



ENGINEERING NOTE PAGE (ENP-1)
PLEASE READ PRIOR TO INSTALLATION

NE0618-004
 GREEN YORK HOMES-LIANA 1 EL 2

RESPONSIBILITIES

THIS DESIGN IS FOR AN INDIVIDUAL BUILDING COMPONENT AND HAS BEEN BASED ON INFORMATION PROVIDED BY THE DESIGN OFFICE OF KOTT LUMBER. THE UNDERSIGNED ENGINEER DISCLAIMS ANY RESPONSIBILITY FOR DAMAGES AS A RESULT OF FAULTY OR INCORRECT INFORMATION, SPECIFICATION AND/OR DESIGNS FURNISHED TO THE ENGINEER. THE UNDERSIGNED ENGINEER IS ONLY RESPONSIBLE FOR THE STRUCTURAL INTEGRITY OF THIS BUILDING COMPONENT FOR THE CONDITIONS AND LOADS SHOWN ON THIS DRAWING. THE STRUCTURAL INTEGRITY OF THE BUILDING AND THE VERIFICATION OF THE DIMENSIONS AND THE DESIGN LOADS USED ARE THE RESPONSIBILITY OF THE BUILDING DESIGNER.

TRUSSES ARE DESIGNED IN CONFORMANCE WITH THE RELEVANT SECTIONS OF THE NATIONAL BUILDING CODE OF CANADA OR THE CANADIAN CODE FOR FARM BUILDINGS, WHICHEVER APPLIES TO THE BUILDING TYPE INDICATED ON THE DRAWING

IT IS THE RESPONSIBILITY OF KOTT LUMBER TO ENSURE THAT TRUSSES ARE MANUFACTURED IN CONFORMANCE WITH THESE DESIGNS AND WITH THE SPECIFICATIONS OUTLINED BELOW. THE UNDERSIGNED ENGINEER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

USE AND OCCUPANCY

- The building is of the type indicated on the drawing

LOADING

- The truss loading intensity and distribution as well as load transfer mechanism is that indicated on the drawing
- No buildings, trees, parapets or other projections higher than the roof for which the trusses are used are located within a distance less than ten (10) times the difference in height, or five metres (16 ft) whichever is greater, unless the drawing indicates that the snow drifting has been taken into account

HANDLING, INSTALLATION AND BRACING

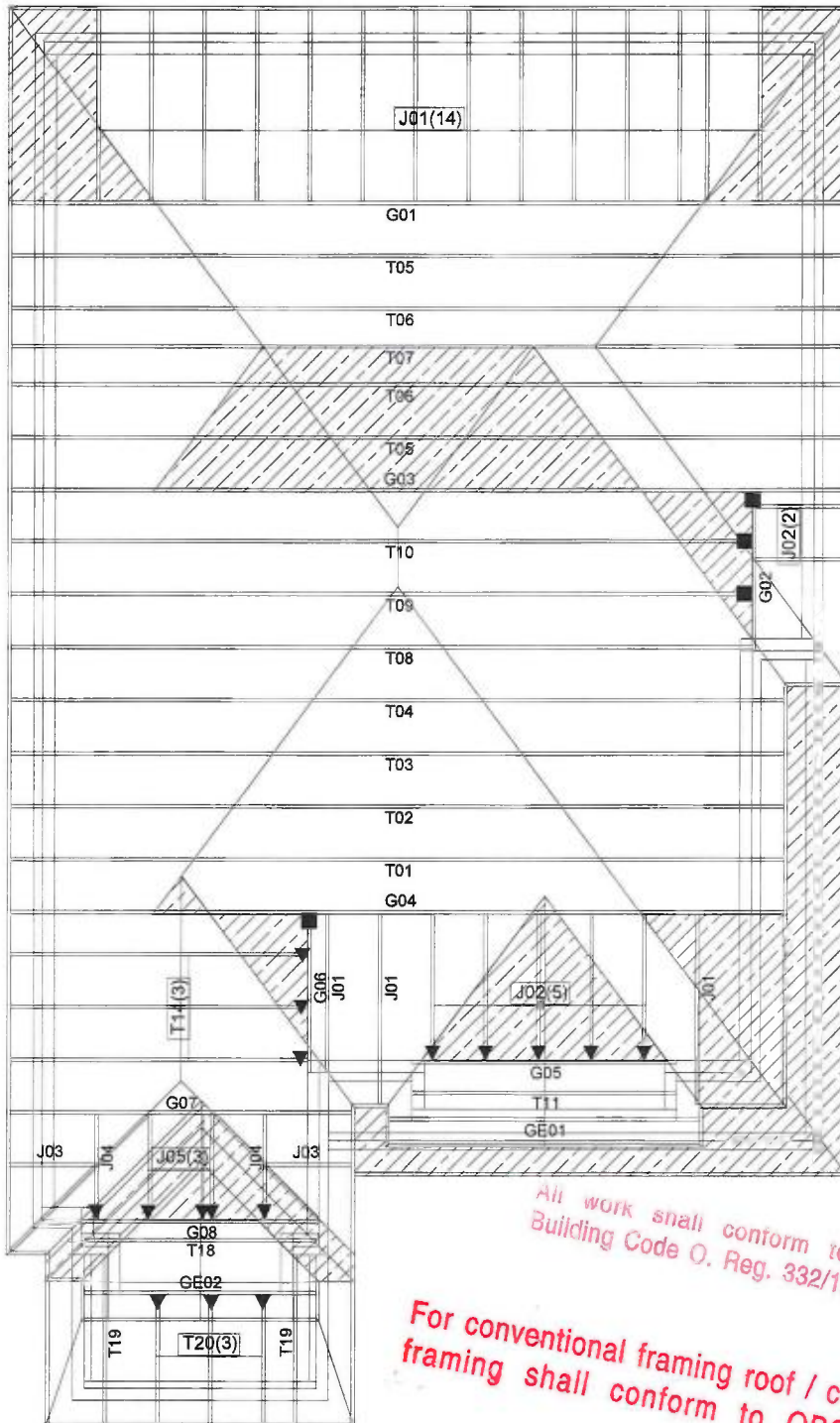
- The trusses must be handled and installed by a qualified professional as per the supplied document titled *Information for Truss Installers* and the BCSI-B1 and BCSI-B3 Summary Sheets
- The compression chords are laterally braced by continuous rigid diaphragm sheathing or as specified on the drawing
- Temporary and permanent bracing must be installed as indicated on the truss drawing and according to the BCSI-B1 and BCSI-B3 Summary Sheets. Bracing for the lateral stability of the truss is to be provided by the building designer
- **It is recommended that a Professional Engineer's advice be obtained for the bracing of trusses spanning more than 12.37m (40'-7")**

SUPPORTS

- The trusses are to be supported at the bearing points indicated and anchored to the supports where considered necessary by the designer of the overall structure
- Bearing sizes shown are the minimum required to prevent crushing of the truss members and do not necessarily take into account stability of the overall building structure
- Elevation of bearings must be carefully checked and shimmed to alignment for solid bearings
- Adequate wood truss bearing is the responsibility of the building designer.

DIMENSIONS

- Geometry of the truss and dimensions indicated on the drawing are identical to those of the installed truss.



THIS CERTIFICATION IS FOR THE ENGINEERING REVIEW OF STRUCTURAL COMPONENTS SHOWN ON THIS DRAWING AND IS LIMITED TO THE COMPONENTS SUPPLIED BY KOTT LUMBER FOR THE LOADS AND CONDITIONS SHOWN ON THE CALCULATION PAGE OF EACH COMPONENT. THE PLACEMENT OF THESE COMPONENTS ON THIS LAYOUT IS SHOWN FOR REFERENCE ONLY. ROOF TRUSSES MUST BE INSTALLED ACCORDING TO MANUFACTURER'S GUIDELINES AT 24" C/C UNLESS NOTED OTHERWISE. CONVENTIONAL FRAMING SHOWN ON THE LAYOUT MUST BE DESIGNED AND APPROVED BY THE PROJECT ENGINEER. TEMPORARY AND PERMANENT BRACINGS OF THE ROOF AND BUILDING ARE THE RESPONSIBILITY OF THE PROJECT ENGINEER AND TO CONFORM TO BCSI GUIDELINES.



ENG.JOB:NE0618-004

The components shown on this layout conform to our recommendations and are adequate to support the design loads required by the Ontario Building Code O.Reg. 332/12 as amended and as shown on the engineering calculation page of each component.

The framing direction and girder locations shown on this layout conform to that shown on the architectural drawing.

Deflection: LL = L360
1.33LL = DL = L360

Architectural Drawing Info:
Date: MAY 22, 2018
Project number: 17-55
Model: LIANA 1

All work shall conform to the Ontario Building Code O. Reg. 332/12 as amended
For conventional framing roof / ceilings framing shall conform to OBC.9.23

19-447186.RR.



CONVENTIONAL FRAMING BY OTHERS

ALL CONVENTIONAL FRAMING TO CONFORM WITH PART 9 OF THE O.B.C. ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4 S.P.F. @ 24" O/C WITH A 2x4 VERTICAL POST TO THE TRUSS UNDERNEATH EACH CROSS POINT. VERTICAL POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END POINT AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

HANGER LEGEND:

▼ LUS24 ■ LJS26DS
● HGUS26 ✕ HGUS26-2

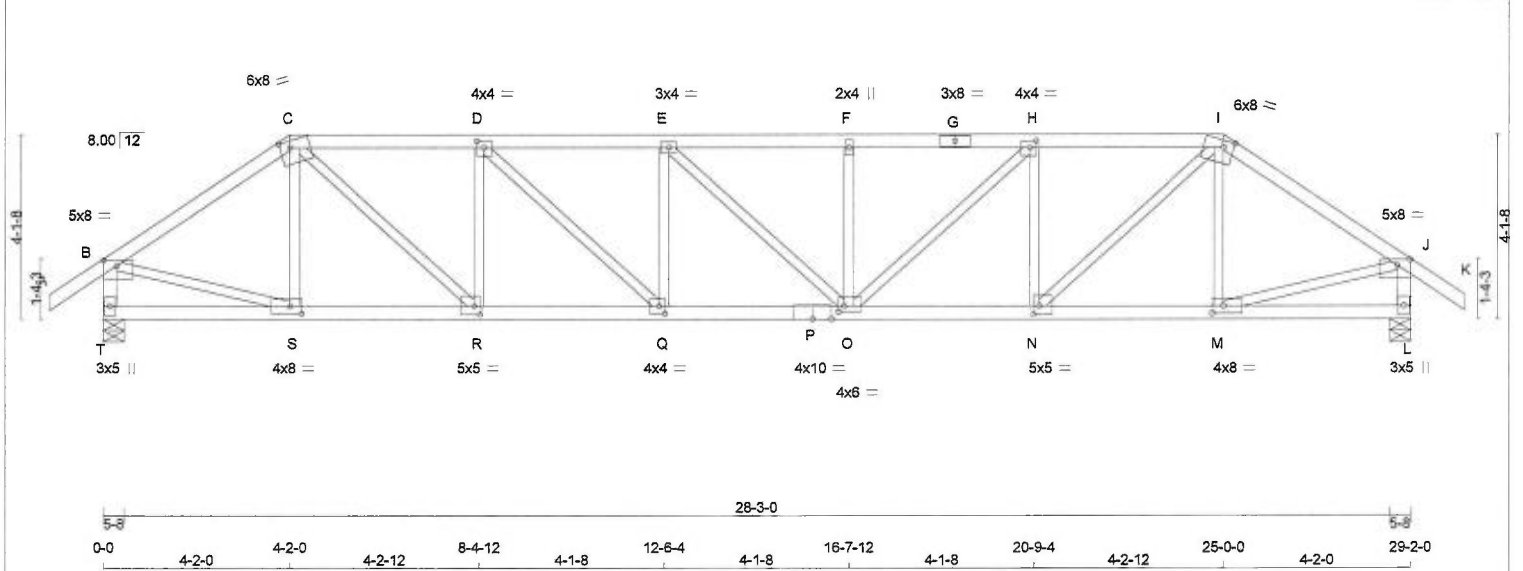
SIZE AND LOCATION OF CONVENTIONAL FRAMING IS APPROXIMATE. ALL AREAS MAY NOT BE SHOWN. REFER TO ARCHITECTURAL PLANS FOR DETAILS.

DESIGN CRITERIA
SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3 PSF
BOT CH. LL = 0 PSF
DL = 7 PSF
TOTAL LOAD = 33.3 PSF
SPACING = 24" O/C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 085-09
- TPIC 2011

Model: LIANA 1 EL 2 3&4 BED
Customer: GREEN YORK HOMES
Project: GRANELLI HOMES
Location: BRAMPTON
Date: 5/30/2018 Drawn by: BB



TOTAL WEIGHT = 120 lb

LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.	FACTORED	MAXIMUM FACTORED	INPUT
A - C	2x4	DRY	No.2	SPF	GROSS REACTION	GROSS REACTION	BRG
C - G	2x4	DRY	No.2	SPF	VERT	DOWN	UPLIFT
G - I	2x4	DRY	No.2	SPF	2664	0	0
I - K	2x4	DRY	No.2	SPF	2664	0	0
T - B	2x4	DRY	No.2	SPF	2664	0	0
L - J	2x4	DRY	No.2	SPF	2664	0	0
T - P	2x4	DRY	No.2	SPF	2664	0	0
P - L	2x4	DRY	No.2	SPF	2664	0	0
ALL WEBS	2x3	DRY	No.2	SPF	2664	0	0
EXCEPT							

DRY: SEASONED LUMBER.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF

DL = 3.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.0 PSF

TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip

SIDE SETBACK = 4'-2-0

END SETBACK = 6'-0-0

END WALL WIDTH = 5'-8

CORNER FRAMING TYPE: CONVENTIONAL

END JACK TYPE: CONVENTIONAL

APPLIED TO FRONT SIDE

-ADDTL LOADS BASED ON 55 % OF GSL

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014

- CSA 086-09

- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")

CALCULATED VERT. DEFL.(LL) = U/999 (0.26")

ALLOWABLE DEFL.(TL)= L/360 (0.97")

CALCULATED VERT. DEFL.(TL) = U/751 (0.47")

CSI: TC=0.83/1.00 (F-H:1), BC=0.96/1.00 (O-Q:1),

WB=0.66/1.00 (B-S:1), SSI=0.31/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00

COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PSI) (PLI) (PLI)

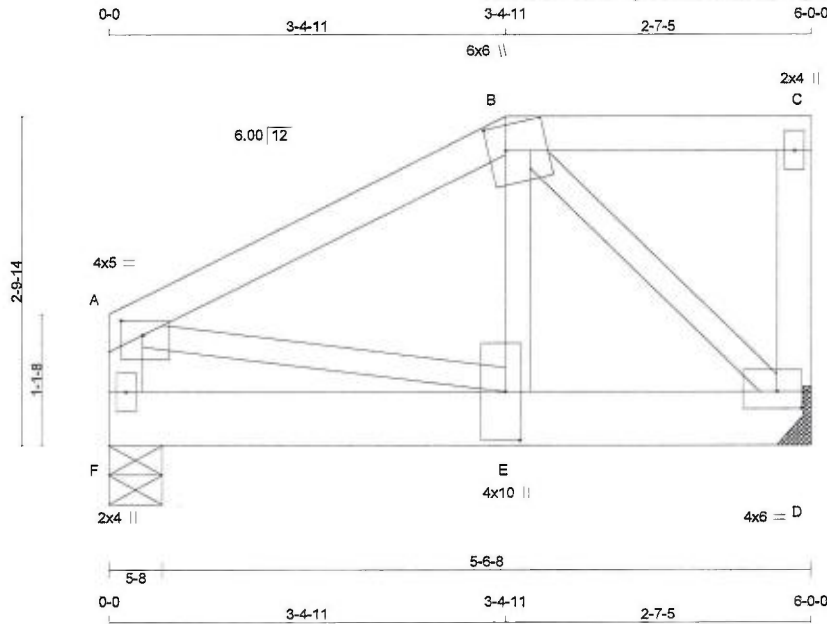
MAX MIN MAX MIN MAX MIN

MT20 618 354 1667 822 2284 1656

CONTINUED ON PAGE 2

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

KOTT



Scale = 1:18.9

TOTAL WEIGHT = 28 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - B	2x4 DRY	No.2		
B - C	2x4 DRY	No.2		
D - C	2x4 DRY	No.2		
F - A	2x4 DRY	No.2		
F - D	2x6 DRY	No.2		

ALL WEBS 2x3 DRY No.2 EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	5.0	1.50	2.25
B	TTWW+m	MT20	6.0	6.0	2.50	1.75
C	TMV+p	MT20	2.0	4.0		
D	BMVW1-t	MT20	4.0	6.0	1.75	2.50
E	BMVW1-t	MT20	4.0	10.0	5.00	1.50
F	BMV1+p	MT20	2.0	4.0		

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 31.0 lbs FACTORED DOWN AT 3-4-11 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

DESCR.	SPF	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REORD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
D	2023	0	2023	0	0
F	2026	0	2026	0	0

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT D TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
D	1419	998 / 0	0 / 0	0 / 0	424 / 0	0 / 0
F	1421	998 / 0	0 / 0	0 / 0	424 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.02 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)
FR-TO		FROM TO			FR-TO		
A-B	-1568 / 0	-77.4 -77.4	0.21 (1)	5.02	E-B	0 / 1830	0.45 (1)
B-C	0 / 0	-72.5 -72.5	0.09 (1)	10.00	B-D	-2024 / 0	0.45 (1)
D-C	-95 / 0	0.0 0.0	0.01 (1)	7.81	A-E	0 / 1432	0.35 (1)
F-A	-1130 / 0	0.0 0.0	0.12 (1)	7.43			
F-E	0 / 0	-594.5 -594.5	0.50 (1)	10.00			
E-D	0 / 1479	-594.5 -594.5	0.56 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
B	3-4-11	-31	-31	--	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3 PSF
DL = 3.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.0 PSF	
TOTAL LOAD = 33.3 PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 3-4-11
RIGHT SETBACK = 0-0
END SETBACK = 2-2-8
END WALL WIDTH = 5-8
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDTL LOADS BASED ON 55 % OF GSL.

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 26-10-0
END DISTANCE = 6-0-0
END SPAN CARRIED = 26-10-0
END WALL WIDTH = 5-8
APPLIED TO BACK SIDE OF BOTTOM CHORD.
- ADDTL LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $\frac{1}{360}$ (0.20")
CALCULATED VERT. DEFL.(LL) = $\frac{1}{999}$ (0.03")
ALLOWABLE DEFL.(TL) = $\frac{1}{360}$ (0.20")
CALCULATED VERT. DEFL.(TL) = $\frac{1}{999}$ (0.06")

CSI: TC=0.21/1.00 (A-B:1), BC=0.56/1.00 (D-E:1), WB=0.45/1.00 (B-E:1), SSI=0.78/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

CONTINUED ON PAGE 2



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX MIN MAX MIN
MT20	618	354	1667 622 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

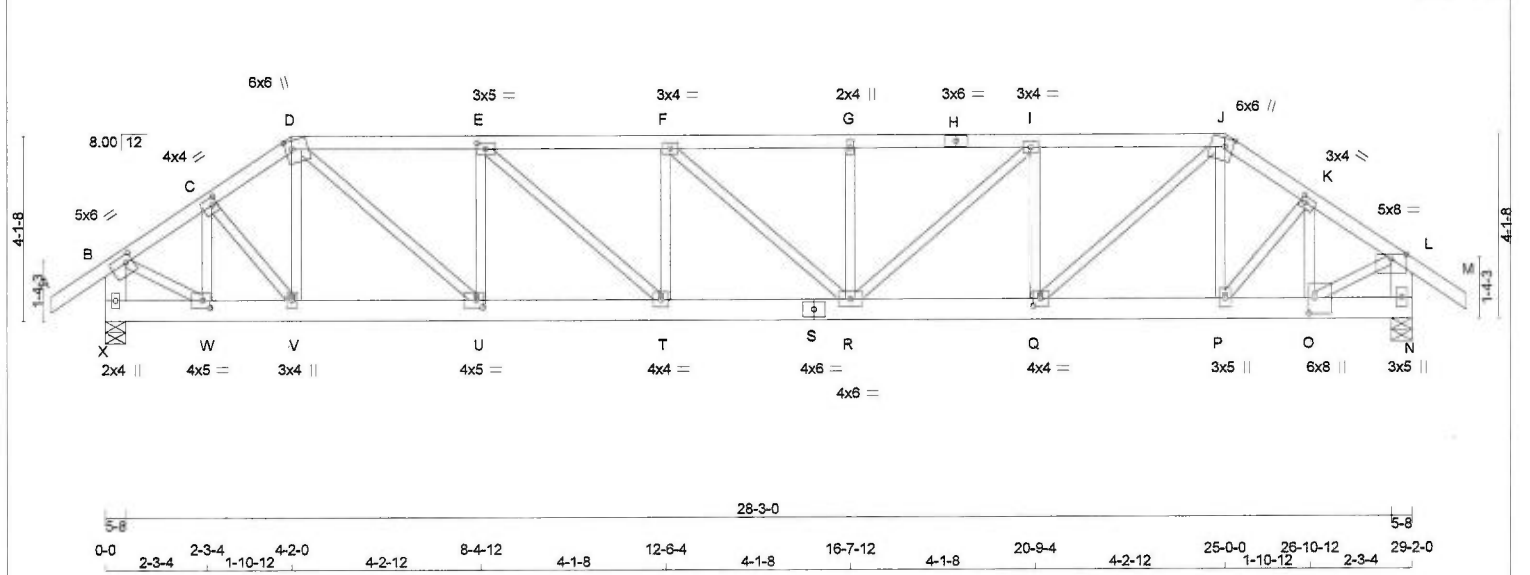
JSI GRIP= 0.89 (E) (INPUT = 0.90)

JSI METAL= 0.48 (E) (INPUT = 1.00)

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Version 8.210 S Mar 12 2018 MiTek Industries, Inc. Sat Jun 2 11:57:09 2018 Page 1
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Scale = 1:49.4



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - H	2x4	DRY	No.2	SPF	
H - J	2x4	DRY	No.2	SPF	
J - M	2x4	DRY	No.2	SPF	
X - B	2x6	DRY	No.2	SPF	
N - L	2x6	DRY	No.2	SPF	
X - S	2x6	DRY	No.2	SPF	
S - N	2x6	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.00	1.75	
C	TMVW-t	MT20	4.0	4.0	2.00	1.50	
D	TTVW+m	MT20	6.0	6.0	2.25	1.75	
E	TMVW-t	MT20	3.0	5.0	1.50	2.25	
F	TMVW-t	MT20	3.0	4.0			
G	TMVW-w	MT20	2.0	4.0			
H	TS-t	MT20	3.0	6.0			
I	TMVW-t	MT20	3.0	4.0			
J	TTVW+m	MT20	6.0	6.0	2.25	2.25	
K	TMVW-t	MT20	3.0	4.0	1.50	1.25	
L	TMVW-p	MT20	5.0	8.0	1.25	4.00	
N	BMV1+p	MT20	3.0	5.0			
O	BMVW+t	MT20	6.0	8.0	4.25	1.50	
P	BMVW+t	MT20	3.0	5.0			
Q	BMVW-t	MT20	4.0	4.0	2.00	1.75	
R	BMVW-t	MT20	4.0	6.0			
S	BS-t	MT20	4.0	6.0			
T	BMVW-t	MT20	4.0	4.0			
U	BMVW-t	MT20	4.0	5.0	2.00	1.75	
V	BMVW-t	MT20	3.0	4.0			
W	BMVW-t	MT20	4.0	5.0	2.00	2.00	
X	BMV1+p	MT20	2.0	4.0			



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DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	UPLIFT
X	1641	0	1641	0
N	3348	0	3348	0

UNFACTORED REACTIONS	1ST LCASE	MAX/MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
X	1149	817 / 0	0 / 0
N	2347	1655 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.49 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	CS1 (LC)	WEBS	MAX. FACTORED MEMB.	FORCE (LBS)	MAX. CS1 (LC)
FR-TO			FROM	TO		FR-TO			
A-B	0 / 28		-77.4	-77.4	0.10 (1)	10.00	W-C	-587 / 0	0.11 (1)
B-C	-1569 / 0		-77.4	-77.4	0.09 (1)	5.16	C-V	0 / 325	0.08 (1)
C-D	-1837 / 0		-77.4	-77.4	0.08 (1)	4.86	V-D	-126 / 4	0.03 (1)
D-E	-2584 / 0		-77.4	-77.4	0.32 (1)	3.99	D-U	0 / 1430	0.35 (1)
E-F	-3179 / 0		-77.4	-77.4	0.37 (1)	3.58	U-E	-894 / 0	0.23 (1)
F-G	-3342 / 0		-77.4	-77.4	0.33 (1)	3.55	E-T	0 / 804	0.20 (1)
G-H	-3342 / 0		-77.4	-77.4	0.39 (1)	3.49	T-F	-460 / 0	0.12 (1)
H-I	-3342 / 0		-77.4	-77.4	0.39 (1)	3.49	F-R	0 / 221	0.05 (1)
I-J	-3103 / 0		-77.4	-77.4	0.37 (1)	3.63	R-G	-307 / 0	0.08 (1)
J-K	-2862 / 0		-77.4	-77.4	0.17 (1)	3.95	R-I	0 / 322	0.08 (1)
K-L	-3340 / 0		-77.4	-77.4	0.23 (1)	3.62	Q-I	-570 / 0	0.15 (1)
L-M	0 / 28		-77.4	-77.4	0.10 (1)	10.00	Q-J	0 / 942	0.23 (1)
X-B	-1596 / 0		0.0	0.0	0.11 (1)	7.74	P-J	0 / 754	0.19 (1)
N-L	-3199 / 0		0.0	0.0	0.23 (1)	5.87	P-K	-633 / 0	0.13 (1)
							O-K	0 / 592	0.15 (1)
X-W	0 / 0		-17.5	-17.5	0.04 (1)	10.00	B-W	0 / 1420	0.35 (1)
W-V	0 / 1311		-17.5	-17.5	0.21 (1)	10.00	O-L	0 / 3007	0.74 (1)
V-U	0 / 1513		-17.5	-17.5	0.23 (1)	10.00			
U-T	0 / 2584		-17.5	-17.5	0.38 (1)	10.00			
T-S	0 / 3179		-17.5	-17.5	0.47 (1)	10.00			
S-R	0 / 3179		-17.5	-17.5	0.47 (1)	10.00			
R-Q	0 / 3103		-17.5	-17.5	0.46 (1)	10.00			
Q-P	0 / 2398		-17.5	-17.5	0.37 (1)	10.00			
P-O	0 / 2776		-17.5	-17.5	0.58 (1)	10.00			
O-N	0 / 0		-17.5	-17.5	0.21 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)	JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
O	25-10-12	-2022	-2022	--	--	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:	TOP CH.	LL	PSF
	DL	=	23.3
	BOT CH.	LL	PSF
	DL	=	0.0
	DL	=	7.0
	TOTAL LOAD	=	33.3

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCG 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.27")

CSI: TC=0.39/1.00 (G-I:1), BC=0.58/1.00 (O-P:1),
WB=0.74/1.00 (L-O:1), SSI=0.17/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	MAX MIN	MAX MIN
MT20	618	354	1667	822	2284	1656

CONTINUED ON PAGE 2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN YORK HOMES-LIANA 1 EL 2	DRWG NO.
NE0618-004	G03	1	1	TRUSS DESC.		PAGE 8 OF 41

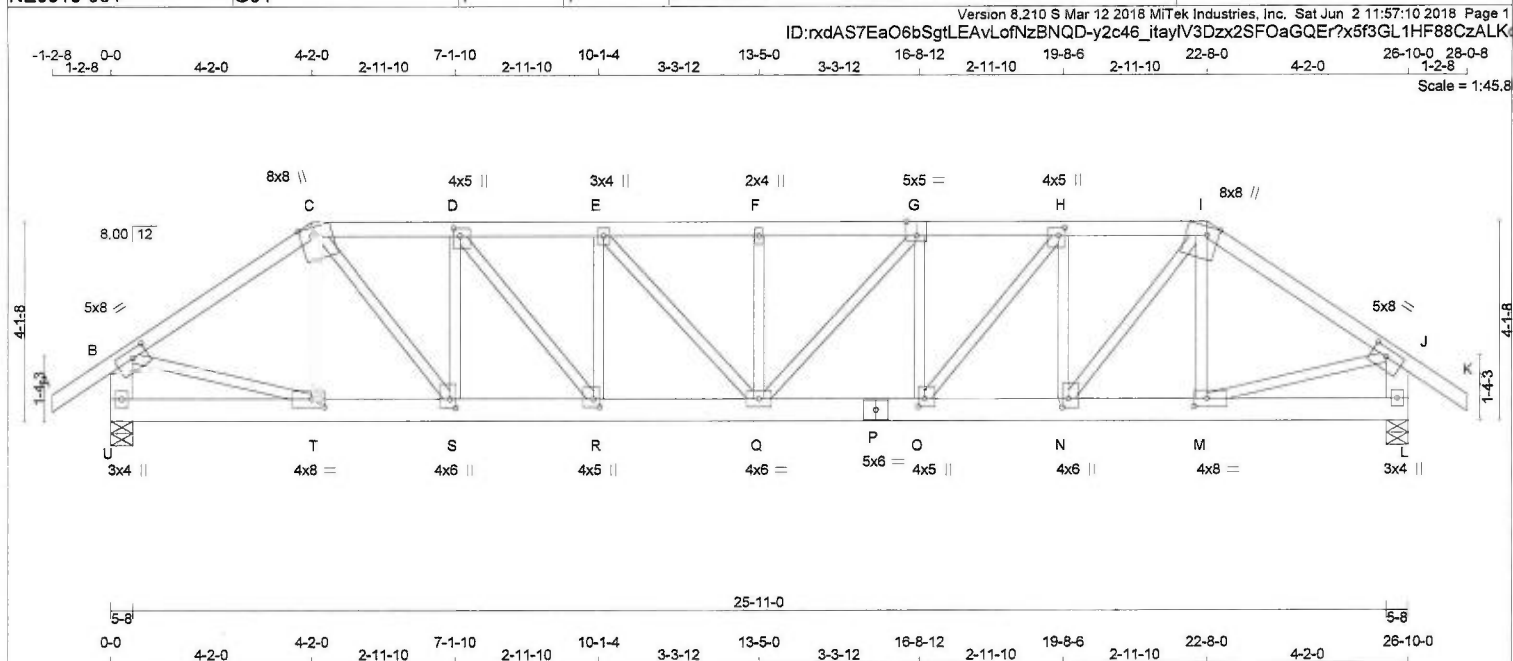
Version 8.210 S Mar 12 2018 MiTek Industries, Inc. Sat Jun 2 11:57:09 2018 Page 2
ID: rxdAS7EaO6bSgtLEAvLofNzBNQD-U52iuehEpdeR3OkUkk923t5lce5wcRCodVacIzALK

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 2021.9 lbs FACTORED DOWN AT
26-10-12 ON BOTTOM CHORD. DESIGN FOR
UNSPECIFIED CONNECTION(S) IS DELEGATED
TO THE BUILDING DESIGNER.

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.88 (S) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 134 lb

LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.	BEARINGS			
A - C	2x4	DRY	No.2	SPF	FACTORED	MAXIMUM FACTORED	INPUT
C - G	2x4	DRY	No.2	SPF	GROSS REACTION	GROSS REACTION	BRG
G - I	2x4	DRY	No.2	SPF	JT VERT	DOWN	UPLIFT
I - K	2x4	DRY	No.2	SPF	U	2339	0
U - B	2x6	DRY	No.2	SPF	L	2591	0
L - J	2x6	DRY	No.2	SPF			
U - P	2x6	DRY	No.2	SPF			
P - L	2x6	DRY	No.2	SPF			

UNFACTORED REACTIONS				1ST LCASE			
JT	COMBINED	SNOW	LIVE	PERM.	LIVE	WIND	DEAD
U	1639	1161 / 0	0 / 0	0 / 0	0 / 0	478 / 0	0 / 0
L	1817	1276 / 0	0 / 0	0 / 0	0 / 0	541 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.72 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-77.4 -77.4	0.10 (1)	T-C	-436 / 0	0.11 (1)	
B-C	-2759 / 0	-77.4 -77.4	0.41 (1)	C-S	0 / 2161	0.53 (1)	
C-D	-3624 / 0	-77.4 -77.4	0.29 (1)	S-D	-1612 / 0	0.41 (1)	
D-E	-4729 / 0	-77.4 -77.4	0.42 (1)	D-R	0 / 1781	0.44 (1)	
E-F	-4894 / 0	-145.9 -145.9	0.53 (1)	R-E	-538 / 0	0.14 (1)	
F-G	-4894 / 0	-145.9 -145.9	0.53 (1)	E-Q	0 / 249	0.06 (1)	
G-H	-4510 / 0	-145.9 -145.9	0.47 (1)	Q-F	-499 / 0	0.13 (1)	
H-I	-3739 / 0	-145.9 -145.9	0.37 (1)	Q-G	0 / 564	0.14 (1)	
I-J	-3058 / 0	-77.4 -77.4	0.45 (1)	O-G	-872 / 0	0.22 (1)	
J-K	0 / 28	-77.4 -77.4	0.10 (1)	O-H	0 / 1242	0.31 (1)	
U-B	-2304 / 0	0.0 0.0	0.16 (1)	N-H	-1437 / 0	0.37 (1)	
L-J	-2525 / 0	0.0 0.0	0.18 (1)	N-I	0 / 1946	0.48 (1)	
				M-I	-422 / 0	0.11 (1)	
U-T	0 / 0	-17.5 -17.5	0.04 (4)	B-T	0 / 2348	0.58 (1)	
T-S	0 / 2280	-17.5 -17.5	0.31 (1)	M-J	0 / 2601	0.64 (1)	
S-R	0 / 3624	-17.5 -17.5	0.55 (1)				
R-Q	0 / 4729	-33.0 -33.0	0.72 (1)				
Q-P	0 / 4525	-33.0 -33.0	0.67 (1)				
P-O	0 / 4525	-33.0 -33.0	0.67 (1)				
O-N	0 / 3739	-33.0 -33.0	0.54 (1)				
N-M	0 / 2528	-33.0 -33.0	0.36 (1)				
M-L	0 / 0	-33.0 -33.0	0.07 (4)				

FACTORED CONCENTRATED LOADS (LBS)				TYPE	
JT	LOC.	LC1	MAX- MAX+	FRONT	DIR.
I	22-8-0	-242	-242	FRONT	VERT
R	10-1-4	-824	-824	FRONT	VERT

CS1: TC=0.53/1.00 (E-F:1), BC=0.72/1.00 (Q-R:1), WB=0.64/1.00 (J-M:1), SS1=0.22/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

CONTINUED ON PAGE 2

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 241.8 lbs FACTORED DOWN AT 22-8-0 ON TOP CHORD, AND 823.6 lbs FACTORED DOWN AT 10-1-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

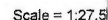
PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (G) (INPUT = 0.90)
JSI METAL= 0.81 (P) (INPUT = 1.00)

The logo for KOTT, featuring a stylized 'K' made of two overlapping triangles (one green, one blue) followed by the word 'KOTT' in a bold, blue, sans-serif font.



TOTAL WEIGHT = 38 lb

DRY: SEASONED LUMBER.

PLATES (table is in inches)

BUILDING BEARINGS

UNFACTORED REACTIONS

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F. D.

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	-738 / 0	-77.4	-77.4	0.39 (1)	6.25	E-B	0 / 432 0.11 (1)
B-C	-738 / 0	-77.4	-77.4	0.39 (1)	6.25	A-E	0 / 622 0.15 (1)
F-A	-702 / 0	0.0	0.0	0.08 (1)	7.81	E-C	0 / 622 0.15 (1)
D-C	-702 / 0	0.0	0.0	0.08 (1)	7.81		
F-E	0 / 0	-101.5	-101.5	0.46 (1)	10.00		
E-D	0 / 0	-101.5	-101.5	0.46 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD =		33.3	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 5-6-8
END DISTANCE = 10-0-0
END SPAN CARRIED = 5-6-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
-ADD'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9 NRCG 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.33")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.33")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.39/1.00 (B-C:1), BC=0.46/1.00 (E-F:1),

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

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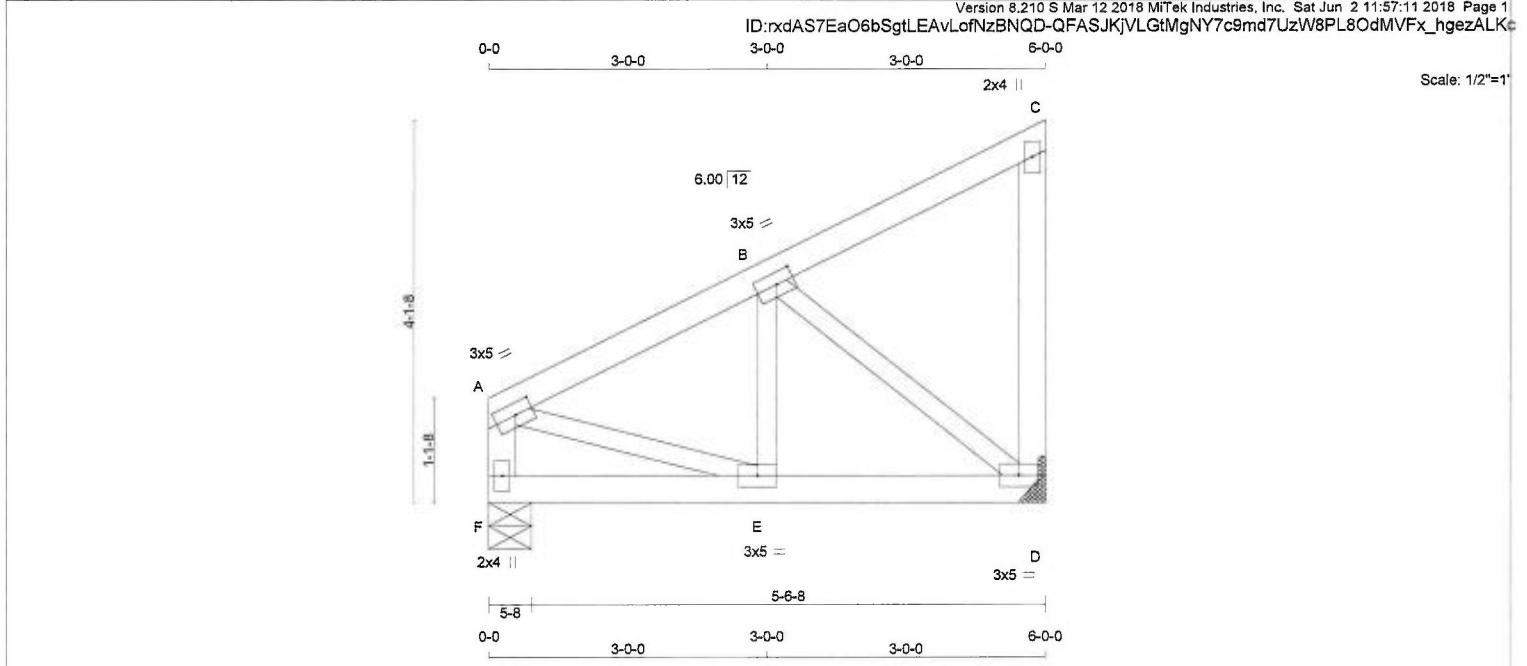


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THE DESIGN OF THIS COMPONENT.**

JSI GRIP= 0.80 (C) (INPUT = 0.90)
JSI METAL= 0.25 (C) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
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THE DESIGN OF THIS COMPONENT.





LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. **DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER** **BEARINGS** **DESIGN CRITERIA** **SPACING = 24.0 IN. C/C** **TOTAL WEIGHT = 26 lb**

CHORDS	SIZE	LUMBER	DESCR.	FACTORED	MAXIMUM FACTORED	INPUT	REQD
F - A	2x4	DRY	No.2	SPF	GROSS REACTION	BRG	BRG
A - C	2x4	DRY	No.2	SPF	DOWN	IN-SX	IN-SX
D - C	2x4	DRY	No.2	SPF	UP	MECHANICAL	
F - D	2x4	DRY	No.2	SPF			
ALL WEBS	2x3	DRY	No.2	SPF			
DRY: SEASONED LUMBER.							

PLATES (table is in inches) **UNFACTORED REACTIONS** **BRACING** **LOADING** **COMPANION LIVE LOAD FACTOR = 0.50**

JT	TYPE	PLATES	W	LEN	Y	X	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
A	TMVW-t	MT20	3.0	5.0	1.50	2.25	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
B	TMVW-t	MT20	3.0	5.0	1.50	2.25						
C	TMVW-p	MT20	2.0	4.0								
D	BMVW1-t	MT20	3.0	5.0								
E	BMVW-t	MT20	3.0	5.0								
F	BMV1-p	MT20	2.0	4.0								

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F **THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010** **THIS DESIGN COMPLIES WITH:** **- PART 9 OF OBC 2012, BCBC 2012, ABC 2014** **- CSA 086-09** **- TPIC 2011** **(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD** **ALLOWABLE DEFL.(LL) = L/360 (0.20")** **CALCULATED VERT. DEFL.(LL) = L/999 (0.01")** **ALLOWABLE DEFL.(TL) = L/360 (0.20")** **CALCULATED VERT. DEFL.(TL) = L/999 (0.03")** **CSI: TC=0.11/1.00 (A-B:1), BC=0.41/1.00 (D-E:1), WB=0.21/1.00 (B-D:1), SSI=0.31/1.00 (E-F:1)** **DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00** **TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.** **NAIL VALUES** **PLATE GRIP(DRY) SHEAR SECTION** **(PSI) (PLI) (PLI)** **MAX MIN MAX MIN MAX MIN** **MT20 618 354 1667 822 2284 1656** **PLATE PLACEMENT TOL. = 0.250 inches** **PLATE ROTATION TOL. = 5.0 Deg.**

MEMB.	MAX. FACTORED	FACTORED	MAX. FACTORED	MEMB.	MAX. FACTORED	MEMB.	MAX. FACTORED
FR-TO	FORCE (LBS)	VERT. LOAD (PLF)	CS (LC)	UNBRAC	FORCE (LBS)	CS (LC)	UNBRAC
F-A	-594 / 0	0.0	0.0 0.07 (1)	7.81	A-E	0 / 673	0.17 (1)
A-B	-715 / 0	-77.4	-77.4 0.11 (1)	6.25	E-B	0 / 547	0.14 (1)
B-C	-11 / 0	-77.4	-77.4 0.11 (1)	6.25	B-D	-821 / 0	0.21 (1)
D-C	-92 / 0	0.0	0.0 0.02 (1)	7.81			
F-E	0 / 0	-197.3	-197.3 0.30 (1)	10.00			
E-D	0 / 649	-197.3	-197.3 0.41 (1)	10.00			

COMPANION LIVE LOAD FACTOR = 0.50 **TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.** **NAIL VALUES** **PLATE GRIP(DRY) SHEAR SECTION** **(PSI) (PLI) (PLI)** **MAX MIN MAX MIN MAX MIN** **MT20 618 354 1667 822 2284 1656** **PLATE PLACEMENT TOL. = 0.250 inches** **PLATE ROTATION TOL. = 5.0 Deg.**

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

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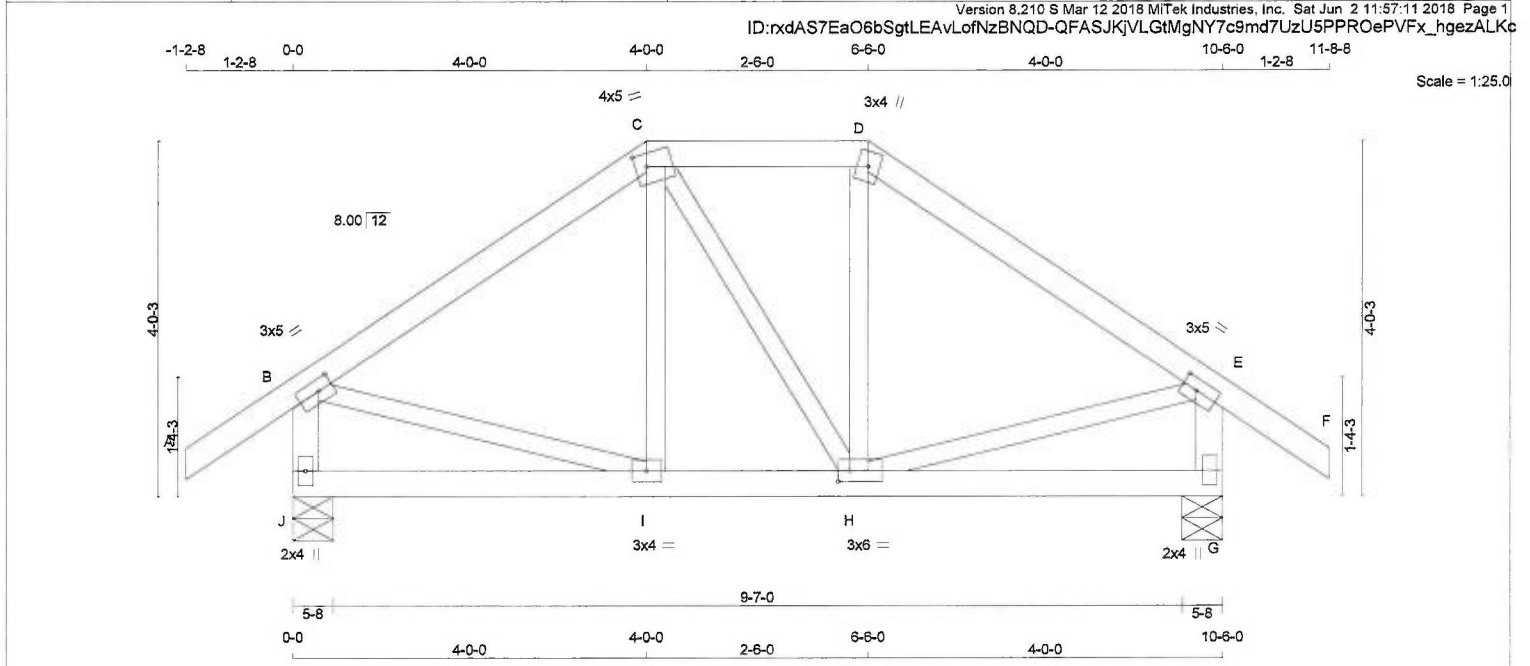
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN YORK HOMES-LIANA 1 EL 2	DRWG NO.
NE0618-004	G06	1	1	TRUSS DESC.		PAGE 14 OF 41

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ID: rxdAS7EaO6bSgtLEAvLofNzBNQD-QFASJKjVLGtMgNY7c9md7UzW8PL8OdMVFX_hgezALK

JSI GRIP= 0.82 (E) (INPUT = 0.90)
JSI METAL= 0.23 (A) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
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THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.





LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4 DRY	No.2	SPF		
C - D	2x4 DRY	No.2	SPF		
D - F	2x4 DRY	No.2	SPF		
J - B	2x4 DRY	No.2	SPF		
G - E	2x4 DRY	No.2	SPF		
J - G	2x4 DRY	No.2	SPF		
ALL WEBS	2x3 DRY	No.2	SPF		
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	3.0	5.0	1.50	2.00	
C	TTWV-m	MT20	4.0	5.0	1.75	1.50	
D	TTW+m	MT20	3.0	4.0			
E	TMVW-t	MT20	3.0	5.0	1.50	2.00	
G	BMV1+p	MT20	2.0	4.0			
H	BMVWV-t	MT20	3.0	6.0	1.50	1.50	
I	BMVW-t	MT20	3.0	4.0			
J	BMV1+p	MT20	2.0	4.0			

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 128.9 lbs FACTORED DOWN AT 6-6-0, AND 128.9 lbs FACTORED DOWN AT 4-0-0 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
J	799	0	799	0
G	799	0	799	0

UNFACTORED REACTIONS	1ST LCASE	MAX/MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
J	559	405 / 0	0 / 0
G	559	405 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MAX. UNBRAC LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)
FR-TO			FROM TO			FR-TO			
A-B	0 / 28	-77.4	-77.4	0.10 (1)	10.00	I-C	-52 / 56	0.02 (4)	
B-C	-656 / 0	-77.4	-77.4	0.25 (1)	6.25	C-H	0 / 0	0.00 (1)	
C-D	-543 / 0	-107.2	-107.2	0.13 (1)	6.25	H-D	-53 / 57	0.02 (4)	
D-E	-655 / 0	-77.4	-77.4	0.25 (1)	6.25	B-I	0 / 562	0.14 (1)	
E-F	0 / 28	-77.4	-77.4	0.10 (1)	10.00	H-E	0 / 561	0.14 (1)	
J-B	-756 / 0	0.0	0.0	0.09 (1)	7.81				
G-E	-756 / 0	0.0	0.0	0.09 (1)	7.81				
J-I	0 / 0	-24.2	-24.2	0.10 (4)	10.00				
I-H	0 / 543	-24.2	-24.2	0.14 (1)	10.00				
H-G	0 / 0	-24.2	-24.2	0.09 (4)	10.00				

FACTORED CONCENTRATED LOADS (LBS)	JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	4-0-0	-129	-129	---	---	FRONT	VERT	TOTAL
D	6-6-0	-129	-129	---	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 4-0-0
END SETBACK = 4-0-0
END WALL WIDTH = 5-8
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDTL LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.25/1.00 (B-C:1), BC=0.14/1.00 (H-I:1), WB=0.14/1.00 (B-I:1), SSI=0.11/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)	(PLI)
	MAX	MIN	MAX	MIN
MT20	618	354	1667	822

CONTINUED ON PAGE 2



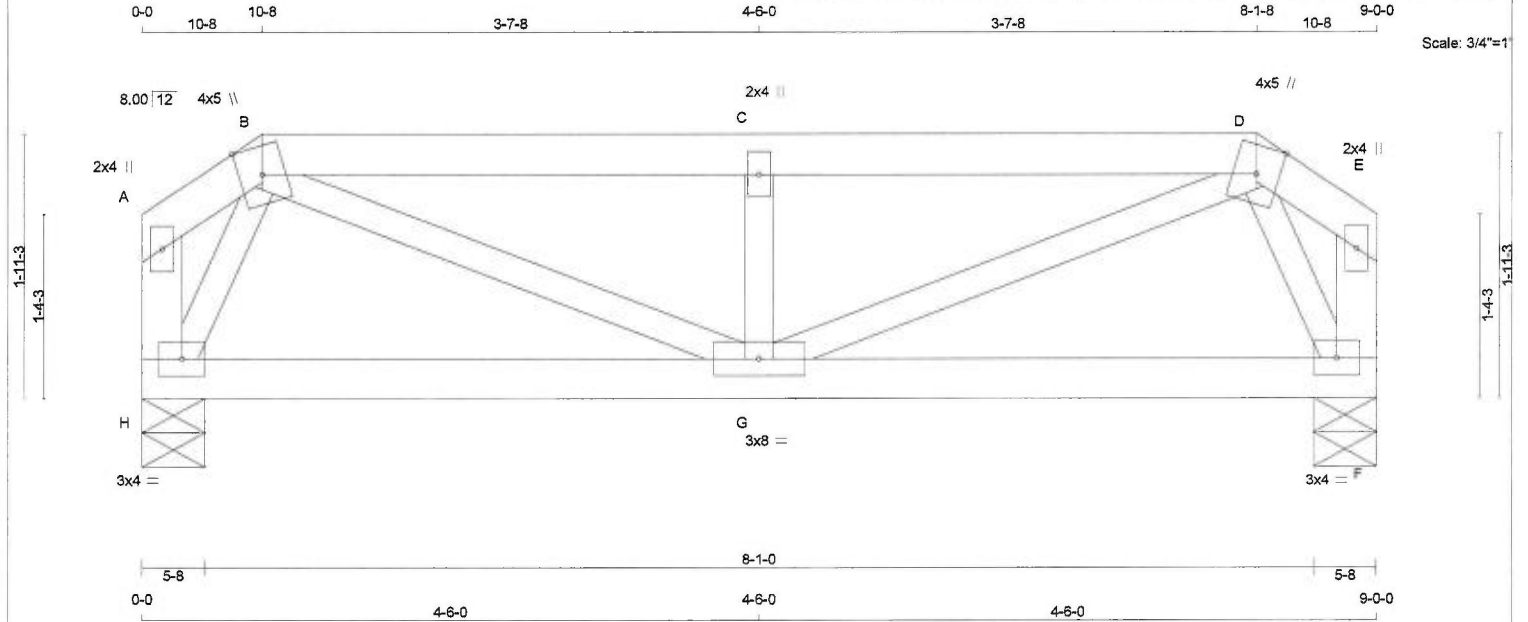
READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP= 0.86 (D) (INPUT = 0.90)
JSI METAL= 0.24 (B) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
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TOTAL WEIGHT = 33 lb [M]

LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0		
B	TTWW+m	MT20	4.0	5.0	Edge	
C	TMW+w	MT20	2.0	4.0		
D	TTWW+m	MT20	4.0	5.0	Edge	
E	TMV+p	MT20	2.0	4.0		
F	BMVW1-t	MT20	3.0	4.0		
G	BMVW1-t	MT20	3.0	8.0		
H	BMVW1-t	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
H	640	0	640	0	5-8	5-8
F	640	0	640	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
H	449	314 / 0	0 / 0	0 / 0	0 / 0	135 / 0	0 / 0
F	449	314 / 0	0 / 0	0 / 0	0 / 0	135 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.99 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 0	-77.4 -77.4 0.01 (1)	10.00	B-G	0 / 829	0.21 (1)	
B-C	-1021 / 0	-77.4 -77.4 0.16 (1)	5.99	G-C	-324 / 0	0.05 (1)	
C-D	-1021 / 0	-77.4 -77.4 0.16 (1)	5.99	G-D	0 / 829	0.21 (1)	
D-E	0 / 0	-77.4 -77.4 0.01 (1)	10.00	H-B	-558 / 0	0.09 (1)	
H-A	-34 / 0	0.0 0.0 0.00 (1)	7.81	D-F	-558 / 0	0.09 (1)	
F-E	-34 / 0	0.0 0.0 0.00 (1)	7.81				
H-G	0 / 264	-64.9 -64.9 0.26 (1)	10.00				
G-F	0 / 264	-64.9 -64.9 0.26 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3 PSF
DL = 3.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.0 PSF	
TOTAL LOAD = 33.3 PSF	

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CSldGirder
START DISTANCE = 0-0
END SPAN CARRIED = 4-0-0
END DISTANCE = 9-0-0
END SPAN CARRIED = 4-0-0
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDTL LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.30")
CALCULATED VERT. DEFL.(LL)= L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.30")
CALCULATED VERT. DEFL.(TL)= L/999 (0.05")

CSI: TC=0.16/1.00 (B-C:1), BC=0.26/1.00 (G-H:1),
WB=0.21/1.00 (D-G:1), SSI=0.15/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
		MAX MIN	MAX MIN
MT20	618	354	1667 822 2284 1656

CONTINUED ON PAGE 2



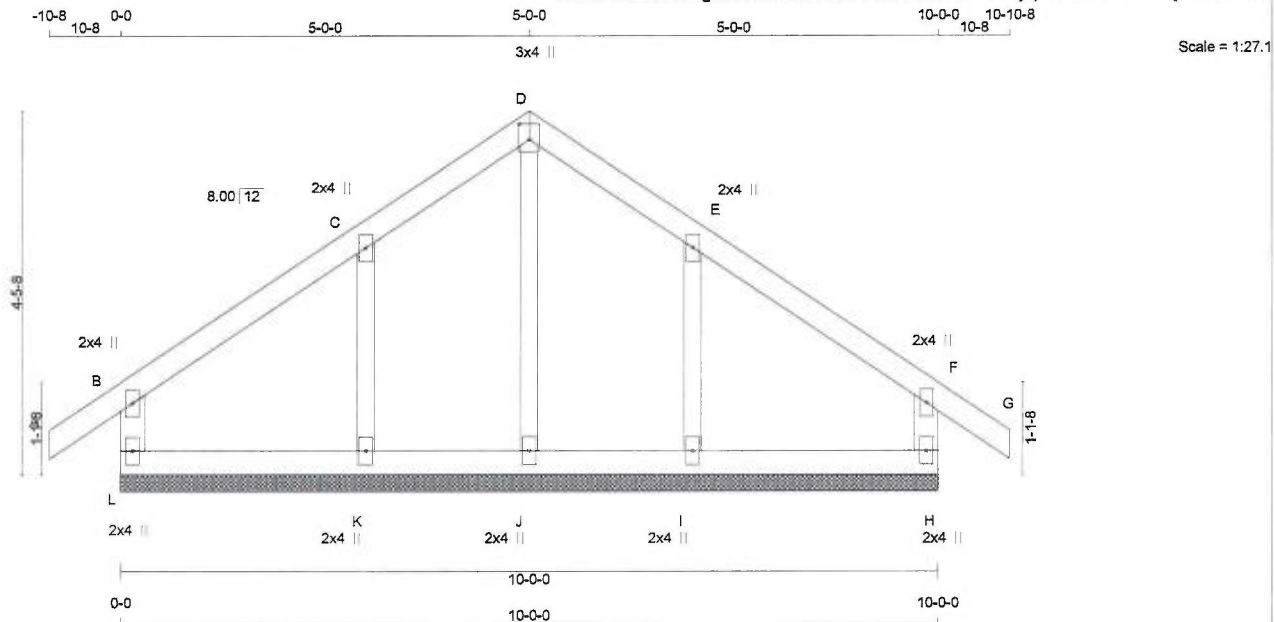
READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP= 0.77 (D) (INPUT = 0.90)
JSI METAL= 0.21 (G) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
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THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 37 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
L - B	2x4	DRY	No.2	SPF	
A - D	2x4	DRY	No.2	SPF	
D - G	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
L - H	2x4	DRY	No.2	SPF	

ALL WEBS	2x3	DRY	No.2	SPF	
ALL GABLE WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

GABLE STUDS SPACED AT 2'-0" OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMW+w	MT20	2.0	4.0		
D	TTW+p	MT20	3.0	4.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMV+p	MT20	2.0	4.0		
H	BMV1+p	MT20	2.0	4.0		
I, J, K						
I	BMW1+w	MT20	2.0	4.0		
L	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED UNBRACED LENGTH	MAX. FACTORED LC1 CSI (LC)
FR-TO		FROM TO		FR-TO			
L-B	-209 / 0	0.0	0.0 0.02 (4)	J-D	-117 / 0	7.81	0.03 (1)
A-B	0 / 21	-77.4	-77.4 0.05 (1)	K-C	-193 / 0	10.00	0.04 (1)
B-C	-32 / 0	-77.4	-77.4 0.08 (1)	I-E	-193 / 0	6.25	0.04 (1)
C-D	-27 / 0	-77.4	-77.4 0.06 (1)			6.25	
D-E	-27 / 0	-77.4	-77.4 0.06 (1)			6.25	
E-F	-32 / 0	-77.4	-77.4 0.08 (1)			6.25	
F-G	0 / 21	-77.4	-77.4 0.05 (1)			10.00	
H-F	-209 / 0	0.0	0.0 0.02 (4)			7.81	
L-K	0 / 23	-17.5	-17.5 0.03 (4)			10.00	
K-J	0 / 18	-17.5	-17.5 0.03 (4)			10.00	
J-I	0 / 18	-17.5	-17.5 0.03 (4)			10.00	
I-H	0 / 23	-17.5	-17.5 0.03 (4)			10.00	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.08/1.00 (B-C:1), BC=0.03/1.00 (K-L:4), WS=0.04/1.00 (C-K:1), SSI=0.08/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

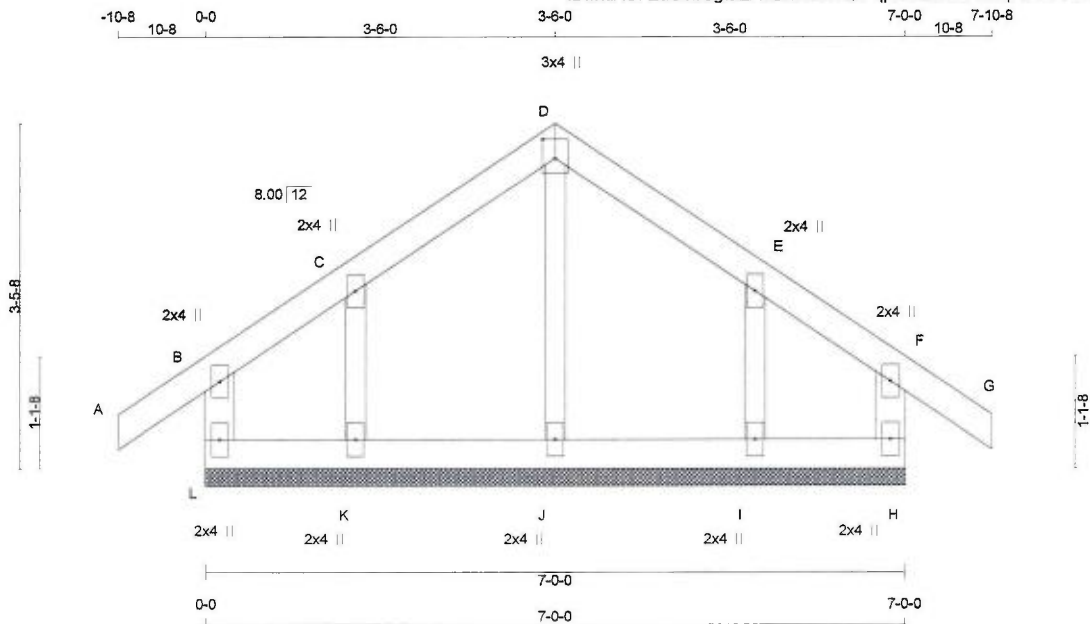
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:22.2

TOTAL WEIGHT = 27 lb [M]

LUMBER			
N, L, G, A, RULES	CHORDS	SIZE	LUMBER
L - B	2x4	DRY	No.2
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
H - F	2x4	DRY	No.2
L - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			
GABLE STUDS SPACED AT 2'-0" OC.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	UNBRACED LENGTH (FT)
FR-TO		FROM TO		FR-TO			
L-B	-135 / 0	0.0	0.0 0.01 (1)	J-D	-150 / 0	0.03 (1)	7.81
A-B	0 / 21	-77.4	-77.4 0.05 (1)	K-C	-136 / 0	0.02 (1)	10.00
B-C	-20 / 0	-77.4	-77.4 0.03 (1)	I-E	-136 / 0	0.02 (1)	6.25
C-D	-15 / 0	-77.4	-77.4 0.04 (1)				6.25
D-E	-15 / 0	-77.4	-77.4 0.04 (1)				6.25
E-F	-20 / 0	-77.4	-77.4 0.03 (1)				6.25
F-G	0 / 21	-77.4	-77.4 0.05 (1)				10.00
H-F	-135 / 0	0.0	0.0 0.01 (1)				7.81
L-K	0 / 21	-17.5	-17.5 0.01 (4)				10.00
K-J	0 / 14	-17.5	-17.5 0.02 (4)				10.00
J-I	0 / 14	-17.5	-17.5 0.02 (4)				10.00
I-H	0 / 21	-17.5	-17.5 0.01 (4)				10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF

DL = 3.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.0 PSF

TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.05/1.00 (F-G:1), BC=0.02/1.00 (I-J:4), WB=0.03/1.00 (D-J:1), SSI=0.05/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

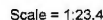
JSI GRIP= 0.10 (B) (INPUT = 0.90)

JSI METAL= 0.06 (D) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

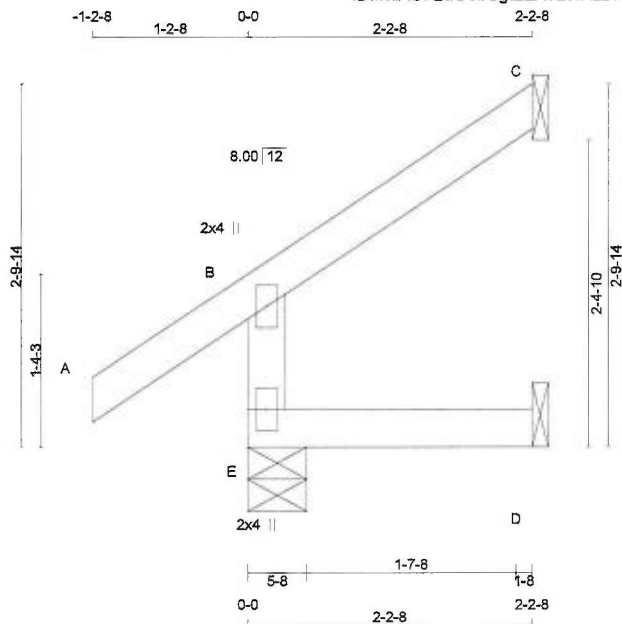




[M][F]

JSI GRIP= 0.22 (B) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)





Scale = 1:17.2

TOTAL WEIGHT = 7 X 8 = 59 lb [M]

LUMBER					DESCR.
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	SPF	
E - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	2.0	4.0		
E BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX
E	227	0	227	0	5-8	5-8
C	64	0	64	0	1-8	1-8
D	17	0	19	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
E	157	124 / 0	0 / 0	0 / 0	0 / 0	33 / 0	0 / 0
C	44	39 / 0	0 / 0	0 / 0	0 / 0	5 / 0	0 / 0
D	14	0 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS		FACTORED		WEBS		MAX. FACTORED	
	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX. (LC)	MAX. UNBRACED LENGTH FR-TO	MEMB.	FORCE (LBS)	MAX. (LC)
FR-TO		FROM	TO					
E-B	-206 / 0	0.0	0.0	0.01 (4)	7.81			
A-B	0 / 28	-77.4	-77.4	0.09 (1)	10.00			
B-C	-12 / 0	-77.4	-77.4	0.06 (1)	6.25			
E-D	0 / 0	-17.5	-17.5	0.02 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
	DL = 3.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.00")

CSI: $TC=0.09/1.00$ (A-B:1), $BC=0.02/1.00$ (D-E:4), $WB=0.00/1.00$ (n/a:0), $SSI=0.07/1.00$ (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
	MAX	MIN	MAX MIN
MT20	618	354	1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

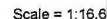
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = $2 \times 8 = 16 \text{ lb}$

BEARINGS							
JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	217	0	217	0	0	5-8	5-8
C	60	0	60	0	0	1-8	1-8
D	16	0	18	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS							
JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	150	119 / 0	0 / 0	0 / 0	0 / 0	31 / 0	0 / 0
C	40	36 / 0	0 / 0	0 / 0	0 / 0	5 / 0	0 / 0
D	13	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

LOADING
TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRAC.	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
E-B	-198 / 0	0.0	0.0	0.01 (4)			7.81
A-B	0 / 28	-77.4	-77.4	0.09 (1)			10.00
B-C	-11 / 0	-77.4	-77.4	0.05 (1)			6.25
E-D	0 / 0	-17.5	-17.5	0.02 (4)			10.00

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD		= 33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.09/1.00 (A-B:1), BC=0.02/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.06/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

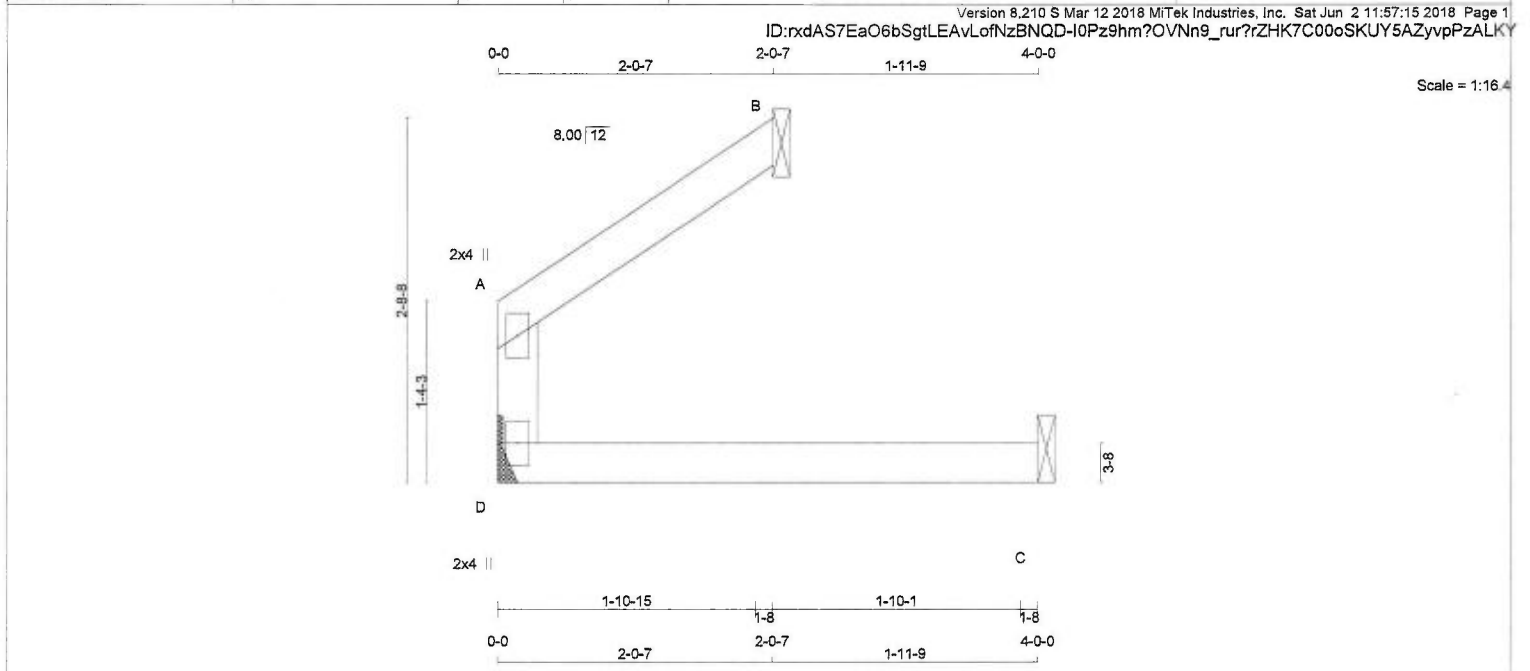
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)



**READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.**





LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
D - A	2x4	DRY	No.2
A - B	2x4	DRY	No.2
D - C	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0	
D	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
D	113	0	113	0	MECHANICAL	
B	81	0	81	0	1-8	1-8
C	34	0	35	0	1-8	1-8

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT D TO RESIST THE MAX FACTORED REACTIONS.

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) B, C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS					SOIL
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	
D	80	49 / 0	0 / 0	0 / 0	0 / 0	31 / 0	0 / 0
B	56	44 / 0	0 / 0	0 / 0	0 / 0	11 / 0	0 / 0
C	27	2 / 0	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

FR-TO	CHORDS		FACTORED		MAX. UNBRAC LENGTH	WEBS		FACTORED	
	MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX		MEMB.	FORCE (LBS)	MAX	CSI (LC)
D-A	-77 / 0	0.0	0.0	0.03 (4)	7.81				
A-B	0 / 4	-77.4	-77.4	0.06 (1)	10.00				
D-C	0 / 0	-17.5	-17.5	0.07 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
	DL = 3.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 33.3	PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.01")

CSI: TC=0.06/1.00 (A-B:1), BC=0.07/1.00 (C-D:4),
WB=0.00/1.00 (n/a:0), SSI=0.05/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

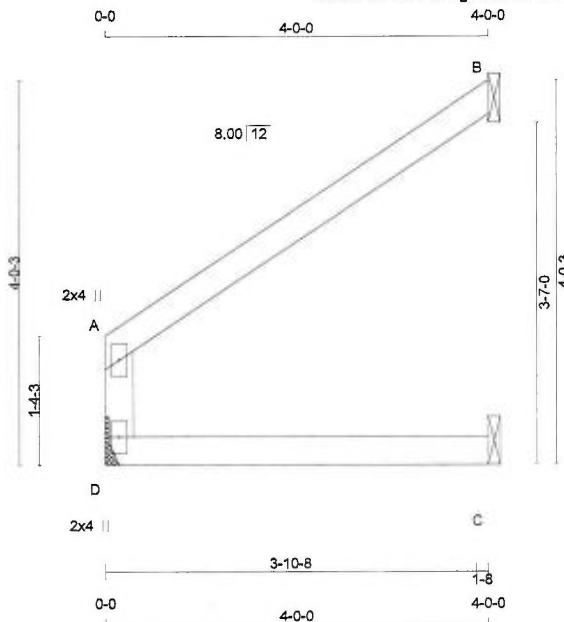
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.05 (A) (INPUT = 0.90)
JSI METAL= 0.02 (A) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:23.1

TOTAL WEIGHT = 3 X 11 = 34 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
D - A	2x4	DRY	No.2
A - B	2x4	DRY	No.2
D - C	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
A	TMV+p	MT20	2.0	4.0
D	BMV1+p	MT20	2.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	DOWN	UPLIFT	BRG	IN-SX	BRG
D	190	0	190	0	0	0	1-8	1-8	1-8
B	142	0	142	0	0	0	1-8	1-8	1-8
C	48	0	48	0	0	0	1-8	1-8	1-8

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT D TO RESIST THE MAX FACTORED REACTIONS.

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) B, C

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
D	133	93 / 0	0 / 0	0 / 0	0 / 0	40 / 0	0 / 0
B	97	84 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0
C	36	9 / 0	0 / 0	0 / 0	0 / 0	27 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	LC1 MAX (LC)	UNBRACED LENGTH FR-TO
D-A	-167 / 0	0.0	0.0 0.06 (1)	A-B	-7 / 0	-77.4 -77.4 0.17 (1)	7.81 10.00
D-C	0 / 0	-17.5	-17.5 0.09 (1)				10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
	DL = 3.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.01")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.02")

CSI: TC=0.17/1.00 (A-B:1), BC=0.09/1.00 (C-D:1), WB=0.00/1.00 (n/a:0), SSI=0.11/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

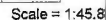
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.11 (A) (INPUT = 0.90)
JSI METAL= 0.04 (A) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

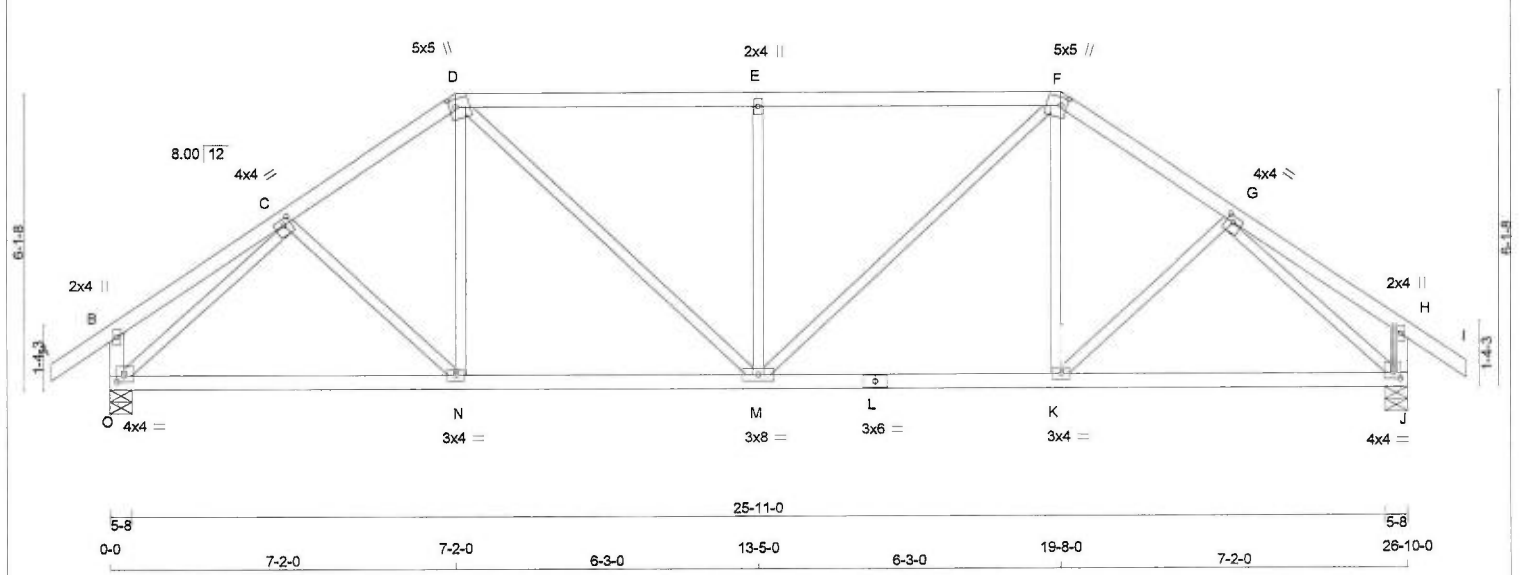




TOTAL WEIGHT = 110 lb

JSI GRIP= 0.86 (M) (INPUT = 0.90)
JSI METAL= 0.48 (B) (INPUT = 1.00)





TOTAL WEIGHT = 112 lb [M] [F]

LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.	BEARINGS	FACTORED	MAXIMUM FACTORED
A - D	2x4	DRY	No.2	SPF	FACTORED	GROSS REACTION	INPUT
D - F	2x4	DRY	No.2	SPF	VERT	DOWN	BRG
F - I	2x4	DRY	No.2	SPF	HORZ	UPLIFT	IN-SX
O - B	2x4	DRY	No.2	SPF			BRG
J - H	2x4	DRY	No.2	SPF			IN-SX
O - L	2x4	DRY	No.2	SPF			
L - J	2x4	DRY	No.2	SPF			

UNFACTORED REACTIONS				DESIGN CRITERIA			
1ST LCASE	MAX.	MIN.	COMPONENT REACTIONS	SPECIFIED LOADS:			
JT	COMBINED	SNOW	LIVE	TOP CH.	LL	=	23.3 PSF
O	961	685 / 0	0 / 0	DL	=	3.0 PSF	
J	961	685 / 0	0 / 0	BOT CH.	LL	=	0.0 PSF
				DL	=	7.0 PSF	
				TOTAL LOAD	=	33.3 PSF	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.83 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-77.4 -77.4	0.09 (1)	C-N	-4 / 48	0.02 (4)	
B-C	0 / 16	-77.4 -77.4	0.14 (1)	N-D	0 / 141	0.04 (4)	
C-D	-1415 / 0	-77.4 -77.4	0.13 (1)	D-M	0 / 501	0.11 (1)	
D-E	-1531 / 0	-77.4 -77.4	0.43 (1)	M-E	-593 / 0	0.35 (1)	
E-F	-1531 / 0	-77.4 -77.4	0.43 (1)	M-F	0 / 501	0.11 (1)	
F-G	-1415 / 0	-77.4 -77.4	0.13 (1)	K-F	0 / 141	0.04 (4)	
G-H	0 / 16	-77.4 -77.4	0.14 (1)	K-G	-4 / 48	0.02 (4)	
H-I	0 / 28	-77.4 -77.4	0.09 (1)	O-C	-1610 / 0	0.63 (1)	
O-B	-209 / 0	0.0 0.0	0.02 (1)	G-J	-1610 / 0	0.63 (1)	
J-H	-209 / 0	0.0 0.0	0.02 (1)				
O-N	0 / 1165	-17.5 -17.5	0.30 (1)				
N-M	0 / 1164	-17.5 -17.5	0.30 (1)				
M-L	0 / 1164	-17.5 -17.5	0.30 (1)				
L-K	0 / 1164	-17.5 -17.5	0.30 (1)				
K-J	0 / 1165	-17.5 -17.5	0.30 (1)				

ALLOWABLE DEFL.(LL)= L/360 (0.89")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.89")
CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CSI: TC=0.43/1.00 (D-E:1), BC=0.30/1.00 (N-O:1),
WB=0.63/1.00 (C-O:1), SS=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

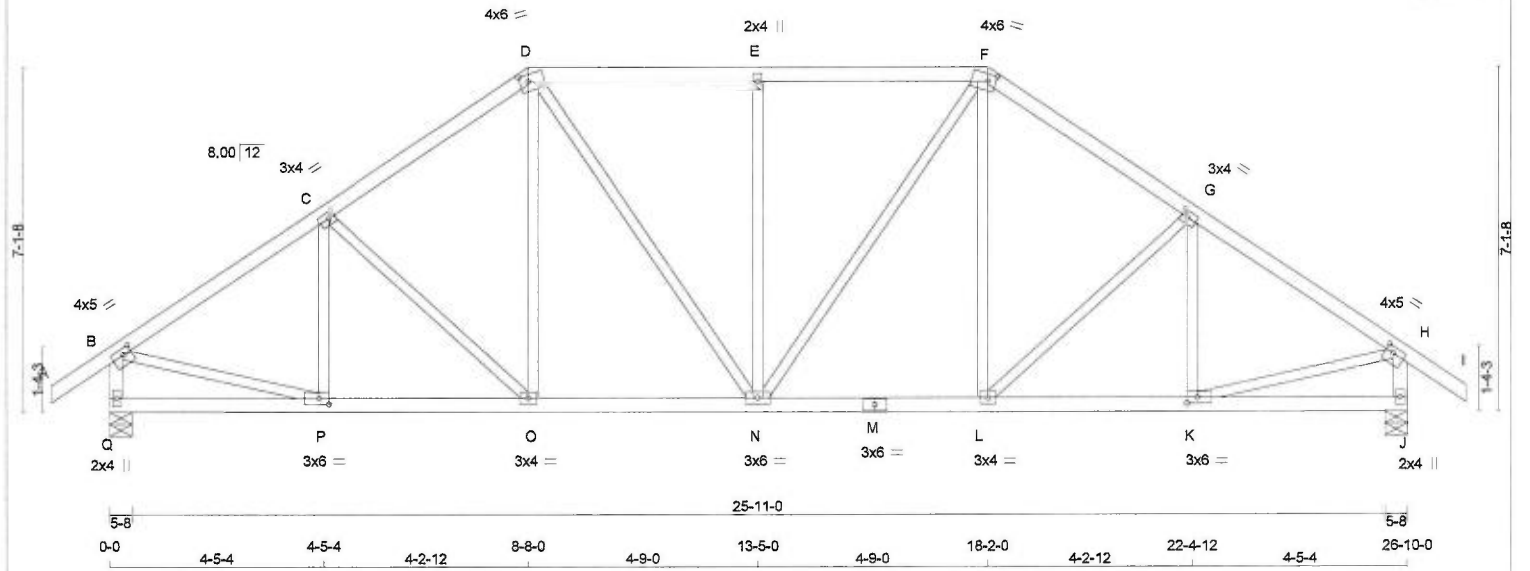
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (O) (INPUT = 0.90)
JSI METAL= 0.59 (G) (INPUT = 1.00)



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
Q - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
Q - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
Q	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	5.0	1.50	2.00
C	TMVW-t	MT20	3.0	4.0	1.50	1.50
D	TTWW-m	MT20	4.0	6.0	1.75	2.00
E	TMVW-w	MT20	2.0	4.0		
F	TTWW-m	MT20	4.0	6.0	1.75	2.00
G	TMVW-t	MT20	3.0	4.0	1.50	1.50
H	TMVW-t	MT20	4.0	5.0	1.50	2.00
J	BMV1+p	MT20	2.0	4.0		
K	BMVW-t	MT20	3.0	6.0	1.50	2.50
L	BMVW-t	MT20	3.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMVWWW-t	MT20	3.0	6.0		
O	BMVW-t	MT20	3.0	4.0		
P	BMVW-t	MT20	3.0	6.0	1.50	2.50
Q	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
Q	1372 0	1372 0	5-8	5-8
J	1372 0	1372 0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. SNOW	MIN. LIVE	PERM. LIVE	WIND	DEAD	SOIL
Q	961	685 / 0	0 / 0	0 / 0	0 / 0	276 / 0	0 / 0
J	961	685 / 0	0 / 0	0 / 0	0 / 0	276 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.22 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-77.4 -77.4	0.09 (1)	10.00	P-C	-206 / 11	0.06 (1)
B-C	-1457 / 0	-77.4 -77.4	0.21 (1)	5.22	C-O	-181 / 0	0.11 (1)
C-D	-1342 / 0	-77.4 -77.4	0.20 (1)	5.39	O-D	0 / 204	0.05 (1)
D-E	-1263 / 0	-77.4 -77.4	0.24 (1)	5.44	D-N	0 / 321	0.07 (1)
E-F	-1263 / 0	-77.4 -77.4	0.24 (1)	5.44	N-E	-446 / 0	0.39 (1)
F-G	-1342 / 0	-77.4 -77.4	0.20 (1)	5.39	N-F	0 / 321	0.07 (1)
G-H	-1457 / 0	-77.4 -77.4	0.21 (1)	5.22	L-F	0 / 204	0.05 (1)
H-I	0 / 28	-77.4 -77.4	0.09 (1)	10.00	L-G	-181 / 0	0.11 (1)
Q-B	-1338 / 0	0.0 0.0	0.14 (1)	7.00	K-G	-206 / 11	0.06 (1)
J-H	-1338 / 0	0.0 0.0	0.14 (1)	7.00	B-P	0 / 1262	0.28 (1)
					K-H	0 / 1262	0.28 (1)
Q-P	0 / 0	-17.5 -17.5	0.07 (4)	10.00			
P-O	0 / 1230	-17.5 -17.5	0.24 (1)	10.00			
O-N	0 / 1100	-17.5 -17.5	0.22 (1)	10.00			
N-M	0 / 1100	-17.5 -17.5	0.22 (1)	10.00			
M-L	0 / 1100	-17.5 -17.5	0.22 (1)	10.00			
L-K	0 / 1230	-17.5 -17.5	0.24 (1)	10.00			
K-J	0 / 0	-17.5 -17.5	0.07 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.89")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.89")
CALCULATED VERT. DEFL.(TL) = L/999 (0.09")

CSI: TC=0.24/1.00 (D-E:1), BC=0.24/1.00 (O-P:1), WB=0.39/1.00 (E-N:1), SSI=0.18/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (N) (INPUT = 0.90)
JSI METAL= 0.48 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

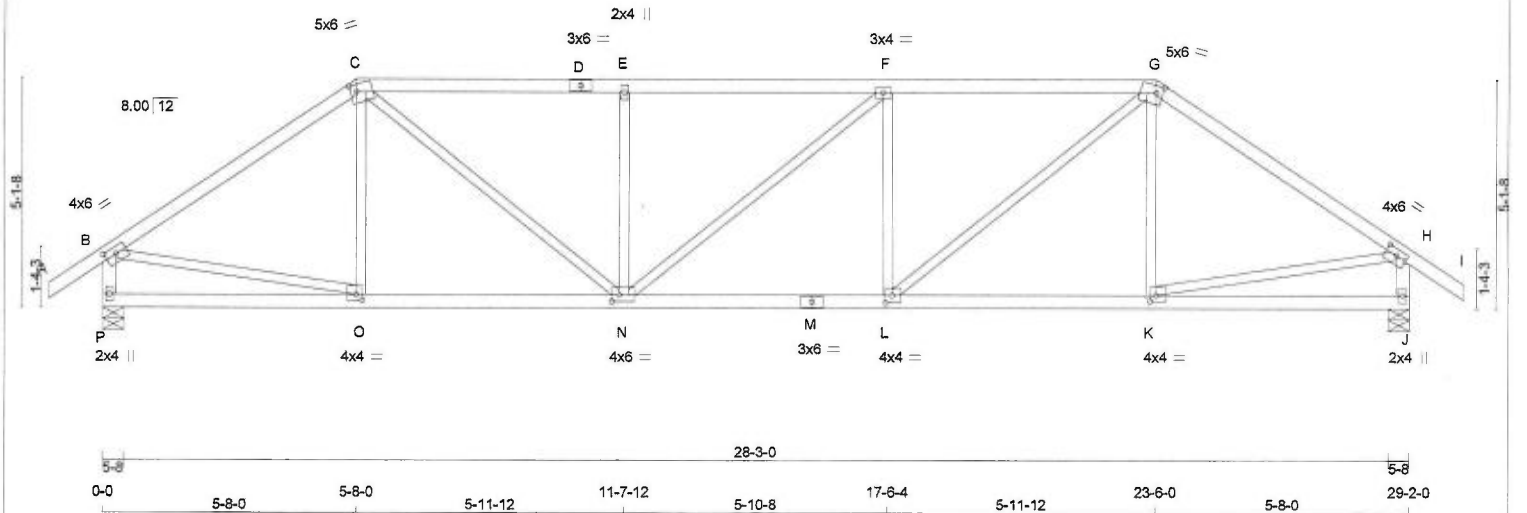




TOTAL WEIGHT = 129 lb
[M][F]

JSI GRIP= 0.88 (B) (INPUT = 0.90)
JSI METAL= 0.49 (B) (INPUT = 1.00)





LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
P - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
P - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+	MT20	4.0	6.0	1.75 3.00
C	TTWW+	MT20	5.0	6.0	2.00 1.75
D	TS+	MT20	3.0	6.0	
E	TMW+	MT20	2.0	4.0	
F	TMW+	MT20	3.0	4.0	
G	TTWW+	MT20	5.0	6.0	2.00 1.75
H	TMVW+	MT20	4.0	6.0	1.75 3.00
J	BMV1+	MT20	2.0	4.0	
K	BMW+	MT20	4.0	4.0	1.50 1.50
L	BMW+	MT20	4.0	4.0	2.00 1.75
M	BS+	MT20	3.0	6.0	
N	BMWW+	MT20	4.0	6.0	1.75 2.00
O	BMW+	MT20	4.0	4.0	1.50 1.50
P	BMV1+	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
P	1483	0	1483	0	5-8
J	1483	0	1483	0	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN	COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1039	739 / 0	0 / 0	0 / 0	0 / 0	299 / 0	0 / 0
J	1039	739 / 0	0 / 0	0 / 0	0 / 0	299 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.22 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	MEMB.	FORCE (LBS)
FR-TO		FROM	TO	FR-TO	
A-B	0 / 28	-77.4	-77.4 0.09 (1)	O-C	-121 / 57
B-C	-1596 / 0	-77.4	-77.4 0.54 (1)	C-N	0 / 949
C-D	-2062 / 0	-77.4	-77.4 0.48 (1)	N-E	-495 / 0
D-E	-2062 / 0	-77.4	-77.4 0.48 (1)	E-F	-2 / 0
E-F	-2062 / 0	-77.4	-77.4 0.49 (1)	F-G	-497 / 0
F-G	-2064 / 0	-77.4	-77.4 0.49 (1)	G-H	0 / 951
G-H	-1596 / 0	-77.4	-77.4 0.54 (1)	H-I	-121 / 57
H-I	0 / 28	-77.4	-77.4 0.09 (1)	I-J	0 / 1347
J-K	-1443 / 0	0.0	0.0 0.15 (1)	K-L	0 / 1347
L-M	-1443 / 0	0.0	0.0 0.15 (1)	M-N	0 / 1347
N-O	0 / 0	-17.5	-17.5 0.14 (4)	O-P	0 / 0
O-P	0 / 1324	-17.5	-17.5 0.29 (1)	P-Q	0 / 0
P-Q	0 / 2064	-17.5	-17.5 0.38 (1)	Q-R	0 / 0
Q-R	0 / 2064	-17.5	-17.5 0.38 (1)	R-S	0 / 0
R-S	0 / 1323	-17.5	-17.5 0.29 (1)	S-T	0 / 0
S-T	0 / 0	-17.5	-17.5 0.14 (4)	T-U	0 / 0

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/999 (0.10")
ALLOWABLE DEFL.(TL) = L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/999 (0.19")

CSI: TC=0.54/1.00 (B-C:1), BC=0.38/1.00 (L-N:1), WB=0.30/1.00 (B-O:1), SS=0.21/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

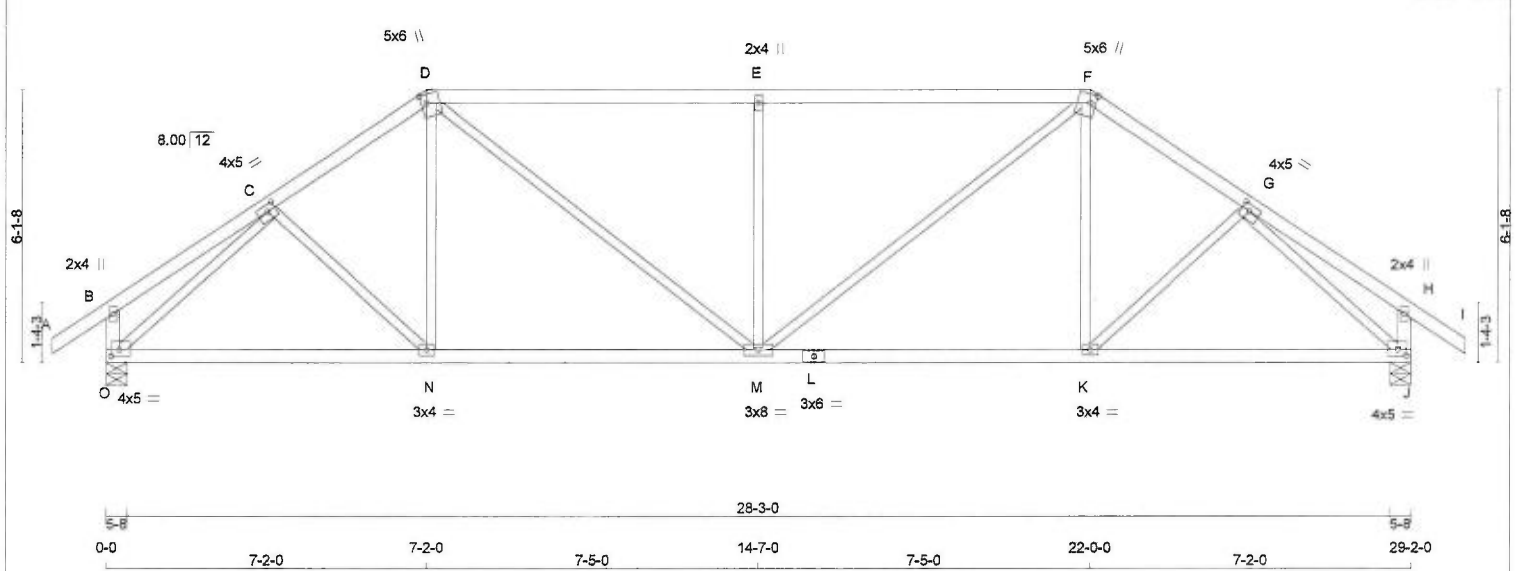
JSI GRIP= 0.89 (G) (INPUT = 0.90)
JSI METAL= 0.56 (M) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



Version 8.210 S Mar 12 2018 MiTek Industries, Inc. Sat Jun 2 11:57:19 2018 Page 1
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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - I	2x4	DRY	No.2	SPF	
O - B	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
O - L	2x4	DRY	No.2	SPF	
L - J	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
DRY: SEASONED LUMBER.					

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
TMV+p	MT20	2.0	4.0				
TMVW+H	MT20	4.0	5.0	1.75	2.25		
TTWV+m	MT20	5.0	6.0	2.25	1.50		
TMV+w	MT20	2.0	4.0				
TTWV+m	MT20	5.0	6.0	2.25	1.50		
TMVW+H	MT20	4.0	5.0	1.75	2.25		
TMV+p	MT20	2.0	4.0				
BMVW1-t	MT20	4.0	5.0	1.75	2.25		
BMVW+H	MT20	3.0	4.0				
BS-t	MT20	3.0	6.0				
BMVWV+H	MT20	3.0	8.0				
BMVW+H	MT20	3.0	4.0				
BMVW1-t	MT20	4.0	5.0	1.75	2.25		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG	
JT VERT	HORZ	DOWN	HORZ	UPLIFT
O 1483	0	1483	0	0
J 1483	0	1483	0	0

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS						
JT COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
O 1039	739 / 0	0 / 0	0 / 0	0 / 0	299 / 0	0 / 0	
J 1039	739 / 0	0 / 0	0 / 0	0 / 0	299 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.61 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	MAX	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MEMB.	FORCE (LBS)
FR-TO		FROM TO		FR-TO	
A-B	0 / 28	-77.4 -77.4	0.09 (1)	C-N	0 / 57
B-C	0 / 16	-77.4 -77.4	0.14 (1)	N-D	0 / 129
C-D	-1581 / 0	-77.4 -77.4	0.18 (1)	D-M	0 / 867
D-E	-1827 / 0	-77.4 -77.4	0.95 (1)	M-E	-706 / 0
E-F	-1827 / 0	-77.4 -77.4	0.95 (1)	M-F	0 / 867
F-G	-1581 / 0	-77.4 -77.4	0.18 (1)	K-F	0 / 129
G-H	0 / 16	-77.4 -77.4	0.14 (1)	K-G	0 / 57
H-I	0 / 28	-77.4 -77.4	0.09 (1)	O-C	-1772 / 0
O-B	-209 / 0	0.0 0.0	0.02 (1)	G-J	-1772 / 0
J-H	-209 / 0	0.0 0.0	0.02 (1)		
O-N	0 / 1282	-17.5 -17.5	0.33 (1)		
N-M	0 / 1302	-17.5 -17.5	0.34 (1)		
M-L	0 / 1302	-17.5 -17.5	0.34 (1)		
L-K	0 / 1302	-17.5 -17.5	0.34 (1)		
K-J	0 / 1282	-17.5 -17.5	0.33 (1)		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 23.3 PSF
DL	= 3.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 33.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/999 (0.15")

CSI: TC=0.95/1.00 (D-E:1), BC=0.34/1.00 (K-M:1), WB=0.69/1.00 (G-J:1), SSI=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

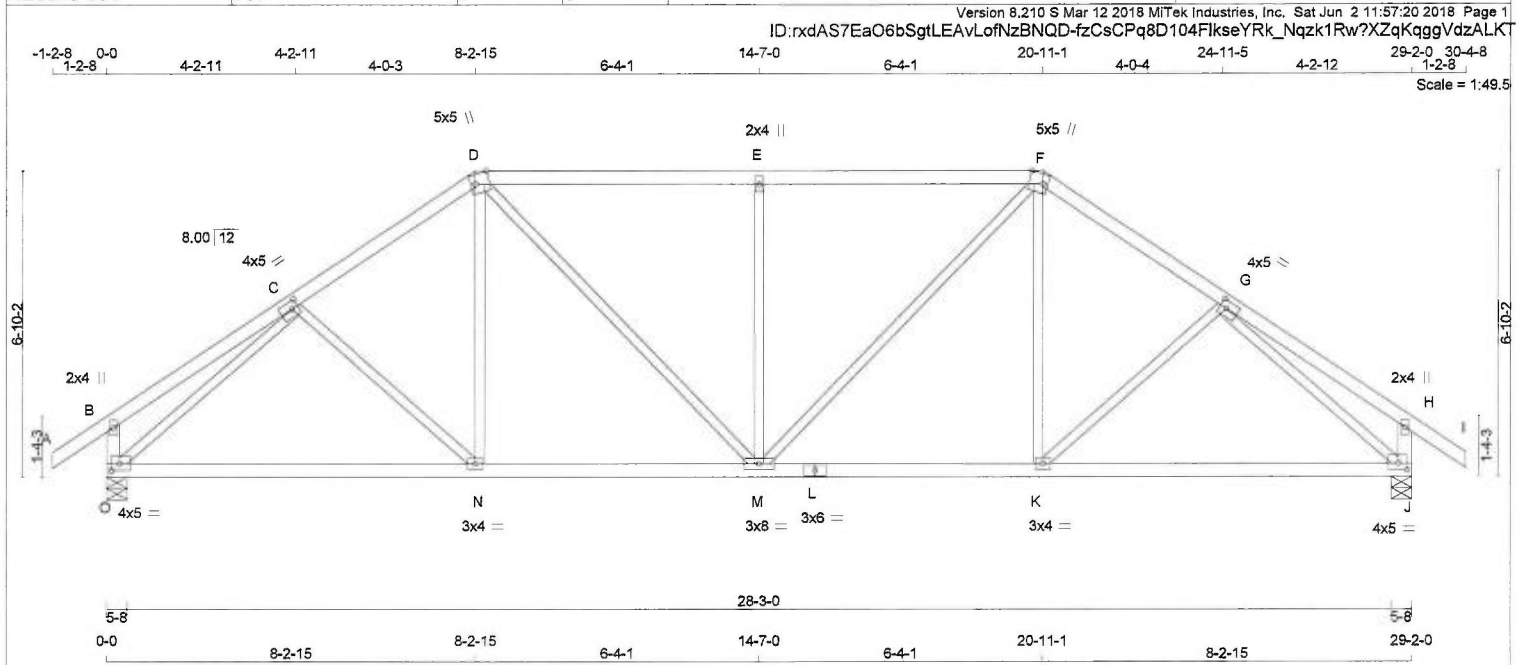
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (C) (INPUT = 0.90)
JSI METAL= 0.52 (C) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 123 lb [M][F]

LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.	BEARINGS			
A - D	2x4	DRY	No.2	FACTORED	MAXIMUM FACTORED	INPUT	REQD
D - F	2x4	DRY	No.2	GROSS REACTION	GROSS REACTION	BRG	BRG
F - I	2x4	DRY	No.2	JT VERT	JT VERT	UPLIFT	IN-SX
O - B	2x4	DRY	No.2	O 1483	0	0	5-8
J - H	2x4	DRY	No.2	J 1483	0	0	5-8
O - L	2x4	DRY	No.2				
L - J	2x4	DRY	No.2				
ALL WEBS	2x3	DRY	No.2				
EXCEPT							
DRY: SEASONED LUMBER.							

UNFACTORED REACTIONS				DESIGN CRITERIA			
1ST LCASE	MAX./MIN.	COMPONENT REACTIONS		SPECIFIED LOADS:			
JT COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
O 1039	739 / 0	0 / 0	0 / 0	0 / 0	299 / 0	0 / 0	
J 1039	739 / 0	0 / 0	0 / 0	0 / 0	299 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.72 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-77.4 -77.4	0.09 (1)	C-N	-57 / 41	0.03 (1)	
B-C	0 / 19	-77.4 -77.4	0.20 (1)	N-D	0 / 193	0.05 (4)	
C-D	-1544 / 0	-77.4 -77.4	0.18 (1)	D-M	0 / 475	0.11 (1)	
D-E	-1600 / 0	-77.4 -77.4	0.44 (1)	M-E	-602 / 0	0.48 (1)	
E-F	-1600 / 0	-77.4 -77.4	0.44 (1)	F-F	0 / 475	0.11 (1)	
F-G	-1544 / 0	-77.4 -77.4	0.18 (1)	K-F	0 / 193	0.05 (4)	
G-H	0 / 19	-77.4 -77.4	0.20 (1)	K-G	-57 / 41	0.03 (1)	
H-I	0 / 28	-77.4 -77.4	0.09 (1)	O-C	-1775 / 0	0.91 (1)	
O-B	-224 / 0	0.0 0.0	0.02 (1)	G-J	-1775 / 0	0.91 (1)	
J-H	-224 / 0	0.0 0.0	0.02 (1)				
O-N	0 / 1309	-17.5 -17.5	0.36 (1)				
N-M	0 / 1269	-17.5 -17.5	0.37 (4)				
M-L	0 / 1269	-17.5 -17.5	0.37 (4)				
L-K	0 / 1269	-17.5 -17.5	0.37 (4)				
K-J	0 / 1309	-17.5 -17.5	0.36 (1)				

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

ALLOWABLE DEFLECTIONS:
ALLOWABLE DEF. (LL) = L/360 (0.97")
CALCULATED VERT. DEF. (LL) = L/999 (0.07")
ALLOWABLE DEF. (TL) = L/360 (0.97")
CALCULATED VERT. DEF. (TL) = L/999 (0.17")

CSI: TC=0.44/1.00 (D-E:1), BC=0.37/1.00 (K-M:4), WB=0.91/1.00 (G-J:1), SS=0.24/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

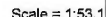
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (F) (INPUT = 0.90)
JSI METAL= 0.52 (C) (INPUT = 1.00)





TOTAL WEIGHT = 121 lb

DRY: SEASONED LUMBER.

BEARINGS

UNFACTORED REACTIONS

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M. H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.97 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MAX. FACTORED		FACTORED			MAX. FACTORED		FACTORED		
MEMB.	FORCE	VERT. LOAD	LC1	MAX	MAX.	MEMB.	FORCE	MAX	FACTORED
	(LBS)	(PLF)	CSI	(LC)	UNBRAC		(LBS)	CSI	(LC)
FR-TO		FROM	TO		LENGTH	FR-TO			
A-B	0 / 28	-77.4	-77.4	0.09 (1)	10.00	L-C	-100 / 71		0.04 (1)
B-C	-1479 / 0	-77.4	-77.4	0.39 (1)	4.97	C-K	-400 / 0		0.48 (1)
C-D	-1175 / 0	-77.4	-77.4	0.36 (1)	5.46	K-D	0 / 333		0.07 (1)
D-E	-956 / 0	-77.4	-77.4	0.13 (1)	6.21	D-J	0 / 2		0.00 (1)
E-F	-1177 / 0	-77.4	-77.4	0.36 (1)	5.45	J-E	0 / 336		0.08 (1)
F-G	-1478 / 0	-77.4	-77.4	0.39 (1)	4.97	E-F	-398 / 0		0.47 (1)
M-B	-1330 / 0	0.0	0.0	0.14 (1)	7.02	I-F	-102 / 70		0.04 (1)
H-G	-1230 / 0	0.0	0.0	0.13 (1)	7.23	B-L	0 / 1274		0.29 (1)
						I-G	0 / 1273		0.29 (1)
M-L	0 / 0	-17.5	-17.5	0.15 (4)	10.00				
L-K	0 / 1255	-17.5	-17.5	0.28 (1)	10.00				
K-J	0 / 955	-17.5	-17.5	0.20 (1)	10.00				
J-I	0 / 1255	-17.5	-17.5	0.28 (1)	10.00				
I-H	0 / 0	-17.5	-17.5	0.15 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.89")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.89")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.39/1.00 (B-C:1), BC=0.28/1.00 (K-L:1),
WB=0.48/1.00 (C-K:1), SS=0.18/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

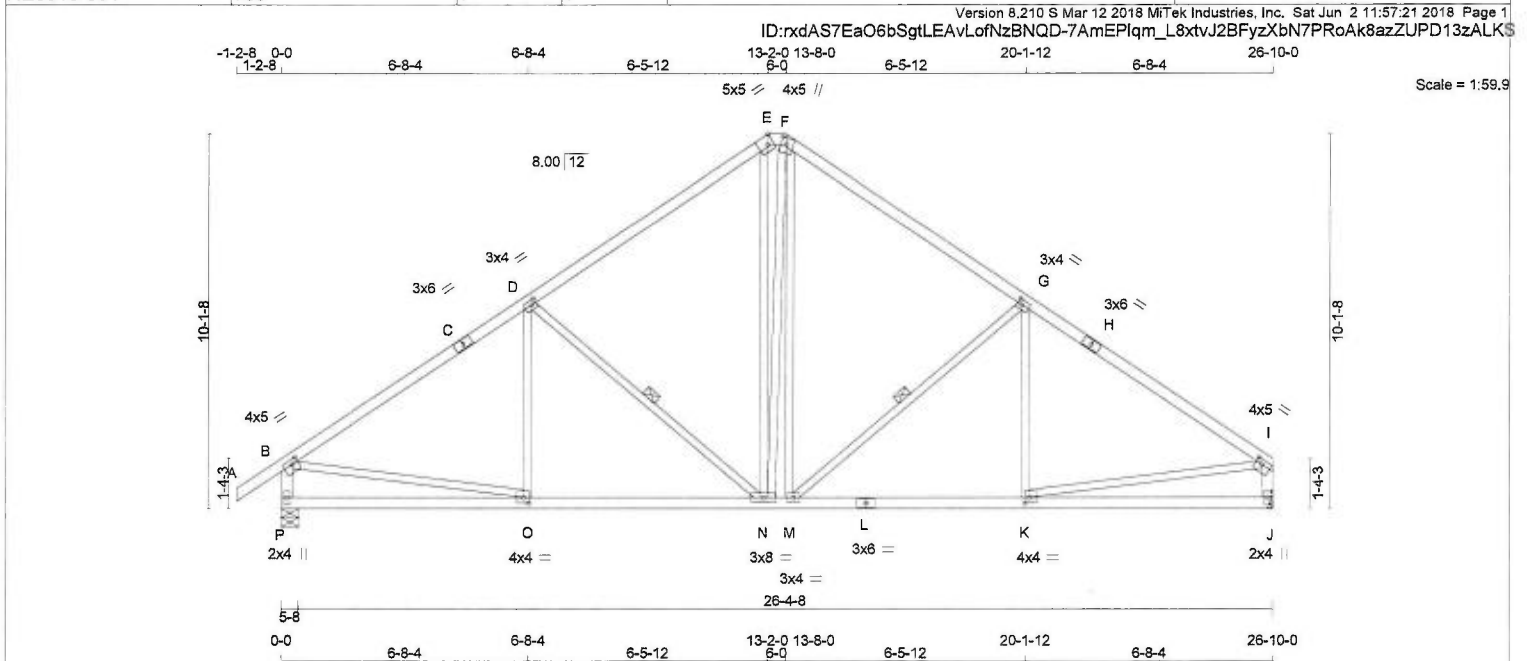
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.50 (B) (INPUT = 1.00)



**READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.**



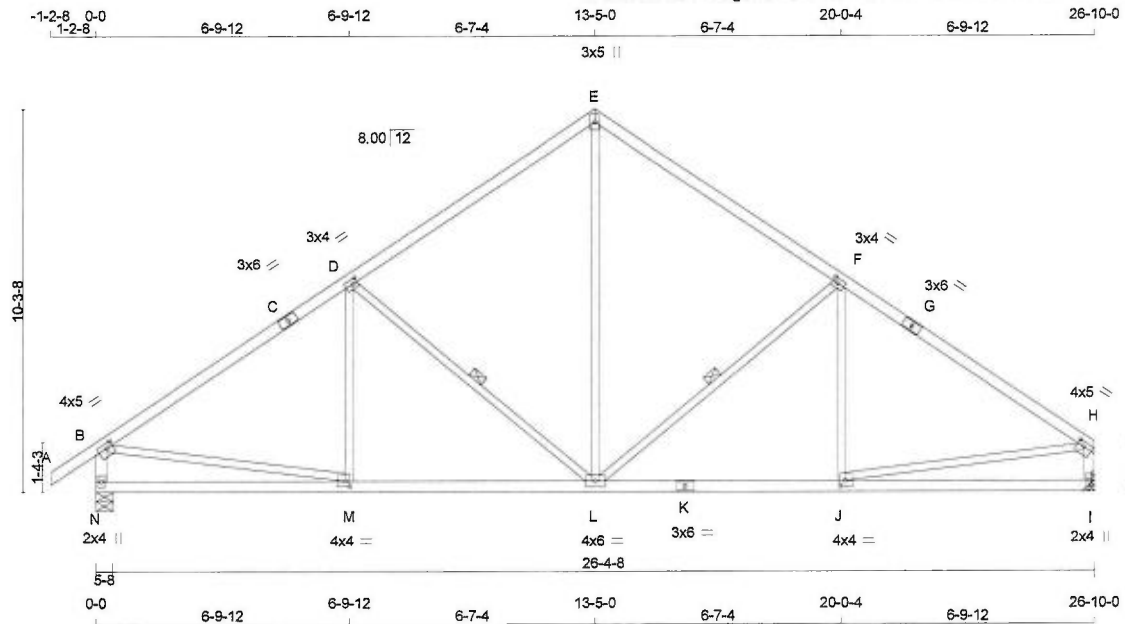


Version 8.210 S Mar 12 2018 MiTek Industries, Inc. Sat Jun 2 11:57:21 2018 Page 1
ID:xdAS7EaO6bSgtLEAvLofNzBNQD-7AmEPlqm_L8xtvJ2BFyzXbN7ProAk8azZUPD13zALKS
Scale = 1:59.9
TOTAL WEIGHT = 127 lb [M][F]

N. L. G. A. RULES				BUILDING DESIGNER										DESIGN CRITERIA					
CHORDS				LUMBER		DESCR.		BEARINGS								SPECIFIED LOADS:			
A - C	2x4	DRY	No.2	SPF	FACTORED				MAXIMUM FACTORED				INPUT	REQD	TOP CH. LL = 23.3 PSF				
C - E	2x4	DRY	No.2	SPF	GROSS REACTION				GROSS REACTION				BRG	BRG	DL = 3.0 PSF				
E - F	2x4	DRY	No.2	SPF	JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	BOT CH. LL = 0.0 PSF						
F - H	2x4	DRY	No.2	SPF	P	1372	0	1372	0	0	5-8	5-8	DL = 7.0 PSF						
H - J	2x4	DRY	No.2	SPF	J	1273	0	1273	0	0	MECHANICAL		TOTAL LOAD = 33.3 PSF						
P - B	2x4	DRY	No.2	SPF	A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT J TO RESIST THE MAX FACTORED REACTIONS.										SPACING = 24.0 IN. C/C				
J - I	2x4	DRY	No.2	SPF											LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.09/12 MINIMUM				
P - L	2x4	DRY	No.2	SPF											THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9. NBCC 2010				
L - J	2x4	DRY	No.2	SPF															
ALL WEBS	2x3	DRY	No.2	SPF															
EXCEPT																			
DRY: SEASONED LUMBER.																			
</																			

PLATES (table is in inches)				UNFACTORED REACTIONS				LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM			
JT	TYPE	PLATES	W	LEN	Y	X	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	LOADING	THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010	
B	TMVW-t	MT20	4.0	5.0	1.50	2.00	JT COMBINED	SNOW LIVE PERMLIVE WIND DEAD SOIL	55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD	THIS DESIGN COMPLIES WITH:	
C	TS-t	MT20	3.0	6.0			P 961	685 / 0 0 / 0 0 / 0 0 / 0 276 / 0 0 / 0		- PART 9 OF OBC 2012, BCBC 2012, ABC 2014	
D	TMVW-t	MT20	3.0	4.0	1.50	1.50	J 893	625 / 0 0 / 0 0 / 0 0 / 0 268 / 0 0 / 0		- CSA 086-09	
E	TTW-t	MT20	5.0	5.0	Edge	2.00	BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P				
F	TTW-t+m	MT20	4.0	5.0	2.25	1.75	BRACING				
G	TMVW-t	MT20	3.0	4.0	1.50	1.50	TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.81 FT.				
H	TS-t	MT20	3.0	6.0			MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.				
I	TMVW-t	MT20	4.0	5.0	1.50	2.00	ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.				
J	BMV1+p	MT20	2.0	4.0			1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF D-N, G-M. DBS = 20-0-0, CBF = 62 LBS.				
K	BMVW-t	MT20	4.0	4.0	1.75	1.50	DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.				
L	BS-t	MT20	3.0	6.0			END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW				
M	BMVW-t	MT20	3.0	4.0			LOADING				
N	BMVW-t	MT20	3.0	8.0			TOTAL LOAD CASES: (4)				
O	BMVW-t	MT20	4.0	4.0	1.75	1.50					
P	BMV1+p	MT20	2.0	4.0							

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-77.4 -77.4 0.09 (1)	10.00	C-D	-68 / 90	0.04 (1)	
B-C	-1470 / 0	-77.4 -77.4 0.51 (1)	4.81	D-N	-490 / 0	0.24 (1)	
C-D	-1470 / 0	-77.4 -77.4 0.51 (1)	4.81	N-E	0 / 381	0.09 (1)	
D-E	-1093 / 0	-77.4 -77.4 0.47 (1)	5.44	M-F	0 / 326	0.07 (1)	
E-F	-875 / 0	-77.4 -77.4 0.01 (1)	6.25	M-G	-498 / 0	0.24 (1)	
F-G	-1089 / 0	-77.4 -77.4 0.47 (1)	5.44	K-G	-59 / 93	0.03 (4)	
G-H	-1472 / 0	-77.4 -77.4 0.51 (1)	4.81	B-O	0 / 1266	0.28 (1)	
H-I	-1472 / 0	-77.4 -77.4 0.51 (1)	4.81	K-I	0 / 1268	0.29 (1)	
P-B	-1324 / 0	0.0 0.0 0.14 (1)	7.03	N-F	0 / 53	0.01 (1)	
J-I	-1225 / 0	0.0 0.0 0.13 (1)	7.25				
P-O	0 / 0	-17.5 -17.5 0.18 (4)	10.00				
O-N	0 / 1251	-17.5 -17.5 0.29 (1)	10.00				
N-M	0 / 872	-17.5 -17.5 0.23 (1)	10.00				
M-L	0 / 1253	-17.5 -17.5 0.30 (1)	10.00				
L-K	0 / 1253	-17.5 -17.5 0.30 (1)	10.00				
K-J	0 / 0	-17.5 -17.5 0.19 (4)	10.00				



Scale = 1:59.5

TOTAL WEIGHT = 112 lb [M][F]

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
G - H	2x4	DRY	No.2	SPF	
N - B	2x4	DRY	No.2	SPF	
I - H	2x4	DRY	No.2	SPF	
N - K	2x4	DRY	No.2	SPF	
K - I	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	5.0	1.50	2.00
C	TS-t	MT20	3.0	6.0		
D	TMVW-t	MT20	3.0	4.0	1.50	1.50
E	TTW+p	MT20	3.0	5.0		
F	TMVW-t	MT20	3.0	4.0	1.50	1.50
G	TS-t	MT20	3.0	6.0		
H	TMVW-t	MT20	4.0	5.0	1.50	2.00
I	BMV1+p	MT20	2.0	4.0		
J	BMVW-t	MT20	4.0	4.0	1.75	1.50
K	BS-t	MT20	3.0	6.0		
L	BMVW-t	MT20	4.0	6.0		
M	BMVW-t	MT20	4.0	4.0	1.75	1.50
N	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
N	1372	0	5-8	5-8
I	1273	0	MECHANICAL	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT I TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. SNOW	MIN. LIVE	PERM. LIVE	WIND	DEAD	SOIL
N	961	685 / 0	0 / 0	0 / 0	0 / 0	276 / 0	0 / 0
I	893	625 / 0	0 / 0	0 / 0	0 / 0	268 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.79 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF F-L, D-L, DBS = 20-0-0, CBF = 64 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MAX. UNBRAC LENGTH	MEMB.	WEBS MAX. FACTORED FORCE (LBS)	MAX. UNBRAC LENGTH
FR-TO		FROM	TO		FR-TO		
A-B	0 / 28	-77.4	-77.4	0.09 (1)	L-E	0 / 766	0.17 (1)
B-C	-1468 / 0	-77.4	-77.4	0.53 (1)	L-F	-509 / 0	0.26 (1)
C-D	-1468 / 0	-77.4	-77.4	0.53 (1)	J-F	-58 / 94	0.03 (4)
D-E	-1071 / 0	-77.4	-77.4	0.48 (1)	D-L	-509 / 0	0.26 (1)
E-F	-1071 / 0	-77.4	-77.4	0.48 (1)	M-D	-58 / 94	0.03 (4)
F-G	-1468 / 0	-77.4	-77.4	0.53 (1)	B-M	0 / 1265	0.28 (1)
G-H	-1468 / 0	-77.4	-77.4	0.53 (1)	J-H	0 / 1265	0.28 (1)
N-B	-1323 / 0	0.0	0.0	0.14 (1)			
I-H	-1224 / 0	0.0	0.0	0.13 (1)			
N-M	0 / 0	-17.5	-17.5	0.19 (4)			
M-L	0 / 1251	-17.5	-17.5	0.30 (1)			
L-K	0 / 1251	-17.5	-17.5	0.30 (1)			
K-J	0 / 1251	-17.5	-17.5	0.30 (1)			
J-I	0 / 0	-17.5	-17.5	0.19 (4)			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.89")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.89")
CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CSI: TC=0.53/1.00 (B-D:1), BC=0.30/1.00 (J-L:1), WB=0.28/1.00 (H-J:1), SSI=0.21/1.00 (B-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

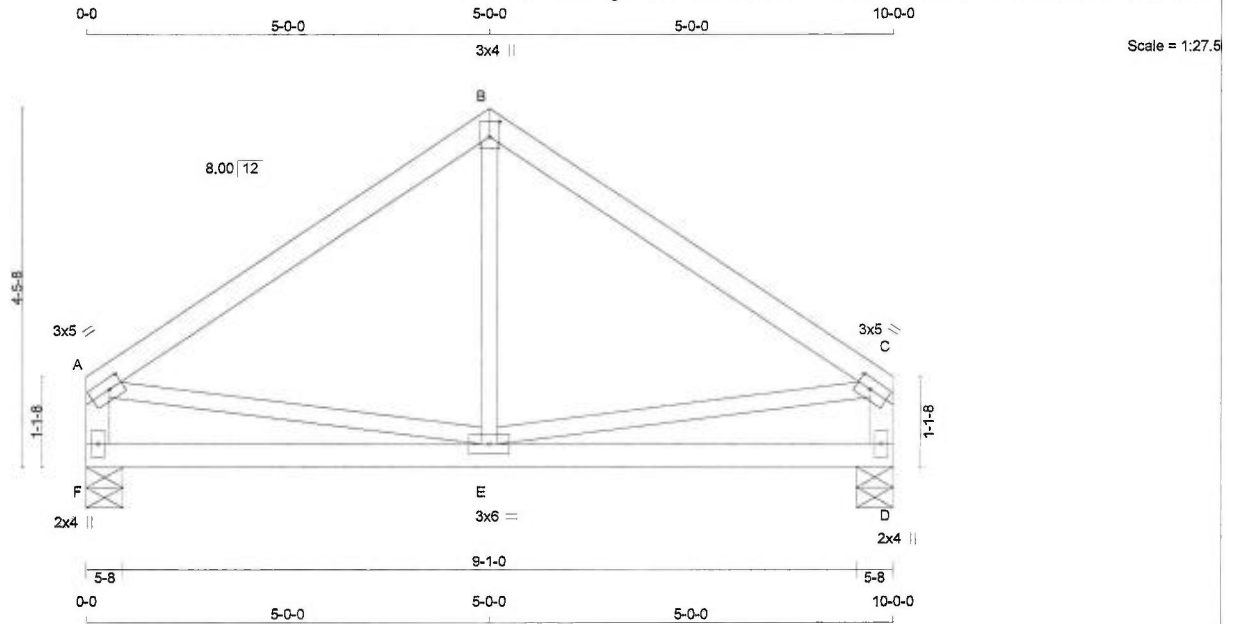
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.50 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 38 lb [M][F]

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - B	2x4 DRY	No.2		
B - C	2x4 DRY	No.2		
F - A	2x4 DRY	No.2		
D - C	2x4 DRY	No.2		
F - D	2x4 DRY	No.2		

ALL WEBS 2x3 DRY No.2 EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	3.0	5.0	1.50	2.00
B	TTW+p	MT20	3.0	4.0	2.25	1.50
C	TMVW-t	MT20	3.0	5.0	1.50	2.00
D	BMV1+p	MT20	2.0	4.0		
E	BMVWW-t	MT20	3.0	6.0		
F	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG
	VERT	HORZ	DOWN	HORZ
F	474	0	474	0
D	474	0	474	0

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	333	233 / 0	0 / 0	0 / 0	0 / 0	100 / 0	0 / 0
D	333	233 / 0	0 / 0	0 / 0	0 / 0	100 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	-359 / 0	-77.4 -77.4	0.25 (1)	E-B	0 / 90	0.03 (4)	
B-C	-359 / 0	-77.4 -77.4	0.25 (1)	A-E	0 / 302	0.07 (1)	
F-A	-440 / 0	0.0 0.0	0.04 (1)	E-C	0 / 302	0.07 (1)	
D-C	-440 / 0	0.0 0.0	0.04 (1)				
F-E	0 / 0	-17.5 -17.5	0.12 (4)				
E-D	0 / 0	-17.5 -17.5	0.12 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3 PSF
DL = 3.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.0 PSF	
TOTAL LOAD	= 33.3 PSF

SPACING = 24.0 IN./G/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.33")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.33")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.02")

CSI: TC=0.25/1.00 (A-B:1), BC=0.12/1.00 (E-F:4), WB=0.07/1.00 (A-E:1), SSI=0.13/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

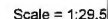
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.63 (E) (INPUT = 0.90)
JSI METAL= 0.14 (A) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = $3 \times 41 = 123$ lb

DRY: SEASONED LUMBER.

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVVW-t	MT20	3.0	5.0	1.50	2.00
C	TTW+p	MT20	3.0	4.0	2.25	1.50
D	TMVVW-t	MT20	3.0	5.0	1.50	2.00
E	BMV1+p	MT20	2.0	4.0		
F	BMWWW-t	MT20	3.0	6.0		
G	BMV1+p	MT20	2.0	4.0		

BUILDING BEARINGS

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT E TO RESIST THE MAX FACTORED REACTIONS.

1ST LCASE MAX./MIN. COMPONENT REACTIONS

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

TOTAL LOAD CASES: (4)

G-1	0/0	-17.5	-17.5	0.10 (4)	10.00
F-E	0/0	-17.5	-17.5	0.13 (4)	10.00

EMBL	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R-TO		
F-C	-23 / 79	0.03 (4)
B-F	0 / 280	0.06 (1)
F-D	0 / 285	0.06 (1)

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	33.3	PSF

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , CBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.33")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.33")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.27/1.00 (B-C:1) , BC=0.13/1.00 (E-F:4) ,
WB=0.06/1.00 (D-F:1) , SSI=0.13/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE	GRIP (DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

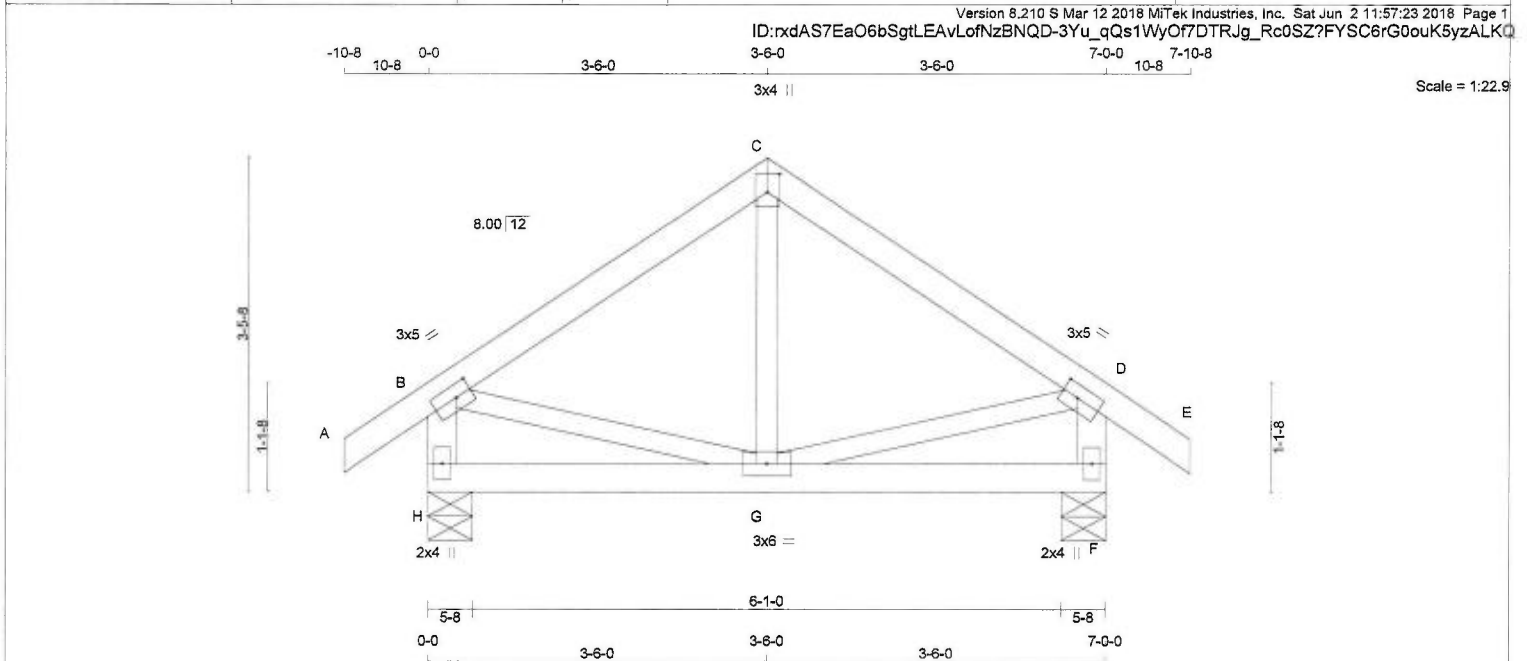
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.61 (F) (INPUT = 0.90)
JSI METAL= 0.15 (B) (INPUT = 1.00)



**READ ALL NOTES ON THIS PAGE AND ON
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NOTE PAGE IS AN INTEGRAL PART OF
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THE DESIGN OF THIS COMPONENT.**





LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER
A - C	2x4	DRY No.2
C - E	2x4	DRY No.2
H - B	2x4	DRY No.2
F - D	2x4	DRY No.2
H - F	2x4	DRY No.2

ALL WEBS 2x3 DRY No.2 EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	3.0	5.0	1.50	2.00
C	TTW+p	MT20	3.0	4.0	2.25	1.50
D	TMVW-t	MT20	3.0	5.0	1.50	2.00
F	BMV1+p	MT20	2.0	4.0		
G	BMVWW-t	MT20	3.0	6.0		
H	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
H	406	0	406	0	5-8	5-8
F	406	0	406	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE MAX./MIN. COMPONENT REACTIONS						
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	283	208 / 0	0 / 0	0 / 0	0 / 0	76 / 0	0 / 0
F	283	208 / 0	0 / 0	0 / 0	0 / 0	76 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 MAX (PLF)			MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)		
		FROM	TO	CSI (LC)			CSI (LC)		
FR-TO									
A-B	0 / 21	-77.4	-77.4	0.05 (1)	10.00	G-C	-15 / 55	0.02 (4)	
B-C	-231 / 0	-77.4	-77.4	0.12 (1)	6.25	B-G	0 / 197	0.04 (1)	
C-D	-231 / 0	-77.4	-77.4	0.12 (1)	6.25	G-D	0 / 197	0.04 (1)	
D-E	0 / 21	-77.4	-77.4	0.05 (1)	10.00				
H-B	-382 / 0	0.0	0.0	0.04 (1)	7.81				
F-D	-382 / 0	0.0	0.0	0.04 (1)	7.81				
H-G	0 / 0	-17.5	-17.5	0.06 (4)	10.00				
G-F	0 / 0	-17.5	-17.5	0.06 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.12/1.00 (B-C:1), BC=0.06/1.00 (G-H:4), WB=0.04/1.00 (D-G:1), SSI=0.09/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

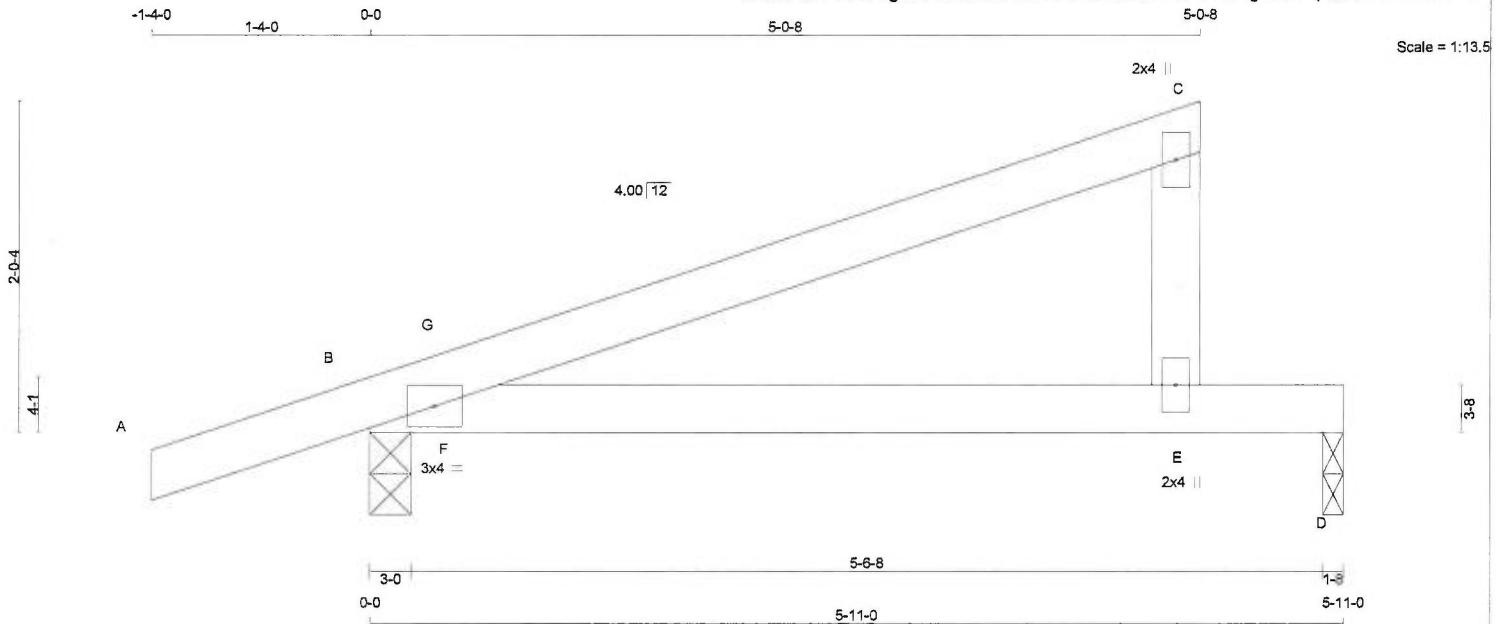
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.36 (G) (INPUT = 0.90)
JSI METAL= 0.10 (D) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 2 X 16 = 32 lb [M][F]

LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER

A - C	2x4	DRY	No.2
E - C	2x4	DRY	No.2
B - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	
C	TMV+p	MT20	2.0	4.0	
E	BMV+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
B	382	382	3-0	3-0
D	218	218	1-8	1-8

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX./MIN. SNOW	MIN. LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	266	199 / 0	0 / 0	0 / 0	0 / 0	67 / 0	0 / 0
D	154	100 / 0	0 / 0	0 / 0	0 / 0	54 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX (LC)	MAX. UNBRACED LENGTH	MEMB.	WEBS MAX. FACTORED FORCE (LBS)	MAX (LC)
FR-TO		FROM TO				FR-TO		
A-B	0 / 16	-77.4	-77.4	0.10 (1)	10.00	F-G	-115 / 66	0.00 (1)
B-G	-29 / 0	-77.4	-77.4	0.07 (4)	6.25			
G-C	0 / 3	-77.4	-77.4	0.29 (1)	10.00			
E-C	-182 / 0	0.0	0.0	0.02 (1)	7.81			
B-F	0 / 0	-17.5	-17.5	0.14 (1)	10.00			
F-E	0 / 0	-17.5	-17.5	0.27 (1)	10.00			
E-D	0 / 0	-17.5	-17.5	0.25 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. G/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/893 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/463 (0.15")

CSI: TC=0.29/1.00 (C-G:1), BC=0.27/1.00 (E-F:1), WB=0.00/1.00 (F-G:1), SS=0.17/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

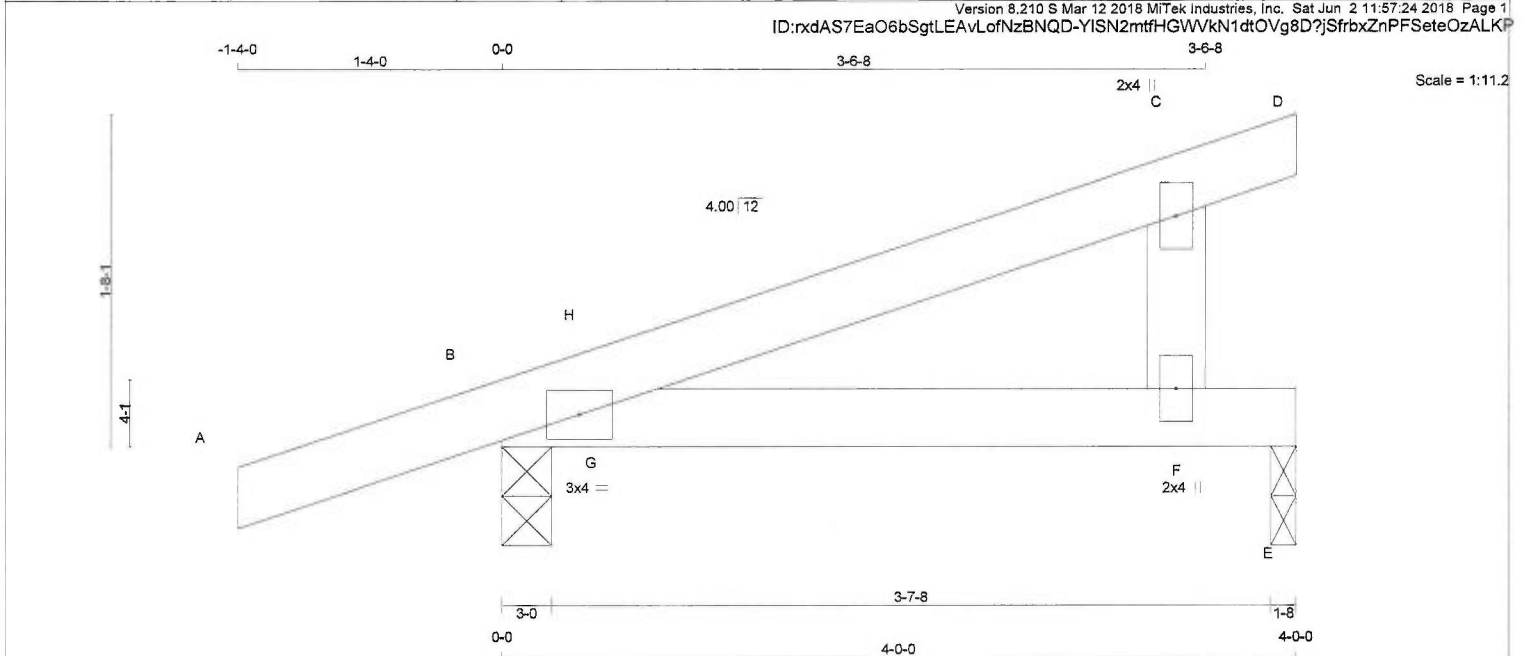
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 3 X 12 = 36 lb

LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.	FACTORED	MAXIMUM FACTORED	INPUT
A - D	2x4	DRY	No.2	SPF	GROSS REACTION	GROSS REACTION	BRG
F - C	2x4	DRY	No.2	SPF	VERT	DOWN	BRG
B - E	2x4	DRY	No.2	SPF	HORZ	HORZ	IN-SX
DRY: SEASONED LUMBER.				BEARINGS			
				JT	VERT	HORZ	UPLIFT
				B	299	0	3-0
				E	191	0	1-8

PLATES (table is in inches)				UNFACTORED REACTIONS			
JT	TYPE	PLATES	W	1ST LCASE	MAX./MIN.	COMPONENT REACTIONS	
B	TMB1-I	MT20	3.0	COMBINED	SNOW	LIVE	PERM.LIVE
C	TMV+p	MT20	2.0	E	134	94 / 0	0 / 0
F	BMV+p	MT20	2.0				

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING				DESIGN CRITERIA			
TOTAL LOAD CASES: (5)				SPECIFIED LOADS:			
				TOP CH. LL = 23.3 PSF			
				DL = 3.0 PSF			
				BOT CH. LL = 0.0 PSF			
				DL = 7.0 PSF			
				TOTAL LOAD = 33.3 PSF			
				SPACING = 24.0 IN. C/C			
				THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010			
				THIS DESIGN COMPLIES WITH:			
				- PART 9 OF OBC 2012, BCBC 2012, ABC 2014			
				- CSA 086-09			
				- TPIC 2011			
				(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD			
				ALLOWABLE DEFL.(LL)= L/360 (0.19")			
				CALCULATED VERT. DEFL(LL)= L/ 999 (0.02")			
				ALLOWABLE DEFL.(TL)= L/360 (0.19")			
				CALCULATED VERT. DEFL.(TL)= L/ 999 (0.03")			
				CSI: TC=0.14/1.00 (C-H:1), BC=0.13/1.00 (F-G:1), WB=0.00/1.00 (G-H:1) : SSI=0.15/1.00 (E-F:1)			
				DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10			
				COMPANION LIVE LOAD FACTOR = 0.50			
				TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.			
				NAIL VALUES			
				PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)			
				MAX MIN MAX MIN MAX MIN			
				MT20 618 354 1667 822 2284 1656			
				PLATE PLACEMENT TOL. = 0.250 inches			
				PLATE ROTATION TOL. = 5.0 Deg.			
				JSI GRIP= 0.15 (B) (INPUT = 0.90)			
				JSI METAL= 0.05 (B) (INPUT = 1.00)			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

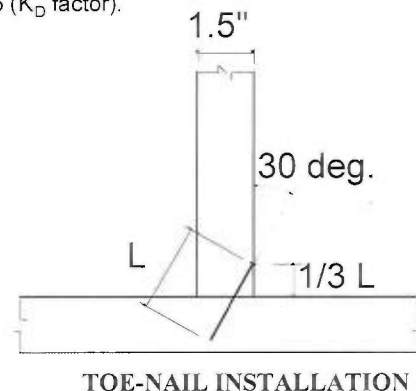
BEARING ANCHORAGE BY TOE-NAILS FOR LATERAL CAPACITY

B37579H1

NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL LATERAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	132	147
	3.25	0.144	132	147
	3.50	0.160	159	177

NOTES:

1. Rafter and ceiling members may be anchored to top and bottom chords of girder truss by toe-nailing rafter and ceiling members to girder chords provided the reaction does not exceed the lateral capacities in the table. Hangers (specified by others) are required for reactions higher than the maximum toe-nail capacity. Reactions are based on factored loads.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-09, section 10.9.4.1.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in tables below.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately $1/3$ the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See next page for nailing on bearing plate).
7. For loads due to wind the nail lateral capacity in this table may be multiplied by 1.15 (K_D factor).
8. Lumber must be dry (< 19% moisture content) at the time of nail installation.
9. Nail values in this table comply with CSA O86-09, section 10.9.4
10. This design is not valid after April 30, 2019.



Nail type	Common wire	Common spiral	Common wire
Nail dia. (in)	0.160	0.152	0.144
	(3.5" nail)		(3" and 3.25" nail)
LUMBER SIZE	MAXIMUM NUMBER OF TOE-NAILS		
2X4 SPF	2	2	3
2X4 D. Fir	2	2	2
2X6 SPF	4	4	4
2X6 D. Fir	3	3	3



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100 Industrial Rd.
Bradford, Ontario L3Z 3G7

PEO
Certificate No. 10889485



April 26, 2017

BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING

B37579H2

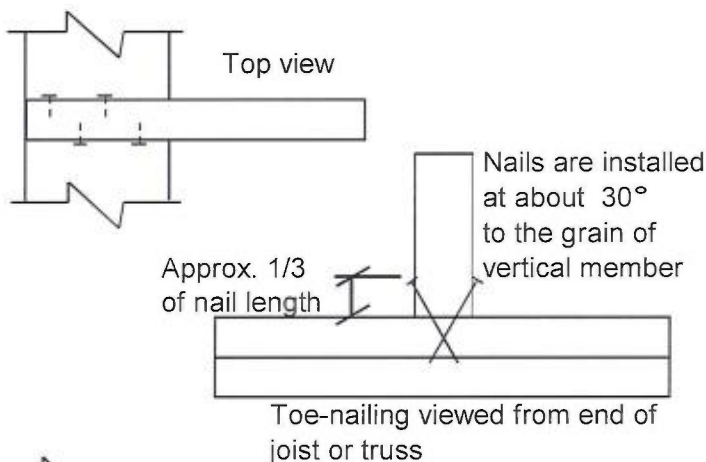
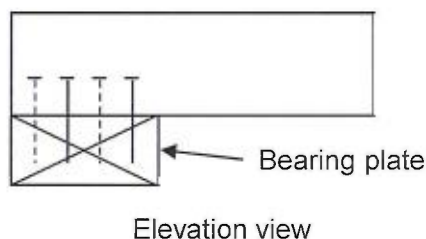
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	30	42
	3.25	0.144	32	45
	3.50	0.160	38	52

Note: If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

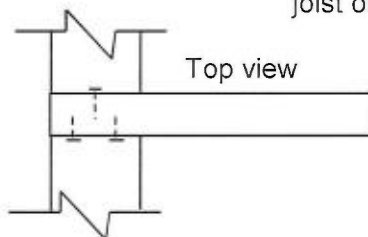
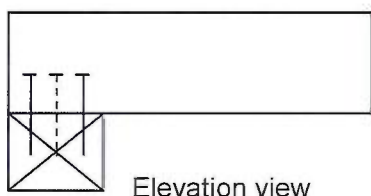
NOTES:

1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to **wind or earthquake** load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-09, section 10.9.5.2.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37579H1).
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-09, section 10.9.5
9. This design is not valid after April 30, 2019

Toe-nailing on 2x6 Bearing Plate



Toe-nailing on 2x4 Bearing Plate



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April 26, 2017