EWP Studio Simpson Strong-Tie® Component Solutions™



ULTIPLE MEMBER CONNECTIONS

GREEN YORK HOMES-GRANELLI HOME CORP-INDIGO 1 (ELEV.1&2

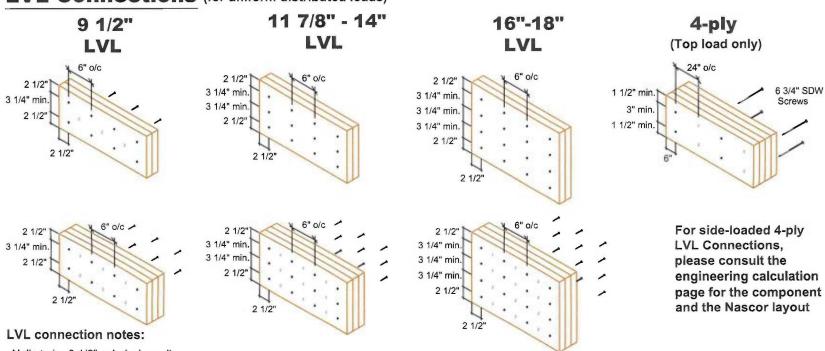
Conventional Connections (for uniform distributed loads)



Conventional connection notes:

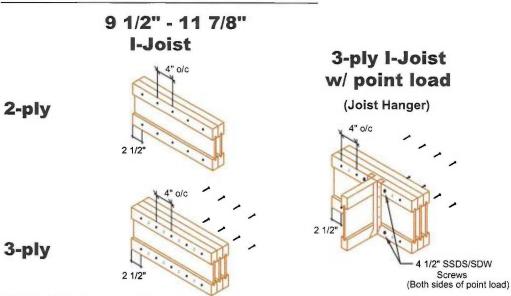
- -Nails to be 3" 10d spiral wire nails.
- -Nails to be located a minimum of 2" from the top and bottom of the member. Start all nails a minimum of 2 1/2" in from ends.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail driven from the opposite side.

LVL Connections (for uniform distributed loads)



- -Nails to be 3 1/2" spiral wire nails.
- -Nails to be located a minimum of 2 1/2" from the top and bottom of the member. Start all nails a minimum of 2 1/2" in from ends.
- -Minimum 3 1/4" spacing between rows.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail or screw driven from the opposite side.

Vertical I-Joist Connections (for uniform distributed loads)



Vertical I-Joist connection notes:

- -Nails to be 3" spiral wire nails.
- -Nails to be located at centre of top and bottom flanges. Start all nails a minimum of 2 1/2" in from ends.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail driven from the opposite side.



MULTI-PLY CONNECTION **DETAILS**

Date: November 30, 2016

KOTT 3228 Moodie Drive Ottawa, ON **K2H 7V1** Ph: 613-838-2775

Fx: 613-838-4751

Engineering Note Page (ENP-2)

REVISION 2009-10-09

Please read all notes prior to installation of the component

GREEN YORK HOMES-GRANELLI HOME CORP-INDIGO 1 (ELEV.1&2

DESIGN INFORMATION

This building component is certified as an individual component for the loads and conditions shown on the calculation and drawing page.

The responsibility of the undersigned engineer is <u>only</u> limited to the calculation of this building component for the loads and conditions shown on this drawing.

The responsibility of the undersigned is limited to the verification of the structural capacity of the NASCOR floor joists and LVL beams based on placement as shown on the layout. The loads applied are limited to the gravity effects of the specified loads. The structural integrity of the building and the effect of wind, uplift, seismic, lateral or other forces, calculation of adequate support and anchorage of components, as well as the dimensions and design loads used to calculate components are the responsibility of the overall building designer.

Floor joists and OSB rim board are designed to carry uniformly distributed loads only. Point loads should be transferred through the floor cavity with squash blocks. Structural elements such as walls, posts, connectors, and squash blocks are the responsibility of the overall building designer.

The undersigned engineer disclaims any responsibility for damages as a result of being furnished faulty or incorrect information, specifications and/or designs.

Installation of NASCOR joists is to be carried out in accordance with the current edition of the manufacturer's approved literature available at http://www.nascor.ca.

CODE

This building component is designed in accordance with the National Building Code of Canada, the Ontario Building Code, CCMC and Canadian Standards Association guidelines.

COMPONENT

- 1. The building component used in construction must be the same as indicated on the drawings.
- 2. The building component must be installed and assembled as per specification shown on the drawing and in accordance with the manufacturer's assembly and installation.
- 3. Members consisting of multiple plies must be connected as per the document "Multi-ply Connection Details".
- 4. Pass-thru squash block framing is required at all point loads over bearings.

HANDLING AND INSTALLATION

Do not drill any hole, cut or notch a certified building component without a written preauthorization.



Client: Project:

Address:

GREEN YORK HOMES

Date: 6/1/2018

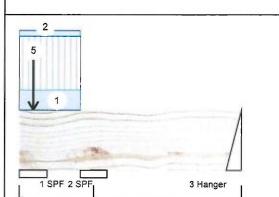
Designer: RCO

Job Name: INDIGO 1 (ELEV.1)

Level: Ground Floor

Project #:

F11-C Forex 2.0E-3000Fb LVL 1.750" X 11.875" - PASSED



3'8 1/16"

2'5 5/16"

Member Inforn	nation
---------------	--------

1'2 3/4"

•					
	Туре:	Girder	Application:	Floor (Residential)	•
	Plies:	1	Design Method:	LSD	
	Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012	
	Deflection LL:	360	Load Sharing:	No	
	Deflection TL:	240	Deck:	Not Checked	
	Importance:	Normal	Vibration:	Not Checked	
	General Load				
	Floor Live:	40 PSF			
	Dead:	15 PSF			

Unfactored Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	Wind
1	104	57	0	0
2	4	11	0	0
3	0 (0)	6	0	0
=				

Analysis Results

Г	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
	Neg Moment	-3 ft-lb	1'2 3/4"	11135 ft-lb	0.000 (0%)	1.4D	Uniform
	Unbraced	-3 ft-lb	1'2 3/4"	11135 ft-lb	0.000 (0%)	1.4D	Uniform
	Pos Moment	3 ft-lb	2'7 1/16"	11135 ft-lb	0.000 (0%)	1.4D	Uniform
	Unbraced	3 ft-lb	2'7 1/16"	10807 ft-lb	0.000 (0%)	1.4D	Uniform
	Shear	11 lb	1'4 5/8"	3769 lb	0.003 (0%)	1.4D	Uniform
	Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
	LL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
	TL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	5.500"	5%	69 / 156	225	L_	1.25D+1.5L
2 - SPF	5.250"	1%	19/0	19	Uniform	1.4D
3 - Hanger	3.000"	0%	7/0	7	Uniform	1.4D

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS



Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Fill all hanger nailing holes.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top braced at bearings.

- 1	5 Bottom braced	at bearings.								
	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
	1	Tie-In	0-0-0 to 1-0-2	(Span)0-11-1	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	2	Part. Uniform	0-0-0 to 1-0-1		Тор	2 PLF	0 PLF	0 PLF	0 PLF	
١	3	Point	0-2-12		Тор	21 lb	55 lb	0 lb	0 lb	J6
	4	Point	0-2-12		Тор	13 lb	35 lb	0 lb	0 lb	J3
ı	5	Point	0-2-12		Тор	13 lb	0 lb	0 lb	0 lb	Wall Self Weight
1		Self Weight				5 PLF				

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- chemicals

Handling & Installation

- andling & Installation
 LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
 approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at beamg points to avoid
 lateral displacement and rotation

- For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex APA: PR-L318







EWP Studio Simpson Strong-Tie® Component Solutions™ Client:

Project: Address:

GREEN YORK HOMES

Date: 6/1/2018

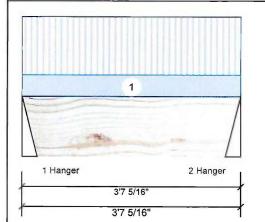
Designer: RCO

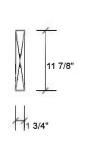
Job Name: INDIGO 1 (ELEV.1)

Level: Ground Floor

Project #:

F11-D Forex 2.0E-3000Fb LVL 1.750" X 11.875" - PASSED





Wind

0

0

M	em	ber	Info	orma	tion

Туре:	Girder	Application:	Floor (Residential)
Plies:	1	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

Unfactored	I Reactions	UNPATTERNED	lb (Uplift)
Brg	Live	Dead	Snow

28

28

0

2

53

53

Bearings and Factored Reactions										
	Length		React D/L lb	Total	Ld. Case	Ld. Comb.				
1 - Hanger	3.000"	3%	35 / 79	114	L	1.25D+1.5L				
2 -	3.000"	3%	35 / 79	114	L	1.25D+1.5L				

Analysis Results

Analysis	Antural	Lagation	A II	0	Ozank	^	-
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case	
Moment	83 ft-lb	1'9 11/16"	17130 ft-lb	0.005 (0%)	1.25D+1.5L	L	
Unbraced	83 ft-lb	1'9 11/16"	13233 ft-lb	0.006 (1%)	1.25D+1.5L	L	
Shear	40 lb	1'2 1/8"	5798 lb	0.007 (1%)	1.25D+1.5L	L	
Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)			
LL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)			
TL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)			

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS



Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.
- 4 Bottom braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 3-7-5	(Span)1-5-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	Self Weight				5 PLF				

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

andling & Installation
LVL beams must not be cut or drilled
Refer to manufacturer's product information
regarding installation requirements, multi-ply
fastening details, beam strength values, and code
approvals
Damaged Beams must not be used
Design assumes top edge is laterally restrained
Provide lateral support at beaming points to avoid
lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent

Manufacturer Info

Forex





EWP Studio Simpson Strong-Tie® Component Solutions™ Client: GREEN YORK HOMES

Project: Address:

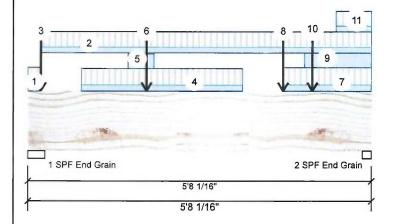
Date: 6/1/2018 Designer: RCO

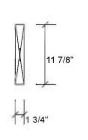
Job Name: INDIGO 1 (ELEV.1)

Project #:

Forex 2.0E-3000Fb LVL 1.750" X 11.875" - PASSED F12-B

Level: Ground Floor





Wind

N	lem	ber	Inf	orma	tion

Girder	Application:	Floor (Residential)
1	Design Method:	LSD
on: Dry	Building Code:	NBCC 2010 / OBC 2012
360	Load Sharing:	No
240	Deck:	Not Checked
Normal	Vibration:	Not Checked
40 PSF		
15 PSF		
	1 on: Dry 360 240 Normal	1 Design Method: Building Code: 360 Load Sharing: 240 Deck: Normal Vibration:

Unfactored Reactions UNPATTERNED Ib (Uplift)

1	190	349	U	U
2	933	449	0	0

Snow

Dead

Analysis Results

L							
	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
l	Moment	2299 ft-lb	2'4 7/16"	17130 ft-lb	0.134 (13%)	1.25D+1.5L	L
l	Unbraced	2299 ft-lb	2'4 7/16"	8377 ft-lb	0.274 (27%)	1.25D+1.5L	L
l	Shear	1466 lb	4'7 3/16"	5798 lb	0.253 (25%)	1.25D+1.5L	L
l	Perm Defl in.	0.008 (L/7964)	2'9 5/8"	0.179 (L/360)	0.050 (5%)	D	Uniform
ı	LL Defl inch	0.018 (L/3518)	2'9 5/8"	0.179 (L/360)	0.100 (10%)	L	L
	TL Defl inch	0.026 (L/2440)	2'9 5/8"	0.268 (L/240)	0.100 (10%)	D+L	L

Bearings and Factored Reactions

Live

Brg

Bearing	Length	Cap. I	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	3.375"	37%	436 / 1197	1633	L	1.25D+1.5L
2 - SPF End Grain	1.750"	86%	562 / 1399	1961	L	1.25D+1.5L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY

BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS

Design Notes

1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.

Location

0-2-9

1-11-7

4-2-9 to 5-8-1 (Span)3-9-1

0-0-0 to 0-2-9

0-2-8 to 5-8-1

0-10-9 to 3-6-9

1-7-13 to 2-0-15

Trib Width

(Span)3-9-1

(Span)3-4-0

Side

Top

Top

Top

Top

Top

Near Face

Near Face

- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.

4 Bottom braced at bearings.

Load Type

Part. Uniform

Part. Uniform

Tie-In

Tie-In

Point

Point

NAILING OR BOLTING REQUIREMENTS. PASS THRU FRAMING SQUASH

Dead Live Snow Wind Comments 15 PSF 40 PSF 0 PSF 0 PSF 15 PSF 40 PSF 0 PSF 0 PSF 23 lb 61 lb 0 lb 0 lb .11 27 PLF 72 PLF 0 PLF 0 PLF 64 PLF 0 PLF 0 PLF 0 PLF Wall Self Weight 188 lb 468 lb 0 lb 0 lb F11 F11

Continued on page 2...

Notes

ID

1

2

3

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

chemicals

Handling & Installation

- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- fastening vewers, approvals approvals approvals Damaged Beams must not be used Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

40 PSF

15 PSF

Manufacturer Info

0 PSF

0 PSF



Page 2 of 2

EWP Studio Simpson Strong-Tie® Component Solutions™ Client:

Project: Address: GREEN YORK HOMES

6/1/2018

Date: Designer:

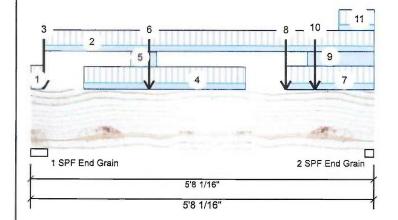
RCO

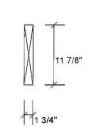
Project #:

F12-B Forex 2.0E-3000Fb LVL 1.750" X 11.875" - PASSED

Level: Ground Floor

Job Name: INDIGO 1 (ELEV.1)





Continued from p	age 1								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
8	Point	4-2-9		Near Face	38 lb	101 lb	0 lb	0 lb	J1
9	Part. Uniform	4-6-13 to 5-8-1		Тор	64 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
10	Point	4-8-5		Тор	154 lb	381 lb	0 lb	0 lb	F11 F11
11	Part, Uniform	5-1-1 to 5-8-1		Тор	25 PLF	65 PLF	0 PLF	0 PLF	J1
	Self Weight				5 PLF				

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARING



Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- andling & Installation
 LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 featsening details, beam strength values, and code
 approvals
 Damaged Beams must not be used
 Dosign assumes top edge is laterally restrained
 Provide lateral support at beaming points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent

Manufacturer Info

Forex





EWP Studio Simpson Strong-Tie® Component Solutions™ Client:

Project:

GREEN YORK HOMES

Address:

Designer: RCO

Job Name: INDIGO 1 (ELEV.1)

Level: Ground Floor

6/1/2018

F12-C Forex 2.0E-3000Fb LVL 1.750" X 11.875" - PASSED

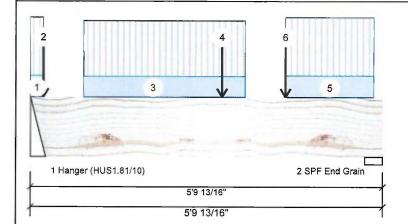
Brg

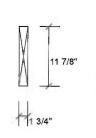
1

Bearing Length

Hanger

3.000"





Wind

Ld. Comb.

1.25D+1.5L

1.25D+1.5L

Member Information

Type:	Girder	Application:	Floor (Residential)
Plies:	1	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

Unfactored Reactions UNPATTERNED Ib (Uplift)

Dead

Cap. React D/L lb

221

Bearing	Bearings and Factored Reactions								
2	613	251	O	0					

276 / 805

Snow

0

Total Ld. Case

1081 L

1234 L

Comments

.11

C4

J1

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2367 ft-lb	3'1 15/16"	17130 ft-lb	0.138 (14%)	1.25D+1.5L	L
Unbraced	2367 ft-lb	3'1 15/16"	8328 ft-lb	0.284 (28%)	1.25D+1.5L	L
Shear	1110 lb	4'7 3/16"	5798 lb	0.191 (19%)	1.25D+1.5L	L
Perm Defl in.	0.007 (L/9287)	3'1 15/16"	0.180 (L/360)	0.040 (4%)	D	Uniform
LL Defl inch	0.017 (L/3759)	3'1 15/16"	0.180 (L/360)	0.100 (10%)	L	L
TL Defl inch	0.024 (L/2676)	3'1 15/16"	0.270 (L/240)	0.090 (9%)	D+L	L

2 - SPF 3.500" 27% 314 / 920 End Grain

28%

Live

537

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA

USED IN THE DESIGN OF THIS COMPONENT. REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY

NAILING OR BOLTING REQUIREMENTS. PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS



Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.
- 4 Bottom braced at bearings

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind
1	Tie-In	0-0-0 to 0-2-9	(Span)3-9-1	Тор	15 PSF	40 PSF	0 PSF	0 PSF
2	Point	0-2-9		Far Face	23 lb	61 lb	0 lb	0 lb
3	Part. Uniform	0-10-9 to 3-6-9		Far Face	27 PLF	72 PLF	0 PLF	0 PLF
4	Point	3-1-15		Тор	264 lb	671 lb	0 lb	0 lb
5	Tie-In	4-2-9 to 5-8-1	(Span)3-9-1	Тор	15 PSF	40 PSF	0 PSF	0 PSF
6	Point	4-2-9		Far Face	38 lb	101 lb	0 lb	0 lb
	Self Weight				5 PLF			

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the Intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- andling & Installation

 LVL beams must not be cut or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-pyf fastening details, beam strength values, and code approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex





Member Information

Page 1 of 2

EWP Studio Simpson Strong-Tie® Component Solutions™ Client: Project: Address:

GREEN YORK HOMES

6/1/2018 Date:

RCO Designer: Job Name: INDIGO 1 (ELEV.1)

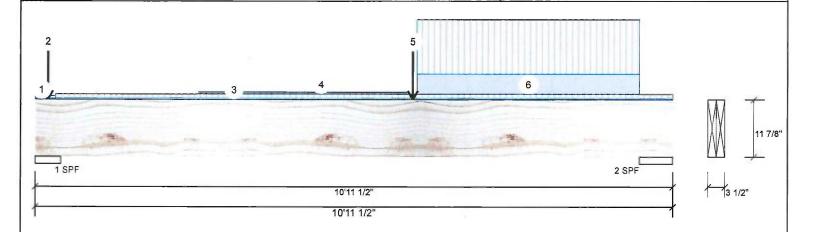
Project #:

Forex 2.0E-3000Fb LVL F14-B

1.750" X 11.875"

2-Ply - PASSED Level: Ground Floor

Unfactored Reactions UNPATTERNED Ib (Uplift)



Type:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind
Plies:	2	Design Method:	LSD	1	532	295	0	0
Moisture Conditio	n: Dry	Building Code:	NBCC 2010 / OBC 2012	2	1275	542	0	0
Deflection LL:	360	Load Sharing:	No	-				
Deflection TL:	240	Deck:	Not Checked					
Importance:	Normal	Vibration:	Not Checked					
General Load								
Floor Live:	40 PSF			Bearings	and Fact	ored Reactions		
Dead:	15 PSF			Bearing	Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
				1 - SPF	5.250"	10% 369 / 798	1167 L	1.25D+1.5L
				2-SPF	6.875"	17% 678 / 1913	2590 L	1.25D+1.5L
Analysis Resul	ts							

ľ							
ſ	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
l	Moment	5728 ft-lb	6'5 7/8"	34261 ft-lb	0.167 (17%)	1.25D+1.5L	L
l	Unbraced	5728 ft-lb	6'5 7/8"	29540 ft-lb	0.194 (19%)	1.25D+1.5L	L
l	Shear	2022 lb	9'5 1/2"	11596 lb	0.174 (17%)	1.25D+1.5L	L
l	Perm Defl in.	0.023 (L/5246)	5'11 1/2"	0.336 (L/360)	0.070 (7%)	D	Uniform
l	LL Defl inch	0.053 (L/2301)	6' 3/8"	0.336 (L/360)	0.160 (16%)	L	L
l	TL Defl inch	0.076 (L/1600)	6' 1/16"	0.504 (L/240)	0.150 (15%)	D+L	L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARING



Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on full section width.

I	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
l	1	Tie-In	0-0-0 to 0-4-2	(Span)0-7-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
I	2	Point	0-2-10		Тор	37 lb	0 lb	0 lb	0 lb	Wall Self Weight
	3	Tie-In	0-4-2 to 10-11-8	(Span)0-11-11 to 0-11-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	4	Tie-In	2-9-11 to 6-5-0	(Span)0-4-5 to 0-4-5	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

Continued on page 2...

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- andling & Installation
 LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
 approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent

Manufacturer Info Forex



Page 2 of 2

EWP Studio Simpson Strong-Tie® Component Solutions™

Client:

Project: Address: GREEN YORK HOMES

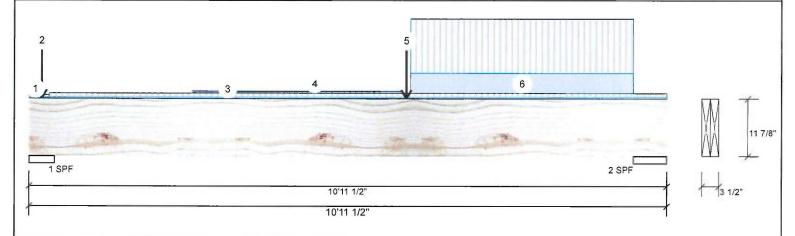
Date: 6/1/2018

Designer: RCO Job Name: INDIGO 1 (ELEV.1)

Project #:

F14-B Forex 2.0E-3000Fb LVL 1.750" X 11.875"

2-Ply - PASSED Level: Ground Floor



.Continued from page 1

ID Trib Width Load Type Location Side Dead Live Snow Wind Comments 5 Point F12 6-5-14 Far Face 221 lb 537 lb 0 lb 0 lb 6 Part. Uniform 101 PLF 270 PLF 0 PLF 0 PLF 6-6-12 to 10-4-10 Top Self Weight 10 PLF

> READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARING



Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- andling & Installation
 LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
 approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent

Manufacturer Info Forex

APA: PR-L318





EWP Studio Simpson Strong-Tie® Component Solutions™ Client:

Project: Address: GREEN YORK HOMES

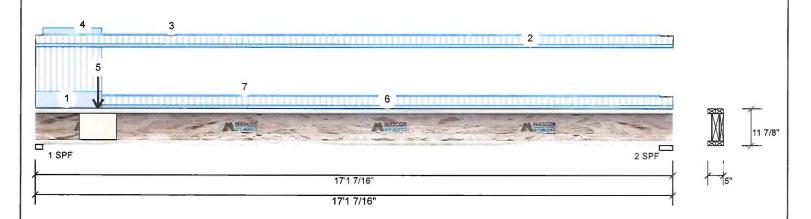
Date: 6/1/2018 Designer:

RCO Job Name: INDIGO 1 (ELEV.1)

Project #:

11.875" 2-Ply - PASSED F15-A NJH

Level: Ground Floor



Member Inforn	nation			Unfacto	red Reac	tions L	NPATTERN	ED lb ((Uplift)
Туре:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Sno	W
Plies:	2	Design Method:	LSD	1	755		381		0
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012	2	279		144		0
Deflection LL:	360	Load Sharing:	No	-					
Deflection TL:	240	Deck:	Not Checked						
Importance:	Normal	Vibration:	Not Checked						
General Load									
Floor Live:	40 PSF			Bearing	s and Fac	tored	Reactions		
Dead:	15 PSF			Bearing	Length	Cap.	React D/L lb	Total	Ld. Case
				1 - SPF	2.375"	49%	477 / 1133	1609	L
				2-SPF	4.319"	17%	180 / 419	599	L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2960 ft-lb	6'8 1/8"	10780 ft-lb	0.275 (27%)	1.25D+1.5L	L
Unbraced	2960 ft-lb	6'8 1/8"	2970 ft-lb	0.997 (100%)	1.25D+1.5L	L
Shear	1588 lb	1 5/8"	3620 lb	0.439 (44%)	1.25D+1.5L	L
Perm Defl in.	0.065 (L/3060)	8' 1/16"	0.556 (L/360)	0.120 (12%)	D	Uniform
LL Defl inch	0.127 (L/1581)	7'11 15/16"	0.556 (L/360)	0.230 (23%)	L	L
TL Defl inch	0.192 (L/1042)	8'	0.834 (L/240)	0.230 (23%)	D+L	L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARING



Wind

0

0

Ld. Comb.

1.25D+1.5L

1.25D+1.5L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 7'9" o.c.

5 Bottom hange	e braced at bearings.								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-9-6	(Span)3-5-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 17-1-7	(Span)0-7-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Part. Uniform	0-2-5 to 16-9-2		Тор	2 PLF	0 PLF	0 PLF	0 PLF	
4	Part. Uniform	0-2-5 to 1-9-6		Тор	9 PLF	0 PLF	0 PLF	0 PLF	
5	Point	1-8-2		Far Face	240 lb	480 lb	0 lb	0 lb	F6
6	Tie-In	1-9-6 to 17-1-7	(Span)0-8-1	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
7	Part. Uniform	1-9-6 to 16-9-1		Тор	2 PLF	0 PLF	0 PLF	0 PLF	

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 Upist not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- tandaing & Installation

 I Joist flanges must not be cut or drilled.
 Refer to latest copy of the Joist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-ply fastening details and handling/erection details.
 Damaged Joists must not be used
 Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.
- 5. Provide lateral support at bearing points to avoid
- 5. Provide lateral support at dealing points to desire lateral displacement and rotation
 6. Web stiffeners for point load as shown Minimum point load bearing length= 3.5 linches
 7. For flat roofs provide proper drainage to prevent
- ponding

Manufacturer Info

Nascor by Kott





GREEN YORK HOMES Client:

Project: Address:

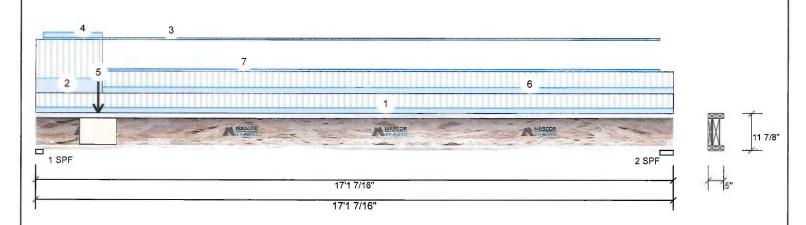
6/1/2018 Date: RCO Designer:

Job Name: INDIGO 1 (ELEV.1)

Project #:

2-Ply - PASSED F15-B 11.875" NJH

Level: Ground Floor



Brg

Bearing Length

1 - SPF 2.375"

2 - SPF 4.319"

Type:	Girder	Application:	Floor (Residential)
Plies:	2	Design Method:	LSD
Moisture Condition	: Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

Unfactored Reactions UNPATTERNED Ib (Uplift)

Dead

Cap. React D/L lb

53%

29%

417

Live

832

2	496	248	0	0
Bearing	s and Factored	d Reactions		

521 / 1248

310 / 744

Snow

n

Total Ld. Case

1769

1054 L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	4604 ft-lb	7'9 15/16"	10780 ft-lb	0.427 (43%)	1.25D+1.5L	L
Unbraced	4604 ft-lb	7'9 15/16"	4611 ft-lb	0.998 (100%)	1.25D+1.5L	L
Shear	1744 lb	1 5/8"	3620 lb	0.482 (48%)	1.25D+1.5L	L
Perm Defl in.	0.100 (L/2004)	8'3 3/16"	0.556 (L/360)	0.180 (18%)	D	Uniform
LL Defl inch	0.198 (L/1013)	8'3 3/16"	0.556 (L/360)	0.360 (36%)	L	L
TL Defl inch	0.298 (L/673)	8'3 3/16"	0.834 (L/240)	0.360 (36%)	D+L	L

READ ALL NOTES ON THIS PAGE AND ON **ENGINEERING NOTE PAGE ENP-2. THIS** NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



Wind

Ld. Comb.

1.25D+1.5L

1.25D+1.5L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 6'2" o.c.

5 Bottom flange	braced at bearings.								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 17 -1- 7	(Span)1-3-1	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 1-9-6	(Span)3-5-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Part. Uniform	0-2-6 to 16-9-1		Тор	3 PLF	0 PLF	0 PLF	0 PLF	
4	Part. Uniform	0-2-6 to 1-9-6		Тор	9 PLF	0 PLF	0 PLF	0 PLF	
5	Point	1-8-2		Near Face	172 lb	343 lb	0 lb	0 lb	F6
6	Tie-In	1-9-6 to 17-1-7	(Span)1-4-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
7	Part. Uniform	1-9-6 to 16-9-1		Тор	4 PLF	0 PLF	0 PLF	0 PLF	

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design orderia and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 Idoist not to be treated with fire retardant or corrosive

Handling & Installation

- Nandling & Installation
 Installation
 Refer to latest copy of the IJoist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-ply fastening details and handling/erection details
 Damaged IJoists must not be used
 Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length≥= 3,5 inches
 For flat roofs provide proper drainage to prevent ponding

Manufacturer Info Nascor by Kott





EWP Studio Simpson Strong-Tie® Component Solutions™ Client:

Project: Address: GREEN YORK HOMES

Date:

6/1/2018

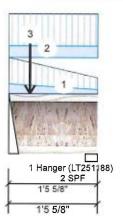
RCO Designer:

Job Name: INDIGO 1 (ELEV.1)

Level: Ground Floor

Project #:

11.875" - PASSED F5-A NJH



11 7/8"

		Information
I	Tuno	Cindo

Type:	Girder
Plies:	1
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	
Floor Live:	40 PSF
Dead:	15 PSF

Application: Floor (Residential) Design Method: LSD NBCC 2010 / OBC 2012 Building Code:

Load Sharing: No Deck:

Not Checked Vibration: Not Checked

Unfactored Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	Wind
1	104	39	0	0
2	79	30	0	0

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	47 ft-lb	7 3/4"	5390 ft-lb	0.009 (1%)	1.25D+1.5L	L
Unbraced	47 ft-lb	7 3/4"	5334 ft-lb	0.009 (1%)	1.25D+1.5L	L
Shear	180 lb	1 1/4"	1810 lb	0.100 (10%)	1.25D+1.5L	L
Perm Defl in.	0.000 (L/999)	٥	999.000 (L/0)	0.000 (0%)		
LL Defl inch	0.000 (L/34775)	7 13/16"	0.041 (L/360)	0.010 (1%)	L	L
TL Defl inch	0.001	7 13/16"	0.061 (L/240)	0.010 (1%)	D+L	L

Bearings and Factored Reactions Bearing Length Cap. React D/L lb

Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.	
1 - Hanger	2.000"	13%	49 / 156	204	L	1.25D+1.5L	
2 - SPF	2.375"	9%	37 / 119	156	L	1.25D+1.5L	

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARING



Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top flange unbraced.
- 4 Bottom flange braced at bearings.

ı										
	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
	1	Tie-In	0-0-0 to 1-5-10	(Span)2-8-1 to 1-2-7	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
ı	2	Tie-In	0-0-0 to 1-5-10	(Span)3-2-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
ı	3	Point	0-4-3		Far Face	12 lb	32 lb	0 lb	0 lb	J1

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive
- Handling & Installation
- 5. Provide lateral support at bearing points to avoid lateral displacement and rotation
 6. Web stiffeners for point load as shown Minimum point load bearing length=3.5 inches point to advantage of the load bearing length=3.5 inches chart, bridging details, multi-ply fastening details and handling/erection details

 C Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

Manufacturer Info

Nascor by Kott





Client:

Project: Address: **GREEN YORK HOMES**

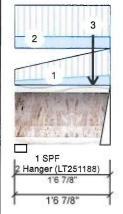
6/1/2018 Designer: RCO

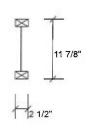
Job Name: INDIGO 1 (ELEV.1)

Project #:

11.875" - PASSED F5-B NJH

Level: Ground Floor





Member Inform	nation		
Туре:	Girder	Application:	Floor (Residential)
Plies:	1	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		

Unfactored Reactions UNPATTERNED Ib (Uplift) Brg Snow Live Dead Wind 1 83 31 0 0 2 114 0

Analysis Results Analysis Actual Location Allowed Capacity Comb. 0.010 (1%) 1.25D+1.5L L Moment 52 ft-lb 10 3/8" 5390 ft-lb Unbraced 52 ft-lb 10 3/8" 5319 ft-lb 0.010 (1%) 1.25D+1.5L L 201 lb 1'5 5/8" 1810 lb 0.111 (11%) 1.25D+1.5L L 0 999.000 (L/0) 0.000 (0%) 10 5/16" 0.044 (L/360) 0.010 (1%) L L

Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1 - SPF 2.375" 10% 39 / 124 163 L 1.25D+1.5L 2.000" 14% 53 / 172 225 L 1.25D+1.5L Hanger

Shear Perm Defl in. 0.000 (L/999) LL Defl inch 0.000 (L/33591) TL Defl inch 0.001 10 5/16" 0.067 (L/240) 0.010 (1%) D+L (L/24438)

NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

READ ALL NOTES ON THIS PAGE AND ON

ENGINEERING NOTE PAGE ENP-2. THIS

Bearings and Factored Reactions

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Design Notes

Dead:

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top flange unbraced.
- 4 Bottom flange braced at bearings.

15 PSF

Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
Tie-In	0-0-0 to 1-6-14	(Span)1-1-15 to 2-8-13	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
Tie-In	0-0-0 to 1-6-14	(Span)3-2-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
Point	1-3-7		Far Face	13 lb	35 lb	0 lb	0 lb	J1
	Tie-In Tie-In	Tie-In 0-0-0 to 1-6-14 Tie-In 0-0-0 to 1-6-14	Tie-In 0-0-0 to 1-6-14 (Span)1-1-15 to 2-8-13 Tie-In 0-0-0 to 1-6-14 (Span)3-2-8	Tie-In 0-0-0 to 1-6-14 (Span)1-1-15 Top to 2-8-13 Tie-In 0-0-0 to 1-6-14 (Span)3-2-8 Top	Tie-In 0-0-0 to 1-6-14 (Span)1-1-15 Top to 2-8-13 Tie-In 0-0-0 to 1-6-14 (Span)3-2-8 Top 15 PSF	Tie-In 0-0-0 to 1-6-14 (Span)1-1-15 Top to 2-8-13 Tie-In 0-0-0 to 1-6-14 (Span)3-2-8 Top 15 PSF 40 PSF	Tie-In 0-0-0 to 1-6-14 (Span)1-1-15 Top 15 PSF 40 PSF 0 PSF to 2-8-13 Tie-In 0-0-0 to 1-6-14 (Span)3-2-8 Top 15 PSF 40 PSF 0 PSF	Tie-In 0-0-0 to 1-6-14 (Span)1-1-15 Top 15 PSF 40 PSF 0 PSF to 2-8-13 Tie-In 0-0-0 to 1-6-14 (Span)3-2-8 Top 15 PSF 40 PSF 0 PSF 0 PSF

Notes

Calculated Structured Designs is responsible only of the Structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

- arrouting & Installation

 Loist flanges must not be cut or drilled
 Refer to latest copy of the Joist product information
 details for framing details, stiffener tables, web hole
 chart, bindging details, multi-py fastening details and
 handling/erection details
 Damaged Joists must not be used
 Design assumes top flange to be laterally restrained
 by attached sheathing or as specified in engineering
 notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing lengths—3.5 inches
 For flat roofs provide proper drainage to prevent

Manufacturer Info

Nascor by Kott









Client: Address:

Project:

GREEN YORK HOMES

Date:

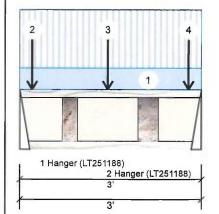
6/1/2018 RCO Designer:

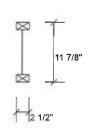
Job Name: INDIGO 1 (ELEV.1)

Project #:

11.875" - PASSED NJH F6-A

Level: Ground Floor





Member Infor	mation		
Type:	Girder	Application:	Floor (Residential)
Plies:	1	Design Method:	LSD
Moisture Condition	n: Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			

Unfactored	Reactions	UNPATTER	NED lb (Uplift)	
Brg	Live	Dead	Snow	Wind
1	441	166	0	0
0	420	105	•	0

General Load	d						
Floor Live:	40 PSF						
Dead:	15 PSF						
Analysis R	esults		· ·	-			
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case	
Moment	571 ft-lb	1'5 7/16"	5390 ft-lb	0.106 (11%)	1.25D+1.5L	L	

Bearings and Factored Reactions									
Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.			
1 - Hanger	2.000"	54%	207 / 662	869	L	1.25D+1.5L			
2 - Hanger	2.000"	53%	206 / 659	865	L	1.25D+1.5L			

571 ft-lb 1'5 7/16" 4800 ft-lb 0.119 (12%) 1.25D+1.5L L Unbraced Shear 862 lb 1 1/4" 1810 lb 0.476 (48%) 1.25D+1.5L L Perm Defl in. 0.002 1'5 7/16" 0.093 (L/360) 0.020 (2%) D Uniform (L/14698) LL Defl inch 0.006 (L/5525) 1'5 7/16" 0.093 (L/360) 0.070 (7%) L L TL Defl inch 0.008 (L/4016) 1'5 7/16" 0.140 (L/240) 0.060 (6%) D+L

NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

READ ALL NOTES ON THIS PAGE AND ON

ENGINEERING NOTE PAGE ENP-2. THIS

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top flange unbraced.

4 bottom liange braced at bearings.					TOTAL CONTROL OF THE						
	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments	•
	1	Tie-In	0-0-0 to 3-0-0	(Span)1-8-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
	2	Point	0-2-7		Far Face	81 lb	215 lb	0 lb	0 lb	J4	
	3	Point	1-5-7		Far Face	126 lb	335 lb	0 lb	0 lb	J4	
	4	Point	2-9-7		Far Face	85 lb	227 lb	0 lb	0 lb	J4	

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

- Dry service conditions, unless noted otherwise
 Upoist not to be treated with fire retardant or corrosive
- chemicals

Handling & Installation

- andling & Installation
 Loist flanges must not be cut or drilled
 Refer to latest copy of the Lioist product information
 details for framing details, sufferer tables, web hole
 chart, bridging details, multi-hyp fastening details and
 handling/erection details
 Damaged blosts must not be used
 Design assumes top flange to be laterally restrained
 by attached sheathing or as specified in engineering
 notes.

- 5. Provide lateral support at bearing points to avoid
- lateral displacement and rotation

 6. Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches

 7. For flat roofs provide proper drainage to prevent
- ponding

Manufacturer Info

Nascor by Kott



EWP Studio Simpson Strong-Tie® Component Solutions™

Client:

Project: Address: GREEN YORK HOMES

Date: 6/1/2018

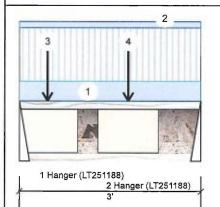
Designer: RCO Page 1 of 1

Job Name: INDIGO 1 (ELEV.1)

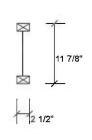
Project #:

11.875" - PASSED F6-B NJH

Level: Ground Floor



3'



Mansh	er Infori	mation
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THE THE PARTY OF T								
Type:	Girder							
Plies:	1							
Moisture Condition:	Dry							
Deflection LL:	360							
Deflection TL:	240							
Importance:	Normal							
General Load								
Floor Live:	40 PSF							
Dead:	15 PSF							

Floor (Residential) Application: LSD Design Method: NBCC 2010 / OBC 2012 **Building Code:** Load Sharing: No Deck: Not Checked Vibration: Not Checked

Unfactored Reactions UNPATTERNED Ib (Uplift) Dead Snow Brg Live Wind 480 240 0 0 1 2 343 172 0 0

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	749 ft-lb	1'9 9/16"	5390 ft-lb	0.139 (14%)	1.25D+1.5L	L
Unbraced	749 ft-lb	1'9 9/16"	4800 ft-lb	0.156 (16%)	1.25D+1.5L	L
Shear	1013 lb	1 1/4"	1810 lb	0.560 (56%)	1.25D+1.5L	L
Perm Defl in.	0.004 (L/9092)	1'9 9/16"	0.093 (L/360)	0.040 (4%)	D	Uniform
LL Defl inch	0.007 (L/4555)	1'9 9/16"	0.093 (L/360)	0.080 (8%)	L	L
TL Defl inch	0.011 (L/3035)	1'9 9/16"	0.140 (L/240)	0.080 (8%)	D+L	L,

Bearings and Factored Reactions Bearing Length Cap. React D/L lb

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - Hanger	2.000"	63%	300 / 721	1021	L	1.25D+1.5L
2 - Hanger	2.000"	45%	215 / 515	729	L	1.25D+1.5L

Comments

J8

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA

USED IN THE DESIGN OF THIS COMPONENT REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL**



Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top flange unbraced.
- 4 Bottom flange braced at bearings.

Point

Γ	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow
l	1	Tie-In	0-0-0 to 3-0-0	(Span)1-8-11	Тор	15 PSF	40 PSF	0 PSF
l	2	Part. Uniform	0-0-0 to 3-0-0		Тор	4 PLF	0 PLF	0 PLF
	3	Point	0-5-9		Near Face	152 lb	304 lb	0 lb

Near Face

4

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 Upoist not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

1-9-9

- ARIGHING & INSTAILATION
 Librist flanges must not be cut or drilled
 Refer to latest copy of the Libist product information
 details for framing details. stiffener tables, web hole
 chart, bridging details, multi-ply fastening details and
 handling/erection details
 Damaged Libists must not be used
 Design assumes top flange to be laterally restrained
 by attached sheathing or as specified in engineering
 notes.

209 lb

416 lb

- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing lengths—3.5 inches
 For flat roofs provide proper drainage to prevent ponding.

Manufacturer Info

Nascor by Kott

0 lb

Kott Lumber Company 14 Anderson Blvd, Ontario Canada L4A 7X4 905-642-4400



0 PSF 0 PLF 0 lb J8

d10



Client: **GREEN YORK HOMES**

> Project: Address:

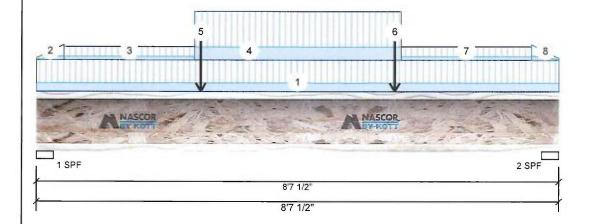
6/1/2018 Date: Designer: RCO

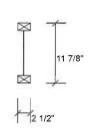
Job Name: INDIGO 1 (ELEV.1)

Project #

11.875" - PASSED F7-A NJH

Level: Ground Floor





Wind

Ld. Comb.

1.25D+1.5L

1.25D+1.5L

Member Information

Туре:	Girder
Plies:	1
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	
Floor Live:	40 PSF
Dead:	15 PSF

Application: Floor (Residential) Design Method: LSD NBCC 2010 / OBC 2012 **Building Code:** Load Sharing: No

Deck: Not Checked Vibration: Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Dead

Cap. React D/L lb

31%

31%

Live

Bearings and Factored Reactions

Bearing Length

1 - SPF 3.375"

2 - SPF 3.375"

1	272	102	0	0
2	272	102	0	0

127 / 408

127 / 408

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1259 ft-lb	4'3 3/4"	5390 ft-lb	0.234 (23%)	1.25D+1.5L	L
Unbraced	1259 ft-lb	4'3 3/4"	1315 ft-lb	0.958 (96%)	1.25D+1.5L	L
Shear	525 lb	8'4 7/8"	1810 lb	0.290 (29%)	1.25D+1.5L	L
Perm Defl in.	0.014 (L/7253)	4'3 13/16"	0.273 (L/360)	0.050 (5%)	D	Uniform
LL Defl inch	0.036 (L/2720)	4'3 13/16"	0.273 (L/360)	0.130 (13%)	L	L
TL Defl inch	0.050 (L/1978)	4'3 13/16"	0.409 (L/240)	0.120 (12%)	D+L	L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Total Ld. Case

535 L

535 L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Top flange unbraced

3 Bottom flange braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 8-7-8	(Span)1-0-9	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 0-5-7	(Span)0-0-13 to 0-6-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Tie-In	0-5-7 to 2-7-4	(Span)0-5-7	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
4	Tie-In	2-7-4 to 6-0-4	(Span)1-7-7	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
5	Point	2-8-8		Near Face	39 lb	104 lb	0 lb	0 lb	F5
6	Point	5-11-0		Near Face	39 lb	104 lb	0 lb	0 lb	F5
7	Tie-In	6-0-4 to 8-2-1	(Span)0-5-7	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
8	Tie-In	8-2-1 to 8-7-8	(Span)0-6-4	Top	15 PSF	40 PSF	0 PSF	0.PSF	

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

- Dry service conditions, unless noted otherwise
 Upoist not to be treated with fire retardant or corrosive
- chemicals

Handling & Installation

- andling & Installation

 Lioist flanges must not be cut or drilled

 Refer to latest copy of the Lioist product information

 details for framing details, stiffener tables, web hole

 eart, bridging details, multi-ply fastening details and

 handling/erection details

 Damaged Joists must not be used

 Design assumes top flange to be laterally restrained

 by attached sheathing or as specified in engineering

 notes.

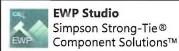
to 0-0-13

- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing lengther 3.5 inches
 For flat roofs provide proper drainage to prevent ponding.
- ponding

Manufacturer Info

Nascor by Kott





Client:

Project: Address:

GREEN YORK HOMES

6/1/2018 Date: RCO Designer:

Brg

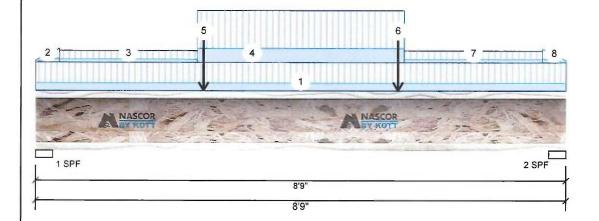
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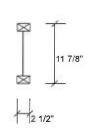
Job Name: INDIGO 1 (ELEV.1)

Project #:

11.875" - PASSED F7-B NJH

Level: Ground Floor





Wind

0

Member Information

Type:	Girder	Application:	Floor (Residential)
Plies:	1	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

Unfactored Reactions UNPATTERNED Ib (Uplift)

Dead

103

103

Snow

0

Live

275

275

	D							
	Bearing			Reactions React D/L lb	Total	Ld. Case	Ld. Comb.	
	1 - SPF	3.375"	31%	129 / 413	542	L	1.25D+1.5L	
4	2 - SPF	3.375"	31%	129 / 413	542	L	1.25D+1.5L	

Analysis Results

ı	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
l	Moment	1323 ft-lb	4'4 1/2"	5390 ft-lb	0.246 (25%)	1.25D+1.5L	L
	Unbraced	1323 ft-lb	4'4 1/2"	1326 ft-lb	0.998 (100%)	1.25D+1.5L	L
ı	Shear	533 lb	2 5/8"	1810 lb	0.294 (29%)	1.25D+1.5L	L
	Perm Defl in.	0.015 (L/6852)	4'4 9/16"	0.277 (L/360)	0.050 (5%)	D	Uniform
	LL Defl inch	0.039 (L/2576)	4'4 9/16"	0.277 (L/360)	0.140 (14%)	L	L
	TL Defl inch	0.053 (L/1872)	4'4 9/16"	0.416 (L/240)	0.130 (13%)	D+L	L

READ ALL NOTES ON THIS PAGE AND ON **ENGINEERING NOTE PAGE ENP-2. THIS** NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT **CONTAINS SPECIFICATIONS AND CRITERIA** USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Top flange must be laterally braced at a maximum of 8'2" o.c.

3 Bottom	flange braced at bearings.								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 8-9-0	(Span)0-11-5	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 0-4-11	(Span)0-0-13 to 0-5-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Tie-In	0-4-11 to 2-8-0	(Span)0-4-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
4	Tie-In	2-8-0 to 6-1-0	(Span)1-8-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
5	Point	2-9-4		Near Face	43 lb	114 lb	0 lb	0 lb	F5
6	Point	5-11-12		Near Face	43 lb	114 lb	0 lb	0 lb	F5
7	Tie-In	6-1-0 to 8-4-5	(Span)0-4-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
8	Tie-In	8-4-5 to 8-9-0	(Span)0-5-8 to 0-0-13	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 Upoist not to be treated with fire retardant or corrosive

Handling & Installation

- nanging & installation

 1. Julist flanges must not be out or drilled

 2. Refer to latest copy of the Julist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-pily festering details and handling/erection details

 3. Damaged Libists must not be used

 4. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.
- 5. Provide lateral support at bearing points to avoid
- lateral displacement and rotation

 6. Web stiffeners for point load as shown Minimum point load bearing length== 3.5 inches

 7. For flat roofs provide proper drainage to prevent

Manufacturer Info

Nascor by Kott





Client:

Project: Address:

GREEN YORK HOMES

6/1/2018

Designer: **RCO**

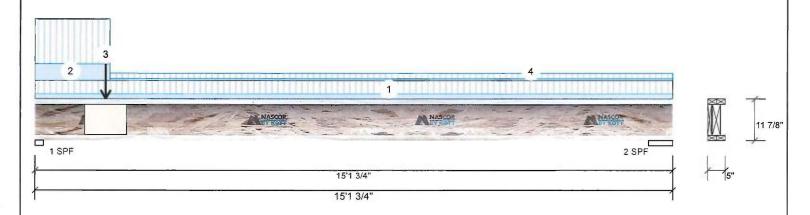
Job Name: INDIGO 1 (ELEV.1)

Project #:

Date:

2-Ply - PASSED F8-A NJH 11.875"

Level: Ground Floor



Member Inform	nation		
Type:	Girder	Application:	Floor (Residential)
Plies:	2	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

Unfactored Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	Wind
1	705	265	0	0
2	272	102	0	0

Bearings and Factored Reactions

Bearing	Length	Cap. F	React D/L lb	lotal	Ld. Case	Ld. Comb.	
1 - SPF	2.375"	42%	331 / 1057	1388	L	1.25D+1.5L	
2-SPF	6.875"	15%	128 / 409	536	L	1.25D+1.5L	

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2312 ft-lb	5'6 3/8"	10780 ft-lb	0.214 (21%)	1.25D+1.5L	L
Unbraced	2312 ft-lb	5'6 3/8"	2331 ft-lb	0.992 (99%)	1.25D+1.5L	L
Shear	1365 lb	1 5/8"	3620 lb	0.377 (38%)	1.25D+1.5L	L.
Perm Defl in.	0.031 (L/5557)	6'10 3/4"	0.483 (L/360)	0.060 (6%)	D	Uniform
LL Defl inch	0.083 (L/2086)	6'10 3/4"	0.483 (L/360)	0.170 (17%)	L	L
TL Defl inch	0.115 (L/1517)	6'10 3/4"	0.725 (L/240)	0.160 (16%)	D+L	L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS**



Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 8'8" o.c.

5 Bottom flange braced at bearings

0 0000011	nange blaced at bearing	go.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 15-1-12	(Span)1-0-1	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 1-9-6	(Span)3-5-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	1-8-2		Near Face	166 lb	441 lb	0 lb	0 lb	F6
4	Tie-In	1-9-6 to 15-1-12	(Span)0-4-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 Upoist not to be treated with fire retardant or corrosive

Handling & Installation

- Randing & Installation
 I. Joist flanges must not be cut or drilled
 Refer to latest copy of the IJoist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-ply festerning details and handling/erection details
 Damaged Lioists must not be used
 Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length ≥ 3,5 inches
 For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Nascor by Kott

14 Anderson Blvd, Ontario



Canada L4A 7X4 905-642-4400

Kott Lumber Company



EWP Studio Simpson Strong-Tie® Component Solutions™ Client:

Project:

Address:

GREEN YORK HOMES

6/1/2018 Date: RCO Designer:

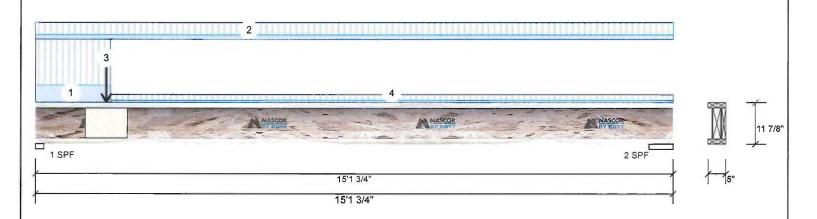
INDIGO 1 (ELEV.1) Job Name:

Project #:

11.875" 2-Ply - PASSED F8-B NJH

Level: Ground Floor

Unfactored Reactions UNPATTERNED Ib (Uplift)



Туре:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind
Plies:	2	Design Method:	LSD	1	690	259	0	0
Moisture Condition	n: Dry	Building Code:	NBCC 2010 / OBC 2012	2	259	97	0	0
Deflection LL:	360	Load Sharing:	No					
Deflection TL:	240	Deck:	Not Checked					
Importance:	Normal	Vibration:	Not Checked					
General Load								
Floor Live:	40 PSF			Bearings	and Fact	ored Reactions		
Dead:	15 PSF			Bearing	Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
				1 - SPF	2.375"	41% 324 / 1036	1360 L	1.25D+1.5L
A				2 - SPF	6.875"	14% 122 / 389	510 L	1.25D+1.5L

Analysis Results

Member Information

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2227 ft-lb	5'5 1/16"	10780 ft-lb	0.207 (21%)	1.25D+1.5L	L
Unbraced	2227 ft-lb	5'5 1/16"	2234 ft-lb	0.997 (100%)	1.25D+1.5L	L
Shear	1336 lb	1 5/8"	3620 lb	0.369 (37%)	1.25D+1.5L	L
Perm Defl in.	0.030 (L/5780)	6'10 9/16"	0.483 (L/360)	0.060 (6%)	D	Uniform
LL Defl inch	0.080 (L/2169)	6'10 9/16"	0.483 (L/360)	0.170 (17%)	L	L
TL Defl inch	0.110 (L/1577)	6'10 9/16"	0.725 (L/240)	0.150 (15%)	D+L	L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 8'9" o.c.

1	5 Bottom nange	e braced at bearings.								
	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
	1	Tie-In	0-0-0 to 1-9-6	(Span)3-5-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	2	Tie-In	0-0-0 to 15-1-12	(Span) 0-10-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	3	Point	1-8-2		Far Face	165 lb	439 lb	0 lb	0 lb	F6
ı	4	Tie-In	1-9-6 to 15-1-12	(Span)0-5-1	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 Upoist not to be treated with fire retardant or corrosive

Handling & Installation

- andling & Installation

 Joist flanges must not be cut or drilled
 Refer to latest copy of the IJoist product information
 details for framing details, stiffener tables, web hole
 chart, bridging details, multi-pyl fastening details and
 handling/erection details
 Damaged IJoists must not be used
 Design assumes top flange to be laterally restrained
 by attached sheathing or as specified in engineering
 notes.
- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing lengthb=3.5 inches
 For flat roofs provide proper drainage to prevent
- ponding

Manufacturer Info

Nascor by Kott



EWP Studio Simpson Strong-Tie® Component Solutions™

Project: Address:

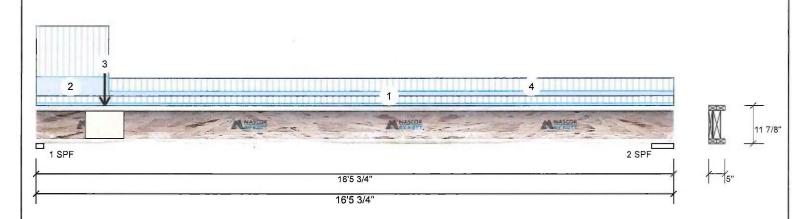
Client: **GREEN YORK HOMES** Date: 6/1/2018

Designer: RCO

Job Name: INDIGO 1 (ELEVA)

Project #:

11.875" F15-A NJH 2-Ply - PASSED Level: Ground Floor



Member Inforn	nation			Unfacto	red React	ions UI	NPATTERNI	ED Ib	(Uplift)	
Туре:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Sno	w	Wind
Plies:	2	Design Method:	LSD	1	684		256		0	0
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012	2	273		102		0	0
Deflection LL:	360	Load Sharing:	No							
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal	Vibration:	Not Checked							
General Load										
Floor Live:	40 PSF	1		Bearing:	s and Fac	tored R	eactions			
Dead:	15 PSF			Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
				1-SPF	2.375"	41%	320 / 1026	1346	L	1.25D+1.5L
				2-SPF	6.875"	15%	128 / 410	538	L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case	
Moment	2487 ft-lb	6'2 13/16"	10780 ft-lb	0.231 (23%)	1.25D+1.5L	L	
Unbraced	2487 ft-lb	6'2 13/16"	2497 ft-lb	0.996 (100%)	1.25D+1.5L	L	
Shear	1325 lb	1 5/8"	3620 lb	0.366 (37%)	1.25D+1.5L	L	
Perm Defl in.	0.039 (L/4815)	7'6 7/8"	0.528 (L/360)	0.070 (7%)	D	Uniform	
LL Defl inch	0.105 (L/1805)	7'6 7/8"	0.528 (L/360)	0.200 (20%)	L	L	
TL Defl inch	0.145 (L/1313)	7'6 7/8"	0.792 (L/240)	0.180 (18%)	D+L	L	

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS



TAGE 22 OF 38

Page 1 of 1

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 8'4" o.c.

5 Bottom flange braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 16-5-12	(Span)0-5-9	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 1-10-8	(Span)3-5-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	1-9-4		Near Face	158 lb	422 lb	0 lb	0 lb	F6
4	Tie-In	1-10-8 to 16-5-12	(Span)0-10-7	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- Libist flanges must not be cut or drilled Refer to latest copy of the Libist product information details for framing details, suffener tables, web hole chart, bridging details, multi-ley flastening details and handling/erection details Damaged bloists must not be used Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing lengthr= 3.5 inches
 For flat roofs provide proper drainage to prevent anonding

Manufacturer Info

Nascor by Kott





GREEN YORK HOMES Client:

Project: Address: Date:

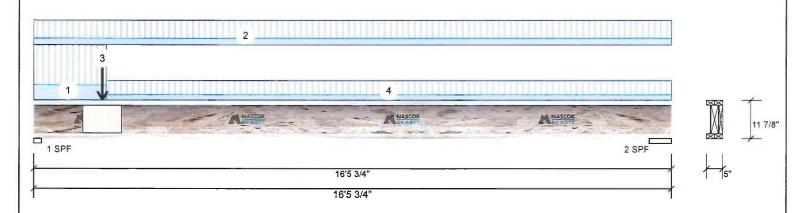
6/1/2018 RCO Designer:

Job Name: INDIGO 1 (ELEV/2)

Project #:

11.875" 2-Ply - PASSED F15-B NJH

Level: Ground Floor



Member Info	rmation			Unfactore	d Reacti	ons UNPATTERN	IED lb (Uplift)	
Type:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind
Plies:	2	Design Method:	LSD	1	846	317	0	0
Moisture Condition	n: Dry	Building Code:	NBCC 2010 / OBC 2012	2	505	189	0	0
Deflection LL:	360	Load Sharing:	No	_				
Deflection TL:	240	Deck:	Not Checked					
Importance:	Normal	Vibration:	Not Checked	1				
General Load		1112						
Floor Live:	40 PSF			Bearings a	and Facto	ored Reactions		
Dead:	15 PSF			Bearing L	ength.	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
				1 - SPF 2	.375"	50% 397 / 1269	1666 L	1.25D+1.5L
nalycic Pocu				2-SPF 6	.875"	27% 237 / 758	994 L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	4073 ft-lb	7'3 9/16"	10780 ft-lb	0.378 (38%)	1.25D+1.5L	L
Unbraced	4073 ft-lb	7'3 9/16"	4080 ft-lb	0.998 (100%)	1.25D+1.5L	L
Shear	1639 lb	1 5/8"	3620 lb	0.453 (45%)	1.25D+1.5L	L
Perm Defl in.	0.065 (L/2937)	7'9 11/16"	0.528 (L/360)	0.120 (12%)	D	Uniform
LL Defl inch	0.172 (L/1102)	7'9 11/16"	0.528 (L/360)	0.330 (33%)	L	L
TL Defl inch	0.237 (L/801)	7'9 11/16"	0.792 (L/240)	0.300 (30%)	D+L	L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARING



Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 6'8" o.c.

5 Bottom flange braced at bearings.

	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
l	1	Tie-In	0-0-0 to 1-10-8	(Span)3-5-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	2	Tie-In	0-0-0 to 16-5-12	(Span)1-6-7	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	3	Point	1-9-4		Far Face	136 lb	362 lb	0 lb	0 l b	F6
ı	4	Tie-In	1-10-8 to 16-5-12	(Span)1-2-9	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

- Dry service conditions, unless noted otherwise
 Usist not to be treated with fire retardant or corrosive
- chemicals

Handling & Installation

- Anoling & Installation
 Lolast flanges must not be out or drilled
 Refer to latest copy of the IJoist product information
 details for framing details, sulfiner tables, web hole
 chart, bridging details, multi-holy fastening details and
 handling/erection details
 Damaged IJoists must not be used
 Design assumes top flange to be laterally restrained
 by attached sheathing or as specified in engineering
 notes.
- 5. Provide lateral support at bearing points to avoid
- is revolve lateral support a beaming built to avoid lateral displacement and rotation.

 6. Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches.

 7. For flat roofs provide proper drainage to prevent

Manufacturer Info

Nascor by Kott





EWP Studio Simpson Strong-Tie® Component Solutions™ Client: **GREEN YORK HOMES**

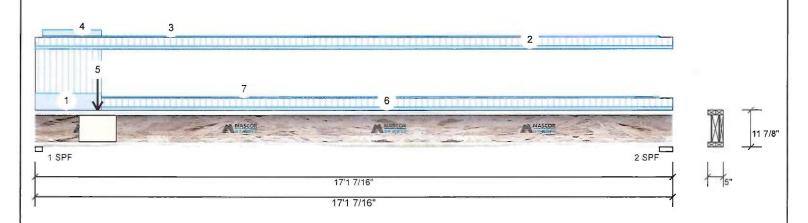
Project: Address:

6/1/2018 Date:

Designer: RCO

Job Name: INDIGO 1 (ELEV.2) Project #:

F15-C NJH 11.875" 2-Ply - PASSED Level: Ground Floor



Member Information Unfactored Reactions UNPATTERNED Ib (Uplift) Girder Type: Application: Floor (Residential) Brg Live Dead Snow Wind Plies 2 Design Method: LSD 755 381 0 0 1 NBCC 2010 / OBC 2012 Moisture Condition: Dry **Building Code:** 2 279 0 0 144 Deflection LL: 360 Load Sharing: Deflection TL: 240 Deck: Not Checked Importance: Normal Vibration: Not Checked General Load Floor Live: 40 PSF Bearings and Factored Reactions Dead: 15 PSF Cap. React D/L lb Bearing Length Total Ld. Case Ld. Comb. 1 - SPF 2.375" 49% 477 / 1133 1609 1.25D+1.5L 2 - SPF 4.319" 17% 180 / 419 599 L 1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case	
Moment	2960 ft-lb	6'8 1/8"	10780 ft-lb	0.275 (27%)	1.25D+1.5L	L	
Unbraced	2960 ft-lb	6'8 1/8"	2970 ft-lb	0.997 (100%)	1.25D+1.5L	L	
Shear	1588 lb	1 5/8"	3620 lb	0.439 (44%)	1.25D+1.5L	L	
Perm Defl in.	0.065 (L/3060)	8' 1/16"	0.556 (L/360)	0.120 (12%)	D	Uniform	
LL Defl inch	0.127 (L/1581)	7'11 15/16"	0.556 (L/360)	0.230 (23%)	L	L	
TL Defl inch	0.192 (L/1042)	8'	0.834 (L/240)	0.230 (23%)	D+L	L	

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARING



Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 7'9" o.c.

5 Bottom flange braced at bearings

ı	o Dottolli lialige	braced at bearings.								
	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
١	1	Tie-In	0-0-0 to 1-9-6	(Span)3-5-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
ı	2	Tie-In	0-0-0 to 17-1-7	(Span)0-7-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
I	3	Part. Uniform	0-2-5 to 16-9-2		Тор	2 PLF	0 PLF	0 PLF	0 PLF	
I	4	Part. Uniform	0-2-5 to 1-9-6		Тор	9 PLF	0 PLF	0 PLF	0 PLF	
I	5	Point	1-8-2		Far Face	240 lb	480 lb	0 lb	0 lb	F6
I	6	Tie-In	1-9-6 to 17-1-7	(Span)0-8-1	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
ı	7	Part. Uniform	1-9-6 to 16-9-1		Тор	2 PLF	0 PLF	0 PLF	0 PLF	
		Tie-In	1-9-6 to 17-1-7	(Span)0-8-1	Тор	15 PSF	40 PSF	0 PSF	0 PSF	F6

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 Upost not to be treated with fire retardant or corrosive

Handling & Installation

- Australing or installation.

 Noist flanges must not be cut or drilled.

 Refer to latest copy of the Joist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-phy fastening details and handling/erection details.

 Damaged Joists must not be used.

 Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

- 5. Provide lateral support at bearing points to avoid lateral displacement and rotation
 6. Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 7. For flat roofs provide proper drainage to prevent appelling.

Manufacturer Info

Nascor by Kott



EWP Studio Simpson Strong-Tie® Component Solutions™ Client: **GREEN YORK HOMES**

Project: Address: Date:

Designer: RCO

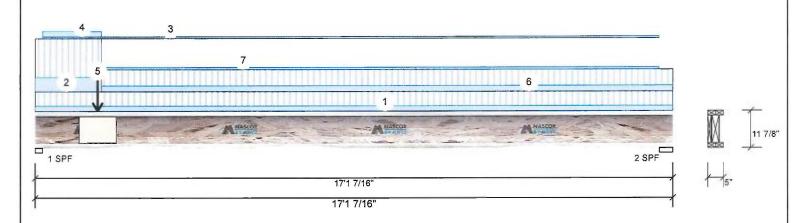
Job Name: INDIGO 1 (ELEV.2)

Project #:

F15-D 11.875" 2-Ply - PASSED NJH

Level: Ground Floor

6/1/2018



Unfactored Reactions UNPATTERNED Ib (Uplift) Member Information Floor (Residential) Brg Live Dead Snow Type: Girder Application: Plies: 2 Design Method: LSD 832 417 0 NBCC 2010 / OBC 2012 Moisture Condition: Dry **Building Code:** 2 496 248 0 Deflection LL: 360 Load Sharing: Deflection TL: Deck: Not Checked 240 Not Checked Importance: Normal Vibration: General Load Floor Live: 40 PSF **Bearings and Factored Reactions** Dead: 15 PSF Cap. React D/L lb Total Ld. Case Bearing Length 1 - SPF 2.375" 53% 521 / 1248 1769 L 2 - SPF 4.319" 29% 310 / 744 1054 L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	4604 ft-lb	7'9 15/16"	10780 ft-lb	0.427 (43%)	1.25D+1.5L	L
Unbraced	4604 ft-lb	7'9 15/16"	4611 ft-lb	0.998 (100%)	1.25D+1.5L	L
Shear	1744 lb	1 5/8"	3620 lb	0.482 (48%)	1.25D+1.5L	L
Perm Defl in.	0.100 (L/2004)	8'3 3/16"	0.556 (L/360)	0.180 (18%)	D	Uniform
LL Defl inch	0.198 (L/1013)	8'3 3/16"	0.556 (L/360)	0.360 (36%)	L	L
TL Defl inch	0.298 (L/673)	8'3 3/16"	0.834 (L/240)	0.360 (36%)	D+L	L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Wind

0

Ld. Comb.

1.25D+1.5L 1.25D+1.5L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 6'2" o.c.

5 Bottom liange	braced at bearings.								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 17-1-7	(Span)1-3-1	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 1-9-6	(Span)3-5-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Part. Uniform	0-2-6 to 16-9-1		Тор	3 PLF	0 PLF	0 PLF	0 PLF	
4	Part. Uniform	0-2-6 to 1-9-6		Тор	9 PLF	0 PLF	0 PLF	0 PLF	
5	Point	1-8-2		Near Face	172 lb	343 lb	0 lb	dl 0	F6
6	Tie-In	1-9-6 to 17-1-7	(Span)1-4-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
7	Part. Uniform	1-9-6 to 16-9-1		Тор	4 PLF	0 PLF	0 PLF	0 PLF	

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 Upoist not to be treated with fire retardant or corrosive

Handling & Installation

- 1.
- Livist flanges must not be cut or drilled Refer to latest copy of the Librist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-hy fastening details and handling/erection details. Damaged bloists must not be used Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roofs provide proper drainage to prevent

Manufacturer Info

Nascor by Kott







Client: Project: Address: **GREEN YORK HOMES**

6/1/2018 Date: Designer:

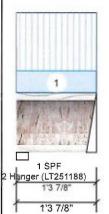
RCO

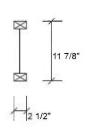
Job Name: INDIGO 1 (ELEV.2)

Project #:

11.875" - PASSED F5-A NJH

Level: Ground Floor





Member Inform	nation		
Туре:	Girder	Application:	Floor (Residential)
Plies:	1	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live	40 PSF		

Brg	Live	Dead	Snow	Wind
1	43	16	0	0
2	41	16	0	0

Analysis Results Location Allowed Analysis Actual Capacity Comb. 19 ft-lb 8 1/8" 5390 ft-lb 0.003 (0%) 1.25D+1.5L L Moment Unbraced 19 ft-lb 8 1/8" 5352 ft-lb 0.003 (0%) 1.25D+1.5L L Shear 68 lb 1 5/8" 1810 lb 0.038 (4%) 1.25D+1.5L L Perm Defl in. 0.000 (L/999) 0 999.000 (L/0) 0.000 (0%) LL Defl inch 0.000 (L/999) 0 999.000 (L/0) 0.000 (0%) TL Defl inch 0.000 (L/999) 0 999.000 (L/0) 0.000 (0%)

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.	
1 - SPF	2.375"	5%	20 / 65	86	L	1.25D+1.5L	
2 - Hanger	2.000"	5%	19 / 62	82	L	1.25D+1.5L	

ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

READ ALL NOTES ON THIS PAGE AND ON

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS



Design Notes

Dead:

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top flange unbraced.
- 4 Bottom flange braced at bearings.

15 PSF

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-3-14	(Span)3-2-8	Top	15 PSF	40 PSF	0 PSF	0 PSF	

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 Usist not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- Locationing or installation.

 Refer to latest copy of the iJoist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-holy fastening details and handling/erection details.

 Damaged Loists must not be used.
 Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

- 5. Provide lateral support at bearing points to avoid
- lateral displacement and rotation

 6. Web stiffeners for point load as shown Minimum point load bearing length>= 3,5 inches

 7. For flat roofs provide proper drainage to prevent

Manufacturer Info

Nascor by Kott





EWP Studio Simpson Strong-Tie® Component Solutions™ Client: GREEN YORK HOMES

Project:

Address:

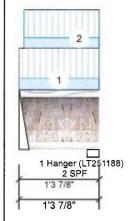
6/1/2018 Date: RCO Designer:

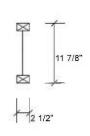
Job Name: INDIGO 1 (ELEV.2)

Project #:

11.875" - PASSED F5-B NJH

Level: Ground Floor





	Member Inform	nation
	Туре:	Girder
	Plies:	1
	Moisture Condition:	Dry
	Deflection LL:	360
i	Deflection TL:	240
1	Importance:	Normal
	General Load	
	Floor Live:	40 PSF

Floor (Residential) Application: Design Method: LSD NBCC 2010 / OBC 2012 **Building Code:** Load Sharing: Deck: Not Checked

Live Dead Snow Wind 74 28 0 0 1 2 83 31 0 0

Unfactored Reactions UNPATTERNED lb (Uplift)

Dead: 15 PSF

Vibration: Not Checked

> Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 35 / 112 147 L 1.25D+1.5L 2.000" 9%

Hanger

2 - SPF 2.375"

39 / 125

164 L

1.25D+1.5L

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	35 ft-lb	7 3/4"	5390 ft-lb	0.007 (1%)	1.25D+1.5L	L
Unbraced	35 ft-lb	7 3/4"	5352 ft-lb	0.007 (1%)	1.25D+1.5L	L
Shear	131 lb	1 1/4"	1810 lb	0.072 (7%)	1.25D+1.5L	L
Perm Defl in	. 0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
LL Defl inch	0.000 (L/40749)	7 3/4"	0.036 (L/360)	0.010 (1%)	L	L
TL Defl inch	0.000 (L/29636)	7 3/4"	0.054 (L/240)	0.010 (1%)	D+L	L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

10%

Bearings and Factored Reactions

REFER TO MULTIPLE MEMBER TO MEMBER **CONNECTION DETAIL FOR PLY TO PLY** NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARING



Design Notes

Analysis Results

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top flange unbraced.
- 4 Bottom flange braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-3-14	(Span)3-2-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-1-0 to 1-3-14	(Span)2-11-3	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design cnteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 Uses not to be treated with fire retardant or corrosive

Handling & Installation

- Anoung & Installation

 Loist flanges must not be cut or drilled

 Refer to latest copy of the IJoist product information
 details for framing details, stiffener tables, web hole
 chart, bridging details, multi-ply festering details and
 handling/erection details

 Design assumes top flange to be laterally restrained
 by attached sheathing or as specified in engineering
 notes.
- 5. Provide lateral support at bearing points to avoid
- lateral displacement and rotation

 8. Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches

 7. For flat roofs provide proper drainage to prevent

Manufacturer Info

Nascor by Kott





EWP Studio Simpson Strong-Tie® Component Solutions™

Client: **GREEN YORK HOMES**

> Project: Address:

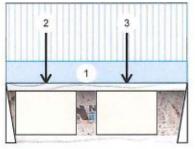
6/1/2018 Date: Designer: RCO

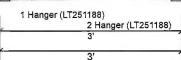
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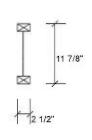
Project #:

11.875" - PASSED NJH F6-A









Member Information

Type:	Girder
Plies:	1
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	
Floor Live:	40 PSF
_	

Application: Floor (Residential) Design Method: LSD **Building Code:** NBCC 2010 / OBC 2012

Deck:

Load Sharing: Not Checked Vibration: Not Checked

422 158 1 2 362 136

Live

Unfactored Reactions UNPATTERNED Ib (Uplift)

Dead

Dead:

15 PSF

Bearings and Factored Reactions

Bea	aring Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.	
1 - Har	2.000" nger	51%	197 / 633	830	L	1.25D+1.5L	
2 -	2.000"	44%	170 / 543	713	L	1.25D+1.5L	

Hanger

Snow

0

0

Wind

0

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	607 ft-lb	1'11 15/16"	5390 ft-lb	0.113 (11%)	1.25D+1.5L	L
Unbraced	607 ft-lb	1'11 15/16"	4800 ft-lb	0.126 (13%)	1.25D+1.5L	L
Shear	823 lb	1 1/4"	1810 lb	0.455 (45%)	1.25D+1.5L	L
Perm Defl in.	0.002 (L/13854)	1'11 1/16"	0.093 (L/360)	0.030 (3%)	D	Uniform
LL Defl inch	0.006 (L/5198)	1'11 1/16"	0.093 (L/360)	0.070 (7%)	L	L
TL Defl inch	0.009 (L/3780)	1'11 1/16"	0.140 (L/240)	0.060 (6%)	D+L	L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS



Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top flange unbraced.
- 4 Bottom flange braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 3-0-0	(Span)1-9-13	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	0-7-15		Far Face	117 lb	313 lb	0 lb	0 lb	J8
3	Point	1-11-15		Far Face	136 lb	362 lb	0 lb	0 lb	J8

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 Usist not to be treated with fire retardant or corrosive

Handling & Installation

- LINGLING & INSTAILATION

 Lolist flanges must not be out or drilled

 Refer to latest copy of the Librist product information details for framing details, sufferer tables, web hole chart, bridging details, multi-hyly fastening details and handling/erection details

 Damaged Librist must not be used

 Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Wish stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Nascor by Kott







EWP Studio Simpson Strong-Tie® Component Solutions™

Client:

Project: Address: GREEN YORK HOMES

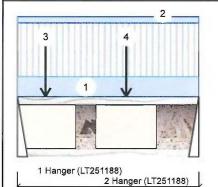
Date: 6/1/2018 RCO Designer:

Job Name: INDIGO 1 (ELEV.2)

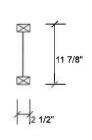
Project #:

11.875" - PASSED F6-B NJH

Level: Ground Floor



3'



Wind 0

Ld. Comb.

1.25D+1.5L

1.25D+1.5L

Member Inform	nation		
Туре:	Girder	Application:	Floor (Residential)
Plies:	1	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF	1	

Unfactor	ed Reactions	UNPATTER	NED lb (Uplift)
Brg	Live	Dead	Snow
1	480	240	0
2	343	172	0

63%

45%

Cap. React D/L lb

300 / 721

215 / 515

4	Analysis Results						
ſ	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
l	Moment	749 ft-lb	1'9 9/16"	5390 ft-lb	0.139 (14%)	1.25D+1.5L	L
l	Unbraced	749 ft-lb	1'9 9/16"	4800 ft-lb	0.156 (16%)	1.25D+1.5L	L
ı	Shear	1013 lb	1 1/4"	1810 lb	0.560 (56%)	1.25D+1.5L	L
l	Perm Defl in.	0.004 (L/9092)	1'9 9/16"	0.093 (L/360)	0.040 (4%)	D	Uniform
	LL Defl inch	0.007 (L/4555)	1'9 9/16"	0.093 (L/360)	0.080 (8%)	L	L
l	TL Defl inch	0.011 (L/3035)	1'9 9/16"	0.140 (L/240)	0.080 (8%)	D+L	L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS

Bearings and Factored Reactions

Bearing Length

Hanger

Hanger

2.000"

2.000"

NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL**



Total Ld. Case

1021 L

729 L

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top flange unbraced.
- 4 Bottom flange braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 3-0-0	(Span)1-8-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Part. Uniform	0-0-0 to 3-0-0		Тор	4 PLF	0 PLF	0 PLF	0 PLF	
3	Point	0-5-9		Near Face	152 lb	304 lb	0 lb	0 lb	J8
4	Point	1-9-9		Near Face	209 lb	416 lb	0 lb	0 lb	J8

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 Upoist not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- LINEATING & INSTALLATION

 Liost flanges must not be cut or drilled
 Refer to latest copy of the Lioist product information
 details for framing details, stiffener tables, web hole
 chart, bridging details, multi-ply fastening details and
 handling/erection details
 Damaged bloists must not be used
 Design assumes top flange to be laterally restrained
 by attached sheathing or as specified in engineering
 notes,

- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stifferers for point load as shown Minimum point load bearing length>= 3,5 inches
 For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Nascor by Kott





EWP Studio Simpson Strong-Tie® Component Solutions™ Client:

Project:

GREEN YORK HOMES

Address:

6/1/2018 Date:

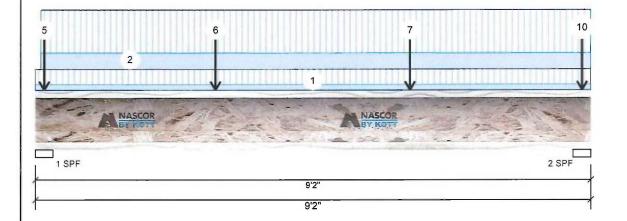
RCO Designer:

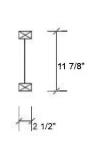
Job Name: INDIGO 1 (ELE)

Project #:

F7-A NJH 11.875" - PASSED

Level: Ground Floor





Wind

0

Member Information

Туре:	Girder
Plies:	1
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	
Floor Live:	40 PSF
Dead:	15 PSF

Floor (Residential) Application: Design Method: LSD NBCC 2010 / OBC 2012 **Building Code:**

Load Sharing: No Deck: Not Checked Vibration: Not Checked

Live

326

328

Brg

2

Unfactored Reactions UNPATTERNED Ib (Uplift)

Dead

394

395

Snow

69

69

Bearings and Factored Reactions

Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.	_
1 - SPF	3.500"	59%	492 / 523	1015	L	1.25D+1.5L +0.5S	
2 - SPF	3.500"	59%	493 / 527	1020	L	1,25D+1.5L +0.5S	

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1148 ft-lb	4'7"	5174 ft-lb	0.222 (22%)	1.25D+1.5L	L
Unbraced	1148 ft-lb	4'7"	1155 ft-lb	0.994 (99%)	1.25D+1.5L	L
Shear	489 lb	8'11 1/4"	1738 lb	0.281 (28%)	1.25D+1.5L	L
Perm Defl in.	0.014 (L/7693)	4'7"	0.290 (L/360)	0.050 (5%)	D	Uniform
LL Defl inch	0.036 (L/2894)	4'7"	0.290 (L/360)	0.120 (12%)	L+0.5S	L
TL Defl inch	0.050 (L/2103)	4'7"	0.435 (L/240)	0.110 (11%)	D+L+0.5S	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Top flange must be laterally braced at a maximum of 8'8" o.c.

3 Bottom flange braced at bearings.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS



ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 9-2-0	(Span)0-6-5	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-1-2 to 9-2-0	(Span)1-5-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	0-1-12		Тор	18 lb	0 lb	0 lb	0 lb	Wall Self Weight
4	Point	0-1-12		Тор	261 lb	71 lb	69 lb	0 lb	F3 F3
5	Point	0-1-12		Тор	19 lb	0 lb	0 lb	0 lb	Wall Self Weight
6	Point	2-11-12		Near Face	28 lb	74 lb	0 lb	0 lb	F5
7	Point	6-2-4		Near Face	28 lb	74 lb	0 lb	0 lb	F5
8	Point	9-0-4		Тор	261 lb	71 lb	69 lb	0 lb	F3 F3
9	Point	9-0-4		Тор	19 lb	0 lb	0 lb	0 lb	Wall Self Weight
10	Point	9-0-4		Тор	18 lb	0 lb	0 lb	0 lb	Wall Self Weight

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 Upoist not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- Libits Itanges must not be cut or drilled Refer to latest copy of the Libist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-ply fastening details and handling/erection details Damaged bloists must not be used Design assumes top flenge to be laterally restrained by attached sheathing or as specified in engineering notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing lengths—3.5 inches
 For flat roofs provide proper drainage to prevent

Manufacturer Info

Nascor by Kott







Client: GREEN YORK HOMES

Project: Address:

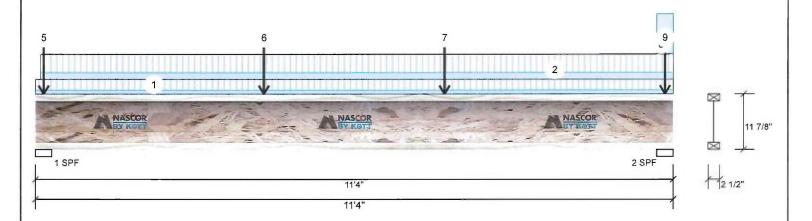
6/1/2018 Date: **RCO** Designer:

INDIGO 1 (ELEV.2) Job Name:

Project #:

11.875" - PASSED F8-A NJH

Level: Ground Floor



Member Information Girder Floor (Residential) Type: Application: Plies 1 Design Method: LSD NBCC 2010 / OBC 2012 Moisture Condition: Dry Building Code: Deflection LL: 360 Load Sharing: No Deflection TL: 240 Deck: Not Checked Importance: Normal Vibration: Not Checked General Load Floor Live: 40 PSF Dead: 15 PSF

Unfactored Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	Wind
1	405	660	136	0
2	407	644	136	0

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1671 ft-lb	5'8"	4851 ft-lb	0.344 (34%)	1.25D+1.5L	L
Unbraced	1671 ft-lb	5'8"	1680 ft-lb	0.994 (99%)	1.25D+1.5L	L
Shear	586 lb	11'1 1/4"	1629 lb	0.360 (36%)	1.25D+1.5L	L
Perm Defl in.	0.028 (L/4732)	5'8"	0.362 (L/360)	0.080 (8%)	D	Uniform
LL Defl inch	0.073 (L/1788)	5'8"	0.362 (L/360)	0.200 (20%)	L+0.5S	L
TL Defl inch	0.101 (L/1298)	5'8"	0.544 (L/240)	0.180 (18%)	D+L+0.5S	L

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.	
1 - SPF	3.500"	92%	825 / 675	1500	L	1.25D+1.5L +0.5S	
2 - SPF	3.500"	91%	805 / 679	1484	L	1.25D+1.5L +0.5S	

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Top flange must be laterally braced at a maximum of 7'2" o.c.
- 3 Bottom flange braced at bearings.

3 Dottoill	nange braced at bearings	· .							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 11-4-0	(Span)0-10-5	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-1-2 to 11-4-0	(Span)1-5-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	0-1-12		Тор	21 lb	0 lb	0 lb	0 lb	Wall Self Weight
4	Point	0-1-12		Тор	512 lb	102 lb	136 lb	0 lb	F4 F4
5	Point	0-1-12		Тор	13 lb	0 lb	0 lb	0 lb	Wall Self Weight
6	Point	4-0-12		Near Face	16 lb	41 lb	0 lb	0 lb	F5
7	Point	7-3-4		Near Face	16 lb	41 lb	0 lb	0 lb	F5
8	Part. Uniform	11-0-8 to 11-4-0		Тор	64 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
9	Point	11-2-4		Тор	510 lb	102 lb	136 lb	0 lb	F4 F4

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

- Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive
- chemicals

Handling & Installation

- andling & Installation

 Loist Hanges must not be out or drilled

 Refer to latest copy of the Liuist product information
 details for framing details, stiffener tables, web hole
 chart, bridging details, multi-ply fastening details and
 handling/erection details

 Design assumes top flange to be laterally restrained
 by attached sheathing or as specified in engineering
 notes.
- 5. Provide lateral support at bearing points to avoid
- lateral displacement and rotation

 8. Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches

 7. For flat roofs provide proper drainage to prevent ponding

Nascor by Kott

Manufacturer Info





EWP Studio Simpson Strong-Tie® Component Solutions™ Client:

Project: Address: **GREEN YORK HOMES**

Date: 6/1/2018

Designer: RCO

Job Name: INDIGO 1 (ELEV.1)

Project #:

F10-A Forex 2.0E-3000Fb LVL 1.750" X 11.875" - PASSED

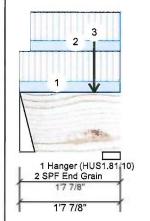
Brg

1

2

Grain

Level: Second Floor



Wind

0

0

Member Information

Type:	Girder	Application:	Floor (Residential)
Plies:	1	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

Unfactored Reactions UNPATTERNED Ib (Uplift)

Live

121

182

Dead

49

73

Snow

0

0

ı								
	Bearings	s and Fac	tored I	Reactions	4.00			
ľ	Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.	
	1 - Hanger	3.000"	6%	62 / 182	244	L	1.25D+1.5L	
	2 - SPF End	3.500"	8%	91 / 273	363	L	1.25D+1.5L	

Analysis Results

	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case	
ı	Moment	72 ft-lb	10 7/16"	17130 ft-lb	0.004 (0%)	1.25D+1.5L	L	
	Unbraced	72 ft-lb	10 7/16"	16556 ft-lb	0.004 (0%)	1.25D+1.5L	L	
	Shear	95 lb	1'2 1/8"	5798 lb	0.016 (2%)	1.25D+1.5L	L	
	Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)			
	LL Defl inch	0.000 (L/67338)	10 3/8"	0.041 (L/360)	0.010 (1%)	L	L	
	TL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)			

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.
- 4 Bottom braced at bearings.

ı	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
ı	1	Tie-In	0-0-0 to 1-7-14	(Span)3-11-9	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
ı	2	Tie-In	0-2-2 to 1-7-14	(Span)3-9-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
ı	3	Point	1-2-12		Far Face	23 lb	60 lb	0 lb	0 lb	J1
ı		Self Weight				5 PLF				

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criterie and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- LVL beams must not be cut or drilled Refer to manufacturer's product information regardring installation requirements, multi-ply fastening details, beam strength values, and code approvals Damaged Beams must not be used 1.
- Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation

Manufacturer Info For flat roofs provide proper drainage to prevent ponding

Forex





EWP Studio

Simpson Strong-Tie® Component Solutions™ Client: Project: Address: GREEN YORK HOMES

Date: 6/1/2018 RCO

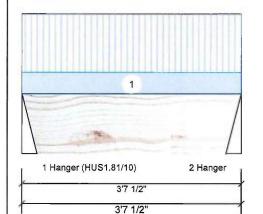
Designer:

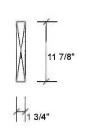
Job Name: INDIGO 1 (ELEV.1)

Project #:

F11-A Forex 2.0E-3000Fb LVL 1.750" X 11.875" - PASSED

Level: Second Floor





Ld. Comb.

1.25D+1.5L

1.25D+1.5L

Member Information					
Туре:	Girder				
Plies:	1				
Moisture Condition:	Dry				
Deflection LL:	360				
Deflection TL:	240				
Importance:	Normal				
General Load					
Floor Live:	40 PSF				

15 PSF

Application: Floor (Residential) Design Method: LSD NBCC 2010 / OBC 2012 **Building Code:** Load Sharing: No Deck: Not Checked Vibration: Not Checked

Unfactored Reactions UNPATTERNED Ib (Uplift) Dead Snow Brg Live Wind 489 192 0 0 489 192 0 0 2

Cap. React D/L lb

240 / 734

Analysis Results Location Allowed Analysis Actual Capacity Comb. Case 709 ft-lb Moment 1'9 3/4" 17130 ft-lb 0.041 (4%) 1.25D+1.5L L Unbraced 709 ft-lb 1'9 3/4" 13201 ft-lb 0.054 (5%) 1.25D+1.5L L Shear 1'2 1/8" 5798 lb 0.059 (6%) 1.25D+1.5L L Perm Defl in. 0.001 1'9 3/4" 0.108 (L/360) 0.010 (1%) D Uniform (L/29614) LL Defl inch 0.003 1'9 3/4" 0.108 (L/360) 0.030 (3%) L L (L/11600)

1'9 3/4" 0.162 (L/240) 0.030 (3%) D+L

3.000" 25% 240 / 734 2 -Hanger READ ALL NOTES ON THIS PAGE AND ON **ENGINEERING NOTE PAGE ENP-2. THIS** NOTE PAGE IS AN INTEGRAL PART OF THIS

25%

Bearings and Factored Reactions

Bearing Length

Hanger

3.000"

CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Total Ld. Case

973 L

973 L

Design Notes

Dead:

1 Fill all hanger nailing holes.

TL Defl inch 0.005 (L/8335)

- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.
- 4 Bottom braced at bearings.

	2,4004 4, 204, 11, 34.								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Part. Uniform	0-0-0 to 3-7-8		Тор	101 PLF	270 PLF	0 PLF	0 PLF	
	Self Weight				5 PLF				

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals 1,
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info Forex APA: PR-L318



Client:

Project: Address: GREEN YORK HOMES

6/1/2018 Date:

Designer: RCO

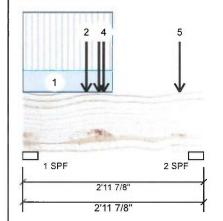
Job Name: INDIGO 1 (ELEV.1)

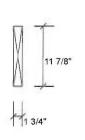
Project #:

Forex 2.0E-3000Fb LVL F11-B

1.750" X 11.875" - PASSED

Level: Second Floor





Member Information

Туре:	Girder				
Plies:	1				
Moisture Condition:	Dry				
Deflection LL:	360				
Deflection TL:	240				
Importance:	Normal				
General Load					
Floor Live:	40 PSF				
Dead:	15 PSF				

Application: Floor (Residential) LSD Design Method: NBCC 2010 / OBC 2012 **Building Code:** Load Sharing: No

Not Checked Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

3rg	Live	Dead	Snow	Wind
1	468	188	0	0
2	381	154	0	0

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.	
1 - SPF	3.000"	29%	235 / 701	936	L	1.25D+1.5L	
2 - SPE	3 000"	24%	193 / 571	764	1	1 25D+1 5I	

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	889 ft-lb	1'4"	17130 ft-lb	0.052 (5%)	1.25D+1.5L	L
Unbraced	889 ft-lb	1'4"	14588 ft-lb	0.061 (6%)	1.25D+1.5L	L
Shear	757 lb	1'9 3/4"	5798 lb	0.131 (13%)	1.25D+1.5L	L
Perm Defl in.	0.001 (L/23259)	1'4"	0.087 (L/360)	0.020 (2%)	D	Uniform
LL Defl inch	0.003 (L/9252)	1'4"	0.087 (L/360)	0.040 (4%)	L	L
TL Defl inch	0.005 (L/6619)	1'4"	0.131 (L/240)	0.040 (4%)	D+L	L

Deck:

Vibration:

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Top braced at bearings.
- 3 Bottom braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-5-12	(Span)3-9-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	1-0-9		Near Face	23 lb	60 lb	0 lb	0 lb	J1
3	Point	1-3-1		Far Face	37 lb	98 lb	0 lb	0 lb	J1
4	Point	1-4-0		Near Face	192 lb	489 lb	0 lb	0 lb	F11
5	Point	2-7-1		Far Face	34 lb	90 lb	0 lb	0 lb	J1
	Self Weight				5 PLF				

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

andling & Installation
LVL beams must not be cut or drilled
Refer to manufacturer's product information
regarding installation requirements, multi-ply
fastening details, beam strength values, and code
approvals
Damaged Beams must not be used
Design assumes top adge is laterally restrained
Provide lateral support at beaming points to avoid
lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex







EWP Studio Simpson Strong-Tie® Component Solutions™

GREEN YORK HOMES Client:

Project: Address:

RCO Designer:

Job Name: INDIGO 1 (ELEV.1)

6/1/2018

Project #:

Date:

Forex 2.0E-3000Fb LVL F12-A

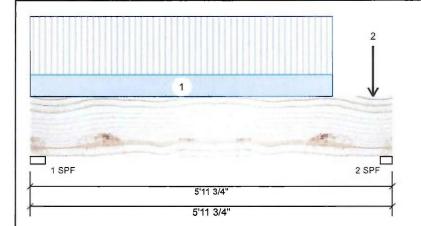
1.750" X 11.875" - PASSED

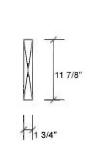
Brg

2

2 - SPF 2.375"

Level: Second Floor





Wind

0

1.25D+1.5L

Member Information

Type:	Girder	Application:	Floor (Residential)
Plies:	1	Design Method:	LSD
Moisture Condition	: Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

Unfactored Reactions UNPATTERNED Ib (Uplift)

Dead

95

Live

214

202

Bearings	and Fac	ctored F	Reactions				
Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.	
1-SPF	3.000"	14%	118 / 322	440	L	1.25D+1.5L	

112 / 303

Snow

0

0

415 L

Analysis Results

	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
ı	Moment	575 ft-lb	2'11 3/4"	17130 ft-lb	0.034 (3%)	1.25D+1.5L	L
l	Unbraced	575 ft-lb	2'11 3/4"	7949 ft-lb	0.072 (7%)	1.25D+1.5L	L
l	Shear	529 lb	4'10 1/4"	5798 lb	0.091 (9%)	1.25D+1.5L	L
	Perm Defl in.	0.002 (L/31643)	3'	0.189 (L/360)	0.010 (1%)	D	Uniform
	LL Defl inch	0.005 (L/14020)	2'11 15/16"	0.189 (L/360)	0.030 (3%)	L	L
	TL Defl inch	0.007 (L/9715)	2'11 15/16"	0.283 (L/240)	0.020 (2%)	D+L	L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT

16%

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARING



Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Top braced at bearings.
- 3 Bottom braced at bearings.

	Dottom Brades at Bearinge.									
Г	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
	1	Part. Uniform	0-0-0 to 4-11-15		Far Face	27 PLF	72 PLF	0 PLF	0 PLF	
	2	Point	5-7-15		Far Face	21 lb	57 lb	0 lb	0 lb	J1
		Self Weight				5 PLF				

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

andling & Installation
LVL beams must not be cut or drilled
Refer to manufacturer's product information
regarding installation requirements, multi-ply
fastening details, beam strength values, and code
approvals
Damaged Beams must not be used
Design assumes top edge is laterally restrained
Provide lateral support at beaming points to avoid
lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info Forex





EWP Studio Simpson Strong-Tie® Component Solutions™ Client: Address: **GREEN YORK HOMES**

Project:

Date: 6/1/2018

Designer: RCO

Job Name: INDIGO 1 (ELEV.1)

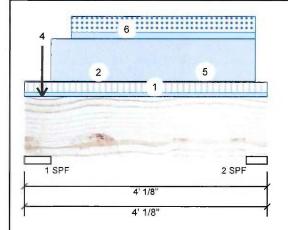
Project #:

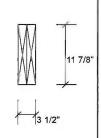
F13-A Forex 2.0E-3000Fb LVL 1.750" X 11.875"

2-Ply - PASSED

2

Level: Second Floor





Wind

0

0

31

42

ı	Member Inform	nation		
ı	Type:	Girder	Application:	Floor (Residential)
ı	Plies:	2	Design Method:	LSD
ı	Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
ı	Deflection LL:	360	Load Sharing:	No
ı	Deflection TL:	240	Deck:	Not Checked
ı	Importance:	Normal	Vibration:	Not Checked
l	General Load			
ĺ	Floor Live:	40 PSF		
ı	Dead:	15 PSF		

Unfactored Reactions UNPATTERNED Ib (Uplift) Brg Live Dead Snow

158

162

33

32

Bearings and Factored Reactions											
Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.					
1 - SPF	5.250"	3%	198 / 50	248	L	1.25D+1.5L					
2 - SPF	4.125"	4%	203 / 63	265	L	1.25D+1.5S					

Analysis Results

L								
I	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case	
l	Moment	208 ft-lb	2' 3/4"	23297 ft-lb	0.009 (1%)	1.25D+1.5S	L	
l	Unbraced	208 ft-lb	2' 3/4"	23297 ft-lb	0.009 (1%)	1.25D+1.5S	L	
l	Shear	104 lb	1'4 3/8"	7885 lb	0.013 (1%)	1.25D+1.5S	L	
ĺ	Perm Defl in.	0.001 (L/65472)	2' 11/16"	0.112 (L/360)	0.010 (1%)	D	Uniform	
l	LL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)			
ĺ	TL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)			

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARING



Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Girders are designed to be supported on the bottom edge only.

- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on full section width

3 Multiple plies must be fastened together as per manufacturer's details. 4 Top loads must be supported equally by all plies. 5 Top braced at bearings.

/ Lateral s	siendenness rado pased c	in full section width.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 4-0-2	(Span)0-9-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Part, Uniform	0-0-0 to 3-8-2		Тор	1 PLF	0 PLF	0 PLF	0 PLF	
4	Point	0-3-8		Тор	8 lb	0 lb	0 lb	0 lb	Wall Self Weight
5	Part. Uniform	0-5-4 to 3-9-12		Тор	64 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
6	Part. Uniform	0-9-4 to 3-9-12		Тор	10 PLF	0 PLF	24 PLF	0 PLF	
	Self Weight				10 PLF				

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Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- andling & Installation
 LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multip-lip
 fastening details, beam strength values, and code
 approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

- For flat roofs provide proper drainage to prevent ponding

Manufacturer Info





EWP Studio Simpson Strong-Tie® Component Solutions™ Client: **GREEN YORK HOMES**

Project: Address:

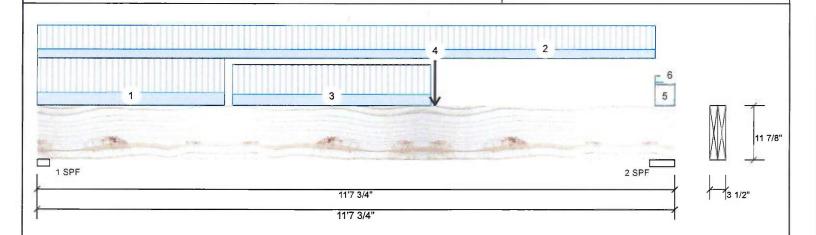
6/1/2018 Designer:

RCO Job Name: INDIGO 1 (ELEV.1)

Project #:

F14-A Forex 2.0E-3000Fb LVL 1.750" X 11.875"

2-Ply - PASSED Level: Second Floor



Member Inform	er Information				Unfactored Reactions UNPATTERNED lb (Uplift)							
Туре:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snov	N	Wind		
Plies:	2	Design Method:	LSD	1	313		173		0	0		
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012	2	268		159		0	0		
Deflection LL:	360	Load Sharing:	No									
Deflection TL:	240	Deck:	Not Checked									
Importance:	Normal	Vibration:	Not Checked									
General Load												
Floor Live:	40 PSF			Bearings	and Fac	tored i	Reactions					
Dead:	15 PSF			Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.		
				1 - SPF	2.750"	12%	216 / 470	686	L	1.25D+1.5L		
				2-SPF	5.500"	5%	199 / 402	601	L	1.25D+1.5L		

Analysis Results

Γ	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
l	Moment	1948 ft-lb	6'1 1/2"	34261 ft-lb	0.057 (6%)	1.25D+1.5L	L,
l	Unbraced	1948 ft-lb	6'1 1/2"	28546 ft-lb	0.068 (7%)	1.25D+1.5L	L
l	Shear	551 lb	1'1 7/8"	11596 lb	0.048 (5%)	1.25D+1.5L	L
	Perm Defl in.	0.012 (L/10848)	5'9 1/8"	0.369 (L/360)	0.030 (3%)	D	Uniform
l	LL Defl inch	0.022 (L/5964)	5'9 1/4"	0.369 (L/360)	0.060 (6%)	L	L
l	TL Defl inch	0.035 (L/3848)	5'9 1/4"	0.554 (L/240)	0.060 (6%)	D+L	L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARING



Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on full section width.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 3-5-2	(Span)1-7-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 11-3-8	(Span)1-1-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Tie-In	3-6-14 to 7-2-6	(Span)1-4-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
4	Point	7-3-4		Far Face	49 lb	121 lb	0 lb	0 lb	F10
5	Tie-In	11-3-6 to 11-7-12	(Span)0-8-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
6	Tie-In	11-3-8 to 11-7-12	(Span)0-3-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	Self Weight				10 PLF				

Notes

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Handling & Installation

- andling & Installation
 LVI beams must not be cut or drilled
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 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding





