

ENGINEERING NOTE PAGE (ENP-1)
PLEASE READ PRIOR TO INSTALLATION

NE0618-003
GREEN YORK HOMES-LIANA 1 EL 1

RESPONSIBILITIES

THIS DESIGN IS FOR AN INDIVIDUAL BUILDING COMPONENT AND HAS BEEN BASED ON INFORMATION PROVIDED BY THE DESIGN OFFICE OF KOTT LUMBER. THE UNDERSIGNED ENGINEER DISCLAIMS ANY RESPONSIBILITY FOR DAMAGES AS A RESULT OF FAULTY OR INCORRECT INFORMATION, SPECIFICATION AND/OR DESIGNS FURNISHED TO THE ENGINEER. THE UNDERSIGNED ENGINEER IS ONLY RESPONSIBLE FOR THE STRUCTURAL INTEGRITY OF THIS BUILDING COMPONENT FOR THE CONDITIONS AND LOADS SHOWN ON THIS DRAWING. THE STRUCTURAL INTEGRITY OF THE BUILDING AND THE VERIFICATION OF THE DIMENSIONS AND THE DESIGN LOADS USED ARE THE RESPONSIBILITY OF THE BUILDING DESIGNER.

TRUSSES ARE DESIGNED IN CONFORMANCE WITH THE RELEVANT SECTIONS OF THE NATIONAL BUILDING CODE OF CANADA OR THE CANADIAN CODE FOR FARM BUILDINGS, WHICHEVER APPLIES TO THE BUILDING TYPE INDICATED ON THE DRAWING

IT IS THE RESPONSIBILITY OF KOTT LUMBER TO ENSURE THAT TRUSSES ARE MANUFACTURED IN CONFORMANCE WITH THESE DESIGNS AND WITH THE SPECIFICATIONS OUTLINED BELOW. THE UNDERSIGNED ENGINEER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

USE AND OCCUPANCY

- The building is of the type indicated on the drawing

LOADING

- The truss loading intensity and distribution as well as load transfer mechanism is that indicated on the drawing
- No buildings, trees, parapets or other projections higher than the roof for which the trusses are used are located within a distance less than ten (10) times the difference in height, or five metres (16 ft) whichever is greater, unless the drawing indicates that the snow drifting has been taken into account

HANDLING, INSTALLATION AND BRACING

- The trusses must be handled and installed by a qualified professional as per the supplied document titled *Information for Truss Installers* and the BCSI-B1 and BCSI-B3 Summary Sheets
- The compression chords are laterally braced by continuous rigid diaphragm sheathing or as specified on the drawing
- Temporary and permanent bracing must be installed as indicated on the truss drawing and according to the BCSI-B1 and BCSI-B3 Summary Sheets. Bracing for the lateral stability of the truss is to be provided by the building designer
- **It is recommended that a Professional Engineer's advice be obtained for the bracing of trusses spanning more than 12.37m (40'-7")**

SUPPORTS

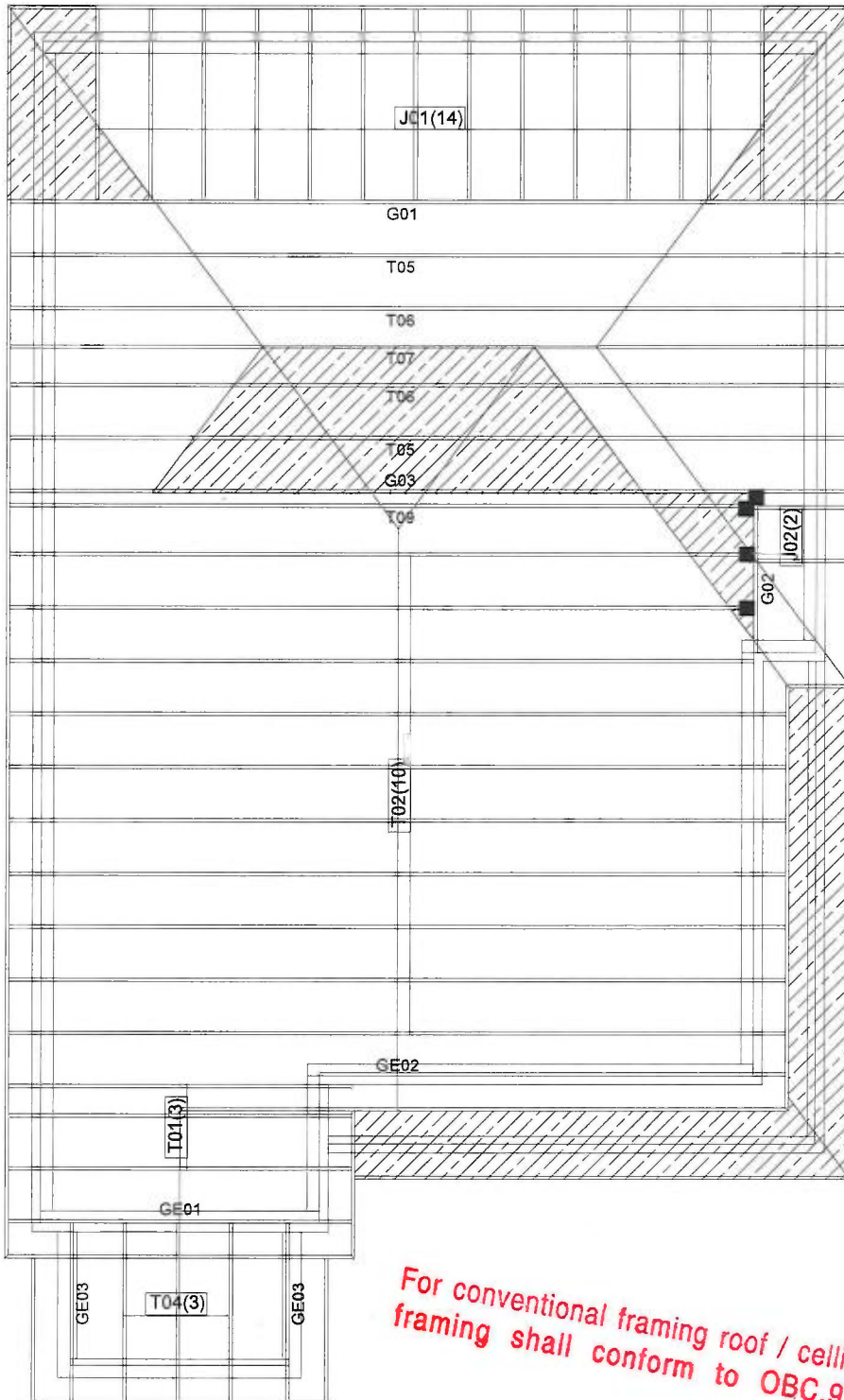
- The trusses are to be supported at the bearing points indicated and anchored to the supports where considered necessary by the designer of the overall structure
- Bearing sizes shown are the minimum required to prevent crushing of the truss members and do not necessarily take into account stability of the overall building structure
- Elevation of bearings must be carefully checked and shimmed to alignment for solid bearings
- Adequate wood truss bearing is the responsibility of the building designer.

DIMENSIONS

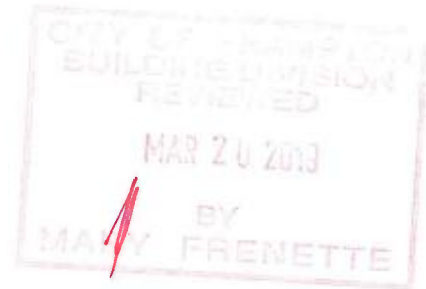
- Geometry of the truss and dimensions indicated on the drawing are identical to those of the installed truss.



THIS CERTIFICATION IS FOR THE ENGINEERING REVIEW OF STRUCTURAL COMPONENTS SHOWN ON THIS DRAWING AND IS LIMITED TO THE COMPONENTS SUPPLIED BY KOTT LUMBER FOR THE LOADS AND CONDITIONS SHOWN ON THE CALCULATION PAGE OF EACH COMPONENT. THE PLACEMENT OF THESE COMPONENTS ON THIS LAYOUT IS SHOWN FOR REFERENCE ONLY. ROOF TRUSSES MUST BE INSTALLED ACCORDING TO MANUFACTURER'S GUIDELINES AT 24" C/C UNLESS NOTED OTHERWISE. CONVENTIONAL FRAMING SHOWN ON THE LAYOUT MUST BE DESIGNED AND APPROVED BY THE PROJECT ENGINEER. TEMPORARY AND PERMANENT BRACINGS OF THE ROOF AND BUILDING ARE THE RESPONSIBILITY OF THE PROJECT ENGINEER AND TO CONFORM TO BCSI GUIDELINES.



ENG.JOB:NE0618-003



The components shown on this layout conform to our recommendations and are adequate to support the design loads required by the Ontario Building Code O.Reg 332/12 as amended and as shown on the engineering calculation page of each component.

The framing direction and girder locations shown on this layout conform to that shown on the architectural drawing.

Deflection: LL = L/360
1.33LL+DL=L/360

Architectural Drawing Info:
Date: MAY 22, 2018
Project number: 17-55
Model: LIANA 1

All work shall conform to the Ontario Building Code O. Reg. 332/12 as amended

19-447187. RA



CONVENTIONAL FRAMING BY OTHERS

ALL CONVENTIONAL FRAMING TO CONFORM WITH PART 9 OF THE O.B.C. ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4 S.P.F. @ 24" O/C WITH A 2x4 VERTICAL POST TO THE TRUSS UNDERNEATH EACH CROSS POINT. VERTICAL POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END POINT AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

HANGER LEGEND:

▼ LUS24 ■ LJS26DS
● HGUS26 ✕ HGUS26-2

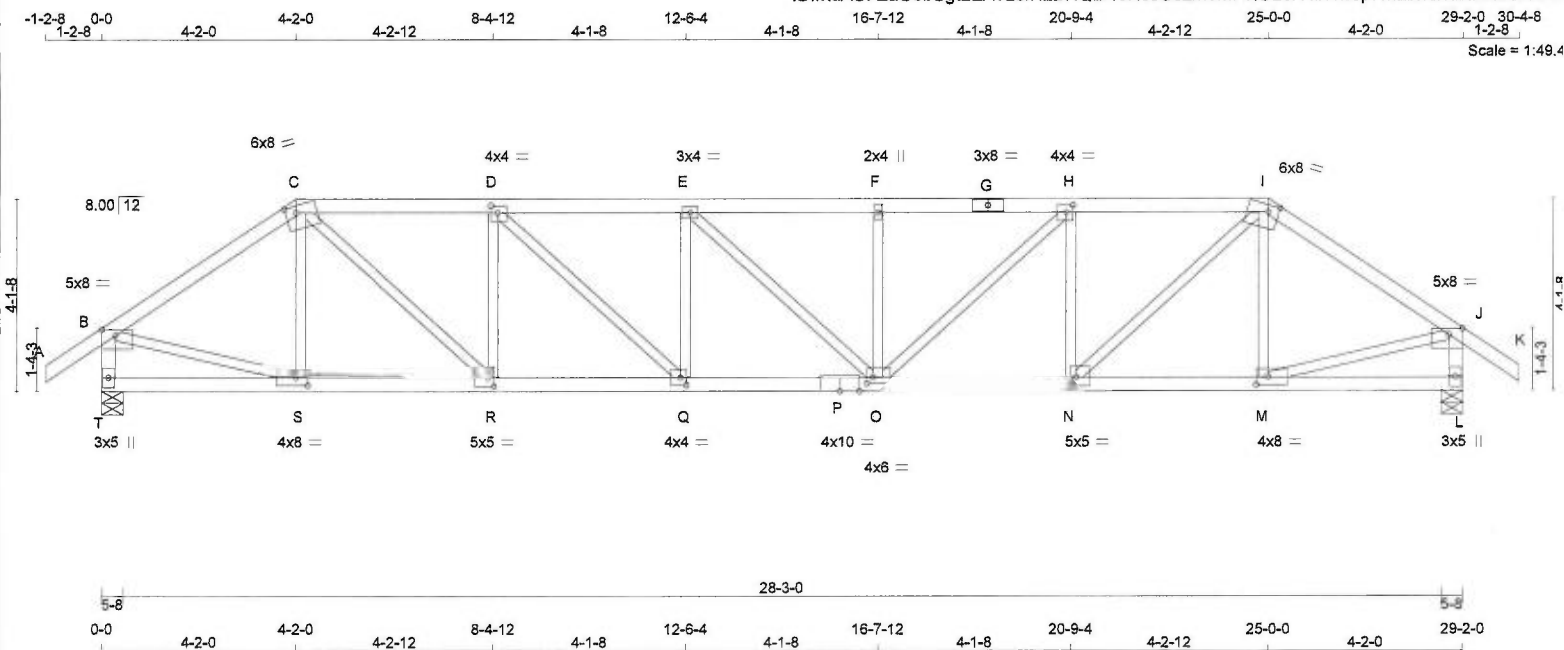
SIZE AND LOCATION OF CONVENTIONAL FRAMING IS APPROXIMATE. ALL AREAS MAY NOT BE SHOWN. REFER TO ARCHITECTURAL PLANS FOR DETAILS.

DESIGN CRITERIA
SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3 PSF
BOT CH. LL = 0 PSF
DL = 7 PSF
TOTAL LOAD = 33.3 PSF
SPACING = 24" O/C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TMC 2011

Model: LIANA 1 EL 1 3&4 BED
Customer: GREEN YORK HOMES
Project: GRANELLI HOMES
Location: BRAMPTON
Date: 5/30/2018 Drawn by: BB



Scale = 1:49.4

TOTAL WEIGHT = 120 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - G	2x4	DRY	No.2	SPF	
G - I	2x4	DRY	No.2	SPF	
I - K	2x4	DRY	No.2	SPF	
T - B	2x4	DRY	No.2	SPF	
L - J	2x4	DRY	No.2	SPF	
T - P	2x4	DRY	No.2	SPF	
P - L	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.50	3.50
C	TTWW-m	MT20	6.0	8.0	1.75	2.75
D	TMVW-t	MT20	4.0	4.0	1.75	1.75
E	TMVW-t	MT20	3.0	4.0		
F	TMVW-w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	8.0		
H	TMVW-t	MT20	4.0	4.0	1.75	1.75
I	TTWW-m	MT20	6.0	8.0	1.75	2.75
J	TMVW-p	MT20	5.0	8.0	1.50	3.50
L	BMV1-p	MT20	3.0	5.0		
M	BMVW-t	MT20	4.0	8.0	2.00	3.00
N	BMVW-t	MT20	5.0	5.0	2.25	1.50
O	BMVW-w	MT20	4.0	6.0	1.50	1.50
P	BS-t	MT20	4.0	10.0		
Q	BMVW-t	MT20	4.0	4.0	2.00	1.50
R	BMVW-t	MT20	5.0	5.0	2.25	1.50
S	BMVW-t	MT20	4.0	8.0	2.00	3.00
T	BMV1-p	MT20	3.0	5.0		

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS								
	FACTORED		MAXIMUM FACTORED			INPUT	REQRD	
	GROSS REACTION		GROSS REACTION			BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
T	2664	0	2664	0	0	5-8	5-8	
L	2664	0	2664	0	0	5-8	5-8	

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN	COMPONENT REACTIONS	WIND	DEAD	SOIL
T	1869	1314 / 0	0 / 0	0 / 0	554 / 0	0 / 0
L	1869	1314 / 0	0 / 0	0 / 0	554 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		LENGTH FR-TO			
A-B	0 / 28	-77.4 -77.4	0.10 (1)	10.00	S-C	-461 / 0	0.13 (1)
B-C	-3100 / 0	-77.4 -77.4	0.45 (1)	3.55	C-R	0 / 2263	0.56 (1)
C-D	-4242 / 0	-145.9 -145.9	0.71 (1)	2.72	R-D	-1390 / 0	0.38 (1)
D-E	-5018 / 0	-145.9 -145.9	0.83 (1)	2.33	D-Q	0 / 1060	0.25 (1)
E-F	-5018 / 0	-145.9 -145.9	0.71 (1)	2.53	Q-E	-584 / 0	0.16 (1)
F-G	-5018 / 0	-145.9 -145.9	0.83 (1)	2.33	E-O	-4 / 0	0.00 (1)
G-H	-5018 / 0	-145.9 -145.9	0.83 (1)	2.33	O-F	-582 / 0	0.16 (1)
H-I	-4243 / 0	-145.9 -145.9	0.71 (1)	2.72	O-H	0 / 1055	0.26 (1)
I-J	-3100 / 0	-77.4 -77.4	0.45 (1)	3.55	N-H	-1388 / 0	0.38 (1)
J-K	0 / 28	-77.4 -77.4	0.10 (1)	10.00	N-I	0 / 2265	0.56 (1)
T-B	-2607 / 0	0.0 0.0	0.29 (1)	5.26	M-I	-461 / 0	0.13 (1)
L-J	-2607 / 0	0.0 0.0	0.29 (1)	5.26	B-S	0 / 2649	0.66 (1)
					M-J	0 / 2649	0.66 (1)
T-S	0 / 0	-33.0 -33.0	0.15 (4)	10.00			
S-R	0 / 2563	-33.0 -33.0	0.54 (1)	10.00			
R-Q	0 / 4242	-33.0 -33.0	0.82 (1)	10.00			
Q-P	0 / 5018	-33.0 -33.0	0.96 (1)	10.00			
P-O	0 / 5018	-33.0 -33.0	0.96 (1)	10.00			
O-N	0 / 4243	-33.0 -33.0	0.83 (1)	10.00			
N-M	0 / 2563	-33.0 -33.0	0.53 (1)	10.00			
M-L	0 / 0	-33.0 -33.0	0.15 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	4-2-0	-242	-242	---	FRONT	VERT	TOTAL
I	25-0-0	-242	-242	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 4-2-0
END SETBACK = 6-0-0
END WALL WIDTH = 5-8
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADD'L LOADS BASED ON 55 % OF GSL

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.26")
ALLOWABLE DEFL.(TL) = L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 751 (0.47")

CSI: TC=0.83/1.00 (F-H:1), BC=0.96/1.00 (O-Q:1), WB=0.66/1.00 (B-S:1), SSI=0.31/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

CONTINUED ON PAGE 2



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN YORK HOMES-LIANA 1 EL 1	DRWG NO.
NE0618-003	G01	1	1	TRUSS DESC.		PAGE 3 OF 21

Version 8.210 S Mar 12 2018 MiTek Industries, Inc. Sat Jun 2 11:50:30 2018 Page 2
ID: rxdAS7EaO6bSgtLEAvLofNzBNQD-f5Rs9Cszm3MF3IO2cR4bNwsp7mLix92T22Mz5OzALC

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 241.8 lbs FACTORED DOWN AT 25-0-0,
AND 241.8 lbs FACTORED DOWN AT 4-2-0 ON
TOP CHORD. DESIGN FOR UNSPECIFIED
CONNECTION(S) IS DELEGATED TO THE
BUILDING DESIGNER.

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

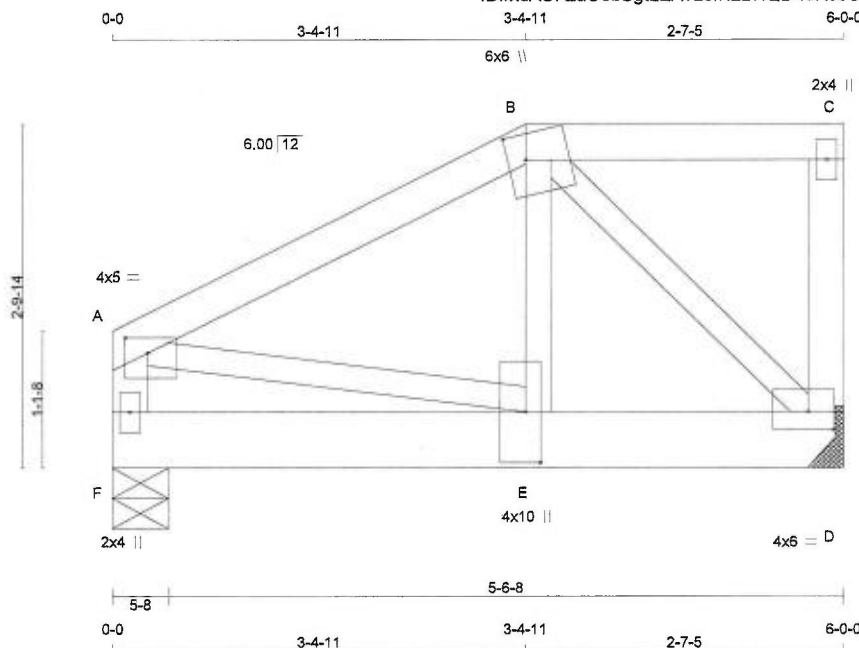
JSI GRIP= 0.90 (N) (INPUT = 0.90)

JSI METAL= 0.95 (P) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.



ID: rxdAS7EaO6bSgtLEAvLofNzBNQD-f5Rs9Cszm3MF3IO2cR4bNwsytms2xCdT22Mz5OzALC



Scale = 1:18.5

TOTAL WEIGHT = 28 lb [M]

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - B	2x4	DRY	No.2
B - C	2x4	DRY	No.2
D - C	2x4	DRY	No.2
F - A	2x4	DRY	No.2
F - D	2x6	DRY	No.2

ALL WEBS 2x3 DRY No.2 EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	5.0	1.50	2.25
B	TTWW+m	MT20	6.0	6.0	2.50	1.75
C	TMV+p	MT20	2.0	4.0		
D	BMVW1-t	MT20	4.0	6.0	1.75	2.50
E	BMVW+t	MT20	4.0	10.0	5.00	1.50
F	BMV1+p	MT20	2.0	4.0		

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 31.0 lbs FACTORED DOWN AT 3'-4-11 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	UPLIFT	IN-SX
D	2023	0	0	MECHANICAL
F	2026	0	0	5-8

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT D TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. LIVE	PERM. LIVE	WIND	DEAD	SOIL
D	1419	998 / 0	0 / 0	0 / 0	0 / 0	424 / 0	0 / 0
F	1421	998 / 0	0 / 0	0 / 0	0 / 0	424 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.02 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 MAX CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CS1 (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	-1568 / 0	-77.4	-77.4 0.21 (1)	5.02	E-B	0 / 1830	0.45 (1)
B-C	0 / 0	-72.5	-72.5 0.09 (1)	10.00	B-D	-2024 / 0	0.45 (1)
D-C	-95 / 0	0.0	0.0 0.01 (1)	7.81	A-E	0 / 1432	0.35 (1)
F-A	-1130 / 0	0.0	0.0 0.12 (1)	7.43			
F-E	0 / 0	-594.5	-594.5 0.50 (1)	10.00			
E-D	0 / 1479	-594.5	-594.5 0.56 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
B	3'-4-11	-31	-31	-	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 3'-4-11
RIGHT SETBACK = 0-0
END SETBACK = 2'-2-8
END WALL WIDTH = 5-8
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADOT'L LOADS BASED ON 55 % OF GSL.

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 26-10-0
END DISTANCE = 6-0-0
END SPAN CARRIED = 26-10-0
END WALL WIDTH = 5-8
APPLIED TO BACK SIDE OF BOTTOM CHORD.
- ADOT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.06")

CSI: TC=0.21/1.00 (A-B:1), BC=0.56/1.00 (D-E:1), WB=0.45/1.00 (B-E:1), SSI=0.78/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

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READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

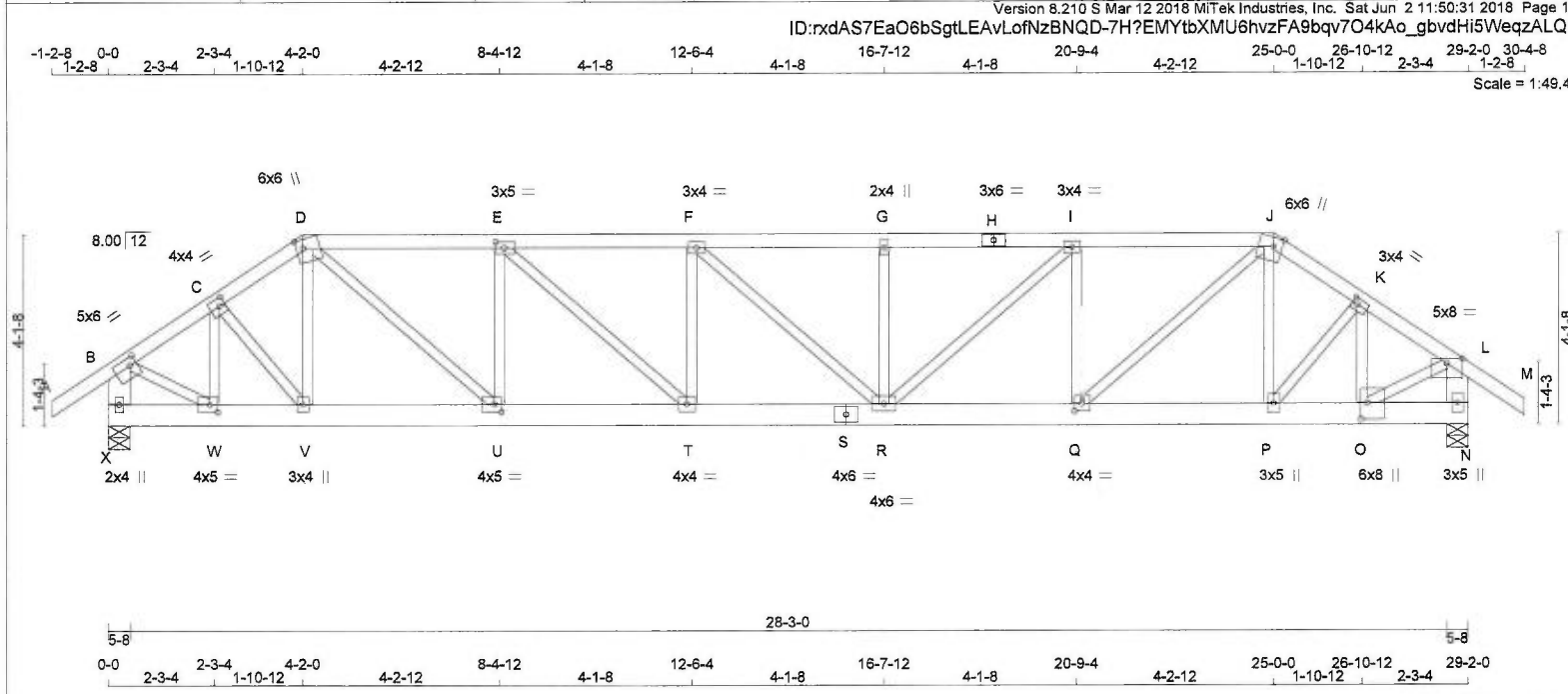
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (E) (INPUT = 0.90)

JSI METAL= 0.48 (E) (INPUT = 1.00)

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LUMBER							DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER										DESIGN CRITERIA									
N. L. G. A. RULES							BEARINGS										*** SPECIAL LOADS ANALYSIS ***									
CHORDS SIZE							FACTORED										GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.									
A - D 2x4 DRY No.2							MAXIMUM FACTORED										LOADS WERE DERIVED FROM USER INPUT									
D - H 2x4 DRY No.2							INPUT										NO FURTHER MODIFICATIONS WERE MADE									
H - J 2x4 DRY No.2							REQRD																			
J - M 2x4 DRY No.2							BRG																			
X - B 2x6 DRY No.2							IN-SX																			
N - L 2x6 DRY No.2							BRG																			
X - S 2x6 DRY No.2							IN-SX																			
S - N 2x6 DRY No.2																										
ALL WEBS 2x3 DRY No.2							UNFACTORED REACTIONS										SPECIFIED LOADS:									
EXCEPT							1ST LCASE										TOP CH. LL = 23.3 PSF									
							MAX/MIN COMPONENT REACTIONS										DL = 3.0 PSF									
							JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL										BOT CH. LL = 0.0 PSF									
							X 1149 817 / 0 0 / 0 0 / 0 0 / 0 333 / 0 0 / 0										DL = 7.0 PSF									
							N 2347 1655 / 0 0 / 0 0 / 0 0 / 0 693 / 0 0 / 0										TOTAL LOAD = 33.3 PSF									
DRY: SEASONED LUMBER.							BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, N										SPACING = 24.0 IN. C/C									
							BRACING										LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM									
							TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.49 FT.										*** NON STANDARD GIRDER ***									
							MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.										ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.									
							ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.										THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010									
PLATES (table is in inches)							LOADING										THIS DESIGN COMPLIES WITH:									
							TOTAL LOAD CASES: (4)										- PART 9 OF OBC 2012, BCBC 2012, ABC 2014									
																	- CSA 086-09									
																	- TPIC 2011									
																	(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD									
																	ALLOWABLE DEFL.(LL)= L/360 (0.97")									
																	CALCULATED VERT. DEFL.(LL) = L/999 (0.15")									
																	ALLOWABLE DEFL.(TL)= L/360 (0.97")									
																	CALCULATED VERT. DEFL.(TL) = L/999 (0.27")									
																	CSI: TC=0.39/1.00 (G-I:1), BC=0.58/1.00 (Q-P:1), WB=0.74/1.00 (L-O:1), SSI=0.17/1.00 (D-E:1)									
																	DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00									
																	COMP=1.00 SHEAR=1.00 TENS= 1.00									
																	COMPANION LIVE LOAD FACTOR = 0.50									
																	TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.									
																	NAIL VALUES									
																	PLATE GRIP(DRY) SHEAR SECTION									
																	(PSI) (PLI) (PLI)									
																	MAX MIN MAX MIN MAX MIN									
																	MT20 618 354 1667 822 2284 1656									

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

LICENSED PROFESSIONAL ENGINEER
N.A. EL-MASRI
JUN 02, 2018

CONTINUED ON PAGE 2

KOTT



READ ALL NOTES ON THIS PAGE AND ON
 ENGINEERING NOTE PAGE ENP-1. THIS
 NOTE PAGE IS AN INTEGRAL PART OF
 THIS DRAWING AS IT CONTAINS
 SPECIFICATIONS AND CRITERIA USED IN
 THE DESIGN OF THIS COMPONENT.

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN YORK HOMES-LIANA 1 EL 1	DRWG NO.
NE0618-003	G03	1	1	TRUSS DESC.		PAGE 7 OF 21

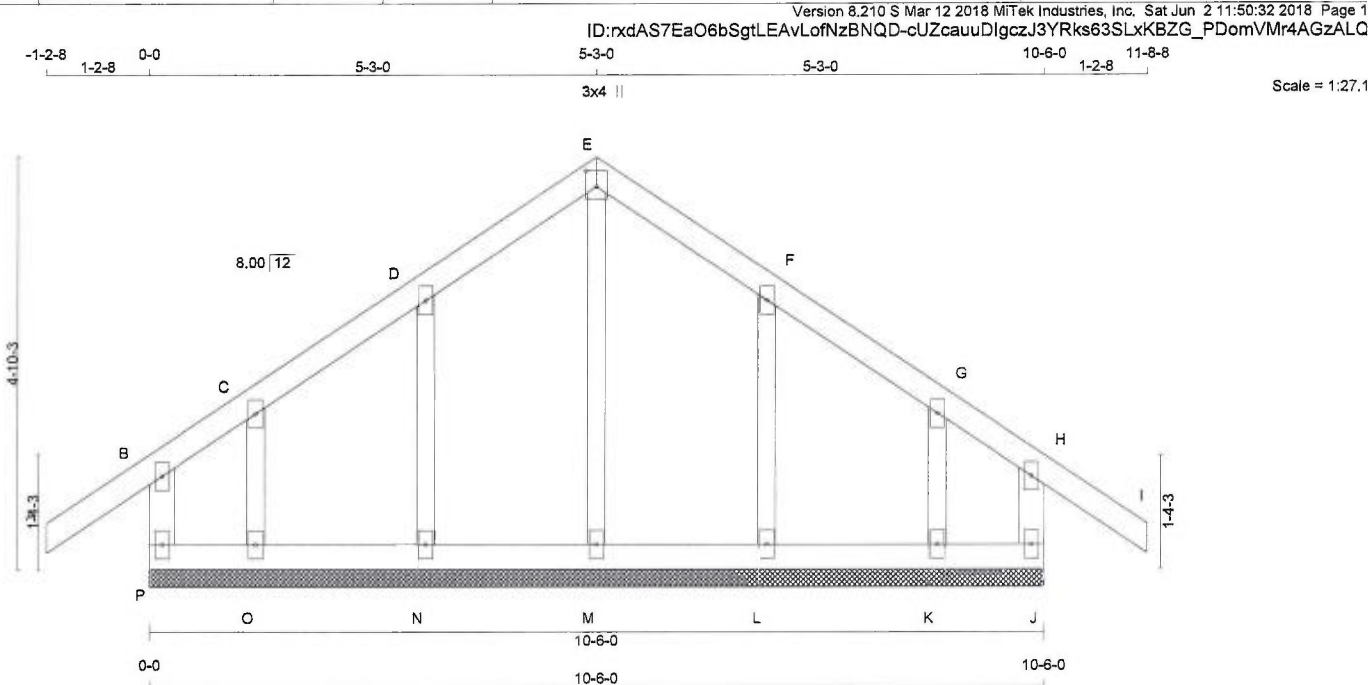
Version 8.210 S Mar 12 2018 MiTek Industries, Inc. Sat Jun 2 11:50:31 2018 Page 2
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HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 2021.9 lbs FACTORED DOWN AT
26-10-12 ON BOTTOM CHORD. DESIGN FOR
UNSPECIFIED CONNECTION(S) IS DELEGATED
TO THE BUILDING DESIGNER.

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.68 (S) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.





Scale = 1:27.1

TOTAL WEIGHT = 43 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
P - B	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

GABLE STUDS SPACED AT 2'-0" OC.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)
FR-TO		FROM TO		FR-TO			
P-B	-157 / 0	0.0	0.03 (1)	7.81	M-E	-189 / 0	0.06 (1)
A-B	0 / 28	-77.4	-77.4 0.09 (1)	10.00	N-D	-167 / 0	0.03 (1)
B-C	0 / 8	-77.4	-77.4 0.06 (1)	10.00	O-C	-87 / 0	0.01 (1)
C-D	0 / 31	-77.4	-77.4 0.05 (1)	10.00	L-F	-167 / 0	0.03 (1)
D-E	0 / 29	-77.4	-77.4 0.05 (1)	10.00	K-G	-87 / 0	0.01 (1)
E-F	0 / 29	-77.4	-77.4 0.05 (1)	10.00			
F-G	0 / 31	-77.4	-77.4 0.05 (1)	10.00			
G-H	0 / 8	-77.4	-77.4 0.06 (1)	10.00			
H-I	0 / 28	-77.4	-77.4 0.09 (1)	10.00			
J-H	-157 / 0	0.0	0.03 (1)	7.81			
P-O	-18 / 0	-17.5	-17.5 0.01 (4)	6.25			
O-N	-22 / 0	-17.5	-17.5 0.01 (4)	6.25			
N-M	-26 / 0	-17.5	-17.5 0.01 (4)	6.25			
M-L	-26 / 0	-17.5	-17.5 0.01 (4)	6.25			
L-K	-22 / 0	-17.5	-17.5 0.01 (4)	6.25			
K-J	-18 / 0	-17.5	-17.5 0.01 (4)	6.25			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.09/1.00 (H-I:1), BC=0.01/1.00 (M-N:4), WB=0.06/1.00 (E-M:1), SSI=0.06/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

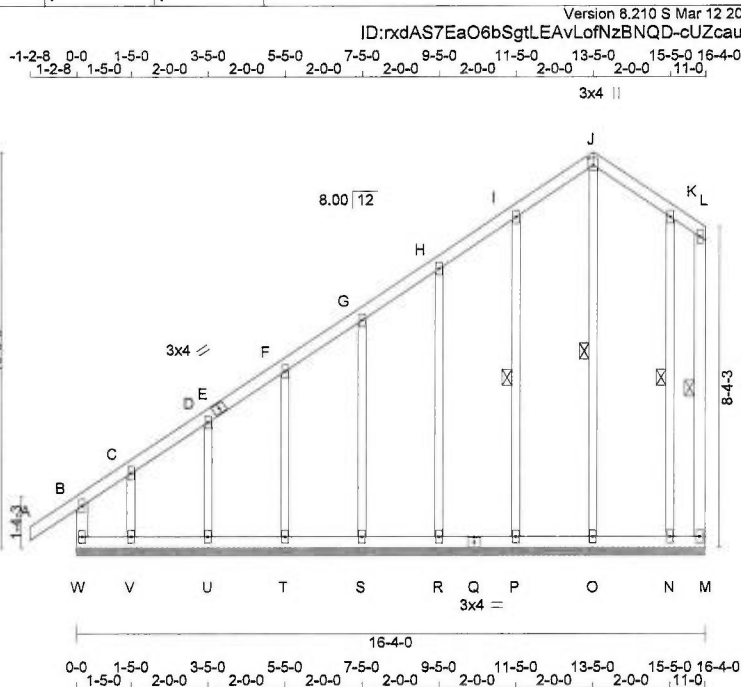
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (E) (INPUT = 0.90)
JSI METAL= 0.06 (E) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 93 lb
[M]

LUMBER				DESCR.
N, L, G, A, RULES	CHORDS	SIZE	LUMBER	
W - B	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
E - J	2x4	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
W - Q	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				
GABLE STUDS SPACED AT 2-0-0 OC.				

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	2.0	4.0
C, D, F, G, H, I, K				
C	TMW+w	MT20	2.0	4.0
E	TS-t	MT20	3.0	4.0
J	TTW+p	MT20	3.0	4.0
L	TMV+p	MT20	2.0	4.0
M	BMV1+p	MT20	2.0	4.0
N, O, P, R, S, T, U, V				
N	BMW1+w	MT20	2.0	4.0
Q	BS-t	MT20	3.0	4.0
W	BMV1+p	MT20	2.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.
1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF L-M, J-O, I-P, K-N, DBS = 20-0-0, CBF = 21 LBS.
DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	
FR-TO		FROM TO		FR-TO			
W-B	-196 / 0	0.0 0.0 0.03 (1)	7.81	O-J	-169 / 0	0.12 (1)	
A-B	0 / 28	-77.4 -77.4 0.09 (1)	10.00	P-I	-157 / 0	0.08 (1)	
B-C	-38 / 0	-77.4 -77.4 0.08 (1)	6.25	R-H	-153 / 0	0.16 (1)	
C-D	-9 / 0	-77.4 -77.4 0.04 (1)	10.00	S-G	-154 / 0	0.10 (1)	
D-E	-10 / 0	-77.4 -77.4 0.04 (1)	6.25	T-F	-151 / 0	0.05 (1)	
E-F	-10 / 0	-77.4 -77.4 0.04 (1)	6.25	U-D	-163 / 0	0.04 (1)	
F-G	-6 / 0	-77.4 -77.4 0.04 (1)	10.00	V-C	-83 / 0	0.01 (1)	
G-H	-4 / 0	-77.4 -77.4 0.04 (1)	10.00	N-K	-118 / 0	0.06 (1)	
H-I	-1 / 0	-77.4 -77.4 0.04 (1)	10.00				
I-J	0 / 0	-77.4 -77.4 0.04 (1)	10.00				
J-K	0 / 4	-77.4 -77.4 0.04 (1)	10.00				
K-L	0 / 6	-77.4 -77.4 0.02 (1)	10.00				
M-L	-20 / 0	0.0 0.0 0.01 (1)	6.25				
W-V	0 / 15	-17.5 -17.5 0.03 (1)	10.00				
V-U	0 / 12	-17.5 -17.5 0.01 (4)	10.00				
U-T	0 / 8	-17.5 -17.5 0.01 (4)	10.00				
T-S	0 / 5	-17.5 -17.5 0.01 (4)	10.00				
S-R	0 / 3	-17.5 -17.5 0.01 (4)	10.00				
R-Q	0 / 1	-17.5 -17.5 0.01 (4)	10.00				
Q-P	0 / 1	-17.5 -17.5 0.01 (4)	10.00				
P-O	0 / 0	-17.5 -17.5 0.01 (4)	10.00				
O-N	0 / 0	-17.5 -17.5 0.01 (4)	10.00				
N-M	0 / 0	-17.5 -17.5 0.01 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.09/1.00 (A-B:1), BC=0.03/1.00 (V-W:1), WB=0.16/1.00 (H-R:1), SSI=0.06/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

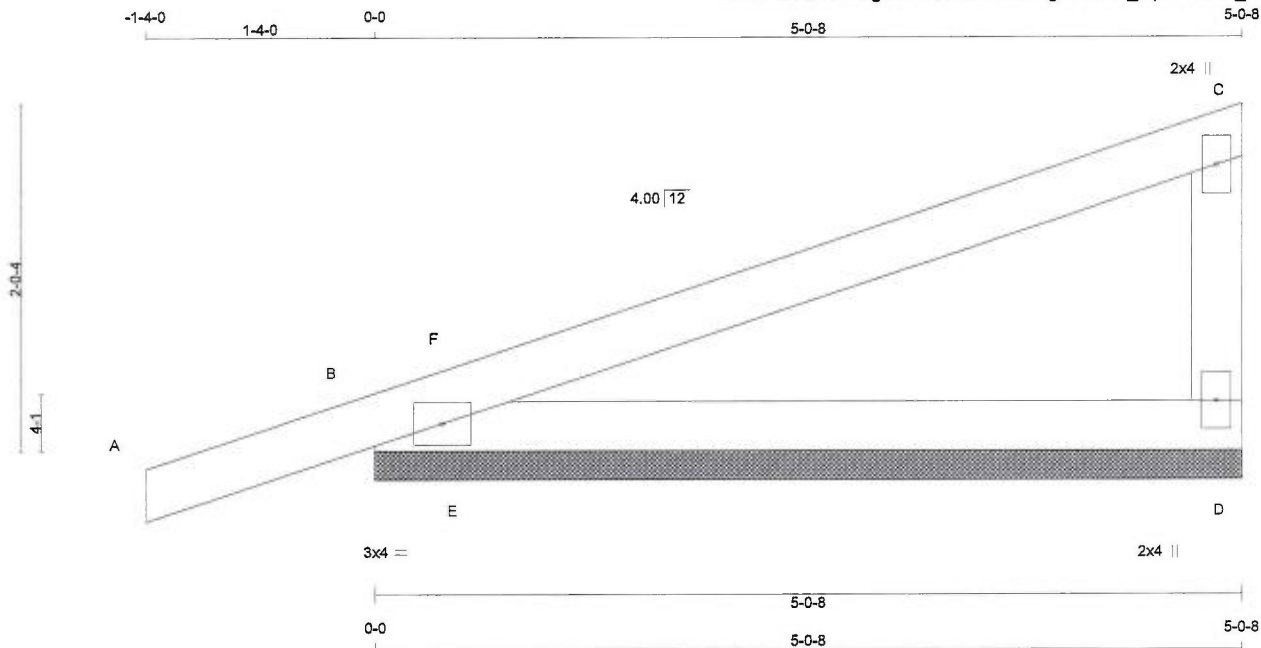
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.54 (J) (INPUT = 0.90)
JSI METAL= 0.06 (J) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:13.4

TOTAL WEIGHT = 2 X 15 = 30 lb [M]

LUMBER

N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2 SPF
D - C	2x4	DRY	No.2 SPF
B - D	2x4	DRY	No.2 SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2'-0" OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TMV+p	MT20	2.0	4.0		
D	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX (LC)		
FR-TO		FROM	TO		FR-TO				
A-B	0 / 16	-77.4	-77.4	0.10 (1)	10.00	E-F	-211 / 35	0.00 (1)	
B-F	-18 / 5	-77.4	-77.4	0.06 (4)	6.25				
F-C	0 / 2	-77.4	-77.4	0.26 (1)	10.00				
D-C	-171 / 0	0.0	0.0	0.02 (1)	7.81				
B-E	0 / 0	-17.5	-17.5	0.18 (1)	10.00				
E-D	0 / 0	-17.5	-17.5	0.18 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD	=	33.3	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.26/1.00 (C-F:1) , BC=0.18/1.00 (D-E:1) , WB=0.00/1.00 (E-F:1) , SS=0.18/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	618	354	1687
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

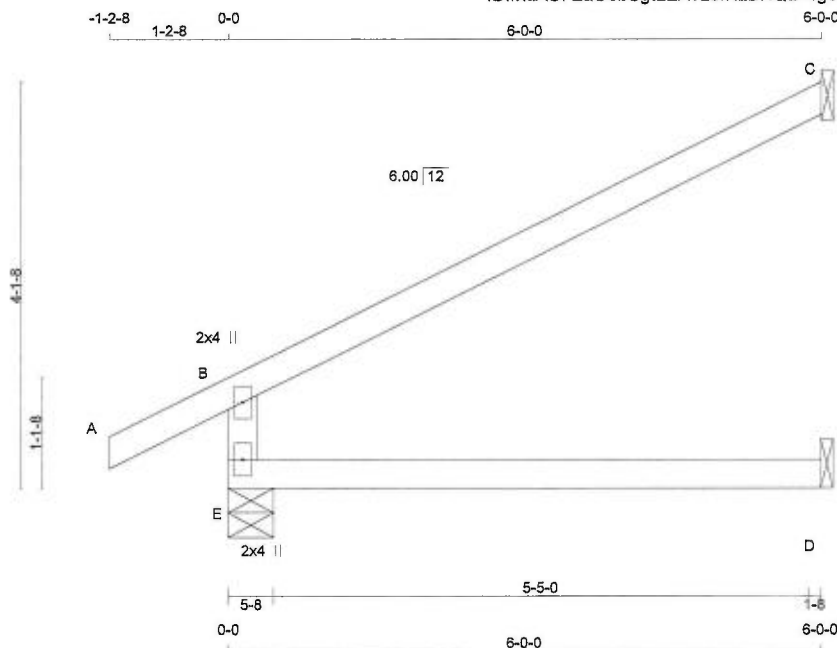
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:23.4

TOTAL WEIGHT = 14 X 17 = 237 lb [M]F

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	450	0	450	0	5-8	5-8
C	174	0	174	0	1-8	1-8
D	43	0	48	0	1-8	1-8

SEE MITTEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
E	314	234 / 0	0 / 0	0 / 0	0 / 0	80 / 0	0 / 0
C	118	105 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0
D	35	0 / 0	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		WEBS	
	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
FR-TO						
E-B	-389 / 0	0.0	0.0	0.11 (4)	7.81	
A-B	0 / 22	-77.4	-77.4	0.09 (1)	10.00	
B-C	-26 / 0	-77.4	-77.4	0.36 (1)	6.25	
E-D	0 / 0	-17.5	-17.5	0.13 (4)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.03")

CSI: TC=0.36/1.00 (B-C:1), BC=0.13/1.00 (D-E:4),
WB=0.00/1.00 (n/a:0), SSI=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

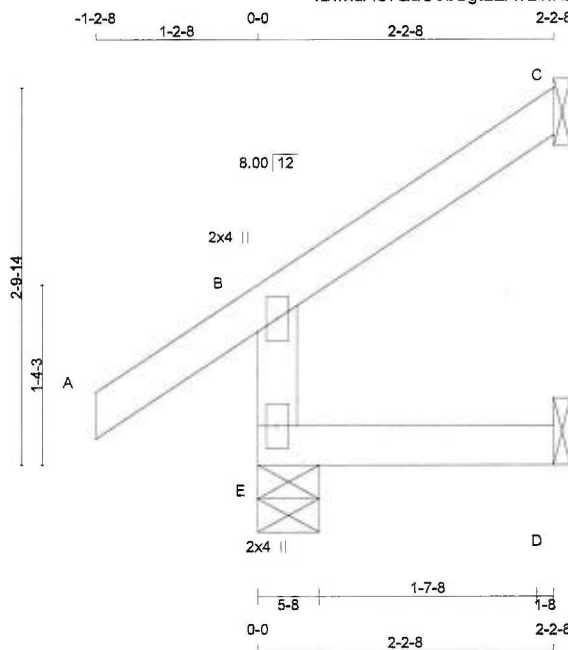
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (B) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:17.2

TOTAL WEIGHT = 2 X 8 = 17 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	No.2	SPF
A - C	2x4	No.2	SPF
E - D	2x4	No.2	SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
E	227	0	227	0	5-8	5-8
C	64	0	64	0	1-8	1-8
D	17	0	19	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	157	124 / 0	0 / 0	0 / 0	0 / 0	33 / 0	0 / 0
C	44	39 / 0	0 / 0	0 / 0	0 / 0	5 / 0	0 / 0
D	14	0 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS		FACTORED		MAX. CSI (LC)	UNBRAC	MEMB.	WEBS	
	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	VERT. LOAD (PLF)	MAX. CSI (LC)				MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO							FR-TO		
E-B	-206 / 0	0.0	0.0	0.01 (4)	7.81				
A-B	0 / 28	-77.4	-77.4	0.09 (1)	10.00				
B-C	-12 / 0	-77.4	-77.4	0.06 (1)	6.25				
E-D	0 / 0	-17.5	-17.5	0.02 (4)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.00")

CSI: TC=0.09/1.00 (A-B:1), BC=0.02/1.00 (D-E:4),
WB=0.00/1.00 (n/a:0), SSI=0.07/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

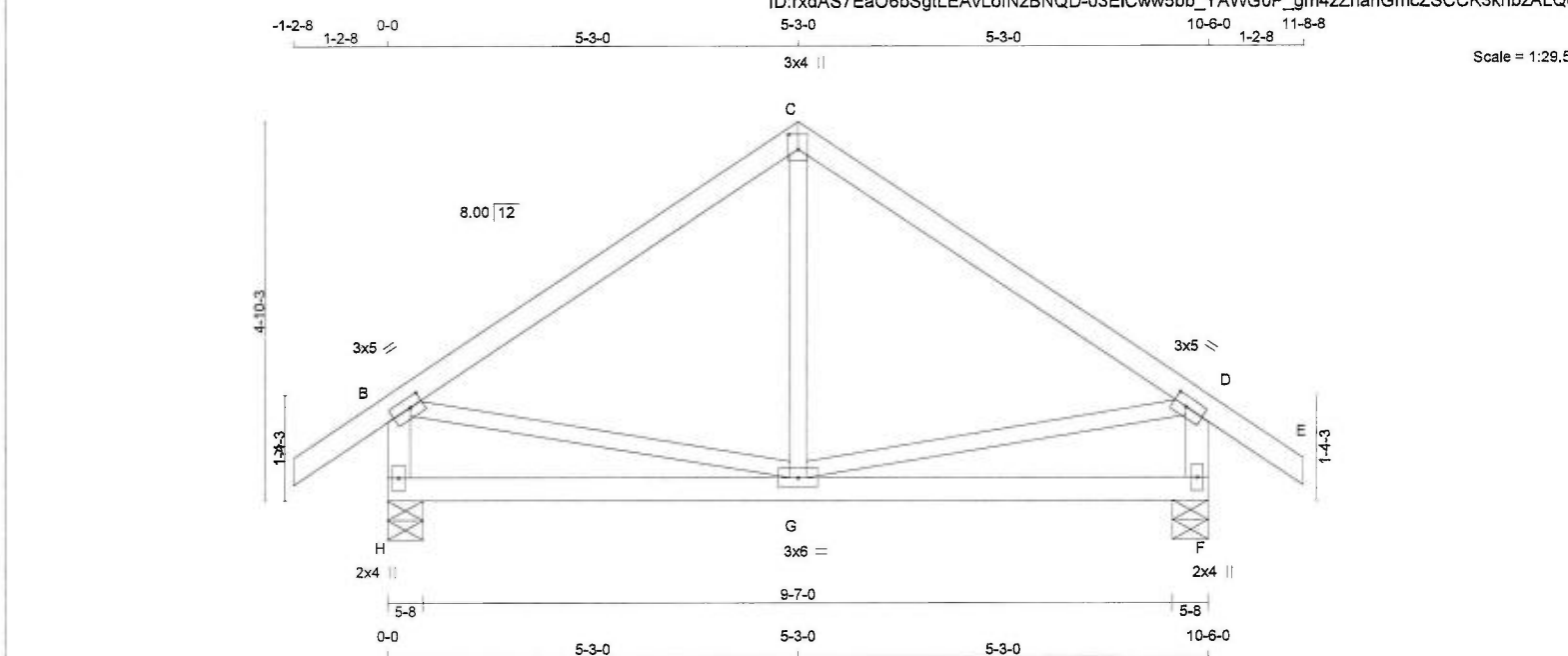
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 3 X 44 = 131 lb [M]F

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
H - B	2x4	DRY	No.2
F - D	2x4	DRY	No.2
H - F	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	3.0	5.0	1.50	2.00
C	TTW+p	MT20	3.0	4.0	2.25	1.50
D	TMVW-t	MT20	3.0	5.0	1.50	2.00
F	BMV1+p	MT20	2.0	4.0		
G	BMVWV-t	MT20	3.0	6.0		
H	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT		VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX	IN-SX	IN-SX
H	598	0	598	0	0	5-8	5-8	5-8	5-8
F	598	0	598	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN. COMPONENT REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL		
H	417	305 / 0	0 / 0	0 / 0	0 / 0	113 / 0	0 / 0		
F	417	305 / 0	0 / 0	0 / 0	0 / 0	113 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED FORCE		FACTORED VERT. LOAD LC1		MAX. FACTORED FORCE		WEBS		MAX. FACTORED FORCE	
MEMB.		(LBS)	(PLF)	CSI (LC)	UNBRAC LENGTH	MEMB.		(LBS)	(PLF)	CSI (LC)	
FR-TO						FR-TO					
A-B	0 / 28	-77.4	-77.4	0.09 (1)	10.00	G-C	-5 / 89	0.03 (4)			
B-C	-362 / 0	-77.4	-77.4	0.27 (1)	6.25	B-G	0 / 307	0.07 (1)			
C-D	-362 / 0	-77.4	-77.4	0.27 (1)	6.25	G-D	0 / 307	0.07 (1)			
D-E	0 / 28	-77.4	-77.4	0.09 (1)	10.00						
H-B	-562 / 0	0.0	0.0	0.06 (1)	7.81						
F-D	-562 / 0	0.0	0.0	0.06 (1)	7.81						
H-G	0 / 0	-17.5	-17.5	0.14 (4)	10.00						
G-F	0 / 0	-17.5	-17.5	0.14 (4)	10.00						

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.27/1.00 (B-C:1), BC=0.14/1.00 (G-H:4), WB=0.07/1.00 (B-G:1), SSI=0.13/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

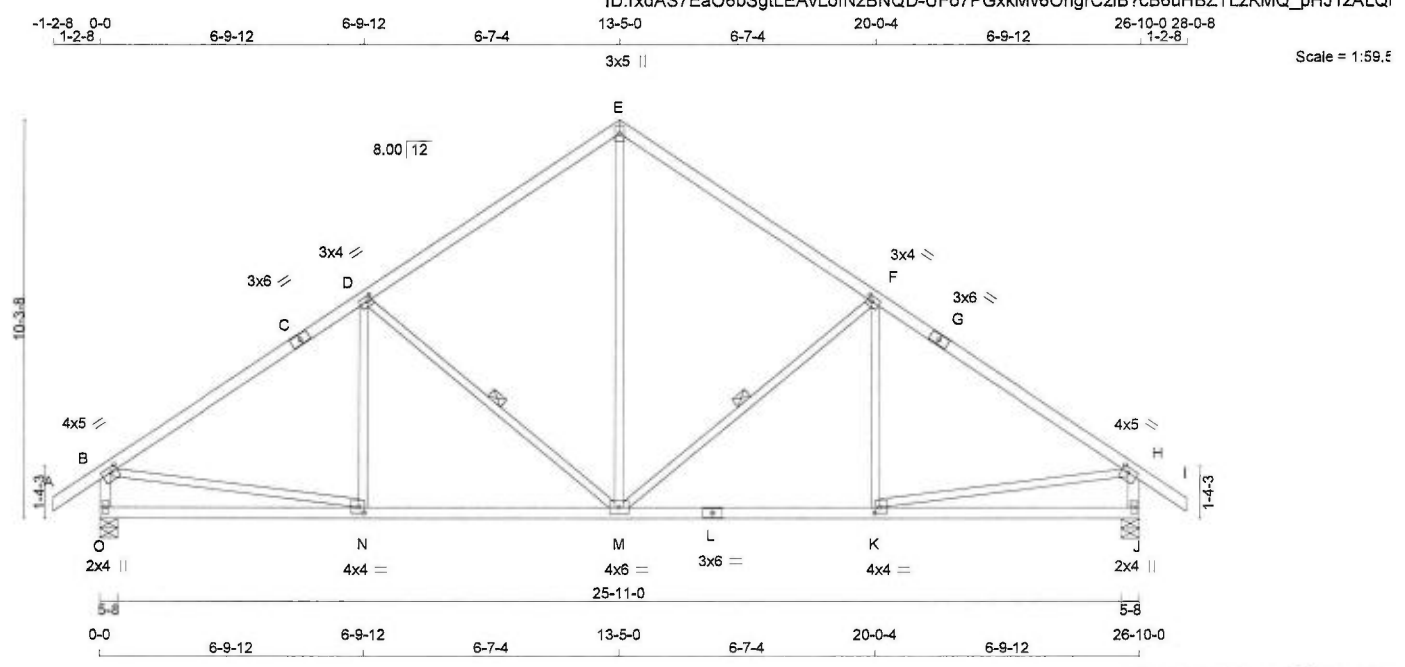
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.67 (G) (INPUT = 0.90)
JSI METAL= 0.16 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
N. L. G. A. RULES				BEARINGS				SPECIFIED LOADS:			
CHORDS SIZE				FACTORED GROSS REACTION				TOP CH. LL = 23.3 PSF			
LUMBER				MAXIMUM FACTORED GROSS REACTION				DL = 3.0 PSF			
DESCR.				INPUT BRG				BOT CH. LL = 0.0 PSF			
SPF				REQRD BRG				DL = 7.0 PSF			
A - C 2x4 DRY No.2				JT VERT HORZ				TOTAL LOAD = 33.3 PSF			
C - E 2x4 DRY No.2				O 1372 0 1372 0 0 5-8 5-8							
E - G 2x4 DRY No.2				J 1372 0 1372 0 0 5-8 5-8							
G - I 2x4 DRY No.2											
O - B 2x4 DRY No.2											
J - H 2x4 DRY No.2											
O - L 2x4 DRY No.2											
L - J 2x4 DRY No.2											
ALL WEBS 2x3 DRY No.2											
EXCEPT											
SPF											
DRY: SEASONED LUMBER.											
UNFACTORED REACTIONS											
1ST LCASE MAX/MIN. COMPONENT REACTIONS											
JT COMBINED SNOW LIVE PERM. LIVE WIND DEAD SOIL											
O 961 685 / 0 0 / 0 0 / 0 0 / 0 276 / 0 0 / 0											
J 961 685 / 0 0 / 0 0 / 0 0 / 0 276 / 0 0 / 0											
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J											
BRACING											
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.79 FT.											
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.											
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.											
1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF F-M, D-M. DBS = 20-0-0. CBF = 64 LBS.											
DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.											
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW											
LOADING											
TOTAL LOAD CASES: (4)											
C H O R D S				W E B S							
MEMB. MAX. FACTORED FORCE (LBS)				MEMB. MAX. FACTORED FORCE (LBS)							
FACTORED VERT. LOAD LC1 MAX UNBRAC LENGTH FR-TO				FACTORED VERT. LOAD LC1 MAX UNBRAC LENGTH FR-TO							
FR-TO				FR-TO							
A-B 0 / 28				M-E 0 / 766							
B-C -1468 / 0				M-F -509 / 0							
C-D -1468 / 0				K-F -58 / 94							
D-E -1071 / 0				D-M -509 / 0							
E-F -1071 / 0				N-D -58 / 94							
F-G -1468 / 0				B-N 0 / 1265							
G-H -1468 / 0				K-H 0 / 1265							
H-I 0 / 28											
O-B -1323 / 0											
J-H -1323 / 0											
O-N 0 / 0											
N-M 0 / 1251											
M-L 0 / 1251											
L-K 0 / 1251											
K-J 0 / 0											

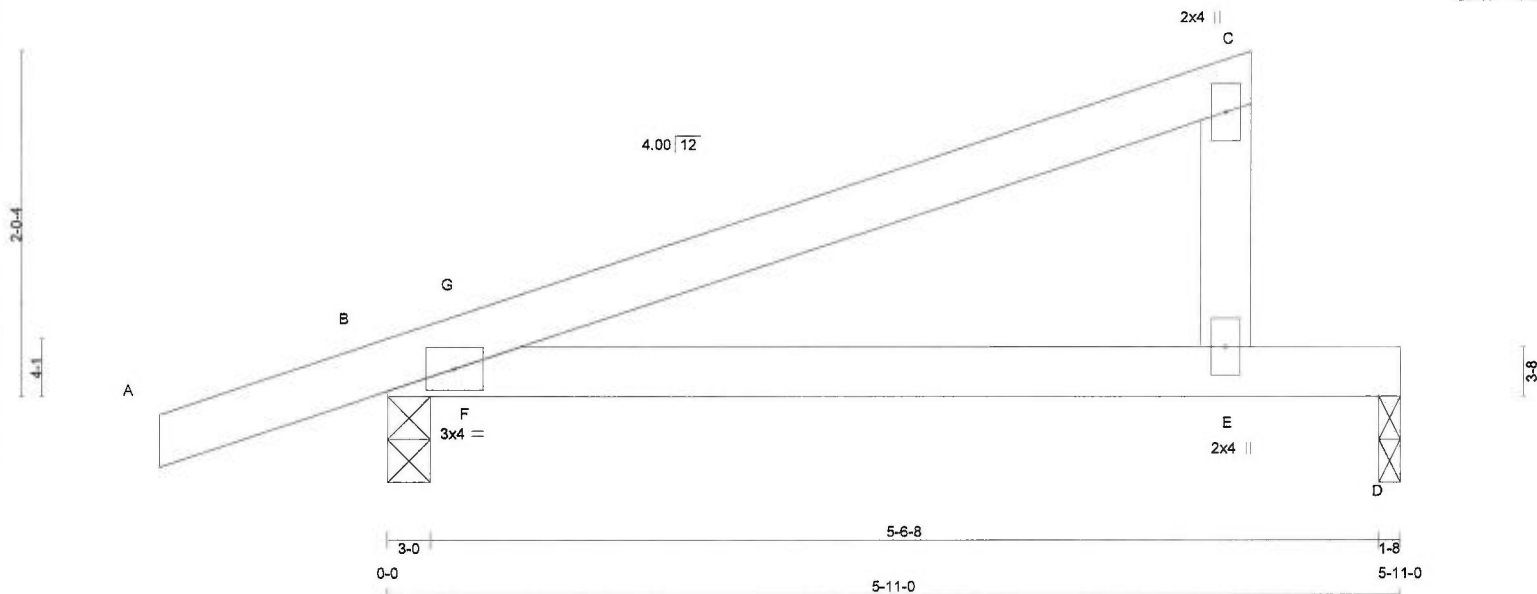


READ ALL NOTES ON THIS PAGE AND ON
 ENGINEERING NOTE PAGE ENP-1. THIS
 NOTE PAGE IS AN INTEGRAL PART OF
 THIS DRAWING AS IT CONTAINS
 SPECIFICATIONS AND CRITERIA USED IN
 THE DESIGN OF THIS COMPONENT.



-1-4-0 1-4-0 0-0 5-0-8 5-0-8

Scale = 1:13.5



TOTAL WEIGHT = 3 X 16 = 48 lb
[M][F]

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
E - C	2x4	DRY	No.2
B - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

DESCR.
SPF
SPF
SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
B	382	0	382	0	3-0	3-0
D	218	0	218	0	1-8	1-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	266	199 / 0	0 / 0	0 / 0	0 / 0	67 / 0	0 / 0
D	154	100 / 0	0 / 0	0 / 0	0 / 0	54 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		MAX. UNBRAC LENGTH	FR-TO	WEBS	
	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX. CSI (LC)			MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
A-B	0 / 16	-77.4	-77.4	0.10 (1)	10.00	F-G	-115 / 66	0.00 (1)
B-G	-29 / 0	-77.4	-77.4	0.07 (4)	6.25			
G-C	0 / 3	-77.4	-77.4	0.29 (1)	10.00			
E-C	-182 / 0	0.0	0.0	0.02 (1)	7.81			
B-F	0 / 0	-17.5	-17.5	0.14 (1)	10.00			
F-E	0 / 0	-17.5	-17.5	0.27 (1)	10.00			
E-D	0 / 0	-17.5	-17.5	0.25 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/893 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/463 (0.15")

CSI: TC=0.29/1.00 (C-G:1), BC=0.27/1.00 (E-F:1), WB=0.00/1.00 (F-G:1), SS=0.17/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

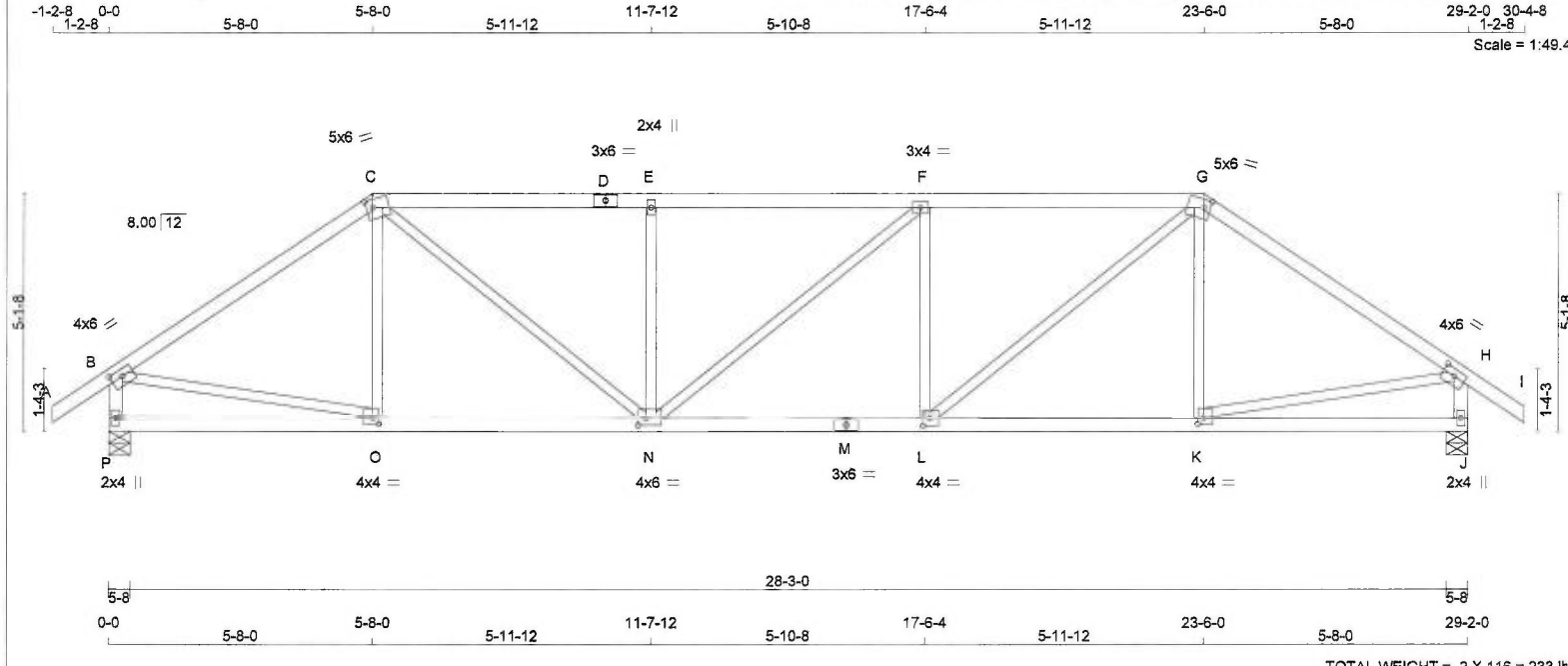


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Version 8.210 S Mar 12 2018 MiTek Industries, Inc. Sat Jun 2 11:50:37 2018 Page 1

ID: rxdAS7EaO6bSgtLEavLofNzBNQD-yRMVdcyM7CEFPqQPXPIE9Oe3ratN4QHvfeYrUzALQr



TOTAL WEIGHT = 2 X 116 = 233 lb

(M)

LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.	BEARINGS		
A - C	2x4	DRY	No.2	SPF	FACTORED	MAXIMUM FACTORED	INPUT
C - D	2x4	DRY	No.2	SPF	GROSS REACTION	GROSS REACTION	BRG
D - G	2x4	DRY	No.2	SPF	JT VERT	DOWN	IN-SX
G - I	2x4	DRY	No.2	SPF	P 1483	0	5-8
P - B	2x4	DRY	No.2	SPF	J 1483	0	5-8
J - H	2x4	DRY	No.2	SPF			
P - M	2x4	DRY	No.2	SPF			
M - J	2x4	DRY	No.2	SPF			
ALL WEBS	2x3	DRY	No.2	SPF	UNFACTORED REACTIONS		
EXCEPT					1ST LCASE	MAX./MIN. COMPONENT REACTIONS	
					JT COMBINED	SNOW	LIVE
					P 1039	739 / 0	0 / 0
					J 1039	739 / 0	0 / 0

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF

DL = 3.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.0 PSF

TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014

- CSA 086-09

- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.97")

CALCULATED VERT. DEFL.(LL) = L/999 (0.10")

ALLOWABLE DEFL.(TL) = L/360 (0.97")

CALCULATED VERT. DEFL.(TL) = L/999 (0.19")

CSI: TC=0.54/1.00 (B-C:1), BC=0.38/1.00 (L-N:1), WB=0.30/1.00 (B-O:1), SSI=0.21/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(P.S.I) (P.LI) (P.LI)

MAX MIN MAX MIN MAX MIN

MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (G) (INPUT = 0.90)

JSI METAL= 0.56 (M) (INPUT = 1.00)

DRY: SEASONED LUMBER.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.22 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

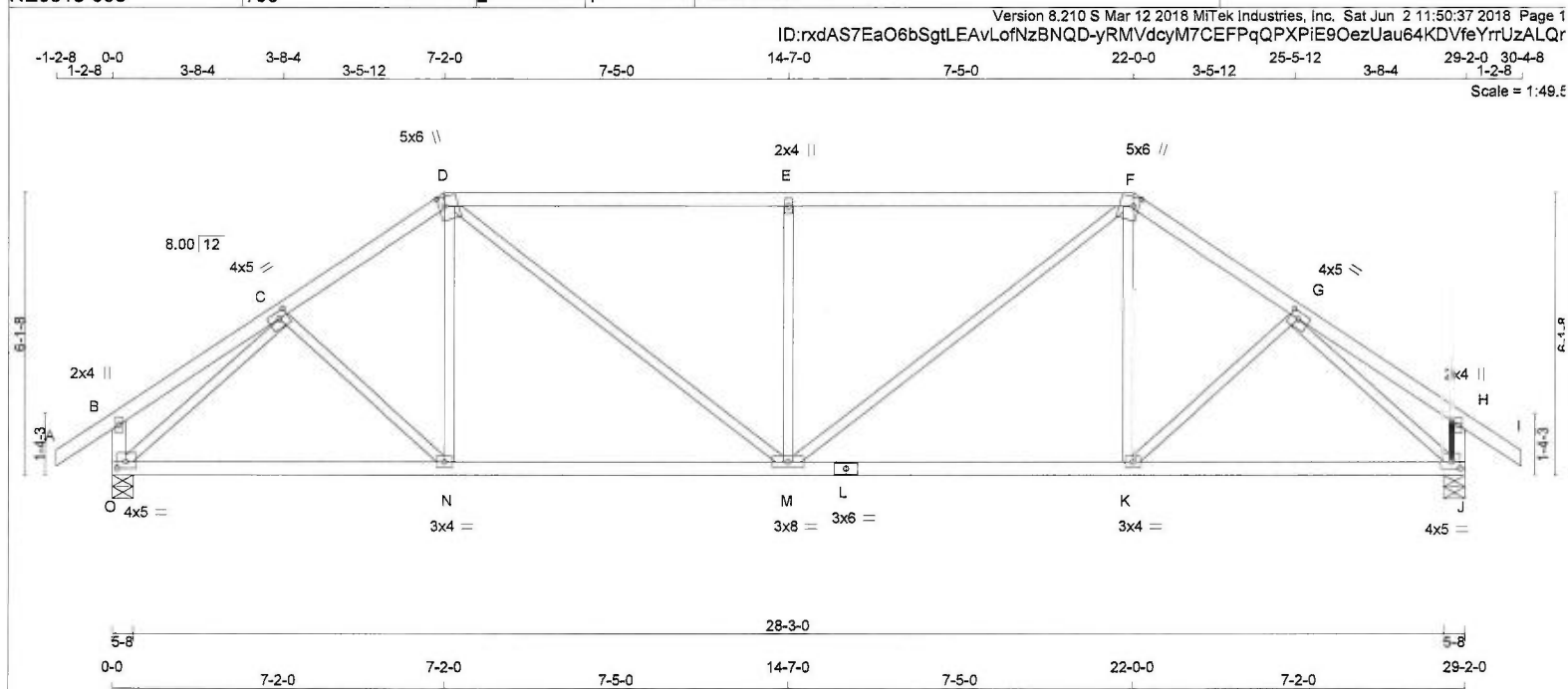
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-77.4 -77.4	0.09 (1)	10.00	O-C	-121 / 57	0.05 (1)
B-C	-1596 / 0	-77.4 -77.4	0.54 (1)	4.60	C-N	0 / 949	0.21 (1)
C-D	-2062 / 0	-77.4 -77.4	0.48 (1)	4.24	N-E	-495 / 0	0.19 (1)
D-E	-2062 / 0	-77.4 -77.4	0.48 (1)	4.24	N-F	-2 / 0	0.00 (1)
E-F	-2062 / 0	-77.4 -77.4	0.48 (1)	4.22	L-F	-497 / 0	0.19 (1)
F-G	-2064 / 0	-77.4 -77.4	0.49 (1)	4.22	L-G	0 / 951	0.21 (1)
G-H	-1596 / 0	-77.4 -77.4	0.54 (1)	4.60	K-G	-121 / 57	0.05 (1)
H-I	0 / 28	-77.4 -77.4	0.09 (1)	10.00	B-O	0 / 1347	0.30 (1)
P-B	-1443 / 0	0.0 0.0	0.15 (1)	6.81	K-H	0 / 1347	0.30 (1)
J-H	-1443 / 0	0.0 0.0	0.15 (1)	6.81			
P-O	0 / 0	-17.5 -17.5	0.14 (4)	10.00			
O-N	0 / 1324	-17.5 -17.5	0.29 (1)	10.00			
N-M	0 / 2064	-17.5 -17.5	0.38 (1)	10.00			
M-L	0 / 2064	-17.5 -17.5	0.38 (1)	10.00			
L-K	0 / 1323	-17.5 -17.5	0.29 (1)	10.00			
K-J	0 / 0	-17.5 -17.5	0.14 (4)	10.00			

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



TOTAL WEIGHT = $2 \times 119 = 238$ lb

LUMBER				
N. L. G. A.	RULES			
CHORDS	SIZE		LUMBER	DESCR
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)						
J	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWV-t	MT20	4.0	5.0	1.75	2.25
D	TTWV+m	MT20	5.0	6.0	2.25	1.50
E	TMW-w	MT20	2.0	4.0		
F	TTWV+m	MT20	5.0	6.0	2.25	1.50
G	TMWV-t	MT20	4.0	5.0	1.75	2.25
H	TMV+p	MT20	2.0	4.0		
J	BMWV-t	MT20	4.0	5.0	1.75	2.25
K	BMWV-t	MT20	3.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWVW-t	MT20	3.0	8.0		
N	BMWV-t	MT20	3.0	4.0		
O	BMWV-t	MT20	4.0	5.0	1.75	2.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS						
JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX
O	1483	0	1483	0	0	5-8
J	1483	0	1483	0	0	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1039	739 / 0	0 / 0	0 / 0	0 / 0	299 / 0	0 / 0
J	1039	739 / 0	0 / 0	0 / 0	0 / 0	299 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O. J.

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.61 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

C H O R D S				W E B S			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 28	-77.4	-77.4 0.09 (1)	10.00	C-N	0 / 57	0.02 (4)
B-C	0 / 16	-77.4	-77.4 0.14 (1)	10.00	N-D	0 / 129	0.04 (4)
C-D	-1581 / 0	-77.4	-77.4 0.18 (1)	5.07	D-M	0 / 667	0.15 (1)
D-E	-1827 / 0	-77.4	-77.4 0.95 (1)	3.61	M-E	-706 / 0	0.42 (1)
E-F	-1827 / 0	-77.4	-77.4 0.95 (1)	3.61	M-F	0 / 667	0.15 (1)
F-G	-1581 / 0	-77.4	-77.4 0.18 (1)	5.07	K-F	0 / 129	0.04 (4)
G-H	0 / 16	-77.4	-77.4 0.14 (1)	10.00	K-G	0 / 57	0.02 (4)
H-I	0 / 28	-77.4	-77.4 0.09 (1)	10.00	O-C	-1772 / 0	0.69 (1)
O-B	-209 / 0	0.0	0.0 0.02 (1)	7.81	G-J	-1772 / 0	0.69 (1)
J-H	-209 / 0	0.0	0.0 0.02 (1)	7.81			
O-N	0 / 1282	-17.5	-17.5 0.33 (1)	10.00			
N-M	0 / 1302	-17.5	-17.5 0.34 (1)	10.00			
M-L	0 / 1302	-17.5	-17.5 0.34 (1)	10.00			
L-K	0 / 1302	-17.5	-17.5 0.34 (1)	10.00			
K-J	0 / 1282	-17.5	-17.5 0.33 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL	=	23.3	PSF
		DL	=	3.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.0	PSF
TOTAL LOAD		=		33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.95/1.00 (D-E:1), BC=0.34/1.00 (K-M:1),
WB=0.69/1.00 (G-J:1), SS=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES						
PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

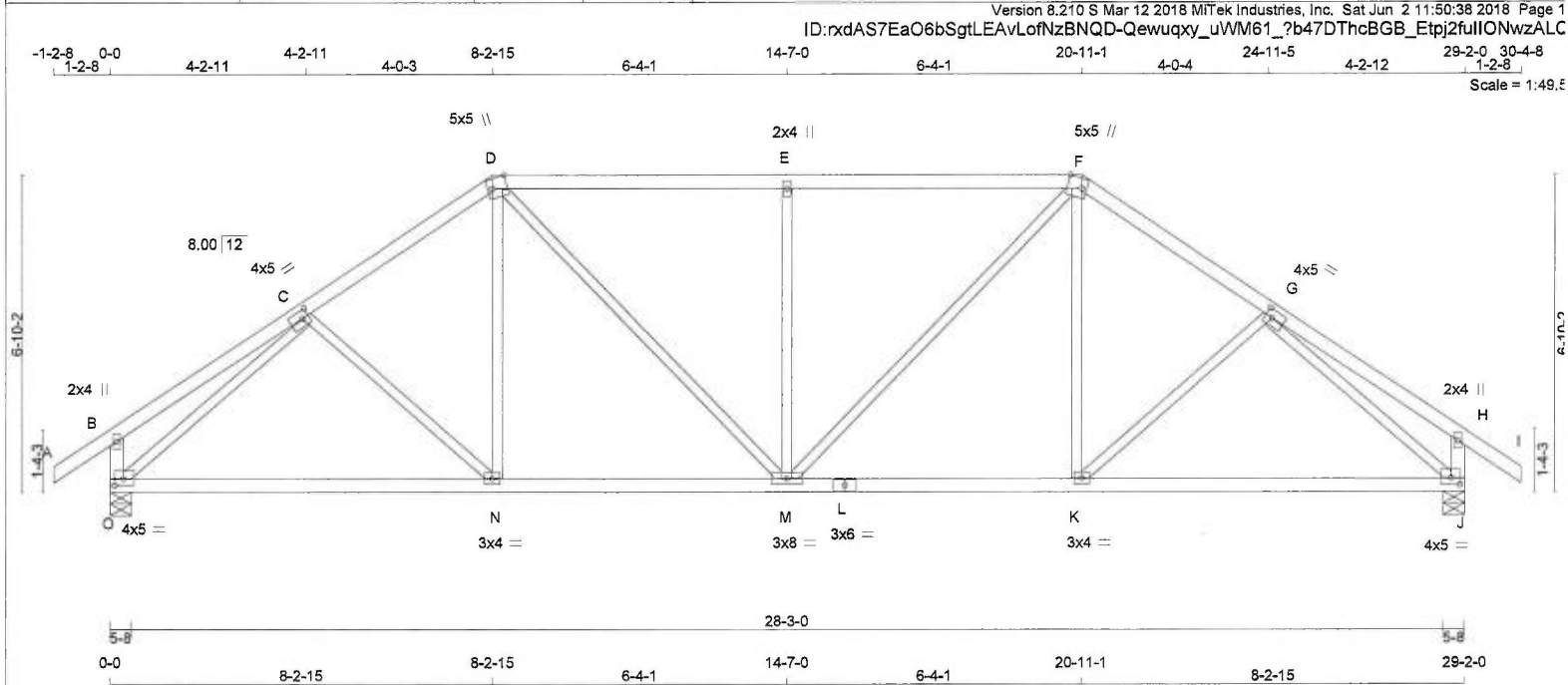
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (C) (INPUT = 0.90)
JSI METAL= 0.52 (C) (INPUT = 1.00)



**READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.**





TOTAL WEIGHT = 123 lb
[M][F]

LUMBER		LUMBER	
N. L. G. A.	RULES		
CHORDS	SIZE		
A - D	2x4 DRY	No.2	SPF
D - F	2x4 DRY	No.2	SPF
F - I	2x4 DRY	No.2	SPF
O - B	2x4 DRY	No.2	SPF
J - H	2x4 DRY	No.2	SPF
O - L	2x4 DRY	No.2	SPF
L - J	2x4 DRY	No.2	SPF

ALL WEBS EXCEPT	2x3	DRY
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DRY: SEASONED LUMBER.

PLATES (table is in inches)

BT	TYPE	PLATES	W	LEN	Y	X
	TMV+p	MT20	2.0	4.0		
	TMWV+t	MT20	4.0	5.0	2.00	1.75
	TTWV+m	MT20	5.0	5.0	Edge	3.75
	TMV+w	MT20	2.0	4.0		
	TTWV+m	MT20	5.0	5.0	Edge	3.75
	TMWV+t	MT20	4.0	5.0	2.00	1.75
	TMV+p	MT20	2.0	4.0		
	BMVWV-1	MT20	4.0	5.0	1.75	2.25
	BMWV-t	MT20	3.0	4.0		
	BS-t	MT20	3.0	6.0		
	BMWVWV-t	MT20	3.0	8.0		
	BMWV-t	MT20	3.0	4.0		
	BMVWV-1	MT20	4.0	5.0	1.75	2.25

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BOILING BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
O	1483	0	1483	0	0	5-8	5-8
J	1483	0	1483	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1039	739 / 0	0 / 0	0 / 0	0 / 0	299 / 0	0 / 0
J	1039	739 / 0	0 / 0	0 / 0	0 / 0	299 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.72 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

C H O R D S					W E B S				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)		
FR-TO		FROM TO		LENGTH	FR-TO				
A-B	0 / 28	-77.4	-77.4	0.09 (1)	10.00	C-N	-57 / 41 0.03 (1)		
B-C	0 / 19	-77.4	-77.4	0.20 (1)	10.00	N-D	0 / 193 0.05 (4)		
C-D	-1544 / 0	-77.4	-77.4	0.18 (1)	5.13	D-M	0 / 475 0.11 (1)		
D-E	-1600 / 0	-77.4	-77.4	0.44 (1)	4.72	M-E	-602 / 0 0.48 (1)		
E-F	-1600 / 0	-77.4	-77.4	0.44 (1)	4.72	M-F	0 / 475 0.11 (1)		
F-G	-1544 / 0	-77.4	-77.4	0.18 (1)	5.13	K-F	0 / 193 0.05 (4)		
G-H	0 / 19	-77.4	-77.4	0.20 (1)	10.00	K-G	-57 / 41 0.03 (1)		
H-I	0 / 28	-77.4	-77.4	0.09 (1)	10.00	Q-C	-1775 / 0 0.91 (1)		
O-B	-224 / 0	0.0	0.0	0.02 (1)	7.81	G-J	-1775 / 0 0.91 (1)		
J-H	-224 / 0	0.0	0.0	0.02 (1)	7.81				
O-N	0 / 1309	-17.5	-17.5	0.36 (1)	10.00				
N-M	0 / 1269	-17.5	-17.5	0.37 (4)	10.00				
M-L	0 / 1269	-17.5	-17.5	0.37 (4)	10.00				
L-K	0 / 1269	-17.5	-17.5	0.37 (4)	10.00				
K-J	0 / 1309	-17.5	-17.5	0.36 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	23.3	PSF
		DL =	3.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , CBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.44/1.00 (D-E:1), BC=0.37/1.00 (K-M:4),
WB=0.91/1.00 (G-J:1), SSI=0.24/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)	(PLI)	(PLI)	(PLI)	(PLI)	(PLI)
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

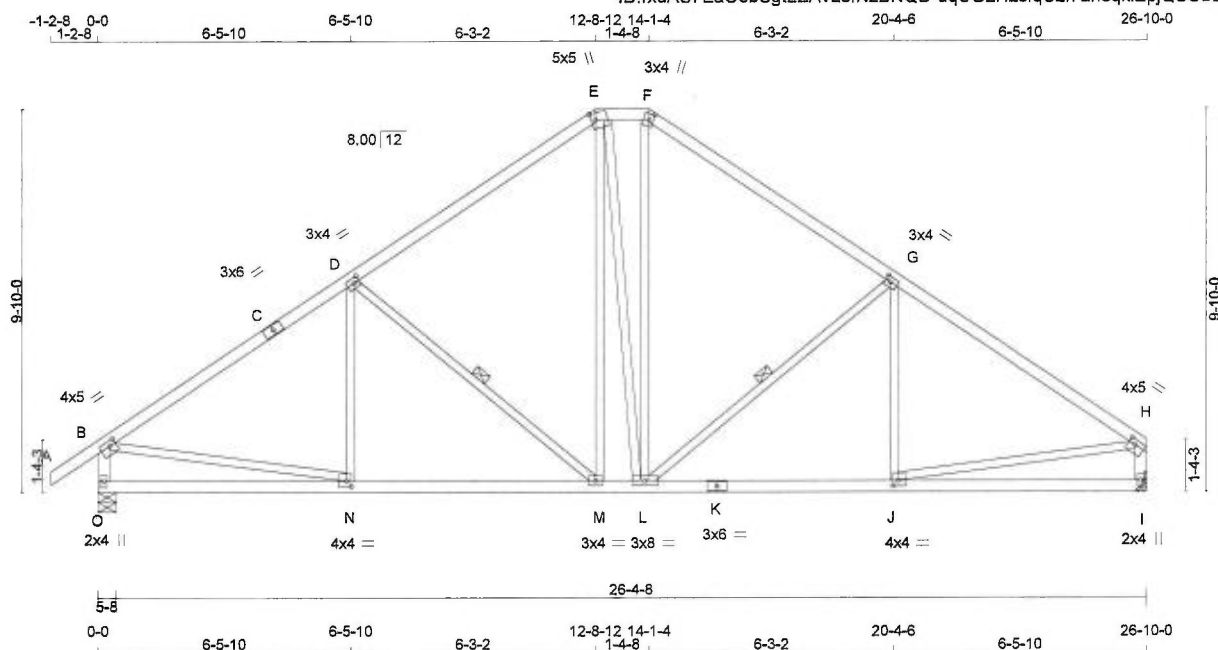
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (F) (INPUT = 0.90)
JSI METAL= 0.52 (C) (INPUT = 1.00)



**READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
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SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.**





Scale = 1:59.0

TOTAL WEIGHT = 125 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - E	2x4	DRY No.2	SPF
E - F	2x4	DRY No.2	SPF
F - H	2x4	DRY No.2	SPF
O - B	2x4	DRY No.2	SPF
I - H	2x4	DRY No.2	SPF
O - K	2x4	DRY No.2	SPF
K - I	2x4	DRY No.2	SPF

ALL WEBS 2x3 DRY
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	BMVW-t	MT20	4.0	5.0	1.50	2.00
C	TS-t	MT20	3.0	6.0		
D	BMVW-t	MT20	3.0	4.0	1.50	1.50
E	TTWW-t+m	MT20	5.0	5.0	2.25	1.25
F	TTW-t+m	MT20	3.0	4.0	2.00	1.25
G	BMVW-t	MT20	3.0	4.0	1.50	1.50
H	BMVW-t	MT20	4.0	5.0	1.50	2.00
I	BMV1+p	MT20	2.0	4.0		
J	BMVW-t	MT20	4.0	4.0	1.75	1.50
K	BS-t	MT20	3.0	6.0		
L	BMVWWW-t	MT20	3.0	8.0		
M	BMVW-t	MT20	3.0	4.0		
N	BMVW-t	MT20	4.0	4.0	1.75	1.50
O	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BUILDING BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
O	1372	0	1372	0	0	5-8	5-8
I	1273	0	1273	0	0	MECHANICAL	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT 1 TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	961	685 / 0	0 / 0	0 / 0	0 / 0	276 / 0	0 / 0
I	893	625 / 0	0 / 0	0 / 0	0 / 0	268 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.86 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1- 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF D-M, G-L. DBS = 20-0-0 . CBF = 59 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)		
FR-TO		FROM TO			FR-TO				
A-B	0 / 28	-77.4	-77.4	0.09 (1)	10.00	N-D	-70 / 88	0.03 (1)	
B-C	-1475 / 0	-77.4	-77.4	0.47 (1)	4.86	D-M	-472 / 0	0.22 (1)	
C-D	-1475 / 0	-77.4	-77.4	0.47 (1)	4.86	M-E	0 / 341	0.08 (1)	
D-E	-1108 / 0	-77.4	-77.4	0.43 (1)	5.46	E-L	0 / 22	0.00 (1)	
E-F	-899 / 0	-77.4	-77.4	0.03 (1)	6.25	L-F	0 / 365	0.08 (1)	
F-G	-1111 / 0	-77.4	-77.4	0.43 (1)	5.46	L-G	-467 / 0	0.22 (1)	
G-H	-1474 / 0	-77.4	-77.4	0.47 (1)	4.86	J-G	-75 / 86	0.04 (1)	
O-B	-1327 / 0	0.0	0.0	0.14 (1)	7.03	B-N	0 / 1270	0.29 (1)	
J-H	-1228 / 0	0.0	0.0	0.13 (1)	7.24	J-H	0 / 1269	0.29 (1)	
O-N	0 / 0	-17.5	-17.5	0.18 (4)	10.00				
N-M	0 / 1254	-17.5	-17.5	0.30 (1)	10.00				
M-L	0 / 897	-17.5	-17.5	0.22 (1)	10.00				
L-K	0 / 1253	-17.5	-17.5	0.29 (1)	10.00				
K-J	0 / 1253	-17.5	-17.5	0.29 (1)	10.00				
J-I	0 / 0	-17.5	-17.5	0.18 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= $L/360$ (0.89")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.05")
ALLOWABLE DEFL.(TL)= $L/360$ (0.89")

CSI: TC=0.47/1.00 (B-D:1), BC=0.30/1.00 (M-N:1),
WB=0.29/1.00 (B-N:1), SSI=0.20/1.00 (B-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY)		SHEAR		SECTION
	(PSI)		(PLI)		(PLI)
	MAX	MIN	MAX	MIN	MAX MIN
MT20	618	354	1667	822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.50 (B) (INPUT = 1.00)



**READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.**



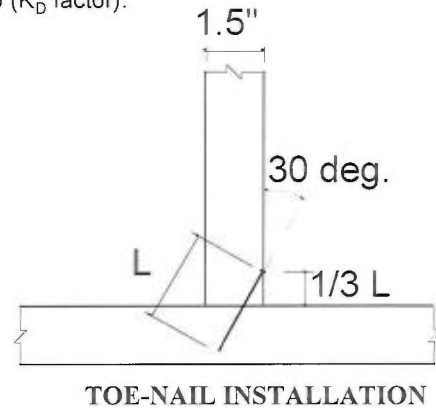
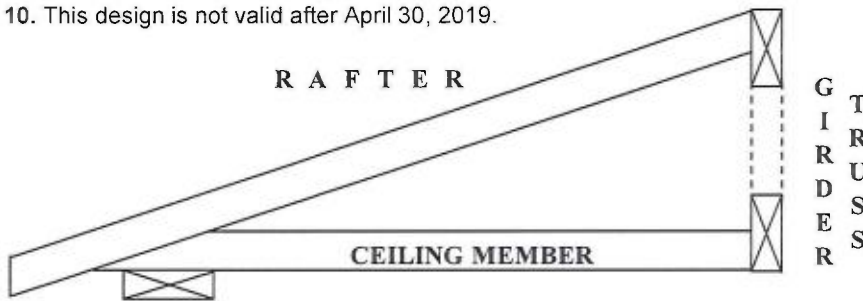
BEARING ANCHORAGE BY TOE-NAILS FOR LATERAL CAPACITY

B37579H1

NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL LATERAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	132	147
	3.25	0.144	132	147
	3.50	0.160	159	177

NOTES:

1. Rafter and ceiling members may be anchored to top and bottom chords of girder truss by toe-nailing rafter and ceiling members to girder chords provided the reaction does not exceed the lateral capacities in the table. Hangers (specified by others) are required for reactions higher than the maximum toe-nail capacity. Reactions are based on factored loads.
2. Toe nail capacities shown in the table are for one toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-09, section 10.9.4.1.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in tables below.
5. Nail values in table are based on the following relative lumber densities: G = 0.42 (SPF), G = 0.49 (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See next page for nailing on bearing plate).
7. For loads due to wind the nail lateral capacity in this table may be multiplied by 1.15 (K_D factor).
8. Lumber must be dry (< 19% moisture content) at the time of nail installation.
9. Nail values in this table comply with CSA O86-09, section 10.9.4
10. This design is not valid after April 30, 2019.



Nail type	Common wire	Common spiral	Common wire
Nail dia. (in)	0.160	0.152	0.144
	(3.5" nail)		(3" and 3.25" nail)
LUMBER SIZE	MAXIMUM NUMBER OF TOE-NAILS		
2X4 SPF	2	2	3
2X4 D. Fir	2	2	2
2X6 SPF	4	4	4
2X6 D. Fir	3	3	3



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Certificate No. 10889485



April 26, 2017

BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING

B37579H2

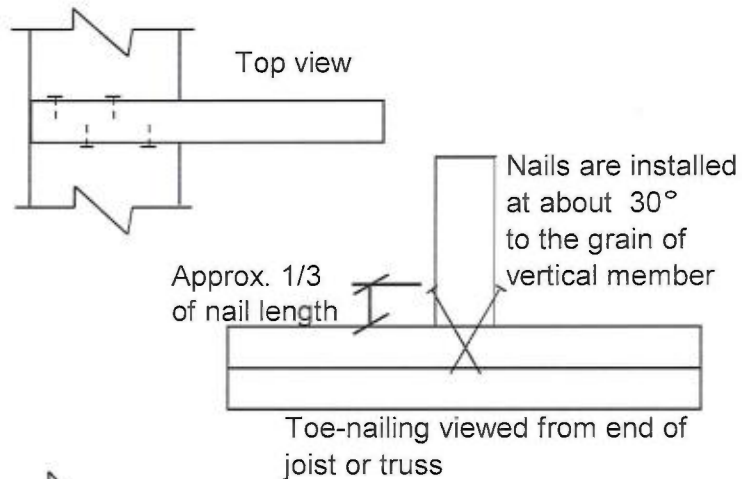
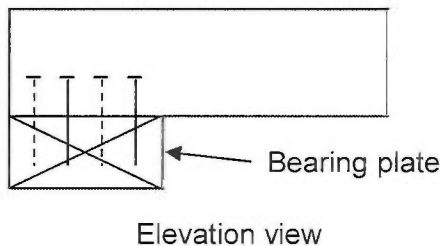
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	30	42
	3.25	0.144	32	45
	3.50	0.160	38	52

Note: If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

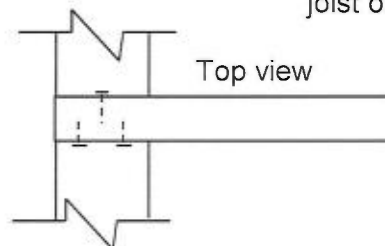
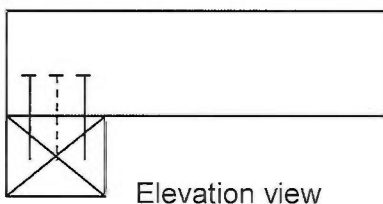
NOTES:

1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to **wind or earthquake** load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-09, section 10.9.5.2.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37579H1).
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-09, section 10.9.5
9. This design is not valid after April 30, 2019

Toe-nailing on 2x6 Bearing Plate



Toe-nailing on 2x4 Bearing Plate



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