Label Description

Forex 2.0E-3000Fb LVL

Forex

Forex

Forex 2.0E-3000Fb LVL

Label Description

F15 LPI 20Plus

F14 LPI 20Plus

J1 LPI 20Plus

J6 LPI 20Plus

J5 LPI 20Plus

J4 LPI 20Plus

J10 LPI 20Plus

J2 LPI 20Plus

Label Description

2

Label Description

BLK1 LPI 20 Plus

Plus 1.125 X 11.875

Pcs Description

25 LF2511

Unknown

Hanger

1 HUS1.81/10

member using a face-mounted hanger.

Rim parallel to joists: 1-1/8" rimboard with

2"x4" block (1/16" longer than rim depth) @ 16" o/c.

All other components and structural elements supporting

HUS1.81/10

Framer to verify dimensions on the architectural drawings.

Refer to Nascor specifier guide for installation details.

Load transfer blocks to be installed under all point loads.

Double joist only require filler/backer ply when supporting another

Install 2x4 blocking @ 24" o/c under parallel non-loadbearing walls.

Refer to Multiple Member Connection Detail to ply to ply nailing or bolting

Install single-ply flush window header along inside face of rimboard/rimjoist

Squash blocks recommended to be installed at end bearing on all first level

Norbord Rimboard 1.125

J7 NJ60H

Rim Board

Hanger

Label

H2

НЗ

H5

Blocking

requirements.

2.0E-3000Fb LVL

2.0E-3000Fb LVL

2.0E-3000Fb LVL

2.0E-3000Fb LVL

Width Depth

1.75 11.875

1.75 11.875

1.75 11.875

1.75 11.875

1.75 11.875

1.75 11.875

Width Depth

2.5 11.875

2.5 11.875

2.5 11.875

2.5 11.875

2.5 11.875

2.5 11.875

2.5 11.875

2.5 11.875

2.5 11.875

Width Depth

11 875

Skew Slope

Width Depth Qty

2.5 11.875 LinFt

Beam/Girder

fasteners

30 10dx1 1/2

12 10d

Plies

LVL/LSL

F9

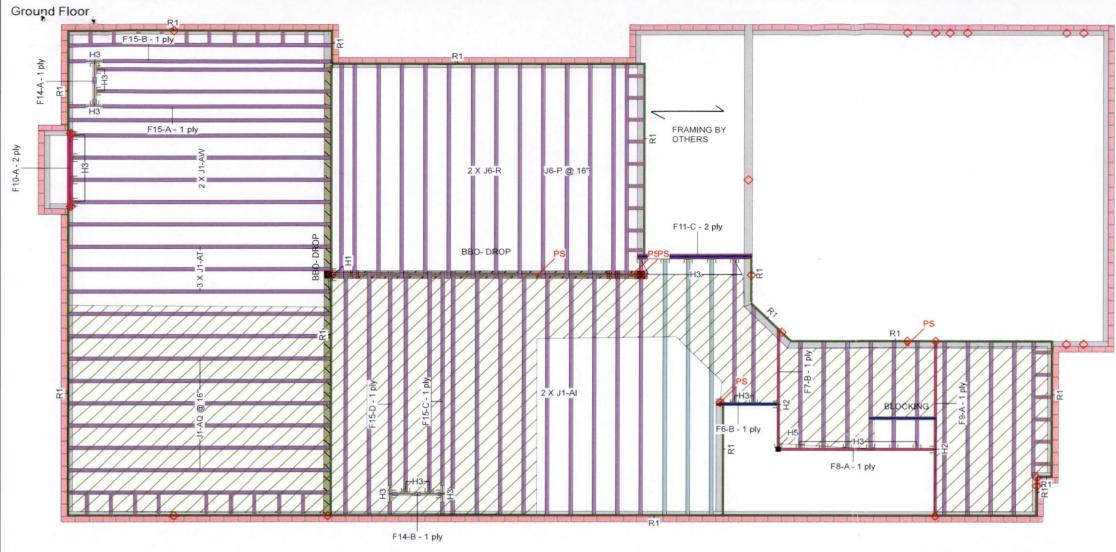
F8

F11

F7

F6

Joist



This certification is to confirm that:

multiple member connection detail.

shown on this layout.

1. The loads used in the calculation of the attached

approved components conform to the floor assembly

2. The floor joists comply with the KOTT span table

The floor system must be assembled in accordance

for the loads and spacing shown on this layout.

to the KOTT Specifier Guide. Multi-ply members

must be attached together as per the included

specifications forming part of the permit drawings.

All work shall conform to the Ontario Building Code O. Reg. 332/12 as amended

PROFESSIONA I.MATIJEVIC 100528832 January 02, 2019

the floor system such as beams, walls, columns and foundation walls and footings including anchorage of components and bracing for lateral stability are the responsibility of others. BUILDING REVIE Hatch area represents ceramic tiled floor with an additional dead load of 5 PSF. The framing shown on this layout may be deviate from the architectural

drawings. Project Engineer to review and approve the deviation prior to construction.

Engineered floor joists shall be installed in accordance with the supplier's layout and

19-447150.000 a

Legend

joists which support loading from above exceeding two levels floor or roof. It shall be the framer's responsibility that floor joists and beams are fastened as per the hanger manufacturer's standards

FRENETTE

Point Load Support Load from Above Wall Norbord Rimboard Plus 1.125 X 11.875 LPI 20Plus 11.875 NJ60H 11.875 Forex 2.0E-3000Fb LVL 1.75 X 11.875 5.25 X 10.25 (Dropped)

Qty Plies Pcs Length 10-0-0 LOT 9 (AMELIA 3 EL-2) 2 8-0-0 Design Method 8-0-0 Description GREEN YORK HOMES GRANELLI HOMES PROJECT BRAMPTON,ON 6-0-0 4-0-0 Created May 29, 2018 Qty Plies Pcs Length Builder 4 16-0-0 Sales Rep 4-0-0 30 16-0-0 14-0-0 Designer 18 12-0-0 10-0-0 Shipping 8-0-0 6-0-0 Project 3 16-0-0 Builder's Project **Kott Lumber Company** Plies Pcs Length 14 Anderson Blvd

Supported

Member

fasteners

Stouffville, Ontario Canada

L4A 7X4 905-642-4400

Ground Floor Design Method LSD Building Code NBCC 2010 / OBC 2012

40

15

360

240

480

10 16d Floor 1 #8x1 1/4WS Loads Live Dead Pcs Length

Deflection Joist Varies 46-0-0 480 L Span L/ 360 TL Span L/ 480 LL Cant 2L/ 360 TL Cant 2L/ Deflection Girder

LL Span L/ TL Span L/ LL Cant 2L/ TL Cant 2L/

360 Decking OSB Deck Thickness 3/4"

Fastener Nailed & Glued

Vibration

M-2057 LOT 9

KOTT

1. OBC 2012 O.Reg 332/12 as amended

2. Nascor CCMC - 13535-R

JOISTS SPACING 16"O/C

NOTED OTHERWISE

Architectural Drawing Info

JARDIN DESIGN GROUP

64 JARDIN DR, SUITE 3A

VAUGHAN, ON L4K 3P3

Model: LOT-9 (AMELIA 3 EL-2)

Project # 17-55

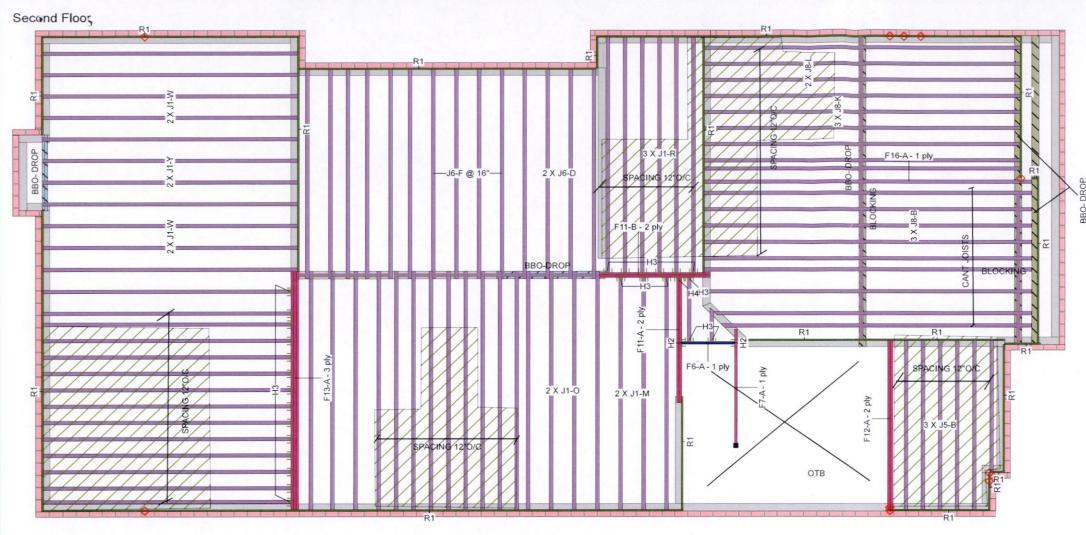
UNLESS

Date: AUG 30, 2018

3. LVL CCMC -12904-R

4. CAN/CSA-086-09

5. CCMC -12787-R APA PR-L310(C)



Architectural Drawing Info

JARDIN DESIGN GROUP 64 JARDIN DR, SUITE 3A VAUGHAN, ON L4K 3P3 Project # 17-55 Model: LOT 9 (AMELIA 3 EL-2) Date: AUG 30, 2018

JOISTS SPACING 16"O/C UNLESS NOTED OTHERWISE

- 1. OBC 2012 O.Reg 332/12 as amended
- 2. Nascor CCMC 13535-R
- 3. LVL CCMC -12904-R
- 4 CAN/CSA-086-09
- 5. CCMC -12787-R APA PR-L310(C)

This certification is to confirm that:

- . The loads used in the calculation of the attached approved components conform to the floor assembly shown on this layout.
- 2. The floor joists comply with the KOTT span table for the loads and spacing shown on this layout. The floor system must be assembled in accordance to the KOTT Specifier Guide. Multi-ply members must be attached together as per the included nultiple member connection detail.





LVL/LSL Width Depth Qty Plies Pcs Length Label Description F13 1.75 11.875 3 3 Forex 2.0E-3000Fb LVL Layout Name F12 Forex 1.75 11.875 2 12-0-0 LOT 9 (AMELIA 3 EL-2) 2.0E-3000Fb LVL Forex 1.75 11.875 8-0-0 Design Method 2.0E-3000Fb LVL F7 1.75 11.875 Forex 8-0-0 Description 2.0E-3000Fb LVL GREEN YORK HOMES F6 Forex 1.75 11.875 4-0-0 GRANELLI HOMES PROJECT 2.0E-3000Fb LVL BRAMPTON,ON Joist Label Description Width Depth Qty Plies Pcs Length Created J8 LPI 20Plus 2.5 11.875 18 20-0-0 May 29, 2018 16-0-0 Builder J1 LPI 20Plus 2.5 11.875 50 2.5 11.875 J6 LPI 20Plus 14 14-0-0 Sales Rep J5 LPI 20Plus 2.5 11.875 5 12-0-0 J10 LPI 20Plus RM 2.5 11.875 8-0-0 4-0-0 Designer J9 LPI 20Plus 2.5 11.875 2 F16 LPI 20Plus 2.5 11.875 1 20-0-0 Rim Board Shipping Label Description Qty Plies Pcs Length Width Depth Project R1 Norbord Rimboard 1.125 11.875 Plus 1.125 X Builder's Project 11.875 Kott Lumber Company Blocking 14 Anderson Blvd Label Description Width Depth Qty Plies Pcs Length Stouffville, Ontario BLK1 LPI 20 Plus 2.5 11.875 LinFt Varies 34-0-0 Canada Hanger L4A 7X4 Beam/Girder Supported 905-642-4400 Member Label Pcs Description Skew Slope Second Floor fasteners fasteners H2 2 HUS1.81/10 30 16d 10 16d Design Method 26 LF2511 Building Code NBCC 2010 / OBC Н3 12 10d 1 #8x1 1/4WS H4 1 HGUS410 46 16d 16 16d Floor NOTES: Loads Framer to verify dimensions on the architectural drawings. Live Double joist only require filler/backer ply when supporting another Dead member using a face-mounted hanger. Deflection Joist Install 2x4 blocking @ 24" o/c under parallel non-loadbearing walls. Install single-ply flush window header along inside face of rimboard/rimjoist LL Span L/ Refer to Nascor specifier guide for installation details. TL Span L/ Squash blocks recommended to be installed at end bearing on all first level LL Cant 2L/ joists which support loading from above exceeding two levels floor or roof. TL Cant 2L/ Load transfer blocks to be installed under all point loads It shall be the framer's responsibility that floor joists and beams Deflection Girder are fastened as per the hanger manufacturer's standards. LL Span L/ TL Span L/ Refer to Multiple Member Connection Detail to ply to ply nailing or bolting requirements. LL Cant 2L/ TL Cant 2L/ Rim parallel to joists: 1-1/8" rimboard with Decking 2"x4" block (1/16" longer than rim depth) @ 16" o/c. All other components and structural elements supporting Deck the floor system such as beams, walls, columns and Thickness foundation walls and footings including anchorage of

Legend

to construction



responsibility of others.

Point Load Support Load from Above Wall Norbord Rimboard Plus 1.125 X 11.875 LPI 20Plus 11.875

Hatch area represents ceramic tiled floor with an additional dead load

The framing shown on this layout may be deviate from the architectural

drawings. Project Engineer to review and approve the deviation prior

components and bracing for lateral stability are the

Forex 2.0E-3000Fb LVL 1.75 X 11.875 1.75 X 9.5 (Dropped) 5 X 10.25 (Dropped)

M-2057 LOT 9

Fastener

Vibration

Ceiling:

LSD

2012

40

15

480

360

480

360

360 240

480

360

OSB

5/8"

Nailed & Glued

Gypsum 1/2"



Engineering Note Page (ENP-2)

REVISION 2018-10-17

M-2057 LOT 9

Please read all notes prior to installation of the component

DESIGN INFORMATION

This building component is certified as an individual component for the loads and conditions shown on the calculation and drawing page.

The responsibility of the undersigned engineer is <u>only</u> limited to the calculation of this building component for the loads and conditions shown on this drawing.

The responsibility of the undersigned is limited to the verification of the structural capacity of the floor joists and LVL beams based on placement as shown on the layout. The loads applied are limited to the gravity effects of the specified loads. The structural integrity of the building and the effect of wind, uplift, seismic, lateral or other forces, calculation of adequate support and anchorage of components, as well as the dimensions and design loads used to calculate components are the responsibility of the overall building designer.

Floor joists and OSB rim board are designed to carry uniformly distributed loads only. Point loads should be transferred through the floor cavity with transfer blocks. Structural elements such as walls, posts, connectors, and transfer blocks are the responsibility of the overall building designer.

The undersigned engineer disclaims any responsibility for damages as a result of being furnished faulty or incorrect information, specifications and/or designs.

Installation of floor joists is to be carried out in accordance with the current edition of the manufacturer's literature available at http://www.kottgroup.com.

CODE

This building component is designed in accordance with the National Building Code of Canada, the Ontario Building Code, CCMC and Canadian Standards Association guidelines.

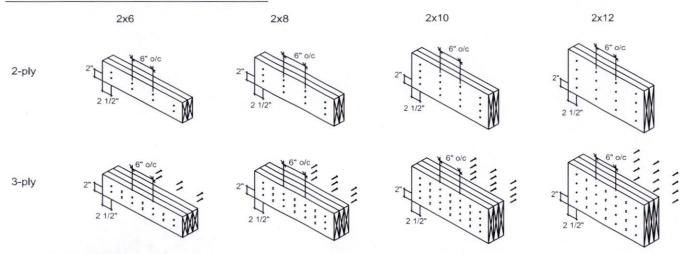
COMPONENT

- 1. The building component used in construction must be the same as indicated on the drawings.
- 2. The building component must be installed and assembled as per specification shown on the drawing and in accordance with the manufacturer's assembly and installation.
- 3. Members consisting of multiple plies must be connected as per the document "Multi-ply Connection Details".
- 4. Pass-thru transfer block framing is required at all point loads over bearings.

HANDLING AND INSTALLATION

Do not drill any hole, cut or notch a certified building component without a written preauthorization.

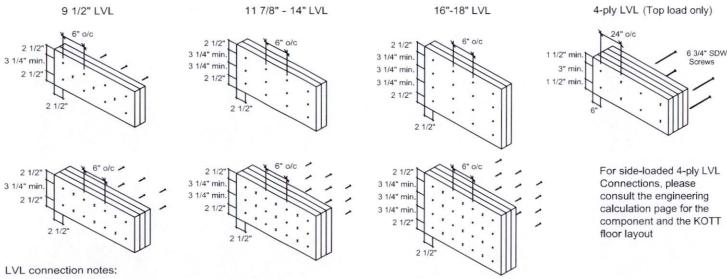
Conventional Connections



Conventional connection notes:

- -Nails to be 3" long wire nails.
- -Nails to be located 2" min. from the top and bottom of the member. Start all nails 2 1/2" min. from ends.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail driven from the opposite side.

LVL Connections



- -LVL ply width is 1-3/4"
- -Nails to be 3 1/2" common wire nails.
- -Nails to be located 2 1/2" min. from the top and bottom of the member. Start all nails 2 1/2" min. from ends.
- -Minimum 3 1/4" spacing between rows.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail or screw driven from the opposite side.

Multiple Member Connections

All connections are for uniformly distributed loads.

For multi-ply connections of I-joists, refer to Manufacturer's Installation Guide



KOTT Inc. 3228 Moodie Drive Ottawa, ON K2H 7V1 613-838-2775



Client

Project: Address Date: 12/18/2018

Designer: SB Job Name: LOT 9 (AMELIA 3 EL-2)

Project #:

Forex 2.0E-3000Fb LVL

1.750" X 11.875"

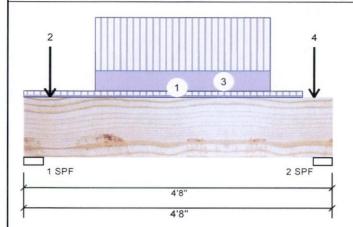
2-Ply - PASSED Level: Ground Floor

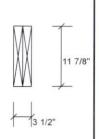
Brg

1

1 - SPF 3.500"

2 - SPF 3.500"





Wind

0

1.25D+1.5L

1.25D+1.5L

Member Information

Г	Type:	Girder	Application:	Floor (Residential)
	Plies:	2	Design Method:	LSD
	Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
	Deflection LL:	360	Load Sharing:	No
	Deflection TL:	240	Deck:	Not Checked
	Importance:	Normal	Vibration:	Not Checked
	General Load			
	Floor Live:	40 PSF		
	Dead:	15 PSF		

Unfactored Reactions UNPATTERNED Ib (Uplift)

21%

20%

Dead

314

Live

776

2	759	308		0	0					
Bearings and Factored Reactions										
Bearing	Length	Cap. React D/L lb	Total	Ld. Case	Ld. Comb.					

393 / 1165

385 / 1139

Snow

1557 L

1524 L

0

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1247 ft-lb	2'3 11/16"	34261 ft-lb	0.036 (4%)	1.25D+1.5L	L
Unbraced	1247 ft-lb	2'3 11/16"	34261 ft-lb	0.036 (4%)	1.25D+1.5L	L
Shear	1546 lb	1'2 5/8"	11596 lb	0.133 (13%)	1.25D+1.5L	L
Perm Defl in.	0.002 (L/33117)	2'3 13/16"	0.140 (L/360)	0.010 (1%)	D	Uniform
LL Defl inch	0.004 (L/13619)	2'3 13/16"	0.140 (L/360)	0.030 (3%)	L	L
TL Defl inch	0.005 (L/9650)	2'3 13/16"	0.210 (L/240)	0.020 (2%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.

/	PROFESSIONA	
LICENSE	I.MATIJEVIC 100528832	CNGINEER
199	DVINCE OF ONT	

January 02, 2019

Comments

o Lateral	sienderness ratio base	ed on full section width.					
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow
1	Tie-In	0-0-0 to 4-2-8	(Span)1-2-0	Тор	15 PSF	40 PSF	0 PSF
2	Point	0-4-10		Near Face	139 lb	371 lb	0 lb
3	Part. Uniform	1-0-10 to 3-8-10		Near Face	106 PLF	281 PLF	0 PLF
4	Point	4-4-10		Near Face	119 lb	317 lb	0 lb
	Self Weight	READ ALL NOTES ON	READ ALL NOTES ON THIS PAGE AND ON THE				

ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Pass Thru Framing Squash Block is required at all point loads over bearings 0 lb J1 Refer to Multiple Member Connection

Wind

0 PSF 0 lb J1

Detail for ply to ply nailing or bolting requirements

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVI. beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation

This design is valid until 10/18/2021

roofs provide proper drainage to prevent



APA: PR-L318







Client:

Project: Address: Date: 12/18/2018

Designer:

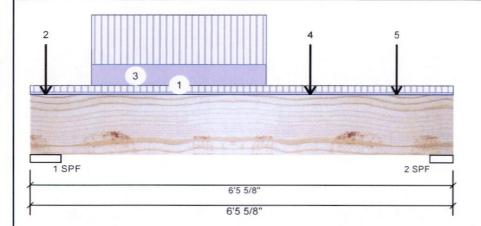
Job Name: LOT 9 (AMELIA 3 EL-2)

Project #:

Forex 2.0E-3000Fb LVL

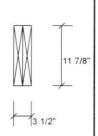
1.750" X 11.875"

2-Ply - PASSED Level: Ground Floor



Deck:

Vibration:



Wind

0

0

Member Information

Type:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	
Floor Live:	40 PSF
Dead:	15 PSF

Floor (Residential) Application: LSD Design Method: Building Code: NBCC 2010 / OBC 2012

Load Sharing: No Not Checked

Not Checked

Brg

1

2

Dead 383 385

Unfactored Reactions UNPATTERNED Ib (Uplift)

Snow 0 0

Bearings and Factored Reactions

Live

837

823

Bearing	Length	Cap. F	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	5.500"	15%	479 / 1256	1735	L	1.25D+1.5L
2 - SPF	4.375"	18%	482 / 1234	1716	L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2763 ft-lb	3'2 11/16"	34261 ft-lb	0.081 (8%)	1.25D+1.5L	L
Unbraced	2763 ft-lb	3'2 11/16"	32711 ft-lb	0.084 (8%)	1.25D+1.5L	L
Shear	1902 lb	5'2 1/8"	11596 lb	0.164 (16%)	1.25D+1.5L	L
Perm Defl in.	0.005 (L/12838)	3'3 1/4"	0.192 (L/360)	0.030 (3%)	D	Uniform
LL Defl inch	0.012 (L/5833)	3'3 1/16"	0.192 (L/360)	0.060 (6%)	L	L
TL Defl inch	0.017 (L/4011)	3'3 1/8"	0.289 (L/240)	0.060 (6%)	D+L	L

Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



January 02, 2019

7 Lateral	slenderness ratio based	on full section width.					
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow
1	Part. Uniform	0-0-0 to 6-5-10		Тор	15 PLF	40 PLF	0 PLF
2	Point	0-2-12		Тор	1 lb	0 lb	0 lb
3	Part. Uniform	0-10-12 to 3-6-12		Near Face	129 PLF	305 PLF	0 PLF
4	Point	4-2-12		Near Face	171 lb	387 lb	0 lb
5	Point	5-6-12		Near Face	94 lb	201 lb	0 lb
	Self Weight				10 PLF		

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- Damaged Beams must not be used

Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent

Manufacturer Info

APA: PR-L318

Wind

0 PLF

Kott Lumber Company 14 Anderson Blvd. Ontario Canada L4A 7X4 905-642-4400



Comments





Client

Project:

Address:

Date: 12/18/2018

Designer:

Job Name: LOT 9 (AMELIA 3 EL-2)

Brg

1

2

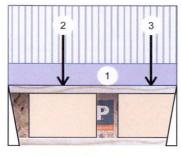
Hanger

Hanger

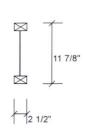
2 -

11.875" - PASSED LPI 20Plus

Level: Ground Floor



1 Hanger (LF2511) 2 Hanger (LF2511) 2'6 1/2" 2'6 1/2"



Wind

0

0

Member Information

Γ	Type:	Girder	Application:	Floor (Residential)
	Plies:	1	Design Method:	LSD
	Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
	Deflection LL:	360	Load Sharing:	No
	Deflection TL:	240	Deck:	Not Checked
	Importance:	Normal	Vibration:	Not Checked
	General Load			
	Floor Live:	40 PSF		
	Dead:	15 PSF		
		4		
		1		

Unfactored Reactions UNPATTERNED Ib (Uplift)

Dead

99

123

Live

266

328

Bearing:	s and Fac	tored R	eactions			
Bearing	Length	Cap. I	React D/L lb	Total	Ld. Case	Ld. Comb.
1 -	2.000"	33%	124 / 399	523	L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	353 ft-lb	9 3/4"	6250 ft-lb	0.056 (6%)	1.25D+1.5L	L
Shear	641 lb	2'5 1/4"	2345 lb	0.273 (27%)	1.25D+1.5L	L
Perm Defl in.	0.001 (L/20184)	9 3/4"	0.078 (L/360)	0.020 (2%)	D	Uniform
LL Defl inch	0.004 (L/7547)	9 3/4"	0.078 (L/360)	0.050 (5%)	L	L
TL Defl inch	0.005 (L/5493)	9 3/4"	0.117 (L/240)	0.040 (4%)	D+L	L

1.25D+1.5L 2.000" 41% 154 / 493 646 L 1.25D+1.5L

Snow

0

0

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.001", Long Term = 0.002"
- 3 Fill all hanger nailing holes.
- 4 See manufacture installation guide note E4 for installation details
- 5 Girders are designed to be supported on the bottom edge only.
- 6 Top flange braced at bearings.
- 7 Bottom flange braced at bearings.

13	PROFESSIONA	2
LICEN	I.MATIJEVIC 100528832	INEER
13	OVINCE OF ONT	
	VCE OF ON	

January 02, 2019

Γ	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
l	1	Tie-In	0-0-0 to 2-6-8	(Span)1-4-9	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
l	2	Point	0-9-12		Near Face	108 lb	289 lb	0 lb	0 lb	J6
l	3	Point	2-1-12		Near Face	88 lb	235 lb	0 lb	0 lb	J6

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

Copyright 2016 All rights reserved by Louisiana Pacific Corp. 414 Union St Suite 2000, Nashville, TN 37219

Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325 www.lpcorp.com CCMC: 12412-R APA: PR-L238C

Kott Lumber Company 14 Anderson Blvd, Ontario Canada L4A 7X4 905-642-4400









Client: Project:

Address:

12/18/2018 Date:

Designer: SB

Job Name: LOT 9 (AMELIA 3 EL-2)

Project #

Brg

2

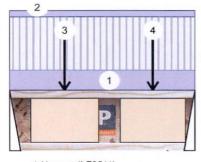
2 -

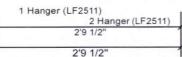
Hanger

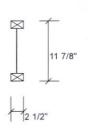
LPI 20Plus

11.875" - PASSED

Level: Ground Floor







Wind

0

0

1.25D+1.5L

Member Information

Type:	Girder
Plies:	1
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	
Floor Live:	40 PSF
Dead:	15 PSF

Floor (Residential) Application: Design Method: LSD NBCC 2010 / OBC 2012 **Building Code:** Load Sharing: No

Not Checked Vibration: Not Checked

Unfactored Reactions UNPATTERNED Ib (Uplift)

Live

304

328

2.000"

Dead

148

161

Snow

694 L

0

Rearing	s and Fac	tored R	eactions			F	_
_			React D/L lb	Total	Ld. Case	Ld. Comb.	_
1 - Hanger	2.000"	40%	185 / 455	640	L	1.25D+1.5L	

202 / 492

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	436 ft-lb	9 3/4"	6250 ft-lb	0.070 (7%)	1.25D+1.5L	L
Shear	689 lb	2'8 1/4"	2345 lb	0.294 (29%)	1.25D+1.5L	L
Perm Defl in.	0.002 (L/14174)	1'1 9/16"	0.086 (L/360)	0.030 (3%)	D	Uniform
LL Defl inch	0.004 (L/6920)	1'1 3/8"	0.086 (L/360)	0.050 (5%)	L	L
TI Defl inch	0.007 (L/4650)	1'1 7/16"	0.129 (L/240)	0.050 (5%)	D+L	L

Deck:

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.002", Long Term = 0.003"
- 3 Fill all hanger nailing holes.
- 4 See manufacture installation guide note E4 for installation details
- 5 Girders are designed to be supported on the bottom edge only.
- 6 Top flange braced at bearings

7 Bottom flange braced at bearings



January 02, 2019

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 2-9-8	(Span)1-3-7	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Part. Uniform	0-0-0 to 2-9-8		Тор	3 PLF	0 PLF	0 PLF	0 PLF	
3	Point	0-9-12		Far Face	141 lb	291 lb	0 lb	0 lb	J6
4	Point	2-1-12		Far Face	133 lb	269 lb	0 lb	Pass ₅ T	hru Framing Squash Block is d at all point loads over bearings

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

Copyright 2016 All rights reserved by Louisiana Pacific Corp. 414 Union St Suite 2000, Nashville, TN 37219



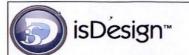
This design is valid until 10/31/2020

Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325 www.lpcorp.com CCMC: 12412-R APA: PR-L238C







Client:

Project: Address: Date: 12/18/2018

Designer: SB

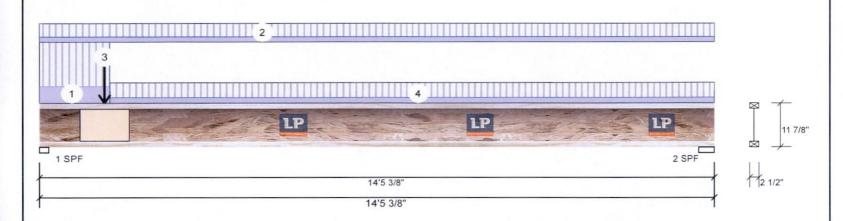
Job Name: LOT 9 (AMELIA 3 EL-2)

Project #

F15-A LPI 20Plus 1

11.875" - PASSED

Level: Ground Floor



Member Information

Type:	Girder
Plies:	1
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	
Floor Live:	40 PSF
Dead:	15 PSF

Application: Floor (Residential)
Design Method: LSD

Design Method: LSD

Building Code: NBCC 2010 / OBC 2012

Load Sharing: No.

Load Sharing: No
Deck: Not Checked
Vibration: Not Checked

Unfactored Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	Wind
1	544	203	0	0
2	280	105	0	0

Bearings and Factored Reactions

Bearing	Length	Cap. F	React D/L lb	Total	Ld. Case	Ld. Comb.	
1 - SPF	2.375"	65%	254 / 816	1070	L	1.25D+1.5L	
2 SDE	4 125"	30%	131 / 420	552	T.	1 25D+1 5I	

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2057 ft-lb	6'5 1/4"	6250 ft-lb	0.329 (33%)	1.25D+1.5L	L
Shear	1051 lb	1 5/8"	2345 lb	0.448 (45%)	1.25D+1.5L	L
Perm Defl in.	0.052 (L/3252)	6'10 7/8"	0.468 (L/360)	0.110 (11%)	D	Uniform
LL Defl inch	0.138 (L/1218)	6'10 7/8"	0.468 (L/360)	0.300 (30%)	L	L
TL Defl inch	0.190 (L/886)	6'10 7/8"	0.702 (L/240)	0.270 (27%)	D+L	L

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.052", Long Term = 0.078"
- 3 See manufacture installation guide note E4 for installation details
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top flange must be laterally braced at a maximum of 7'3" o.c.

6 Bottom flange braced at bearings.

15	PROFESSIONAL	2
LICEN	I.MATUEVIC 100528832	GINEER
12	DVINCE OF ONTA	
	VCE OF ON	

January 02, 2019

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-5-4	(Span)2-9-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 14-5-6	(Span)0-10-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	1-4-0		Far Face	99 lb	266 lb	0 lb	0 lb	F14
4	Tie-In	1-5-4 to 14-5-6	(Span)0-11-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

Copyright 2016 All rights reserved by Louisiana Pacific



This design is valid until

Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325 www.lpcorp.com CCMC: 12412-R APA: PR-L238C







Client:

Project: Address: Date: 12/18/2018

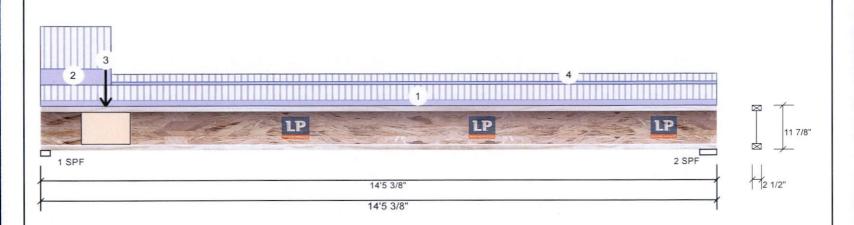
Designer: S B

Job Name: LOT 9 (AMELIA 3 EL-2)

Project #

F15-B LPI 20Plus 11.875" - PASSED

Level: Ground Floor



Unfactored Reactions UNPATTERNED Ib (Uplift) Member Information Application: Floor (Residential) Brg Live Dead Snow Wind Type Girder LSD Design Method: 576 216 0 Plies 1 0 Building Code: NBCC 2010 / OBC 2012 Moisture Condition: Dry 2 250 94 0 0 Load Sharing: No Deflection LL: 360 Deflection TL: 240 Deck: Not Checked Importance: Normal Vibration Not Checked General Load **Bearings and Factored Reactions** Floor Live: 40 PSF Dead: 15 PSF Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1 - SPF 2.375" 69% 270 / 864 1134 L 1.25D+1.5L 2 - SPF 4.500" 27% 117 / 375 493 1.25D+1.5L

Analysis Results

Ī	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
	Moment	1902 ft-lb	6'1 5/16"	6250 ft-lb	0.304 (30%)	1.25D+1.5L	L
	Shear	1114 lb	1 5/8"	2345 lb	0.475 (48%)	1.25D+1.5L	L
	Perm Defl in.	0.048 (L/3510)	6'9 5/8"	0.467 (L/360)	0.100 (10%)	D	Uniform
	LL Defl inch	0.128 (L/1316)	6'9 5/8"	0.467 (L/360)	0.270 (27%)	L	L
	TI Deflinch	0.176 (L/957)	6'9 5/8"	0.700 (L/240)	0.250 (25%)	D+L	L

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.048", Long Term = 0.072"
- 3 See manufacture installation guide note E4 for installation details
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top flange must be laterally braced at a maximum of 7'6" o.c.

6 Bottom flange braced at bearings.

	30 VINCE OF ON
	January 02, 2019
Wind	Comments
0 PSF	
0 PSF	

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 14-5-6	(Span)1-0-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 1-5-4	(Span)2-9-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	1-4-0		Near Face	123 lb	328 lb	0 lb	0 lb	F14
4	Tie-In	1-5-4 to 14-5-6	(Span)0-6-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Pass-Thru Framing Squash Block is required at all point loads over bearings

PROFESSIONAL

I.MATIJEVIC 100528832

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

Copyright 2016 All rights reserved by Louisiana Pacific



This design is valid until 10/31/2020

Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325 www.lpcorp.com CCMC: 12412-R APA: PR-L238C







Client: Project:

Address:

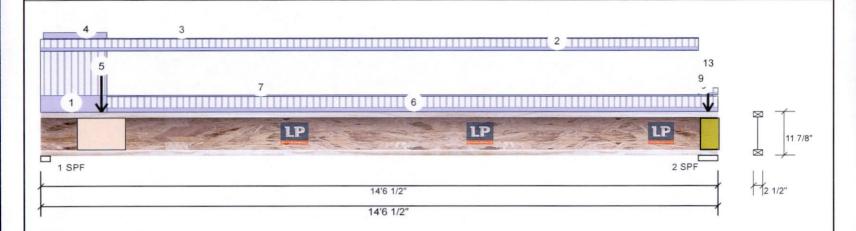
12/18/2018 Date:

Designer: SB

Job Name: LOT 9 (AMELIA 3 EL-2)

Project #

LPI 20Plus 11.875" - PASSED Level: Ground Floor



Unfactored Reactions UNPATTERNED Ib (Uplift) Member Information Floor (Residential) Dead Wind Type Application: Bra Live Plies: 1 Design Method: LSD 551 270 0 0 1 NBCC 2010 / OBC 2012 Moisture Condition: Dry Building Code: 2 522 267 0 0 Load Sharing: No Deflection LL 360 Deflection TL: 240 Deck: Not Checked Not Checked Vibration: Importance: Normal General Load **Bearings and Factored Reactions** Floor Live: 40 PSF Dead: 15 PSF Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1 - SPF 2.375" 71% 338 / 827 1164 L 1.25D+1.5L 2 - SPF 5.250" 61% 333 / 783 1117 L 1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1834 ft-lb	6'1 1/8"	6250 ft-lb	0.293 (29%)	1.25D+1.5L	L
Shear	1144 lb	1 5/8"	2345 lb	0.488 (49%)	1.25D+1.5L	L
Perm Defl in.	0.056 (L/2990)	6'9 3/4"	0.468 (L/360)	0.120 (12%)	D	Uniform
LL Defl inch	0.115 (L/1461)	6'9 3/4"	0.468 (L/360)	0.250 (25%)	L	L
TI Defl inch	0.172 (L/982)	6'9 3/4"	0.702 (L/240)	0.240 (24%)	D+L	L

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Applied loads over end bearings and loads exceeding 250 lbs over intermediate bearings must be transferred directly to the support by rim board, blocking, squash blocks, or other device
- 3 Dead Load Deflection: Instant = 0.056", Long Term = 0.084"
- 4 See manufacture installation guide note E4 for installation details
- 5 Girders are designed to be supported on the bottom edge only.
- 6 Top flange must be laterally braced at a maximum of 7'7" o.c.

7 Bottom f	lange braced at bearings.								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-4-2	(Span)3-0-0 to 3-0-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 14-1-4	(Span)0-7-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	hru Framing Squash Block is
3	Part. Uniform	0-0-10 to 14-1-4		Тор	1 PLF	0 PLF	0 PLF		ed at all point loads over bearings
4	Part. Uniform	0-0-12 to 1-4-2		Тор	8 PLF	0 PLF	0 PLF	0 PLF	
5	Point	1-2-14		Far Face	161 lb	328 lb	0 lb		o Multiple Member Connection for ply to ply nailing or bolting
ontinued on	page 2	DEAD ALL NOT	ES ON THIS DA	GE AND ON TH	IE .			require	

5 Co

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the

Copyright 2016 All rights reserved by Louisiana Pacific Corp. 414 Union St Suite 2000, Nashville, TN 37219

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



This design is valid until 10/31/2020

Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325 www.lpcorp.com CCMC: 12412-R APA: PR-L238C Kott Lumber Company 14 Anderson Blvd, Ontario Canada L4A 7X4 905-642-4400

I.MATIJEVIC 100528832

BOVINCE OF

January 02, 2019







Client:

Project: Address: Date: 12/18/2018

Designer: S B

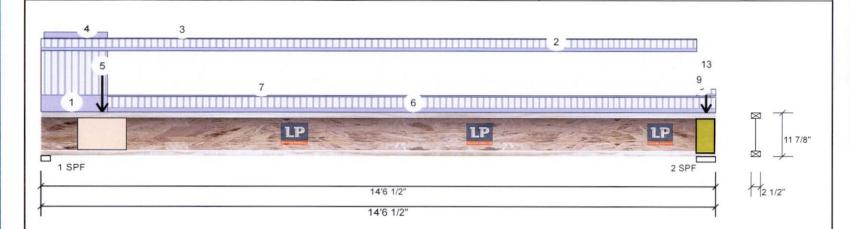
Job Name: LOT 9 (AMELIA 3 EL-2)

Project #

F15-C LPI 20Plus

11.875" - PASSED

Level: Ground Floor



Continued fr	rom page 1								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
6	Tie-In	1-4-2 to 14-6-8	(Span)0-9-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
7	Part. Uniform	1-4-2 to 14-5-1		Тор	2 PLF	0 PLF	0 PLF	0 PLF	
8	Tie-In	14-1-4 to 14-6-8	(Span)0-4-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
9	Part. Uniform	14-1-4 to 14-5-1		Тор	1 PLF	0 PLF	0 PLF	0 PLF	
10	Point	14-3-14		Тор	61 lb	155 lb	0 lb	0 lb	J1
	Bearing Length	0-1-8							
11	Point	14-3-14		Тор	4 lb	10 lb	0 lb	0 lb	J6
	Bearing Length	0-1-8							
12	Point	14-3-14		Тор	50 lb	134 lb	0 lb	0 lb	J6
	Bearing Length	0-1-8							
13	Point	14-3-14		Тор	43 lb	0 lb	0 lb	0 lb	Wall Self Weight
	Bearing Length	0-1-8							



Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

Copyright 2016 All rights reserved by Louisiana Pacific



READ ALL NOTES ON THIS PAGE AND ON THE

IN THE DESIGN OF THIS COMPONENT.

ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED

This design is valid until 10/31/2020

Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325 www.lpcorp.com CCMC: 12412-R APA: PR-L238C







Project:

Address

Date: 12/18/2018

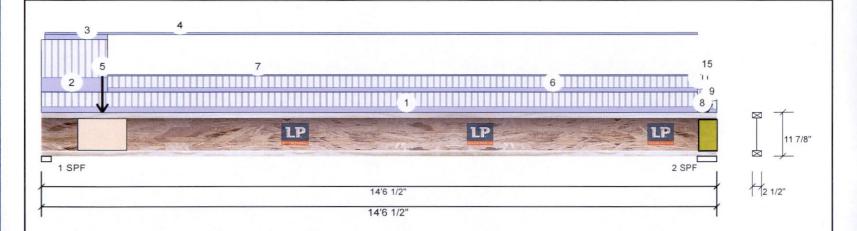
Designer: SB

Job Name: LOT 9 (AMELIA 3 EL-2)

Project #

LPI 20Plus 11.875" - PASSED F15-D

Level: Ground Floor



Member Information **Unfactored Reactions UNPATTERNED Ib (Uplift)** Type: Girder Application: Floor (Residential) Brg Live Dead Snow Wind Plies: Design Method: LSD 1 632 311 0 0 Moisture Condition: Dry **Building Code:** NBCC 2010 / OBC 2012 2 698 354 0 0 Deflection LL: Load Sharing: No 360 Deflection TL: 240 Deck: Not Checked Not Checked Importance: Normal Vibration: General Load **Bearings and Factored Reactions** Floor Live: 40 PSF Dead: 15 PSF Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1 - SPF 2.375" 82% 389 / 948 1336 L 1.25D+1.5L 2 - SPF 5.250" 81% 442 / 1048 1490 L 1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2576 ft-lb	6'6 5/16"	6250 ft-lb	0.412 (41%)	1.25D+1.5L	L
Shear	1313 lb	1 5/8"	2345 lb	0.560 (56%)	1.25D+1.5L	L
Perm Defl in.	0.079 (L/2119)	6'11 3/16"	0.468 (L/360)	0.170 (17%)	D	Uniform
LL Defl inch	0.161 (L/1047)	6'11 3/16"	0.468 (L/360)	0.340 (34%)	L	L
TI Defl inch	0.240 (L/701)	6'11 3/16"	0.702 (L/240)	0.340 (34%)	D+L	L

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Applied loads over end bearings and loads exceeding 250 lbs over intermediate bearings must be transferred directly to the support by rim board, blocking, squash blocks, or other device.
- 3 Dead Load Deflection: Instant = 0.079", Long Term = 0.119"
- 4 See manufacture installation guide note E4 for installation details
- 5 Girders are designed to be supported on the bottom edge only.
- 6 Top flange must be laterally braced at a maximum of 6'6" o.c.

7 Bottom flange braced at bearings.



January 02, 2019

Detail for ply to ply nailing or bolting

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind Comments
1	Tie-In	0-0-0 to 14-1-4	(Span)1-2-0 to 1-2-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF
2	Tie-In	0-0-0 to 1-4-2	(Span)3-0-0 to 3-0-0	Тор	15 PSF	40 PSF	0 PSF	PSF Pass-Thru Framing Squash Block is
3	Part. Uniform	0-0-14 to 1-4-2		Тор	8 PLF	0 PLF	0 PLF	required at all point loads over bearings
4	Part, Uniform	0-0-15 to 14-1-4		Тор	3 PLF	0 PLF	0 PLF	Porte to Multiple Member Connection

Continued on page 2...

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



This design is valid until 10/31/2020

Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325 www.lpcorp.com CCMC: 12412-R APA: PR-L238C

requirements







Client:

Project: Address Date: 12/18/2018

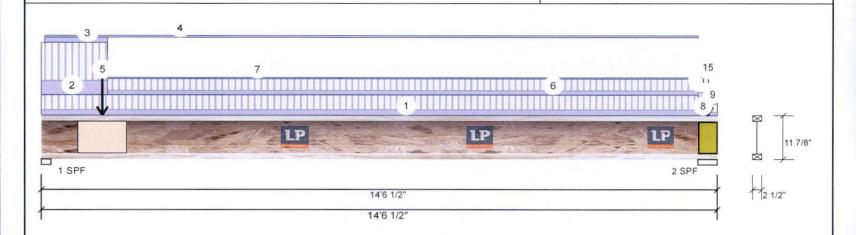
Designer: SB

Job Name: LOT 9 (AMELIA 3 EL-2)

Project #:

11.875" - PASSED F15-D LPI 20Plus

Level: Ground Floor



Continued from	page 1								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
5	Point	1-2-14		Near Face	148 lb	304 lb	0 lb	0 lb	F14
6	Tie-In	1-4-2 to 14-1-4	(Span)0-11-0 to 0-11-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
7	Part. Uniform	1-4-2 to 14-1-4		Тор	2 PLF	0 PLF	0 PLF	0 PLF	
8	Tie-In	14-1-4 to 14-6-8	(Span)0-8-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
9	Tie-In	14-1-4 to 14-6-8	(Span)0-8-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
10	Part. Uniform	14-1-4 to 14-5-5		Тор	2 PLF	0 PLF	0 PLF	0 PLF	
11	Part. Uniform	14-1-4 to 14-5-4		Тор	2 PLF	0 PLF	0 PLF	0 PLF	
12	Point	14-3-14		Тор	69 lb	185 lb	0 lb	0 lb	J1
	Bearing Length	0-1-8							
13	Point	14-3-14		Тор	41 lb	109 lb	0 lb	0 lb	J6
	Bearing Length	0-1-8							
14	Point	14-3-14		Тор	29 lb	77 lb	0 lb	0 lb	J6
	Bearing Length	0-1-8							
15	Point	14-3-14		Тор	53 lb	0 lb	0 lb	0 lb	Wall Self Weight
	Bearing Length	0-1-8							

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



Notes

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

Copyright 2016 All rights reserved by Louisiana Pacific Corp. 414 Union St Suite 2000, Nashville, TN 37219



This design is valid until 10/31/2020

Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325 www.lpcorp.com CCMC: 12412-R APA: PR-L238C







Project:

Address:

12/18/2018 Date:

Designer: SB

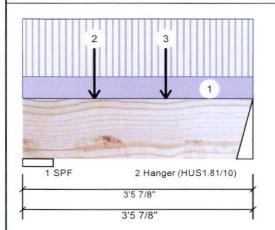
Job Name: LOT 9 (AMELIA 3 EL-2)

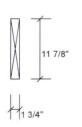
Project #:

Forex 2.0E-3000Fb LVL

1.750" X 11.875" - PASSED

Level: Ground Floor





Member Information

viember Information							
Girder							
1							
Dry							
360							
240							
Normal							
40 PSF							
15 PSF							

Application: Floor (Residential) Design Method: LSD

> Building Code: Load Sharing: Deck:

Not Checked Vibration: Not Checked

No

NBCC 2010 / OBC 2012

Unfactored Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	Wind
1	360	170	0	0
2	259	122	0	0

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	5.500"	13%	213 / 540	753	L	1.25D+1.5L
2 - Hanger	3.000"	14%	152 / 389	541	L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	485 ft-lb	1'11 1/4"	17130 ft-lb	0.028 (3%)	1.25D+1.5L	L
Unbraced	485 ft-lb	1'11 1/4"	13987 ft-lb	0.035 (3%)	1.25D+1.5L	L
Shear	526 lb	1'4 5/8"	5798 lb	0.091 (9%)	1.25D+1.5L	L
Perm Defl in.	0.001 (L/36212)	1'10 7/16"	0.097 (L/360)	0.010 (1%)	D	Uniform
LL Defl inch	0.002 (L/17367)	1'10 5/16"	0.097 (L/360)	0.020 (2%)	L	L
TL Defl inch	0.003 (L/11738)	1'10 5/16"	0.145 (L/240)	0.020 (2%)	D+L	L



January 02, 2019

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.

Load Type

- 3 Top braced at bearings.
- 4 Bottom braced at bearings.

1		Part. Uniform	0-0-0 to 3-5-14	Тор	30 PLF
2		Point	1-0-12	Far Face	102 lb
3	3 Point		2-1-12	Far Face	69 lb
		Self Weight			5 PLF

Location

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Side

Trib Width

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Comments

ID

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- andling & Installation
 LVI beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
 approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

- 6. For flat roofs provide proper drainage to prevent

Dead

Live

80 PLF

206 lb

134 lb

Manufacturer Info

Snow

OPIF

0 lb

0 lb

Wind

OPIF

0 lb .14

0 lb

APA: PR-L318

Kott Lumber Company 14 Anderson Blvd, Ontario Canada L4A 7X4 905-642-4400







Client:

Project: Address: Date: 12/18/2018

Designer: SB

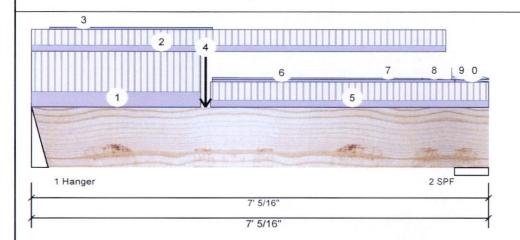
Job Name: LOT 9 (AMELIA 3 EL-2)

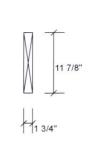
Project #

Forex 2.0E-3000Fb LVL

1.750" X 11.875" - PASSED

Level: Ground Floor





Wind

Member Information

Type:	Girder
Plies:	1
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	
Floor Live:	40 PSF
Dead:	15 PSF

Application: Floor (Residential) Design Method: LSD

Building Code: Load Sharing: Deck:

No Not Checked Vibration: Not Checked

NBCC 2010 / OBC 2012

Unfactored Reactions UNPATTERNED Ib (Uplift)

	1	410	199	0	0
	2	295	156	0	0
ı					

Dead

Bearings and Factored Reactions

Live

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.	
1 - Hanger	3.000"	22%	248 / 615	864	L	1.25D+1.5L	
2 - SPF	6.438"	9%	195 / 442	637	L	1.25D+1.5L	

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1478 ft-lb	2'7 7/16"	17130 ft-lb	0.086 (9%)	1.25D+1.5L	L
Unbraced	1478 ft-lb	2'7 7/16"	7067 ft-lb	0.209 (21%)	1.25D+1.5L	L
Shear	649 lb	1'2 1/8"	5798 lb	0.112 (11%)	1.25D+1.5L	L
Perm Defl in.	0.006 (L/11753)	3' 11/16"	0.212 (L/360)	0.030 (3%)	D	Uniform
LL Defl inch	0.013 (L/5910)	3' 1/8"	0.212 (L/360)	0.060 (6%)	L	L
TL Defl inch	0.019 (L/3933)	3' 5/16"	0.318 (L/240)	0.060 (6%)	D+L	L

195 / 442 637 L 1.25D+1.5L

Snow

I.MATIJEVIC 100528832

NOVINCE OF ONTRE

January 02, 2019

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

4 Bottom b	raced at bearings.								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	(
1	Tie-In	0-0-0 to 2-6-9	(Span)3-1-13	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 6-4-2	(Span)1-3-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Part. Uniform	0-3-4 to 2-8-10		Тор	3 PLF	0 PLF	0 PLF	0 PLF	
4	Point	2-7-7		Far Face	122 lb	259 lb	0 lb	0 lb	F6
5	Tie-In	2-8-5 to 7-0-5	(Span)1-5-0	Тор	15 PSF	40 PSF	0 PSF	PassSFh	ru Fra

Top

Continued on page 2...

PassSFhru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

6

7

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Part. Uniform

Tapered Start

Dry service conditions, unless noted otherwise LVL not to be treated with fire retardant or corrosive

Handling & Installation

2-8-10 to 6-5-14

2-8-10

- andling & Installation
 LVI beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
 approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent

4 PLF

3 PLF

0 PLF

0 PLF

Manufacturer Info

Forex APA: PR-L318

0 PLF

0 PLF

Kott Lumber Company 14 Anderson Blvd, Ontario Canada L4A 7X4 905-642-4400







Client: Project:

Address:

Date: 12/18/2018

Designer: SB

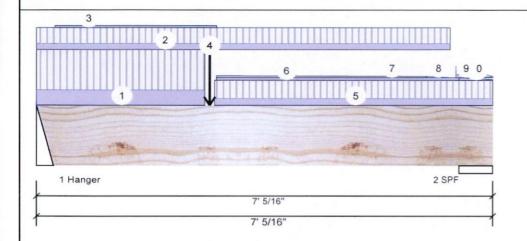
Job Name: LOT 9 (AMELIA 3 EL-2)

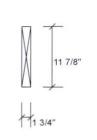
Project #

Forex 2.0E-3000Fb LVL

1.750" X 11.875" - PASSED

Level: Ground Floor





Continued fr	rom page 1								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
	End	5-11-10			2 PLF	0 PLF	0 PLF	0 PLF	
8	Tapered Start	5-11-10		Тор	2 PLF	0 PLF	0 PLF	0 PLF	
	End	6-4-2			0 PLF	0 PLF	0 PLF	0 PLF	
9	Tie-In	6-5-4 to 7-0-5	(Span)0-7-13 to 0-0-13	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
10	Tapered Start	6-5-14		Тор	4 PLF	0 PLF	0 PLF	0 PLF	
	End	7-0-5			1 PLF	0 PLF	0 PLF	0 PLF	
	Self Weight				5 PLF				



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled Refer to manufacturer's product informati regarding installation requirements, multi-fastening details, beam strength values, and co Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals Damaged Beams must not be used Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex

APA: PR-L318

Kott Lumber Company 14 Anderson Blvd, Ontario Canada L4A 7X4 905-642-4400







Client:

Project: Address Date: 12/18/2018

Designer: SB

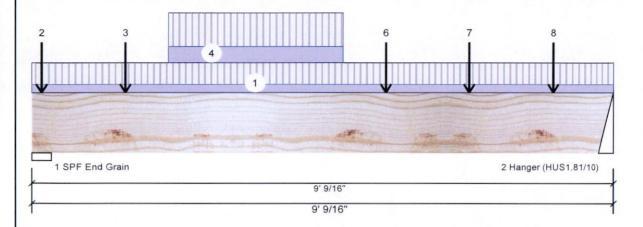
Job Name: LOT 9 (AMELIA 3 EL-2)

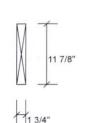
Project #

Forex 2.0E-3000Fb LVL

1.750" X 11.875" - PASSED

Level: Ground Floor





Member Information

Girder Type Plies: 1 Moisture Condition: Dry Deflection LL: 360 Deflection TI 240 Importance: Normal General Load Floor Live: 40 PSF Dead: 15 PSF Application: Floor (Residential) Design Method: LSD

Building Code: NBCC 2010 / OBC 2012 Load Sharing: No

Deck: Not Checked Vibration: Not Checked Unfactored Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	Wind
1	1200	535	0	0
2	1290	544	0	0

Analysis Results

Analysis Actual Location Allowed Capacity Comb. Case Moment 5061 ft-lb 5'4 3/4" 17130 ft-lb 0.295 (30%) 1.25D+1.5L L 5061 ft-lb Unbraced 5'4 3/4" 5210 ft-lb 0.971 (97%) 1.25D+1.5L L Shear 2425 lb 7'10 7/16" 5798 lb 0.418 (42%) 1.25D+1.5L L Perm Defl in. 0.035 (L/2951) 4'8 1/8" 0.288 (L/360) 0.120 (12%) D Uniform LL Defl inch 0.081 (L/1283) 4'8 9/16" 0.288 (L/360) 0.280 (28%) L TL Defl inch 0.116 (L/894) 4'8 7/16" 0.432 (L/240) 0.270 (27%) D+L

Bearings and Factored Reactions

Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1 - SPF 3.500" 54% 668 / 1800 2468 | 1 25D+1 5I End Grain 2 -3.000" 680 / 1936 1 25D+1 5L 67% 2616 L

Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Fill all hanger nailing holes.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top braced at bearings.

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT

Hanger

5 Bottom	braced at bearings.		IIN	THE DESIGN	OF THIS COMP	ONENI.		
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	
1	Tie-In	0-0-0 to 9-0-9	(Span)3-11-7 to 3-11-7	Тор	15 PSF	40 PSF	0 PSF	
2	Point	0-1-12		Тор	110 lb	248 lb	0 lb	
3	Point	1-4-12		Far Face	77 lb	158 lb	0 lb	
4	Part. Uniform	2-0-12 to 4-8-12		Far Face	59 PLF	123 PLF	0 PLF	
6	Point	5-4-12		Far Face	126 lb	296 lb	0 lb	
7	Point	6-8-12		Far Face	156 lb	393 lb	0 lb	
Continued o	n page 2							



January 02, 2019

0 lb J10 Pass Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- Andling & Installation
 LVL beams must not be out or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-opt
 fastening details, beam strength values, and code
 approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

- 6. For flat roofs provide proper drainage to prevent

Manufacturer Info

APA: PR-L318

Kott Lumber Company 14 Anderson Blvd, Ontario Canada L4A 7X4 905-642-4400



Wind

0 PSF

0 lb C4







Client

Project: Address: Date: 12/18/2018

Designer: SB

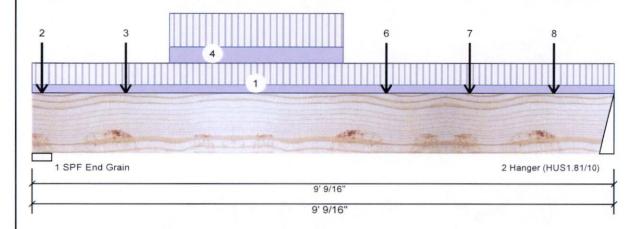
Job Name: LOT 9 (AMELIA 3 EL-2)

Project #

Forex 2.0E-3000Fb LVL

1.750" X 11.875" - PASSED

Level: Ground Floor



.Continued from page 1

ID Load Type 8 Point

Self Weight

Location

Trib Width 8-0-12

Side Far Face Dead 141 lb

5 PLF

Live 352 lb Wind

Snow

0 lb

Comments 0 lb J10

> PROFESSIONAL ENGINEER I.MATUEVIC 100528832 NOVINCE OF ONTHE January 02, 2019

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals Handling & Installation

- andling & Installation

 LVL beams must not be cut or drilled

 Refer to manufacturer's product information regarding installation requirements, multi-pby fastening details, beam strength values, and code approvals

 Damaged Beams must not be used

 Design assumes top edge is laterally restrained

 Provide lateral support at bearing points to avoid lateral displacement and rotation

- For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318

Kott Lumber Company 14 Anderson Blvd, Ontario Canada L4A 7X4 905-642-4400







Project: Address: 12/18/2018

Designer: SB

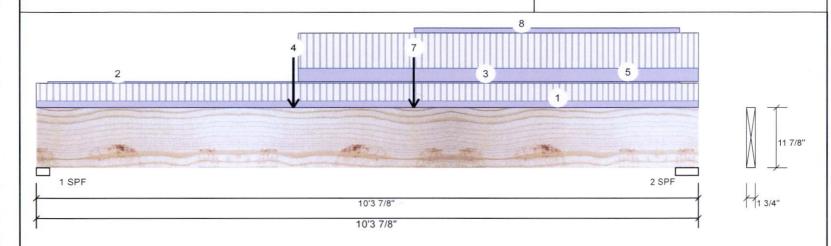
Job Name: LOT 9 (AMELIA 3 EL-2)

Project #:

Forex 2.0E-3000Fb LVL

1.750" X 11.875" - PASSED

Level: Ground Floor



Member Information

Type: Girder Plies 1 Moisture Condition: Dry Deflection LL: 360 Deflection TL: 240 Importance: Normal General Load Floor Live: 40 PSF

15 PSF

Application: Design Method: **Building Code:**

Load Sharing:

Deck:

Floor (Residential) LSD NBCC 2010 / OBC 2012 No

Not Checked Vibration: Not Checked

Unfactored Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	Wind
1	949	425	0	0
2	722	334	0	0

Bearings and Factored Reactions

Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1 - SPF 2.375" 76% 531 / 1424 1955 L 1.25D+1.5L 2 - SPF 4.500" 31% 417 / 1083 1501 1.25D+1.5L

Analysis Results

Dead:

Analysis Actual Location Allowed Capacity Comb. Case Moment 7161 ft-lb 3'10 7/8" 17130 ft-lb 0.418 (42%) 1.25D+1.5L L 0.995 1.25D+1.5L L 7161 ft-lb 3'10 7/8" 7194 ft-lb Unbraced (100%)0.332 (33%) 1.25D+1.5L L Shear 1924 lb 1'1 1/2" 5798 lb Perm Defl in. 0.054 (L/2188) 4'7 7/8" 0.329 (L/360) 0.160 (16%) D Uniform LL Defl inch 0.123 (L/965) 4'7 11/16" 0.329 (L/360) 0.370 (37%) L L TL Defl inch 0.177 (L/670) 4'7 3/4" 0.494 (L/240) 0.360 (36%) D+L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only
- 2 Top must be laterally braced at a maximum of 6'3" o.c.

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT

3 Bottom	braced at bearings.		113	THE DESIGN	OI IIIIO COMI	OILLIVI.	
ID	Load Type	Location	Trib Width	Side	Dead	Live	
1	Tie-In	0-0-0 to 10-3-14	(Span)0-6-5	Тор	15 PSF	40 PSF	
2	Part. Uniform	0-2-1 to 2-3-1		Тор	1 PLF	0 PLF	
3	Part. Uniform	2-3-1 to 10-0-4		Тор	1 PLF	0 PLF	
4	Point	3-10-14		Far Face	544 lb	1290 lb	
5	Tie-In	3-11-12 to 10-3-14	(Span)1-0-11	Тор	15 PSF	40 PSF	
6	Point	5-9-3		Тор	48 lb	127 lb	
7	Point	5-9-3		Тор	4 lb	12 lb	
8	Part. Uniform	5-9-5 to 10-0-3		Тор	3 PLF	0 PLF	
	Self Weight				5 PLF		

0 PLF 0 PLF 0 lb 0 lb O PSFPass-ThirdFFraming Squash Block is

Wind

0 PSF

0 PLF

^{0 lb}Refer to Multiple Member Connection O PLFDetail for usly to ply nailing or bolting requirements

0 lbrequired at all point loads over bearings

Calculated Structured Designs is responsible only of the Structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or conditions.
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product informati-regarding installation requirements, multi-fastening details, beam strength values, and co-
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent

Manufacturer Info

APA: PR-L318

Snow

0 PSF

0 PLF

Kott Lumber Company 14 Anderson Blvd, Ontario Canada L4A 7X4 905-642-4400

PROFESSIONAL

I.MATIJEVIC

100528832

SHOVINCE OF

January 02, 2019

ONTARIO







Project: Address: Date: 12/18/2018

SB Designer:

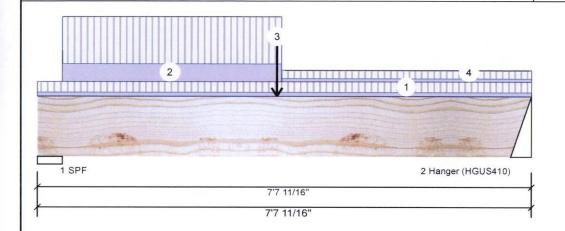
Job Name: LOT 9 (AMELIA 3 EL-2)

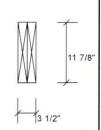
Project #:

Forex 2.0E-3000Fb LVL

1.750" X 11.875"

2-Ply - PASSED Level: Second Floor





Member Information

Type:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	
Floor Live:	40 PSF
Dead:	15 PSF

Application: Floor (Residential) Design Method: LSD **Building Code:** NBCC 2010 / OBC 2012

Not Checked

Load Sharing: No Not Checked

Deck: Vibration: **Unfactored Reactions UNPATTERNED Ib (Uplift)**

	Brg	Live	Dead	Snow	Wind
	1	489	224	0	0
2	2	372	179	0	0

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	4.467"	11%	280 / 733	1013	L	1.25D+1.5L
2 - Hanger	4.000"	8%	224 / 558	782	L	1.25D+1.5L

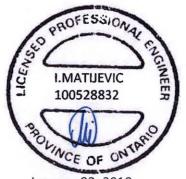
Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2402 ft-lb	3'7 11/16"	34261 ft-lb	0.070 (7%)	1.25D+1.5L	L
Unbraced	2402 ft-lb	3'7 11/16"	31940 ft-lb	0.075 (8%)	1.25D+1.5L	L
Shear	838 lb	1'3 9/16"	11596 lb	0.072 (7%)	1.25D+1.5L	L
Perm Defl in.	0.006 (L/15375)	3'7 3/4"	0.235 (L/360)	0.020 (2%)	D	Uniform
LL Defl inch	0.012 (L/6820)	3'7 3/4"	0.235 (L/360)	0.050 (5%)	L	L
TL Defl inch	0.018 (L/4725)	3'7 3/4"	0.353 (L/240)	0.050 (5%)	D+L	L



- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- Lateral slenderness ratio based on full section width

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



January 02, 2019

L	/ Lateral Sie	enderness ratio based to	on full section width.					
ľ	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow
l	1	Tie-In	0-0-0 to 7-7-11	(Span)0-9-0	Тор	15 PSF	40 PSF	0 PSF
١	2	Tie-In	0-4-7 to 3-8-9	(Span)3-4-0	Тор	15 PSF	40 PSF	0 PSF
l	3	Point	3-7-11		Near Face	187 lb	477 lb	0 lb
ı	4	Tie-In	3-8-9 to 7-7-11	(Span)0-7-0	Тор	15 PSF	40 PSF	0 PSF
l		Self Weight				10 PLF		

Pass-Thru Framing Squash Block is required at all point loads over bearings

Comments

0 PSF

Wind

0 PSF

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Handling & Installation

- andling & Installation
 LVI beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
 approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent

Manufacturer Info

APA: PR-L318

14 Anderson Blvd, Ontario Canada L4A 7X4 905-642-4400





Kott Lumber Company



Client: Project: Address: Date: 12/18/2018

Designer: SB

Job Name: LOT 9 (AMELIA 3 EL-2)

Project #:

Forex 2.0E-3000Fb LVL F11-B

1.750" X 11.875"

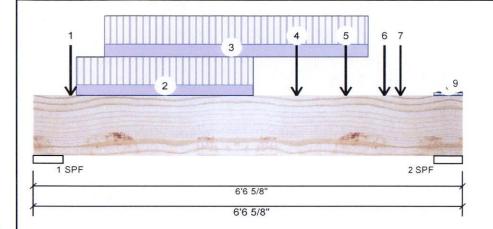
2-Ply - PASSED Level: Second Floor

Brg

2

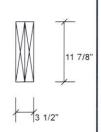
1 - SPF 5.250"

2 - SPF 5.500"



Deck:

Vibration:



Wind

0

0

1.25D+1.5L

1.25D+1.5L

Member Information

Туре:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	
Floor Live:	40 PSF
Dead:	15 PSF

Application: Floor (Residential) LSD Design Method: NBCC 2010 / OBC 2012 **Building Code:** Load Sharing: No

Not Checked Not Checked

Unfactored Reactions UNPATTERNED Ib (Uplift)

30%

26%

Dead

720

667

Live

1641

1474

Bearings and Fac	tored Reactions		
Bearing Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.

900 / 2461

834 / 2211

Wind

0 lb

0 PLF

Analysis Results

	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
	Moment	4871 ft-lb	3'3 5/8"	34261 ft-lb	0.142 (14%)	1.25D+1.5L	L
	Unbraced	4871 ft-lb	3'3 5/8"	32706 ft-lb	0.149 (15%)	1.25D+1.5L	L
	Shear	3590 lb	5'2"	11596 lb	0.310 (31%)	1.25D+1.5L	L
	Perm Defl in.	0.009 (L/7417)	3'3 5/8"	0.193 (L/360)	0.050 (5%)	D	Uniform
	LL Defl inch	0.021 (L/3252)	3'3 7/16"	0.193 (L/360)	0.110 (11%)	L	L
	TL Defl inch	0.031 (L/2261)	3'3 1/2"	0.289 (L/240)	0.110 (11%)	D+L	L
-							

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



Snow

3361 L

3044 L

0

January 02, 2019

6 Lateral	slenderness ratio based	on full section width.		Reservation				
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	
1	Point	0-6-10		Far Face	116 lb	256 lb	0 lb	
2	Part. Uniform	0-7-10 to 3-3-10		Near Face	105 PLF	280 PLF	0 PLF	
3	Part. Uniform	1-0-10 to 5-0-10		Far Face	127 PLF	286 PLF	0 PLF	
4	Point	3-11-10		Near Face	111 lb	296 lb	0 lb	
5	Point	4-8-10		Near Face	179 lb	372 lb	0 lb	
6	Point	5-3-10		Near Face	25 lb	67 lb	0 lb	1

0 PLF Pass-Thru Framing Squash Block is required at all point loads over bearings 0 lb F11

Comments

J1

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Continued on page 2...

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

Andling & Installation
LVL beams must not be cut or drilled
Refer to manufacturer's product information
regarding installation requirements, multi-ply
fastening details, beam strength values, and code
approvals
Damaged Beams must not be used
Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent

Manufacturer Info

APA: PR-L318

Kott Lumber Company 14 Anderson Blvd, Ontario Canada L4A 7X4 905-642-4400







Client: Project:

Address:

12/18/2018 Date: Designer: SB

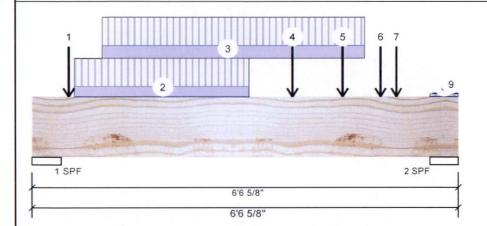
Job Name: LOT 9 (AMELIA 3 EL-2)

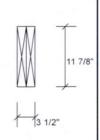
Project #:

Forex 2.0E-3000Fb LVL F11-B

1.750" X 11.875"

2-Ply - PASSED Level: Second Floor





Continued fro	om page 1	ı
---------------	-----------	---

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
7	Point	5-6-10		Far Face	101 lb	221 lb	0 lb	0 lb	J1
8	Tie-In	6-1-2 to 6-6-10	(Span)1-0-1	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
9	Tie-In	6-1-11 to 6-6-10	(Span)0-3-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	Self Weight				10 PLF				



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- andling & Installation
 LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
 approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

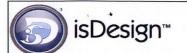
Manufacturer Info

APA: PR-L318

Kott Lumber Company 14 Anderson Blvd, Ontario Canada L4A 7X4 905-642-4400







Project: Address: Date: 12/18/2018

Designer: S B

Job Name: LOT 9 (AMELIA 3 EL-2)

Project #:

Forex 2.0E-3000Fb LVL

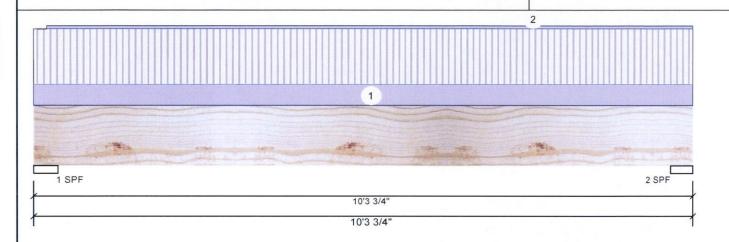
1.750" X 11.875"

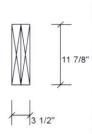
2-Ply - PASSED Level: Second Floor

Brg

1

2





Wind

0

0

Member Information

Γ	Туре:	Girder	Application:	Floor (Residential)
	Plies:	2	Design Method:	LSD
	Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
	Deflection LL:	360	Load Sharing:	No
	Deflection TL:	240	Deck:	Not Checked
	Importance:	Normal	Vibration:	Not Checked
	General Load			
	Floor Live:	40 PSF		
	Dead:	15 PSF		

Unfactored Reactions UNPATTERNED Ib (Uplift)

Live

97

97

Dead

91

91

Bearings	Bearings and Factored Reactions								
Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.			
1 - SPF	4.375"	3%	113 / 146	259	L	1.25D+1.5L			
2 - SPF	4.375"	3%	113 / 146	259	L	1.25D+1.5L			

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	592 ft-lb	5'1 7/8"	34261 ft-lb	0.017 (2%)	1.25D+1.5L	L
Unbraced	592 ft-lb	5'1 7/8"	29876 ft-lb	0.020 (2%)	1.25D+1.5L	L
Shear	194 lb	9' 1/4"	11596 lb	0.017 (2%)	1.25D+1.5L	L
Perm Defl in.	0.004 (L/27908)	5'1 7/8"	0.324 (L/360)	0.010 (1%)	D	Uniform
LL Defl inch	0.004 (L/26047)	5'1 7/8"	0.324 (L/360)	0.010 (1%)	L	L
TL Defl inch	0.009 (L/13473)	5'1 7/8"	0.485 (L/240)	0.020 (2%)	D+L	L

Design Notes

- Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on full section width.

Load Type

l				ONTE			requi		
	Self Weight			1/1/1	10 PLF			Pass	
2	Part. Uniform	0-2-5 to 10-3-12		Тор	1 PLF	0 PLF	0 PLF	0 F	
1	Tie-In	0-0-0 to 10-3-12	(Span)0-11-5	Тор	15 PSF	40 PSF	0 PSF	0 F	

Side

Trib Width

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Location



Snow

0

0

January 02, 2019

0 PLF s-Thru Framing Squash Block is uired at all point loads over bearings

Comments

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

ID

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Dead

Live

Manufacturer Info

Snow

APA: PR-L318

Kott Lumber Company 14 Anderson Blvd, Ontario Canada L4A 7X4 905-642-4400

Wind

PSF



0

0

1.25D+1.5L

1.25D+1.5L

Page 1 of 2



Project: Address: Date: 12/18/2018

Designer: SB

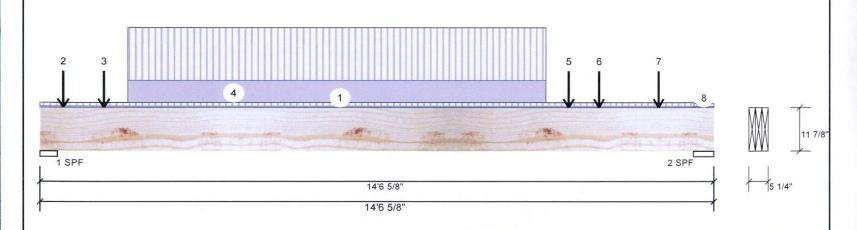
Job Name: LOT 9 (AMELIA 3 EL-2)

Project #:

Forex 2.0E-3000Fb LVL

1.750" X 11.875"

3-Ply - PASSED Level: Second Floor



Member Information **Unfactored Reactions UNPATTERNED Ib (Uplift)** Application: Floor (Residential) Type Brg Live Dead Snow Wind Plies: 3 Design Method: LSD 2085 972 0 1 Moisture Condition: Dry **Building Code:** NBCC 2010 / OBC 2012 0 2 2008 911 Deflection LL: 360 Load Sharing: Yes Deflection TL: 240 Deck: Not Checked Importance: Normal Vibration: Not Checked General Load **Bearings and Factored Reactions** Floor Live: 40 PSF 15 PSF Dead: Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb.

Analysis Results

lysis	Actual	Location	Allowed	Capacity	Comb.	Case
ment	14769 ft-lb	7'2 13/16"	53447 ft-lb	0.276 (28%)	1.25D+1.5L	L
raced	14769 ft-lb	7'2 13/16"	50470 ft-lb	0.293 (29%)	1.25D+1.5L	L
ar	4273 lb	1'3 1/2"	17394 lb	0.246 (25%)	1.25D+1.5L	L
m Defl in.	0.083 (L/1993)	7'2 5/8"	0.462 (L/360)	0.180 (18%)	D	Uniform
Defl inch	0.181 (L/920)	7'2 7/8"	0.462 (L/360)	0.390 (39%)	L	L
Defl inch	0.264 (L/629)	7'2 13/16"	0.693 (L/240)	0.380 (38%)	D+L	L
		ment 14769 ft-lb braced 14769 ft-lb ear 4273 lb m Defl in. 0.083 (L/1993) Defl inch 0.181 (L/920)	ment 14769 ft-lb 7'2 13/16" praced 14769 ft-lb 7'2 13/16" praced 14769 ft-lb 7'2 13/16" praced 14769 ft-lb 7'2 13/16" T'2 13/16"	ment 14769 ft-lb 7'2 13/16" 53447 ft-lb praced 14769 ft-lb 7'2 13/16" 50470 ft-lb ear 4273 lb 1'3 1/2" 17394 lb m Defl in. 0.083 (L/1993) 7'2 5/8" 0.462 (L/360) Defl inch 0.181 (L/920) 7'2 7/8" 0.462 (L/360)	ment 14769 ft-lb 7'2 13/16" 53447 ft-lb 0.276 (28%) praced 14769 ft-lb 7'2 13/16" 50470 ft-lb 0.293 (29%) par 4273 lb 1'3 1/2" 17394 lb 0.246 (25%) pm Defl in. 0.083 (L/1993) 7'2 5/8" 0.462 (L/360) 0.180 (18%) Defl inch 0.181 (L/920) 7'2 7/8" 0.462 (L/360) 0.390 (39%)	ment 14769 ft-lb 7'2 13/16" 53447 ft-lb 0.276 (28%) 1.25D+1.5L braced 14769 ft-lb 7'2 13/16" 50470 ft-lb 0.293 (29%) 1.25D+1.5L ear 4273 lb 1'3 1/2" 17394 lb 0.246 (25%) 1.25D+1.5L m Defl in. 0.083 (L/1993) 7'2 5/8" 0.462 (L/360) 0.180 (18%) D Defl inch 0.181 (L/920) 7'2 7/8" 0.462 (L/360) 0.390 (39%) L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

1 - SPF 4.375"

2 - SPF 5.500"

31%

23%

1215 / 3127

1139 / 3012

Wind

0 PSF 0 lb J1

0 lb



4342 L

4151 I

January 02, 2019

Comments

6 Lateral	slenderness ratio based	on full section width.		Book strategy				_
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	
1	Tie-In	0-0-0 to 14-1-4	(Span)0-11-0	Тор	15 PSF	40 PSF	0 PSF	
2	Point	0-5-13		Far Face	81 lb	190 lb	0 lb	
3	Point	1-3-13		Far Face	108 lb	255 lb	0 lb	
4	Part. Uniform	1-9-13 to 10-9-13		Far Face	117 PLF	278 PLF	0 PLF	
5	Point	11-3-13		Far Face	91 lb	232 lb	0 lb	
6	Point	11-11-13		Far Face	104 lb	278 lb	0 lb	
ontinued or	n page 2							

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVIL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals Damaged Beams must not be used

- Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent

Manufacturer Info

APA: PR-L318

Kott Lumber Company 14 Anderson Blvd, Ontario Canada L4A 7X4 905-642-4400







Client:

Project: Address: Date: 12/18/2018

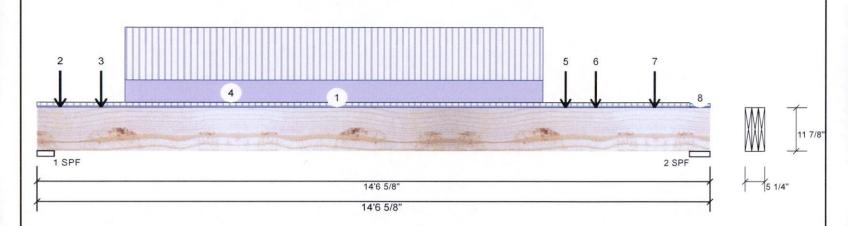
Designer: SB

Job Name: LOT 9 (AMELIA 3 EL-2)

Project #:

F13-A Forex 2.0E-3000Fb LVL 1.750" X 11.875"

3-Ply - PASSED Level: Second Floor



.Continued from page 1

Trib Width ID Load Type Location Side Dead Live Snow Wind Comments 7 Point 13-3-13 Far Face 139 lb 371 lb 0 lb 0 lb J1 8 Tie-In 14-1-4 to 14-6-10 (Span)0-8-8 15 PSF 40 PSF 0 PSF 0 PSF Top Self Weight 14 PLF



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise LVL not to be treated with fire retardant or corrosive

Handling & Installation

- andling & Installation
 LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
 approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent

Manufacturer Info

APA: PR-L318

Kott Lumber Company 14 Anderson Blvd. Ontario Canada L4A 7X4 905-642-4400







Client:

Project: Address Date: 12/18/2018

Designer: SB

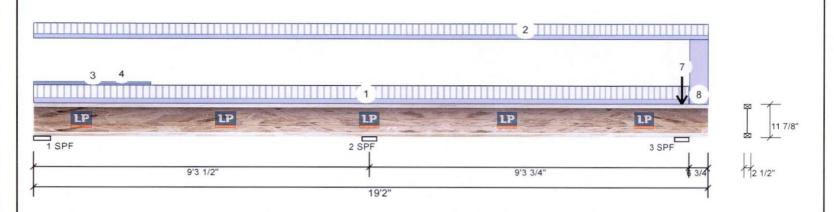
Job Name: LOT 9 (AMELIA 3 EL-2)

Project #:

F16-A LPI 20Plus

11.875" - PASSED

Level: Second Floor



Member Information Type Girder Floor (Residential) Application: Plies LSD 1 Design Method: Moisture Condition: Dry Building Code: NBCC 2010 / OBC 2012 Deflection LL: 360 Load Sharing: No Deflection TL: 240 Deck: Not Checked Importance: Normal Vibration: Not Checked General Load 40 PSF Floor Live: Dead: 15 PSF

Ana	lysis	Resu	ts

Ī	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
	Neg Moment	-578 ft-lb	9'3 1/2"	6250 ft-lb	0.092 (9%)	1.25D+1.5L	LL_
	Pos Moment	433 ft-lb	4'1 11/16"	5500 ft-lb	0.079 (8%)	1.25D+1.5L	L
	Shear	656 lb	18'7 1/4"	1853 lb	0.354 (35%)	1.25D+1.5S	_L
	Perm Defl in.	0.005 (L/21091)	4'5 1/4"	0.297 (L/360)	0.020 (2%)	D	Uniform
	LL Defl inch	0.015 (L/6927)	4'8 1/2"	0.297 (L/360)	0.050 (5%)	L+0.5S	L_L
	TL Defl inch	0.020 (L/5219)	4'7 11/16"	0.445 (L/240)	0.050 (5%)	D+L+0.5S	L_L
	LL Cant	-0.002 (2L/5898)	Rt Cant	0.200 (2L/480)	0.011 (1%)	L	_L_
	TL Cant	0.002 (2L/5508)	Rt Cant	0.300 (2L/360)	0.008 (1%)	D+L+0.5S	L_L

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Applied loads over end bearings and loads exceeding 250 lbs over intermediate bearings must be transferred directly to the support by rim board, blocking, squash blocks, or other device.
- 3 Dead Load Deflection: Instant = 0.005", Long Term = 0.008"
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top flange braced at bearings.
- 6 Bottom flange braced at bearings.

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Unfactored Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	Wind	
1	116	55	1	0	
2	322	118	0 (-4)	0	
3	331	337	201	0	
1					

Bearings and Factored Reactions

Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	5.500"	14%	67 / 193	260	L	1.25D+1.5L
2 - SPF	5.000"	16%	150 / 502	652	LL_	1.25D+1.5L
3 - SPF	5.000"	34%	421 / 617	1038	_LL	1.25D+1.5L +0.5S



January 02, 2019

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

Copyright 2016 All rights reserved by Louisiana Pacif



This design is valid until 10/31/2020

Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325 www.lpcorp.com CCMC: 12412-R APA: PR-L238C







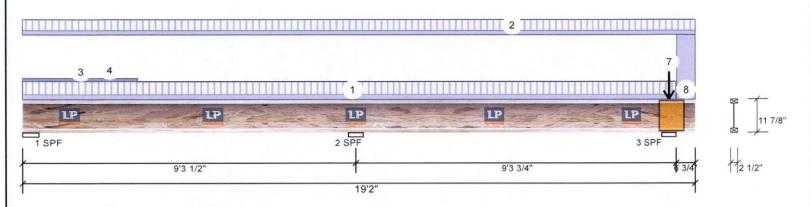
Client: Project: Address: Date: Designer: SB

12/18/2018

Job Name: LOT 9 (AMELIA 3 EL-2)

11.875" - PASSED F16-A LPI 20Plus

Level: Second Floor



ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments	
1	Tie-In	0-0-0 to 18-7-3	(Span)0-9-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
2	Tie-In	0-0-0 to 19-2-0	(Span)0-8-5	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
3	Part. Uniform	0-0-0 to 3-2-5		Тор	2 PLF	0 PLF	0 PLF	0 PLF		
4	Part. Uniform	0-0-0 to 3-2-6		Тор	2 PLF	0 PLF	0 PLF	0 PLF		
5	Point	18-4-12		Тор	199 lb	204 lb	197 lb	0 lb	F2 F2	
	Bearing Length	0-1-8								
6	Point	18-4-12		Тор	7 lb	0 lb	0 lb	0 lb	Wall Self Weight	
	Bearing Length	0-1-8								
7	Point	18-4-12		Тор	36 lb	0 lb	0 lb	0 lb	Wall Self Weight	
	Bearing Length	0-1-8								
8	Part. Uniform	18-7-8 to 19-2-0		Тор	80 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight	



January 02, 2019

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT

IN THE DESIGN OF THIS COMPONENT.

CONTAINS SPECIFICATIONS AND CRITERIA USED

This design is valid until 10/31/2020

Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325 www.lpcorp.com CCMC: 12412-R APA: PR-L238C







Client: Project: Address:

Date: 12/18/2018

Designer: SB

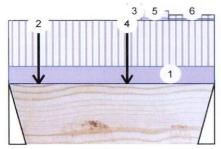
Job Name: LOT 9 (AMELIA 3 EL-2)

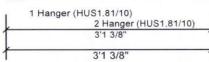
Project #

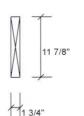
Forex 2.0E-3000Fb LVL

1.750" X 11.875" - PASSED

Level: Second Floor







Wind

Member Information

Type:	Girder
Plies:	1
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	
Floor Live:	40 PSF
Dead:	15 PSF

Floor (Residential) Application: Design Method: LSD

Building Code: NBCC 2010 / OBC 2012 Load Sharing: No

Not Checked Not Checked

Unfactored Reactions UNPATTERNED Ib (Uplift) Brg Live Dead Snow

1	4//	187	0	U
2	449	176	0	0

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	568 ft-lb	1'8 1/8"	17130 ft-lb	0.033 (3%)	1.25D+1.5L	L
Unbraced	568 ft-lb	1'8 1/8"	14337 ft-lb	0.040 (4%)	1.25D+1.5L	L
Shear	386 lb	1'2 1/8"	5798 lb	0.067 (7%)	1.25D+1.5L	L
Perm Defl in.	0.001 (L/35583)	1'7 9/16"	0.091 (L/360)	0.010 (1%)	D	Uniform
LL Defl inch	0.002 (L/13914)	1'7 9/16"	0.091 (L/360)	0.030 (3%)	L	L
TL Defl inch	0.003 (L/10003)	1'7 9/16"	0.137 (L/240)	0.020 (2%)	D+L	L

Deck:

Vibration:

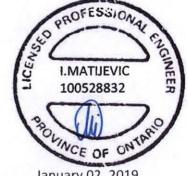
Bearings and Factored Reactions

-							
Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.	
1 - Hanger	3.000"	24%	233 / 716	949	L	1.25D+1.5L	
2 - Hanger	3.000"	23%	220 / 674	894	L	1.25D+1.5L	

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.
- 4 Rottom braced at hearings

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



January 02, 2019 Comments

4 DOLLOTTI DI acc	ed at bearings.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	
1	Part. Uniform	0-0-0 to 3-1-6		Тор	90 PLF	240 PLF	0 PLF	
2	Point	0-5-4		Far Face	28 lb	74 lb	0 lb	
3	Tie-In	1-9-4 to 1-11-14	(Span)3-1-11 to 3-1-11	Тор	15 PSF	40 PSF	0 PSF	
4	Point	1-9-4		Far Face	23 lb	61 lb	0 lb	
5	Tie-In	1-11-14 to 2-5-0	(Span)1-9-11	Тор	15 PSF	40 PSF	0 PSF	
6	Tie-In	2-5-0 to 3-1-6	(Span)1-0-11	Тор	15 PSF	40 PSF	0 PSF	
	Self Weight				5 PLF			

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection OPPRail for ply to ply nailing or bolting requirements

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- andling & Installation

 LVL beams must not be cut or drilled

 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
 approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent

Manufacturer Info

APA: PR-L318

Kott Lumber Company 14 Anderson Blvd, Ontario Canada L4A 7X4 905-642-4400



This design is valid until 10/18/2021

Wind

0 PLF 0 lb .19



Client:

Project: Address: Date: 12/18/2018

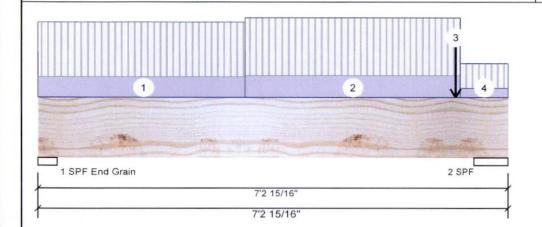
SB Designer:

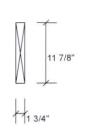
Job Name: LOT 9 (AMELIA 3 EL-2)

Forex 2.0E-3000Fb LVL

1.750" X 11.875" - PASSED

Level: Second Floor





Member Information

Type:	Girder	Application:	Floor (Residential)
Plies:	1	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

Unfactored Reactions UNPATTERNED Ib (Uplift)

3rg	Live	Dead	Snow	Wind
1	248	110	0	0
2	643	266	0	0

Bearings and Factored Reactions

Snow

0 PSF

0 PSF

0 PSF

0 lb

Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	3.500"	11%	138 / 372	510	L	1.25D+1.5L
2 - SPF	6.438"	19%	333 / 965	1298	L	1.25D+1.5L

Wind

0 PSF

0 PSF

0 PSF

0 lb F6

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	884 ft-lb	3'10 9/16"	17130 ft-lb	0.052 (5%)	1.25D+1.5L	L
Unbraced	884 ft-lb	3'10 9/16"	6876 ft-lb	0.129 (13%)	1.25D+1.5L	L
Shear	1154 lb	5'9 3/8"	5798 lb	0.199 (20%)	1.25D+1.5L	L
Perm Defl in.	0.004 (L/19225)	3'7 3/4"	0.218 (L/360)	0.020 (2%)	D	Uniform
LL Defl inch	0.009 (L/8390)	3'7 15/16"	0.218 (L/360)	0.040 (4%)	L	L
TL Defl inch	0.013 (L/5841)	3'7 7/8"	0.327 (L/240)	0.040 (4%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Top braced at bearings.
- 3 Bottom braced at bearings

ID	Load Type	Location	Trib Width	Side	Dead
1	Tie-In	0-0-0 to 3-1-12	(Span)3-1-13	Тор	15 PSF
2	Tie-In	3-1-12 to 6-5-14	(Span)3-4-0	Тор	15 PSF
3	Point	6-5-0		Far Face	176 lb
4	Tie-In	6-5-14 to 7-2-15	(Span)1-5-0	Тор	15 PSF
	Self Weight				5 PLF

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



January 02, 2019

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- LVIL beams must not be cut or drilled Refer to manufacturer's product informatio regarding installation requirements, multi-pi fastening details, beam strength values, and cod approvals Darmaged Beams must not be used

Handling & Installation

- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Live

40 PSF

40 PSF

449 lb

40 PSF

APA: PR-L318

Manufacturer Info

Kott Lumber Company 14 Anderson Blvd, Ontario Canada L4A 7X4 905-642-4400



