Layout Name

Description

May 29, 2018

Created

Builder

Sales Rep

Designer

Shipping

6-0-0 Builder's Project

Canada

L4A 7X4

905-642-4400

Design Method

Deflection Joist

LL Span L/

TL Span L/

LL Cant 2L/

TL Cant 2L/

LL Span L/

TL Span L/

LL Cant 2L/

TL Cant 2L/

Decking

Thickness

Fastener

Vibration

Deck

Deflection Girder

Floor

Loads

Live

Dead

14 Anderson Rlvd

Stouffville, Ontario

\LIANA 1 (ELEV.2).isI

RCO

11 8-0-0 Project

LIANA 1 (ELEV.2)

GRANELLI HOME CORP.

GREEN YORK HOMES

Kott Lumber Company

D:\Users\rochavillo\WORK FROM HOME\GREEN YORK HOMES \GRANELLI HOME CORP\MODELS \LIANA 1\LIANA 1 ELEV 2\FLOOR

Building Code NBCC 2010 / OBC

LSD

2012

40

15

480

360

480

360

360

240

480

360

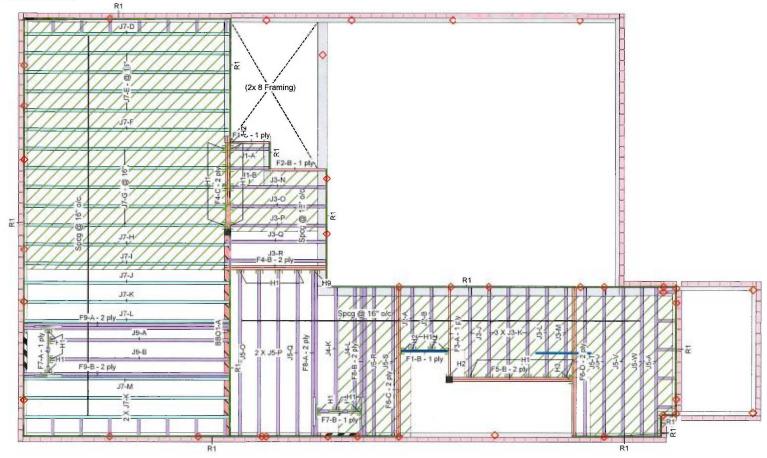
OSB

3/4"

Nailed & Glued

BRAMPTON, ONT.

Ground Floor



THIS CERTIFICATION IS TO CONFIRM THAT:

- 1. THE LOADS USED IN THE CALCULATION OF THE ATTACHED APPROVED COMPONENTS CONFORM TO THE FLOOR ASSEMBLY SHOWN ON THIS LAYOUT.
- 2. THE FLOOR JOISTS COMPLY WITH THE NASCOR SPAN TABLE FOR THE LOADS AND SPACING SHOWN ON THIS LAYOUT.

THE FLOOR SYSTEM MUST BE ASSEMBLED IN ACCORDANCE TO THE NASCOR SPECIFIER GUIDE. MULTI-PLY MEMBERS MUST BE ATTACHED TOGETHER AS PER THE INCLUDED MULTIPLE MEMBER CONNECTION DETAIL.

ALL OTHER COMPONENTS AND STRUCTURAL ELEMENTS SUPPORTING THE FLOOR SYSTEM SUCH AS BEAMS, WALLS, **COLUMNS AND FOUNDATION WALLS AND FOOTINGS** INCLUDING ANCHORAGE OF COMPONENTS AND BRACING FOR LATERAL STABILITY ARE THE RESPONSIBILITY OF OTHERS.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS. PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS







Load from Above Wall Wall Opening Norbord Rimboard Plus 1.125 X 9.5 NJ60H 9.5 NJH 9.5

- 1. OBC 2012 O.Reg 332/12 as amended
- 2. Nascor CCMC 13535-R
- 4. CAN/CSA-O86-09 Engineering/floor joists shall be installed
 5. CCMC -12787-R APPAPACOIDE to the supplier's layout a specifications forming part of the permit draw

Legend

- 3. LVL CCMC -14056-R

All work shall conform to the Building Gode O Reg. 382/12 as

2.0E-3000Fb LVL 1.75 9.5 Forex 2 8-0-0 Design Method 2.0E-3000Fb LVL F3 1.75 Forex 9.5 8-0-0 2.0E-3000Fb LVL F2 1.75 9.5 Forex 6-0-0 1 2.0E-3000Fb LVL F1 Forex 1.75 9.5 2 4-0-0 2.0E-3000Fb LVL Joist (Flush) Label Description Width Depth Qty Plies Pcs | Length J7 NJ60H 2.5 9.5 19 16-0-0 F9 NJH 2.5 9.5 16-0-0 F8 NJH 9.5 2 2 12-0-0 2.5 4 F7 NJH 2.5 9.5 4-0-0 J9 NJH 2.5 9.5 2 14-0-0 J5 NJH 2.5 9.5 11 12-0-0 J4 NJH 2.5 9.5 10-0-0

Width Depth

9.5

9.5

1.75

1.75

Qty

Plies

2

2

Pcs Length

12-0-0

10-0-0

4

2

2

2

J1 NJH 2.5 9.5 4-0-0 Rim Board Pcs Length Label Description Width Depth Qty Plies Norbord Rimboard 1.125 9.5 12 Plus 1.125 X 9.5

9.5

9.5

2.5

2.5

Hanger

J3 NJH

J2 NJH

Ground Floor LVL/LSL (Flush) Label Description

> Forex 2.0E-3000Fb LVL

Forex

F6

F5

					beam/Girder	Supported	303-042-4400	
						Member	Job Path	
Label	Pcs	Description	Skew	Slope	fasteners	fasteners	D:\Users\rochaville	
H1	30	LT259			4 10dx1 1/2	2 10dx1 1/2	HOME\GREEN YO	
H2	4	HUS1.81/10			30 16d	10 16d	\GRANELLI HOMI	
НЗ	1	HGUS410			46 16d	16 16d	LIANA 1\LIANA 1	
H9	1	THAI-2 (Min)			2 10d	2 10dx1 1/2	\LIANA 1 (ELEV.2)	
NOTES-			101				Ground Floor	

- . Framer to verify dimensions on the architectural drawings.
- 2. Double joist only require filler/backer ply when supporting
- another member using a face-mounted hanger. 3. Install 2x4 blocking @ 24" o/c under parallel non-load bearing walls
- I. Install single-ply flush window header along inside face of
- rimboard/rimjoist.
- Refer to Nascor specifier guide for installation works. 6. Squash blocks recommended to be installed at end bearing on all first level joists which support loading from above exceeding
- two levels floor or roof. . Load transfer blocks to be installed under all point loads.
- 3. It shall be the framer's responsibility that floor joists and beams are fastened as per the hanger manufacturer's standards.

Refer to Multiple Member Connection Detail to ply to ply nailing or bolting requirements.

Rim parallel to joists: 1-1/8" rimboard with 2"x 4" block (1/16" longer than rim depth @ 16" o/c). All other components and structural elements supporting the floor system such as beams, walls, columns, and foundation walls and footings including anchorage of components and cing for lateral stability are the responsibility of Others.

atch area represents ceramic tiled floor with an addtional dead load

The framing shown on this layout may deviate from the architectural and structural drawings. Project Engineer to review and approve the deviation prior to construction.

ARCHITECTURAL DRAWINGS:

JARDIN DESIGN GROUP INC. 64 Jardin Dr., Suite 3A, Vaughan, ON Date: Rev.2; May 22,2018 Project No: 17-55 Model: Liana 1

M-2057

19-447186.00.0D.RR.



LIANA 1 (ELEV.2) 4 BEDROOM

Layout Name

Pcs Length

16 16d

Second Floor

Design Method

Deflection Joist

LL Span L/

TL Span L/

LL Cant 2L/

TL Cant 2L/

LL Span L/

TL Span L/

LL Cant 2L/

TL Cant 2L/

Decking

Thickness

Fastener

Vibration

Deck

Deflection Girder

Floor

Loads

Live

Dead

Building Code NBCC 2010 / OBC

LSD

2012

40

15

480

360

480

360

360

240

480

360

OSB

5/8"

Nailed & Glued

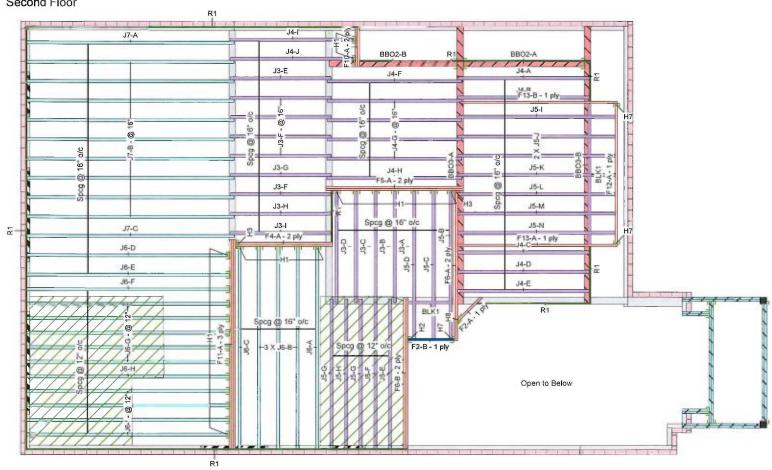
Gypsum 1/2"

46 16d

16-0-0

12-0-0

Second Floor



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PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS.





Load from Above Norbord Rimboard Plus 1,125 X 9,5

- 2. Nascor CCMC 13535-R
- 3. LVL CCMC -14056-R
- 5. CCMC -12787-R APA PR-L310(C)

Legend



Wall Opening NJ60H 9.5

- 1. OBC 2012 O.Reg 332/12 as amended

- 4. CAN/CSA-O86-09

F13	Forex 2.0E-300	0Fb LVL	1.3	75	9.5			2	12-0-0	Design Method	
F5	Forex 2.0E-300	0Fb LVL	1.7	75	9.5	1	2	2	10-0-0	Description	
F12	Forex 2.0E-300	0Fb LVL	1.	75	9.5			1	10-0-0	GRANELLI HOME CORP. BRAMPTON, ONT.	
F4	Forex 2.0E-300	0Fb LVL	1.7	75	9.5	1	2	2	8-0-0	Created May 29, 2018	
F2	Forex 2.0E-300	0Fb LVL	1.7	75	9.5			2	4-0-0	Builder	
F10	Forex 2,0E-300	0Fb LVL	1.7	75	9.5	1	2	2	4-0-0	GREEN YORK HOMES	
I Joist	(Flush)								-	Sales Rep	
Label	Descript	ion	Wid	th De	pth	Qty	Plies	Pcs	Length	RM	
J7	NJ60H		2	.5	9.5			11	16-0-0	Designer	
J6	NJ60H		2	.5	9.5			18	14-0-0	RCO	
J5	NJH		2	2.5	9.5			15	12-0-0	Shipping	
J4	NJH		2	.5	9.5			13	10-0-0	Project	
J3	NJH		2	.5	9.5			13	8-0-0	Builder's Project	
Rim Bo	pard						V				
Label	Descript	ion	Wid	th De	pth	Qty	Plies	Pcs	Length	Kott Lumber Company	
R1	Norbord I		1.12	25	9.5			10	12	14 Anderson Blvd	
	Plus 1.12	5 X 9.5								Stouffville, Ontario	
Blockin	-					_				Canada	
Label		ion	Wid		pth	Qty	Plies	Pcs	Length	L4A 7X4	
BLK1	NJH		2	.5	9.5	LinFt		Varies	11-0-0	905-642-4400	
Hange	r					Ве	am/Girde		ported ember	Job Path D:\Users\rochavillo\WORK FROM HOME\GREEN YORK HOMES	
Label	Pcs D	escriptio	n	Skew	Slop	e fa	asteners	fas	teners	\GRANELLI HOME CORP\MODELS	
H1	27 L	Г259					10dx1 1/2	2 10	dx1 1/2	\LIANA 1\LIANA 1 ELEV 2\FLOOR\4	
H2	1 H	US1.81/1	0				30 16d	1	0 16d	BEDRM\LIANA 1 (ELEV.2).isl	

Qty

2

Width Depth

9.5

9.5

1.75

1.75

Plies

2

H8 NOTES:

Н3

H4

Hanger

HUCQ1.81/9-

2 HGUS410

9 Unknown

1 LSSUI25

3

Second Floor LVL/LSL (Flush)

F11

F6

Label Description

Forex

Forex

2.0E-3000Fb LVL

2.0E-3000Fb LVL

- Framer to verify dimensions on the architectural drawings.
- 2. Double joist only require filler/backer ply when supporting another member using a face-mounted hanger
- 3. Install 2x4 blocking @ 24" o/c under parallel non-load bearing walls.

Right

- I. Install single-ply flush window header along inside face of
- rimboard/rimioist. . Refer to Nascor specifier guide for installation works.
- 6. Squash blocks recommended to be installed at end bearing on all first level joists which support loading from above exceeding two levels floor or roof.
- Load transfer blocks to be installed under all point loads.
- . It shall be the framer's responsibility that floor joists and beams are fastened as per the hanger manufacturer's standards.

Refer to Multiple Member Connection Detail to ply to ply nailing or bolting requirements.

Rim parallel to joists: 1-1/8" rimboard with 2"x 4" block (1/16" longer than rim depth @ 16" o/c). All other components and structural elements supporting the floor system such as beams, walls, columns, and foundation walls and footings including anchorage of components and bracing for lateral stability are the responsibility of Others.

Hatch area represents ceramic tiled floor with an addtional dead load

The framing shown on this layout may deviate from the architectural and structural drawings. Project Engineer to review and approve the deviation prior to construction.

ARCHITECTURAL DRAWINGS:

JARDIN DESIGN GROUP INC. 64 Jardin Dr., Suite 3A, Vaughan, ON Date: Rev.2: May 22,2018 Project No: 17-55 Model: Liana 1

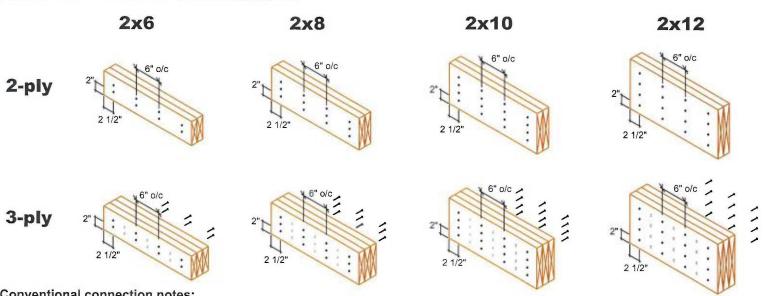
M-2057



TIPLE MEMBER CONNECTIONS

GRANELLI HOME CORP-LIANA 1 (ELEV.2

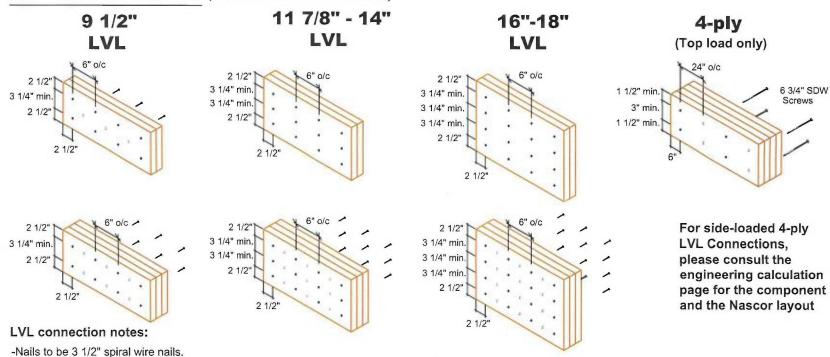
Conventional Connections (for uniform distributed loads)



Conventional connection notes:

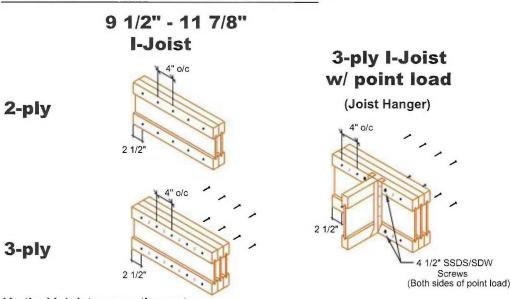
- -Nails to be 3" 10d spiral wire nails.
- -Nails to be located a minimum of 2" from the top and bottom of the member. Start all nails a minimum of 2 1/2" in from ends.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail driven from the opposite side.

LVL Connections (for uniform distributed loads)



- -Nails to be located a minimum of 2 1/2" from the top and bottom of the member. Start all nails a minimum of 2 1/2" in from ends.
- -Minimum 3 1/4" spacing between rows.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail or screw driven from the opposite side.

Vertical I-Joist Connections (for uniform distributed loads)



Vertical I-Joist connection notes:

- -Nails to be 3" spiral wire nails.
- -Nails to be located at centre of top and bottom flanges. Start all nails a minimum of 2 1/2" in from ends.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail driven from the opposite side.



MULTI-PLY CONNECTION **DETAILS**

Date: November 30, 2016

Scale: NTS

3228 Moodie Drive Ottawa, ON K2H 7V1

Ph: 613-838-2775 Fx: 613-838-4751

Engineering Note Page (ENP-2)

REVISION 2009-10-09

M-2057

LOT 29

Please read all notes prior to installation of the component

GREEN YORK HOMES-GRANELLI HOME CORP-LIANA 1 (ELEV.2

DESIGN INFORMATION

This building component is certified as an individual component for the loads and conditions shown on the calculation and drawing page.

The responsibility of the undersigned engineer is <u>only</u> limited to the calculation of this building component for the loads and conditions shown on this drawing.

The responsibility of the undersigned is limited to the verification of the structural capacity of the NASCOR floor joists and LVL beams based on placement as shown on the layout. The loads applied are limited to the gravity effects of the specified loads. The structural integrity of the building and the effect of wind, uplift, seismic, lateral or other forces, calculation of adequate support and anchorage of components, as well as the dimensions and design loads used to calculate components are the responsibility of the overall building designer.

Floor joists and OSB rim board are designed to carry uniformly distributed loads only. Point loads should be transferred through the floor cavity with squash blocks. Structural elements such as walls, posts, connectors, and squash blocks are the responsibility of the overall building designer.

The undersigned engineer disclaims any responsibility for damages as a result of being furnished faulty or incorrect information, specifications and/or designs.

Installation of NASCOR joists is to be carried out in accordance with the current edition of the manufacturer's approved literature available at http://www.nascor.ca.

CODE

This building component is designed in accordance with the National Building Code of Canada, the Ontario Building Code, CCMC and Canadian Standards Association guidelines.

COMPONENT

- 1. The building component used in construction must be the same as indicated on the drawings.
- 2. The building component must be installed and assembled as per specification shown on the drawing and in accordance with the manufacturer's assembly and installation.
- 3. Members consisting of multiple plies must be connected as per the document "Multi-ply Connection Details".
- 4. Pass-thru squash block framing is required at all point loads over bearings.

HANDLING AND INSTALLATION

Do not drill any hole, cut or notch a certified building component without a written preauthorization.



Page 1 of 1

EWP Studio Simpson Strong-Tie® Component Solutions™

Client:

GREEN YORK HOMES

Project: Address:

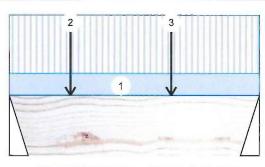
Date: 5/31/2018

RCO Designer:

Job Name: LIANA 1 (ELEV.2)

Forex 2.0E-3000Fb LVL

1.750" X 9.500" - PASSED Level: Ground Floor



1 Hanger (HUS1.81/10) 2 Hanger (HUS1.81/10) 3'3 11/16"

3'3 11/16"

Mind

Member Information

Type: Girden Plies: Moisture Condition: Dry Deflection LL: 360 Deflection TL: 240 Importance: Normal General Load

40 PSF 15 PSF

258 ft-lb

258 ft-lb

(L/45769)

(L/21849)

(L/14790)

274 lb

Application:

Location Allowed

2' 7/16" 11362 ft-lb

2' 7/16" 10006 ft-lb

1'9 1/4" 0.098 (L/360) 0.010 (1%) D

1'9" 0.098 (L/360) 0.020 (2%) L

1'9 1/16" 0.146 (L/240) 0.020 (2%) D+L

11 3/4" 4638 lb

Design Method: **Building Code:**

Capacity

LSD NBCC 2010 / OBC 2012 Load Sharing:

Vibration:

Not Checked Not Checked

Floor (Residential)

Unfactored Reactions UNPATTERNED lb (Uplift)

Big	Live	Dead	SHOW	vviria
1	171	80	0	0
2	152	73	0	0
1				

Bearings and Factored Reactions

	Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.	
	1 - Hanger	3.000"	9%	100 / 256	356	L	1.25D+1.5L	
	2 - Hanger	3.000"	8%	91 / 229	320	L	1.25D+1.5L	
_					-			

Comb. Case READ ALL NOTES ON THIS PAGE AND ON 0.023 (2%) 1.25D+1.5L L ENGINEERING NOTE PAGE ENP-2. THIS 0.026 (3%) 1.25D+1.5L L NOTE PAGE IS AN INTEGRAL PART OF THIS 0.059 (6%) 1.25D+1.5L L CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA Uniform USED IN THE DESIGN OF THIS COMPONENT. L

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Design Notes

Floor Live:

Moment

Unbraced

Shear

Analysis Results Analysis

Perm Defl in. 0.001

LL Defl inch 0.002

TL Defl inch 0.002

Dead:

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.
- 4 Bottom braced at bearings

4 DOLLOTTI	braced at bearings.								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Part. Uniform	0-0-0 to 3-3-11		Тор	15 PLF	40 PLF	0 PLF	0 PLF	
2	Point	0-9-11		Far Face	41 lb	90 lb	0 lb	0 lb	J2
3	Point	2-1-11		Far Face	50 lb	101 lb	0 lb	0 lb	J2
	Self Weight				4 PI F				

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. If is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- chemicals
- Handling & Installation LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- papprovals

 Damaged Beams must not be used

 Design assumes top edge is laterally restrained

 Provide lateral support at bearing points to avoid lateral displacement and rotation

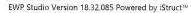
6. For flat roofs provide proper drainage to prevent

Manufacturer Info

Forex APA: PR-L318







Page 1 of 1

EWP Studio Simpson Strong-Tie® Component Solutions™

Client:

GREEN YORK HOMES

Project: Address:

5/31/2018

RCO

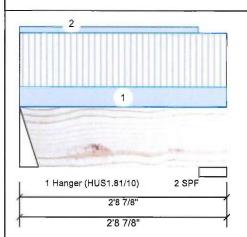
Designer: Job Name: LIANA 1 (ELEV.2)

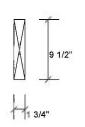
Project #:

Date:

Forex 2.0E-3000Fb LVL

1.750" X 9.500" - PASSED Level: Ground Floor





	Member Information								
	Туре:	Girder							
	Plies:	1							
	Moisture Condition:	Dry							
	Deflection LL:	360							
	Deflection TL:	240							
ı	Importance:	Normal							
	General Load								
	Floor Live:	40 PSF							

15 PSF

Application: Design Method: **Building Code:** Load Sharing:

Deck:

Vibration:

Floor (Residential) NBCC 2010 / OBC 2012 No Not Checked Not Checked

Brg 1 2

Unfactored Reactions UNPATTERNED Ib (Uplift) Live Dead Snow Wind 12 11 0 0 11 0 0 13

Bearings and Factored Reactions

Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 -	3.000"	1%	13 / 17	31	L	1.25D+1.5L
Hanger						
2 - SPF	4.375"	1%	14 / 19	33	L	1.25D+1.5L

Analysis Results

Floor Live: Dead:

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	15 ft-lb	1'3 3/4"	11362 ft-lb	0.001 (0%)	1.25D+1.5L	L
Unbraced	15 ft-lb	1'3 3/4"	10562 ft-lb	0.001 (0%)	1.25D+1.5L	L
Shear	8 lb	1'7 3/4"	4638 lb	0.002 (0%)	1.25D+1.5L	L
Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
LL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
TL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		

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PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.
- 4 Bottom braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 2-8-14	(Span)0-5-5	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Part. Uniform	0-0-0 to 2-4-6		Тор	1 PLF	0 PLF	0 PLF	0 PLF	
	Self Weight				4 PLF				

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

- Dry service conditions, unless noted otherwise
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- chemicals
- Handling & Installation
- LVL beams must not be cut or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
- approvers
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

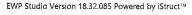
For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318









Client:

GREEN YORK HOMES

Project: Address:

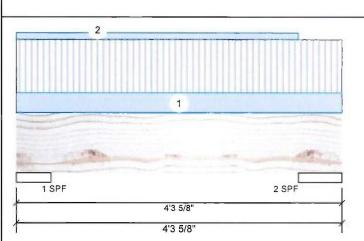
5/31/2018

RCO

Designer: Job Name: LIANA 1 (ELEV.2)

Date:

1.750" X 9.500" - PASSED Level: Ground Floor F2-B Forex 2.0E-3000Fb LVL



Page 1 of 1

Member Information

Type:	Girder
Plies:	1
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	

40 PSF

15 PSF

Application: Design Method: **Building Code:** Load Sharing:

> Deck: Vibration:

Floor (Residential) NBCC 2010 / OBC 2012

Not Checked Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	vviria
1	50	33	0	0
2	53	33	0	0

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.	
1 - SPF	5.500"	2%	41 / 76	117	L	1.25D+1.5L	
2 - SPF	6.875"	2%	42 / 80	121	L	1.25D+1.5L	

Analysis Results

Floor Live:

Dead:

I	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case	_
l	Moment	81 ft-lb	2'1 1/8"	11362 ft-lb	0.007 (1%)	1.25D+1.5L	L	
	Unbraced	81 ft-lb	2'1 1/8"	9540 ft-lb	0.008 (1%)	1.25D+1.5L	L	
	Shear	51 lb	3'	4638 lb	0.011 (1%)	1.25D+1.5L	L	
Į	Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)			
l	LL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)			
١	TL Defl inch	0.001 (L/46449)	2'1 3/16"	0.170 (L/240)	0.010 (1%)	D+L	L	

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Top braced at bearings,
- 3 Bottom braced at bearings.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 4-3-10	(Span)1-2-7	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Part. Uniform	0-0-0 to 3-8-12		Тор	3 PLF	0 PLF	0 PLF	0 PLF	
	Self Weight				4 PLF				

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the untended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

LVL beams must not be cut or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
 Damaged Beams must not be used.

Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318







Client:

GREEN YORK HOMES

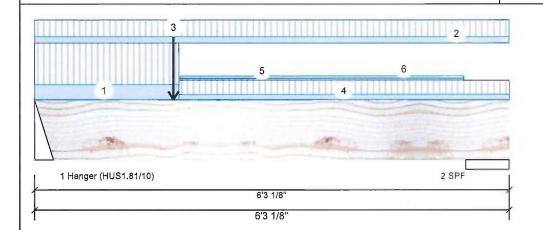
Project: Address: Date:

5/31/2018

RCO Designer:

Job Name: LIANA 1 (ELEV.2)

Level: Ground Floor 1.750" X 9.500" - PASSED F3-A Forex 2.0E-3000Fb LVL



	Member Inform	nation
	Туре:	Girder
	Plies:	1
	Moisture Condition:	Dry
	Deflection LL:	360
I	Deflection TL:	240
	importance:	Normal
	General Load	
	Floor Live:	40 PSF

Application: Floor (Residential) Design Method: LSD Building Code: NBCC 2010 / OBC 2012 Load Sharing: Deck: Not Checked

Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift) Live Dead Snow Wind 340 159 0 0 1 233 122 0 0 2

40 PS 15 PSF Dead: **Analysis Results**

Vibration:

Bearings and Factored Reactions

_							
Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.	
1 -	3.000"	18%	199 / 511	710	L	1.25D+1,5L	
Hanger							
2 - SPF	6.875"	7%	152 / 349	501	L	1.25D+1.5L	

Analysis Actual Location Allowed Capacity Comb. Case Moment 839 ft-lb 1'11 7/8" 11362 ft-lb 0.074 (7%) 1.25D+1.5L L Unbraced 839 ft-lb 1'11 7/8" 6701 ft-lb 0.125 (13%) 1.25D+1.5L L Shear 514 lb 11 3/4" 4638 lb 0.111 (11%) 1.25D+1.5L L 2'9 1/8" 0.185 (L/360) 0.030 (3%) D Perm Defl in. 0.006 Uniform (L/11950) LL Defl inch 0.011 (L/6054) 2'8 9/16" 0.185 (L/360) 0.060 (6%) L TL Defl inch 0.017 (L/4019) 2'8 13/16" 0.278 (L/240) 0.060 (6%) D+L L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.
- 4 Bottom braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-10-15	(Span)3-6-5	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 6-3-2	(Span)1-5-2	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	1-10-1		Far Face	73 lb	152 lb	dI 0	0 lb	F1
4	Tie-In	1-10-15 to 6-3-2	(Span)1-2-14	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
5	Part. Uniform	1-11-2 to 5-8-0		Тор	3 PLF	0 PLF	0 PLF	0 PLF	
6	Part. Uniform	1-11-2 to 5-8-0		Тор	4 PLF	0 PLF	0 PLF	0 PLF	
	Self Weight				4 PLF				

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- amaged Beams must not be used
- Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318







Client:

Address:

GREEN YORK HOMES Project:

Date: 5/31/2018

RCO Job Name: LIANA 1 (ELEV.2)

Project #:

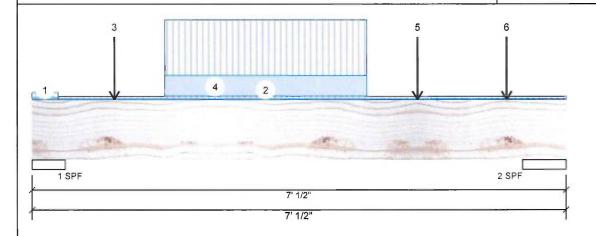
Designer:

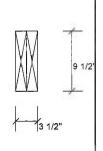
Forex 2.0E-3000Fb LVL

1.750" X 9.500"

2-Ply - PASSED

Level: Ground Floor





Wind

Page 1 of 1

Member Information

Type: Plies: Moisture Condition: Dry Deflection LL: 360 Deflection TL: 240 Importance: Normal General Load 40 PSF Floor Live:

15 PSF

Application: Design Method:

Building Code:

Floor (Residential)

NBCC 2010 / OBC 2012 No

Load Sharing: Deck: Not Checked

Not Checked Vibration:

Unfactored Reactions UNPATTERNED lb (Uplift)

1	669	277	0	0
2	862	351	0	0

Snow

Dead

Bearings and Factored Reactions

Bearing	Length	Сар. К	eact D/L ib	iotai	Ld. Case	La. Comb.
1 - SPF	5.250"	12%	346 / 1004	1350	L	1.25D+1.5L
2 - SPF	6.875"	12%	439 / 1293	1732	L	1.25D+1.5L

Analysis Results

Dead:

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2221 ft-lb	3'5 13/16"	22724 ft-lb	0.098 (10%)	1.25D+1.5L	L
Unbraced	2221 ft-lb	3'5 13/16"	21975 ft-lb	0.101 (10%)	1.25D+1.5L	L
Shear	1694 lb	5'8 7/8"	9277 lb	0.183 (18%)	1.25D+1.5L	L
Perm Defl	in. 0.008 (L/9580)	3'5 5/8"	0.205 (L/360)	0.040 (4%)	D	Uniform
LL Defl inc	h 0.019 (L/3901)	3'5 5/8"	0.205 (L/360)	0.090 (9%)	L	L
TL Defl inc	h 0.027 (L/2772)	3'5 5/8"	0.308 (L/240)	0.090 (9%)	D+L	L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on full section width.

L	o Lateral Stelluel	ness ratio based on run	Section width.							
Γ	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
l	1	Tie-In	0-0-0 to 0-4-2	(Span)1-0-10	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
l	2	Tie-In	0-4-2 to 7-0-8	(Span)0-5-14	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
l	3	Point	1-1-1		Near Face	88 lb	234 lb	0 lb	0 lb	J5
l	4	Part. Uniform	1-9-1 to 4-5-1		Near Face	86 PLF	230 PLF	0 PLF	0 PLF	
l	5	Point	5-1-1		Near Face	108 lb	288 lb	0 lb	0 lb	J5
l	6	Point	6-3-2		Near Face	122 lb	323 lb	0 lb	0 lb	F8
l		Self Weight				8 PLF				

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

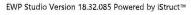
For flat roofs provide proper dramage to prevent ponding

Manufacturer Info

APA: PR-L318







Page 1 of 2

EWP Studio Simpson Strong-Tie® Component Solutions™ Client:

GREEN YORK HOMES

Project: Address: Date: 5/31/2018

Designer: RCO

Job Name: LIANA 1 (ELEV.2)

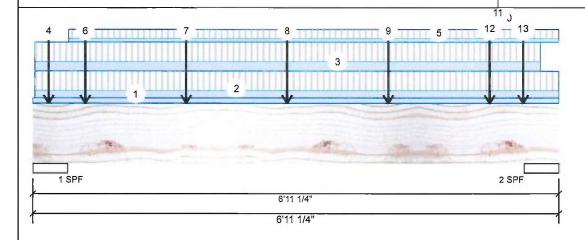
Project #:

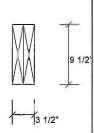
Forex 2.0E-3000Fb LVL

1.750" X 9.500"

2-Ply - PASSED

Level: Ground Floor





Member Information

Туре: Girden Plies: 2 Moisture Condition: Dry Deflection LL: 360 Deflection TL: 240 Importance: Normal General Load 40 PSF

15 PSF

Application: Design Method:

Floor (Residential) LSD

Building Code:

NBCC 2010 / OBC 2012

Load Sharing: Deck:

Not Checked Vibration: Not Checked

Unfactored Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	VVind
1	5607	2698	0	0
2	2555	1390	0	0

Bearings and Factored Reactions

Cap. React D/L lb Total Ld. Case Ld. Comb. Bearing Length 1.25D+1.5L 1-SPF 5.500" 99% 3372 / 8411 11783 L 1.25D+1.5L 2 - SPF 5.500" 47% 1737 / 3833 5570 L

> Comments Wall Self Weight

J7

JЗ

F11 F11

0 lb

0 PLF

Analysis Results

Floor Live:

Dead:

ı	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
	Moment	7859 ft-lb	3'4 11/16"	22724 ft-lb	0.346 (35%)	1.25D+1.5L	L
l	Unbraced	7859 ft-lb	3'4 11/16"	21978 ft-lb	0.358 (36%)	1.25D+1.5L	L
	Shear	4616 lb	1'2 1/4"	9277 lb	0.498 (50%)	1.25D+1.5L	L
	Perm Defl in.	0.033 (L/2213)	3'5 7/16"	0.205 (L/360)	0.160 (16%)	D	Uniform
l	LL Defl inch	0.062 (L/1195)	3'5 3/8"	0.205 (L/360)	0.300 (30%)	L	L
L	TL Defl inch	0.095 (L/776)	3'5 3/8"	0.307 (L/240)	0.310 (31%)	D+L	L
_							

Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3), Assumed point load size; beam width X 4.5.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



	7 Lateral slenderness ratio based on full section width.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind
1	Part. Uniform	0-0-0 to 6-11-4		Тор	80 PLF	0 PLF	0 PLF	0 PLF
2	Part. Uniform	0-0-5 to 6-11-4		Тор	106 PLF	283 PLF	0 PLF	0 PLF
3	Part. Uniform	0-0-5 to 6-8-5		Far Face	136 PLF	280 PLF	0 PLF	0 PLF

Top

Тор

Continued on page 2...

5

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Point

Part. Uniform

Lumber

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- chemicals

0-5-11 to 6-11-4

Handling & Installation

0-2-8

- 1. IVI. beams must not be cut or drilled
 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strangth values, and code approvals
 3. Damaged Beams must not be used
 4. Design assumes top edge is laterally restrained
 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

1254 lb

50 PLF

2945 lb

133 PLF

Manufacturer Info

APA: PR-L318

0 lb

0 PLF





Project: Address:

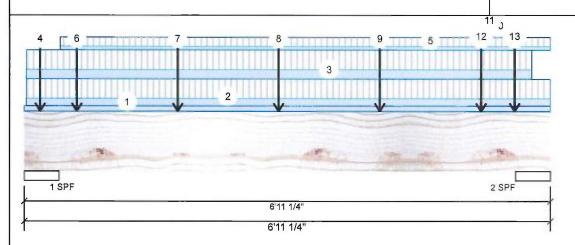
GREEN YORK HOMES

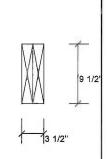
5/31/2018 Date:

RCO Designer:

Job Name: LIANA 1 (ELEV.2)

Level: Ground Floor F4-C Forex 2.0E-3000Fb LVL 1.750" X 9.500" 2-Ply - PASSED





Continued fr	rom page 1								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
6	Point	0-8-5		Near Face	73 lb	154 lb	dl 0	0 lb	J3
7	Point	2-0-5		Near Face	82 lb	167 lb	0 lb	0 lb	J3
8	Point	3-4-5		Near Face	29 lb	59 lb	0 lb	0 lb	J3
9	Point	4-8-5		Near Face	32 lb	68 lb	0 lb	0 lb	J1
10	Tie-In	6-0-5 to 6-6-8	(Span)2-11-3	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
11	Part. Uniform	6-0-5 to 6-5-1		Тор	7 PLF	0 PLF	0 PLF	0 PLF	
12	Point	6-0-5		Near Face	22 lb	45 lb	0 lb	0 lb	J1
13	Point	6-5-10		Near Face	11 lb	12 lb	0 lb	0 lb	F1
	Self Weight				8 PLF				
1									

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS.

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- Handling & Installation

 1. LVIL beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at beaming points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex APA: PR-L318





Client: Project: Address: **GREEN YORK HOMES**

5/31/2018

RCO Designer:

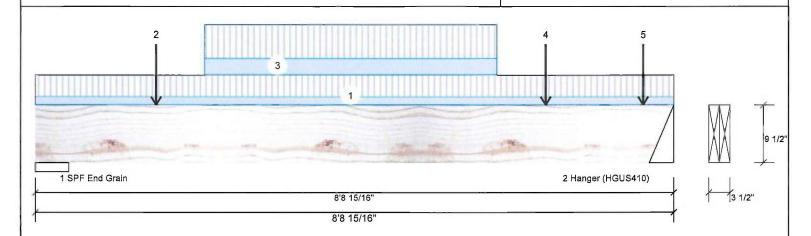
Job Name: LIANA 1 (ELEV.2)

Forex 2.0E-3000Fb LVL

1.750" X 9.500"

2-Ply - PASSED

Level: Ground Floor



Member Information Type: Girder Application: Floor (Residential) Plies: 2 Design Method: LSD Moisture Condition: Dry **Building Code:** NBCC 2010 / OBC 2012 Deflection LL: 360 Load Sharing: Deflection TL: Not Checked 240 Importance: Normal Vibration: Not Checked General Load Floor Live: 40 PSF Dead: 15 PSF

Location Allowed

7'8 3/16" 9277 lb

4'7 3/4" 22724 ft-lb

4'7 3/4" 21435 ft-lb

4'5 15/16" 0.269 (L/360) 0.080 (8%) D

4'6 3/16" 0.269 (L/360) 0.180 (18%) L

4'6 1/16" 0.404 (L/240) 0.170 (17%) D+L

Capacity

Comb.

0.161 (16%) 1.25D+1.5L L

0.171 (17%) 1.25D+1.5L L

0.241 (24%) 1.25D+1.5L L

Case

Uniform

L

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	795	377	0	0
2	1170	525	0	0

Bearings and Factored Reactions

l	Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.	
	1 - SPF End Grain	5.500"	12%	471 / 1192	1663	L	1.25D+1.5L	
l	2 -	4.000"	23%	656 / 1755	2411	L	1.25D+1.5L	

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS



Design Notes

Analysis Results Analysis

Moment

Unbraced

Shear

1 Fill all hanger nailing holes.

Perm Defl in. 0.022 (L/4332)

LL Defl inch 0.048 (L/2031)

TL Defl inch 0.070 (L/1383)

3658 ft-lb

3658 ft-lb

- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on full section width.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 8-8-15	(Span)3-10-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	1-7-14		Far Face	78 lb	162 lb	0 lb	0 lb	J3
3	Part. Uniform	2-3-14 to 6-3-14		Far Face	57 PLF	117 PLF	0 PLF	0 PLF	
4	Point	6-11-14		Far Face	162 lb	386 lb	0 lb	0 lb	J3
5	Point	8-3-14		Far Face	114 lb	275 lb	0 lb	0 lb	J3
	Self Weight				8 PLF				

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- chemicals Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's produ
- DVI. beams must not be cut or drillied Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals Damaged Beams must not be used Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding









INEUU 10-UZU FAGE 12 UF 41 Page 1 of 2

EWP Studio Simpson Strong-Tie® Component Solutions™

Client: Project:

Address:

GREEN YORK HOMES

5/31/2018 Date:

RCO Designer:

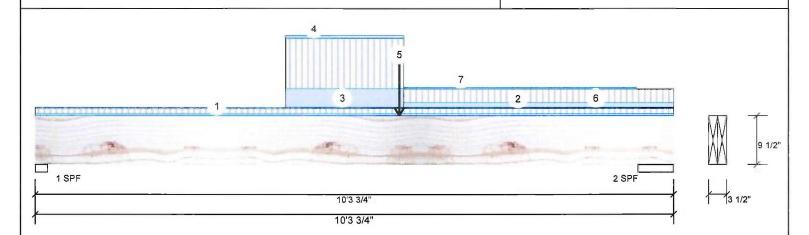
Job Name: LIANA 1 (ELEV.2)

Forex 2.0E-3000Fb LVL

1.750" X 9.500"

2-Ply - PASSED

Level: Ground Floor



Member Information Unfactored Reactions UNPATTERNED lb (Uplift) Type: Girder Application: Floor (Residential) Live Dead Snow Wind Plies: 2 Design Method: LSD 1 188 125 0 Moisture Condition: Dry **Building Code:** NBCC 2010 / OBC 2012 279 169 0 0 2 Deflection LL: 360 Load Sharing: Deflection TL: 240 Not Checked Vibration: Importance: **Not Checked** Normal General Load Floor Live: 40 PSF **Bearings and Factored Reactions** Dead: 15 PSF Cap. React D/L lb Total Ld. Case Ld. Comb. Bearing Length 1 - SPF 2.375" 9% 156 / 282 439 L 1.25D+1.5L 1.25D+1.5L 2 - SPF 6.875" 4% 212 / 418 629 L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1837 ft-lb	5'10 11/16"	22724 ft-lb	0.081 (8%)	1.25D+1.5L	L
Unbraced	1837 ft-lb	5'10 11/16"	20878 ft-lb	0.088 (9%)	1.25D+1.5L	L
Shear	546 lb	9' 1/8"	9277 lb	0.059 (6%)	1.25D+1.5L	L
Perm Defl in.	0.016 (L/7265)	5'2 1/8"	0.322 (L/360)	0.050 (5%)	D	Uniform
LL Defl inch	0.027 (L/4220)	5'2 11/16"	0.322 (L/360)	0.090 (9%)	L	L
TL Defl inch	0.043 (L/2670)	5'2 7/16"	0.483 (L/240)	0.090 (9%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on full section width.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



O Lateral sie	ilueilless latto baseu	on full section width.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 10-3-12	(Span)0-4-9	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Part. Uniform	0-0-0 to 9-8-9		Тор	1 PLF	0 PLF	0 PLF	0 PLF	
3	Tie-In	4-0-10 to 5-11-9	(Span)3-6-5	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
4	Part. Uniform	4-0-10 to 5-11-9		Тор	4 PLF	0 PLF	0 PLF	0 PLF	
5	Point	5-10-11		Near Face	80 lb	171 lb	0 lb	0 lb	F1
6	Tie-In	5-11-9 to 10-3-12	(Span)0-11-7	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

Continued on page 2...

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- chemicals
- Handling & Installation
- 1. IVL beams must not be out or drilled
 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
 3. Damaged Beams must not be used
 4. Design assumes top edge is laterally restrained
 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318







FAGE 13 UF 41 Page 2 of 2

EWP Studio Simpson Strong-Tie® Component Solutions™

Project:

Address:

GREEN YORK HOMES

Date: Designer: 5/31/2018

RÇO

Job Name: LIANA 1 (ELEV.2)

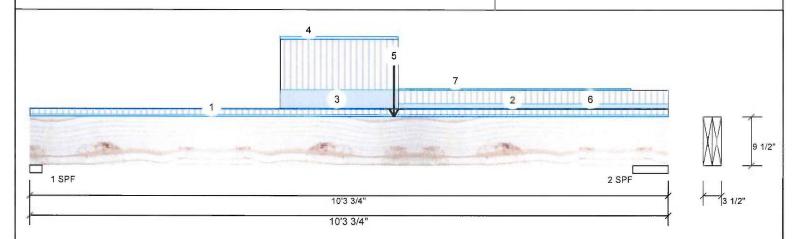
Project #:

Forex 2.0E-3000Fb LVL

1.750" X 9.500"

2-Ply - PASSED

Level: Ground Floor



.Continued from page 1

ID Load Type 7 Part. Uniform

Self Weight

Location Trib Width 5-11-9 to 9-8-9

Side Top

Dead 2 PLF

8 PLF

Live 0 PLF Snow 0 PLF Wind Comments

0 PLF

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS



Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design ortheria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

chemicals

- andling & Installation.

 LVL beams must not be cut or drilled.
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals.

 Damaged Beams must not be used.
 Design assumes top edge is laterally restrained.
 Provide lateral support at beaming points to avoid lateral displacement and rotation.

6. For flat roofs provide proper drainage to prevent

Manufacturer Info

APA: PR-L318





Client: Project: Address: **GREEN YORK HOMES**

5/31/2018

Designer: RCO

Job Name: LIANA 1 (ELEV.2)

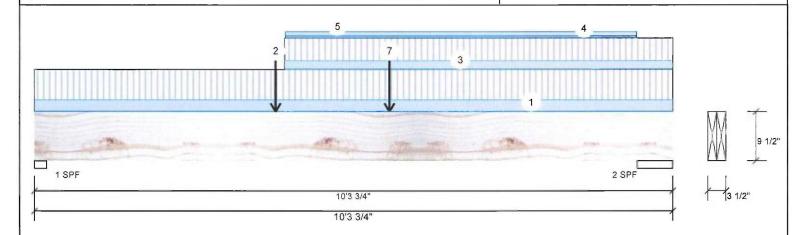
Project #:

Forex 2.0E-3000Fb LVL F6-D

1.750" X 9.500"

2-Ply - PASSED

Level: Ground Floor



Member Infor	mation			Unfactored Reactions UNPATTERNED Ib (Uplift)					
Туре:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind	
Plies:	2	Design Method:	LSD	1	876	424	0	0	
Moisture Conditio	n: Dry	Building Code:	NBCC 2010 / OBC 2012	2	683	342	0	0	
Deflection LL:	360	Load Sharing:	No	- 01					
Deflection TL:	240	Deck:	Not Checked						
Importance:	Normal	Vibration:	Not Checked						
General Load									
Floor Live:	40 PSF			Bearings	and Fact	ored Reactions			
Dead:	15 PSF			Bearing	Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.	
				1 - SPF :	2.375"	36% 530 / 1315	1845 L	1.25D+1.5L	
				2 - SPF (6.875"	10% 428 / 1024	1452 L	1.25D+1.5L	

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	6654 ft-lb	3'10 7/8"	22724 ft-lb	0.293 (29%)	1.25D+1.5L	L
Unbraced	6654 ft-lb	3'10 7/8"	20878 ft-lb	0.319 (32%)	1.25D+1.5L	L
Shear	1808 lb	11 1/8"	9277 lb	0.195 (19%)	1,25D+1,5L	L
Perm Defl in.	0.048 (L/2440)	4'7 3/4"	0.322 (L/360)	0.150 (15%)	D	Uniform
LL Defl inch	0.100 (L/1158)	4'7 9/16"	0.322 (L/360)	0.310 (31%)	L	L
TL Defl inch	0.148 (L/785)	4'7 11/16"	0.483 (L/240)	0.310 (31%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on full section width

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



Page 1 of 2

L	6 Lateral siende	rness ratio based or	full section width.							
	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
l	1	Tie-In	0-0-0 to 10-3-12	(Span)0-9-3	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
l	2	Point	3-10-14		Far Face	525 lb	1170 lb	0 lb	0 lb	F5
l	3	Tie-In	4-0-10 to 10-3-12	(Span)0-6-13	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
l	4	Part. Uniform	4-0-13 to 9-8-12		Тор	1 PLF	0 PLF	0 PLF	0 PLF	
l	5	Part. Uniform	4-0-13 to 9-8-12		Тор	2 PLF	0 PLF	0 PLF	0 PLF	
١	6	Point	5-8-15		Тор	25 lb	68 lb	0 lb	0 lb	
1.		_								

Continued on page 2...

Notes

Calculated Structured Designs is responsible only of the calculated subcured beagins is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- chemicals
- Handling & Installation
- andling & Installation
 LVL beams must not be cut or drillled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
 approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at beaming points to avoid
 lateral displacement and rotation

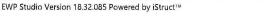
6. For flat roofs provide proper drainage to prevent

Manufacturer Info

Forex APA: PR-L318







Client: Project: Address:

GREEN YORK HOMES

5/31/2018

RCO

Job Name: LIANA 1 (ELEV.2)

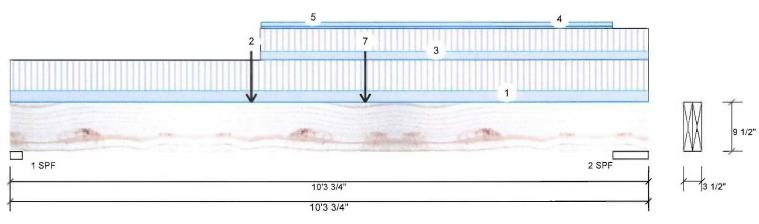
Project #:

Designer:

F6-D Forex 2.0E-3000Fb LVL 1.750" X 9.500"

2-Ply - PASSED

Level: Ground Floor



.Continued from page 1

ID Load Type 7 Point

Self Weight

Location Trib Width 5-8-15

Side Top

Dead 35 lb

8 PLF

Live 92 lb Snow 0 lb Wind Comments

0 lb

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

chemicals

Handling & Installation

- andling & Installation
 LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multipply
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 Damaged Beams must not be used
 Design assumes top udge is laterally restrained
 Provide lateral support at beaming points to avoid
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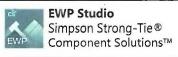
6. For flat roofs provide proper drainage to prevent

Manufacturer Info

Forex APA: PR-L318







Client: Project: Address:

GREEN YORK HOMES

5/31/2018 Date:

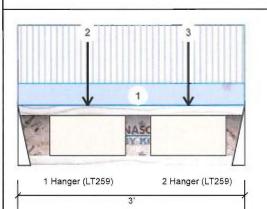
Designer: RCO

Job Name: LIANA 1 (ELEV.2)

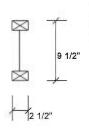
Level: Ground Floor

Project #:

9.500" - PASSED F7-A NJH



3'



Page 1 of 1

Member	Information
INCHINCE	momation

Type:	Girder	Application:	Floor (Residential)
Plies:	1	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

Unfactored Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	Wind
1	331	124	0	0
2	358	134	0	0

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	505 ft-lb	11 1/16"	3830 ft-lb	0.132 (13%)	1.25D+1.5L	L
Unbraced	505 ft-lb	11 1/16"	3411 ft-lb	0.148 (15%)	1.25D+1.5L	L
Shear	698 lb	2'10 3/4"	1580 lb	0.442 (44%)	1.25D+1.5L	L
Perm Defl in.	0.003 (L/12051)	1'3 15/16"	0.093 (L/360)	0.030 (3%)	D	Uniform
LL Defl inch	0.007 (L/4523)	1'3 15/16"	0.093 (L/360)	0.080 (8%)	L	L
TI Deflinch	0.010 (L/3288)	1'3 15/16"	0.140 (1/240)	0.070 (7%)	D+I	1

Bearings and Factored Reactions

	Comb.											
1 - 2.000" 41% 155 / 497 652 L 1.2 Hanger	5D+1.5L											
2 - 2.000" 45% 168 / 537 705 L 1.2 Hanger	5D+1.5L											

ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

READ ALL NOTES ON THIS PAGE AND ON

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top flange unbraced
- 4 Bottom flange braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 3-0-0	(Span)1-8-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	0-11-1		Near Face	114 lb	304 lb	0 lb	0 lb	J9
3	Point	2-3-1		Near Face	106 lb	282 lb	0 lb	0 lb	J9

Notes

Calculated Structured Designs is responsible only of the Calculated Substance Designs is responsible only or the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

- Handling & Installation

chemicals

- Andling & Installation
 | Joist flanges must not be cut or drilled
 | Refer to latest copy of the Libest product information
 | details for framing details, stiffener tables, web hole
 | that, bridging details, multi-hip/fastening details and
 | handling/erection details
 | Damaged Libests must not be used
 | Design assumes top flange to be laterally restrained
 | by attached sheathing or as specified in engineering
 | notes.

- 5. Provide lateral support at bearing points to avoid
- lateral displacement and rotation

 6. Web stiffeners for point load as shown Minimum point load bearing length>= 3,5 inches

 7. For flat roofs provide proper drainage to prevent
- ponding

Manufacturer Info

Nascor by Kott









Client:

GREEN YORK HOMES

Project: Address: Date:

5/31/2018

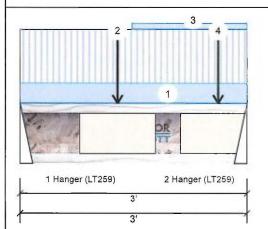
Level: Ground Floor

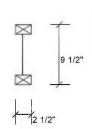
Designer:

Job Name: LIANA 1 (ELEV.2)

Project #:

9.500" - PASSED F7-B NJH





Page 1 of 1

Member Information								
Туре:	Girder	Application:	Floor (Residential)					
Plies:	1	Design Method:	LŞD					
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012					
Deflection LL:	360	Load Sharing:	No					
Deflection TL:	240	Deck:	Not Checked					
Importance:	Normal	Vibration:	Not Checked					
General Load								
Floor Live:	40 PSF							

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind	
1	200	82	0	0	
2	290	131	0	0	

Analysis Results

15 PSF

Dead:

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	420 ft-lb	1'3 7/16"	3830 ft-lb	0.110 (11%)	1.25D+1.5L	L
Unbraced	420 ft-lb	1'3 7/16"	3411 ft-lb	0.123 (12%)	1.25D+1.5L	L
Shear	591 lb	2'10 3/4"	1580 lb	0.374 (37%)	1.25D+1.5L	L
Perm Defl in.	0.002 (L/13745)	1'3 7/16"	0.093 (L/360)	0.030 (3%)	D	Uniform
LL Defl inch	0.006 (L/5760)	1'3 7/16"	0.093 (L/360)	0.060 (6%)	L	L
TL Defl inch	0.008 (L/4059)	1'3 7/16"	0.140 (L/240)	0.060 (6%)	D+L	L

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.	
1 - Hanger	2.000"	26%	103 / 300	403	L	1.25D+1.5L	
2 - Hanger	2.000"	38%	164 / 435	599	L	1.25D+1.5L	
_							

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top flange unbraced.
- 4 Bottom flange braced at bearings

ı	· Dotto	Brace at bearings.	_							
I	1D	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
I	1	Tie-In	0-0-0 to 3-0-0	(Span)1-8-11 to 1-8-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
I	2	Point	1-3-7		Far Face	94 lb	230 lb	0 lb	0 lb	J4
I	3	Part. Uniform	1-5-12 to 3-0-0		Тор	4 PLF	0 PLF	0 PLF	0 PLF	
I	4	Point	2-7-7		Far Face	75 lb	156 lb	0 lb	0 lb	J4

Notes

Calculated Structured Designs is responsible only of the Handling & Installation Structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 Upost not to be treated with fire retardant or corrosive

- andling & Installation

 Jost flanges must not be cut or drilled

 Refer to latest copy of the boost product information
 details for framing details, stiffener tables, web note
 chart, bridging details, multi-ply fastening details and
 handling/erection details

 Damaged bloists must not be used

 Design assumes top flange to be laterally restrained
 by attached sheathing or as specified in engineering
 notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation.
 Web stitleners for point bad as shown Minimum point load bearing length>= 3.5 inches
 For flat roots provide proper drainage to prevent ponding

Manufacturer Info

Nascor by Kott







Client:

GREEN YORK HOMES

Project: Address: Date: 5/31/2018

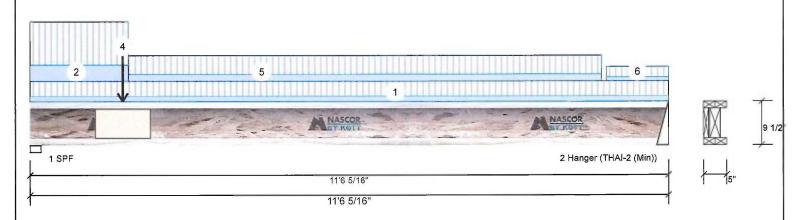
Designer: RCO

Job Name: LIANA 1 (ELEV.2)

Project #:

2-Ply - PASSED F8-A NJH 9.500"

Level: Ground Floor



Member Information Unfactored Reactions UNPATTERNED lb (Uplift) Wind Girder Application: Floor (Residential) Live Dead Snow Type: Brg Design Method: 210 0 Plies: LSD 543 2 1 Moisture Condition: Dry **Building Code:** NBCC 2010 / OBC 2012 2 323 122 0 Deflection II: 360 Load Sharing: No Deflection TL: 240 Deck: Not Checked Not Checked Importance: Normal Vibration: General Load **Bearings and Factored Reactions** Floor Live: 40 PSF Dead: **15 PSF** Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1 - SPF 2.375"

2 -

Hanger

2,500"

				_
Λ.	-	lveic.	Resu	40
М	IIа	17212	nesu.	LLS

ı	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
	Moment	2033 ft-lb	5'1 9/16"	7660 ft-lb	0.265 (27%)	1.25D+1.5L	L
	Unbraced	2033 ft-lb	5'1 9/16"	2038 ft-lb	0.997 (100%)	1.25D+1.5L	L
	Shear	1052 lb	1 5/8"	3160 lb	0.333 (33%)	1.25D+1.5L	L
	Perm Defl in.	0.029 (L/4604)	5'6 1/2"	0.375 (L/360)	0.080 (8%)	D	Uniform
l	LL Defl inch	0.077 (L/1754)	5'6 5/8"	0.375 (L/360)	0.210 (21%)	L	L
	TL Defl inch	0.106 (L/1270)	5'6 5/8"	0.562 (L/240)	0.190 (19%)	D+L	L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

34%

20%

262 / 814

153 / 484

1076 L

637 L

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



0

0

1.25D+1.5L 1.25D+1.5L

Page 1 of 1

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top flange must be laterally braced at a maximum of 7'10" o.c.
- 6 Bottom flange braced at bearings.

Γ	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
l	1	Tie-In	0-0-0 to 11-6-5	(Span)1-2-1	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
l	2	Tie-In	0-0-0 to 1-9-6	(Span)3-5-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
l	4	Point	1-8-2		Near Face	82 lb	200 lb	0 lb	0 lb	F7
l	5	Tie-In	1-9-6 to 10-3-12	(Span)1-5-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
l	6	Tie-In	10-4-14 to 11-6-5	(Span)0-9-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

Notes

Calculated Structured Designs is responsible only of the Structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

- Dry service conditions, unless noted otherwise
 Worst not to be treated with fire retardant or corrosive
- Handling & Installation
- Andling & Installation

 Joist flanges must not be cut or drilled

 Refer to latest copy of the Libist product information
 details for framing details, stiffener tables, web hole
 othart, bridging details, multi-uply lastening details and
 handling/erection details

 Damaged Iblosts must not be used
 Design assumes top flange to be laterally restrained
 by attached sheathing or as specified in engineering
 notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length≍ 3.5 inches
 For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Nascor by Kott





Project: Address: GREEN YORK HOMES

5/31/2018 Date:

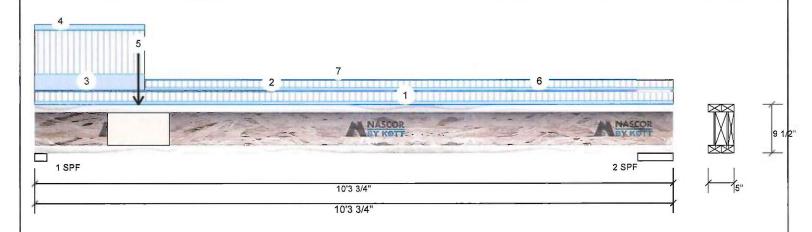
RCO Designer:

Job Name: LIANA 1 (ELEV.2)

Project #:

2-Ply - PASSED F8-B NJH 9.500"

Level: Ground Floor



Member Information Unfactored Reactions UNPATTERNED Ib (Uplift) Type: Girder Application: Floor (Residential) Live Dead Snow Plies: Design Method: LSD 469 223 0 Moisture Condition: Dry **Building Code:** NBCC 2010 / OBC 2012 197 93 0 2 Deflection LL: 360 Load Sharing: Deflection TL: 240 Deck: Not Checked Not Checked Importance: Normal Vibration: General Load **Bearings and Factored Reactions** Floor Live: 40 PSF Dead: 15 PSF Cap. React D/L lb Total Ld. Case Bearing Length 1 - SPF 2.375" 31% 278 / 704 982 L

Analysis Results

Г	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
l	Moment	1313 ft-lb	2'11 5/8"	7660 ft-lb	0.171 (17%)	1.25D+1.5L	L
l	Unbraced	1313 ft-lb	2'11 5/8"	1322 ft-lb	0.993 (99%)	1.25D+1.5L	L
l	Shear	958 lb	1 5/8"	3160 lb	0.303 (30%)	1.25D+1.5L	L
	Perm Defl in.	0.017 (L/6969)	4'6"	0.322 (L/360)	0.050 (5%)	D	Uniform
	LL Defl inch	0.035 (L/3306)	4'5 7/8"	0.322 (L/360)	0.110 (11%)	L	L
	TL Defl inch	0.052 (L/2242)	4'5 7/8"	0.483 (L/240)	0.110 (11%)	D+L	L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

13%

116 / 295

411 L

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS**

2 - SPF 6.875"



Wind

0

Ld. Comb.

1.25D+1.5L

1.25D+1.5L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 9'6" o.c.

5 Bottom flange braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 10-3-12	(Span)0-8-15		15 PSF	40 PSF	0 PSF	0 PSF	
2	Part. Uniform	0-0-0 to 9-8-8		Тор	2 PLF	0 PLF	0 PLF	0 PLF	
3	Tie-In	0-0-0 to 1-9-6	(Span)3-5-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
4	Part, Uniform	0-0-0 to 1-9-6		Тор	9 PLF	0 PLF	0 PLF	0 PLF	
5	Point	1-8-2		Far Face	131 lb	290 lb	0 lb	0 lb	F7
6	Tie-In	1-9-6 to 10-3-12	(Span)0-7-1	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
7	Part. Uniform	1-9-6 to 9-8-8		Тор	1 PLF	0 PLF	0 PLF	0 PLF	

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

- Dry service conditions, unless noted otherwise
 Upist not to be treated with fire retardant or corrosive
- chemicals

Handling & Installation

- Lost flanges must not be out or drilled.
 Refer to latest copy of the IJolist product information details for framing details, stiffener tables, web hole chart, bridgang details, multi-ply fastening details and handling/erection details.
 Damaged IJolists must not be used.
 Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing leightbs: 3,5 inches
 For flat roofs provide proper drainage to prevent

Manufacturer Info

Nascor by Kott





Page 1 of 1

EWP Studio Simpson Strong-Tie® Component Solutions™

Client: Project:

Address:

GREEN YORK HOMES

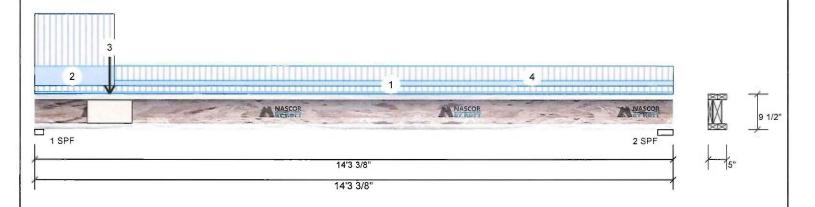
5/31/2018 Date:

Designer: RCO Job Name: LIANA 1 (ELEV.2)

Project #:

2-Ply - PASSED F9-A NJH 9.500"

Level: Ground Floor



Unfactored Reactions UNPATTERNED Ib (Uplift) Type: Girder Application: Floor (Residential) Live Dead Snow Wind Plies: Design Method: LSD 590 221 0 1 Moisture Condition: Dry **Building Code:** NBCC 2010 / OBC 2012 2 237 89 Ω 0 Deflection LL: 360 Load Sharing: Deflection TL: 240 Deck: Not Checked Importance: Not Checked Normal Vibration: General Load **Bearings and Factored Reactions** Floor Live: 40 PSF Dead: 15 PSF Cap. React D/L lb Total Ld. Case Ld. Comb. Bearing Length 1 - SPF 2.375" 37% 276 / 884 1161 L 1.25D+1.5L 2 - SPF 4.125" 15% 111 / 355 467 L 1.25D+1.5L

Analysis Results

Member Information

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1944 ft-lb	5'4 3/4"	7660 ft-lb	0.254 (25%)	1.25D+1.5L	L
Unbraced	1944 ft-lb	5'4 3/4"	1951 ft-lb	0.996 (100%)	1.25D+1.5L	L
Shear	1140 lb	1 5/8"	3160 lb	0.361 (36%)	1.25D+1.5L	L
Perm Defl in.	0.040 (L/4158)	6'7 5/8"	0.462 (L/360)	0.090 (9%)	D	Uniform
LL Defl inch	0.107 (L/1558)	6'7 5/8"	0.462 (L/360)	0.230 (23%)	L	L
TL Defl inch	0.147 (L/1134)	6'7 5/8"	0.693 (L/240)	0.210 (21%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 8' o.c.

5 Bottom flange braced at bearings.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



ı	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
I	1	Tie-In	0-0-0 to 14-3-6	(Span)0-4-9	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
I	2	Tie-In	0-0-0 to 1-9-6	(Span)3-5-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
ı	3	Point	1-8-2		Near Face	134 lb	358 lb	0 lb	0 lb	F7
ı	4	Tie-In	1-9-6 to 14-3-6	(Span)0-11-7	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
ı										

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design enterns and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- I, Jlost flanges must not be cut or drilled
 Refer to latest copy of the Lloist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-ply fastening details and handling/orection details
 Damlaged Lloists must not be used
 Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation.
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 linches
 For flat roofs provide proper drainage to prevent pending.

Manufacturer Info

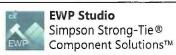
Nascor by Kott



Kott Lumber Company 14 Anderson Blvd, Ontario Canada



Page 1 of 1



Client:

GREEN YORK HOMES

Project: Address: Date: 5/31/2018

Designer: RCO

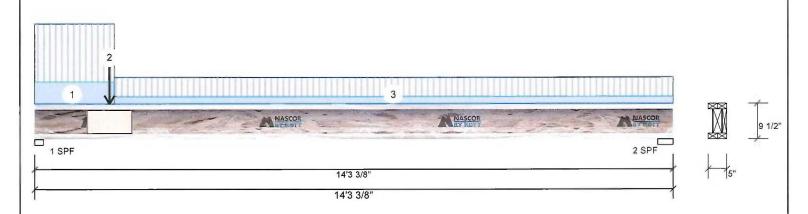
Job Name: LIANA 1 (ELEV.2)

Project #:

F9-B NJH 9.500"

2-Ply - PASSED

Level: Ground Floor



Member Information Girden Plies: Moisture Condition: Dry Deflection LL: 360 Deflection TL: 240 Importance: Normal General Load Floor Live: 40 PSF

15 PSF

Application: Design Method:

Building Code:

Load Sharing:

Deck:

Vibration:

Floor (Residential)

NBCC 2010 / OBC 2012

Not Checked

Not Checked

Unfactored Reactions UNPATTERNED Ib (Uplift)

1	531	199	0	0
2	204	77	0	0

Dead

Bearings and Factored Reactions Cap. React D/L lb

Live

Bearing Length	Cap. R	eact D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF 2.375"	33%	249 / 796	1045	L	1.25D+1.5L
2 - SPF 4.125"	13%	96 / 306	402	L	1.25D+1.5L

Analysis Results

Dead:

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1707 ft-lb	5'2 15/16"	7660 ft-lb	0.223 (22%)	1.25D+1.5L	L
Unbraced	1707 ft-lb	5'2 15/16"	1714 ft-lb	0.996 (100%)	1.25D+1.5L	L
Shear	1027 lb	1 5/8"	3160 lb	0.325 (32%)	1.25D+1.5L	L
Perm Defl in.	0.035 (L/4746)	6'7 5/16"	0.462 (L/360)	0.080 (8%)	D	Uniform
LL Defl inch	0.094 (L/1779)	6'7 5/16"	0.462 (L/360)	0.200 (20%)	L	L
TL Defl inch	0.129 (L/1294)	6'7 5/16"	0.693 (L/240)	0.190 (19%)	D+L	L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Wind

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 8'6" o.c.

5 Bottom flange braced at bearings

		*1							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-9-6	(Span)3-5-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	1-8-2		Far Face	124 lb	331 lb	0 lb	0 lb	F7
3	Tie-In	1-9-6 to 14-3-6	(Span)1-1-9	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design enteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

- Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

- Joist flanges must not be cut or drilled Refer to latest copy of the Joset product information details for framing details, stifflener tables, web hole chart, bridging details, multi-ply fastening details and handling-irection details Damaged Joists must not be used Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stitteners for point load as shown Minimum point load bearing length>= 3,5 inches
 For flat roofs provide proper drainage to prevent ponding.

Manufacturer Info

Nascor by Kott







Project: Address:

GREEN YORK HOMES Client:

Date:

5/31/2018

RCO

Job Name: LIANA 1 (ELEV.2)

Project #:

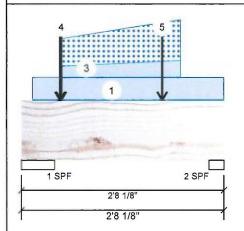
Designer:

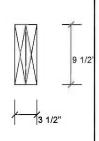
Forex 2.0E-3000Fb LVL

1.750" X 9.500"

2-Ply - PASSED

_evel: Second Floor





Type:	Girder
Plies:	2

Member Information

Moisture Condition: Dry Deflection LL: 360 Deflection TL: 240 Importance: Normal

General Load Floor Live: 40 PSF Dead: 15 PSF

Application:

Design Method: LSD Building Code:

NBCC 2010 / OBC 2012

Floor (Residential)

Load Sharing: No Deck: Not Checked

Vibration: Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Live	Dead	Snow	Wind
272	661	1056	0
143	198	134	0
	272	272 661	272 661 1056

Bearings and Factored Reactions

bearings and ractored reactions											
Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.					
1 - SPF	5.250"	25%	826 / 1720	2546	L	1.25D+1.5S +0.5L					
2 - SPF	2.375"	11%	248 / 214	462	L	1.25D+1.5L					

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	395 ft-lb	1'1 9/16"	22724 ft-lb	0.017 (2%)	1.25D+1.5S +0.5L	L
Unbraced	395 ft-lb	1'1 9/16"	22724 ft-lb	0.017 (2%)	1.25D+1.5S +0.5L	L
Shear	638 lb	1'2"	7792 lb	0.082 (8%)	1.25D+1.5L	L
Perm Defl in.	0.001 (L/39438)	1'4 5/8"	0.072 (L/360)	0.010 (1%)	D	Uniform
LL Defl inch	0.001 (L/29384)	1'3"	0.072 (L/360)	0.010 (1%)	S+0.5L	L
TL Defl inch	0.002 (L/16882)	1'3 3/4"	0.108 (L/240)	0.010 (1%)	D+S+0.5L	L

Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on full section width.

READ ALL NOTES ON THIS PAGE AND ON **ENGINEERING NOTE PAGE ENP-2. THIS** NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design orthern and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

- LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
 approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at beamg points to avoid
 lateral displacement and rotation

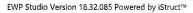
For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex APA: PR-L318









Page 2 of 2

EWP Studio Simpson Strong-Tie® Component Solutions™

Client: Project:

Address:

GREEN YORK HOMES

5/31/2018 Date:

Job Name: LIANA 1 (ELEV.2)

Designer: RCO

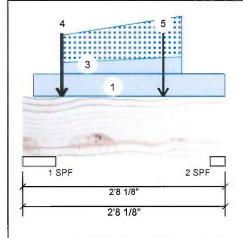
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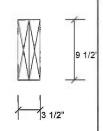
Forex 2.0E-3000Fb LVL

1.750" X 9.500"

2-Ply - PASSED

Level: Second Floor





ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Part. Uniform	0-1-12 to 2-8-2		Тор	64 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
2	Point	0-6-2		Тор	458 lb	0 lb	1038 lb	0 lb	F14 F14
3	Tapered Start	0-6-2		Тор	32 PLF	0 PLF	76 PLF	0 PLF	
	End	2-1-4			48 PLF	0 PLF	115 PLF	0 PLF	
4	Point	0-6-5		Far Face	86 lb	230 lb	0 lb	0 lb	J4
5	Point	1-10-5		Far Face	69 lb	185 lb	0 lb	0 lb	J4
	Self Weight				8 PLF				

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- 1. LVI. beams must not be cut or drilled
 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
 3. Darnaged Beams must not be used
 4. Design assumes top edge is laterally restrained
 5. Provide lateral support at beaming points to avoid lateral displacement and rotation.

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex APA: PR-L318







Wind 0 0

Ld. Comb. 1.25D+1.5L 1.25D+1.5L

Page 1 of 1

EWP Studio Simpson Strong-Tie® Component Solutions™

GREEN YORK HOMES

Project: Address: Date:

5/31/2018

Designer: RCO Job Name: LIANA 1 (ELEV.2)

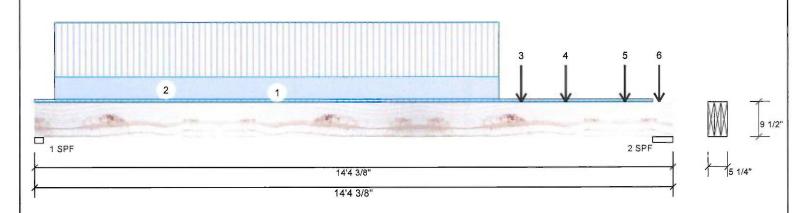
Project #:

Forex 2.0E-3000Fb LVL

1.750" X 9.500"

3-Ply - PASSED

Level: Second Floor



Member Inforn	nation			Unfactore	d React	ions U	NPATTERN	ED lb (Uplift)
Туре:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Sno	W
Plies:	3	Design Method:	LSD	1	1909		863		0
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012	2	2945		1254		0
Deflection LL:	360	Load Sharing:	Yes	15					
Deflection TL:	240	Deck:	Not Checked						
Importance:	Normal	Vibration:	Not Checked						
General Load									
Floor Live:	40 PSF			Bearings	and Fact	ored F	Reactions		
Dead:	15 PSF			Bearing L	ength	Cap.	React D/L lb	Total	Ld. Case
				1 - SPF 2	2.375"	51%	1079 / 2864	3943	L
				2 - SPE 5	500"	34%	1567 / 4417	5984	1

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	14206 ft-lb	7' 5/8"	35449 ft-lb	0.401 (40%)	1.25D+1.5L	L
Unbraced	14206 ft-lb	7' 5/8"	34190 ft-lb	0.416 (42%)	1.25D+1.5L	L
Shear	4299 lb	13'2 1/8"	13915 lb	0.309 (31%)	1.25D+1.5L	L
Perm Defl in.	0.149 (L/1117)	7' 1/2"	0.461 (L/360)	0.320 (32%)	D	Uniform
LL Defl inch	0.333 (L/499)	7' 11/16"	0.461 (L/360)	0.720 (72%)	L	L
TL Defl inch	0.481 (L/345)	7' 11/16"	0.692 (L/240)	0.700 (70%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on full section width,

READ ALL NOTES ON THIS PAGE AND ON **ENGINEERING NOTE PAGE ENP-2. THIS** NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER **CONNECTION DETAIL FOR PLY TO PLY** NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.

D	Load Type Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
[Tie-In 0-0-0 to 13-10-15	(Span)0-6-2		15 PSF	40 PSF	0 PSF	0 PSF	Comments
		(Spair)0-6-2						
2	Part. Uniform 0-5-7 to 10-5-7		Far Face	115 PLF	278 PLF	0 PLF	0 PLF	
3	Point 10-11-7		Far Face	104 lb	278 lb	0 lb	0 lb	J6
4	Point 11-11-7		Far Face	122 lb	324 lb	0 lb	0 lb	J6
5	Point 13-3-7		Far Face	139 lb	371 lb	0 lb	0 lb	J6
6	Point 14-0-11		Near Face	384 lb	959 lb	0 lb	0 lb	F4
	Self Weight			11 PLF				
I								



Notes

Calculated Structured Designs is responsible only of the Structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Handling & Installation

- andling & Installation
 LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
 approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at beaming points to avoid
 lateral displacement and rotation

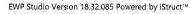
6 For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318







Page 1 of 2

EWP Studio Simpson Strong-Tie® Component Solutions™ Client:

GREEN YORK HOMES

Project: Address: Date:

5/31/2018

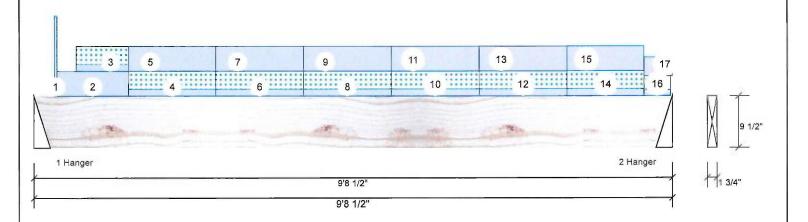
Designer: RCO LIANA 1 (ELEV.2) Job Name:

Project #:

Forex 2.0E-3000Fb LVL F12-A

1.750" X 9.500" - PASSED

Level: Second Floor



Member Information Application: Floor (Residential) Type: Plies: Design Method: LSD Moisture Condition: Dry Building Code: NBCC 2010 / OBC 2012 Deflection LL: 360 Load Sharing: No Deflection TL: 240 Not Checked Deck: Not Checked Importance: Normal Vibration:

			11010111				
General Load							
Floor Live:	40 PSF						
Dead:	15 PSF						
Analysis R	esults						
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case	
Moment	779 ft-lb	4'10 5/16"	9544 ft-lb	0.082 (8%)	1.25D+1.5S	L	

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	779 ft-lb	4'10 5/16"	9544 ft-lb	0.082 (8%)	1.25D+1.5S	L
Unbraced	779 ft-lb	4'10 5/16"	3994 ft-lb	0.195 (20%)	1.25D+1.5S	L
Shear	278 lb	11 3/4"	3896 lb	0.071 (7%)	1.25D+1.5S	L
Perm Defl in.	0.027 (L/4130)	4'10 1/4"	0.311 (L/360)	0.090 (9%)	D	Uniform
LL Defl inch	0.014 (L/8256)	4'10 5/16"	0,311 (L/360)	0.040 (4%)	S	L
TL Defl inch	0.041 (L/2753)	4'10 1/4"	0.467 (L/240)	0.090 (9%)	D+S	L
						L

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.
- 4 Bottom braced at bearings

Unfactored Reactions U	NPATTERNED lb (Uplift)
-------------------------------	------------------------

Brg	Live	Dead	Snow	Wind
1	0	164	76	0
2	0	171	85	0

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.	
1 - Hanger	3.000"	10%	205 / 114	319	L	1.25D+1.5S	
2 - Hanger	3.000"	10%	213 / 128	341	L	1.25D+1.5S	

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARING



4 Bottom	braced at bearings.									
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments	
1	Part. Uniform	0-3-12 to 0-4-4		Тор	80 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight	
2	Part, Uniform	0-4-4 to 1-5-5		Тор	25 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight	
3	Part. Uniform	0-7-12 to 1-5-5		Тор	7 PLF	0 PLF	18 PLF	0 PLF		
4	Part, Uniform	1-5-5 to 2-9-5		Тор	7 PLF	0 PLF	18 PLF	0 PLF		
5	Part. Uniform	1-5-5 to 2-9-5		Тор	25 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight	
6	Part. Uniform	2-9-5 to 4-1-5		Тор	7 PLF	0 PLF	18 PLF	0 PLF		
7	Part, Uniform	2-9-5 to 4-1-5		Тор	25 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight	
8	Part. Uniform	4-1-5 to 5-5-5		Тор	7 PLF	0 PLF	18 PLF	0 PLF		

Continued on page 2...

Notes

Calculated Structured Designs is responsible only of the Carcinated structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Handling & Installation

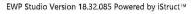
- and ling & Installation
 LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
 approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info Forex APA: PR-L318







Page 2 of 2

EWP Studio Simpson Strong-Tie® Component Solutions™

Client:

Address:

GREEN YORK HOMES

Project:

5/31/2018 Date:

RCO

Job Name: LIANA 1 (ELEV.2)

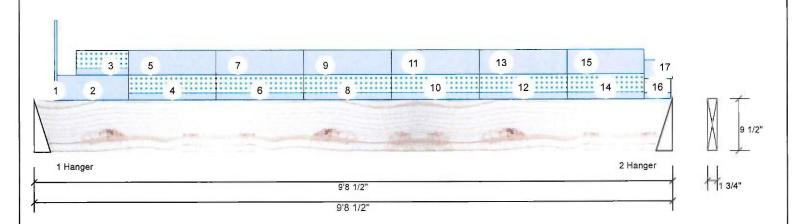
Project #

Designer:

F12-A Forex 2.0E-3000Fb LVL

1.750" X 9.500" - PASSED

Level: Second Floor



Continued	from page 1							
ID	Load Type	Location Trib Width	Side	Dead	Live	Snow	Wind	Comments
9	Part, Uniform	4-1-5 to 5-5-5	Тор	25 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
10	Part, Uniform	5-5-5 to 6-9-5	Тор	7 PLF	0 PLF	18 PLF	0 PLF	
11	Part. Uniform	5-5-5 to 6-9-5	Тор	25 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
12	Part. Uniform	6-9-5 to 8-1-5	Тор	7 PLF	0 PLF	18 PLF	0 PLF	
13	Part. Uniform	6-9-5 to 8-1-5	Тор	25 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
14	Part, Uniform	8-1-5 to 9-3-5	Тор	7 PLF	0 PLF	18 PLF	0 PLF	
15	Part, Uniform	8-1-5 to 9-3-5	Тор	25 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
16	Part. Uniform	9-3-5 to 9-8-3	Тор	6 PLF	0 PLF	14 PLF	0 PLF	
17	Part, Uniform	9-3-5 to 9-8-8	Тор	19 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
	Self Weight			4 PLF				

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

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PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

andling & Installation
LVL beams must not be cut or drilled
Refer to manufacturer's product information
regarding installation requirements, multi-ply
fastening details, beam strength values, and code
approvals
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Design assumes top adge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

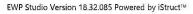
For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex APA: PR-L318







Client: Project:

Address:

GREEN YORK HOMES

5/31/2018

RCO Designer:

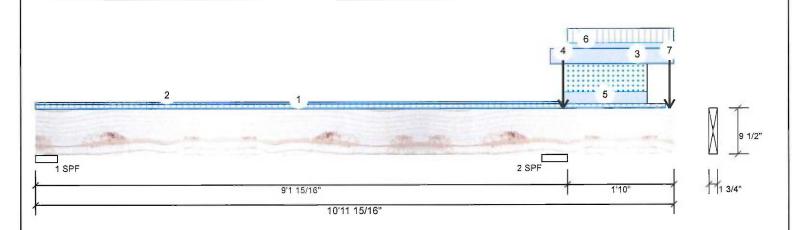
Job Name: LIANA 1 (ELEV.2)

Project #:

Forex 2.0E-3000Fb LVL

1.750" X 9.500" - PASSED

Level: Second Floor



			4.
IN/IAM	nar	Informa	TION
IAICIII	201		LIVII

Type:	Girder
Plies:	1
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	

Application: Design Method:

Floor (Residential) LSD

NBCC 2010 / OBC 2012 **Building Code:** Load Sharing: No

> Not Checked Not Checked

Deck: Vibration:

2

Brg

1

Dead (-1)826

Unfactored Reactions UNPATTERNED lb (Uplift)

Wind Snow 0 (-33) 719

0

0

Floor Live: 40 PSF Dead: 15 PSF

Bearings and Factored Reactions

1 ive

105

310

_							
Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.	
1 - SPF	4.563"	5%	-1 / 185	185 (-55)	L_	0.9D+1.5L	
2 - SPF	5.250"	40%	1032 / 1233	2265	LL	1.25D+1.5S	

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Neg Moment	-930 ft-lb	9'1 15/16"	7726 ft-lb	0.120 (12%)	1.25D+1.5L	_L
Unbraced	-1198 ft-lb	9'1 15/16"	4322 ft-lb	0.277 (28%)	1.25D+1.5S +0.5L	_L
Pos Moment	269 ft-lb	3'6 1/4"	8294 ft-lb	0.032 (3%)	0.9D+1.5L	L_
Unbraced	269 ft-lb	3'6 1/4"	5773 ft-lb	0.047 (5%)	0.9D+1.5L	L_
Shear	701 lb	9'11 7/16"	4638 lb	0.151 (15%)	1.25D+1.5S +0.5L	_L
Perm Defl in.	0.011 (L/9417)	5'9 3/16"	0.287 (L/360)	0.040 (4%)	D	Uniform
LL Defl inch	0.015 (L/6903)	4'7 9/16"	0.287 (L/360)	0.050 (5%)	L	L_
TL Defl inch	0.023 (L/4543)	5'6 3/8"	0.431 (L/240)	0.050 (5%)	D+S+0.5L	_L
LL Cant	0.018 (2L/2401)	Rt Cant	0.200 (2L/480)	0.092 (9%)	S+0.5L	_L
TL Cant	0.042 (2L/1038)	Rt Cant	0,300 (2L/360)	0.141 (14%)	D+S+0.5L	_L

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PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS



Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Tie-down connection required at bearing 1 for uplift 55 lb (Combination 1.25D+1.5S+0.5L, Load Case _L).
- 4 Top braced at bearings.
- 5 Bottom braced at bearings

Notes

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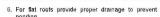
Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- andling & Installation
 LVL beams must not be out or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
 approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation



Manufacturer Info

APA: PR-L318









Page 2 of 2

EWP Studio Simpson Strong-Tie® Component Solutions™

Client:

Address:

GREEN YORK HOMES

Project:

5/31/2018 Date:

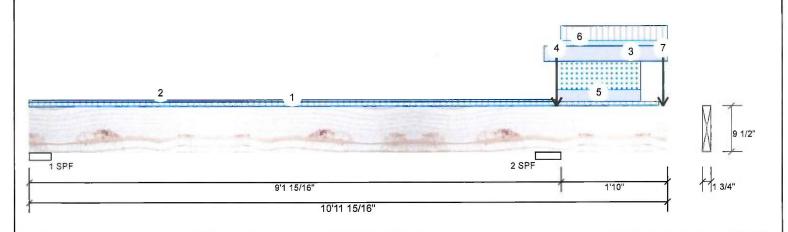
RCO Designer:

Job Name: LIANA 1 (ELEV.2)

Project #

1.750" X 9.500" - PASSED F13-A Forex 2.0E-3000Fb LVL

Level: Second Floor



ľ	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
l	1	Tie-In	0-0-0 to 10-10-3	(Span)0-10-3	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	2	Tie-In	0-0-0 to 9-0-13	(Span)0-5-13	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	3	Part. Uniform	8-10-7 to 10-11-15		Тор	80 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
l	4	Point	9-1-1		Тор	202 lb	0 lb	390 lb	0 lb	F15 F15
l	5	Part. Uniform	9-1-1 to 10-6-7		Тор	63 PLF	0 PLF	152 PLF	0 PLF	
l	6	Tie-In	9-1-15 to 10-11-15	(Span) 3-10-14	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
١	7	Point	10-11-1		Far Face	164 lb	0 lb	76 lb	0 lb	F12
١		Self Weight				4 PLF				

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS.

Notes

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Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

Handling & Installation

1. LVL beams must not be cut or drilled

2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastering details, beam strength values, and code approvals

3. Damaged Beams must not be used

4. Design assumes top edge is laterally restrained

5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318







Page 1 of 2

EWP Studio Simpson Strong-Tie® Component Solutions™

Client:

GREEN YORK HOMES

Project: Address:

Date: 5/31/2018

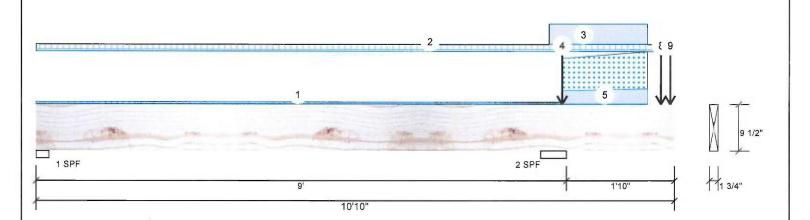
Designer: RCO

Job Name: LIANA 1 (ELEV.2)

Project #:

1.750" X 9.500" - PASSED Forex 2.0E-3000Fb LVL

Level: Second Floor



Member Information Туре: Girden Application: Floor (Residential) Plies: Design Method: Moisture Condition: Dry **Building Code:** NBCC 2010 / OBC 2012 Deflection LL: 360 Load Sharing: Deflection TL: 240 Deck: Not Checked Not Checked Importance: Normal Vibration: General Load Floor Live: 40 PSF 15 PSF Dead:

Unfactored	Reactions	UNPATTERNED	lb	(Uplift)
------------	-----------	--------------------	----	----------

Brg	Live	Dead	Snow	Wind
1	116	1	0 (-37)	0
2	157	801	797	0

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.	
1 - SPF	2.625"	9%	1 / 179 18	0 (-54)	L_	1.25D+1.5L	
2 - SPF	5.250"	40%	1001 / 1274	2275	LL	1.25D+1.5S +0.5I	

Analysis Results

Ī	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
	Neg Moment	~1144 ft-lb	9'	11248 ft-lb	0.102 (10%)	1.25D+1.5S +0.5L	_L
	Unbraced	-1144 ft-lb	9'	4322 ft-lb	0.265 (26%)	1.25D+1.5S +0.5L	LL
	Pos Moment	280 ft-lb	3'5 1/8"	8408 ft-lb	0.033 (3%)	0.9D+1.5L	L_
	Unbraced	280 ft-lb	3'5 1/8"	5672 ft-lb	0.049 (5%)	0.9D+1.5L	L_
	Shear	664 lb	9'9 1/2"	4638 lb	0.143 (14%)	1.25D+1.5S +0.5L	_L
	Perm Defl in.	0.010 (L/10431)	5'7 13/16"	0.288 (L/360)	0.030 (3%)	D	Uniform
	LL Defl inch	0.015 (L/6902)	4'5 5/8"	0.288 (L/360)	0.050 (5%)	L	L_
	TL Defl inch	0.021 (L/4995)	5'4 3/4"	0.431 (L/240)	0.050 (5%)	D+S+0.5L	_L
	LL Cant	0.017 (2L/2601)	Rt Cant	0.200 (2L/480)	0.085 (8%)	S+0.5L	_L
	TL Cant	0.039 (2L/1118)	Rt Cant	0.300 (2L/360)	0.131 (13%)	D+S+0.5L	_L

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PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Tie-down connection required at bearing 1 for uplift 54 lb (Combination 0.9D+1.5S, Load Case _L).
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.

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Lumber

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

- Handling & Installation
- EVL beams must not be cut or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
 - Damaged Beams must not be used
- Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318







Client:

GREEN YORK HOMES

Project: Address:

Date: 5/31/2018

Designer: RCO

Job Name: LIANA 1 (ELEV.2)

Project #:

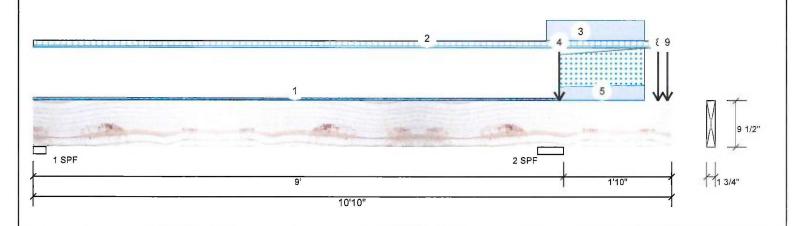
Forex 2.0E-3000Fb LVL

1.750" X 9.500" - PASSED

Level: Second Floor

FAGE 32 UF 41

Page 2 of 2



ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 8-10-14	(Span)0-3-15 to 0-3-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 10-8-4	(Span)1-0-1 to 1-0-1	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Part. Uniform	8-8-8 to 10-4-8		Тор	80 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
4	Point	8-11-2		Тор	232 lb	0 lb	460 lb	0 lb	F15 F15
5	Tapered Start	8-11-2		Тор	53 PLF	0 PLF	128 PLF	0 PLF	
	End	10-4-8			61 PLF	0 PLF	146 PLF	0 PLF	
6	Point	10-7-4		Тор	6 lb	0 lb	0 lb	0 lb	Wall Self Weight
7	Point	10-7-4		Тор	7 lb	0 lb	17 lb	d1 0	
8	Point	10-7-4		Тор	26 lb	0 lb	0 lb	0 lb	Wall Self Weight
9	Point	10-9-2		Near Face	171 lb	0 lb	85 lb	0 lb	F12
	Self Weight				4 PLF				

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.

Notes

Notes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Handling & Installation

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For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318







Client:

GREEN YORK HOMES

Project: Address:

Date: 5/31/2018

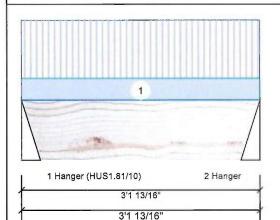
Designer: RCO

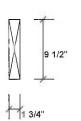
Job Name: LIANA 1 (ELEV.2)

Project #:

Forex 2.0E-3000Fb LVL F1-A

1.750" X 9.500" - PASSED Level: Second Floor





Page 1 of 1

Member Information

Туре:	Girder
Plies:	1
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	
Floor Live:	40 PSF

Application:

Design Method: LSD **Building Code:** NBCC 2010 / OBC 2012 Load Sharing: No Deck: Not Checked

Not Checked

Floor (Residential)

Unfactored Reactions UNPATTERNED lb (Uplift) Brg Live 378 1 2 378

Wind Snow 0 0 0 0

Floor Live:	40
Dead:	15

PSF

Bearings and Factored Reactions

_							
Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.	
1 - Hanger	3.000"	19%	185 / 567	752	L	1.25D+1.5L	
2 - Hanger	3.000"	19%	185 / 567	752	L	1.25D+1.5L	

Dead

148

148

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	460 ft-lb	1'6 15/16"	11362 ft-lb	0.040 (4%)	1.25D+1.5L	L.
Unbraced	460 ft-lb	1'6 15/16"	10144 ft-lb	0.045 (5%)	1.25D+1.5L	L
Shear	285 lb	2'2 1/16"	4638 lb	0.061 (6%)	1.25D+1.5L	L
Perm Defl in.	0.001 (L/29538)	1'6 15/16"	0.093 (L/360)	0.010 (1%)	D	Uniform
LL Defl inch	0.003 (L/11546)	1'6 15/16"	0.093 (L/360)	0.030 (3%)	L	L
TL Defl inch	0.004 (L/8301)	1'6 15/16"	0.139 (L/240)	0,030 (3%)	D+L	L

Vibration:

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REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARING



Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.

4 Dottom braces	at bearings.								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Part, Uniform	0-0-0 to 3-1-13		Тор	90 PLF	240 PLF	0 PLF	0 PLF	
	Self Weight				4 PLF				

Notes

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Handling & Installation

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6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318





Client:

Address:

GREEN YORK HOMES

Project:

Date:

5/31/2018

Designer: RCO

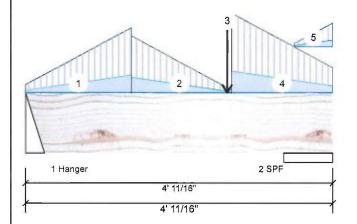
Job Name: LIANA 1 (ELEV.2)

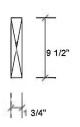
Project #:

Forex 2.0E-3000Fb LVL F2-A

1.750" X 9.500" - PASSED

Level: Second Floor





Ld. Comb. 1.25D+1.5L

1.25D+1.5L

Page 1 of 1

Member Information			
Туре:	Girder		
Plies:	1		
Moisture Condition:	Dry		
Deflection LL:	360		
Deflection TL:	240		
Importance:	Normal		
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

Actual

94 ft-lb

94 ft-lb

(L/60406)

(L/40550)

88 lb

Perm Defl in. 0.000 (L/999)

LL Defl inch 0.001

TL Defl inch 0.001

Application: Floor (Residential) Design Method: **Building Code:** NBCC 2010 / OBC 2012 Load Sharing: No Deck: Not Checked Vibration: Not Checked

0 999.000 (L/0) 0.000 (0%)

1'11 5/8" 0.164 (L/240) 0.010 (1%) D+L

1'11 13/16" 0.109 (L/360) 0.010 (1%)

Unfacto	red Reaction	S UNPATTER	NED lb (Uplif	t)
Brg	Live	Dead	Snow	Wind
1	42	23	0	0
2	80	39	0	0

Cap. React D/L lb

29 / 63

48 / 121

			2 -
Capacity	Comb.	Case	
0.008 (1%)	1.25D+1.5L	L	RE
0.010 (1%)	1.25D+1.5L	L	EN NO
0.019 (2%)	1.25D+1.5L	L	CA
0.000 (0%)			CC
0.010 (1%)	L	L	JUS
			RE
0.010 (1%)	D+I	L	CC

EAD ALL NOTES ON THIS PAGE AND ON IGINEERING NOTE PAGE ENP-2. THIS TE PAGE IS AN INTEGRAL PART OF THIS ALCULATION SUMMARY PAGE AS IT ONTAINS SPECIFICATIONS AND CRITERIA SED IN THE DESIGN OF THIS COMPONENT.

2%

2%

Bearings and Factored Reactions

Bearing Length

SPF 7.778"

Hanger

3.000"

EFER TO MULTIPLE MEMBER TO MEMBER ONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



Total Ld. Case

92 L

169 L

Design Notes

Analysis Results

Analysis

Moment

Unbraced

Shear

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.

4 Bottom brac	ed at bearings.								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-4-13	(Span)0-2-8 to 1-7-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	1-4-13 to 2-8-11	(Span)1-4-13 to 0-0-14	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	2-8-0		Far Face	15 lb	40 lb	0 lb	di 0	J8
4	Tie-In	2-8-11 to 4-0-11	(Span)1-11-8 to 0-7-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
5	Tie-In	3-6-9 to 4-0-11	(Span)0-0-14 to 0-7-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	Self Weight				4 PLF				

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Location Allowed

2'2 7/16" 11362 ft-lb

2'2 7/16" 9657 ft-lb

2'8 3/16" 4638 lb

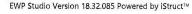
- Handling & Installation
- LVI. beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-pty fastening details, beam strength values, and code approvals
- approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info APA: PR-L318







Client: Project: Address:

GREEN YORK HOMES

5/31/2018

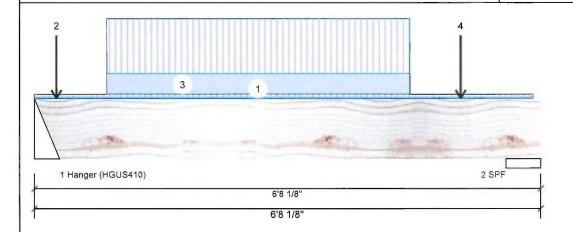
Designer: RCO Job Name: LIANA 1 (ELEV.2)

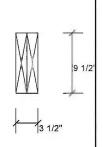
Project #:

Forex 2.0E-3000Fb LVL

1.750" X 9.500" 2-Ply - PASSED

Level: Second Floor





1	Member	Information
١	T	0:-4

Type:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	
Floor Live;	40 PSF

15 PSF

Application: Design Method: Building Code:

Load Sharing:

Deck: Vibration:

Floor (Residential) LSD NBCC 2010 / OBC 2012 No

Not Checked Not Checked

Unfactored Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	VVind
1	959	384	0	0
2	855	349	0	0

Bearings and Factored Reactions

Bearing Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.	
1 - 4.000"	18%	481 / 1439	1919	L	1.25D+1.5L	
Hanger						
2 - SPE 5.500"	15%	437 / 1283	1720	1	1.25D+1.5L	

Analysis Results

Dead:

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2571 ft-lb	3'3 5/8"	22724 ft-lb	0.113 (11%)	1.25D+1.5L	L
Unbraced	2571 ft-lb	3'3 5/8"	22010 ft-lb	0.117 (12%)	1.25D+1.5L	L
Shear	1733 lb	5'5 7/8"	9277 lb	0.187 (19%)	1.25D+1.5L	L
Perm Defl in.	0.008 (L/8511)	3'3 1/2"	0.200 (L/360)	0.040 (4%)	D	Uniform
LL Defl inch	0.021 (L/3423)	3'3 1/2"	0.200 (L/360)	0.110 (11%)	L	L
TL Defl inch	0.030 (L/2441)	3'3 1/2"	0.301 (L/240)	0.100 (10%)	D+L	L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on full section width.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 6-6-14	(Span)0-9-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	0-3-8		Near Face	96 lb	256 lb	0 lb	0 lb	J6
3	Part, Uniform	0-11-8 to 4-11-8		Near Face	104 PLF	278 PLF	0 PLF	0 PLF	
4	Point	5-7-8		Near Face	133 lb	345 lb	0 lb	0 lb	J6
	Self Weight				8 PLF				

Notes

Calculated Structured Designs is responsible only of the calculated solutions of being in the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- andling & Installation
 LVI, beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
 approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex APA: PR-L318







Client:

GREEN YORK HOMES

Project: Address: Date: 5/31/2018

Designer: RCO

Job Name: LIANA 1 (ELEV.2)

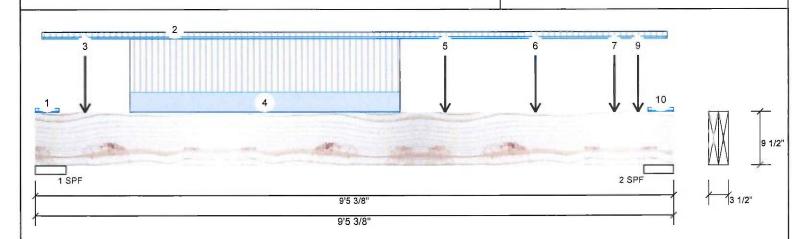
Project #:

Forex 2.0E-3000Fb LVL

1.750" X 9.500"

2-Ply - PASSED

Level: Second Floor



Member Info	rmation			Unfacto	ored React	ions UNPATTERN	IED lb (Uplif	t)
Type:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	
Plies:	2	Design Method:	LSD	1	708 (-2)	303	0	
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	649 (-115)	259	0	
Deflection LL:	360	Load Sharing:	No					
Deflection TL:	240	Deck:	Not Checked					
Importance:	Normal	Vibration:	Not Checked					
General Load								
Floor Live:	40 PSF			Bearing	gs and Fact	ored Reactions		
Dead:	15 PSF			Bearing	Length	Cap. React D/L lb	Total Ld. C	ase

Analysis Results

ı	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
l	Moment	3030 ft-lb	4'8 3/16"	22724 ft-lb	0.133 (13%)	1.25D+1.5L	L
l	Unbraced	3030 ft-lb	4'8 3/16"	21237 ft-lb	0.143 (14%)	1.25D+1.5L	L
l	Shear	1394 lb	1'2 1/4"	9277 lb	0.150 (15%)	1.25D+1.5L	L
l	Perm Defl in.	0.019 (L/5399)	4'8 11/16"	0.289 (L/360)	0.070 (7%)	D	Uniform
l	LL Defl inch	0.046 (L/2287)	4'8 11/16"	0.289 (L/360)	0.160 (16%)	L	L
L	TL Defl inch	0.065 (L/1607)	4'8 11/16"	0.434 (L/240)	0.150 (15%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on full section width.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

12%

11%

379 / 1063

324 / 974

1441 L

1297 1

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS

1 - SPF 5.500"

2 - SPF 5.250"



Wind

0

Ld. Comb.

1.25D+1.5L

1.25D+1.5L

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 0-4-4	(Span)0-4-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-1-4 to 9-4-4	(Span)0-8-9	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	0-8-14		Near Face	55 lb	145 lb	dl 0	0 lb	J3
4	Part. Uniform	1-4-14 to 5-4-14		Near Face	57 PLF	151 PLF	0 PLF	0 PLF	
5	Point	6-0-14		Near Face	72 lb	191 lb	0 lb	0 lb	J5
6	Point	7-4-14		Near Face	65 lb	174 lb	0 lb	0 lb	J5

Continued on page 2...

Notes

Cabulated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the Intended application, and to verify the dimensions and loads.

Lumber

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled.
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals.
 Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

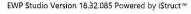
6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex APA: PR-L318







Client: Project: Address:

GREEN YORK HOMES

Date:

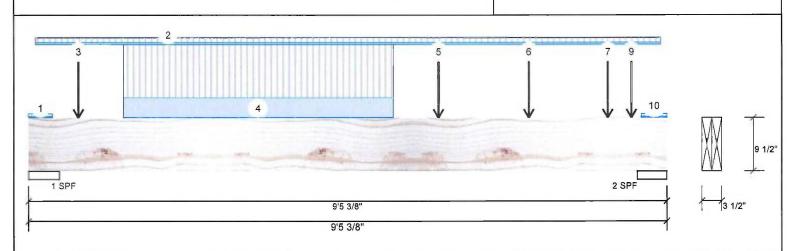
5/31/2018 RCO

Designer: Job Name: LIANA 1 (ELEV.2)

Project #:

Forex 2.0E-3000Fb LVL F5-A

1.750" X 9.500" 2-Ply - PASSED Level: Second Floor



..Continued from page 1

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
7	Point	8-6-14		Near Face	40 lb	105 lb	0 lb	0 lb	J5
8	Point	8-11-1		Near Face	-22 lb	0 lb	0 lb	0 lb	F6
9	Point	8-11-1		Near Face	0 lb	-117 lb	0 lb	0 lb	F6
10	Tie-In 9-	-0-13 to 9-5-6	(Span)0-4-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	Self Weight				8 PLF				

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the Intended application, and to verify the dimensions and loads.

Lumber

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- chemicals

Handling & Installation

- Handling & Installation

 1. VIL beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastering details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex APA: PR-L318





Client: Project: Address: **GREEN YORK HOMES**

Date: 5/31/2018

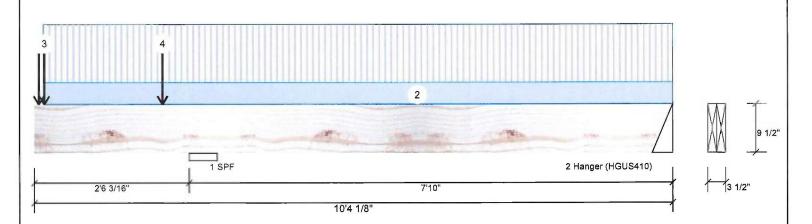
Designer: RCO

Job Name: LIANA 1 (ELEV.2)

F6-A Forex 2.0E-3000Fb LVL 1.750" X 9.500"

2-Ply - PASSED

Level: Second Floor



Hanger

Member Inforr	nation			Unfactored Reactions UNPATTERNED Ib (Uplift)							
Туре:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snow	Wind		
Plies:	2	Design Method:	LSD	1	658		316	0	0		
Moisture Condition	: Dry	Building Code:	NBCC 2010 / OBC 2012	2	0 (-117)		(-22)	0	0		
Deflection LL:	360	Load Sharing:	No								
Deflection TL:	240	Deck:	Not Checked								
Importance:	Normal	Vibration:	Not Checked								
General Load				-					7/5- 7		
Floor Live:	40 PSF			Bearings	s and Fac	tored R	leactions				
Dead:	15 PSF			Bearing	Length	Cap.	React D/L lb	Total Ld. C	ase Ld. Comb.		
				1 - SPF	5.500"	12%	395 / 987	1382 LL	1.25D+1.5L		
				2 -	4.000"	0%	-20 / 41 22	2 (-242) _L	0.9D+1.5L		

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Neg Moment	-2162 ft-lb	2'8 15/16"	22724 ft-lb	0.095 (10%)	1.25D+1.5L	<u></u>
Unbraced	-2162 ft-lb	2'8 15/16"	21662 ft-lb	0.100 (10%)	1.25D+1.5L	L_
Pos Moment	7 ft-lb	9'2 15/16"	14770 ft-lb	0.000 (0%)	0.9D+1.5L	_L
Unbraced	7 ft-lb	9'2 15/16"	14770 ft-lb	0.000 (0%)	0.9D+1.5L	_L
Shear	978 lb	1'8 11/16"	9277 lb	0.105 (11%)	1.25D+1.5L	L_
Perm Defl in.	0.004 (L/22511)	5'7 5/16"	0.244 (L/360)	0.020 (2%)	D	Uniform
LL Defl inch	0.013 (L/6952)	5'10 1/8"	0.244 (L/360)	0.050 (5%)	L	L_
TL Defl inch	0.017 (L/5316)	5'9 7/16"	0.367 (L/240)	0.050 (5%)	D+L	L_
LL Cant	0.035 (2L/1727)	Lt Cant	0.200 (2L/480)	0.175 (17%)	L	L_
TL Cant	0.048 (2L/1247)	Lt Cant	0.300 (2L/360)	0.161 (16%)	D+L	L_

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Tie-down connection required at bearing 2 for uplift 242 lb (Combination 1.25D+1.5L, Load Case L_).
- 6 Top braced at bearings.
- 7 Bottom braced at bearings.
- 8 Lateral slenderness ratio based on full section width.

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

- Handling & Installation
- andling & Installation
 LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
 approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at beaming points to avoid
 lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318







Page 2 of 2

EWP Studio Simpson Strong-Tie® Component Solutions™

Client: Project:

Address:

GREEN YORK HOMES

Date:

5/31/2018 RCO

Designer: Job Name: LIANA 1 (ELEV.2)

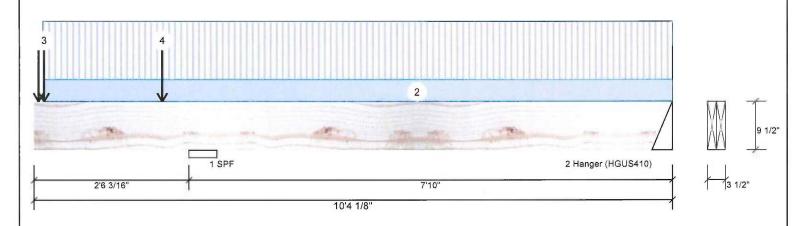
Project #:

Forex 2.0E-3000Fb LVL

1.750" X 9.500"

2-Ply - PASSED

Level: Second Floor



ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Point	0-0-14		Far Face	148 lb	378 lb	0 lb	0 lb	F1
2	Tie-In	0-1-12 to 10-4-2	(Span)0-4-3	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	0-1-15		Near Face	23 lb	42 lb	0 lb	0 lb	F2
4	Point	2-1-0		Near Face	18 lb	49 lb	0 lb	0 lb	J8
	Self Weight				8 PLF				

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS.

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the Intended application, and to verify the dimensions and loads.

Lumber

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- LVL beams must not be cut or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained.
 Provide lateral support at beaming points to avoid lateral displacement and rotation 1.

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318







Client: Project: Address:

GREEN YORK HOMES

Date: 5/31/2018

Designer: RCO

Job Name: LIANA 1 (ELEV.2)

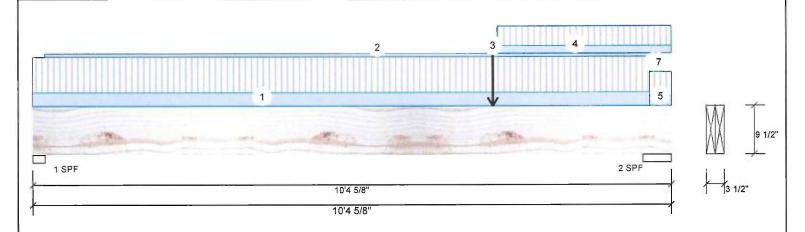
Project #:

Forex 2.0E-3000Fb LVL F6-B

1.750" X 9.500"

2-Ply - PASSED

Level: Second Floor



Member Info	rmation			Unfactored Reactions UNPATTERNED Ib (Uplift)							
Type:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snov	N	Wind	
Plies:	2	Design Method:	LSD	1	208		128		0	0	
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	424		214		0	0	
Deflection LL:	360	Load Sharing:	No								
Deflection TL:	240	Deck:	Not Checked								
Importance:	Normal	Vibration:	Not Checked								
General Load					-						
Floor Live:	40 PSF			Bearings	and Fac	tored l	Reactions				
Dead:	15 PSF			Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.	
				1 - SPF	2.375"	9%	160 / 312	472	L	1.25D+1.5L	

Ana	lysis	Resu	lts

ſ	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
l	Moment	1955 ft-lb	7'5 13/16"	22724 ft-lb	0.086 (9%)	1.25D+1.5L	L
l	Unbraced	1955 ft-lb	7'5 13/16"	20806 ft-lb	0.094 (9%)	1.25D+1.5L	L
l	Shear	817 lb	9'2 3/8"	9277 lb	0.088 (9%)	1.25D+1.5L	L
١	Perm Defl in.	0.016 (L/7225)	5'5 1/8"	0.328 (L/360)	0.050 (5%)	D	Uniform
l	LL Defl inch	0.031 (L/3827)	5'6 3/4"	0.328 (L/360)	0.090 (9%)	L	L
l	TL Defl inch	0.047 (L/2502)	5'6 1/4"	0.493 (L/240)	0.100 (10%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on full section width.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

8%

268 / 636

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.

2 - SPF 5.500"



1.25D+1.5L 1.25D+1.5L

904 L

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o Lateral	siendemess ratio based	on full section width.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 10-0-6	(Span)1-0-14	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Part. Uniform	0-2-6 to 10-0-6		Тор	2 PLF	0 PLF	0 PLF	0 PLF	
3	Point	7-5-13		Near Face	148 lb	378 lb	0 lb	0 lb	F1
4	Tìe-In	7-6-11 to 10-4-10	(Span)0-7-2	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
5	Tie-In	10-0-6 to 10-4-10	(Span)0-8-14	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
6	Part. Uniform	10-0-6 to 10-1-14		Тор	1 PLF	0 PLF	0 PLF	0 PLF	
ontinued or	n page 2								

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
- approvers
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318







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EWP Studio Simpson Strong-Tie® Component Solutions™

Client: Project: Address:

GREEN YORK HOMES

Date: 5/31/2018

Designer: RCO

Job Name: LIANA 1 (ELEV.2)

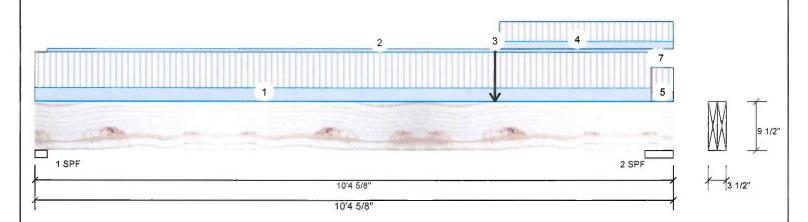
Project #:

Forex 2.0E-3000Fb LVL

1.750" X 9.500"

2-Ply - PASSED

Level: Second Floor



..Continued from page 1

ID Load Type Location Trib Width Side Live Wind Comments Dead Snow 7 Tapered Start 10-1-14 1 PLF 0 PLF 0 PLF 0 PLF Top 10-2-14 0 PLF 0 PLF 0 PLF 0 PLF Self Weight 8 PLF

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the ousformer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- chemicals

Handling & Installation

- andling & Installation
 LVL beams must not be cut or drilled
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 regarding installation requirements, multi-ply
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Forex APA: PR-L318

Manufacturer Info



