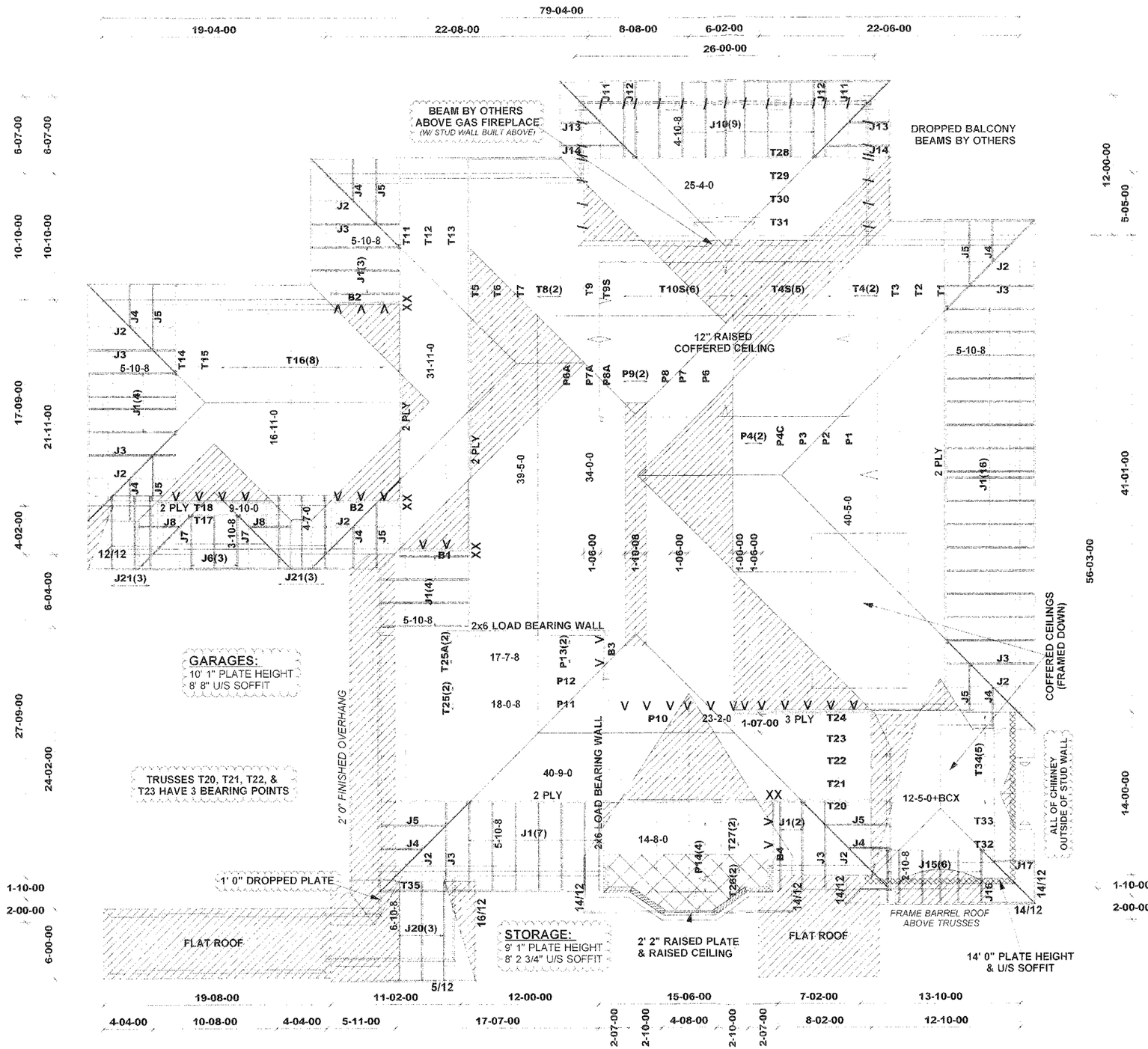

ASPHALT SHINGLES
FINISHED OVERHANG: 16"
CLADDING ALLOWANCE: 5"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

All conventional framing to conform with Part 9 of O.B.C. 2012. Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DENOTES:
CONVENTIONAL
FRAMING

DESIGN CONFORMS WITH OBC 2012
OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 31.3 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 25.6 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LJS26DS - (**V**)
HGUS26-2 - (**XX**)
H2.5T - (*I*)

BEAMS:
B1 - B4: 2 - 2 x 10 SPF #2

ALL ROOF SLOPES ARE 9/12 UNLESS OTHERWISE NOTED

M12616

TAMARACK LUMBER INC.
A LP'S LUMBER GROUP

Job Track: 50916
Plan Log: 201766
Layout ID: 405684

Builder / Location:
GREENYORK HOMES / RICHMOND HILL

Project: **BALTRIO**

Date: 9/27/2019 Sales: Mario DiCano Designer: CH

Model / Elevation:
AVIGNON 3

Mitek ver 8.3.0.234

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50916
Builder:	GREENYORK HOMES	PlanLog:	201766
Project:	BALTRIO	Layout ID:	405684
Location:	RICHMOND HILL	Ref #	11515
Model:	AVIGNON 3	Page:	1 of 6
Lot #:		Date:	09/27/2019
Elevation:	CUSTOM	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T1 Hip Girder	9/12	**40-05-00**	6-02-02	2 x 4 2 x 6	1-07-08 1-07-08	1-09-04 1-09-04	397.26 245.33		
	1	T2 Hip	9/12	**40-05-00**	7-08-02	2 x 4 2 x 6	1-07-08 1-07-08	1-09-04 1-09-04	211.62 131.50		
	1	T3 Hip	9/12	**40-05-00**	9-02-02	2 x 4 2 x 6	1-07-08 1-07-08	1-09-04 1-09-04	226.4 139.50		
	2	T4 Hip	9/12	**40-05-00**	10-08-02	2 x 4 2 x 6		1-09-04 1-09-04	464.87 285.33		
	5	T4S Piggyback Base	9/12	**40-05-00**	10-08-02	2 x 4 2 x 6		1-09-04 1-09-04	1231.54 760.83		
	1 2-ply	T5 Half Hip Girder	9/12	**39-05-00**	6-02-02	2 x 4 2 x 6	1-07-08	1-09-04 6-02-02	392.08 240.67		
	1	T6 Half Hip	9/12	**39-05-00**	7-08-02	2 x 4 2 x 6	1-07-08	1-09-04 7-08-02	211.17 128.50		
	1	T7 Half Hip	9/12	**39-05-00**	9-02-02	2 x 4 2 x 6	1-07-08	1-09-04 9-02-02	257.08 157.17		
	2	T8 Half Hip	9/12	**39-05-00**	10-08-02	2 x 4 2 x 6	1-07-08	1-09-04 10-08-02	513.19 302.67		
	1	T9 Piggyback Base	9/12	**34-00-00**	10-08-02	2 x 4 2 x 6		5-10-00 10-08-02	240.21 143.33		
	1	T9S Piggyback Base	9/12	**34-00-00**	10-08-02	2 x 4 2 x 6		5-10-00 10-08-02	267.03 161.67		
	6	T10S Half Hip	9/12	**40-05-00**	10-08-02	2 x 4 2 x 6		5-10-00 10-08-02	1863.14 1118.00		
	1 2-ply	T11 Hip Girder	9/12	**31-11-00**	6-02-02	2 x 4 2 x 6	1-07-08 1-07-08	1-09-04 1-09-04	314.27 194.67		
	1	T12 Hip	9/12	**31-11-00**	7-08-02	2 x 4	1-07-08 1-07-08	1-09-04 1-09-04	153.59 97.50		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

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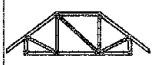
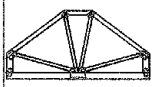
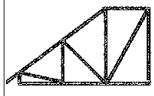
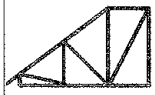
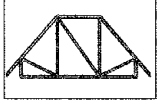
Model:	AVIGNON 3	Page:	2 of 6
Lot #:		Date:	09/27/2019
Elevation:	CUSTOM	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	T13 Hip	9 /12	**31-11-00**	9-02-02	2 x 4	1-07-08 1-07-08	1-09-04 1-09-04	160.34 99.17		
	1	T14 Hip Girder	9 /12	**16-11-00**	6-02-02	2 x 4	1-07-08 1-07-08	1-09-04 1-09-04	78.46 50.33		
	1	T15 Hip	9 /12	**16-11-00**	7-08-02	2 x 4	1-07-08 1-07-08	1-09-04 1-09-04	81.41 52.67		
	8	T16 Common	9 /12	**16-11-00**	8-01-06	2 x 4	1-07-08 1-07-08	1-09-04 1-09-04	614.84 378.67		
	1	T17 Hip Girder	9 /12	**9-10-00**	3-11-02	2 x 4	1-07-08 1-07-08	1-09-04 1-09-04	46.46 30.17		
	1 2-ply	T18 Hip Girder	9 /12	**9-10-00**	4-08-08	2 x 4 2 x 6		1-09-04 1-09-04	100.07 66.67		
	1 2-ply	T20 Hip Girder	9 /12	**40-09-00**	6-02-02	2 x 4 2 x 6	1-07-08 1-07-08	1-09-04 1-09-04	407.1 250.67		
	1	T21 Hip	9 /12	**40-09-00**	7-08-02	2 x 4 2 x 6	1-07-08 1-07-08	1-09-04 1-09-04	227.84 141.67		
	1	T22 Hip	9 /12	**40-09-00**	9-02-02	2 x 4 2 x 6	1-07-08 1-07-08	1-09-04 1-09-04	228.96 140.67		
	1	T23 Hip	9 /12	**40-09-00**	10-08-02	2 x 4 2 x 6	1-07-08 1-07-08	1-09-04 1-09-04	248.92 152.67		
	1 3-ply	T24 Piggyback Base Girder	9 /12	**23-02-00**	10-08-02	2 x 6		1-09-04 10-08-02	531.53 320.00		
	2	T25 Piggyback Base	9 /12	**18-00-08**	10-08-02	2 x 4	1-07-08	1-09-04 10-08-02	201.55 127.67		
	2	T25A Piggyback Base	9 /12	**17-07-08**	10-08-02	2 x 4	1-07-08	1-09-04 10-08-02	199.2 125.00		
	2	T26 Piggyback Base	14 /12	**14-08-00**	7-10-00	2 x 4	1-07-08 1-07-08	2-06-00 2-06-00	154.21 97.67		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

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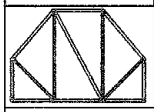
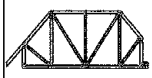
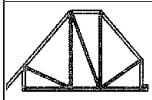
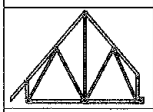
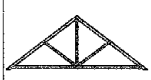
Project:	BALTRIO	Layout ID:	405684
Location:	RICHMOND HILL	Ref #	11515
Model:	AVIGNON 3	Page:	3 of 6
Lot #:		Date:	09/27/2019
Elevation:	CUSTOM	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	2	T27 Piggyback Base	14 /12	**14-08-00**	10-00-00	2 x 4		4-08-00 4-08-00	173.93 111.33		
	1	T28 Hip Girder	9 /12	**25-04-00**	5-04-06	2 x 4	1-06-08 1-06-08	1-08-08 1-08-08	113.5 72.00		
	1	T29 Hip	9 /12	**25-04-00**	6-10-06	2 x 4	1-06-08 1-06-08	1-08-08 1-08-08	116.34 73.50		
	1	T30 Hip	9 /12	**25-04-00**	8-04-06	2 x 4	1-06-08 1-06-08	1-08-08 1-08-08	123.06 77.33		
	1	T31 Hip	9 /12	**25-04-00**	9-10-06	2 x 4	1-06-08 1-06-08	1-08-08 1-08-08	126.12 80.33		
	1	T32 Hip Girder	14 /12	**12-05-00**	5-10-04	2 x 4	1-07-08	2-06-00 3-02-03	70.44 46.67		
	1	T33 Hip	14 /12	**12-05-00**	8-02-04	2 x 4	1-07-08	2-06-00 3-02-03	71.42 46.00		
	5	T34 Common	14 /12	**12-05-00**	10-01-00	2 x 4	1-07-08	2-06-00 3-02-03	373.08 238.33		
	1	T35 Roof Special Girder	9 /12	**6-08-00**	3-10-15	2 x 4	2-03-08 1-07-08	1-10-04 1-11-05	37.8 25.33		
	1	P1 Piggyback	9 /12	**16-08-00**	1-06-00	2 x 4			40.82 25.83		
	1	P2 Piggyback	9 /12	**16-08-00**	3-00-00	2 x 4			51.88 32.83		
	1	P3 Piggyback	9 /12	**16-08-00**	4-06-00	2 x 4			50.69 31.67		
	2	P4 Piggyback	9 /12	**16-08-00**	6-03-00	2 x 4			106.1 66.67		
	1	P4C Piggyback	9 /12	**16-08-00**	6-00-00	2 x 4			52.71 33.33		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

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Lumber Yard:	TAMARACK LUMBER	Job Track:	50916
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Project:	BALTRIO	Layout ID:	405684
Location:	RICHMOND HILL	Ref #	11515
Model:	AVIGNON 3	Page:	4 of 6
Lot #:		Date:	09/27/2019
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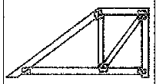
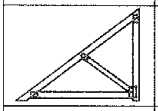
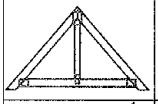
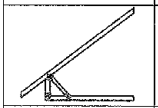
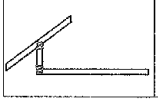
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	P6 Piggyback	9 /12	33-11-08	1-06-00	2 x 4		1-06-00	86.5 53.33		
	1	P6A Piggyback	9 /12	27-06-08	1-06-00	2 x 4		1-06-00	70.95 44.17		
	1	P7 Piggyback	9 /12	33-11-08	3-00-00	2 x 4		1-05-10	113.88 72.50		
	1	P7A Piggyback	9 /12	27-06-08	3-00-00	2 x 4		3-00-00	93.12 59.33		
	1	P8 Piggyback	9 /12	33-11-08	4-01-08	2 x 4		1-05-10	119.74 74.67		
	1	P8A Piggyback	9 /12	27-06-08	4-01-08	2 x 4		4-01-08	99.12 62.67		
	2	P9 Piggyback	9 /12	33-11-08	5-07-08	2 x 4		1-05-10	264.58 164.00		
	1 3-ply	P10 Piggyback	9 /12	11-03-08	1-02-04	2 x 4		1-02-04	89.74 58.00		
	1	P11 Piggyback	9 /12	6-02-00	1-06-00	2 x 4		1-06-00	18.6 13.17		
	1	P12 Piggyback	9 /12	6-02-00	3-00-00	2 x 4		3-00-00	21.36 14.33		
	2	P13 Piggyback	9 /12	5-09-00	4-03-12	2 x 4		4-03-12	40.87 25.67		
	4	P14 Piggyback	14 /12	5-06-05	3-02-11	2 x 4			66.95 46.00		
	36	J1 Jack-Open	9 /12	5-10-08	6-02-02	2 x 4	1-07-08	1-09-04 6-02-02	745.27 468.00		
	8	J2 Jack-Open Girder	9 /12	1-09-07	3-01-05	2 x 4	1-07-08 4-01-01	1-09-04 3-01-05	106.66 69.33		

TAMARACK
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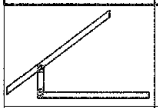
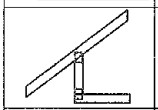
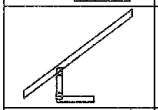
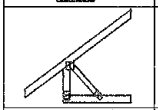
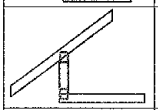
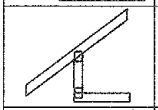
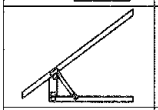
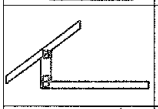
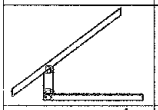
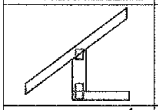
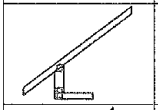
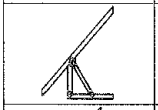
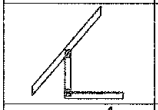
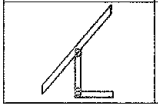
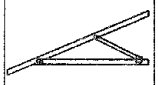
	7	J3 Jack-Open	9 /12	3-09-07	4-07-05	2 x 4	1-07-08 2-01-01	1-09-04 4-07-05	113.28 70.00		
	8	J4 Jack-Open	9 /12	1-09-07	3-01-05	2 x 4	1-07-08 1-01	1-09-04 3-01-05	70.17 48.00		
	8	J5 Jack-Open	9 /12	1-10-08	4-07-05	2 x 4	1-07-08 1-10-15	1-09-04 3-02-02	92.97 58.67		
	3	J6 Jack-Open	9 /12	2-10-08	3-11-02	2 x 4	1-07-08	1-09-04 3-11-02	39.01 25.00		
	2	J7 Jack-Open Girder	9 /12	1-09-07	3-01-05	2 x 4	1-07-08 1-01-01	1-09-04 3-01-05	19.82 13.33		
	2	J8 Jack-Open	9 /12	1-09-07	3-01-05	2 x 4	1-07-08 1-01	1-09-04 3-01-05	17.54 12.00		
	9	J10 Jack-Open	9 /12	4-10-08	5-04-06	2 x 4	1-06-08	1-08-08 5-04-06	170.44 111.00		
	2	J11 Jack-Open Girder	9 /12	1-09-07	3-00-09	2 x 4	1-06-08 3-01-01	1-08-08 3-00-09	26.11 17.33		
	2	J12 Jack-Open	9 /12	3-09-07	4-06-09	2 x 4	1-06-08 1-01-01	1-08-08 4-06-09	31.81 20.00		
	2	J13 Jack-Open	9 /12	1-09-07	3-00-09	2 x 4	1-06-08 1-01	1-08-08 3-00-09	19.27 13.33		
	2	J14 Jack-Open	9 /12	1-10-08	4-06-09	2 x 4	1-06-08 1-10-15	1-08-08 3-01-06	24.97 16.00		
	6	J15 Jack-Open	14 /12	2-10-08	5-10-04	2 x 4	1-07-08	2-06-00 5-10-04	95.99 65.00		
	1	J16 Jack-Open Girder	14 /12	1-09-07	4-07-00	2 x 4	1-07-08 1-01-01	2-06-00 4-07-00	12.04 8.00		
	1	J17 Jack-Open	14 /12	1-09-07	4-07-00	2 x 4	1-07-08 1-01	2-06-00 4-07-00	10.9 7.33		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50916
Builder:	GREENYORK HOMES	PlanLog:	201766
Project:	BALTRIO	Layout ID:	405684
Location:	RICHMOND HILL	Ref #	11515
Model:	AVIGNON 3	Page:	6 of 6
Lot #:		Date:	09/27/2019
Elevation:	CUSTOM	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	3	J20 Jack-Open	5 /12	**6-10-08**	3-10-05	2 x 4	1-07-08	4-01 3-02-07	62.88 40.00		
	6	J21 Jack-Open	12 /12	**4-07-00**	6-07-07	2 x 4	1-07-08	9-08 5-04-08	96.02 64.00		

TOTAL # TRUSS= **203** TOTAL BFT OF ALL TRUSSES= **8915.68** BFT. TOTAL WEIGHT OF ALL TRSSES **14372.76** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
28	Hardware	LJS26DS	
4	Hardware	HGUS26-2	
27	Hardware	H2.5T	

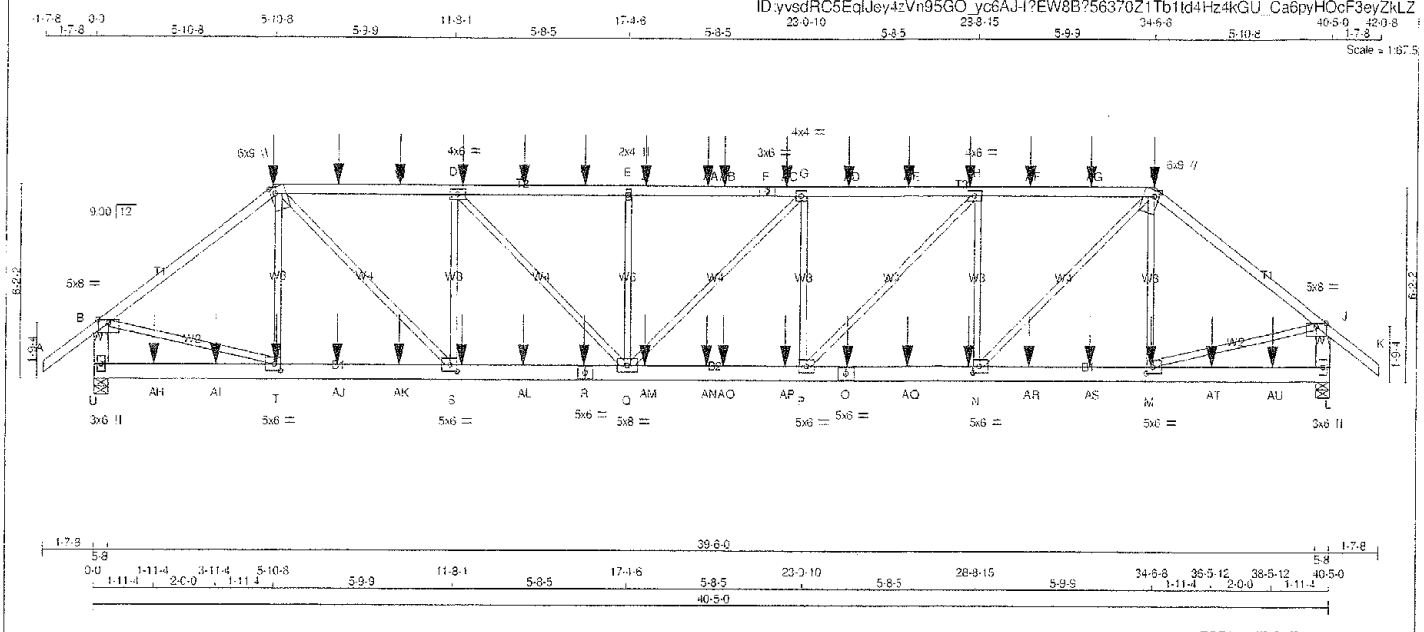
TOTAL NUMBER OF ITEMS= **59**

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405684	T1	1	2	GREENYORK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.300 S May 10 2019 MiTek Industries, Inc. Thu Sep 26 15:23:06 2019 Page 1

ID:yvsdRC5EqlJey4zVn95GO_yc6AJ-I?EW8B?56370Z1Tb1td4Hz4kGU_Ca6pyHOcF3eyZkLZ


1-7-8
5-10-8
5-9-9
5-8-5
17-4-6
5-8-5
23-0-10
5-8-5
28-8-15
5-9-9
34-6-8
5-10-8
40-5-0
42-0-8
1-7-8
Scale = 1:67.5
9.00 12
6x8 =
4x4 =
6x8 =
6x8 =
6x8 =
3x6 =
2x4 =
4x6 =
6-9-7
6-9-2
1-9-4
T1
W1
W2
W3
W4
W5
W6
W7
W8
A
B
C
D
E
F
G
H
J
K
L
M
N
O
P
Q
R
S
T
U
AH
AI
AJ
AK
AL
AM
AN
AO
AP
AQ
AR
AS
AT
AU
3x6 =
5x6 =
5x8 =

TOTAL WEIGHT = 2 X 199 = 397 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
U - R	2x6	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	SIDE(61.0)
C-F	1	12	SIDE(61.0)
F-I	1	12	SIDE(61.0)
I-K	1	12	SIDE(61.0)
U-B	2	12	TOP
L-J	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
U-R	2	12	SIDE(183.1)
R-O	2	12	SIDE(0.0)
O-L	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	6.0	9.0	Edge	2.00
D	TMWW-t	MT20	4.0	6.0		
E	TMW+w	MT20	3.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TMWW-t	MT20	4.0	6.0		
I	TTWW+m	MT20	6.0	9.0	Edge	2.00
J	TMVW-p	MT20	5.0	8.0	Edge	
L	BMV1+p	MT20	3.0	6.0		
M	BMWW-t	MT20	5.0	6.0	2.50	2.50
N	BMWW-t	MT20	5.0	6.0	2.50	2.75
O	BS-t	MT20	5.0	6.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	5.0	8.0		
R	BS-t	MT20	5.0	6.0		
S	BMWW-t	MT20	5.0	6.0	2.50	2.75
T	BMWW-t	MT20	5.0	6.0	2.50	2.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	4396	0	4396	0	0	5-8	5-8
L	4396	0	4396	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	3108	2046 / 0	0 / 0	0 / 0	0 / 0	1061 / 0	0 / 0
L	3108	2046 / 0	0 / 0	0 / 0	0 / 0	1061 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.78 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

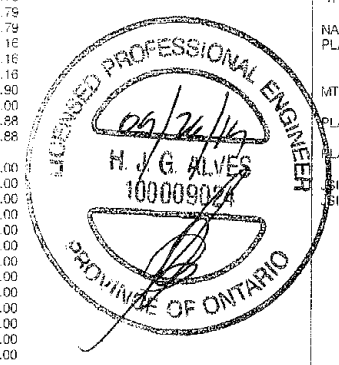
CHORDS

MEMB. FR-TO	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 47	-91.8	-91.8	0.10 (1)	10.00	
B-C	-4986 / 0	-91.8	-91.8	0.57 (1)	3.90	
C-V	-6591 / 0	-91.8	-91.8	0.76 (1)	3.15	
V-W	-6591 / 0	-91.8	-91.8	0.76 (1)	3.15	
W-D	-6591 / 0	-91.8	-91.8	0.76 (1)	3.15	
D-X	-7845 / 0	-91.8	-91.8	0.88 (1)	2.78	
X-Y	-7845 / 0	-91.8	-91.8	0.88 (1)	2.78	
Y-E	-7845 / 0	-91.8	-91.8	0.88 (1)	2.78	
E-Z	-7845 / 0	-91.8	-91.8	0.83 (1)	2.86	
Z-AA	-7845 / 0	-91.8	-91.8	0.83 (1)	2.86	
AA-AB	-7845 / 0	-91.8	-91.8	0.83 (1)	2.86	
AB-F	-7845 / 0	-91.8	-91.8	0.83 (1)	2.86	
F-AC	-7845 / 0	-91.8	-91.8	0.83 (1)	2.86	
AC-G	-7845 / 0	-91.8	-91.8	0.83 (1)	2.86	
G-AD	-7858 / 0	-91.8	-91.8	0.88 (1)	2.79	
AD-AE	-7858 / 0	-91.8	-91.8	0.88 (1)	2.79	
AE-H	-7858 / 0	-91.8	-91.8	0.88 (1)	2.79	
H-AF	-6588 / 0	-91.8	-91.8	0.76 (1)	3.16	
AF-AG	-6588 / 0	-91.8	-91.8	0.76 (1)	3.16	
AG-I	-6588 / 0	-91.8	-91.8	0.76 (1)	3.16	
I-J	-4986 / 0	-91.8	-91.8	0.57 (1)	3.90	
J-K	0 / 47	-91.8	-91.8	0.10 (1)	10.00	
U-B	-4331 / 0	0.0	0.0	0.16 (1)	6.88	
L-J	-4332 / 0	0.0	0.0	0.16 (1)	6.88	

WEBS

MEMB. FR-TO	FORCE (LBS)	MAX CSI (LC)
T-C	-673 / 0	0.13 (1)
C-S	0 / 3702	0.46 (1)
S-D	-2408 / 0	0.69 (1)
D-Q	0 / 179	0.22 (1)
Q-E	-1068 / 0	0.31 (1)
Q-G	-18 / 0	0.01 (1)
P-G	-1063 / 0	0.31 (1)
P-H	0 / 1813	0.22 (1)
N-H	-2421 / 0	0.70 (1)
N-I	0 / 3697	0.46 (1)
M-I	-669 / 0	0.19 (1)
B-T	0 / 4084	0.51 (1)
M-J	0 / 4085	0.51 (1)

U-AH	0 / 0	-18.5	-18.5	0.07 (4)	10.00	
AH-AI	0 / 0	-18.5	-18.5	0.07 (4)	10.00	
AI-T	0 / 0	-18.5	-18.5	0.07 (4)	10.00	
T-AJ	0 / 3971	-18.5	-18.5	0.29 (1)	10.00	
AJ-AK	0 / 3971	-18.5	-18.5	0.29 (1)	10.00	
AK-S	0 / 3971	-18.5	-18.5	0.29 (1)	10.00	
S-AL	0 / 6591	-18.5	-18.5	0.48 (1)	10.00	
AL-R	0 / 6591	-18.5	-18.5	0.48 (1)	10.00	
R-Q	0 / 6591	-18.5	-18.5	0.48 (1)	10.00	
Q-AM	0 / 7858	-18.5	-18.5	0.58 (1)	10.00	
AM-AN	0 / 7858	-18.5	-18.5	0.58 (1)	10.00	
AN-AO	0 / 7858	-18.5	-18.5	0.58 (1)	10.00	
AO-AP	0 / 7858	-18.5	-18.5	0.58 (1)	10.00	
AP-P	0 / 7858	-18.5	-18.5	0.58 (1)	10.00	
P-O	0 / 6588	-18.5	-18.5	0.47 (1)	10.00	
O-AQ	0 / 6588	-18.5	-18.5	0.47 (1)	10.00	
AQ-N	0 / 6588	-18.5	-18.5	0.47 (1)	10.00	



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.35")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.23")
ALLOWABLE DEFL.(TL)= L/360 (1.35")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.43")

CSI: TC=0.88-1.00 (D-E:1) , BC=0.58/1.00 (P-Q:1) , WB=0.70/1.00 (H-N:1) , SSI=0.27/1.00 (E-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	513	354	1867	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SSI GRIP = 0.89 (S) (INPUT = 0.90)
SSI METAL = 0.62 (R) (INPUT = 1.00)

Structural component only 1/2
DWG# T-1924944 CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC	GREENYORK HOMES	DRWG NO.
405684	T1	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

ID:yvsdRC5EqIJey4zVn95GO_yc6AJ-I?EW8B?56370Z1Tb1td4Hz4kGU_Ca6pyHOcF3evZkLZ

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
U	BMV14p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LOADING

TOTAL LOAD CASES: (4)

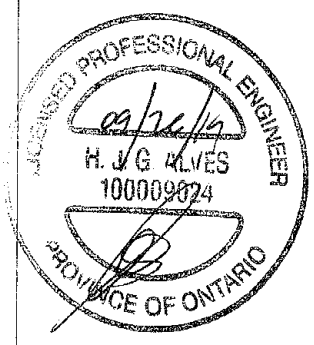
CHORDS MEMB FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	WEBS MEMB. FR-TO	MAX. UNBRAC LENGTH	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N-AR	0 / 3971	-18.5	-18.5	0.29 (1)		10.00		
AR-AS	0 / 3971	-18.5	-18.5	0.29 (1)		10.00		
AS-M	0 / 3971	-18.5	-18.5	0.29 (1)		10.00		
M-AT	0 / 0	-18.5	-18.5	0.07 (4)		10.00		
AT-AU	0 / 0	-18.5	-18.5	0.07 (4)		10.00		
AU-L	0 / 0	-18.5	-18.5	0.07 (4)		10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-8	-428	-428	---	BACK	VERT	TOTAL	---	C1
D	11-11-4	-178	-178	---	BACK	VERT	TOTAL	---	C1
H	28-5-12	-178	-178	---	BACK	VERT	TOTAL	---	C1
I	34-6-8	-428	-428	---	BACK	VERT	TOTAL	---	C1
M	34-5-12	-36	-36	---	BACK	VERT	TOTAL	---	C1
N	28-5-12	-36	-36	---	BACK	VERT	TOTAL	---	C1
O	24-5-12	-36	-36	---	BACK	VERT	TOTAL	---	C1
R	15-11-4	-36	-36	---	BACK	VERT	TOTAL	---	C1
S	11-11-4	-36	-36	---	BACK	VERT	TOTAL	---	C1
T	5-11-4	-36	-36	---	BACK	VERT	TOTAL	---	C1
V	7-11-4	-178	-178	---	BACK	VERT	TOTAL	---	C1
W	9-11-4	-178	-178	---	BACK	VERT	TOTAL	---	C1
X	13-11-4	-178	-178	---	BACK	VERT	TOTAL	---	C1
Y	15-11-4	-178	-178	---	BACK	VERT	TOTAL	---	C1
Z	17-11-4	-178	-178	---	BACK	VERT	TOTAL	---	C1
AA	19-11-4	-178	-178	---	BACK	VERT	TOTAL	---	C1
AB	20-5-12	-178	-178	---	BACK	VERT	TOTAL	---	C1
AC	22-5-12	-178	-178	---	BACK	VERT	TOTAL	---	C1
AD	24-5-12	-178	-178	---	BACK	VERT	TOTAL	---	C1
AE	26-5-12	-178	-178	---	BACK	VERT	TOTAL	---	C1
AF	30-5-12	-178	-178	---	BACK	VERT	TOTAL	---	C1
AG	32-5-12	-178	-178	---	BACK	VERT	TOTAL	---	C1
AH	1-11-4	-23	-25	---	BACK	VERT	TOTAL	---	C1
AI	3-11-4	-28	-28	---	BACK	VERT	TOTAL	---	C1
AJ	7-11-4	-36	-36	---	BACK	VERT	TOTAL	---	C1
AK	9-11-4	-36	-36	---	BACK	VERT	TOTAL	---	C1
AL	13-11-4	-36	-36	---	BACK	VERT	TOTAL	---	C1
AM	17-11-4	-36	-36	---	BACK	VERT	TOTAL	---	C1
AN	19-11-4	-36	-36	---	BACK	VERT	TOTAL	---	C1
AO	20-5-12	-36	-36	---	BACK	VERT	TOTAL	---	C1
AP	22-5-12	-36	-36	---	BACK	VERT	TOTAL	---	C1
AQ	26-5-12	-36	-36	---	BACK	VERT	TOTAL	---	C1
AR	30-5-12	-36	-36	---	BACK	VERT	TOTAL	---	C1
AS	32-5-12	-36	-36	---	BACK	VERT	TOTAL	---	C1
AT	36-5-12	-28	-28	---	BACK	VERT	TOTAL	---	C1
AU	38-5-12	-23	-25	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-1924944

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405684	T2	1	1	GREENYORK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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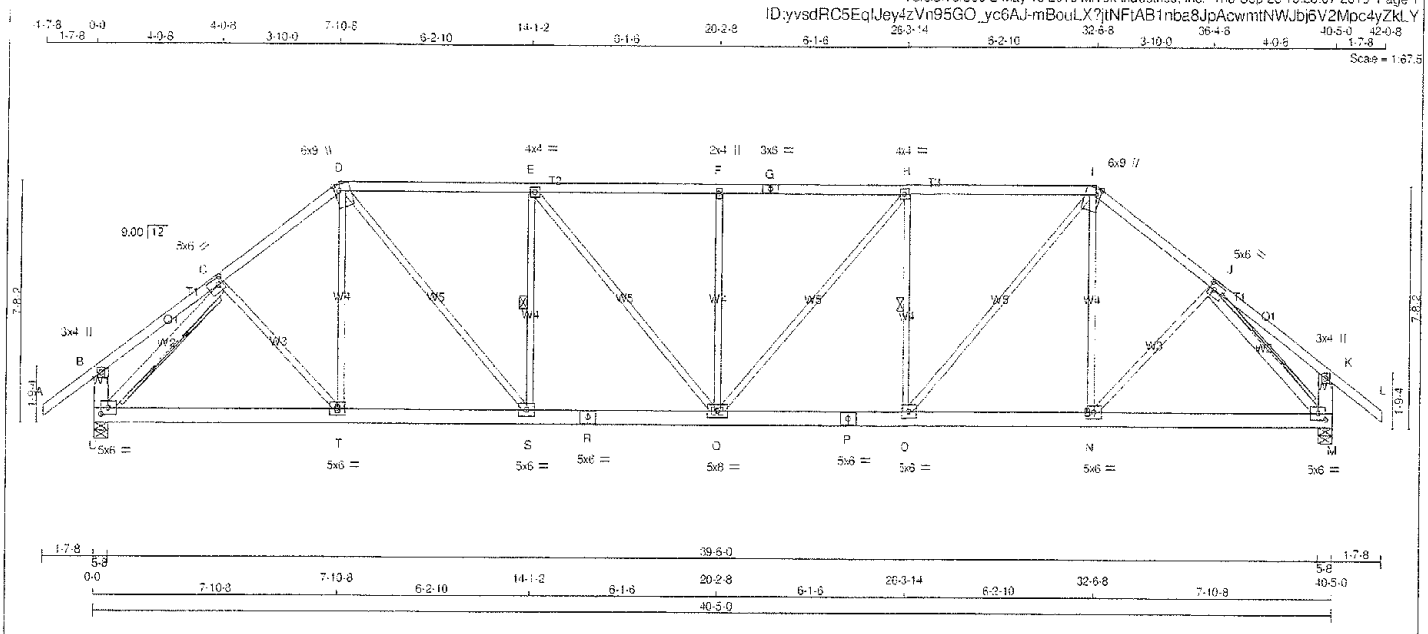


TOTAL WEIGHT = 212 lb [M]

LUMBER

N. L. G. A. RULES

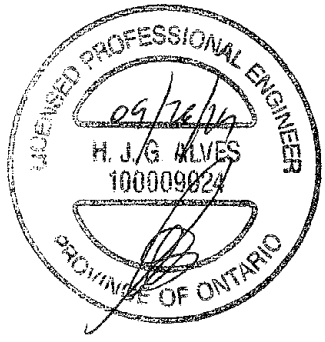
CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
U - R	2x6	DRY	No.2	SPF
R - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
U - C	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.00
D	TTWW+m	MT20	6.0	9.0	Edge	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW+m	MT20	6.0	9.0	Edge	2.00
J	TMWW-t	MT20	5.0	6.0	2.50	2.00
K	TMV+p	MT20	3.0	4.0		
M	BMWW1-t	MT20	5.0	6.0	2.50	2.75
N, O, S, T						
N	BMWW-t	MT20	5.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	5.0	8.0		
R	BS-t	MT20	5.0	6.0		
U	BMWW1-t	MT20	5.0	6.0	2.50	2.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2385	0	2385	0	0	5-8	5-8
M	2385	0	2385	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1684	1122 / 0	0 / 0	0 / 0	0 / 0	562 / 0	0 / 0
M	1684	1122 / 0	0 / 0	0 / 0	0 / 0	562 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.17 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-S, H-O.
2x4 DRY SPF No.2 T-BRACE AT C-U, J-M

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FROM	TO	FACTORED VERT. LOAD LC1 (PLF)	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO							FR-TO		
A-B	0 / 47			-91.8	-91.8 0.19 (1)	10.00	C-T	0 / 154	0.04 (4)
B-C	0 / 24			-91.8	-91.8 0.22 (1)	10.00	T-D	0 / 98	0.03 (4)
C-D	-2471 / 0			-91.8	-91.8 0.33 (1)	4.10	D-S	0 / 1388	0.31 (1)
D-E	-2855 / 0			-91.8	-91.8 0.79 (1)	3.33	S-E	-949 / 0	0.34 (1)
E-F	-3115 / 0			-91.8	-91.8 0.83 (1)	3.17	E-Q	0 / 405	0.09 (1)
F-G	-3115 / 0			-91.8	-91.8 0.83 (1)	3.17	Q-F	-516 / 0	0.53 (1)
G-H	-3115 / 0			-91.8	-91.8 0.83 (1)	3.17	Q-H	0 / 405	0.09 (1)
H-I	-2855 / 0			-91.8	-91.8 0.79 (1)	3.33	O-H	-949 / 0	0.34 (1)
I-J	-2471 / 0			-91.8	-91.8 0.33 (1)	4.10	C-I	0 / 1388	0.31 (1)
J-K	0 / 24			-91.8	-91.8 0.22 (1)	10.00	N-I	0 / 98	0.03 (4)
K-L	0 / 47			-91.8	-91.8 0.19 (1)	10.00	N-J	0 / 154	0.04 (4)
U-B	-296 / 0			0.0	0.0 0.02 (1)	7.81	U-C	-2743 / 0	0.53 (1)
M-K	-296 / 0			0.0	0.0 0.02 (1)	7.81	J-M	-2743 / 0	0.53 (1)
U-T	0	1848		-18.5	-18.5 0.29 (1)	10.00			
T-S	0	1954		-18.5	-18.5 0.30 (1)	10.00			
S-R	0	2855		-18.5	-18.5 0.39 (1)	10.00			
R-Q	0	2855		-18.5	-18.5 0.39 (1)	10.00			
Q-P	0	2855		-18.5	-18.5 0.39 (1)	10.00			
P-O	0	2855		-18.5	-18.5 0.39 (1)	10.00			
O-N	0	1954		-18.5	-18.5 0.30 (1)	10.00			
N-M	0	1848		-18.5	-18.5 0.29 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.35")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (1.35")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.29")

CSI: TC=0.83/1.00 (E-F,1) , BC=0.39/1.00 (Q-S,1) , WB=0.53/0.90 (C-U,1) , SSI=0.27/1.00 (D-E,1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (U) (INPUT = 0.90)
JSI METAL= 0.68 (U) (INPUT = 1.00)

Structural component only
DWG# T-1924945

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405684	T3	1	1	GREENYORK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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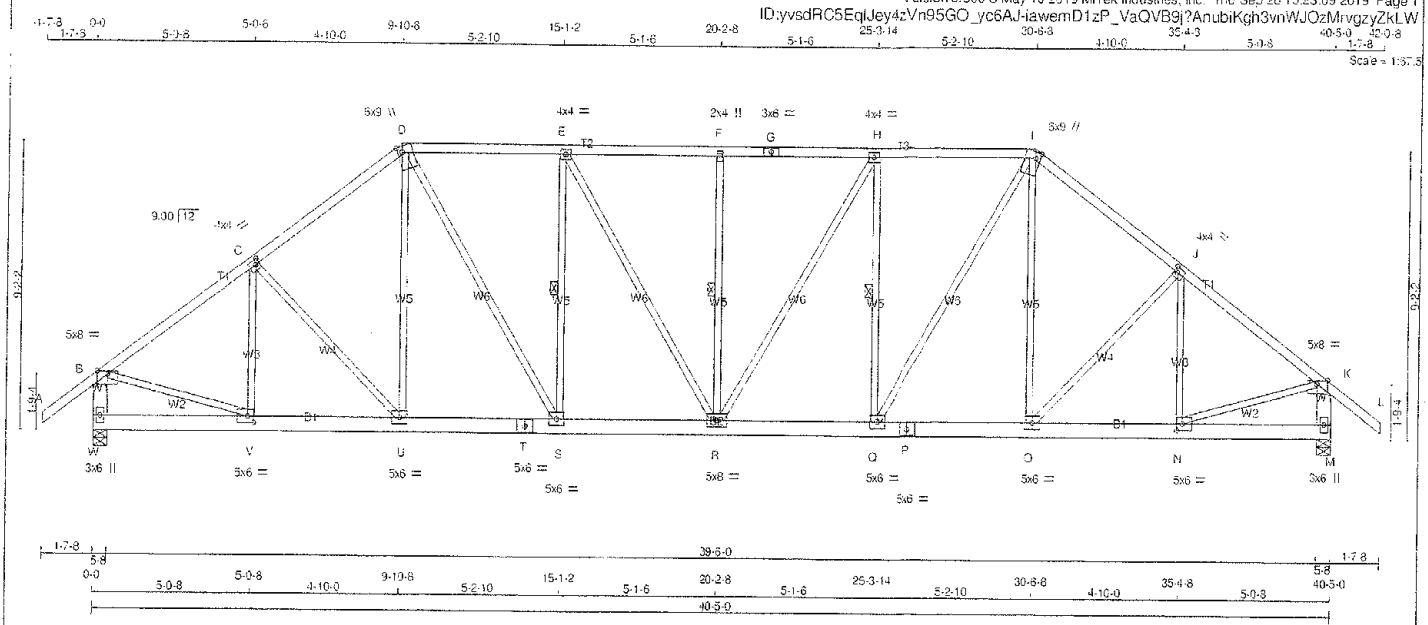


TOTAL WEIGHT = 226 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
W - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
W - T	2x6	DRY	No.2	SPF
T - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	9.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	6.0	9.0	Edge	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW+m	MT20	6.0	9.0	Edge	2.00
J	TMWW-t	MT20	4.0	4.0	2.00	1.50
K	TMVW-p	MT20	5.0	8.0	Edge	
M	BMV1+p	MT20	3.0	6.0		
N	BMWW-t	MT20	5.0	6.0	2.50	2.25
Q, S, U						
O	BMWW-t	MT20	5.0	6.0		
P	BS-t	MT20	5.0	6.0		
R	BMWWW-t	MT20	5.0	8.0		
T	BS-t	MT20	5.0	6.0		
V	BMWW-t	MT20	5.0	6.0	2.50	2.25
W	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
W	2385	0	2385	0	0	5-8	5-8
M	2385	0	2385	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX. MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	1684	1122 / 0	0 / 0	0 / 0	0 / 0	562 / 0	0 / 0
M	1684	1122 / 0	0 / 0	0 / 0	0 / 0	562 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.81 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-S, F-R, H-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

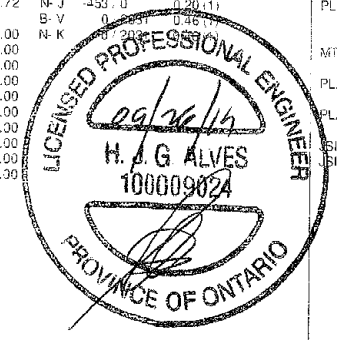
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	(LC)	MAX. UNBRAC LENGTH
A-B	0 / 47	-91.8	-91.8	0	0.19	(1)	10.00
B-C	-2420 / 0	-91.8	-91.8	0	0.55	(1)	3.91
C-D	-2408 / 0	-91.8	-91.8	0	0.53	(1)	3.92
D-E	-2427 / 0	-91.8	-91.8	0	0.49	(1)	3.92
E-F	-2572 / 0	-91.8	-91.8	0	0.50	(1)	3.81
F-G	-2572 / 0	-91.8	-91.8	0	0.50	(1)	3.81
G-H	-2572 / 0	-91.8	-91.8	0	0.50	(1)	3.81
H-I	-2427 / 0	-91.8	-91.8	0	0.49	(1)	3.92
I-J	-2408 / 0	-91.8	-91.8	0	0.53	(1)	3.92
J-K	-2420 / 0	-91.8	-91.8	0	0.55	(1)	3.91
K-L	0 / 47	-91.8	-91.8	0	0.19	(1)	10.00
W-B	-2335 / 0	0.0	0.0	0	0.16	(1)	6.72
M-K	-2335 / 0	0.0	0.0	0	0.16	(1)	6.72
W-V	0 / 0	-18.5	-18.5	0	0.05	(4)	10.00
V-U	0 / 1961	-18.5	-18.5	0	0.26	(1)	10.00
U-T	0 / 1899	-18.5	-18.5	0	0.26	(1)	10.00
T-S	0 / 1899	-18.5	-18.5	0	0.26	(1)	10.00
S-R	0 / 2427	-18.5	-18.5	0	0.33	(1)	10.00
R-Q	0 / 2427	-18.5	-18.5	0	0.33	(1)	10.00
Q-P	0 / 1899	-18.5	-18.5	0	0.26	(1)	10.00
P-O	0 / 1899	-18.5	-18.5	0	0.26	(1)	10.00
O-N	0 / 1961	-18.5	-18.5	0	0.26	(1)	10.00
N-M	0 / 0	-18.5	-18.5	0	0.05	(4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	CSI	(LC)
V-C	-453 / 0	0.20	(1)
C-U	-93 / 0	0.09	(1)
U-D	0 / 176	0.04	(4)
D-S	0 / 1034	0.23	(1)
S-E	-784 / 0	0.41	(1)
E-R	0 / 290	0.07	(1)
R-F	-431 / 0	0.23	(1)
R-H	0 / 290	0.07	(1)
Q-H	-784 / 0	0.41	(1)
Q-I	0 / 1034	0.23	(1)
O-I	0 / 176	0.04	(4)
O-J	-93 / 0	0.09	(1)
N-J	-453 / 0	0.20	(1)
B-V	0 / 2035	0.46	(1)
N-K	0 / 2035	0.46	(1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.35")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (1.35")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.55/1.00 (B-C:1) , BC=0.33/1.00 (R-S:1) , WB=0.46/1.00 (B-V:1) , SSI=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.85 (N) (INPUT = 0.90)
SI METAL= 0.49 (N) (INPUT = 1.00)

Structural component only
DWG# T-1924946

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405684	T4	2	1	GREENYORK HOMES TRUSS DESC.	

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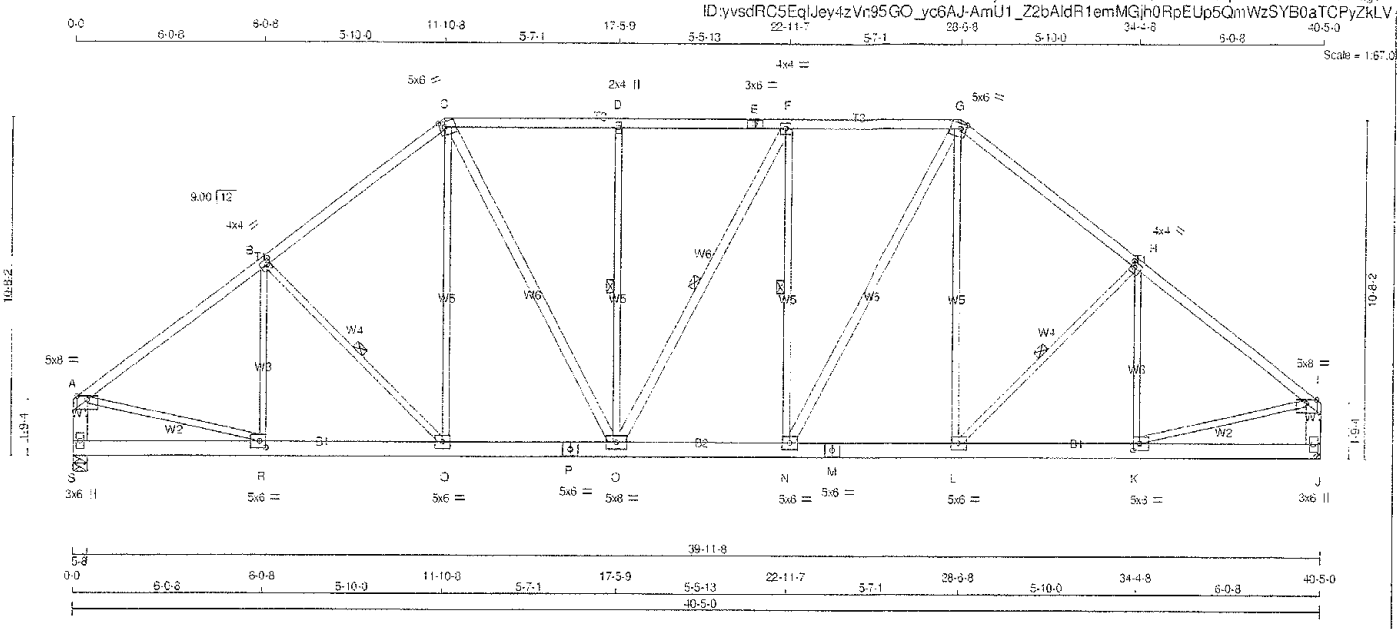


TOTAL WEIGHT = 2 X 232 = 465 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
S - A	2x6	DRY	No.2	SPF
J - I	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - O	2x4	DRY	No.2	SPF
O - F	2x4	DRY	No.2	SPF
N - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-p	MT20	5.0	8.0	Edge	
B	TMWW-t	MT20	4.0	4.0	2.00	1.50
C	TTWW-m	MT20	5.0	6.0	2.00	2.00
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTWW-m	MT20	5.0	6.0	2.00	2.00
H	TMWW-t	MT20	4.0	4.0	2.00	1.50
I	TMVW-p	MT20	5.0	8.0	Edge	
J	BMV1+p	MT20	3.0	6.0		
K	BMWW-t	MT20	5.0	6.0	2.50	2.50
L, N, Q						
L	BMWW-t	MT20	5.0	6.0		
M	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BS-t	MT20	5.0	6.0		
R	BMWW-t	MT20	5.0	6.0	2.50	2.50
S	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT	REQRD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	BRG
S	2228	0	2228	0	0	5-8	5-8
J	2228	0	2228	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 3-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1576	1034 / 0	0 / 0	0 / 0	0 / 0	542 / 0	0 / 0
J	1576	1034 / 0	0 / 0	0 / 0	0 / 0	542 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.85 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF B-Q, D-O, F-O, F-N, H-L

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

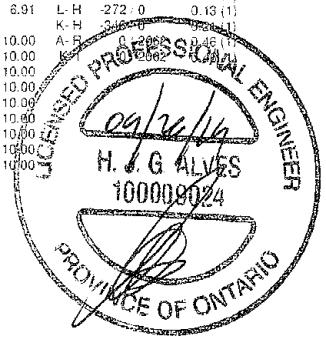
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	-2476 / 0	-91.8	-91.8	0.59 (1)		3.85
B-C	-2318 / 0	-91.8	-91.8	0.54 (1)		3.99
C-D	-2172 / 0	-91.8	-91.8	0.39 (1)		4.25
D-E	-2172 / 0	-91.8	-91.8	0.37 (1)		4.28
E-F	-2172 / 0	-91.8	-91.8	0.37 (1)		4.28
F-G	-2176 / 0	-91.8	-91.8	0.39 (1)		4.24
G-H	-2317 / 0	-91.8	-91.8	0.54 (1)		3.99
H-I	-2476 / 0	-91.8	-91.8	0.59 (1)		3.85
S-A	-2174 / 0	0.0	0.0	0.15 (1)		6.91
J-I	-2174 / 0	0.0	0.0	0.15 (1)		6.91
S-R	0 / 0	-18.5	-18.5	0.07 (4)		10.00
R-O	0 / 2012	-18.5	-18.5	0.27 (1)		10.00
O-P	0 / 1823	-18.5	-18.5	0.25 (1)		10.00
P-O	0 / 1823	-18.5	-18.5	0.25 (1)		10.00
O-N	0 / 2176	-18.5	-18.5	0.29 (1)		10.00
N-M	0 / 1822	-18.5	-18.5	0.25 (1)		10.00
M-L	0 / 1822	-18.5	-18.5	0.25 (1)		10.00
L-K	0 / 2012	-18.5	-18.5	0.27 (1)		10.00
K-J	0 / 0	-18.5	-18.5	0.07 (4)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R-B	-348 / 0	0.21 (1)
Q-C	0 / 312	0.07 (1)
O-C	0 / 731	0.12 (1)
O-D	-548 / 0	0.42 (1)
O-F	-9 / 0	0.01 (1)
N-F	-544 / 0	0.42 (1)
N-G	0 / 742	0.12 (1)
L-G	0 / 304	0.07 (1)
L-H	-272 / 0	0.13 (1)
K-H	-346 / 0	0.21 (1)
A-R	0 / 2005	0.46 (1)
K-J	0 / 2002	0.46 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.35")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.35")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.59/1.00 (H-I:1) , BC=0.29/1.00 (N-O:1) , WB=0.45/1.00 (I-K:1) , SSI=0.24/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS SEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (K) (INPUT = 0.90)
JSI METAL= 0.50 (K) (INPUT = 1.00)

Structural component only
DWG# T-1924947

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405684	T4S	5	1	GREENYORK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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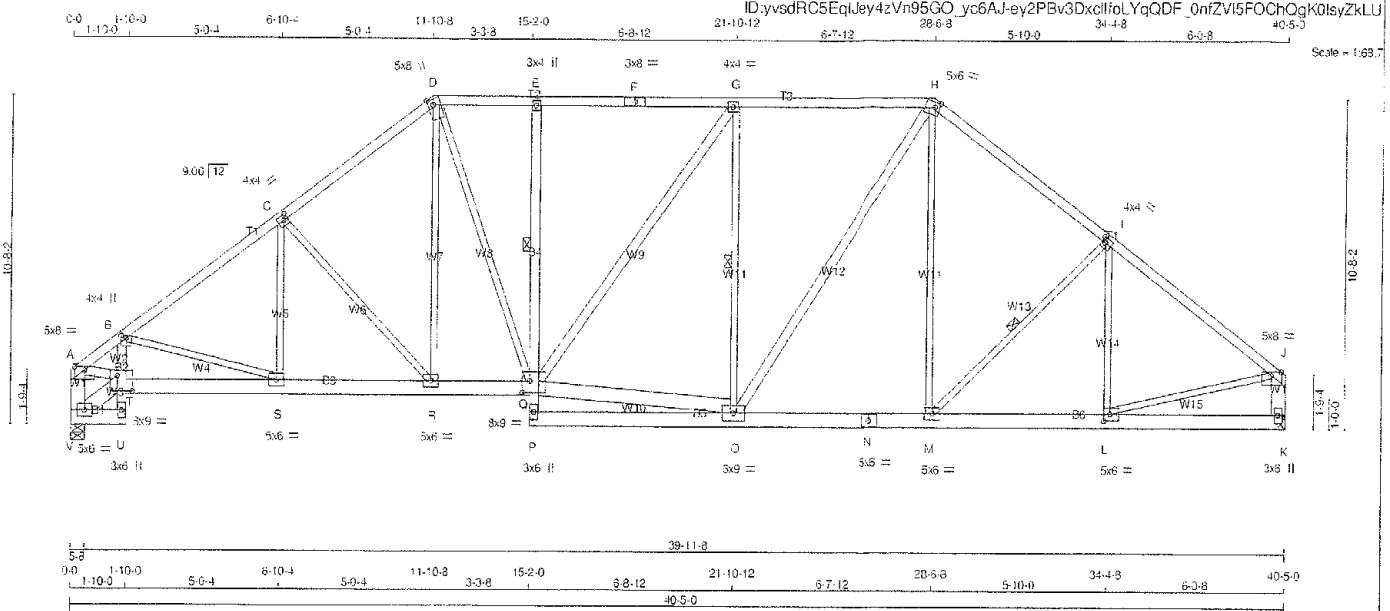


TOTAL WEIGHT = 5 X 247 = 1233 lb

[M][F]

LUMBER

N. L. G. A. RULES

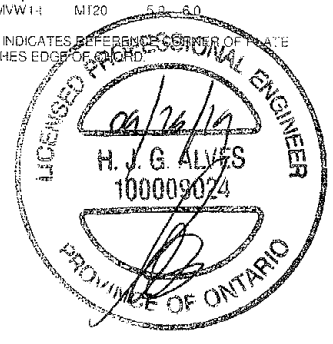
CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
V - A	2x6	DRY	No.2	SPF
K - J	2x6	DRY	No.2	SPF
V - U	2x6	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
T - Q	2x6	DRY	No.2	SPF
P - E	2x4	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
Q - H	2x4	DRY	No.2	SPF
V - T	2x6	DRY	No.2	SPF
A - T	2x4	DRY	No.2	SPF
Q - O	2x6	DRY	No.2	SPF
Q - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	8.0	Edge	
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	8.0	Edge	2.90
E	TMV+p	MT20	3.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW-m	MT20	5.0	6.0	2.00	2.00
I	TMWW-t	MT20	4.0	4.0	2.00	1.50
J	TMVW-p	MT20	5.0	8.0	Edge	
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.50
M, R, S						
M	BMWW-t	MT20	5.0	6.0		
N	SS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	6.0	9.0		
P	BMV+p	MT20	3.0	6.0		
Q	BVMWWW-t	MT20	8.0	9.0	4.50	3.00
T	BVMWW-t	MT20	8.0	9.0	6.00	6.00
U	BMV+p	MT20	3.0	6.0		
V	BMVW1+t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
K	2228	0	2228	0	0	MECHANICAL	
V	2228	0	2228	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K. MINIMUM BEARING LENGTH AT JOINT K = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	1576	1034 / 0	0 / 0	0 / 0	0 / 0	542 / 0	0 / 0
V	1576	1034 / 0	0 / 0	0 / 0	0 / 0	542 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.74 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-Q, G-O, I-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A-B	-3085 / 0	-91.8	-91.8	0.21 (1)	3.83
B-C	-2922 / 0	-91.8	-91.8	0.44 (1)	3.74
C-D	-2551 / 0	-91.8	-91.8	0.40 (1)	4.00
D-E	-2300 / 0	-91.8	-91.8	0.28 (1)	4.26
E-F	-2298 / 0	-91.8	-91.8	0.57 (1)	3.91
F-G	-2298 / 0	-91.8	-91.8	0.57 (1)	3.91
G-H	-2215 / 0	-91.8	-91.8	0.58 (1)	3.96
H-I	-2318 / 0	-91.8	-91.8	0.54 (1)	3.99
I-J	-2475 / 0	-91.8	-91.8	0.59 (1)	3.85
V-A	-2157 / 0	0.0	0.0	0.15 (1)	6.92
K-J	-2174 / 0	0.0	0.0	0.15 (1)	6.91
V-U	0 / 65	-18.5	-18.5	0.01 (1)	10.00
U-T	0 / 17	0.0	0.0	0.11 (1)	10.00
T-B	-174 / 0	0.0	0.0	0.08 (1)	7.81
T-S	0 / 2551	-18.5	-18.5	0.34 (1)	10.00
S-R	0 / 2352	-18.5	-18.5	0.31 (1)	10.00
R-Q	0 / 2018	-18.5	-18.5	0.27 (1)	10.00
P-Q	0 / 55	0.0	0.0	0.05 (1)	10.00
Q-E	-474 / 0	0.0	0.0	0.09 (1)	6.25
P-O	0 / 27	-18.5	-18.5	0.10 (4)	10.00
O-N	0 / 1822	-18.5	-18.5	0.27 (1)	10.00
N-M	0 / 1822	-18.5	-18.5	0.27 (1)	10.00
M-L	0 / 2012	-18.5	-18.5	0.27 (1)	10.00
L-K	0 / 0	-18.5	-18.5	0.07 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B-S	-207 / 0	0.10 (1)
S-C	0 / 103	0.04 (4)
C-R	-497 / 0	0.55 (1)
R-D	0 / 465	0.10 (1)
D-Q	0 / 815	0.18 (1)
Q-G	-809 / 0	0.62 (1)
G-H	0 / 731	0.12 (1)
M-H	0 / 308	0.07 (1)
M-I	-271 / 0	0.13 (1)
L-I	-347 / 0	0.21 (1)
L-J	0 / 2062	0.48 (1)
V-T	-79 / 0	0.01 (1)
A-T	0 / 2513	0.49 (1)
Q-O	0 / 2220	0.26 (1)
Q-G	0 / 130	0.02 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6:00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.35")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/240 (1.35")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.59/1.00 (I-J:1) , BC=0.34/1.00 (S-T:1) , WB=0.62/1.00 (G-O:1) , SSI=0.29/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE	GRIP(DRY)		SHEAR (PSI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	798	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (L) (INPUT = 0.90)
JSI METAL= 0.50 (L) (INPUT = 1.00)

Structural component only
DWG# T-1924948

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405684	T5	1	2	GREENYORK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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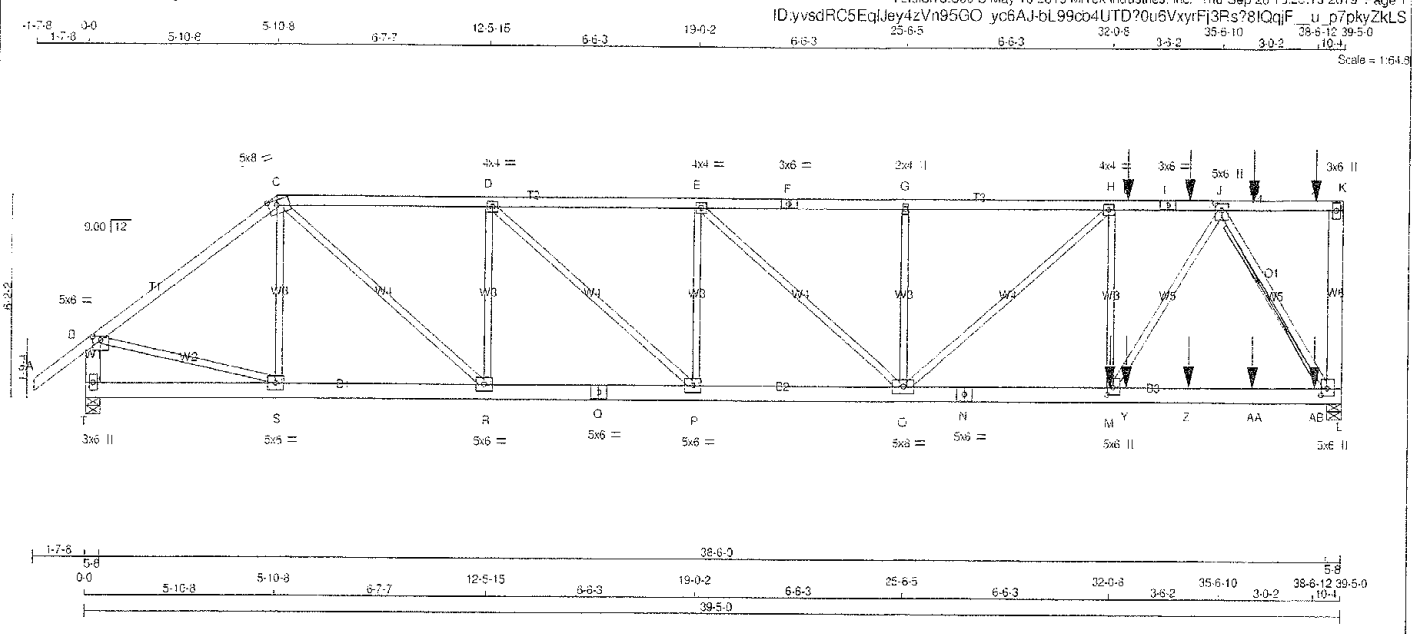


TOTAL WEIGHT = 2 X 196 = 392 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
L - K	2x6	DRY	No.2	SPF
T - B	2x6	DRY	No.2	SPF
T - Q	2x6	DRY	No.2	SPF
Q - N	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- F	1	12	TOP
F- I	1	12	SIDE(0.0)
I- K	1	12	SIDE(0.0)
K- L	2	12	TOP
T- B	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
T- Q	2	12	TOP
Q- N	2	12	TOP
N- L	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
H- M	1	6	SIDE(405.8)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.25	3.00
C	TTWW-m	MT20	5.0	8.0	1.75	3.25
D, E, H						
D	TMVW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
I	TS-t	MT20	3.0	6.0		
J	TMW W+t	MT20	5.0	6.0	2.50	2.50
K	TMV+p	MT20	3.0	6.0		
L	BMVW1+p	MT20	5.0	6.0	2.50	2.25
M	BMWW+t	MT20	5.0	6.0	2.75	2.25
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	8.0		
P, R, S						
P	BMWW+t	MT20	5.0	6.0		
Q	BS-t	MT20	5.0	6.0		
T	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	4787	0	4787	0	0	5-8	5-8
T	2836	0	2836	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX. MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	3380	2248 : 0	0 : 0	0 : 0	0 : 0	1132 : 0	0 : 0
T	2001	1337 : 0	0 : 0	0 : 0	0 : 0	664 : 0	0 : 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, T

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.73 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED

2x6 DRY SPF No.2 T-BRACE AT J-L

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 : 47	-91.8	-91.8	0.10 (1)	10.00
B- C	-3021 : 0	-91.8	-91.8	0.41 (1)	4.90
C- D	-4358 : 0	-91.8	-91.8	0.50 (1)	4.16
D- E	-5395 : 0	-91.8	-91.8	0.58 (1)	3.74
E- F	-5646 : 0	-91.8	-91.8	0.54 (1)	3.73
F- G	-5646 : 0	-91.8	-91.8	0.54 (1)	3.73
G- H	-5646 : 0	-91.8	-91.8	0.54 (1)	3.73
H- U	-5091 : 0	-91.8	-91.8	0.32 (1)	4.00
U- I	-5091 : 0	-91.8	-91.8	0.32 (1)	4.00
I- V	-5091 : 0	-91.8	-91.8	0.32 (1)	4.00
V- J	-5091 : 0	-91.8	-91.8	0.32 (1)	4.00
J- W	0 : 0	-91.8	-91.8	0.20 (1)	10.00
W- X	0 : 0	-91.8	-91.8	0.20 (1)	10.00
X- K	0 : 0	-91.8	-91.8	0.20 (1)	10.00
L- K	-297 : 0	0.0	0.0	0.67 (1)	7.81
T- B	-2793 : 0	0.0	0.0	0.10 (1)	7.81
T- S	0 : 0	-18.5	-18.5	0.05 (4)	10.00
S- R	0 : 2406	-18.5	-18.5	0.18 (1)	10.00
R- Q	0 : 4358	-18.5	-18.5	0.31 (1)	10.00
Q- P	0 : 4358	-18.5	-18.5	0.31 (1)	10.00
P- O	0 : 5395	-18.5	-18.5	0.39 (1)	10.00
O- N	0 : 5091	-18.5	-18.5	0.36 (1)	10.00
N- M	0 : 5091	-18.5	-18.5	0.36 (1)	10.00
M- Y	0 : 2905	-18.5	-18.5	0.27 (1)	10.00
Y- Z	0 : 2905	-18.5	-18.5	0.27 (1)	10.00
Z-AA	0 : 2905	-18.5	-18.5	0.27 (1)	10.00
AA-AB	0 : 2905	-18.5	-18.5	0.27 (1)	10.00
AB- L	0 : 2905	-18.5	-18.5	0.27 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- C	-412 : 0	0.12 (1)
C- R	0 : 2592	0.32 (1)
R- D	-1595 : 0	0.45 (1)
D- P	0 : 1389	0.17 (1)
P- E	-794 : 0	0.23 (1)
E- O	0 : 335	0.04 (1)
O- G	-623 : 0	0.18 (1)
O- H	0 : 743	0.09 (1)
M- H	-1124 : 0	0.32 (1)
B- S	0 : 2475	0.31 (1)
M- J	0 : 225	0.03 (1)
J- L	-241 : 0	0.03 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.31")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (1.31")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.30")

CSI: TC=0.58/1.00 (D-E:1) , BC=0.39/1.00 (O-P:1) , WB=0.80/1.00 (J-L:1) , SSI=0.18/1.90 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

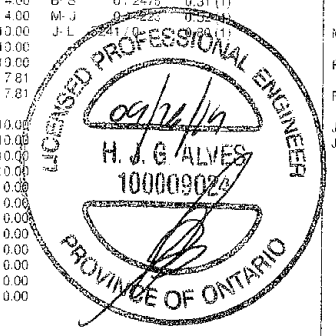
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	513	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (C) (INPUT = 0.90)
JSI METAL= 0.90 (J) (INPUT = 1.00)



Structural component only 1/2
DWG# T-1924949 CONTINUED ON PAGE 2

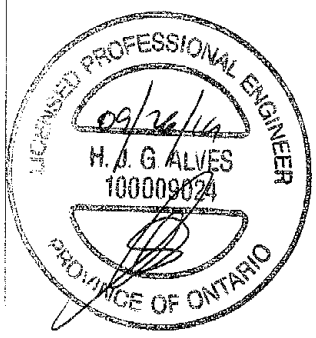
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405684	T5	1	2	GREENYORK HOMES TRUSS DESC.	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
M	32-0-8	-2257	-2257	---	FRONT	VERT	TOTAL	---	C1
U	32-6-12	-178	-178	---	FRONT	VERT	TOTAL	---	C1
V	34-6-12	-178	-178	---	FRONT	VERT	TOTAL	---	C1
W	36-6-12	-178	-178	---	FRONT	VERT	TOTAL	---	C1
X	38-6-12	-184	-184	---	FRONT	VERT	TOTAL	---	C1
Y	32-6-12	-36	-36	---	FRONT	VERT	TOTAL	---	C1
Z	34-6-12	-36	-36	---	FRONT	VERT	TOTAL	---	C1
AA	36-6-12	-36	-36	---	FRONT	VERT	TOTAL	---	C1
AB	38-6-12	-37	-37	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-1924949 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405684	T6	1	1	GREENYORK HOMES TRUSS DESC.	

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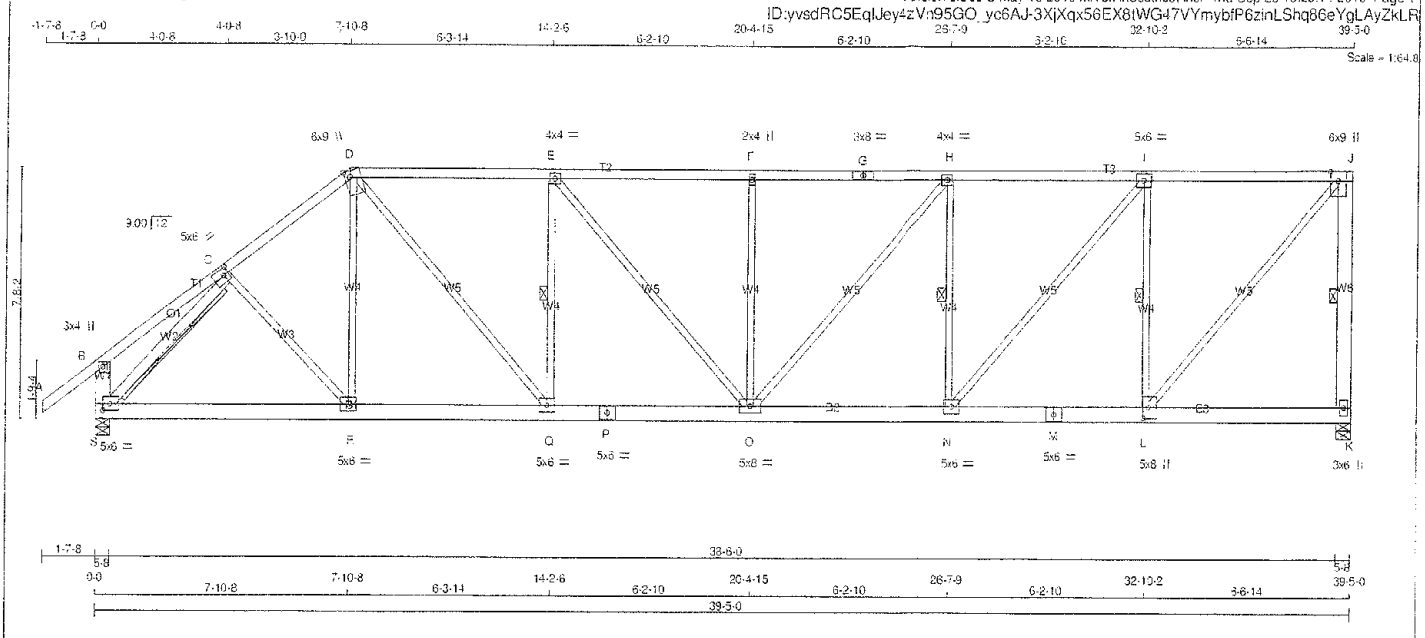


TOTAL WEIGHT = 211 lb

LUMBER

N. L. G. A. RULES

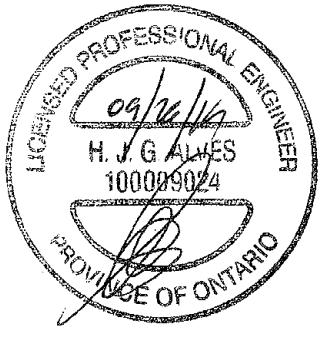
CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
K - J	2x6	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.00
D	TTWW+m	MT20	6.0	9.0	Edge	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	8.0		
H	TMWW-t	MT20	4.0	4.0		
I	TMWW-t	MT20	5.0	6.0		
J	TMVW+p	MT20	6.0	9.0	Edge	
K	BMV1+p	MT20	3.0	6.0		
L	BMWW+t	MT20	5.0	8.0	4.00	2.00
M	BS-t	MT20	5.0	6.0		
N, Q, R						
N	BMWW-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BS-t	MT20	5.0	6.0		
S	BMVW1-t	MT20	5.0	6.0	2.50	2.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	2173	0	2173	0	0	5-8	5-8
S	2330	0	2330	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE — MAX. MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	1537	1008 / 0	0 / 0	0 / 0	0 / 0	528 / 0	0 / 0
S	1645	1096 / 0	0 / 0	0 / 0	0 / 0	549 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, S

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF J-K, H-L, E-Q, H-N.
2x3 DRY SPF No.2 T-BRACE AT C-S

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	LC	MAX. UNBRAC LENGTH
A- B	0 / 47	-91.8	-91.8	0.19	(1)		10.00
B- C	0 / 24	-91.8	-91.8	0.22	(1)		10.00
C- D	-2396 / 0	-91.8	-91.8	0.32	(1)		4.15
D- E	-2758 / 0	-91.8	-91.8	0.80	(1)		3.38
E- F	-2966 / 0	-91.8	-91.8	0.83	(1)		3.25
F- G	-2966 / 0	-91.8	-91.8	0.65	(1)		3.52
G- H	-2966 / 0	-91.8	-91.8	0.65	(1)		3.52
H- I	-2610 / 0	-91.8	-91.8	0.81	(1)		3.42
I- J	-1692 / 0	-91.8	-91.8	0.69	(1)		4.25
K- J	-2122 / 0	0.0	0.0	0.34	(1)		5.36
S- B	-296 / 0	0.0	0.0	0.02	(1)		7.81
S- R	0 / 1797	-18.5	-18.5	0.28	(1)		10.00
R- Q	0 / 1894	-18.5	-18.5	0.29	(1)		10.00
Q- P	0 / 2758	-18.5	-18.5	0.37	(1)		10.00
P- O	0 / 2758	-18.5	-18.5	0.37	(1)		10.00
O- N	0 / 2610	-18.5	-18.5	0.36	(1)		10.00
N- M	0 / 1692	-18.5	-18.5	0.24	(1)		10.00
M- L	0 / 1692	-18.5	-18.5	0.24	(1)		10.00
L- K	0 / 0	-18.5	-18.5	0.09	(4)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI	LC
C- R	0 / 140	0.03	(4)
R- D	0 / 102	0.04	(4)
S- C	-2668 / 0	0.89	(1)
L- J	0 / 2529	0.57	(1)
D- Q	0 / 1318	0.30	(1)
L- I	-1746 / 0	0.62	(1)
Q- E	-887 / 0	0.31	(1)
N- I	0 / 1417	0.32	(1)
E- O	0 / 321	0.07	(1)
N- H	-967 / 0	0.34	(1)
O- F	-554 / 0	0.56	(1)
O- H	0 / 549	0.12	(1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00-12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.31")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (1.31")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.31")

CSI: TC=0.83/1.00 (E-F;1) , BC=0.37/1.00 (O-Q;1) , WB=0.89/0.90 (C-S;1) , SSI=0.28/1.00 (I-J;1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.88 (S) (INPUT = 0.90)
JSI METAL = 0.66 (C) (INPUT = 1.00)

Structural component only
DWG# T-1924950

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405684	T7	1	1	GREENYORK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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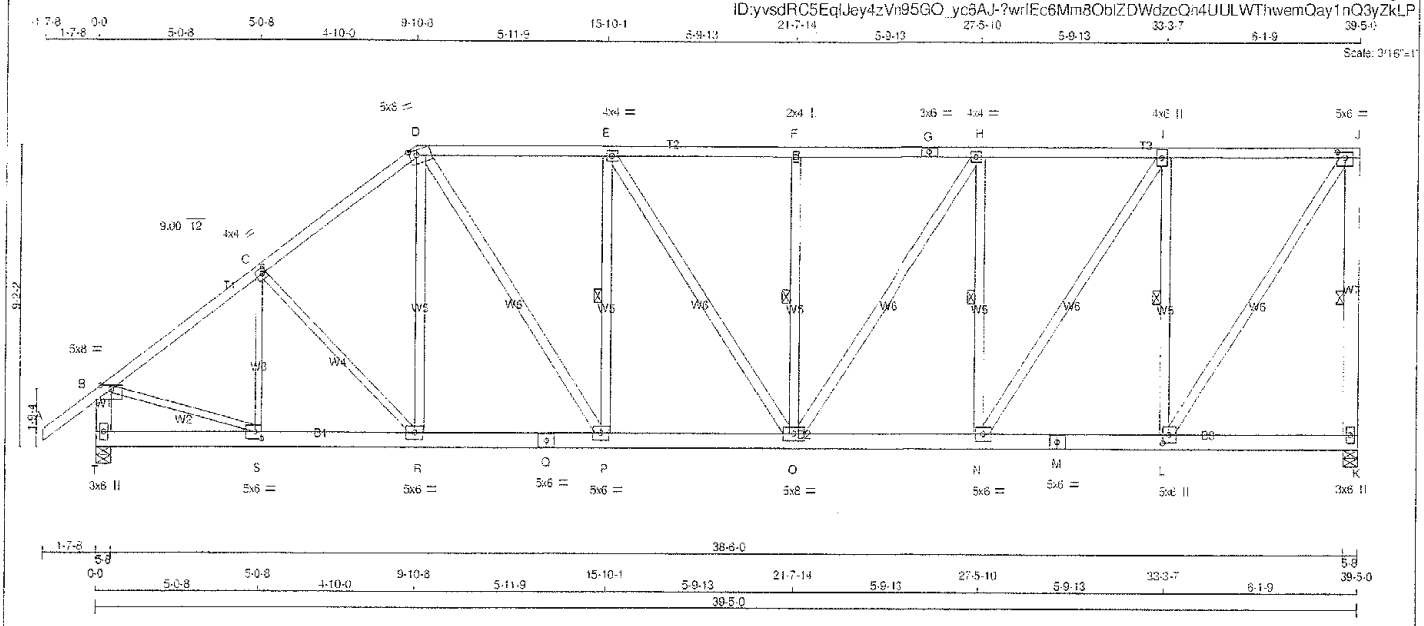


TOTAL WEIGHT = 257 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
K - J	2x6	DRY	No.2	SPF
T - B	2x6	DRY	No.2	SPF
T - Q	2x6	DRY	No.2	SPF
Q - M	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
S - C	2x3	DRY	No.2	SPF
C - R	2x3	DRY	No.2	SPF
B - S	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	8.0	1.75	2.50
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0		
I	TMWW+t	MT20	4.0	6.0		
J	TMVW-t	MT20	5.0	6.0	2.00	3.00
K	BMV1+p	MT20	3.0	6.0		
L	BMWW+t	MT20	5.0	6.0	3.00	2.25
M	BS-t	MT20	5.0	6.0		
N, P, R						
N	BMWW-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	8.0		
Q	BS-t	MT20	5.0	6.0		
S	BMWW-t	MT20	5.0	6.0	2.50	2.50
T	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
K	2173	0	2173	0	0	5-8	5-8
T	2330	0	2330	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	1537	1008 / 0	0 / 0	0 / 0	0 / 0	528 / 0	0 / 0
T	1645	1096 / 0	0 / 0	0 / 0	0 / 0	549 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, T

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.74 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF J-K, H-L, E-P, H-N, F-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

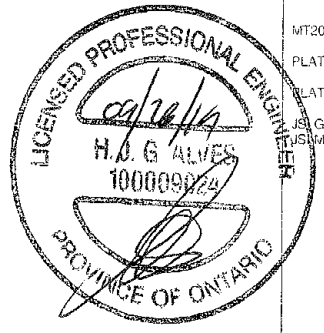
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)		FACTORED VERT. LOAD LC (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47		-91.8	-91.8	0.19 (1)	10.00
B- C	-2353 / 0		-91.8	-91.8	0.54 (1)	3.96
C- D	-2330 / 0		-91.8	-91.8	0.52 (1)	3.98
D- E	-2381 / 0		-91.8	-91.8	0.65 (1)	3.78
E- F	-2437 / 0		-91.8	-91.8	0.65 (1)	3.74
F- G	-2437 / 0		-91.8	-91.8	0.49 (1)	3.97
G- H	-2437 / 0		-91.8	-91.8	0.49 (1)	3.97
H- I	-2085 / 0		-91.8	-91.8	0.62 (1)	3.99
I- J	-1324 / 0		-91.8	-91.8	0.55 (1)	4.88
K- J	-2126 / 0		0.0	0.0	0.51 (1)	5.36
T- B	-2281 / 0		0.0	0.0	0.16 (1)	6.52
T- S	0 / 0		-18.5	-18.5	0.05 (4)	10.00
S- R	0 / 1967		-18.5	-18.5	0.26 (1)	10.00
R- Q	0 / 1837		-18.5	-18.5	0.25 (1)	10.00
Q- P	0 / 1837		-18.5	-18.5	0.25 (1)	10.00
P- O	0 / 2381		-18.5	-18.5	0.32 (1)	10.00
O- N	0 / 2085		-18.5	-18.5	0.29 (1)	10.00
N- M	0 / 1324		-18.5	-18.5	0.19 (1)	10.00
M- L	0 / 1324		-18.5	-18.5	0.19 (1)	10.00
L- K	0 / 0		-18.5	-18.5	0.08 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- C	-437 / 0	0.19 (1)
C- R	-105 / 0	0.10 (1)
R- D	0 / 189	0.03 (4)
B- S	0 / 1976	0.44 (1)
L- J	0 / 2316	0.37 (1)
D- P	0 / 968	0.16 (1)
L- I	-1775 / 0	0.67 (1)
P- E	-689 / 0	0.26 (1)
N- I	0 / 138	0.22 (1)
E- O	0 / 100	0.02 (1)
N- H	-1047 / 0	0.39 (1)
O- F	-517 / 0	0.19 (1)
O- H	0 / 638	0.10 (1)



DESIGN CRITERIA

SPECIFIED LOADS.

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.30/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.31")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (1.31")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.23")

CSI: TC=0.65/1.00 (D-E:1) , BC=0.32/1.00 (O-P:1) , WB=0.67/0.90 (H-L:1) , SSI=0.26/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	619	354	1567	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

LUS GRIP = 0.89 (D) (INPUT = 0.90)
LUS METAL = 0.48 (L) (INPUT = 1.00)

Structural component only
DWG# T-1924951

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405684	T8	2	1	GREENYORK HOMES TRUSS DESC.	

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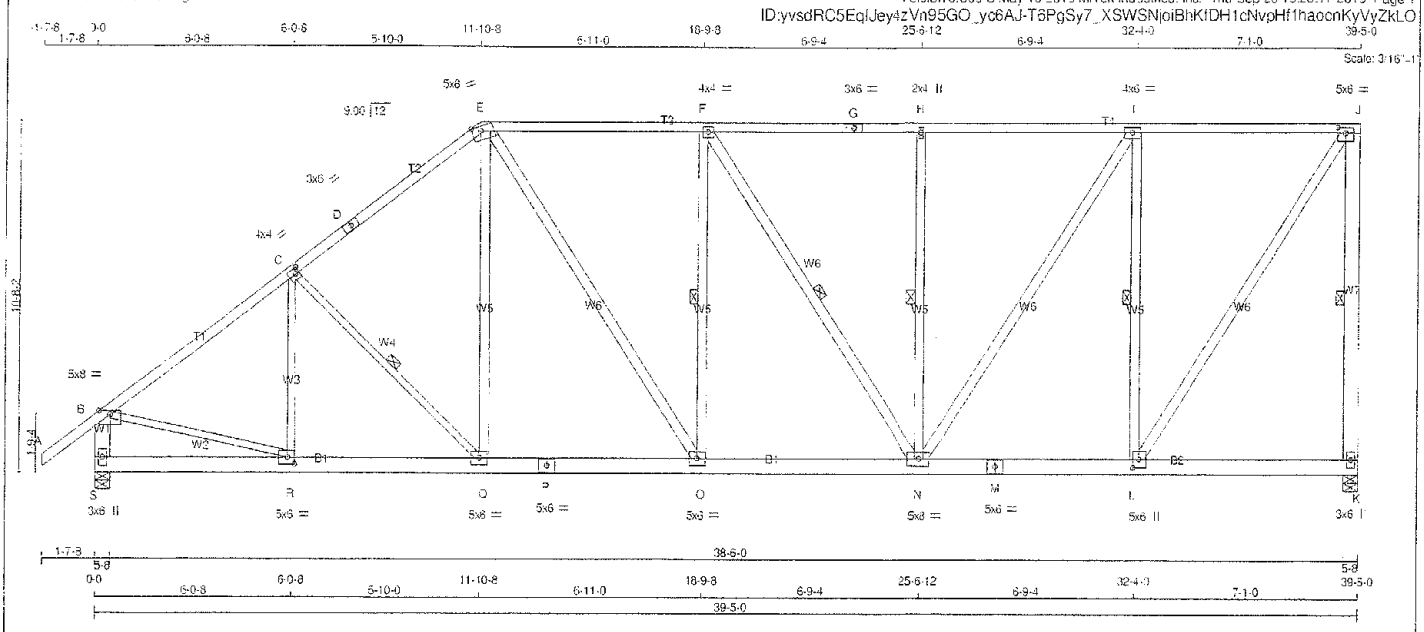


TOTAL WEIGHT = 2 X 257 = 513 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
K - J	2x6	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
R - C	2x3	DRY	No.2	SPF
C - Q	2x3	DRY	No.2	SPF
B - R	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	5.0	8.0	1.75	3.25
F	TMWW-t	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMW+w	MT20	2.0	4.0		
I	TMWW-t	MT20	4.0	6.0		
J	TMVW-t	MT20	5.0	6.0	2.00	3.00
K	BMV1+p	MT20	3.0	6.0		
L	BMWW+t	MT20	5.0	6.0	3.00	2.25
M	BS-t	MT20	5.0	6.0		
N	BMWWW-t	MT20	5.0	8.0		
O	BMWW-t	MT20	5.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWW-t	MT20	5.0	6.0		
R	BMWW-t	MT20	5.0	6.0	2.50	2.50
S	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
K	2173	0	2173	0	0	5-8	5-8
S	2330	0	2330	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	1537	1008 / 0	0 / 0	0 / 0	0 / 0	528 / 0	0 / 0
S	1645	1096 / 0	0 / 0	0 / 0	0 / 0	549 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, S

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF J-K, C-Q, F-O, F-N, H-N, H-L.

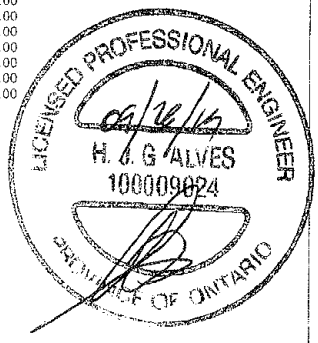
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 47	-91.8	-91.8	0.19 (1)		10.00
B-C	-2406 / 0	-91.8	-91.8	0.88 (1)		3.58
C-D	-2238 / 0	-91.8	-91.8	0.82 (1)		3.71
D-E	-2238 / 0	-91.8	-91.8	0.82 (1)		3.71
E-F	-2125 / 0	-91.8	-91.8	0.89 (1)		3.58
F-G	-1925 / 0	-91.8	-91.8	0.84 (1)		3.76
G-H	-1925 / 0	-91.8	-91.8	0.84 (1)		3.76
H-I	-1925 / 0	-91.8	-91.8	0.86 (1)		3.71
I-J	-1280 / 0	-91.8	-91.8	0.77 (1)		4.49
K-J	-2120 / 0	0.0	0.0	0.74 (1)		5.58
S-B	-2276 / 0	0.0	0.0	0.16 (1)		6.79
S-R	0 / 0	-18.5	-18.5	0.07 (4)		10.00
R-Q	0 / 1956	-18.5	-18.5	0.27 (1)		10.00
Q-P	0 / 1760	-18.5	-18.5	0.25 (1)		10.00
P-O	0 / 1760	-18.5	-18.5	0.25 (1)		10.00
O-N	0 / 2125	-18.5	-18.5	0.29 (1)		10.00
N-M	0 / 1280	-18.5	-18.5	0.21 (1)		10.00
M-L	0 / 1280	-18.5	-18.5	0.21 (1)		10.00
L-K	0 / 0	-18.5	-18.5	0.11 (4)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R-C	-336 / 0	0.20 (1)
C-Q	-261 / 0	0.13 (1)
Q-E	0 / 324	0.05 (1)
E-O	0 / 654	0.11 (1)
O-F	-411 / 0	0.23 (1)
F-N	-365 / 0	0.32 (1)
N-H	-567 / 0	0.31 (1)
N-I	0 / 1174	0.19 (1)
L-I	-1712 / 0	0.94 (1)
L-J	0 / 2259	0.36 (1)
B-R	0 / 2005	0.45 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C:C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00:12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.31")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (1.31")
CALCULATED VERT. DEFL.(TL) = L/999 (0.20")

CSI: TC=0.89/1.00 (E-F:1) , BC=0.29/1.00 (N-O:1) , WB=0.94/1.00 (I-L:1) , SSI=0.31/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1657	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.87 (L) (INPUT = 0.90)
JSI METAL = 0.48 (R) (INPUT = 1.00)

Structural component only
DWG# T-1924952

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405684	T9	1	1	GREENYORK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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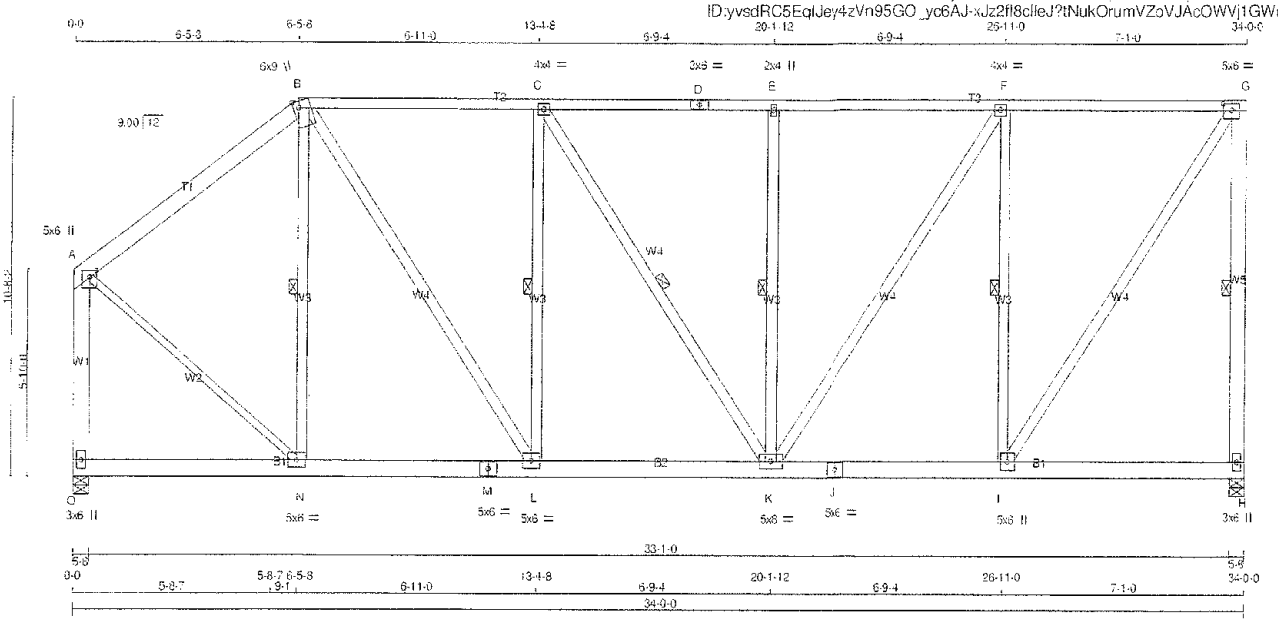

TOTAL WEIGHT = 240 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x6	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
D G	2x4	DRY	No.2	SPF
H - G	2x6	DRY	No.2	SPF
O - A	2x6	DRY	No.2	SPF
O - M	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
A - N	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW+p	MT20	5.0	6.0	2.25	2.25
B	TTWW+m	MT20	6.0	9.0	Edge	2.00
C	TMWW-t	MT20	4.0	4.0		
D	TS-t	MT20	3.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMW-t	MT20	5.0	6.0	2.25	3.00
H	BMV1+p	MT20	3.0	6.0		
I	BMWW+t	MT20	5.0	6.0		
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	5.0	8.0		
L	BMWW-t	MT20	5.0	6.0		
M	BS-t	MT20	5.0	6.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	1874	0	1874	0	0	5-8	5-8
O	1874	0	1874	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1325	870 / 0	0 / 0	0 / 0	0 / 0	456 / 0	0 / 0
O	1325	870 / 0	0 / 0	0 / 0	0 / 0	456 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.15 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-H, B-N, C-L, C-K, E-K, F-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1228 / 0	-91.8	-91.8	0.34 (1)		6.25
B- C	-1526 / 0	-91.8	-91.8	0.78 (1)		4.21
C- D	-1524 / 0	-91.8	-91.8	0.78 (1)		4.21
D- E	-1524 / 0	-91.8	-91.8	0.78 (1)		4.21
E- F	-1524 / 0	-91.8	-91.8	0.80 (1)		4.15
F- G	-1075 / 0	-91.8	-91.8	0.75 (1)		4.83
H- G	-1821 / 0	0.0	0.0	0.64 (1)		5.92
O- A	-1827 / 0	0.0	0.0	0.70 (1)		7.39
O- N	0 / 0	-18.5	-18.5	0.10 (4)		10.00
N- M	0 / 968	-18.5	-18.5	0.17 (1)		10.00
M- L	0 / 968	-18.5	-18.5	0.17 (1)		10.00
L- K	0 / 1526	-18.5	-18.5	0.22 (1)		10.00
K- J	0 / 1075	-18.5	-18.5	0.18 (1)		10.00
J- I	0 / 1075	-18.5	-18.5	0.18 (1)		10.00
I- H	0 / 0	-18.5	-18.5	0.11 (4)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- B	-645 / 0	0.36 (1)
B- L	0 / 997	0.16 (1)
L- C	-702 / 0	0.39 (1)
C- K	-4 / 3	0.00 (1)
K- E	-567 / 0	0.31 (1)
K- F	0 / 818	0.13 (1)
I- F	-1414 / 0	0.78 (1)
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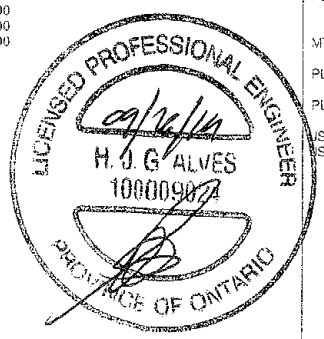
A- N	0 / 1254	0.28 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.13")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (1.13")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.80/1.00 (E-F:1) , BC=0.22/1.00 (K-L:1) , WB=0.78/1.00 (F-I:1) , SSI=0.31/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1356

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

(SI GRIP= 0.85 (F) (INPUT = 0.90)
SI METAL= 0.50 (A) (INPUT = 1.00)

Structural component only
DWG# T-1924953

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405684	T9S	1	1	GREENYORK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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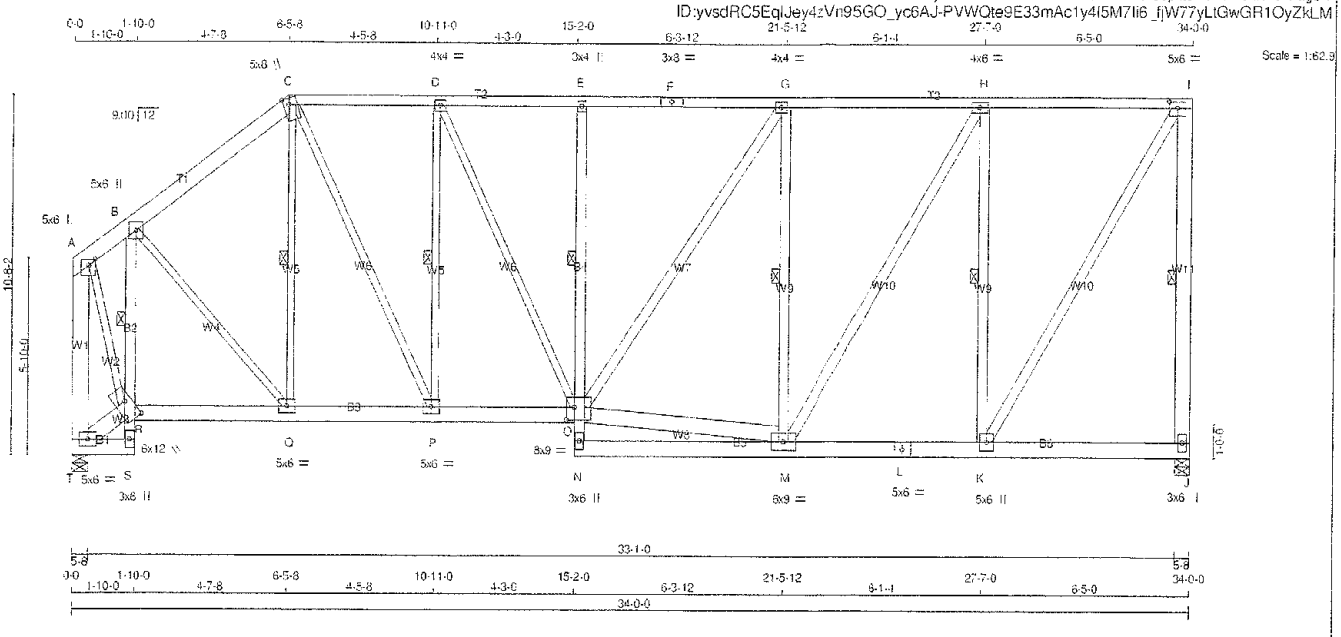


TOTAL WEIGHT = 267 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
J - I	2x6	DRY	No.2	SPF
T - A	2x6	DRY	No.2	SPF
T - S	2x6	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
N - E	2x4	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
M - G	2x4	DRY	No.2	SPF
M - H	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
T - R	2x6	DRY	No.2	SPF
O - M	2x6	DRY	No.2	SPF
O - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	2.25	2.25
B	TMVW+p	MT20	5.0	6.0		
C	TTWW+m	MT20	5.0	8.0	Edge	2.00
D	TMWW-t	MT20	4.0	4.0		
E	TMV+p	MT20	3.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMWW-t	MT20	4.0	4.0		
H	TMWW-t	MT20	4.0	6.0		
I	TMVW-t	MT20	5.0	6.0	2.25	3.00
J	BMV1+p	MT20	3.0	6.0		
K	BMWW+t	MT20	5.0	6.0		
L	BS-t	MT20	5.0	6.0		
M	BMWWW-t	MT20	6.0	9.0		
N	BMV+p	MT20	3.0	6.0		
O	BVMWWW-t	MT20	8.0	9.0	4.50	3.00
P	BMWW-t	MT20	5.0	6.0		
Q	BMWW-t	MT20	5.0	6.0		
R	BVMWW-w	MT20	6.0	12.0	2.25	7.00
S	BMV+p	MT20	3.0	6.0		
T	BMVW1-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT	REQRD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG IN-SX	BRG IN-SX
J	1874	0	1874	0	0	5-8	5-8
T	1874	0	1874	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
J	1325	870 / 0	0 / 0	0 / 0	0 / 0	456 / 0	0 / 0
T	1325	870 / 0	0 / 0	0 / 0	0 / 0	456 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, T

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.54 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 5.46 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF I-J, B-R, E-O, C-O, D-P, G-M, H-K.

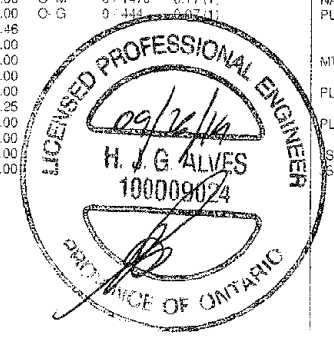
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-649 / 0	-91.8	-91.8	1	0.10 (1)	6.25
B- C	-1371 / 0	-91.8	-91.8	1	0.14 (1)	6.25
C- D	-1511 / 0	-91.8	-91.8	1	0.26 (1)	5.06
D- E	-1719 / 0	-91.8	-91.8	1	0.36 (1)	4.67
E- F	-1717 / 0	-91.8	-91.8	1	0.49 (1)	4.54
F- G	-1717 / 0	-91.8	-91.8	1	0.49 (1)	4.54
G- H	-1464 / 0	-91.8	-91.8	1	0.61 (1)	4.58
H- I	-990 / 0	-91.8	-91.8	1	0.58 (1)	5.38
J- I	-1825 / 0	0.0	0.0	1	0.64 (1)	5.91
T- A	-1844 / 0	0.0	0.0	1	0.71 (1)	7.36
T- S	0 / 25	-18.5	18.5	1	0.01 (4)	10.00
S- R	0 / 17	0.0	0.0	1	0.04 (1)	10.00
R- B	-1398 / 0	0.0	0.0	1	0.10 (1)	5.46
R- Q	0 / 564	-18.5	-18.5	1	0.11 (1)	10.00
Q- P	0 / 1070	-18.5	-18.5	1	0.17 (1)	10.00
P- O	0 / 1512	-18.5	-18.5	1	0.21 (1)	10.00
N- O	0 / 54	0.0	0.0	1	0.03 (1)	10.00
O- E	-489 / 0	0.0	0.0	1	0.08 (1)	6.25
N- M	0 / 20	-18.5	-18.5	1	0.07 (4)	10.00
M- L	0 / 990	-18.5	-18.5	1	0.16 (1)	10.00
L- K	0 / 990	-18.5	-18.5	1	0.16 (1)	10.00
K- J	0 / 0	-18.5	-18.5	1	0.08 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- Q	0 / 777	0.17 (1)
Q- C	-475 / 0	0.28 (1)
C- P	0 / 1017	0.23 (1)
P- D	-844 / 0	0.51 (1)
D- O	0 / 486	0.11 (1)
M- G	-928 / 0	0.51 (1)
M- H	0 / 940	0.15 (1)
K- H	-1481 / 0	0.81 (1)
K- I	0 / 1872	0.30 (1)
T- R	-29 / 0	0.00 (1)
A- R	0 / 1537	0.35 (1)
O- M	0 / 1470	0.17 (1)
O- G	0 / 444	0.07 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.13")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (1.13")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.71/1.00 (A-T:1) , BC=0.21/1.00 (O-P:1) , WB=0.81/1.00 (H-K:1) , SSI=0.28/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.83 (I) (INPUT = 0.90)
SI METAL= 0.39 (K) (INPUT = 1.00)

Structural component only
DWG# T-1924954

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405684	T10S	6	1	GREENYORK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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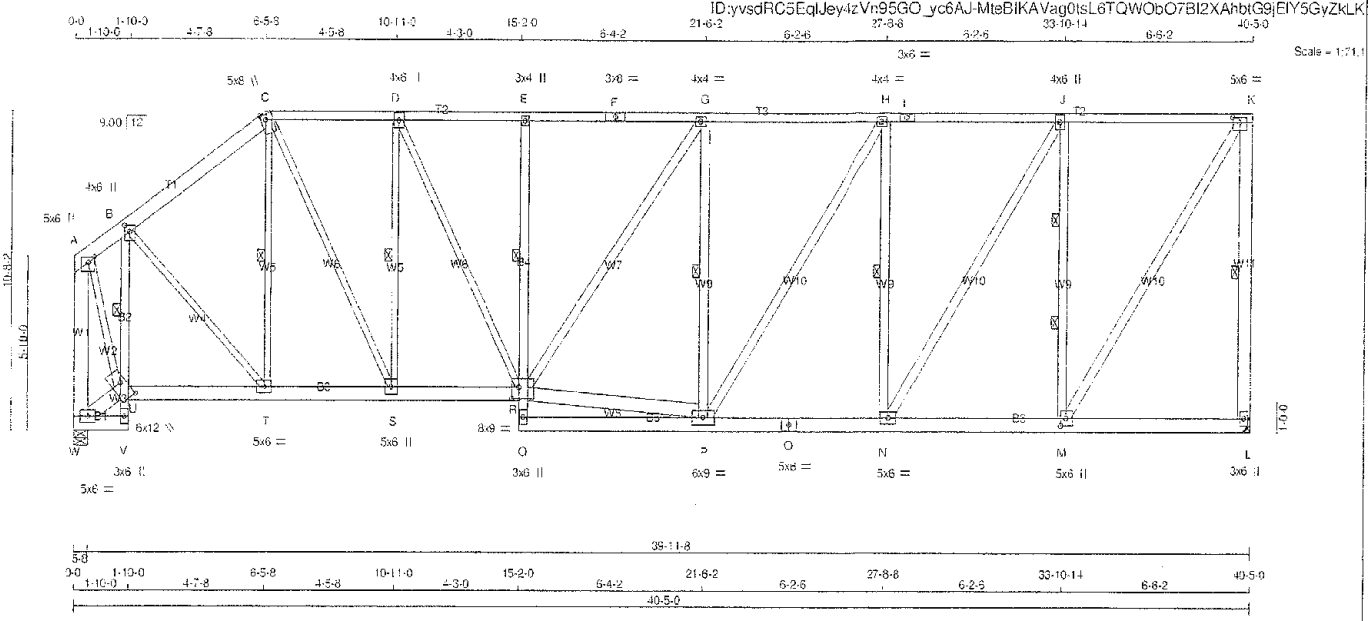


TOTAL WEIGHT = 6 X 311 = 1863 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
L - K	2x6	DRY	No.2	SPF
W - A	2x6	DRY	No.2	SPF
W - V	2x6	DRY	No.2	SPF
V - B	2x4	DRY	No.2	SPF
U - R	2x6	DRY	No.2	SPF
Q - E	2x4	DRY	No.2	SPF
Q - O	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
B - T	2x3	DRY	No.2	SPF
T - C	2x3	DRY	No.2	SPF
C - S	2x3	DRY	No.2	SPF
S - D	2x3	DRY	No.2	SPF
D - R	2x3	DRY	No.2	SPF
W - U	2x6	DRY	No.2	SPF
A - U	2x3	DRY	No.2	SPF
R - P	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	2.25	2.25
B	TMVW+p	MT20	4.0	6.0	2.50	2.00
C	TTWW+m	MT20	5.0	8.0	Edge	2.00
D	TMWW+t	MT20	4.0	6.0		
E	TMV+p	MT20	3.0	4.0		
F	TS+t	MT20	3.0	8.0		
G	TMWW+t	MT20	4.0	4.0		
H	TMWW+t	MT20	4.0	4.0		
I	TS+t	MT20	3.0	6.0		
J	TMWW+t	MT20	4.0	6.0		
K	TMVW+t	MT20	5.0	6.0	2.00	3.00
L	BMV1+p	MT20	3.0	6.0		
M	BMWW+t	MT20	5.0	6.0	3.00	2.25
N	BMWW+t	MT20	5.0	6.0		
O	BS+t	MT20	5.0	6.0		
P	BMWWW+t	MT20	6.0	9.0		
Q	BMV+p	MT20	3.0	6.0		
R	BVMWWW+t	MT20	8.0	9.0	4.50	3.00
S	BMWW+t	MT20	5.0	6.0		
T	BMWW+t	MT20	5.0	6.0		
U	BVMWW+w	MT20	6.0	12.0	2.25	7.00
V	BMV+p	MT20	3.0	6.0		
W	BMVW1+t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT	REQRD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG IN-SX	BRG IN-SX
L	2228	0	2228	0	0	MECHANICAL	
W	2228	0	2228	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT L. MINIMUM BEARING LENGTH AT JOINT L = 3-8.

UNFACTORED REACTIONS

	1ST LOASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1576	1034 / 0	0 / 0	0 / 0	0 / 0	542 / 0	0 / 0
W	1576	1034 / 0	0 / 0	0 / 0	0 / 0	542 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 4.98 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF K-L, B-U, E-R, C-T, D-S, G-P, H-N.
2 LATERAL BRACE(S) AT 1/ 3 LENGTH OF J-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	-782 / 0	-91.8	-91.8	0.09 (1)		6.25
B-C	-1686 / 0	-91.8	-91.8	0.15 (1)		5.93
C-D	-1926 / 0	-91.8	-91.8	0.29 (1)		4.57
D-E	-2300 / 0	-91.8	-91.8	0.39 (1)		4.13
E-F	-2298 / 0	-91.8	-91.8	0.63 (1)		3.88
F-G	-2298 / 0	-91.8	-91.8	0.63 (1)		3.88
G-H	-2209 / 0	-91.8	-91.8	0.62 (1)		3.96
H-I	-1911 / 0	-91.8	-91.8	0.70 (1)		4.02
I-J	-1911 / 0	-91.8	-91.8	0.70 (1)		4.02
J-K	-1226 / 0	-91.8	-91.8	0.63 (1)		4.88
L-K	-2179 / 0	0.0	0.0	0.76 (1)		5.30
W-A	-2195 / 0	0.0	0.0	0.84 (1)		6.61
W-V	0 / 29	-18.5	-18.5	0.01 (4)		10.00
V-U	0 / 17	0.0	0.0	0.04 (1)		10.00
U-B	-1671 / 0	0.0	0.0	0.14 (1)		4.98
U-T	0 / 669	-18.5	-18.5	0.13 (1)		10.00
T-S	0 / 1318	-18.5	-18.5	0.20 (1)		10.00
S-R	0 / 1927	-18.5	-18.5	0.25 (1)		10.00
Q-R	0 / 52	0.0	0.0	0.04 (1)		10.00
R-E	-479 / 0	0.0	0.0	0.09 (1)		6.25
Q-P	0 / 25	-18.5	-18.5	0.08 (4)		10.00
P-O	0 / 1911	-18.5	-18.5	0.26 (1)		10.00
O-N	0 / 1911	-18.5	-18.5	0.26 (1)		10.00
N-M	0 / 1226	-18.5	-18.5	0.19 (1)		10.00
M-L	0 / 0	-18.5	-18.5	0.09 (4)		10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B-T	0 / 997	0.22 (1)
T-C	-637 / 0	0.38 (1)
C-S	0 / 1399	0.31 (1)
S-D	1191 / 0	0.71 (1)
D-R	0 / 875	0.20 (1)
P-G	-721 / 0	0.40 (1)
P-H	0 / 583	0.09 (1)
N-H	-1038 / 0	0.57 (1)
N-J	0 / 1329	0.21 (1)
M-J	-1804 / 0	0.45 (1)
M-K	0 / 2296	0.37 (1)
W-U	-3 / 0	0.00 (1)
A-U	0 / 2218	0.94 (1)
R-E	0 / 2218	0.94 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.35")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (1.35")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.25")

CSI: TC=0.84/1.00 (A-W:1) , BC=0.26/1.00 (N-P:1) , WB=0.71/0.90 (D-S:1) , SSI=0.28/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

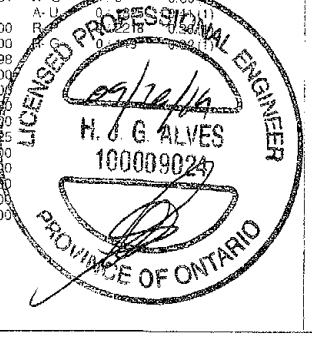
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.48 (B) (INPUT = 1.00)



Structural component only
DWG# T-1924955

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405684	T11	1	2	GREENYORK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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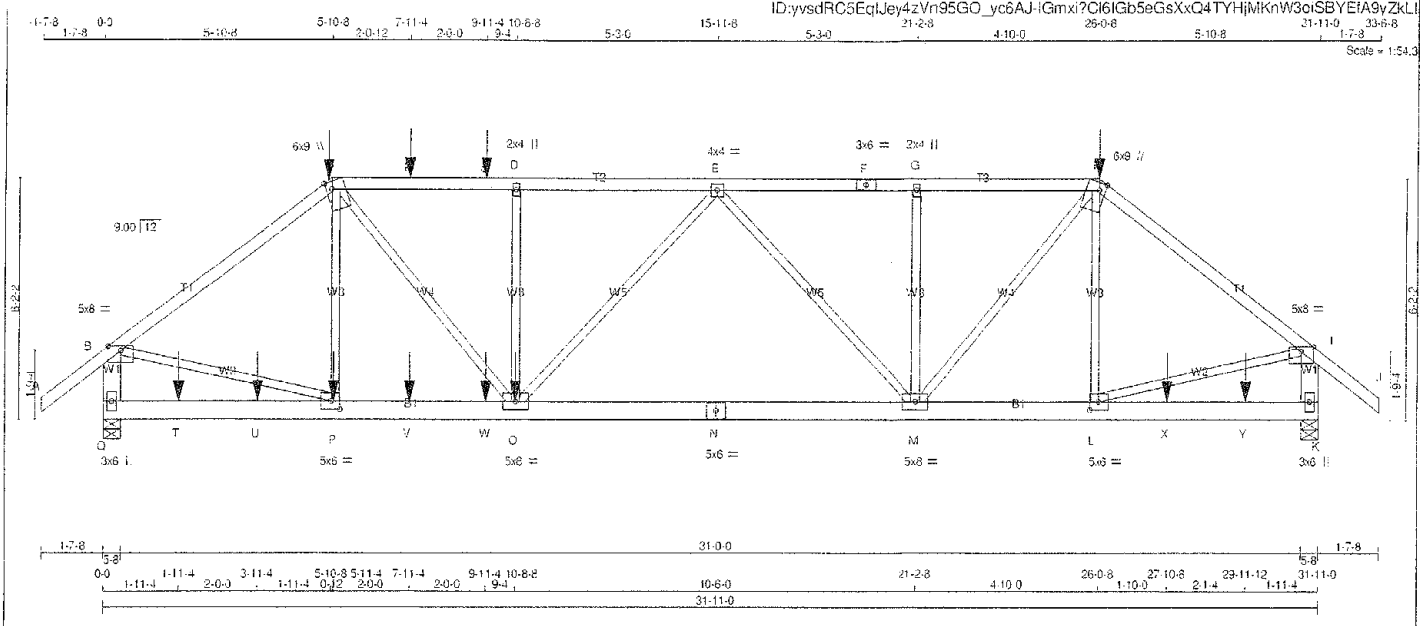

TOTAL WEIGHT = 2 X 157 = 314 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
Q - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
Q - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	SIDE(61.0)
C-F	1	12	SIDE(61.0)
F-H	1	12	SIDE(61.0)
H-J	1	12	SIDE(61.0)
Q-B	2	12	TOP
K-I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
Q-N	2	12	SIDE(133.1)
N-K	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
D-O	1	6	SIDE(258.3)

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	6.0	9.0	Edge	2.00
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW+m	MT20	6.0	9.0	Edge	2.00
I	TMVW-p	MT20	5.0	8.0	Edge	
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.75
M	BMWWW-t	MT20	5.0	8.0		
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BMWW-t	MT20	5.0	6.0	2.50	2.75
Q	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	4064	0	4064	0	0	5-8	5-8
K	4439	0	4439	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
Q	2866	1926 / 0	0 / 0	0 / 0	0 / 0	940 / 0	0 / 0
K	3126	2128 / 0	0 / 0	0 / 0	0 / 0	998 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.68 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 47	-91.8	-91.8	0.10 (1)		10.00
B-C	-4555 / 0	-91.8	-91.8	0.53 (1)		4.08
C-R	-5608 / 0	-91.8	-91.8	0.47 (1)		3.68
R-S	-5608 / 0	-91.8	-91.8	0.47 (1)		3.68
S-D	-5608 / 0	-91.8	-91.8	0.47 (1)		3.68
D-E	-5608 / 0	-91.8	-91.8	0.43 (1)		3.75
E-F	-4614 / 0	-91.8	-91.8	0.29 (1)		4.24
F-G	-4614 / 0	-91.8	-91.8	0.29 (1)		4.24
G-H	-4614 / 0	-91.8	-91.8	0.28 (1)		4.24
H-I	-4541 / 0	-91.8	-91.8	0.53 (1)		4.08
I-J	0 / 47	-91.8	-91.8	0.10 (1)		10.00
Q-B	-3994 / 0	0.0	0.0	0.15 (1)		7.11
K-I	-3983 / 0	0.0	0.0	0.15 (1)		7.11
Q-T	0 / 0	-18.5	-18.5	0.07 (4)		10.00
T-U	0 / 0	-18.5	-18.5	0.07 (4)		10.00
U-P	0 / 0	-18.5	-18.5	0.07 (4)		10.00
P-V	0 / 3627	-18.5	-18.5	0.27 (1)		10.00
V-W	0 / 3627	-18.5	-18.5	0.27 (1)		10.00
W-O	0 / 3627	-18.5	-18.5	0.27 (1)		10.00
O-N	0 / 5310	-18.5	-18.5	0.44 (1)		10.00
N-M	0 / 5310	-18.5	-18.5	0.44 (1)		10.00
M-L	0 / 3643	-18.5	-18.5	0.62 (1)		10.00
L-X	0 / 0	-18.5	-18.5	0.60 (1)		10.00
X-Y	0 / 0	-18.5	-18.5	0.60 (1)		10.00
Y-K	0 / 0	-18.5	-18.5	0.60 (1)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P-C	-633 / 0	0.16 (1)
C-O	0 / 3089	0.38 (1)
O-D	-764 / 0	0.22 (1)
O-E	0 / 444	0.05 (1)
E-M	-1057 / 0	0.65 (1)
M-G	-504 / 0	0.15 (1)
M-H	0 / 1515	0.19 (1)
L-H	0 / 864	0.11 (1)
B-P	0 / 3732	0.46 (1)
L-I	0 / 3720	0.46 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	COMM.
C	5-10-8	-428	-428	---	FRONT	VERT	TOTAL	---	C1
R	26-0-8	-243	-243	---	FRONT	VERT	TOTAL	---	C1
O	10-8-8	-1730	-1730	---	FRONT	VERT	TOTAL	---	C1
P	5-11-4	-36	-36	---	FRONT	VERT	TOTAL	---	C1
R	7-11-4	-178	-178	---	FRONT	VERT	TOTAL	---	C1
S	9-11-4	-178	-178	---	FRONT	VERT	TOTAL	---	C1
T	1-11-4	-23	25	---	FRONT	VERT	TOTAL	---	C1
U	3-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
V	7-11-4	36	-36	---	FRONT	VERT	TOTAL	---	C1
W	9-11-4	36	36	---	FRONT	VERT	TOTAL	---	C1
X	27-10-8	-1730	-1730	---	FRONT	VERT	TOTAL	---	C1
Y	29-11-12	-23	-25	---	FRONT	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.06")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (1.06")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.25")

CSI: TC=0.53/1.00 (B-C,1) , BC=0.62/1.00 (L-M,1) , WB=0.65/1.00 (E-M,1) , SSI=0.47/1.00 (K-L,1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	788	1987	1356

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (O) (INPUT = 0.90)
JSI METAL= 0.57 (N) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
09/26/19
H. J. G. ALVES
100089024
PROVINCE OF ONTARIO

Structural component only
DWG# T-1924956

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405684	T12	1	1	GREENYORK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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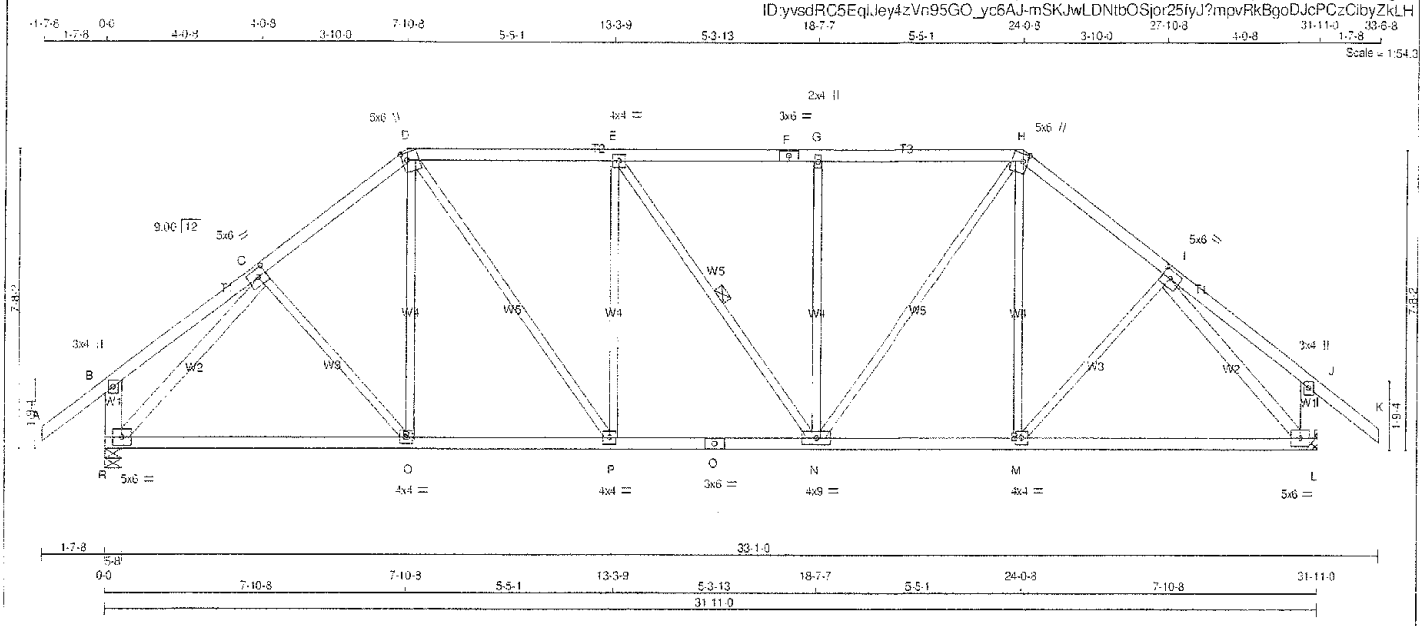


TOTAL WEIGHT = 154 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
R - C	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.75
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW+m	MT20	5.0	6.0	2.25	1.50
I	TMWW-t	MT20	5.0	6.0	2.50	2.75
J	TMV+p	MT20	3.0	4.0		
L	BMVW+t	MT20	5.0	6.0		
M, P, Q						
M	BMWW-t	MT20	4.0	4.0		
N	BMWWW-t	MT20	4.0	9.0		
O	BS-t	MT20	3.0	6.0		
R	BMVW+t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
R	1917	0	1917	0	0	5-8	5-8
L	1917	0	1917	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT L. MINIMUM BEARING LENGTH AT JOINT L = 3-8.

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1352	904 / 0	0 / 0	0 / 0	0 / 0	448 / 0	0 / 0
L	1352	904 / 0	0 / 0	0 / 0	0 / 0	448 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.40 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 47	-91.8	-91.8	0.19 (1)	10.00	
B-C	0 / 25	-91.8	-91.8	0.22 (1)	10.00	
C-D	-1816 / 0	-91.8	-91.8	0.28 (1)	4.69	
D-E	-1867 / 0	-91.8	-91.8	0.45 (1)	4.40	
E-F	-1885 / 0	-91.8	-91.8	0.45 (1)	4.41	
F-G	-1885 / 0	-91.8	-91.8	0.45 (1)	4.41	
G-H	-1885 / 0	-91.8	-91.8	0.44 (1)	4.42	
H-I	-1816 / 0	-91.8	-91.8	0.28 (1)	4.69	
I-J	0 / 25	-91.8	-91.8	0.22 (1)	10.00	
J-K	0 / 47	-91.8	-91.8	0.19 (1)	10.00	
R-B	-298 / 0	0.0	0.0	0.02 (1)	7.81	
L-J	-298 / 0	0.0	0.0	0.02 (1)	7.81	
R-Q	0 / 1394	-18.5	-18.5	0.36 (1)	10.00	
Q-P	0 / 1432	-18.5	-18.5	0.37 (4)	10.00	
P-O	0 / 1887	-18.5	-18.5	0.36 (1)	10.00	
O-N	0 / 1887	-18.5	-18.5	0.36 (1)	10.00	
N-M	0 / 1432	-18.5	-18.5	0.37 (4)	10.00	
M-L	0 / 1396	-18.5	-18.5	0.37 (1)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-Q	0 / 68	0.02 (4)
Q-D	0 / 119	0.04 (4)
D-P	0 / 768	0.17 (1)
P-E	-533 / 0	0.56 (1)
E-N	-3 / 0	0.00 (1)
N-G	-532 / 0	0.56 (1)
N-H	0 / 765	0.17 (1)
M-H	0 / 119	0.04 (4)
M-I	0 / 68	0.02 (4)
R-C	-2092 / 0	0.82 (1)
I-L	-2092 / 0	0.82 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00:12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(65 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.06")
CALCULATED VERT. DEFL.(LL) = L 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (1.06")
CALCULATED VERT. DEFL.(TL) = L 999 (0.16")

CSI: TC=0.45/1.00 (D-E:1) , BC=0.37/1.00 (M-N:4) , WB=0.82/1.00 (I-L:1) , SSI=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

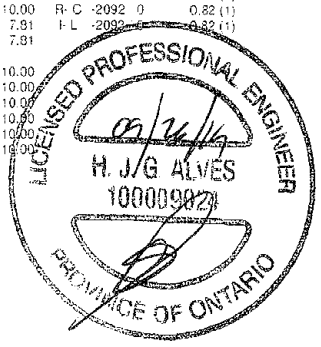
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (H) (INPUT = 0.90) JSI METAL= 0.64 (O) (INPUT = 1.00)



Structural component only
DWG# T-1924957

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405684	T13	1	1	GREENYORK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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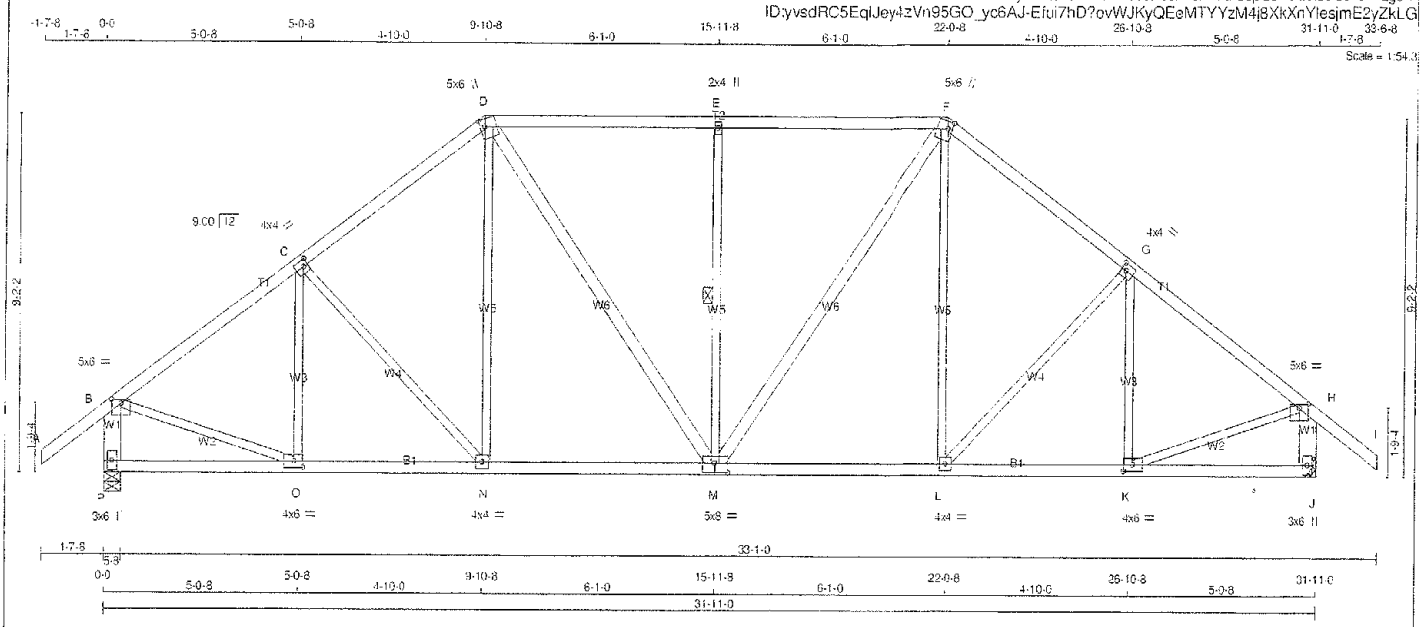


TOTAL WEIGHT = 160 lb

[MiTek]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
P - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - M	2x4	DRY	No.2	SPF
M - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.25	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW-p	MT20	5.0	6.0	1.25	3.00
J	BMV1+p	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	6.0	2.00	2.75
L	BMWW-t	MT20	4.0	4.0		
M	BSWWW-t	MT20	5.0	8.0	3.00	4.00
N	BMWW-t	MT20	4.0	4.0		
O	BMWW-t	MT20	4.0	6.0	2.00	2.75
P	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT	REQRD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	BRG
P	1917	0	1917	0	0	5-8	5-8
J	1917	0	1917	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 3-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
P	1352	904 / 0	0 / 0	0 / 0	0 / 0	448 / 0	0 / 0
J	1352	904 / 0	0 / 0	0 / 0	0 / 0	448 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.63 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-M.

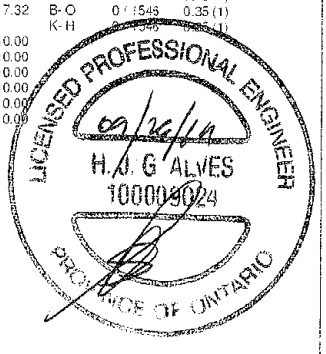
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	LC	MAX. UNBRAC LENGTH
A-B	0 / 47	-91.8	-91.8	0.19	(1)		10.00
B-C	-1828 / 0	-91.8	-91.8	0.34	(1)		4.63
C-D	-1733 / 0	-91.8	-91.8	0.33	(1)		4.74
D-E	-1826 / 0	-91.8	-91.8	0.48	(1)		4.63
E-F	-1828 / 0	-91.8	-91.8	0.48	(1)		4.63
F-G	-1733 / 0	-91.8	-91.8	0.33	(1)		4.74
G-H	-1825 / 0	-91.8	-91.8	0.34	(1)		4.63
H-I	0 / 47	-91.8	-91.8	0.19	(1)		10.00
P-B	-1876 / 0	0.0	0.0	0.13	(1)		7.32
J-H	-1876 / 0	0.0	0.0	0.13	(1)		7.32
P-O	0 / 0	-18.5	-18.5	0.10	(4)		10.00
O-N	0 / 1486	-18.5	-18.5	0.32	(1)		10.00
N-M	0 / 1361	-18.5	-18.5	0.30	(1)		10.00
M-L	0 / 1361	-18.5	-18.5	0.30	(1)		10.00
L-K	0 / 1486	-18.5	-18.5	0.32	(1)		10.00
K-J	0 / 0	-18.5	-18.5	0.10	(4)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI	LC
O-C	-333 / 0	0.16	(1)
C-N	-187 / 0	0.19	(1)
N-D	0 / 253	0.06	(4)
D-M	0 / 470	0.08	(1)
M-E	-684 / 0	0.38	(1)
M-F	0 / 470	0.08	(1)
L-F	0 / 253	0.06	(4)
L-G	-187 / 0	0.19	(1)
K-G	-333 / 0	0.16	(1)
B-O	0 / 1546	0.35	(1)
K-H	0 / 1546	0.35	(1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00:12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.06")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (1.06")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.48/1.00 (D-E:1) , BC=0.32/1.00 (K-L:1) , WB=0.38/1.00 (E-M:1) , SSI=0.27/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (O) (INPUT = 0.90)
JSI METAL= 0.41 (H) (INPUT = 1.00)

Structural component only
DWG# T-1924958

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405684	T14	1	1	GREENYORK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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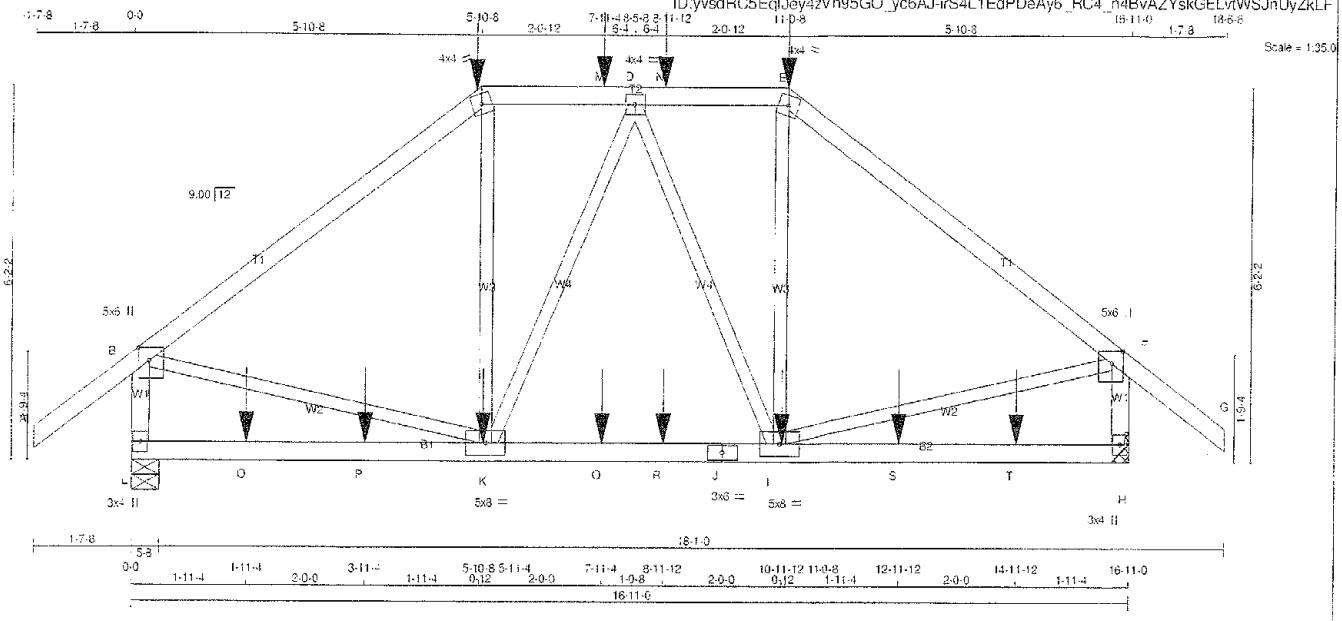

TOTAL WEIGHT = 78 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.50	2.25
C	TTW-m	MT20	4.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	4.0		
F	TMVW+p	MT20	5.0	6.0	2.50	2.25
H	BMV1+p	MT20	3.0	4.0		
I	BMWWW-t	MT20	5.0	8.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	5.0	8.0		
L	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
L	1819	0	1819	0	0	IN-SX	IN-SX
H	1819	0	1819	0	0	5-8	5-8
						MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1285	850 / 0	0 / 0	0 / 0	0 / 0	435 / 0	0 / 0
H	1285	850 / 0	0 / 0	0 / 0	0 / 0	435 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.06 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	(LC)	MAX. UNBRAC LENGTH
A-B	0 / 47	-91.8	-91.8	0.21	(1)		10.00
B-C	-1673 / 0	-91.8	-91.8	0.79	(1)		4.06
C-M	-1341 / 0	-91.8	-91.8	0.21	(1)		5.32
M-D	-1341 / 0	-91.8	-91.8	0.21	(1)		5.32
D-N	-1341 / 0	-91.8	-91.8	0.21	(1)		5.32
N-E	-1341 / 0	-91.8	-91.8	0.21	(1)		5.32
E-F	-1673 / 0	-91.8	-91.8	0.79	(1)		4.06
F-G	0 / 47	-91.8	-91.8	0.21	(1)		10.00
L-B	-1753 / 0	0.0	0.0	0.21	(1)		6.26
H-F	-1753 / 0	0.0	0.0	0.21	(1)		6.26
L-O	0 / 0	-18.5	-18.5	0.27	(4)		10.00
O-P	0 / 0	-18.5	-18.5	0.27	(4)		10.00
P-K	0 / 0	-18.5	-18.5	0.27	(4)		10.00
K-Q	0 / 1465	-18.5	-18.5	0.40	(1)		10.00
Q-R	0 / 1465	-18.5	-18.5	0.40	(1)		10.00
R-J	0 / 1465	-18.5	-18.5	0.40	(1)		10.00
J-I	0 / 1465	-18.5	-18.5	0.40	(1)		10.00
I-S	0 / 0	-18.5	-18.5	0.27	(4)		10.00
S-T	0 / 0	-18.5	-18.5	0.27	(4)		10.00
T-H	0 / 0	-18.5	-18.5	0.27	(4)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI	(LC)
K-C	0 / 193	0.07	(4)
I-E	0 / 193	0.07	(4)
B-K	0 / 1374	0.34	(1)
I-F	0 / 1374	0.34	(1)
K-D	-323 / 0	0.24	(1)
D-I	-323 / 0	0.24	(1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-8	-428	-428	---	FRONT	VERT	TOTAL	---	C1
E	11-0-8	-428	-428	---	FRONT	VERT	TOTAL	---	C1
I	10-11-12	-36	-36	---	FRONT	VERT	TOTAL	---	C1
K	5-11-4	-36	-36	---	FRONT	VERT	TOTAL	---	C1
M	7-11-4	-178	-178	---	FRONT	VERT	TOTAL	---	C1
N	8-11-12	-178	-178	---	FRONT	VERT	TOTAL	---	C1
O	1-11-4	-23	-25	---	FRONT	VERT	TOTAL	---	C1
P	3-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
Q	7-11-4	-36	-36	---	FRONT	VERT	TOTAL	---	C1
R	8-11-12	-36	-36	---	FRONT	VERT	TOTAL	---	C1
S	12-11-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
T	14-11-12	-23	-25	---	FRONT	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.56")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.56")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.79/1.00 (B-C:1) , BC=0.40/1.00 (I-K:1) , WB=0.34/1.00 (F-I:1) , SSI=0.28/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)		SHEAR (PSI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.87 (E) (INPUT = 0.90)
JSI METAL = 0.49 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
09/16/14
H. J/G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-1924959

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405684	T15	1	1	GREENYORK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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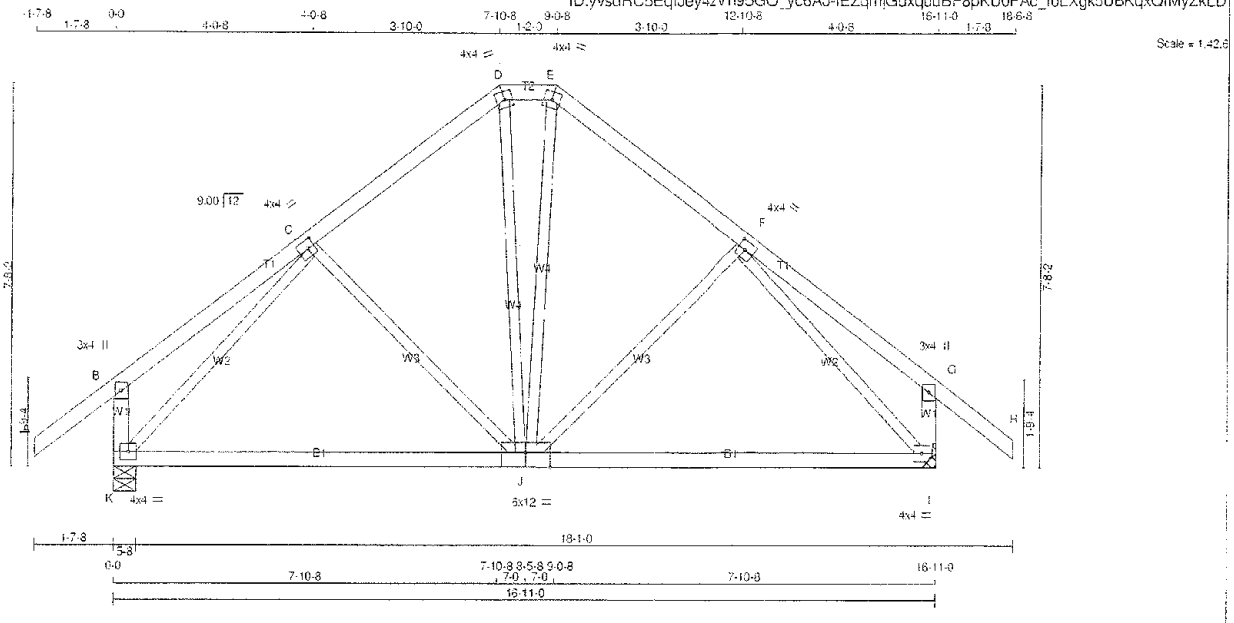


TOTAL WEIGHT = 81 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW+t	MT20	4.0	4.0	2.00	1.75
D	TTW-m	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	4.0		
F	TMWW+t	MT20	4.0	4.0	2.00	1.75
G	TMV+p	MT20	3.0	4.0		
I	BMVW1+t	MT20	4.0	4.0		
J	BSWWWW+t	MT20	6.0	12.0	Edge	6.00
K	BMVW1+t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
K	1090	0	1090	0	0	5-8	5-8
I	1090	0	1090	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	768	520 / 0	0 / 0	0 / 0	0 / 0	247 / 0	0 / 0
I	768	520 / 0	0 / 0	0 / 0	0 / 0	247 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

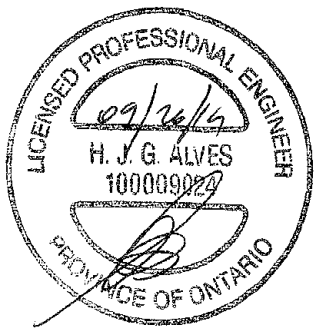
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH
A-B	0 / 47	-91.8	-91.8	0.19 (1)	10.00	
B-C	0 / 26	-91.8	-91.8	0.23 (1)	10.00	
C-D	-673 / 0	-91.8	-91.8	0.18 (1)	6.25	
D-E	-532 / 0	-91.8	-91.8	0.02 (1)	6.25	
E-F	-673 / 0	-91.8	-91.8	0.18 (1)	6.25	
F-G	0 / 26	-91.8	-91.8	0.23 (1)	10.00	
G-H	0 / 47	-91.8	-91.8	0.19 (1)	10.00	
K-B	-294 / 0	0.0	0.0	0.03 (1)	7.81	
I-G	-294 / 0	0.0	0.0	0.03 (1)	7.81	
K-J	0 / 642	-18.5	-18.5	0.43 (4)	10.00	
J-I	0 / 642	-18.5	-18.5	0.43 (4)	10.00	

WEBS MEMB. FR-TO	MAX. FORCE (LBS)	MAX CSI (LC)
C-J	-172 / 0	0.12 (1)
J-F	-172 / 0	0.12 (1)
K-C	-963 / 0	0.56 (1)
F-I	-963 / 0	0.56 (1)
D-J	0 / 198	0.04 (1)
J-E	0 / 198	0.04 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00:12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(65 % OF 31.3 P.S.F. G.S.L. PLUS 8.1 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.56")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.56")
CALCULATED VERT. DEFL.(TL) = L/ 823 (0.25")

CSI: TC=0.23(1.00 (B-C:1) , BC=0.43(1.00 (J-K:4) , WB=0.56(1.00 (C-K:1) , SSI=0.14/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS SEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (F) (INPUT = 0.90)
JSI METAL= 0.34 (C) (INPUT = 1.00)

Structural component only
DWG# T-1924960

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405684	T16	8	1	GREENYORK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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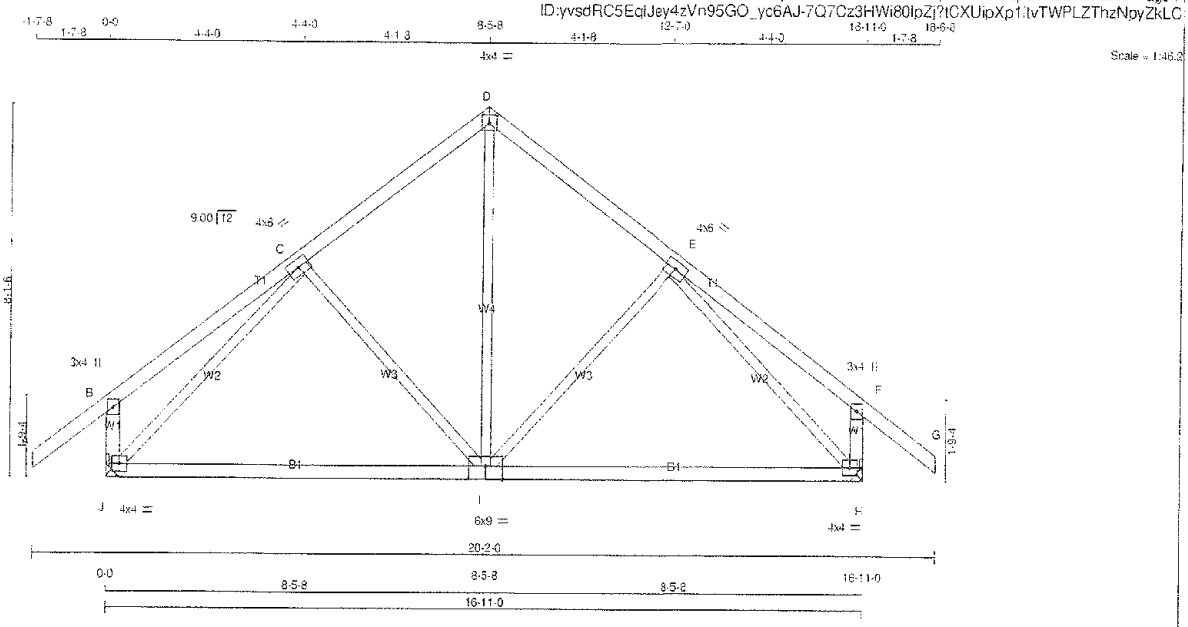


TOTAL WEIGHT = 8 X 77 = 615 lb (MiTek)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-I	MT20	4.0	6.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMWW-I	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-I	MT20	4.0	4.0		
I	BSWWW-I	MT20	6.0	9.0	Edge	4.50
J	BMVW1-I	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	1090	0	1090	0	0	MECHANICAL	
H	1090	0	1090	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J, H. MINIMUM BEARING LENGTH AT JOINT J = 3-8, JOINT H = 3-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	768	520 / 0	0 / 0	0 / 0	0 / 0	247 / 0	0 / 0
H	768	520 / 0	0 / 0	0 / 0	0 / 0	247 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	(LC)	MAX. UNBRAC LENGTH
A-B	0 / 47	-91.8	-91.8	3	0.19	(1)	10.00
B-C	0 / 28	-91.8	-91.8	3	0.27	(1)	10.00
C-D	-660 / 0	-91.8	-91.8	3	0.21	(1)	5.25
D-E	-660 / 0	-91.8	-91.8	3	0.21	(1)	5.25
E-F	0 / 28	-91.8	-91.8	3	0.27	(1)	10.00
F-G	0 / 47	-91.8	-91.8	3	0.19	(1)	10.00
J-B	-304 / 0	0.0	0.0	3	0.03	(1)	7.81
H-F	-304 / 0	0.0	0.0	3	0.03	(1)	7.81
J-I	0 / 647	-18.5	-18.5	3	0.43	(4)	10.00
I-H	0 / 647	-18.5	-18.5	3	0.43	(4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI	(LC)
I-D	0 / 461	0.10	(1)
I-E	-207 / 0	0.14	(1)
C-I	-207 / 0	0.14	(1)
J-C	-958 / 0	0.64	(1)
E-H	-958 / 0	0.64	(1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.56")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.56")
CALCULATED VERT. DEFL.(TL) = L/ 816 (0.25")

CSI: TC=0.27/1.00 (B-C:1) , BC=0.43/1.00 (I-J:4) , WB=0.64/1.00 (E-H:1) , SSI=0.15/1.00 (C-D:1)

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

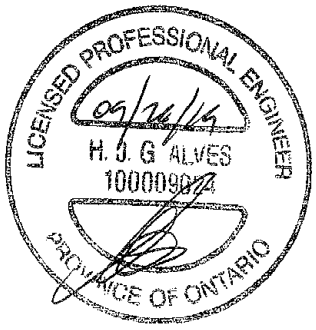
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.83 (D) (INPUT = 0.90)
JSI METAL = 0.24 (J) (INPUT = 1.00)



Structural component only
DWG# T-1924961

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405684	T17	1	1	GREENYORK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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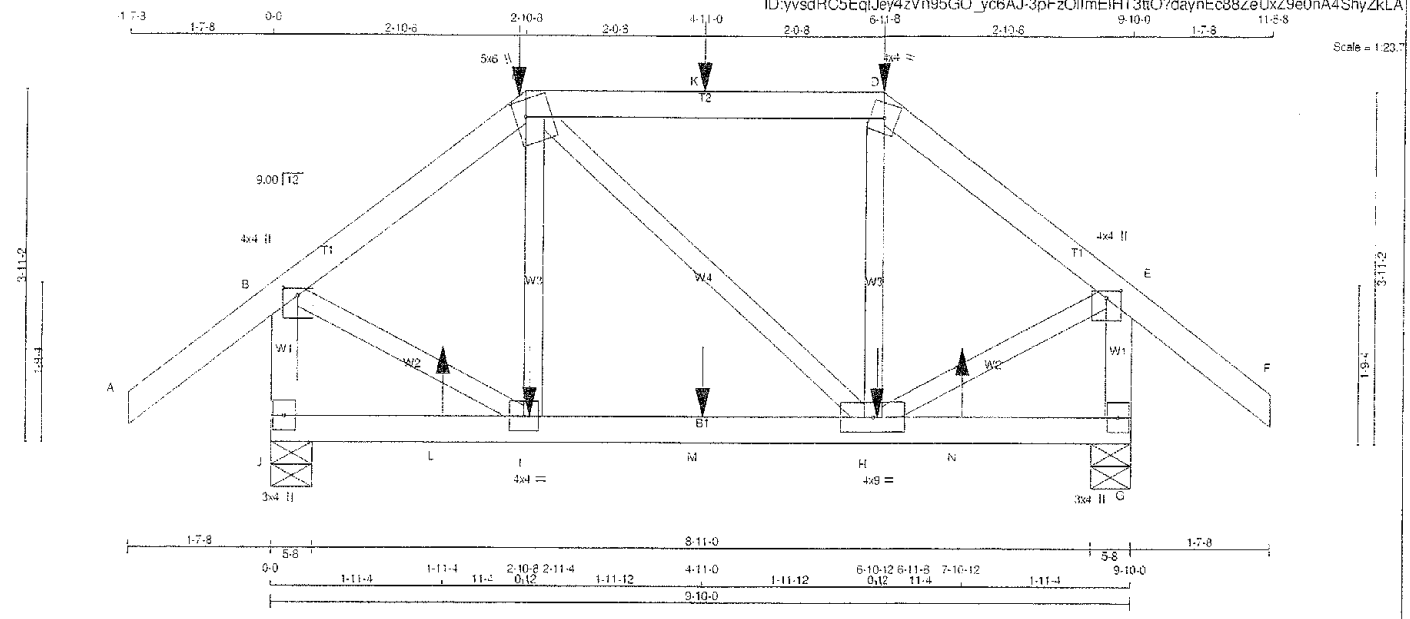

TOTAL WEIGHT = 46 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	4.0	1.00	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	814	0	814	0	0	5-8	5-8
G	814	0	814	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX. MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	574	389 / 0	0 / 0	0 / 0	0 / 0	185 / 0	0 / 0
G	574	389 / 0	0 / 0	0 / 0	0 / 0	185 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 47	-91.8	-91.8	0.21	(1)	10.00	I- C	-118 / 26	0.03 (1)
B- C	-512 / 0	-91.8	-91.8	0.15	(1)	6.25	C- H	0 / 0	0.00 (1)
C- K	-404 / 0	-91.8	-91.8	0.35	(1)	6.25	H- D	-119 / 26	0.03 (1)
K- D	-404 / 0	-91.8	-91.8	0.35	(1)	6.25	B- I	0 / 453	0.11 (1)
D- E	-511 / 0	-91.8	-91.8	0.15	(1)	6.25	H- E	0 / 453	0.11 (1)
E- F	0 / 47	-91.8	-91.8	0.21	(1)	10.00			
J- B	-795 / 0	0.0	0.0	0.09	(1)	7.81			
G- E	-795 / 0	0.0	0.0	0.09	(1)	7.81			
J- L	0 / 0	-18.5	-18.5	0.06	(4)	10.00			
L- I	0 / 0	-18.5	-18.5	0.06	(4)	10.00			
I- M	0 / 405	-18.5	-18.5	0.11	(1)	10.00			
M- H	0 / 405	-18.5	-18.5	0.11	(1)	10.00			
H- N	0 / 0	-18.5	-18.5	0.06	(4)	10.00			
N- G	0 / 0	-18.5	-18.5	0.06	(4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-10-8	-88	-88	155	FRONT	VERT	TOTAL	---	C1
D	6-11-8	-88	-88	155	FRONT	VERT	TOTAL	---	C1
H	6-10-12	-8	-8	---	FRONT	VERT	TOTAL	---	C1
I	2-11-4	-8	-8	---	FRONT	VERT	TOTAL	---	C1
K	4-11-0	-40	-40	---	FRONT	VERT	TOTAL	---	C1
L	1-11-4	5	1	5	FRONT	VERT	TOTAL	---	C1
M	4-11-0	-8	-8	---	FRONT	VERT	TOTAL	---	C1
N	7-10-12	5	1	5	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.33")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.33")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.35/1.00 (C-D:1) , BC=0.11/1.00 (B-I:1) , WB=0.11/1.00 (B-I:1) , SSI=0.18/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	783	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (B) (INPUT = 0.90)
JSI METAL= 0.18 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/10/19
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-1924962

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405684	T18	1	2	GREENYORK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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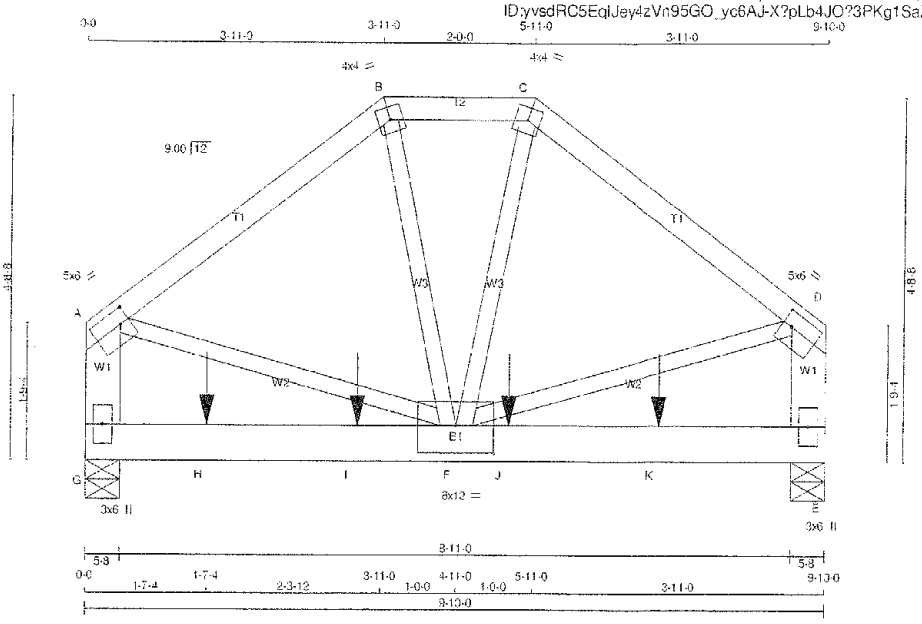

TOTAL WEIGHT = 2 X 50 = 100 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
G - A	2x6	DRY	No.2	SPF
E - D	2x6	DRY	No.2	SPF
G - E	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- B	1	12	TOP
B- C	1	12	TOP
C- D	1	12	TOP
G- A	2	12	TOP
E- D	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
G- E	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0	2.25	1.50
B	TTW-m	MT20	4.0	4.0		
C	TTW-m	MT20	4.0	4.0		
D	TMVW-t	MT20	5.0	6.0	2.25	1.50
E	BMV1+p	MT20	3.0	5.0		
F	BMWWWW'mMT20		8.0	12.0	4.25	6.00
G	BMV1+p	MT20	3.0	5.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	3431	0	3431	0	0	5-8	5-8
E	2667	0	2667	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
G	2418	1635 / 0	0 / 0	0 / 0	0 / 0	783 / 0	0 / 0
E	1878	1280 / 0	0 / 0	0 / 0	0 / 0	598 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.53 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-2516 / 0	-91.8	-91.8	0.17 (1)		5.53	A- F	0 / 2088	0.26 (1)
B- C	-2285 / 0	-91.8	-91.8	0.05 (1)		5.88	F- D	0 / 2088	0.26 (1)
C- D	-2516 / 0	-91.8	-91.8	0.17 (1)		5.53	B- F	0 / 1270	0.16 (1)
G- A	-2248 / 0	0.0	0.0	0.08 (1)		7.81	F- C	0 / 1270	0.16 (1)
E- D	-2248 / 0	0.0	0.0	0.08 (1)		7.81			
G- H	0 / 0	-18.5	-18.5	0.69 (1)		10.00			
H- I	0 / 0	-18.5	-18.5	0.69 (1)		10.00			
I- F	0 / 0	-18.5	-18.5	0.69 (1)		10.00			
F- J	0 / 0	-18.5	-18.5	0.65 (1)		10.00			
J- K	0 / 0	-18.5	-18.5	0.65 (1)		10.00			
K- E	0 / 0	-18.5	-18.5	0.65 (1)		10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
H	1-7-4	-1800	-1800	---	BACK	VERT	TOTAL	---	C1
I	3-7-4	-1071	-1071	---	BACK	VERT	TOTAL	---	C1
J	5-7-4	-1071	-1071	---	BACK	VERT	TOTAL	---	C1
K	7-7-4	-1071	-1071	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1. C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.33")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.33")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.17/1.00 (A-B:1) , BC=0.69/1.00 (F-G:1) , WB=0.26/1.00 (A-F:1) , SSI=0.30/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg

JOINT GRIP= 0.88 (D) (INPUT = 0.90)
JOINT METAL= 0.33 (A) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
ONTARIO

Structural component only
DWG# T-1924963

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405684	T20	1	2	GREENYORK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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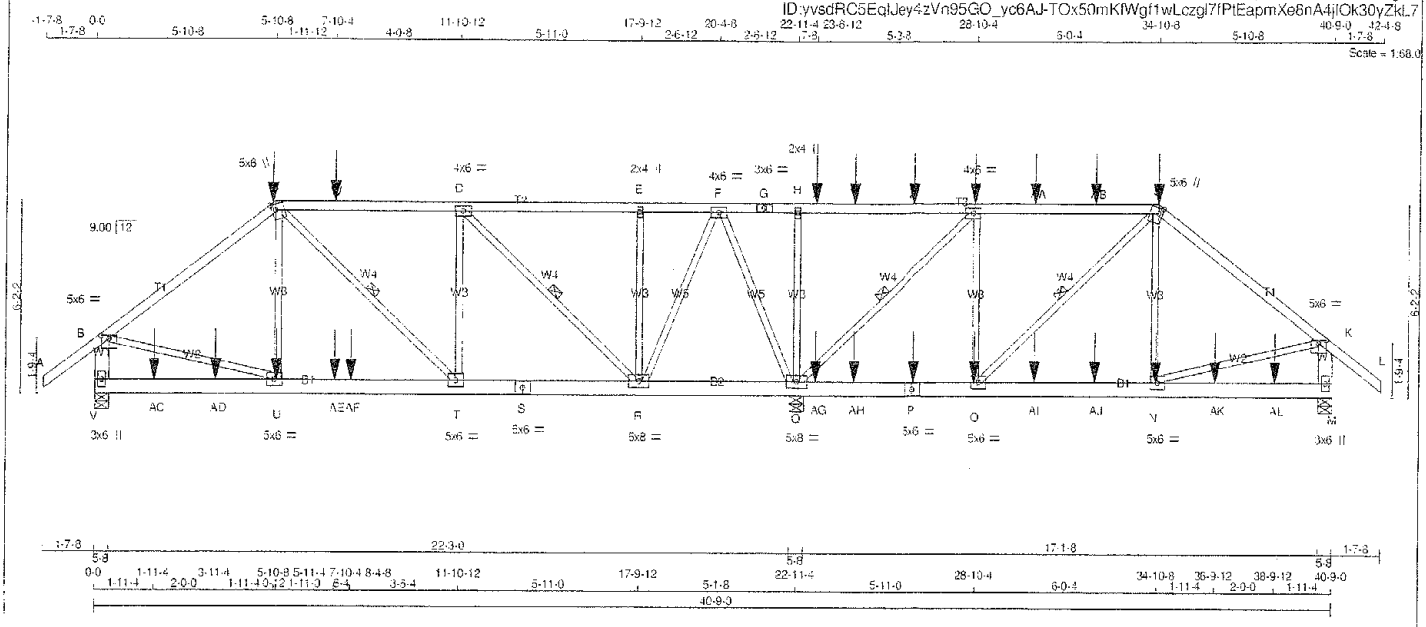


TOTAL WEIGHT = 2 X 204 = 407 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
V - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
V - S	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	SIDE(61.0)
C-G	1	12	SIDE(61.0)
G-J	1	12	SIDE(61.0)
J-L	1	12	SIDE(61.0)
V-B	2	12	TOP
M-K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
V-S	2	12	SIDE(183.1)
S-P	2	12	SIDE(0.0)
P-M	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.25	3.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D, F, I						
D	TMWW-t	MT20	4.0	6.0		
E	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMW+w	MT20	2.0	4.0		
J	TTWW+m	MT20	5.0	6.0	2.25	1.50
K	TMVW-p	MT20	5.0	6.0	1.25	3.00
M	BMV1+p	MT20	3.0	6.0		
N, O, T, U						
N	BMWW-t	MT20	5.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWWW1-t	MT20	5.0	8.0		
R	BMWWW-t	MT20	5.0	8.0		
S	BS-t	MT20	5.0	6.0		
V	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REORD BRG IN-SX
V	2446	0	2446	0	0	5-8	5-8
Q	4531	0	4531	0	0	5-8	5-8
M	1530	0	1530	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX. MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	1728	1147 / 0	0 / 0	0 0	0 / 0	581 / 0	0 / 0
Q	3200	2123 / 0	0 / 0	0 / 0	0 0	1077 / 0	0 / 0
M	1082	709 / 0	0 / 0	0 / 0	0 0	374 / 0	0 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, Q, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.21 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-T, D-R, I-Q, J-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-91.8	-91.8	0.10 (1)	10.00
B- C	-2603 / 0	-91.8	-91.8	0.39 (1)	5.21
C- W	-2091 / 0	-91.8	-91.8	0.40 (1)	5.60
W- D	-2091 / 0	-91.8	-91.8	0.40 (1)	5.60
D- E	-654 / 0	-91.8	-91.8	0.34 (1)	6.25
E- F	-654 / 0	-91.8	-91.8	0.11 (1)	6.25
F- G	0 / 874	-91.8	-91.8	0.39 (1)	10.00
G- H	0 / 874	-91.8	-91.8	0.39 (1)	10.00
H- X	0 / 874	-91.8	-91.8	0.58 (1)	10.00
X- Y	0 / 874	-91.8	-91.8	0.58 (1)	10.00
Y- Z	0 / 874	-91.8	-91.8	0.58 (1)	10.00
Z- I	0 / 874	-91.8	-91.8	0.58 (1)	10.00
I-AA	812 / 0	-91.8	-91.8	0.52 (1)	6.25
AA-AB	812 / 0	-91.8	-91.8	0.52 (1)	6.25
AB- J	812 / 0	-91.8	-91.8	0.52 (1)	6.25
J- K	-1327 / 0	-91.8	-91.8	0.33 (1)	6.25
K- L	0 / 47	-91.8	-91.8	0.10 (1)	10.00
V- B	-2465 / 0	0.0	0.0	0.09 (1)	7.81
M- K	-1466 / 0	0.0	0.0	0.05 (1)	7.81
V-AC	0 / 0	-18.5	-18.5	0.22 (1)	10.00
AC-AD	0 / 0	-18.5	-18.5	0.22 (1)	10.00
AD- U	0 / 0	-18.5	-18.5	0.22 (1)	10.00
U-AE	0 / 2089	-18.5	-18.5	0.59 (1)	10.00
AE-AF	0 / 2089	-18.5	-18.5	0.59 (1)	10.00
AF- T	0 / 2089	-18.5	-18.5	0.59 (1)	10.00
T- S	0 / 2091	-18.5	-18.5	0.32 (1)	10.00
S- R	0 / 2091	-18.5	-18.5	0.32 (1)	10.00
R- Q	-114 / 0	-18.5	-18.5	0.10 (1)	6.25
Q-AG	0 / 812	-18.5	-18.5	0.15 (1)	10.00
AG-AH	0 / 812	-18.5	-18.5	0.15 (1)	10.00
AH- P	0 / 812	-18.5	-18.5	0.15 (1)	10.00
P- O	0 / 812	-18.5	-18.5	0.15 (1)	10.00
O-AI	0 / 1060	-18.5	-18.5	0.11 (4)	10.00
AI-AJ	0 / 1060	-18.5	-18.5	0.11 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
U- C	0 / 537	0.07 (1)
T- D	-13 / 3	0.00 (4)
T- D	0 / 687	0.09 (1)
D- R	-2012 / 0	0.47 (1)
R- E	-415 / 0	0.12 (1)
R- F	0 / 1902	0.24 (1)
F- Q	1880 / 0	0.67 (1)
Q- H	-865 / 0	0.25 (1)
Q- I	-2361 / 0	0.55 (1)
O- I	0 / 401	0.06 (4)
O- J	-344 / 0	0.06 (1)
N- J	2 / 175	0.03 (4)
B- U	0 / 2132	0.86 (1)
N- K	0 / 1087	0.43 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.76")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.76")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.58/1.00 (H-I:1) , BC=0.59/1.00 (T-U:1) , WB=0.67/1.00 (F-Q:1) , SSI=0.26/1.00 (T-U:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.78 (F) (INPUT = 0.90)
SI METAL= 0.32 (G) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
09/26/19
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only 1/2
DWG# T-1924964 CONTINUED ON PAGE 2

LOADING
TOTAL LOAD CASES: (4)

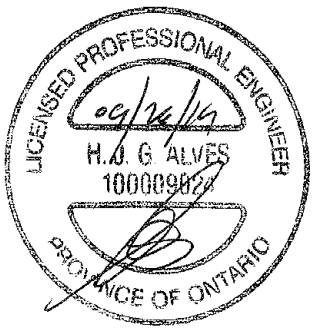
CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
AJ-N	0 : 1060	-18.5	-18.5	0.11 (4)	10.00			
N-AK	0 : 0	-18.5	-18.5	0.07 (4)	10.00			
AK-AL	0 : 0	-18.5	-18.5	0.07 (4)	10.00			
AL-M	0 : 0	-18.5	-18.5	0.07 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-8	-428	-428	---	BACK	VERT	TOTAL	---	C1
I	28-9-12	-178	-178	---	BACK	VERT	TOTAL	---	C1
J	34-10-8	-428	-428	---	BACK	VERT	TOTAL	---	C1
N	34-9-12	-36	-36	---	BACK	VERT	TOTAL	---	C1
O	28-9-12	-36	-36	---	BACK	VERT	TOTAL	---	C1
P	26-9-12	-36	-36	---	BACK	VERT	TOTAL	---	C1
U	5-11-4	-36	-36	---	BACK	VERT	TOTAL	---	C1
W	7-10-4	-178	-178	---	BACK	VERT	TOTAL	---	C1
X	23-6-12	-178	-178	---	BACK	VERT	TOTAL	---	C1
Y	24-9-12	-178	-178	---	BACK	VERT	TOTAL	---	C1
Z	26-9-12	-178	-178	---	BACK	VERT	TOTAL	---	C1
AA	30-9-12	-178	-178	---	BACK	VERT	TOTAL	---	C1
AB	32-9-12	-178	-178	---	BACK	VERT	TOTAL	---	C1
AC	1-11-4	-23	-25	---	BACK	VERT	TOTAL	---	C1
AD	3-11-4	-28	-28	---	BACK	VERT	TOTAL	---	C1
AE	7-10-4	-36	-36	---	BACK	VERT	TOTAL	---	C1
AF	8-4-8	-1174	-1174	---	BACK	VERT	TOTAL	---	C1
AG	23-6-12	-36	-36	---	BACK	VERT	TOTAL	---	C1
AH	24-9-12	-36	-36	---	BACK	VERT	TOTAL	---	C1
AI	30-9-12	-36	-36	---	BACK	VERT	TOTAL	---	C1
AJ	32-9-12	-36	-36	---	BACK	VERT	TOTAL	---	C1
AK	36-9-12	-28	-28	---	BACK	VERT	TOTAL	---	C1
AL	38-9-12	-23	-25	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-1924964 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405684	T21	1	1	GREENYORK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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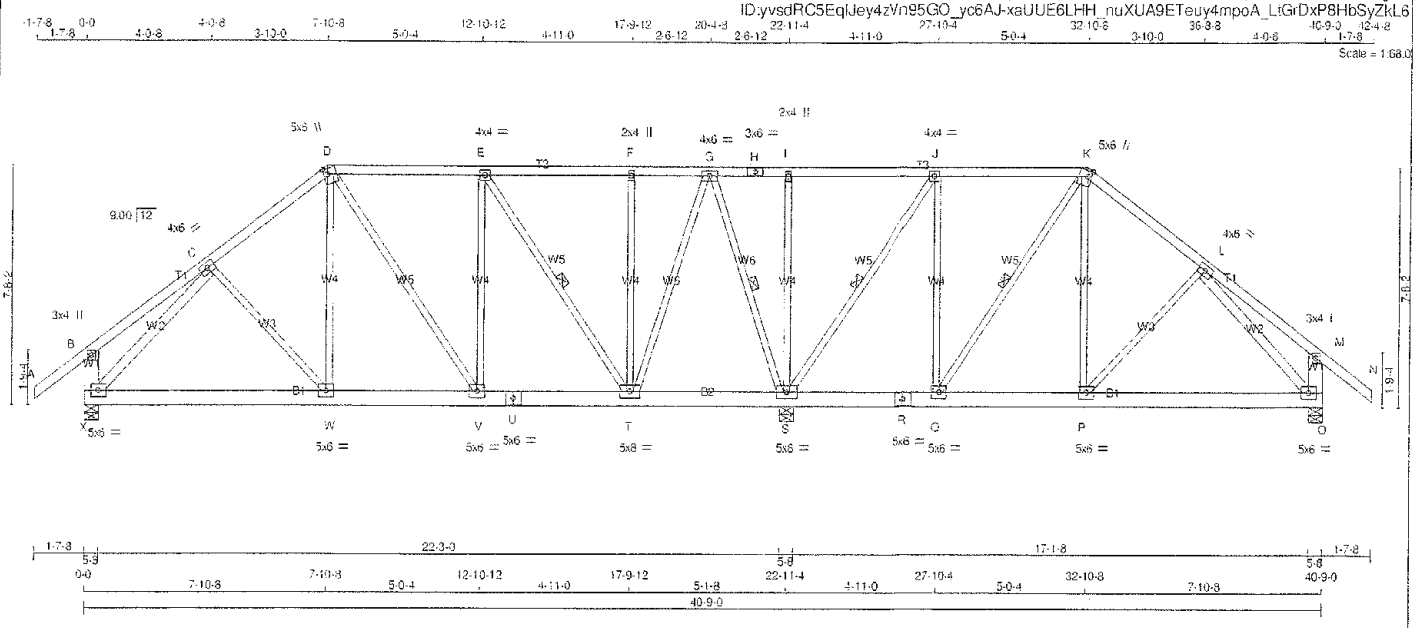

TOTAL WEIGHT = 228 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
K - N	2x4	DRY	No.2	SPF
X - B	2x6	DRY	No.2	SPF
O - M	2x6	DRY	No.2	SPF
X - U	2x6	DRY	No.2	SPF
U - R	2x6	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
X - C	2x4	DRY	No.2	SPF
L - O	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C, G, L						
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
H	TS-t	MT20	3.0	6.0		
I	TMW+w	MT20	2.0	4.0		
J	TMWW-t	MT20	4.0	4.0		
K	TTWW+m	MT20	5.0	6.0	2.25	1.50
M	TMV+p	MT20	3.0	4.0		
O	BMVW1-t	MT20	5.0	6.0		
P, Q, V, W						
P	BMWW-t	MT20	5.0	6.0		
R	BS-t	MT20	5.0	6.0		
S	BMWWW1-t	MT20	5.0	8.0		
T	BMWWW-t	MT20	5.0	8.0		
U	BS-t	MT20	5.0	6.0		
X	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	2464	0	2464	0	0	5-8	5-8
X	1327	0	1327	0	0	5-8	5-8
O	1017	0	1017	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX. MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1740	1153 / 0	0 / 0	0 / 0	0 / 0	587 / 0	0 / 0
X	936	626 / 0	0 / 0	0 / 0	0 / 0	310 / 0	0 / 0
O	717	481 / 0	0 / 0	0 / 0	0 / 0	236 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, X, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.85 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-T, G-S, J-S, K-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A-B	0 / 47	-91.8	-91.8	0.19 (1)		10.00
B-C	0 / 26	-91.8	-91.8	0.23 (1)		10.00
C-D	-1033 / 0	-91.8	-91.8	0.25 (1)		5.85
D-E	-846 / 0	-91.8	-91.8	0.37 (1)		6.10
E-F	-471 / 0	-91.8	-91.8	0.35 (1)		6.25
F-G	-471 / 0	-91.8	-91.8	0.15 (1)		6.25
G-H	0 / 260	-91.8	-91.8	0.23 (1)		10.00
H-I	0 / 260	-91.8	-91.8	0.23 (1)		10.00
I-J	0 / 260	-91.8	-91.8	0.38 (1)		10.00
J-K	-293 / 0	-91.8	-91.8	0.34 (1)		6.25
K-L	-614 / 0	-91.8	-91.8	0.24 (1)		6.25
L-M	0 / 26	-91.8	-91.8	0.23 (1)		10.00
M-N	0 / 47	-91.8	-91.8	0.19 (1)		10.00
X-B	-294 / 0	0.0	0.0	0.02 (1)		7.81
O-M	-294 / 0	0.0	0.0	0.02 (1)		7.81
X-W	0 / 884	-18.5	-18.5	0.18 (1)		10.00
W-V	0 / 868	-18.5	-18.5	0.17 (4)		10.00
V-U	0 / 846	-18.5	-18.5	0.14 (1)		10.00
U-T	0 / 846	-18.5	-18.5	0.14 (1)		10.00
T-S	0 / 133	-18.5	-18.5	0.11 (1)		10.00
S-R	0 / 293	-18.5	-18.5	0.13 (1)		10.00
R-Q	0 / 293	-18.5	-18.5	0.13 (1)		10.00
Q-P	0 / 474	-18.5	-18.5	0.15 (4)		10.00
P-O	0 / 602	-18.5	-18.5	0.15 (4)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-W	-115 / 13	0.07 (1)
W-D	0 / 229	0.05 (4)
D-V	0 / 67	0.02 (1)
V-E	0 / 77	0.03 (4)
E-T	-672 / 0	0.35 (1)
T-F	-347 / 0	0.25 (1)
T-G	0 / 1022	0.23 (1)
G-S	-1186 / 0	0.47 (1)
S-I	-369 / 0	0.38 (1)
S-J	-991 / 0	0.52 (1)
Q-J	0 / 297	0.07 (1)
Q-K	-316 / 0	0.17 (1)
R-K	0 / 294	0.07 (1)
P-L	-193 / 0	0.11 (1)
X-C	-1312 / 0	0.49 (1)
L-O	-894 / 0	0.33 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00:12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.76")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.76")
CALCULATED VERT. DEFL.(TL) = L/999 (0.06")

CSI: TC=0.38/1.00 (I-J:1) , BC=0.18/1.00 (W-X:1) , WB=0.52/1.00 (J-S:1) , SSI=0.22/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1567	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (G) (INPUT = 0.90)
JSI METAL= 0.32 (L) (INPUT = 1.00)

Structural component only
DWG# T-1924965

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405684	T22	1	1	GREENYORK HOMES TRUSS DESC.	

Tamarack Roof Trusss, Burlington

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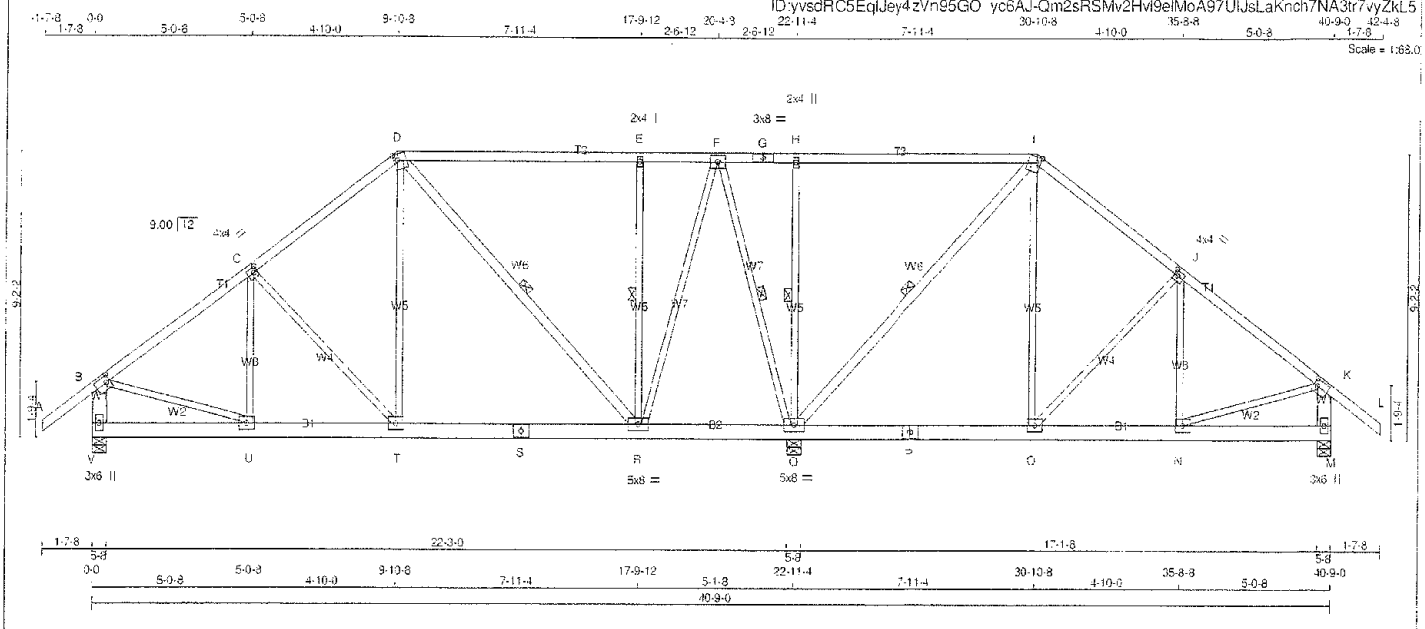


TOTAL WEIGHT = 229 lb

LUMBER

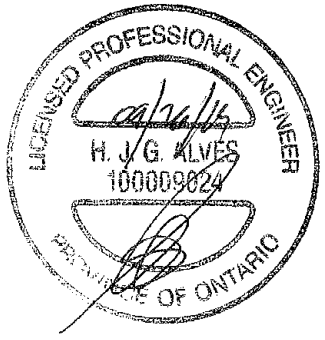
N. L. G. A. RULES

CHORDS	SIZE		LUMEER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
V - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
V - S	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - R	2x4	DRY	No.2	SPF
Q - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	1.50
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	5.0	6.0		
G	TS-t	MT20	3.0	8.0		
H	TMW+w	MT20	2.0	4.0		
I	TTWW+m	MT20	5.0	6.0	2.25	1.50
J	TMWW-t	MT20	4.0	4.0	2.00	1.50
K	TMVW-t	MT20	5.0	6.0	2.50	1.50
M	BMV1+p	MT20	3.0	6.0		
N, O, T, U						
N	BMWW-t	MT20	5.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWW/W1-t	MT20	5.0	8.0		
R	BMWWW-t	MT20	5.0	8.0		
S	BS-t	MT20	5.0	6.0		
V	BMV1+p	MT20	3.0	6.0		



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
V	1302	0	1302	0	0	5-8	5-8
Q	2520	0	2520	0	0	5-8	5-8
M	985	0	985	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX. MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	918	616 / 0	0 / 0	0 / 0	0 / 0	302 / 0	0 / 0
Q	1781	1176 / 0	0 / 0	0 / 0	0 / 0	606 / 0	0 / 0
M	694	469 / 0	0 / 0	0 / 0	0 / 0	226 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, Q, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.47 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-R, E-R, F-Q, H-Q, I-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	(LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-91.8	-91.8	0.19	(1)		10.00
B- C	-1097 / 0	-91.8	-91.8	0.42	(1)		5.47
C- D	-886 / 0	-91.8	-91.8	0.41	(1)		5.94
D- E	-396 / 0	-91.8	-91.8	0.81	(1)		6.25
E- F	-396 / 0	-91.8	-91.8	0.77	(1)		6.25
F- G	0 / 229	-91.8	-91.8	0.84	(1)		10.00
G- H	0 / 229	-91.8	-91.8	0.84	(1)		10.00
H- I	0 / 229	-91.8	-91.8	0.84	(1)		10.00
I- J	-434 / 0	-91.8	-91.8	0.39	(1)		6.25
J- K	-712 / 0	-91.8	-91.8	0.40	(1)		6.25
K- L	0 / 47	-91.8	-91.8	0.19	(1)		10.00
V- B	-1256 / 0	0.0	0.0	0.09	(1)		7.81
M- K	942 / 0	0.0	0.0	0.06	(1)		7.81
V- U	0 / 0	-18.5	-18.5	0.05	(4)		10.00
U- T	0 / 903	-18.5	-18.5	0.16	(1)		10.00
T- S	0 / 885	-18.5	-18.5	0.14	(1)		10.00
S- R	0 / 885	-18.5	-18.5	0.14	(1)		10.00
R- O	0 / 33	-18.5	-18.5	0.11	(4)		10.00
O- P	0 / 324	-18.5	-18.5	0.14	(1)		10.00
P- O	0 / 324	-18.5	-18.5	0.14	(1)		10.00
O- N	0 / 396	-18.5	-18.5	0.11	(4)		10.00
N- M	0 / 0	-18.5	-18.5	0.05	(4)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI	(LC)
U- C	-167 / 14	0.07	(1)
C- T	-321 / 0	0.31	(1)
T- D	0 / 373	0.08	(1)
D- R	-431 / 0	0.34	(1)
R- E	-837 / 0	0.44	(1)
R- F	0 / 1298	0.28	(1)
F- Q	-937 / 0	0.54	(1)
Q- H	-853 / 0	0.45	(1)
Q- I	-825 / 0	0.61	(1)
O- I	0 / 398	0.09	(1)
O- J	-399 / 0	0.38	(1)
N- J	-70 / 47	0.03	(1)
B- U	0 / 935	0.21	(1)
N- K	0 / 617	0.14	(1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.76")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.76")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.84/1.00 (H-I:1) , BC=0.15/1.00 (T-U:1) , WB=0.64/1.00 (I-Q:1) , SSI=0.34/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1556

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (K) (INPUT = 0.90)
JSI METAL= 0.59 (G) (INPUT = 1.00)

Structural component only
DWG# T-1924966

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405684	T23	1	1	GREENYORK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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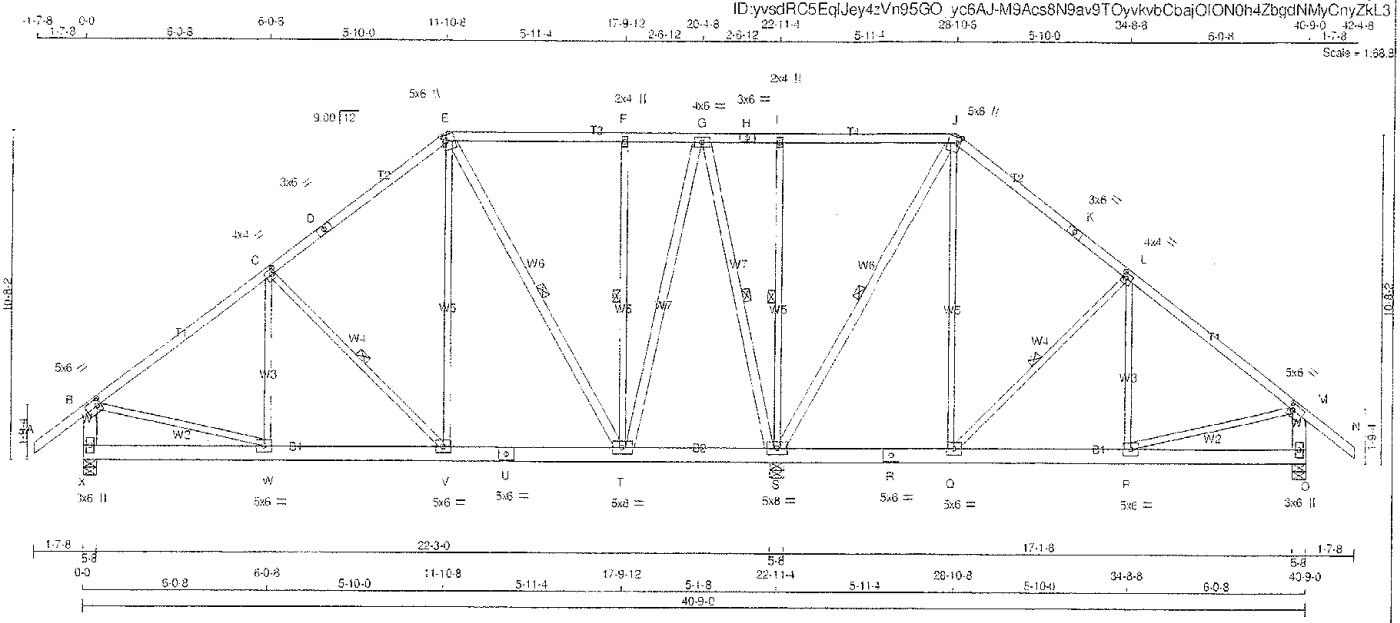


TOTAL WEIGHT = 249 lb

[M][F]

LUMBER

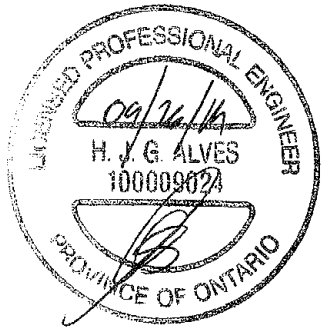
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
J - K	2x4	DRY	No.2	SPF
K - N	2x4	DRY	No.2	SPF
X - B	2x6	DRY	No.2	SPF
O - M	2x6	DRY	No.2	SPF
X - U	2x6	DRY	No.2	SPF
U - R	2x6	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
E - T	2x4	DRY	No.2	SPF
T - G	2x4	DRY	No.2	SPF
G - S	2x4	DRY	No.2	SPF
S - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	1.50
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
O, H, K						
D	TS-t	MT20	3.0	6.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMW+w	MT20	2.0	4.0		
G	TMWW-t	MT20	4.0	6.0		
I	TMW+w	MT20	2.0	4.0		
J	TTWW+m	MT20	5.0	6.0	2.25	1.50
L	TMWW-t	MT20	4.0	4.0	2.00	1.50
M	TMVW-t	MT20	5.0	6.0	2.50	1.50
C	BMV1+p	MT20	3.0	6.0		
P, Q, V, W						
P	BMWW-t	MT20	5.0	6.0		
R	BS-t	MT20	5.0	6.0		
S	BMWWW1-t	MT20	5.0	8.0		
T	BMWWW-t	MT20	5.0	8.0		
U	BS-t	MT20	5.0	6.0		
X	BMV1+p	MT20	3.0	6.0		



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
X	1268	0	1268	0	0	5-8	5-8
S	2598	0	2598	0	0	5-8	5-8
O	941	0	941	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
X	394	600 / 0	0 / 0	0 / 0	0 / 0	294 / 0	0 / 0
S	1836	1212 / 0	0 / 0	0 / 0	0 / 0	624 / 0	0 / 0
O	963	448 / 0	0 / 0	0 / 0	0 / 0	215 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, S, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.54 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1-2 LENGTH OF C-V, E-T, F-T, G-S, I-S, J-S, L-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	LC	MAX. UNBRAC LENGTH
A-B	0 / 47	-91.8	-91.8	0.19	(1)		10.00
B-C	-1050 / 0	-91.8	-91.8	0.45	(1)		5.54
C-D	-700 / 0	-91.8	-91.8	0.43	(1)		6.25
D-E	-700 / 0	-91.8	-91.8	0.43	(1)		6.25
E-F	-251 / 0	-91.8	-91.8	0.36	(1)		6.25
F-G	-251 / 0	-91.8	-91.8	0.31	(1)		6.25
G-H	0 / 299	-91.8	-91.8	0.48	(1)		10.00
H-I	0 / 299	-91.8	-91.8	0.48	(1)		10.00
I-J	0 / 299	-91.8	-91.8	0.48	(1)		10.00
J-K	-223 / 0	-91.8	-91.8	0.42	(1)		6.25
K-L	-223 / 0	-91.8	-91.8	0.42	(1)		6.25
L-M	-634 / 0	-91.8	-91.8	0.44	(1)		6.25
M-N	0 / 47	-91.8	-91.8	0.19	(1)		10.00
X-B	-1218 / 0	0.0	0.0	0.08	(1)		7.81
O-M	-894 / 0	0.0	0.0	0.06	(1)		7.81
X-W	0 / 0	-18.5	-18.5	0.07	(4)		10.00
W-V	0 / 871	-18.5	-18.5	0.13	(1)		10.00
V-U	0 / 532	-18.5	-18.5	0.10	(1)		10.00
U-T	0 / 532	-18.5	-18.5	0.10	(1)		10.00
T-S	-29 / 0	-18.5	-18.5	0.07	(1)		6.25
S-R	0 / 151	-18.5	-18.5	0.10	(1)		10.00
R-O	0 / 151	-18.5	-18.5	0.10	(1)		10.00
Q-P	0 / 539	-18.5	-18.5	0.11	(4)		10.00
P-O	0 / 0	-18.5	-18.5	0.07	(4)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	CSI	LC
W-C	-81 / 70	0.05	(1)
C-V	-485 / 0	0.23	(1)
V-E	0 / 459	0.13	(1)
E-T	-560 / 0	0.44	(1)
T-F	-571 / 0	0.44	(1)
T-G	0 / 1162	0.19	(1)
G-S	-1120 / 0	0.66	(1)
S-I	-588 / 0	0.45	(1)
S-J	-900 / 0	0.71	(1)
Q-J	0 / 472	0.11	(1)
Q-L	-554 / 0	0.26	(1)
P-L	0 / 99	0.03	(4)
B-W	0 / 893	0.20	(1)
P-M	0 / 552	0.12	(1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00:12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.76")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.75")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.48-1.00 (H-J:1) , BC=0.13-1.00 (V-W:1) , WB=0.71-1.00 (J-S:1) , SSI=0.25-1.00 (H-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PSI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	318	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (G) (INPUT = 0.90)
JSI METAL= 0.33 (M) (INPUT = 1.00)

Structural component only
DWG# T-1924967

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405684	T24	1	3	GREENYORK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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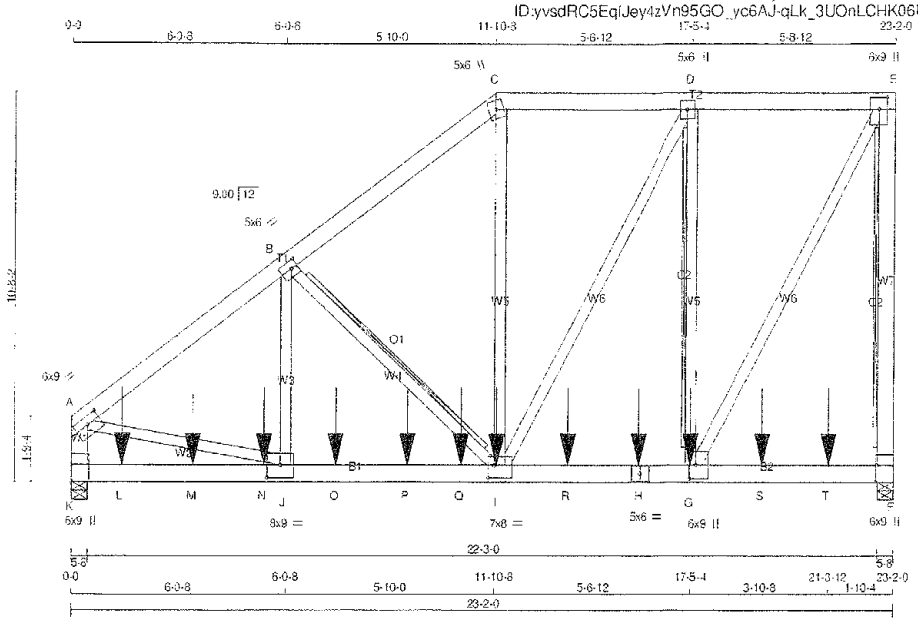


TOTAL WEIGHT = 3 X 177 = 532 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x8	DRY	No.2	SPF
C - E	2x8	DRY	No.2	SPF
F - E	2x6	DRY	No.2	SPF
K - A	2x8	DRY	No.2	SPF
K - H	2x8	DRY	2100F 1.8E	SPF
H - F	2x8	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	2	12	TOP
C-E	2	12	TOP
E-F	2	12	TOP
K-A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
K-H	2	6	SIDE(1230.3)
H-F	2	7	SIDE(1152.8)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	9.0	2.50	4.50
B	TMVW-t	MT20	5.0	6.0	2.50	2.25
C	TTW+m	MT20	5.0	6.0		
D	TMVW+;	MT20	5.0	6.0		
E	TMVW+p	MT20	5.0	9.0	4.00	2.75
F	BMV1+t	MT20	5.0	9.0	Edge	0.50
G	BMVW+;	MT20	5.0	9.0	4.50	2.00
H	BS-;	MT20	5.0	6.0		
I	BMVWW-t	MT20	7.0	8.0	4.25	2.00
J	BMVW-t	MT20	5.0	9.0	4.25	4.50
K	BMV1+t	MT20	5.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	14463	0	14463	0	0	5-8	5-8
K	14605	0	14605	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
F	10218	6764 / 0	0 / 0	0 / 0	0 / 0	3454 / 0	0 / 0
K	10318	6830 / 0	0 / 0	0 / 0	0 / 0	3488 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.69 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x8 DRY SPF No.2 T-BRACE AT E-F
2x6 DRY SPF No.2 T-BRACE AT B-I, D-G

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A-B	-15833 / 0	-91.8	-91.8	0.27 (1)		3.89
B-C	-11603 / 0	-91.8	-91.8	0.17 (1)		4.28
C-D	-9344 / 0	-91.8	-91.8	0.13 (1)		4.71
D-E	-6973 / 0	-91.8	-91.8	0.11 (1)		5.30
F-E	-12695 / 0	0.0	0.0	0.71 (1)		7.81
K-A	-12399 / 0	0.0	0.0	0.27 (1)		5.29
K-L	0 / 0	-18.5	-18.5	0.52 (1)		10.00
L-M	0 / 0	-18.5	-18.5	0.52 (1)		10.00
M-N	0 / 0	-18.5	-18.5	0.52 (1)		10.00
N-J	0 / 0	-18.5	-18.5	0.52 (1)		10.00
J-O	0 / 12674	-18.5	-18.5	0.72 (1)		10.00
O-P	0 / 12674	-18.5	-18.5	0.72 (1)		10.00
P-Q	0 / 12674	-18.5	-18.5	0.72 (1)		10.00
Q-I	0 / 12674	-18.5	-18.5	0.72 (1)		10.00
I-R	0 / 6973	-18.5	-18.5	0.49 (1)		10.00
R-H	0 / 6973	-18.5	-18.5	0.49 (1)		10.00
H-G	0 / 6973	-18.5	-18.5	0.49 (1)		10.00
G-S	0 / 0	-18.5	-18.5	0.39 (1)		10.00
S-T	0 / 0	-18.5	-18.5	0.39 (1)		10.00
T-F	0 / 0	-18.5	-18.5	0.39 (1)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI
J-R	0 / 5254	0.28 (1)
B-I	-4762 / 0	0.39 (1)
I-C	0 / 6437	0.34 (1)
I-D	0 / 5064	0.27 (1)
G-D	-4954 / 0	0.64 (1)
G-E	0 / 14258	0.76 (1)
A-J	0 / 12995	0.69 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC	LC	MAX-	MAX+	FACE	DIR.	TYPE	SHEET	COMM
G	17-5-4	-2210	-2210	---	FRONT	VERT	TOTAL	---	C1
H	15-11-4	-2210	-2210	---	FRONT	VERT	TOTAL	---	C1
I	11-11-4	-2210	-2210	---	FRONT	VERT	TOTAL	---	C1
L	1-5-4	-2210	-2210	---	FRONT	VERT	TOTAL	---	C1
M	3-5-4	-2210	-2210	---	FRONT	VERT	TOTAL	---	C1
N	5-5-4	-2210	-2210	---	FRONT	VERT	TOTAL	---	C1
O	7-5-4	-2210	-2210	---	FRONT	VERT	TOTAL	---	C1
P	9-5-4	-2210	-2210	---	FRONT	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.77")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.77")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.26")

CSI: TC=0.71/1.00 (E-F:1) , BC=0.72/1.00 (I J:1) , WB=0.76/1.00 (E-G:1) , SSI=0.91/1.00 (J K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSF) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	613	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

USI GRIP = 0.90 (G) (INPUT = 0.90)
USI METAL = 0.39 (A) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009021
PROVINCE OF ONTARIO

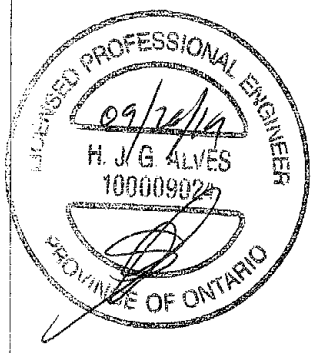
Structural component only 1/2
DWG# T-1924968 CONTINUED ON PAGE 2

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
Q	10-11-4	-2210	-2210	---	FRONT	VERT	TOTAL	---	C1
R	13-11-4	-2210	-2210	---	FRONT	VERT	TOTAL	---	C1
S	16-5-4	-2210	-2210	---	FRONT	VERT	TOTAL	---	C1
T	21-3-12	-2210	-2210	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-1924968

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC	GREENYORK HOMES	DRWG NO.
405684	T25	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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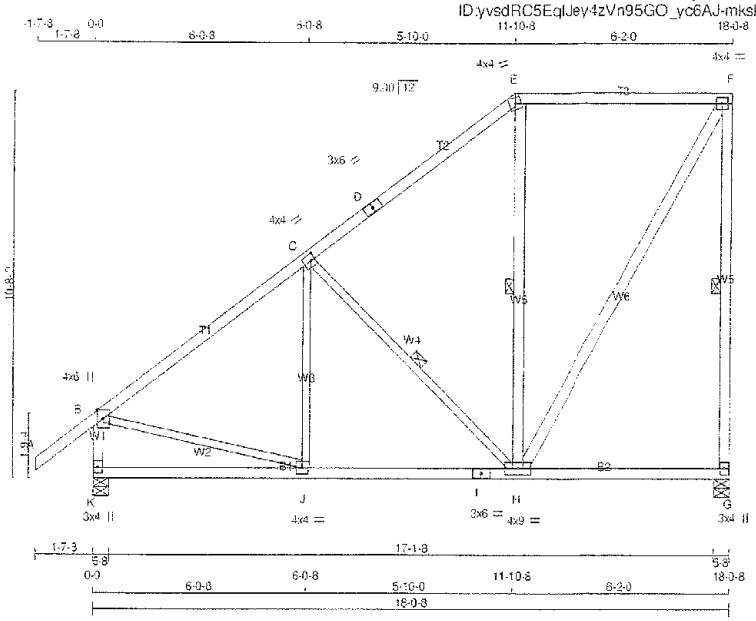


TOTAL WEIGHT = 2 X 101 = 202 lb

MiTek

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTW-m	MT20	4.0	4.0		
F	TMVW-t	MT20	4.0	4.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
G	995	0	995	0	0	5-8	5-8
K	1152	0	1152	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX. MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	703	462 / 0	0 / 0	0 / 0	0 / 0	242 / 0	0 / 0
K	812	549 / 0	0 / 0	0 / 0	0 / 0	262 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, C-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-91.8	-91.8	0.19 (1)		10.00
B- C	-893 / 0	-91.8	-91.8	0.45 (1)		5.88
C- D	-529 / 0	-91.8	-91.8	0.43 (1)		6.25
D- E	-529 / 0	-91.8	-91.8	0.43 (1)		6.25
E- F	-387 / 0	-91.8	-91.8	0.45 (1)		6.25
G- F	-949 / 0	0.0	0.0	0.55 (1)		6.25
K- B	-1106 / 0	0.0	0.0	0.12 (1)		7.53
K- J	0 / 0	-18.5	-18.5	0.15 (4)		10.00
J- I	0 / 746	-18.5	-18.5	0.22 (4)		10.00
I- H	0 / 746	-18.5	-18.5	0.22 (4)		10.00
H- G	0 / 0	-18.5	-18.5	0.15 (4)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- C	-58 / 76	0.04 (1)
C- H	-504 / 0	0.24 (1)
H- E	-190 / 14	0.11 (1)
H- F	0 / 772	0.12 (1)
B- J	0 / 767	0.17 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.60")
CALCULATED VERT. DEFL.(LL) = L 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.60")
CALCULATED VERT. DEFL.(TL) = L 999 (0.05")

CSI: TC=0.55/1.00 (F-G:1) , BC=0.22/1.00 (H-J:4) , WB=0.24/1.00 (C-H:1) , SSI=0.22/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

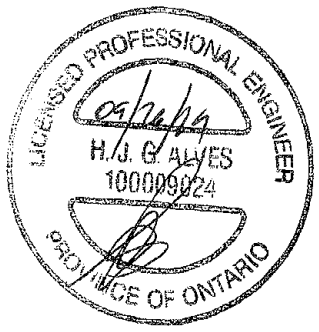
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	738	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (J) (INPUT = 0.90)
JSI METAL= 0.51 (B) (INPUT = 1.00)



Structural component only
DWG# T-1924969

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405684	T25A	2	1	GREENYORK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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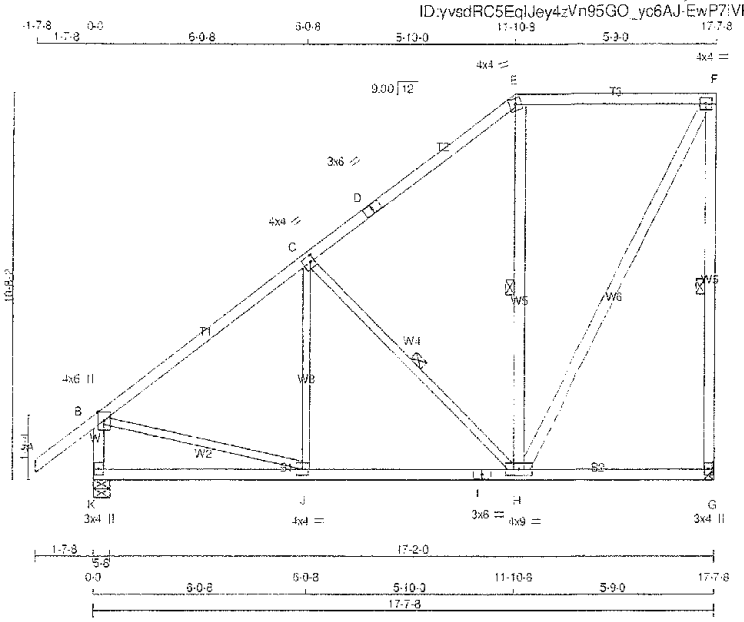


Scale = 1:58.6

TOTAL WEIGHT = 2 X 100 = 199 lb

(M)(F)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTW-m	MT20	4.0	4.0		
F	TMVW-t	MT20	4.0	4.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	972	0	972	0	0	MECHANICAL	
K	1129	0	1129	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX. MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	687	451 / 0	0 / 0	0 / 0	0 / 0	236 / 0	0 / 0
K	795	539 / 0	0 / 0	0 / 0	0 / 0	257 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.96 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, C-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

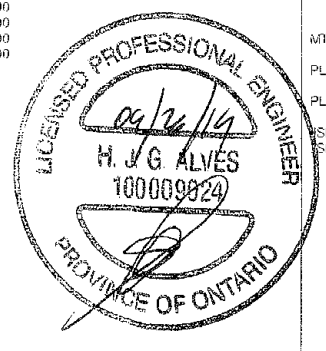
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF) CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-91.8	-91.8	0.19 (1)	10.00
B- C	-864 / 0	-91.8	-91.8	0.45 (1)	5.96
C- D	-495 / 0	-91.8	-91.8	0.42 (1)	6.25
D- E	-495 / 0	-91.8	-91.8	0.42 (1)	6.25
E- F	-360 / 0	-91.8	-91.8	0.39 (1)	6.25
G- F	-930 / 0	0.0	0.0	0.53 (1)	6.25
K- B	-1083 / 0	0.0	0.0	0.12 (1)	7.58
K- J	0 / 0	-18.5	-18.5	0.15 (4)	10.00
J- I	0 / 723	-18.5	-18.5	0.22 (4)	10.00
I- H	0 / 723	-18.5	-18.5	0.22 (4)	10.00
H- G	0 / 0	-18.5	-18.5	0.14 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- C	-50 / 80	0.03 (1)
C- H	-509 / 0	0.25 (1)
H- E	-191 / 11	0.11 (1)
H- F	0 / 758	0.12 (1)
B- J	0 / 743	0.17 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.53/1.00 (F-G:1) ; BC=0.22/1.00 (H-J:4) ; WB=0.25/1.00 (C-H:1) ; SSI=0.21/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1356

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

(SI GRIP= 0.87 (J) (INPUT = 0.90)
(SI METAL= 0.50 (B) (INPUT = 1.00)

Structural component only
DWG# T-1924970

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405684	T26	2	1	GREENYORK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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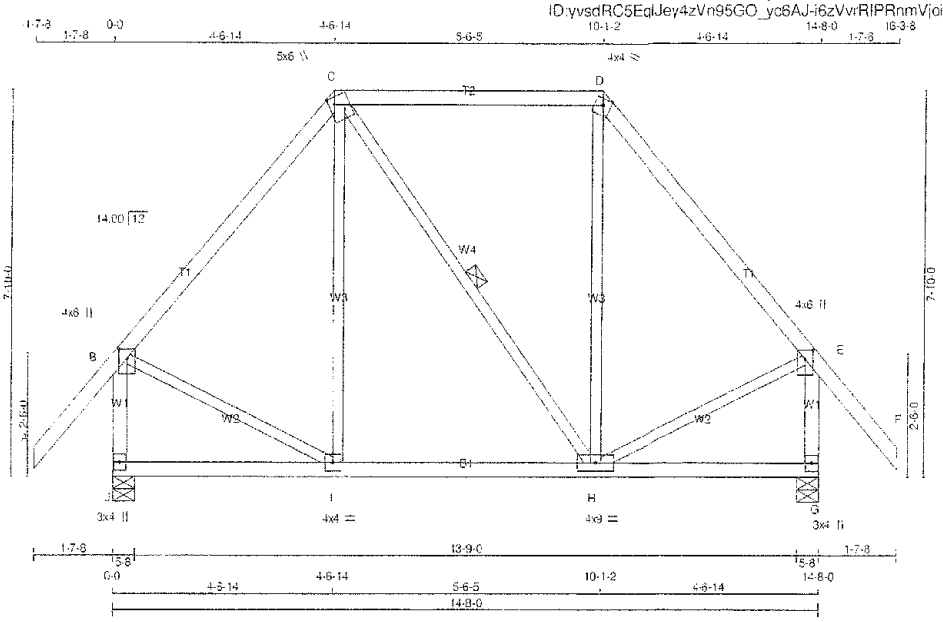


Scale = 1:42.9

TOTAL WEIGHT = 2 X 77 = 154 lb

[MiTek]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.25	2.00
C	TTWW+m	MT20	5.0	6.0	1.75	1.50
D	TTW.m	MT20	4.0	4.0	Edge	
E	TMVW+p	MT20	4.0	6.0	2.25	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW+t	MT20	4.0	9.0		
I	BMWW+t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	968	0	968	0	0	5-8	5-8
G	968	0	968	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	681	464 / 0	0 / 0	0 / 0	0 / 0	217 / 0	0 / 0
G	681	464 / 0	0 / 0	0 / 0	0 / 0	217 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

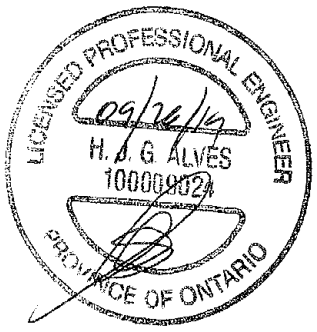
1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC FROM TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 60	-91.8 -91.8	0.20 (1)	10.00
B-C	-534 / 0	-91.8 -91.8	0.25 (1)	6.25
C-D	-346 / 0	-91.8 -91.8	0.36 (1)	6.25
D-E	-534 / 0	-91.8 -91.8	0.25 (1)	6.25
E-F	0 / 60	-91.8 -91.8	0.20 (1)	10.00
J-B	-934 / 0	0.0 0.0	0.13 (1)	7.81
G-E	-934 / 0	0.0 0.0	0.13 (1)	7.81
J-I	0 / 0	-18.5 -18.5	0.10 (4)	10.00
I-H	0 / 346	-18.5 -18.5	0.14 (4)	10.00
H-G	0 / 0	-18.5 -18.5	0.11 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I-C	-56 / 59	0.06 (1)
C-H	0 / 0	0.00 (1)
H-D	-56 / 59	0.06 (1)
B-I	0 / 381	0.09 (1)
H-E	0 / 381	0.09 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00:12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.49")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.49")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.36/1.00 (C-D:1) , BC=0.14/1.00 (H:L4) , WB=0.09/1.00 (B-I:1) , SSI=0.20/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1557	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.56 (B) (INPUT = 0.90)
JSI METAL = 0.27 (B) (INPUT = 1.00)

Structural component only
DWG# T-1924971

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405684	T27	2	1	GREENYORK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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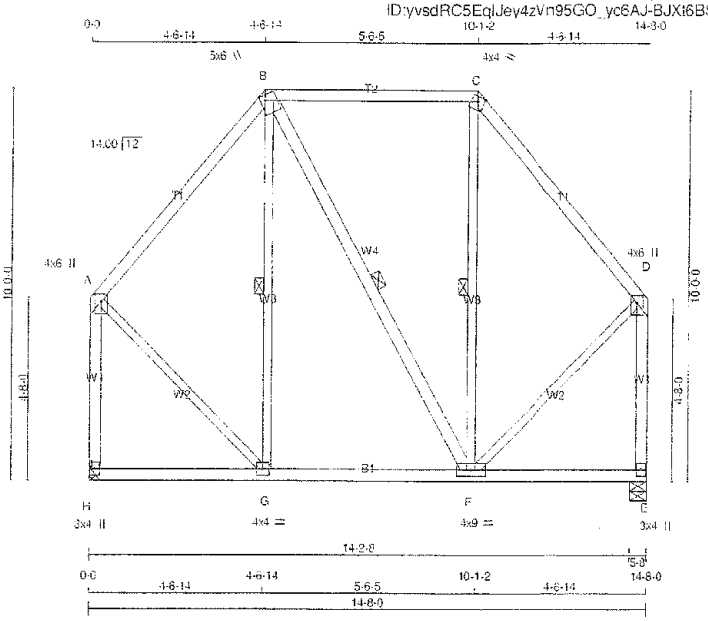


TOTAL WEIGHT = 2 X 87 = 174 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.25	2.00
B	TTWW+m	MT20	5.0	8.0	1.75	1.50
C	TTW-m	MT20	4.0	4.0	Edge	
D	TMVW+p	MT20	4.0	6.0	2.25	2.00
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	9.0		
G	BMWW+t	MT20	4.0	4.0		
H	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	809	0	809	0	0	IN-SX	IN-SX
E	809	0	809	0	0	MECHANICAL 5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	572	375 / 0	0 / 0	0 / 0	0 / 0	197 / 0	0 / 0
E	572	375 / 0	0 / 0	0 / 0	0 / 0	197 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

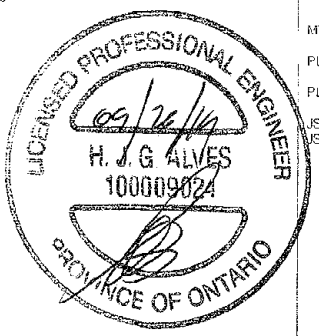
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF B-G, B-F, C-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	(LC)	MAX. UNBRAC LENGTH
A-B	-416 / 0	-91.8	-91.8	0.25	(1)		6.25
B-C	-268 / 0	-91.8	-91.8	0.36	(1)		6.25
C-D	-416 / 0	-91.8	-91.8	0.25	(1)		6.25
H-A	-775 / 0	0.0	0.0	0.30	(1)		7.81
E-D	-774 / 0	0.0	0.0	0.30	(1)		7.81
H-G	0 / 0	-18.5	-18.5	0.11	(4)		10.00
G-F	0 / 238	-18.5	-18.5	0.13	(4)		10.00
F-E	0 / 0	-18.5	-18.5	0.11	(4)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI	(LC)
G-B	-146 / 27	0.10	(1)
B-F	0 / 0	0.00	(1)
F-C	-146 / 27	0.10	(1)
A-G	0 / 366	0.08	(1)
F-D	0 / 366	0.08	(1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.49")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.49")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.36/1.00 (B-C:1) , BC=0.13/1.00 (F-G:4) , WB=0.10/1.00 (C-F:1) , SSI=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

USI GRIP = 0.46 (A) ; INPUT = 0.90)
USI METAL = 0.19 (A) ; INPUT = 1.00)

Structural component only
DWG# T-1924972

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405684	T28	1	1	GREENYORK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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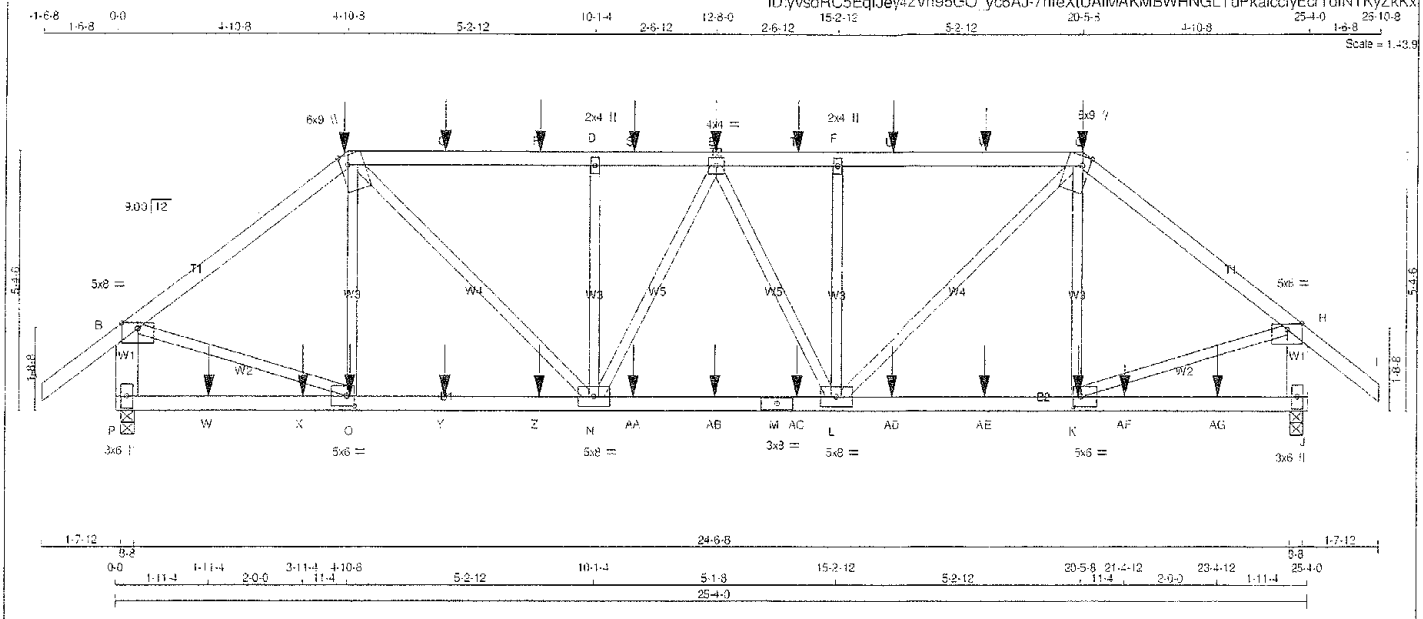


TOTAL WEIGHT = 114 lb

(M)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
P - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	6.0	9.0	Edge	2.00
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TTWW+m	MT20	6.0	9.0	Edge	2.00
H	TMVW-p	MT20	5.0	8.0	Edge	
J	BMV1+p	MT20	3.0	6.0		
K	BMWW-t	MT20	5.0	8.0	2.50	2.00
L	BMWWW-t	MT20	5.0	8.0		
M	BS-t	MT20	3.0	8.0		
N	BMWWW-t	MT20	5.0	8.0		
O	BMWW-t	MT20	5.0	8.0	2.50	2.00
P	BMV1+p	MT20	3.0	5.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
P	2472	0	2472	0	0	3-8	3-8
J	2472	0	2472	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1747	1156 / 0	0 / 0	0 / 0	0 / 0	590 / 0	0 / 0
J	1747	1156 / 0	0 / 0	0 / 0	0 / 0	590 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.93 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	(LC)	MAX. UNBRAC LENGTH
A-B	0 : 45	-91.8	-91.8	0.19	(1)		10.00
B-C	-2496 : 0	-91.8	-91.8	0.62	(1)		3.71
C-Q	-3074 : 0	-91.8	-91.8	0.84	(1)		2.93
Q-R	-3074 : 0	-91.8	-91.8	0.84	(1)		2.93
R-D	-3074 : 0	-91.8	-91.8	0.84	(1)		2.93
D-S	-3074 : 0	-91.8	-91.8	0.72	(1)		2.93
S-E	-3074 : 0	-91.8	-91.8	0.72	(1)		2.93
E-T	-3074 : 0	-91.8	-91.8	0.72	(1)		2.93
T-F	-3074 : 0	-91.8	-91.8	0.72	(1)		2.93
F-U	-3074 : 0	-91.8	-91.8	0.84	(1)		2.93
U-V	-3074 : 0	-91.8	-91.8	0.84	(1)		2.93
V-G	-3074 : 0	-91.8	-91.8	0.84	(1)		2.93
G-H	-2496 : 0	-91.8	-91.8	0.62	(1)		3.71
H-I	0 : 45	-91.8	-91.8	0.19	(1)		10.00
P-B	-2428 : 0	0.0	0.0	0.19	(1)		6.58
J-H	-2428 : 0	0.0	0.0	0.19	(1)		6.58
P-W	0 : 0	-18.5	-18.5	0.19	(4)		10.00
W-X	0 : 0	-18.5	-18.5	0.19	(4)		10.00
X-O	0 : 0	-18.5	-18.5	0.19	(4)		10.00
O-Y	0 : 1986	-18.5	-18.5	0.45	(1)		10.00
Y-Z	0 : 1986	-18.5	-18.5	0.45	(1)		10.00
Z-N	0 : 1986	-18.5	-18.5	0.45	(1)		10.00
N-AA	0 : 3102	-18.5	-18.5	0.64	(1)		10.00
AA-AB	0 : 3102	-18.5	-18.5	0.64	(1)		10.00
AB-M	0 : 3102	-18.5	-18.5	0.64	(1)		10.00
M-AC	0 : 3102	-18.5	-18.5	0.64	(1)		10.00
AC-L	0 : 3102	-18.5	-18.5	0.64	(1)		10.00
L-AD	0 : 1986	-18.5	-18.5	0.45	(1)		10.00
AD-AE	0 : 1986	-18.5	-18.5	0.45	(1)		10.00
AE-K	0 : 1986	-18.5	-18.5	0.45	(1)		10.00
K-AF	0 : 0	-18.5	-18.5	0.19	(4)		10.00
AF-AG	0 : 0	-18.5	-18.5	0.19	(4)		10.00
AG-J	0 : 0	-18.5	-18.5	0.19	(4)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI	(LC)
O-C	-375 : 11	0.17	(1)
C-N	0 : 1514	0.37	(1)
N-D	-836 : 0	0.37	(1)
L-F	-836 : 0	0.37	(1)
L-G	0 : 1514	0.37	(1)
K-G	-375 : 11	0.17	(1)
B-O	0 : 2069	0.51	(1)
K-H	0 : 2069	0.51	(1)
N-E	-60 : 0	0.03	(1)
E-L	-60 : 0	0.03	(1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	4-10-8	-314	-314	---	FRONT	VERT	TOTAL	---	C1
E	12-8-0	-132	-132	---	FRONT	VERT	TOTAL	---	C1
G	20-5-8	-314	-314	---	FRONT	VERT	TOTAL	---	C1
K	20-4-12	-27	-27	---	FRONT	VERT	TOTAL	---	C1
C	4-11-4	-27	-27	---	FRONT	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.84")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.84")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.84/1.00 (C-D:1) , BC=0.64/1.00 (L-N:1) , WB=0.51/1.00 (B-O:1) , SSI=0.40/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	518	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (K) (INPUT = 0.90)
JSI METAL= 0.95 (M) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
09/26/19
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

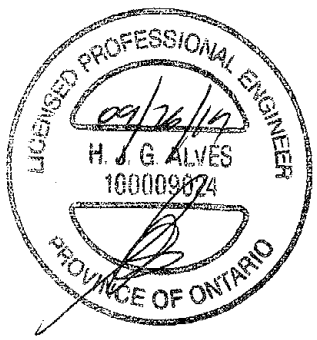
Structural component only 1/2
DWG# T-1924973 CONTINUED ON PAGE 2

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
Q	6-11-4	-132	-132	---	FRONT	VERT	TOTAL	---	C1
R	8-11-4	-132	-132	---	FRONT	VERT	TOTAL	---	C1
S	10-11-4	-132	-132	---	FRONT	VERT	TOTAL	---	C1
T	12-4-12	-132	-132	---	FRONT	VERT	TOTAL	---	C1
U	16-4-12	-132	-132	---	FRONT	VERT	TOTAL	---	C1
V	18-4-12	-132	-132	---	FRONT	VERT	TOTAL	---	C1
W	1-11-4	-13	-15	---	FRONT	VERT	TOTAL	---	C1
X	3-11-4	-17	-17	---	FRONT	VERT	TOTAL	---	C1
Y	6-11-4	-27	-27	---	FRONT	VERT	TOTAL	---	C1
Z	8-11-4	-27	-27	---	FRONT	VERT	TOTAL	---	C1
AA	10-11-4	-27	-27	---	FRONT	VERT	TOTAL	---	C1
AB	12-8-0	-27	-27	---	FRONT	VERT	TOTAL	---	C1
AC	14-4-12	-27	-27	---	FRONT	VERT	TOTAL	---	C1
AD	16-4-12	-27	-27	---	FRONT	VERT	TOTAL	---	C1
AE	18-4-12	-27	-27	---	FRONT	VERT	TOTAL	---	C1
AF	21-4-12	-17	-17	---	FRONT	VERT	TOTAL	---	C1
AG	23-4-12	-13	-15	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-1924973 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405684	IT29	1	1	GREENYORK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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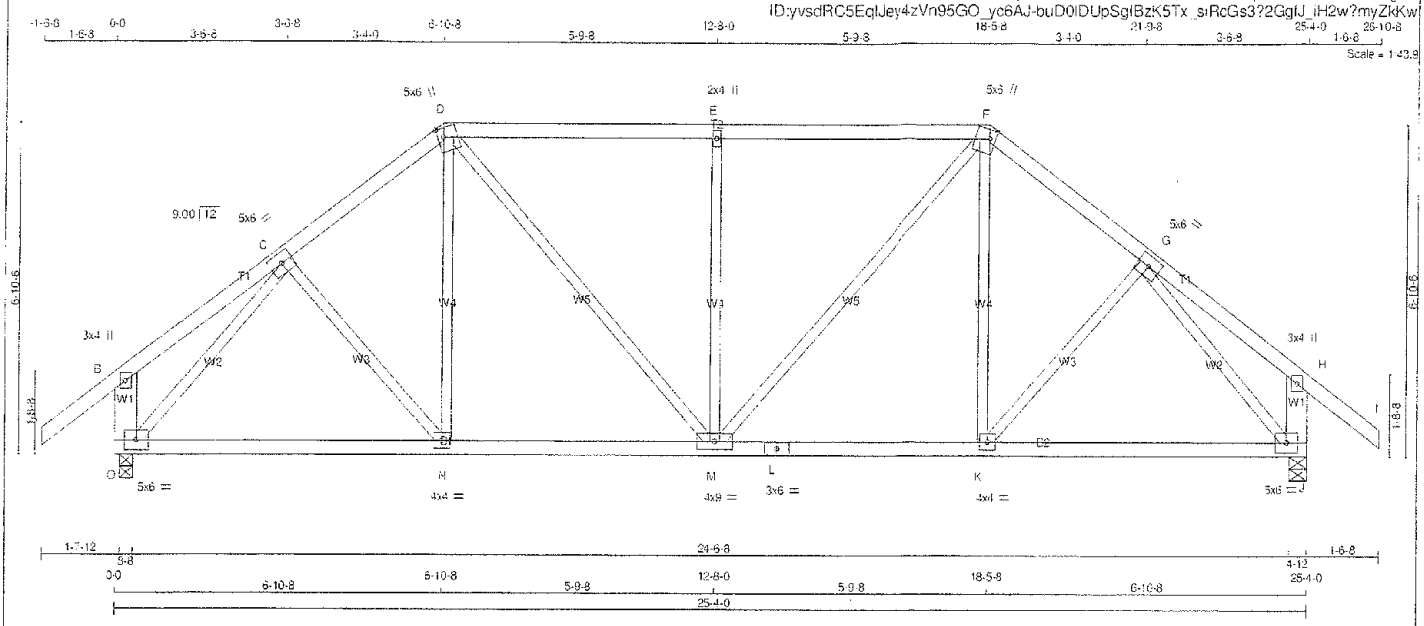

TOTAL WEIGHT = 116 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	5.0	6.0		
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	5.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	9.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
J	1546	0	1546	0	0	4-12	4-12
O	1546	0	1546	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX. MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	1091	732 / 0	0 / 0	0 / 0	0 / 0	359 / 0	0 / 0
O	1091	731 / 0	0 / 0	0 / 0	0 / 0	359 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, O

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.99 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (+)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 45	-91.8	-91.8	0.17 (1)	10.00
B- C	0 / 20	-91.8	-91.8	0.16 (1)	10.00
C- D	-1373 / 0	-91.8	-91.8	0.15 (1)	5.41
D- E	-1406 / 0	-91.8	-91.8	0.42 (1)	4.99
E- F	-1406 / 0	-91.8	-91.8	0.42 (1)	4.99
F- G	-1373 / 0	-91.8	-91.8	0.15 (1)	5.41
G- H	0 / 20	-91.8	-91.8	0.16 (1)	10.00
H- I	0 / 45	-91.8	-91.8	0.17 (1)	10.00
O- B	-272 / 0	0.0	0.0	0.02 (1)	7.81
J- H	-272 / 0	0.0	0.0	0.02 (1)	7.81
O- N	0 / 1058	-18.5	-18.5	0.29 (4)	10.00
N- M	0 / 1081	-18.5	-18.5	0.30 (4)	10.00
M- L	0 / 1081	-18.5	-18.5	0.30 (4)	10.00
L- K	0 / 1081	-18.5	-18.5	0.30 (4)	10.00
K- J	0 / 1058	-18.5	-18.5	0.29 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- N	0 / 54	0.02 (4)
N- D	0 / 117	0.04 (4)
D- M	0 / 491	0.11 (1)
M- E	-651 / 0	0.52 (1)
M- F	0 / 491	0.11 (1)
K- F	0 / 117	0.04 (4)
K- G	0 / 54	0.02 (4)
O- C	-1616 / 0	0.68 (1)
G- J	-1616 / 0	0.68 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00:12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.84")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.84")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.42(1.00 ;D-E:1) , BC=0.30(1.00 ;M-N:4) , WB=0.68(1.00 ;C-O:1) , SSI=0.26(1.00 ;D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1657	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (C) (INPUT = 0.90) JSI METAL= 0.40 (G) (INPUT = 1.00)

Structural component only
DWG# T-1924974

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405684	T30	1	1	GREENYORK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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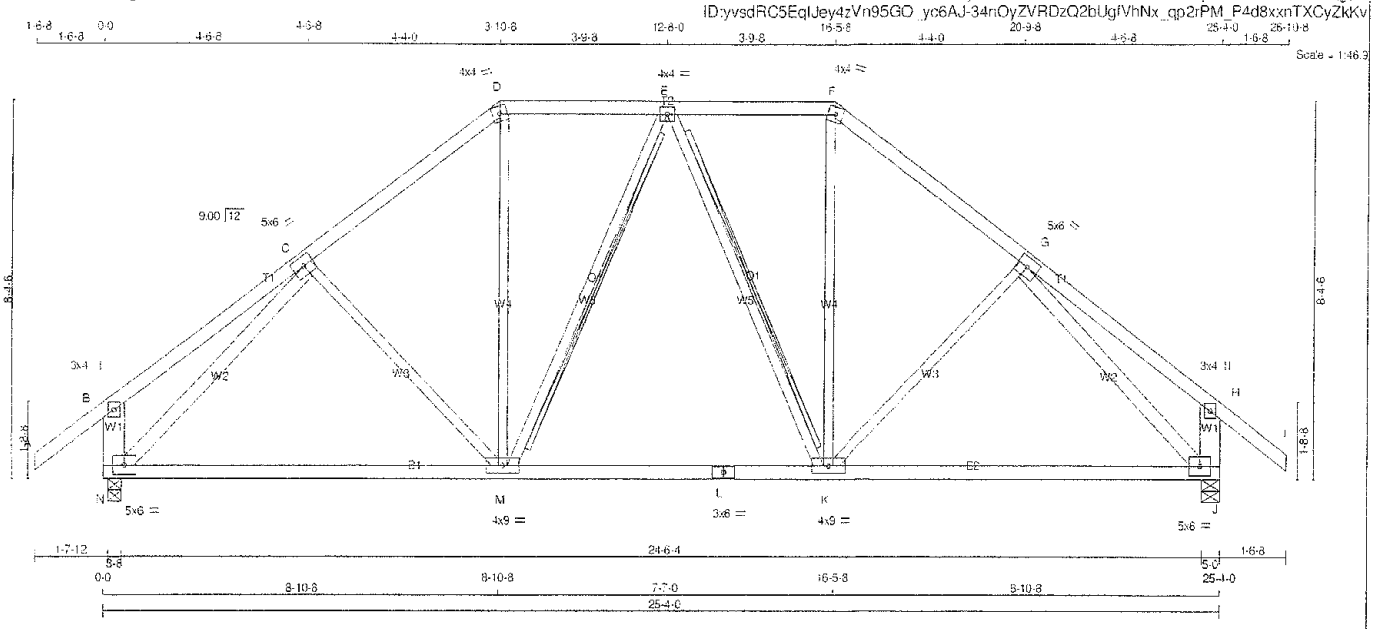

TOTAL WEIGHT = 123 lb

(M)(F)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
N - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
N - C	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMV+p	MT20	3.0	4.0	
C	TMWW-t	MT20	5.0	6.0	
D	TTW-m	MT20	4.0	4.0	
E	TMWW-t	MT20	4.0	4.0	
F	TTW-m	MT20	4.0	4.0	
G	TMWW-t	MT20	5.0	6.0	
H	TMV+p	MT20	3.0	4.0	
J	BMVW1-t	MT20	5.0	6.0	
K	BMWWW-t	MT20	4.0	9.0	
L	BS-t	MT20	3.0	6.0	
M	BMWWW-t	MT20	4.0	9.0	
N	BMVW1-t	MT20	5.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
J	1546	0	1546	0	0	5-0	5-0
N	1546	0	1546	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	1091	731 / 0	0 / 0	0 / 0	0 / 0	359 / 0	0 / 0
N	1091	732 / 0	0 / 0	0 / 0	0 / 0	359 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.41 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT E-M, E-K

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 45	-91.8	-91.8	0.17 (1)	10.00	
B- C	0 / 29	-91.8	-91.8	0.29 (1)	10.00	
C- D	-1297 / 0	-91.8	-91.8	0.25 (1)	5.41	
D- E	1019 / 0	-91.8	-91.8	0.17 (1)	6.02	
E- F	-1019 / 0	-91.8	-91.8	0.17 (1)	6.02	
F- G	-1297 / 0	-91.8	-91.8	0.25 (1)	5.41	
G- H	0 / 29	-91.8	-91.8	0.29 (1)	10.00	
H- I	0 / 45	-91.8	-91.8	0.17 (1)	10.00	
N- B	-304 / 0	0.0	0.0	0.02 (1)	7.81	
J- H	-304 / 0	0.0	0.0	0.02 (1)	7.81	
N- M	0 / 1103	-18.5	-18.5	0.39 (4)	10.00	
M- L	0 / 1114	-18.5	-18.5	0.39 (4)	10.00	
L- K	0 / 1114	-18.5	-18.5	0.39 (4)	10.00	
K- J	0 / 1103	-18.5	-18.5	0.39 (4)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- M	-130 / 13	0.10 (1)
M- D	0 / 467	0.11 (1)
M- E	-229 / 0	0.32 (1)
E- K	-229 / 0	0.32 (1)
K- F	0 / 467	0.11 (1)
K- G	-130 / 13	0.10 (1)
N- C	-1610 / 0	0.80 (1)
G- J	-1610 / 0	0.80 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00:12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.84")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.84")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.29/1.00 (B-C:1) , BC=0.39/1.00 (K-M:4) , WE=0.80/1.00 (C-N:1) , SSI=0.17/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP = 0.68 (G) (INPUT = 0.90)
SI METAL = 0.39 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
09/26/19
H. J. G ALVES
100009824
PROVINCE OF ONTARIO

Structural component only
DWG# T-1924975

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO
405684	T31	1	1	GREENYORK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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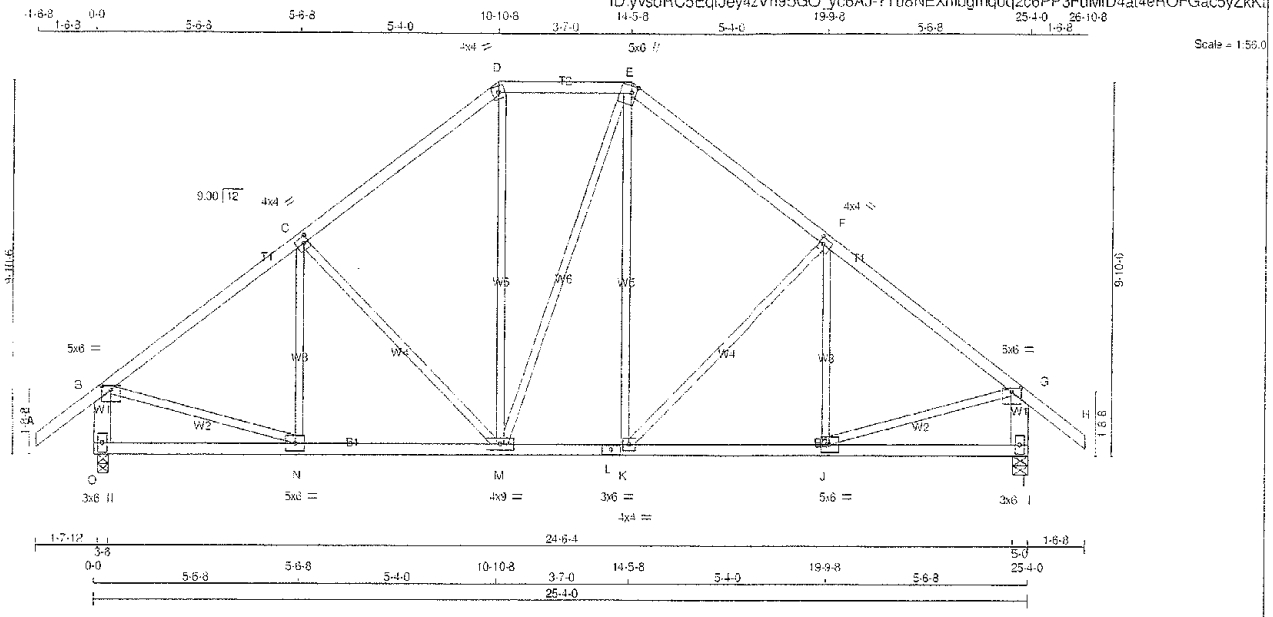


TOTAL WEIGHT = 126 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
O - B	2x6	DRY	No.2	SPF
I - G	2x6	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	5.0	6.0	1.25	3.00
C	TMWW-;	MT20	4.0	4.0	2.00	1.50
D	TTW-m	MT20	4.0	4.0		
E	TTWW-m	MT20	5.0	6.0	2.25	1.50
F	TMWW-;	MT20	4.0	4.0	2.00	1.50
G	TMWW-p	MT20	5.0	6.0	1.25	3.00
I	BMV1+p	MT20	3.0	6.0		
J	BMWW-;	MT20	5.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	9.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	1546	0	1546	0	0	5-0	5-0
O	1546	0	1546	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1091	731 / 0	0 / 0	0 / 0	0 / 0	359 / 0	0 / 0
O	1091	732 / 0	0 / 0	0 / 0	0 / 0	359 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.04 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

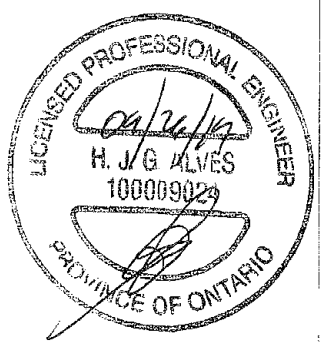
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 45	-91.8	-91.8	0.17 (1)	10.00	
B-C	-1415 / 0	-91.8	-91.8	0.39 (1)	5.04	
C-D	-1173 / 0	-91.8	-91.8	0.37 (1)	5.44	
D-E	-911 / 0	-91.8	-91.8	0.16 (1)	6.25	
E-F	-1172 / 0	-91.8	-91.8	0.37 (1)	5.44	
F-G	-1416 / 0	-91.8	-91.8	0.39 (1)	5.04	
G-H	0 / 45	-91.8	-91.8	0.17 (1)	10.00	
O-B	1504 / 0	0.0	0.0	0.11 (1)	7.81	
I-G	1504 / 0	0.0	0.0	0.11 (1)	7.81	
O-N	0 / 0	-18.5	-18.5	0.14 (4)	10.00	
N-M	0 / 1161	-18.5	-18.5	0.25 (1)	10.00	
M-L	0 / 910	-18.5	-18.5	0.19 (1)	10.00	
L-K	0 / 910	-18.5	-18.5	0.19 (1)	10.00	
K-J	0 / 1161	-18.5	-18.5	0.26 (1)	10.00	
J-I	0 / 0	-18.5	-18.5	0.14 (4)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N-C	-175 / 33	0.09 (1)
C-M	-361 / 0	0.44 (1)
M-D	0 / 336	0.08 (1)
M-E	0 / 3	0.00 (1)
K-E	0 / 333	0.07 (1)
K-F	-363 / 0	0.45 (1)
J-F	173 / 34	0.09 (1)
B-N	0 / 1196	0.27 (1)
J-G	0 / 1196	0.27 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.84")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.84")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.39/1.00 (F-G;1) , BC=0.26/1.00 (J-K;1) , WB=0.45/1.00 (F-K;1) , SSI=0.20/1.00 (F-G;1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.66 (G) (INPUT = 0.90)
JSI METAL= 0.33 (G) (INPUT = 1.00)

Structural component only
DWG# T-1924976

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405684	T32	1	1	GREENYORK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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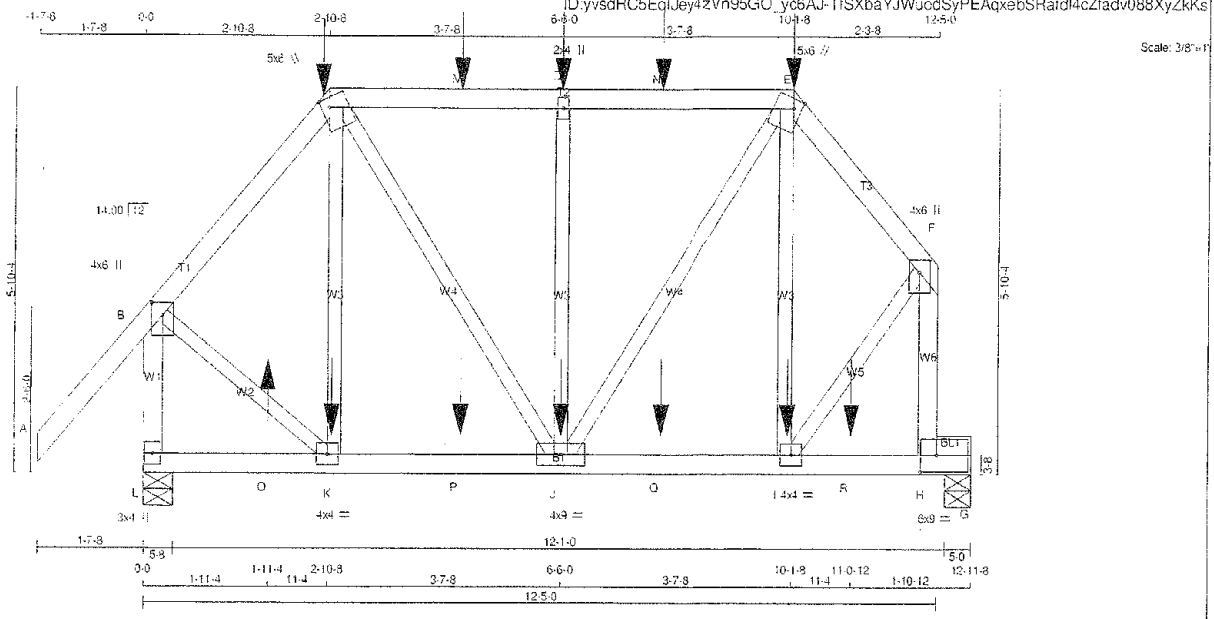


TOTAL WEIGHT = 70 lb

LUMBER

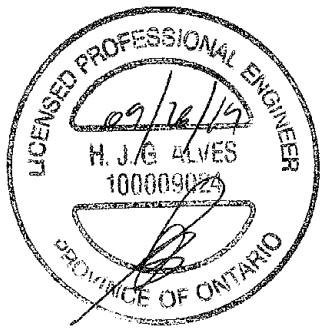
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - G	2x4	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	4.0	6.0	2.25	2.00
C	TTWW+m	MT20	5.0	3.0	1.75	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	1.75	1.50
F	TMWW+p	MT20	4.0	6.0	2.25	2.00
H	BMWKm	MT20	6.0	9.0	3.00	3.00
I	BMWW+	MT20	4.0	4.0		
J	BMWWW+t	MT20	4.0	9.0		
K	BMWW+t	MT20	4.0	4.0		
L	BMV1+p	MT20	3.0	4.0		



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
L	997	0	997	0	0	5-8	5-8
G	800	0	800	0	0	5-0	5-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	704	479 / 0	0 / 0	0 / 0	0 / 0	234 / 0	0 / 0
G	568	360 / 0	0 / 0	0 / 0	0 / 0	208 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC: MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 60	-91.8	-91.8	0.22 (1)	10.00	K-C	-169 / 3	0.09 (1)
B-C	-563 / 0	-91.8	-91.8	0.15 (1)	6.25	C-J	0 / 344	0.09 (5)
C-M	-541 / 0	-91.8	-91.8	0.25 (1)	6.25	J-D	-498 / 0	0.27 (1)
M-D	-541 / 0	-91.8	-91.8	0.25 (1)	6.25	J-E	0 / 306	0.07 (1)
D-N	-541 / 0	-91.8	-91.8	0.25 (1)	6.25	I-E	-98 / 22	0.05 (1)
N-E	-541 / 0	-91.8	-91.8	0.25 (1)	6.25	B-K	0 / 447	0.11 (1)
E-F	-587 / 0	-91.8	-91.8	0.10 (1)	6.25	I-F	0 / 670	0.17 (1)
L-B	-981 / 0	0.0	0.0	0.14 (1)	7.81			
H-F	-1065 / 0	0.0	0.0	0.20 (1)	7.81			
L-O	0 / 0	-18.5	-18.5	0.06 (4)	10.00			
O-K	0 / 0	-18.5	-18.5	0.06 (4)	10.00			
K-P	0 / 361	-18.5	-18.5	0.11 (1)	10.00			
P-J	0 / 361	-18.5	-18.5	0.11 (1)	10.00			
J-Q	0 / 377	-18.5	-18.5	0.26 (1)	10.00			
Q-I	0 / 377	-18.5	-18.5	0.26 (1)	10.00			
I-R	-55 / 0	-18.5	-18.5	0.02 (1)	6.25			
R-H	55 / 0	-18.5	-18.5	0.02 (1)	6.25			
H-G	0 / 0	18.5	-18.5	0.34 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC:	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-10-8		-47	-47	238	BACK	VERT	TOTAL	---	C1
D	6-6-0		-40	-40	---	BACK	VERT	TOTAL	---	C1
E	10-1-8		-47	-47	---	BACK	VERT	TOTAL	---	C1
I	10-0-12		-8	-8	---	BACK	VERT	TOTAL	---	C1
J	6-6-0		-8	-8	---	BACK	VERT	TOTAL	---	C1
K	2-11-4		-8	-8	---	BACK	VERT	TOTAL	---	C1
M	4-11-4		-40	-40	---	BACK	VERT	TOTAL	---	C1
N	8-0-12		-40	-40	---	BACK	VERT	TOTAL	---	C1
O	1-11-4		-8	-8	---	BACK	VERT	TOTAL	---	C1
P	4-11-4		-8	-8	---	BACK	VERT	TOTAL	---	C1
Q	8-0-12		-8	-8	---	BACK	VERT	TOTAL	---	C1
R	11-0-12		-8	-8		BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.25/1.00 (D-E:1) , BC=0.82/1.00 (H-I:1) , WB=0.27/1.00 (D-J:1) , SSI=0.34/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.71 (1) (INPUT = 0.90)
JSI METAL= 0.26 (3) (INPUT = 1.00)

Structural component only
DWG# T-1924977

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405684	T33	1	1	GREENYORK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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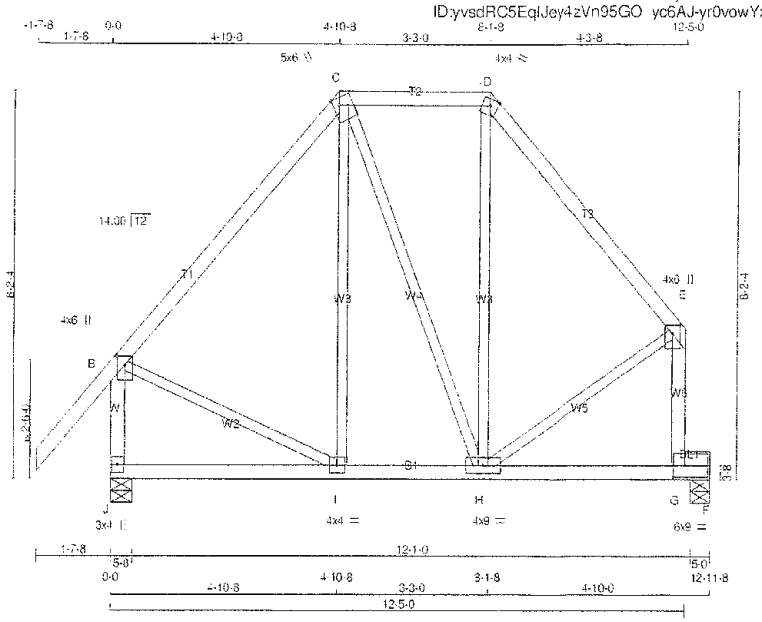

TOTAL WEIGHT = 71 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - F	2x4	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	BMVW+p	MT20	4.0	6.0	2.25	2.00
C	TTWW+m	MT20	5.0	6.0	1.75	1.50
D	TTW-m	MT20	4.0	4.0	Edge	
E	TMVW+p	MT20	4.0	6.0	2.25	2.00
G	BMVKm	MT20	6.0	9.0	3.00	3.00
H	BMWWW+t	MT20	4.0	9.0		
I	BMWW+t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	872	0	872	0	0	5-8	5-8
F	666	0	666	0	0	5-0	5-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	514	420 / 0	0 / 0	0 / 0	0 / 0	194 / 0	0 / 0
F	472	304 / 0	0 / 0	0 / 0	0 / 0	167 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 60	-91.8	-91.8	0.20 (1)		10.00
B- C	-429 / 0	-91.8	-91.8	0.39 (1)		6.25
C- D	-295 / 0	-91.8	-91.8	0.17 (1)		6.25
D- E	-455 / 0	-91.8	-91.8	0.30 (1)		6.25
J- B	-829 / 0	0.0	0.0	0.11 (1)		7.81
G- E	-758 / 0	0.0	0.0	0.14 (1)		7.81
J- I	0 / 0	-18.5	-18.5	0.10 (4)		10.00
I- H	0 / 277	-18.5	-18.5	0.26 (1)		10.00
H- G	-39 / 0	-18.5	-18.5	0.62 (1)		6.25
G- F	0 / 0	-18.5	-18.5	0.26 (1)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- C	-90 / 34	0.12 (1)
H- D	0 / 57	0.02 (4)
B- I	0 / 303	0.07 (1)
H- E	0 / 397	0.09 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 954 (0.18")

CSI: TC=0.39/1.00 (B-C:1) , BC=0.62/1.00 (G-H:1) , WB=0.12/1.00 (C-I:1) , SSI=0.26/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS SEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.49 (B) (INPUT = 0.90)
JSI METAL = 0.24 (B) (INPUT = 1.00)

Structural component only
DWG# T-1924978

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405684	T34	5	1	GREENYORK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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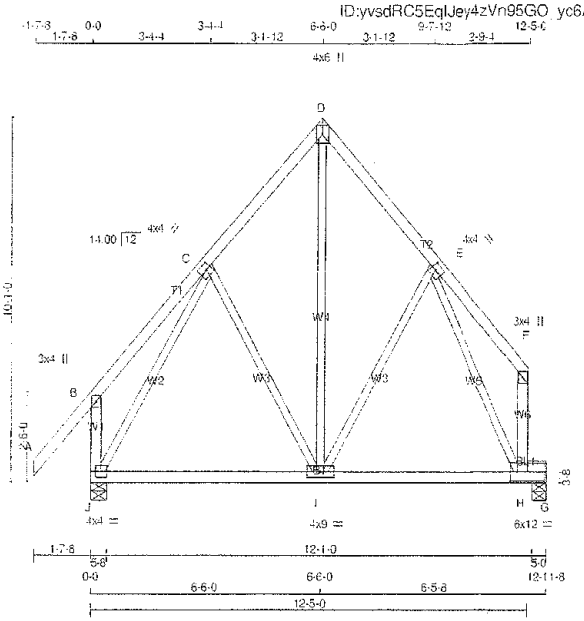


TOTAL WEIGHT = 5 X 75 = 373 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW+p	MT20	4.0	8.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.50
F	TMV+p	MT20	3.0	4.0		
H	BMVWK-t	MT20	6.0	12.0	3.00	2.50
I	BMWWW-t	MT20	4.0	9.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	666	0	666	0	0	5-0	5-0
J	872	0	872	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	472	304 / 0	0 / 0	0 / 0	0 / 0	167 / 0	0 / 0
J	614	420 / 0	0 / 0	0 / 0	0 / 0	194 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, J

BRACING

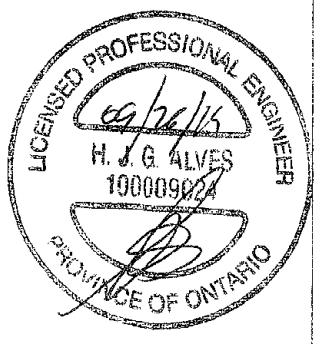
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS		
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 60	-91.8	-91.8	0.20 (1)	10.00	C-I	-110 / 0	0.09 (1)
B-C	0 / 26	-91.8	-91.8	0.16 (1)	10.00	I-D	0 / 357	0.08 (1)
C-D	-420 / 0	-91.8	-91.8	0.16 (1)	6.25	I-E	-9 / 27	0.01 (4)
D-E	-416 / 0	-91.8	-91.8	0.13 (1)	6.25	J-C	-650 / 0	0.53 (1)
E-F	0 / 34	-91.8	-91.8	0.13 (1)	10.00	E-H	-685 / 0	0.56 (1)
J-B	-271 / 0	0.0	0.0	0.04 (1)	7.81			
H-F	-79 / 0	0.0	0.0	0.01 (1)	7.81			
J-I	0 / 311	-18.5	-18.5	0.30 (1)	10.00			
I-H	0 / 261	-18.5	-18.5	0.66 (1)	10.00			
H-G	0 / 0	-18.5	-18.5	0.26 (1)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 564 (0.28")

CSI: TC=0.20/1.00 (A-B:1) , BC=0.66/1.00 (H-I:1) , WB=0.53/1.00 (C-J:1) , SSI=0.26/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1387	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.39 (E) (INPUT = 0.90)
JSI METAL= 0.25 (C) (INPUT = 1.00)

Structural component only
DWG# T-1924979

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405684	T35	1	1	GREENYORK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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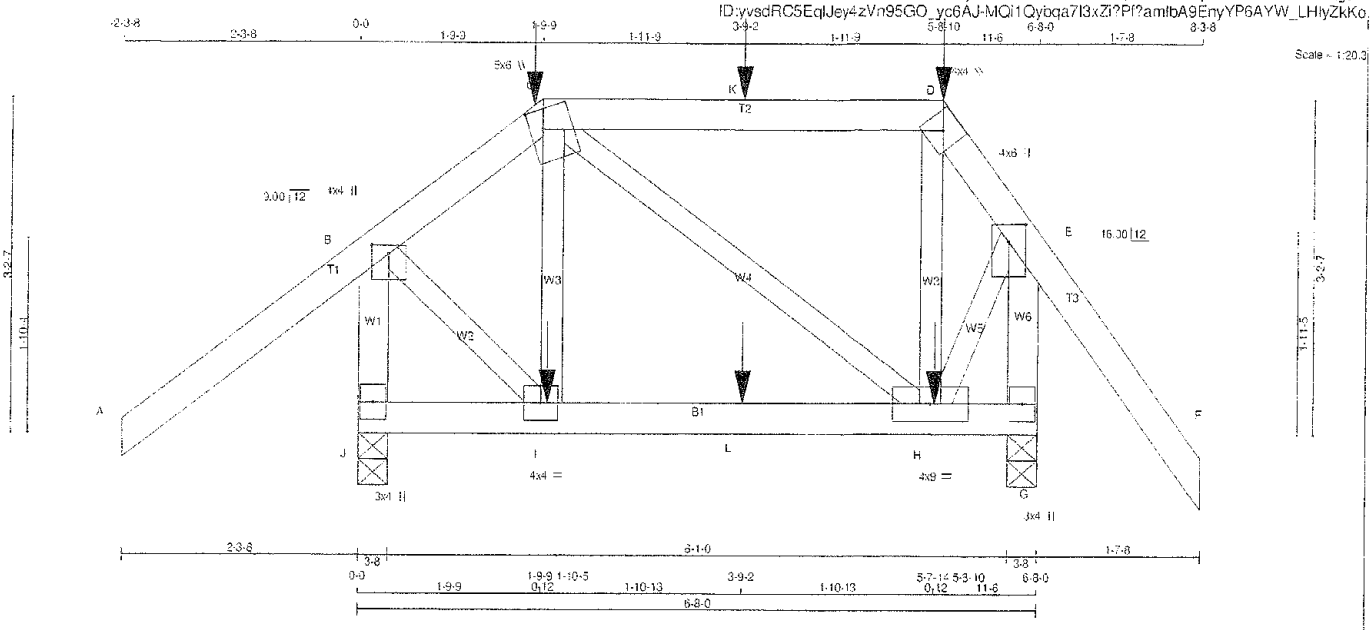


TOTAL WEIGHT = 38 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW-h	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	6.0	2.00	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	9.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	962	0	962	0	0	3-8	3-8
G	972	0	972	0	0	3-8	3-8

UNFACTORED REACTIONS

	1ST LCASE	MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	676	466 0	0 / 0	0 / 0	0 / 0	211 0	0 / 0
G	685	461 -22	0 / 0	0 / 0	0 / 0	225 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 65	-91.8	-91.8	0.61 (5)	10.00	I-C	0 / 189	0.05 (1)	
B-C	-526 / 0	-91.8	-91.8	0.59 (5)	6.25	C-H	-121 / 27	0.05 (1)	
C-K	-260 / 0	-91.8	-91.8	0.29 (1)	6.25	H-D	0 / 236	0.06 (1)	
K-D	-260 / 0	-91.8	-91.8	0.29 (1)	6.25	B-I	0 / 448	0.11 (1)	
D-E	-549 / 0	-91.8	-91.8	0.21 (1)	6.25	H-E	0 / 449	0.11 (1)	
E-F	0 / 64	-91.8	-91.8	0.22 (1)	10.00				
J-B	-991 / 0	0.0	0.0	0.12 (1)	7.81				
G-E	-1079 / 0	0.0	0.0	0.13 (1)	7.59				
J-I	0 / 0	-18.5	-18.5	0.13 (1)	10.00				
I-L	0 / 356	-18.5	-18.5	0.31 (1)	10.00				
L-H	0 / 356	-18.5	-18.5	0.31 (1)	10.00				
H-G	0 / 0	-18.5	-18.5	0.17 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	1-9-9	-25	-25	---	FRONT	VERT	TOTAL	---	C1
D	5-8-10	-25	-25	---	FRONT	VERT	TOTAL	---	C1
H	5-7-14	-251	-251	---	FRONT	VERT	TOTAL	---	C1
I	1-10-5	-251	-251	---	FRONT	VERT	TOTAL	---	C1
K	3-9-2	-18	-18	---	FRONT	VERT	TOTAL	---	C1
L	3-9-2	-251	-251	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.61/1.00 (A-B:5) , BC=0.31/1.00 (H-I:1) , WB=0.11/1.00 (E-H:1) , SSI=0.23/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

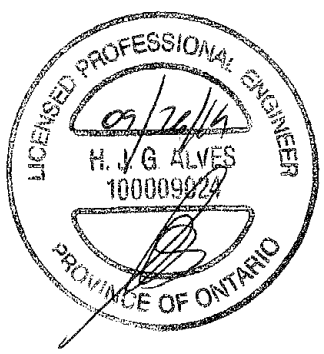
NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.73 (B) (INPUT = 0.90)
JSI METAL = 0.22 (E) (INPUT = 1.00)



Structural component only
DWG# T-1924980

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405684	P1	1	1	GREENYORK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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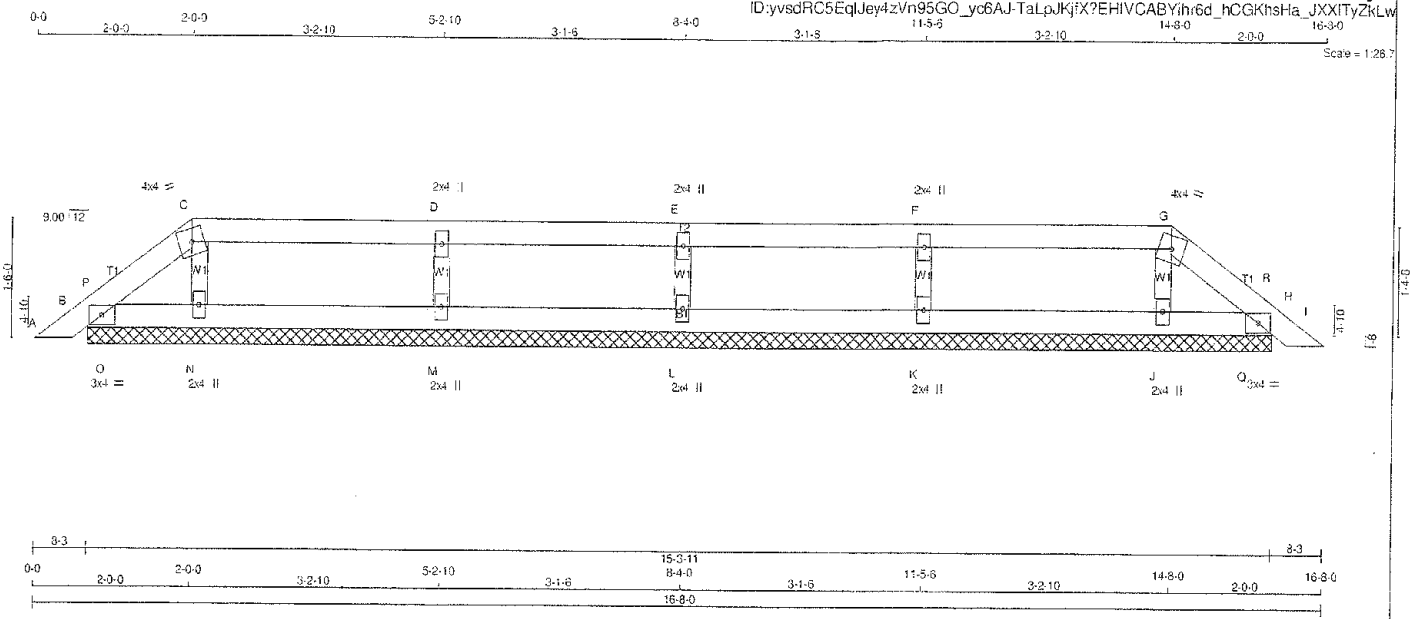


TOTAL WEIGHT = 41 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
B - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB1-I	MT20	3.0	4.0	
C	TTW-m	MT20	4.0	4.0	
D, E, F					
D	TMW+w	MT20	2.0	4.0	
G	TTW-m	MT20	4.0	4.0	
H	TMB1-I	MT20	3.0	4.0	
J, K, L, M, N					
J	BMW1+w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	137	0	137	0	0	15-3-11	15-3-11
H	137	0	137	0	0	15-3-11	15-3-11
N	210	0	210	0	0	15-3-11	15-3-11
M	391	0	391	0	0	15-3-11	15-3-11
L	319	0	319	0	0	15-3-11	15-3-11
K	391	0	391	0	0	15-3-11	15-3-11
J	210	0	210	0	0	15-3-11	15-3-11

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	95	74 / 0	0 / 0	0 / 0	0 / 0	21 / 0	0 / 0
H	95	74 / 0	0 / 0	0 / 0	0 / 0	21 / 0	0 / 0
N	150	90 / 0	0 / 0	0 / 0	0 / 0	60 / 0	0 / 0
M	276	185 / 0	0 / 0	0 / 0	0 / 0	91 / 0	0 / 0
L	226	146 / 0	0 / 0	0 / 0	0 / 0	80 / 0	0 / 0
K	276	185 / 0	0 / 0	0 / 0	0 / 0	91 / 0	0 / 0
J	150	90 / 0	0 / 0	0 / 0	0 / 0	60 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, H, N, M, L, K, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

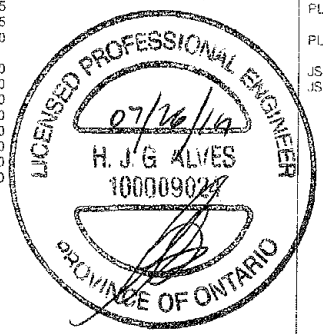
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD FROM	TO	LC1 (PLF)	MAX CSI (LC)	MAX UNBRAC LENGTH
A-B	0 / 15	-91.8	-91.8	0.02 (1)	10.00	
B-P	-26 / 0	-91.8	-91.8	0.00 (1)	6.25	
P-C	-30 / 0	-91.8	-91.8	0.02 (1)	6.25	
C-D	-8 / 0	-91.8	-91.8	0.14 (1)	10.00	
D-E	-8 / 0	-91.8	-91.8	0.14 (1)	10.00	
E-F	-8 / 0	-91.8	-91.8	0.14 (1)	10.00	
F-G	-8 / 0	-91.8	-91.8	0.14 (1)	10.00	
G-R	-30 / 0	-91.8	-91.8	0.02 (1)	6.25	
R-H	-26 / 0	-91.8	-91.8	0.00 (1)	6.25	
H-I	0 / 15	-91.8	-91.8	0.02 (1)	10.00	
B-O	0 / 23	-18.5	-18.5	0.02 (1)	10.00	
O-N	0 / 23	-18.5	-18.5	0.03 (4)	10.00	
N-M	0 / 8	-18.5	-18.5	0.04 (4)	10.00	
M-L	0 / 8	-18.5	-18.5	0.04 (4)	10.00	
L-K	0 / 8	-18.5	-18.5	0.04 (4)	10.00	
K-J	0 / 8	-18.5	-18.5	0.04 (4)	10.00	
J-Q	0 / 23	-18.5	-18.5	0.03 (4)	10.00	
Q-H	0 / 23	18.5	-18.5	0.02 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N-C	-146 / 0	0.02 (1)
M-D	-334 / 0	0.05 (1)
L-E	-262 / 0	0.04 (1)
K-F	-334 / 0	0.05 (1)
J-G	-146 / 0	0.02 (1)
O-P	-65 / 0	0.00 (1)
Q-R	-65 / 0	0.00 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (F-G:1) , BC=0.04/1.00 (M-N:4) , WB=0.05/1.00 (D-M:1) , SSI=0.14/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (M) (INPUT = 0.90)
JSI METAL= 0.07 (F) (INPUT = 1.00)

Structural component only
DWG# T-1924927

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405684	P2	1	1	GREENYORK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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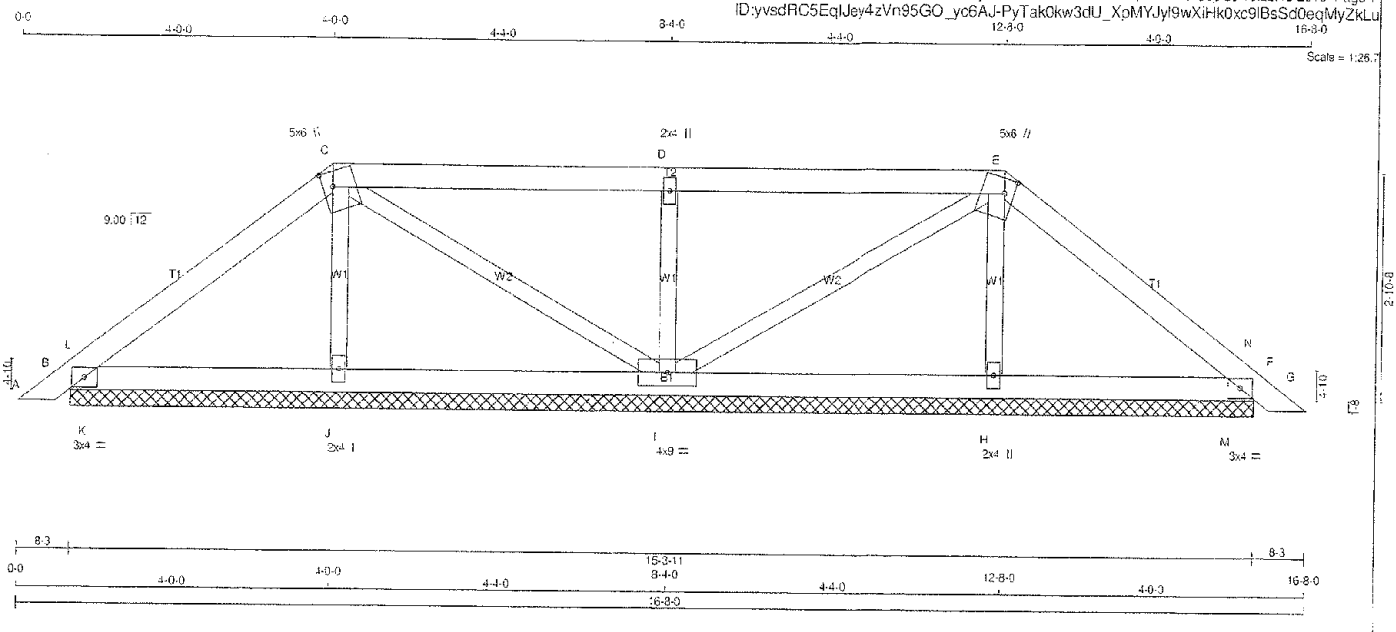


TOTAL WEIGHT = 52 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMB1-I	MT20	3.0	4.0		
H	BMW1+w	MT20	2.0	4.0		
I	BMWWW1-I	MT20	4.0	9.0		
J	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	284	0	284	0	0	15-3-11	15-3-11
F	284	0	284	0	0	15-3-11	15-3-11
J	304	0	304	0	0	15-3-11	15-3-11
I	620	0	620	0	0	15-3-11	15-3-11
H	304	0	304	0	0	15-3-11	15-3-11

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	199	144 / 0	0 / 0	0 / 0	0 / 0	55 / 0	0 / 0
F	199	144 / 0	0 / 0	0 / 0	0 / 0	55 / 0	0 / 0
J	217	128 / 0	0 / 0	0 / 0	0 / 0	89 / 0	0 / 0
I	436	300 / 0	0 / 0	0 / 0	0 / 0	135 / 0	0 / 0
H	217	128 / 0	0 / 0	0 / 0	0 / 0	89 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, J, I, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	(LC)	MAX. UNBRAC LENGTH
A- B	0 / 15	-91.8	-91.8	0.02	(1)		10.00
B- L	-23 / 0	-91.8	-91.8	0.05	(1)		6.25
L- C	-103 / 0	-91.8	-91.8	0.12	(1)		6.25
C- D	-19 / 0	-91.8	-91.8	0.29	(1)		6.25
D- E	-19 / 0	-91.8	-91.8	0.29	(1)		6.25
E- N	-103 / 0	-91.8	-91.8	0.12	(1)		6.25
N- F	-23 / 0	-91.8	-91.8	0.05	(1)		6.25
F- G	0 / 15	-91.8	-91.8	0.02	(1)		10.00
B- K	0 / 77	-18.5	-18.5	0.11	(1)		10.00
K- J	0 / 77	-18.5	-18.5	0.11	(1)		10.00
J- I	0 / 69	-18.5	-18.5	0.07	(1)		10.00
I- H	0 / 69	-18.5	-18.5	0.07	(1)		10.00
H- M	0 / 77	-18.5	-18.5	0.11	(1)		10.00
M- F	0 / 77	-18.5	-18.5	0.11	(1)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI	(LC)
J- C	-190 / 0	0.03	(1)
C- I	-58 / 0	0.02	(1)
I- D	-496 / 0	0.09	(1)
I- E	-58 / 0	0.02	(1)
H- E	-190 / 0	0.03	(1)
K- L	-281 / 0	0.00	(1)
M- N	-281 / 0	0.00	(1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.29/1.00 (C-D:1) , BC=0.11/1.00 (J-K:1) , WB=0.09/1.00 (D-I:1) , SSI=6.22:1.00 (B-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

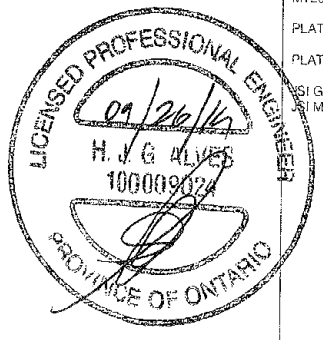
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1657	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

(SI GRIP= 0.26 (D) (INPUT = 0.90)
SI METAL= 0.10 (D) (INPUT = 1.00)



Structural component only
DWG# T-1924928

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO
405684	P3	1	1	GREENYORK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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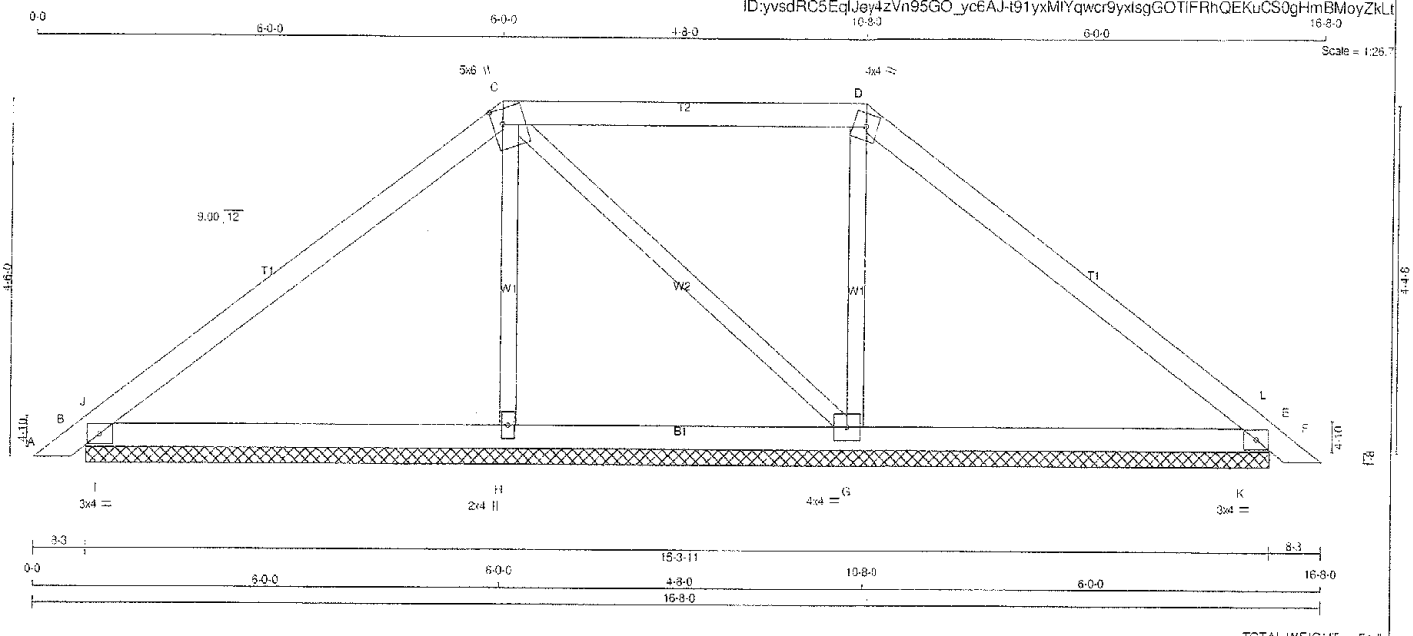


TOTAL WEIGHT = 51 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB14	MT20	3.0	4.0		
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW+m	MT20	4.0	4.0		
E	TMB14	MT20	3.0	4.0		
G	BMWW14	MT20	4.0	4.0		
H	BMW14+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	492	0	492	0	0	15-3-11	15-3-11
E	462	0	462	0	0	15-3-11	15-3-11
H	370	0	370	0	0	15-3-11	15-3-11
G	473	0	473	0	0	15-3-11	15-3-11

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	345	244 / 0	0 / 0	0 / 0	0 / 0	100 / 0	0 / 0
E	324	227 / 0	0 / 0	0 / 0	0 / 0	97 / 0	0 / 0
H	264	157 / 0	0 / 0	0 / 0	0 / 0	108 / 0	0 / 0
G	335	218 / 0	0 / 0	0 / 0	0 / 0	119 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

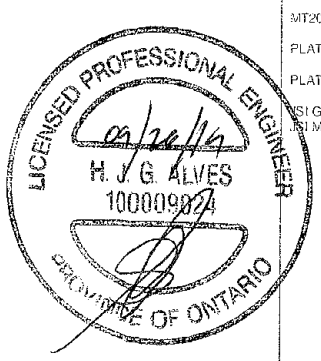
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 15	-91.8	-91.8	0.02 (1)	10.00	
B-J	-69 / 10	-91.8	-91.8	0.16 (1)	6.25	
J-C	-274 / 0	-91.8	-91.8	0.33 (1)	6.25	
C-D	-161 / 0	-91.8	-91.8	0.34 (1)	6.25	
D-L	-225 / 0	-91.8	-91.8	0.33 (1)	6.25	
L-E	-61 / 63	-91.8	-91.8	0.17 (1)	6.25	
E-F	0 / 15	-91.8	-91.8	0.02 (1)	10.00	
B-I	0 / 208	-18.5	-18.5	0.27 (1)	10.00	
I-H	0 / 208	-18.5	-18.5	0.27 (1)	10.00	
H-G	0 / 202	-18.5	-18.5	0.15 (1)	10.00	
G-K	0 / 169	-18.5	-18.5	0.27 (1)	10.00	
K-E	0 / 169	-18.5	-18.5	0.27 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H-C	-230 / 0	0.07 (1)
C-G	-55 / 0	0.03 (1)
G-D	-296 / 0	0.08 (1)
I-J	-655 / 0	0.00 (1)
K-L	-660 / 0	0.00 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.34/1.00 (C-D:1) , BC=0.27/1.00 (H-I:1) , WB=0.08/1.00 (D-G:1) , SSI=0.52/1.00 (E-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	613	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SSI GRIP = 0.41 (B) (INPUT = 0.90)
SSI METAL = 0.11 (B) (INPUT = 1.00)

Structural component only
DWG# T-1924929

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405684	P4	2	1	GREENYORK HOMES TRUSS DESC.	

Tamarack Roof Truss, Budington

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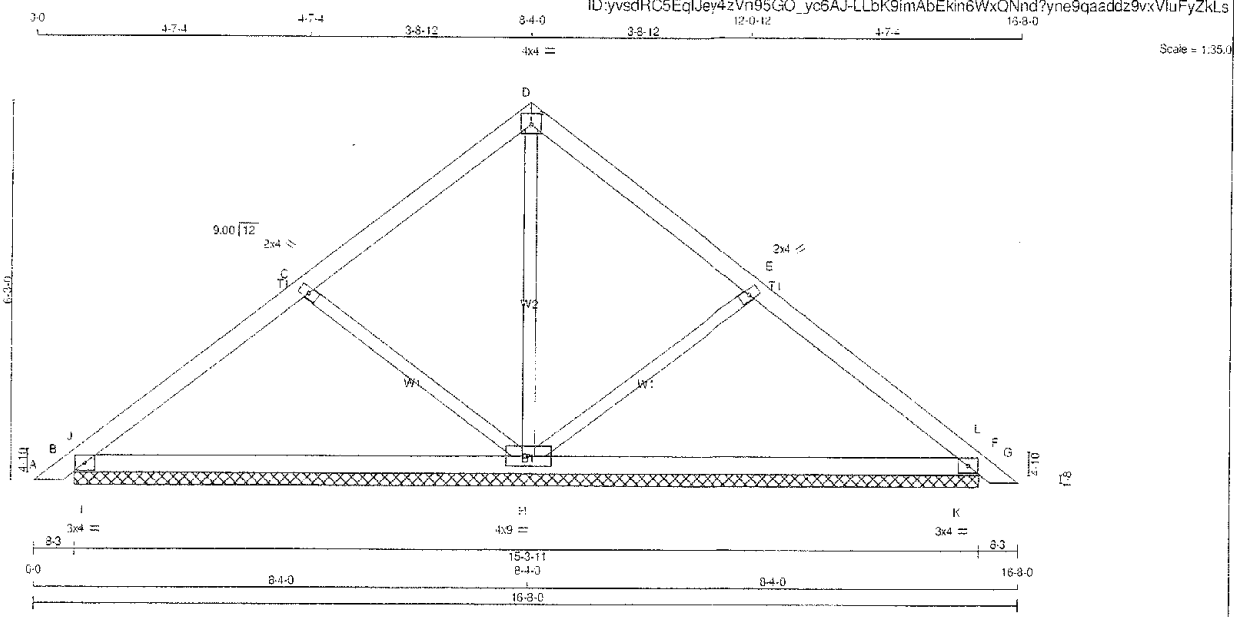


TOTAL WEIGHT = 2 X 53 = 106 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB14	MT20	3.0	4.0		
C	TMW+w	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMW+w	MT20	2.0	4.0		
F	TMB14	MT20	3.0	4.0		
H	BMWWW14	MT20	4.0	9.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	420	0	420	0	0	15-3-11	15-3-11
F	420	0	420	0	0	15-3-11	15-3-11
H	956	0	956	0	0	15-3-11	15-3-11

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	296	199 / 0	0 / 0	0 / 0	0 / 0	97 / 0	0 / 0
F	296	199 / 0	0 / 0	0 / 0	0 / 0	97 / 0	0 / 0
H	676	446 / 0	0 / 0	0 / 0	0 / 0	230 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF)	CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 15	-91.8	-91.8	0.02 (1)	10.00	
B- J	-415 / 0	-91.8	-91.8	0.12 (4)	6.25	
J- C	-262 / 0	-91.8	-91.8	0.23 (1)	6.25	
C- D	-5 / 39	-91.8	-91.8	0.23 (1)	10.00	
D- E	-5 / 39	-91.8	-91.8	0.23 (1)	10.00	
E- L	-262 / 0	-91.8	-91.8	0.23 (1)	6.25	
L- F	-415 / 0	-91.8	-91.8	0.12 (4)	6.25	
F- G	0 / 15	-91.8	-91.8	0.02 (1)	10.00	
B- I	0 / 237	-18.5	-18.5	0.10 (4)	10.00	
I- H	0 / 237	-18.5	-18.5	0.27 (4)	10.00	
H- K	0 / 237	-18.5	-18.5	0.27 (4)	10.00	
K- F	0 / 237	-18.5	-18.5	0.10 (4)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- D	-341 / 0	0.20 (1)
H- E	-364 / 0	0.14 (1)
C- H	-364 / 0	0.14 (1)
I- J	0 / 212	0.00 (1)
K- L	0 / 212	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.23/1.00 (C-D:1) , BC=0.27/1.00 (H-I:4) , WB=0.20/1.00 (D-H:1) , SSI=0.18/1.00 (B-J:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

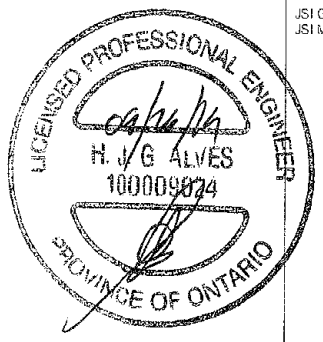
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1567	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg

JSI GRIP= 0.31 (F) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)



Structural component only
DWG# T-1924930

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405684	P4C	1	1	GREENYORK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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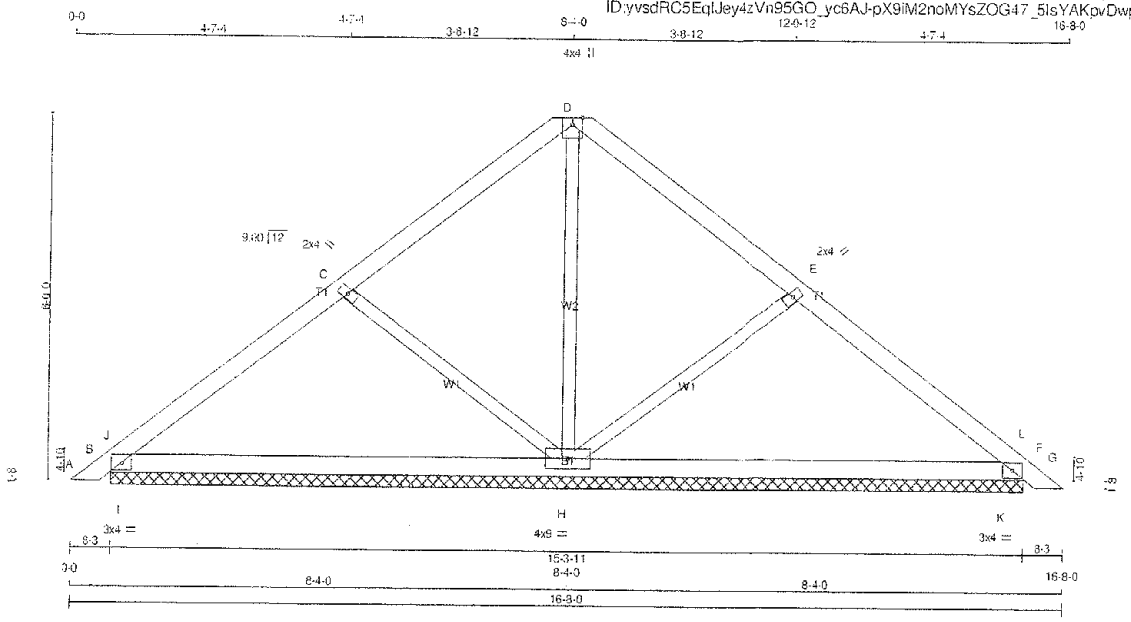


TOTAL WEIGHT = 52 lb (M

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB14	MT20	3.0	4.0		
C	TMW+w	MT20	2.0	4.0		
D	TMTMW+p	MT20	4.0	4.0	Edge	
E	TMW+w	MT20	2.0	4.0		
F	TMB14	MT20	3.0	4.0		
H	BMWWW14	MT20	4.0	9.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
B	420	0	420	0	0	15-3-11	15-3-11
F	420	0	420	0	0	15-3-11	15-3-11
H	956	0	956	0	0	15-3-11	15-3-11

UNFACTORED REACTIONS

	1ST LCASE	MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	296	199 / 0	0 / 0	0 / 0	0 / 0	97 / 0	0 / 0
F	296	199 / 0	0 / 0	0 / 0	0 / 0	97 / 0	0 / 0
H	678	448 / 0	0 / 0	0 / 0	0 / 0	230 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 15	-91.8	-91.8	0.02 (1)		10.00
B-J	-415 / 0	-91.8	-91.8	0.12 (4)		6.25
J-C	-262 / 0	-91.8	-91.8	0.23 (1)		6.25
C-D	-5 / 39	-91.8	-91.8	0.23 (1)		10.00
D-E	-5 / 39	-91.8	-91.8	0.23 (1)		10.00
E-L	-262 / 0	-91.8	-91.8	0.23 (1)		6.25
L-F	-415 / 0	-91.8	-91.8	0.12 (4)		6.25
F-G	0 / 15	-91.8	-91.8	0.02 (1)		10.00
B-I	0 / 237	-18.5	-18.5	0.10 (4)		10.00
I-H	0 / 237	-18.5	-18.5	0.27 (4)		10.00
H-K	0 / 237	-18.5	-18.5	0.27 (4)		10.00
K-F	0 / 237	-18.5	-18.5	0.10 (4)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H-D	-341 / 0	0.20 (1)
H-E	-364 / 0	0.14 (1)
C-H	-364 / 0	0.14 (1)
I-J	0 / 212	0.00 (1)
K-L	0 / 212	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF SCBC 2012 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.23/1.00 (C-D:1) , BC=0.27/1.00 (H-K:4) , WB=0.20/1.00 (D-H:1) , SSI=0.18/1.00 (B-J:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

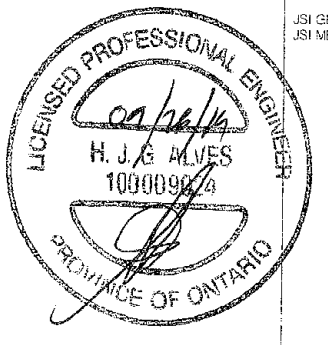
NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.31 (B) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)



Structural component only
DWG# T-1924931

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405684	P6	1	1	GREENYORK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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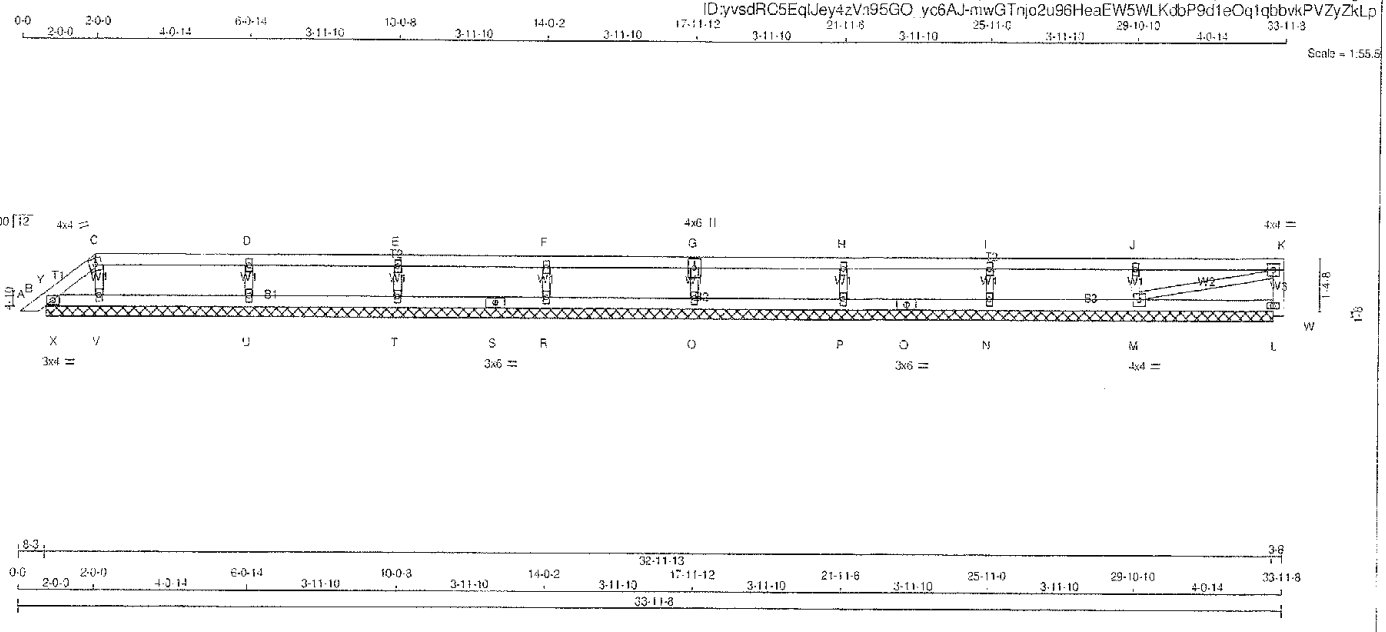

TOTAL WEIGHT = 87 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - K	2x4	DRY	No.2	SPF
W - K	2x4	DRY	No.2	SPF
B - S	2x4	DRY	No.2	SPF
S - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB14	MT20	3.0	4.0		
C	TTW-m	MT20	4.0	4.0		
D, E, F, H, I, J						
D	TMW+w	MT20	2.0	4.0		
G	TSW+l	MT20	4.0	6.0		
K	TMVW-t	MT20	4.0	4.0		
L	BVM1-l	MT20	2.0	4.0		
M	BMWW1-t	MT20	4.0	4.0		
N, P, Q, R, T, U, V						
N	BMW1+w	MT20	2.0	4.0		
O	BS-t	MT20	3.0	6.0		
S	BS-t	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	129	0	129	0	0	32-11-13 (9-1[illegible]-13	
V	256	0	256	0	0	32-11-13 (9-1[illegible]-13	
U	497	0	497	0	0	32-11-13 (9-1[illegible]-13	
T	409	0	409	0	0	32-11-13 (9-1[illegible]-13	
R	490	0	490	0	0	32-11-13 (9-1[illegible]-13	
Q	359	0	359	0	0	32-11-13 (9-1[illegible]-13	
P	491	0	491	0	0	32-11-13 (9-1[illegible]-13	
N	407	0	407	0	0	32-11-13 (9-1[illegible]-13	
M	507	0	507	0	0	32-11-13 (9-1[illegible]-13	
L	178	0	178	0	0	32-11-13 (9-1[illegible]-13	

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	89	72 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
V	183	108 / 0	0 / 0	0 / 0	0 / 0	75 / 0	0 / 0
U	351	235 / 0	0 / 0	0 / 0	0 / 0	116 / 0	0 / 0
T	290	188 / 0	0 / 0	0 / 0	0 / 0	102 / 0	0 / 0
R	346	232 / 0	0 / 0	0 / 0	0 / 0	113 / 0	0 / 0
Q	255	160 / 0	0 / 0	0 / 0	0 / 0	96 / 0	0 / 0
P	346	232 / 0	0 / 0	0 / 0	0 / 0	114 / 0	0 / 0
N	288	188 / 0	0 / 0	0 / 0	0 / 0	100 / 0	0 / 0
M	358	236 / 0	0 / 0	0 / 0	0 / 0	123 / 0	0 / 0
L	126	83 / 0	0 / 0	0 / 0	0 / 0	43 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, V, U, T, R, Q, P, N, M, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 15	-91.8	-91.8	0.02 (1)	10.00
B-Y	-16 / 4	-91.8	-91.8	0.01 (4)	6.25
Y-C	-26 / 0	-91.8	-91.8	0.01 (1)	6.25
C-D	-1 / 0	-91.8	-91.8	0.22 (1)	10.00
D-E	-2 / 0	-91.8	-91.8	0.22 (1)	10.00
E-F	-2 / 0	-91.8	-91.8	0.21 (1)	10.00
F-G	-2 / 0	-91.8	-91.8	0.21 (1)	10.00
G-H	-2 / 0	-91.8	-91.8	0.21 (1)	10.00
H-I	-2 / 0	-91.8	-91.8	0.21 (1)	10.00
I-J	-2 / 0	-91.8	-91.8	0.22 (1)	10.00
J-K	-2 / 0	-91.8	-91.8	0.22 (1)	10.00
W-L	0 / 0	0.0	0.0	0.00 (1)	10.00
L-K	-148 / 0	0.0	0.0	0.02 (1)	7.81
B-X	0 / 19	-18.5	-18.5	0.02 (1)	10.00
X-V	0 / 19	-18.5	-18.5	0.05 (4)	10.00
V-U	0 / 2	-18.5	-18.5	0.06 (4)	10.00
U-T	0 / 2	-18.5	-18.5	0.06 (4)	10.00
T-S	0 / 2	-18.5	-18.5	0.06 (4)	10.00
S-R	0 / 2	-18.5	-18.5	0.06 (4)	10.00
R-Q	0 / 2	-18.5	-18.5	0.06 (4)	10.00
Q-P	0 / 2	-18.5	-18.5	0.06 (4)	10.00
P-O	0 / 2	-18.5	-18.5	0.06 (4)	10.00
O-N	0 / 2	-18.5	-18.5	0.06 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V-C	-178 / 0	0.03 (1)
U-D	-423 / 0	3.06 (1)
T-E	-336 / 0	3.05 (1)
R-F	-416 / 0	3.06 (1)
Q-G	-286 / 0	0.04 (1)
P-H	-416 / 0	0.06 (1)
N-I	-336 / 0	0.05 (1)
M-J	-423 / 0	0.06 (1)
M-K	0 / 2	0.00 (1)
X-Y	-75 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.22/1.00 (D-E:1) , BC=0.07/1.00 (M-N:4) , WB=0.06/1.00 (D-U:1) , SSI=0.18/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (M) (INPUT = 0.90)
JSI METAL= 0.13 (L) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO
09/26/19

Structural component only 1/2
DWG# T-1924932

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENYORK HOMES	DRWG NO.
405684	P6	1	1	TRUSS DESC.	

LOADING
TOTAL LOAD CASES: (4)

CHORDS						WEBS		
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI	MAX (LC)	MAX UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N-M	0 : 2	-18.5	-18.5	0.07	(4)	10.00			
M-L	0 : 0	-18.5	-18.5	0.07	(4)	10.00			

LICENSED PROFESSIONAL ENGINEER
09/26/19
H. J/G KLYES
100090924
PROVINCE OF ONTARIO

Structural component only
DWG# T-1924932

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405684	P6A	1	1	GREENYORK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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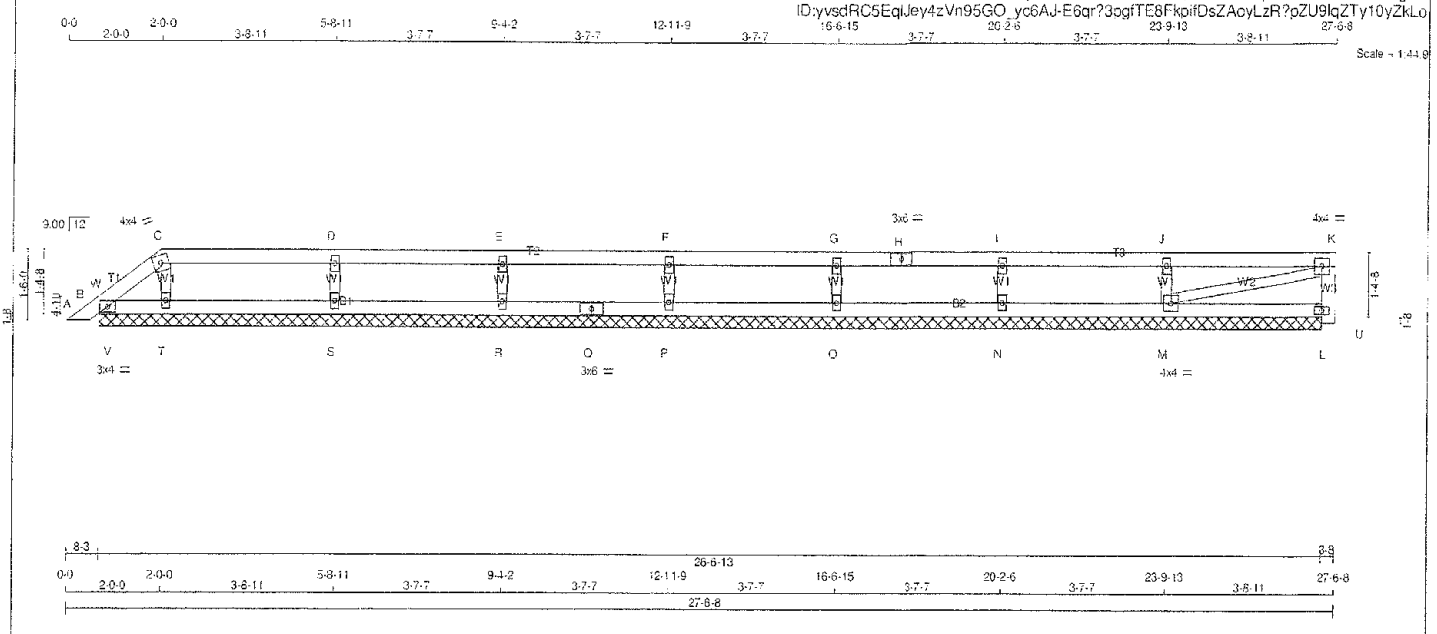


TOTAL WEIGHT = 71 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
U - K	2x4	DRY	No.2	SPF
B - Q	2x4	DRY	No.2	SPF
Q - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-t	MT20	3.0	4.0		
C	TTW-m	MT20	4.0	4.0		
D, E, F, G, I, J						
D	TMW+w	MT20	2.0	4.0		
H	TS-t	MT20	3.0	6.0		
K	TMVW-t	MT20	4.0	4.0		
L	BVM1-t	MT20	2.0	4.0		
M	BMWW1-t	MT20	4.0	4.0		
N, O, P, R, S, T						
N	BMW1+w	MT20	2.0	4.0		
Q	BS-t	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	131	0	131	0	0	26-6-13 (15-1 2618-13	
T	239	0	239	0	0	26-6-13 (15-1 2618-13	
S	451	0	451	0	0	26-6-13 (15-1 2618-13	
R	385	0	385	0	0	26-6-13 (15-1 2618-13	
P	402	0	402	0	0	26-6-13 (15-1 2618-13	
O	402	0	402	0	0	26-6-13 (15-1 2618-13	
N	383	0	383	0	0	26-6-13 (15-1 2618-13	
M	460	0	460	0	0	26-6-13 (15-1 2618-13	
L	163	0	163	0	0	26-6-13 (15-1 2618-13	

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	90	72 / 0	0 / 0	0 / 0	0 / 0	19 / 0	0 / 0
T	171	102 / 0	0 / 0	0 / 0	0 / 0	69 / 0	0 / 0
S	318	213 / 0	0 / 0	0 / 0	0 / 0	105 / 0	0 / 0
R	273	178 / 0	0 / 0	0 / 0	0 / 0	95 / 0	0 / 0
P	284	187 / 0	0 / 0	0 / 0	0 / 0	97 / 0	0 / 0
O	285	187 / 0	0 / 0	0 / 0	0 / 0	98 / 0	0 / 0
N	270	178 / 0	0 / 0	0 / 0	0 / 0	93 / 0	0 / 0
M	325	213 / 0	0 / 0	0 / 0	0 / 0	112 / 0	0 / 0
L	115	76 / 0	0 / 0	0 / 0	0 / 0	40 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, T, S, R, P, O, N, M, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 15	-91.8	-91.8	0.02 (1)		10.00
B-W	-17 / 1	-91.8	-91.8	0.00 (4)		6.25
W-C	-25 / 0	-91.8	-91.8	0.02 (1)		6.25
C-D	-2 / 0	-91.8	-91.8	0.18 (1)		10.00
D-E	-2 / 0	-91.8	-91.8	0.18 (1)		10.00
E-F	-2 / 0	-91.8	-91.8	0.14 (1)		10.00
F-G	-2 / 0	-91.8	-91.8	0.14 (1)		10.00
G-H	-2 / 0	-91.8	-91.8	0.14 (1)		10.00
H-I	-2 / 0	-91.8	-91.8	0.14 (1)		10.00
I-J	-2 / 0	-91.8	-91.8	0.18 (1)		10.00
J-K	-2 / 0	-91.8	-91.8	0.18 (1)		10.00
U-L	0 / 0	0.0	0.0	0.00 (1)		10.00
L-K	-138 / 0	0.0	0.0	0.01 (1)		7.81
S-V	0 / 18	-18.5	-18.5	0.02 (1)		10.00
V-T	0 / 18	-18.5	-18.5	0.04 (2)		10.00
T-S	0 / 2	-18.5	-18.5	0.05 (4)		10.00
S-R	0 / 2	-18.5	-18.5	0.05 (4)		10.00
R-Q	0 / 2	-18.5	-18.5	0.05 (4)		10.00
Q-P	0 / 2	-18.5	-18.5	0.05 (4)		10.00
P-O	0 / 2	-18.5	-18.5	0.05 (4)		10.00
O-N	0 / 2	-18.5	-18.5	0.05 (4)		10.00
N-M	0 / 2	-18.5	-18.5	0.06 (4)		10.00
M-L	0 / 0	-18.5	-18.5	0.06 (4)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T-C	-167 / 0	0.02 (1)
S-D	-383 / 0	0.05 (1)
R-E	-318 / 0	0.04 (1)
P-F	-335 / 0	0.05 (1)
O-G	-335 / 0	0.05 (1)
N-I	-318 / 0	0.04 (1)
M-J	-383 / 0	0.05 (1)
M-K	0 / 2	0.00 (1)
V-W	-71 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2019, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.18/1.00 (D-E:1) , BC=0.06/1.00 (L-M:4) , WB=0.05/1.00 (D-S:1) , SSI=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

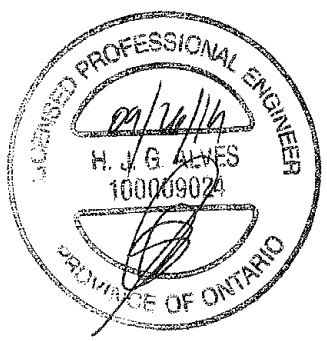
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.65 (M) (INPUT = 0.90)
JSI METAL= 0.12 (L) (INPUT = 1.00)



Structural component only
DWG# T-1924933

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405684	P7	1	1	GREENYORK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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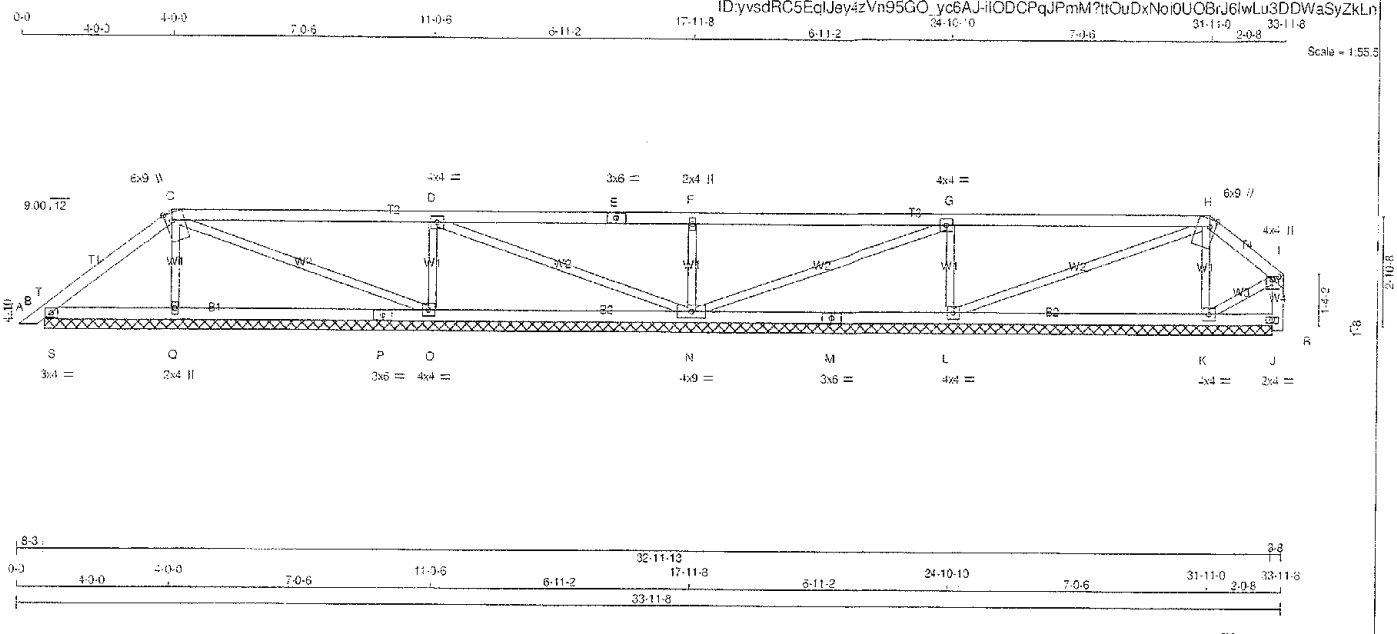


TOTAL WEIGHT = 114 lb

[M]

LUMBER

N. L. G. A. RULES

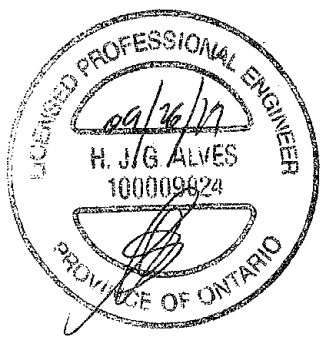
CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
H - I	2x4	DRY	No.2	SPF
R - I	2x4	DRY	No.2	SPF
B - P	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-t	MT20	3.0	4.0		
C	TTWW+m	MT20	8.0	9.0	Edge	2.00
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW+m	MT20	6.0	9.0	Edge	2.00
I	TMVW+p	MT20	4.0	4.0	1.00	2.00
J	BVMI-I	MT20	2.0	4.0		
K, L, O						
K	BMWW1-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW1-t	MT20	4.0	9.0		
P	BS-t	MT20	3.0	6.0		
Q	BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	280	0	280	0	0	32-11-13	(11-32-12-13
Q	445	0	445	0	0	32-11-13	(11-32-12-13
O	855	0	855	0	0	32-11-13	(11-32-12-13
N	757	0	757	0	0	32-11-13	(11-32-12-13
L	854	0	854	0	0	32-11-13	(11-32-12-13
K	381	0	381	0	0	32-11-13	(11-32-12-13
J	152	0	152	0	0	32-11-13	(11-32-12-13

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX. MIN. COMPONENT REACTIONS SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
B	194	148 / 0	0 / 0	0 / 0	0 / 0	46 / 0	0 / 0
Q	318	187 / 0	0 / 0	0 / 0	0 / 0	131 / 0	0 / 0
O	603	403 / 0	0 / 0	0 / 0	0 / 0	200 / 0	0 / 0
N	535	352 / 0	0 / 0	0 / 0	0 / 0	183 / 0	0 / 0
L	603	403 / 0	0 / 0	0 / 0	0 / 0	200 / 0	0 / 0
K	275	153 / 0	0 / 0	0 / 0	0 / 0	122 / 0	0 / 0
J	194	88 / 0	0 / 0	0 / 0	0 / 0	15 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, Q, O, N, L, K, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 15	-91.8	-91.8	0.02 (1)	10.00	
B- T	-2 / 2	-91.8	-91.8	0.06 (1)	10.00	
T- C	-108 / 0	-91.8	-91.8	0.12 (1)	6.25	
C- D	-24 / 0	-91.8	-91.8	0.66 (1)	6.25	
D- E	0 / 39	-91.8	-91.8	0.66 (1)	10.00	
E- F	0 / 39	-91.8	-91.8	0.66 (1)	10.00	
F- G	0 / 39	-91.8	-91.8	0.66 (1)	10.00	
G- H	-16 / 0	-91.8	-91.8	0.66 (1)	6.25	
H- I	-66 / 0	-91.8	-91.8	0.07 (1)	6.25	
R- J	0 / 0	0.0	0.0	0.00 (1)	10.00	
J- I	-159 / 0	0.0	0.0	0.02 (1)	7.81	
B- S	0 / 80	-18.5	-18.5	0.12 (1)	10.00	
S- Q	0 / 80	-18.5	-18.5	0.14 (4)	10.00	
Q- P	0 / 67	-18.5	-18.5	0.19 (4)	10.00	
P- O	0 / 67	-18.5	-18.5	0.19 (4)	10.00	
O- N	0 / 24	-18.5	-18.5	0.12 (4)	10.00	
N- M	0 / 16	-18.5	-18.5	0.19 (4)	10.00	
M- L	0 / 16	-18.5	-18.5	0.19 (4)	10.00	
L- K	0 / 41	-18.5	-18.5	0.19 (4)	10.00	
K- J	0 / 0	-18.5	-18.5	0.13 (4)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-305 / 0	0.05 (1)
C- O	-46 / 0	0.05 (1)
O- D	-710 / 0	0.12 (1)
D- N	-67 / 0	0.07 (1)
N- F	-586 / 0	0.10 (1)
N- G	-58 / 0	0.06 (1)
L- G	-713 / 0	0.12 (1)
L- H	-26 / 0	0.03 (1)
K- H	-299 / 0	0.05 (1)
K- I	0 / 58	0.01 (1)
S- T	-306 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.66/1.00 (D-F:1) , BC=0.19/1.00 (K-L:4) , WB=0.12/1.00 (G-L:1) , SSI=0.31/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP:DRY (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1567	798	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (O) (INPUT = 0.90)
JSI METAL= 0.27 (E) (INPUT = 1.00)

Structural component only
DWG# T-1924934

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405684	P7A	1	1	GREENYORK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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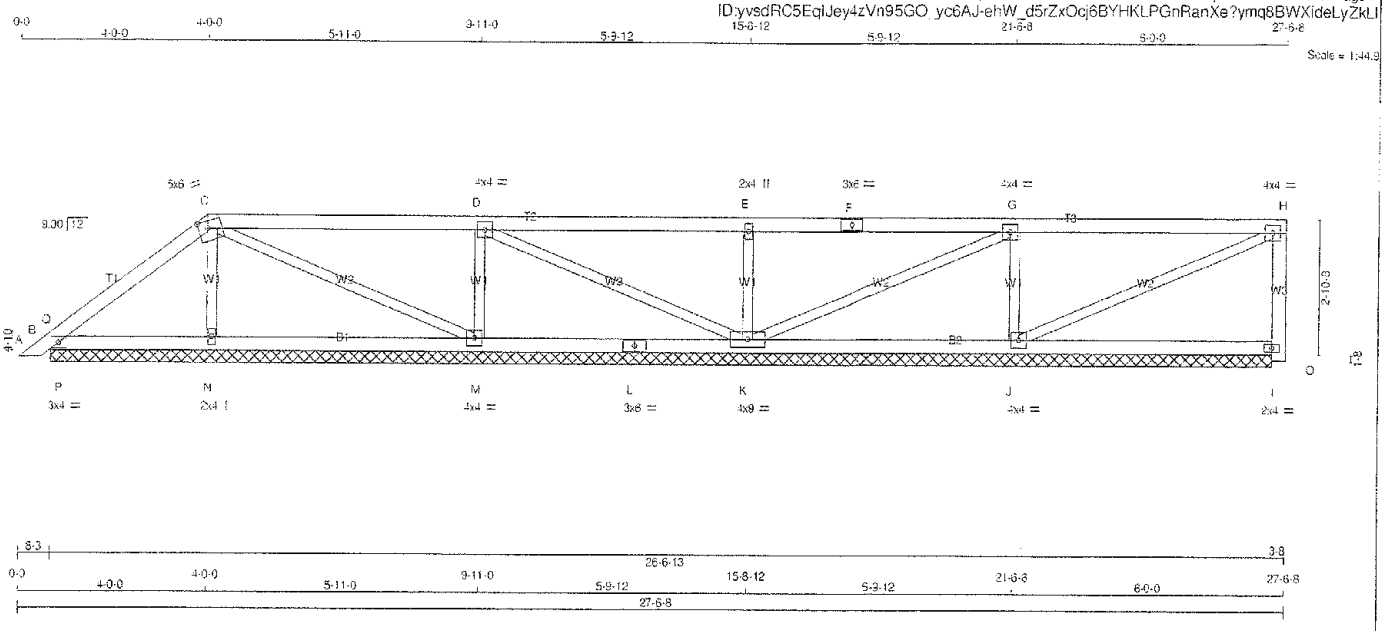


TOTAL WEIGHT = 93 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
C - H	2x4	DRY	No.2	SPF
B - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-t	MT20	3.0	4.0		
C	TTWW-m	MT20	5.0	6.0	1.75	2.25
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TMWW-t	MT20	4.0	4.0		
I	BVM1-t	MT20	2.0	4.0		
J	BMWW1-t	MT20	4.0	4.0		
K	BMWWW1-t	MT20	4.0	9.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW1-t	MT20	4.0	4.0		
N	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	284	0	284	0	0	26-6-13 (13-12/8-13	
N	387	0	387	0	0	26-6-13 (13-12/8-13	
M	712	0	712	0	0	26-6-13 (13-12/8-13	
K	849	0	849	0	0	26-6-13 (13-12/8-13	
J	720	0	720	0	0	26-6-13 (13-12/8-13	
I	264	0	264	0	0	26-6-13 (13-12/8-13	

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	198	147 / 0	0 / 0	0 / 0	0 / 0	51 / 0	0 / 0
N	277	164 / 0	0 / 0	0 / 0	0 / 0	113 / 0	0 / 0
M	503	335 / 0	0 / 0	0 / 0	0 / 0	168 / 0	0 / 0
K	458	305 / 0	0 / 0	0 / 0	0 / 0	153 / 0	0 / 0
J	509	332 / 0	0 / 0	0 / 0	0 / 0	178 / 0	0 / 0
I	187	122 / 0	0 / 0	0 / 0	0 / 0	64 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, N, M, K, J, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 15	-91.8	-91.8	0.02 (1)	10.00
B- Q	-12 / 0	-91.8	-91.8	0.05 (1)	6.25
Q- C	-108 / 0	-91.8	-91.8	0.12 (1)	6.25
C- D	-23 / 0	-91.8	-91.8	0.46 (1)	6.25
D- E	0 / 48	-91.8	-91.8	0.47 (1)	10.00
E- F	0 / 48	-91.8	-91.8	0.48 (1)	10.00
F- G	0 / 48	-91.8	-91.8	0.48 (1)	10.00
G- H	-5 / 0	-91.8	-91.8	0.47 (1)	10.00
O- I	0 / 0	0.0	0.0	0.00 (1)	10.00
I- H	-220 / 0	0.0	0.0	0.04 (1)	7.81
B- P	0 / 80	-18.5	-18.5	0.12 (1)	10.00
P- N	0 / 80	-18.5	-18.5	0.12 (1)	10.00
N- M	0 / 69	-18.5	-18.5	0.13 (4)	10.00
M- L	0 / 23	-18.5	-18.5	0.13 (4)	10.00
L- K	0 / 23	-18.5	-18.5	0.13 (4)	10.00
K- J	0 / 6	-18.5	-18.5	0.16 (4)	10.00
J- I	0 / 0	-18.5	-18.5	0.16 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- C	-261 / 0	0.04 (1)
C- M	-50 / 0	0.04 (1)
M- D	-585 / 0	0.10 (1)
D- K	-78 / 0	0.06 (1)
K- E	-489 / 0	0.08 (1)
K- G	-59 / 0	0.04 (1)
J- G	598 / 0	0.10 (1)
J- H	0 / 7	0.06 (1)
P- Q	292 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.48/1.00 (E-G:1) , BC=0.16/1.00 (J-K:4) , WB=0.10/1.00 (G-J:1) , SSI=0.26/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

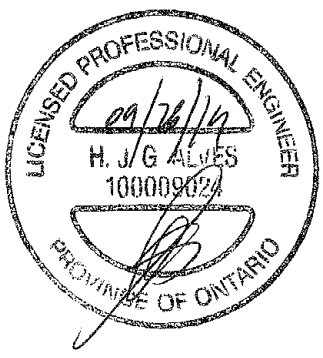
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	798	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (C) (INPUT = 0.80)
JSI METAL= 0.26 (F) (INPUT = 1.00)



Structural component only
DWG# T-1924935

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405684	P8	1	1	GREENYORK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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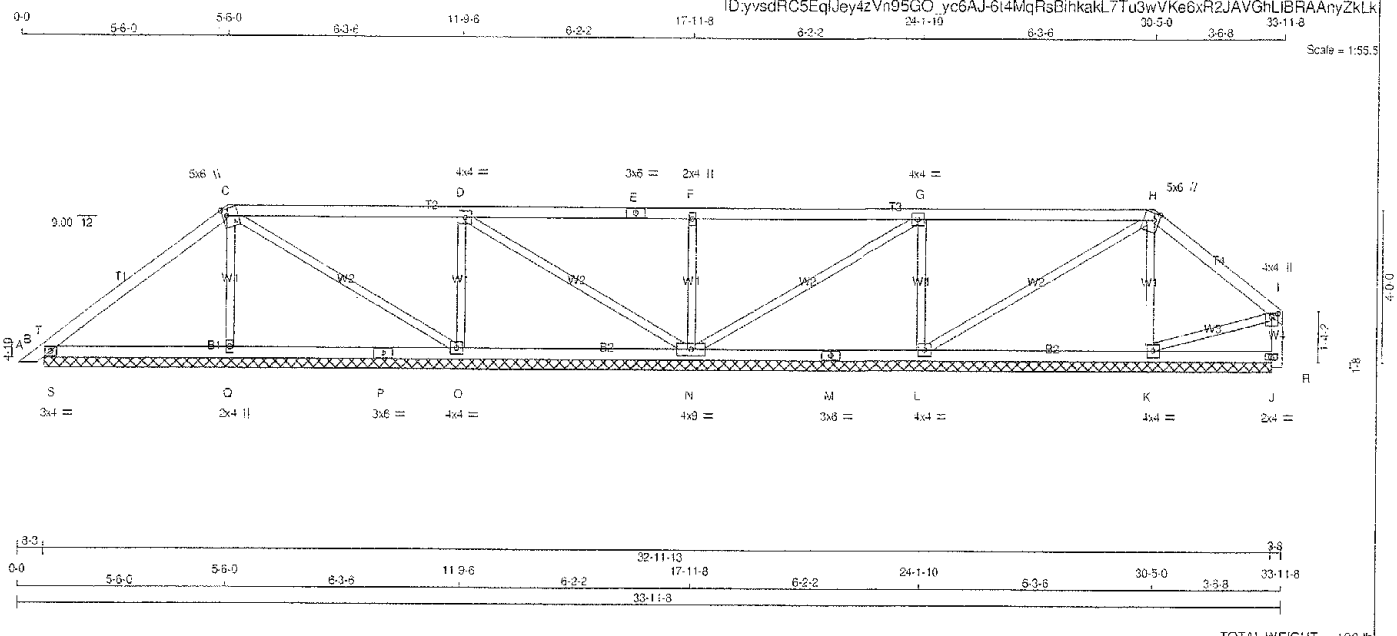

TOTAL WEIGHT = 120 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
H - I	2x4	DRY	No.2	SPF
R - I	2x4	DRY	No.2	SPF
B - P	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW+m	MT20	5.0	6.0	2.25	1.50
I	TMVW+p	MT20	4.0	4.0	1.00	2.00
J	BWM1-I	MT20	2.0	4.0		
K, L, O						
K	BMWW1-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW1-t	MT20	4.0	9.0		
P	BS-t	MT20	3.0	6.0		
Q	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	404	0	404	0	0	32-11-13	11-32-12-13
Q	429	0	429	0	0	32-11-13	11-32-12-13
O	738	0	738	0	0	32-11-13	11-32-12-13
N	762	0	762	0	0	32-11-13	11-32-12-13
L	742	0	742	0	0	32-11-13	11-32-12-13
K	391	0	391	0	0	32-11-13	11-32-12-13
J	258	0	258	0	0	32-11-13	11-32-12-13

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	282	203 / 0	0 / 0	0 / 0	0 / 0	79 / 0	0 / 0
Q	307	180 / 0	0 / 0	0 / 0	0 / 0	127 / 0	0 / 0
O	521	346 / 0	0 / 0	0 / 0	0 / 0	175 / 0	0 / 0
N	537	362 / 0	0 / 0	0 / 0	0 / 0	176 / 0	0 / 0
L	524	348 / 0	0 / 0	0 / 0	0 / 0	176 / 0	0 / 0
K	281	162 / 0	0 / 0	0 / 0	0 / 0	119 / 0	0 / 0
J	180	132 / 0	0 / 0	0 / 0	0 / 0	48 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, Q, O, N, L, K, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	LC	MAX. UNBRAC LENGTH
A-B	0 / 15	-91.8	-91.8	0.02	(1)		10.00
B-T	-29 / 61	-91.8	-91.8	0.14	(1)		6.25
T-C	-178 / 0	-91.8	91.8	0.26	(1)		6.25
C-D	-44 / 0	-91.8	-91.8	0.52	(1)		6.25
D-E	0 / 70	-91.8	-91.8	0.53	(1)		10.00
E-F	0 / 70	-91.8	-91.8	0.53	(1)		10.00
F-G	0 / 70	-91.8	-91.8	0.53	(1)		10.00
G-H	-21 / 0	-91.8	-91.8	0.52	(1)		6.25
H-I	-91 / 0	-91.8	-91.8	0.20	(1)		6.25
R-J	0 / 0	0.0	0.0	0.00	(1)		10.00
J-I	-238 / 0	0.0	0.0	0.02	(1)		7.81
B-S	0 / 132	-18.5	-18.5	0.23	(1)		10.00
S-O	0 / 132	-18.5	-18.5	0.23	(1)		10.00
Q-P	0 / 124	-18.5	-18.5	0.15	(4)		10.00
P-O	0 / 124	-18.5	-18.5	0.15	(4)		10.00
O-N	0 / 45	-18.5	-18.5	0.14	(4)		10.00
N-M	0 / 21	-18.5	-18.5	0.15	(4)		10.00
M-L	0 / 21	-18.5	-18.5	0.15	(4)		10.00
L-K	0 / 84	-18.5	-18.5	0.15	(4)		10.00
K-J	0 / 0	-18.5	-18.5	0.11	(4)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI	LC
Q-C	-269 / 0	0.07	(1)
C-O	-93 / 0	0.09	(1)
O-D	-586 / 0	0.14	(1)
D-N	-134 / 0	0.13	(1)
N-F	-522 / 0	0.13	(1)
N-G	-107 / 0	0.11	(1)
L-G	-600 / 0	0.15	(1)
L-H	-49 / 0	0.05	(1)
K-H	-310 / 0	0.08	(1)
K-I	0 / 75	0.02	(1)
S-T	-567 / 0	0.00	(1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.53/1.00 (F-G:1) , BC=0.23/1.00 (B-S:1) , WB=0.15/1.00 (G-L:1) , SSI=0.44/1.00 (B-S:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	318	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (C) (INPUT = 0.90)
JSI METAL= 0.21 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
09/26/19
H. J/G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-1924936

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405684	P8A	1	1	GREENYORK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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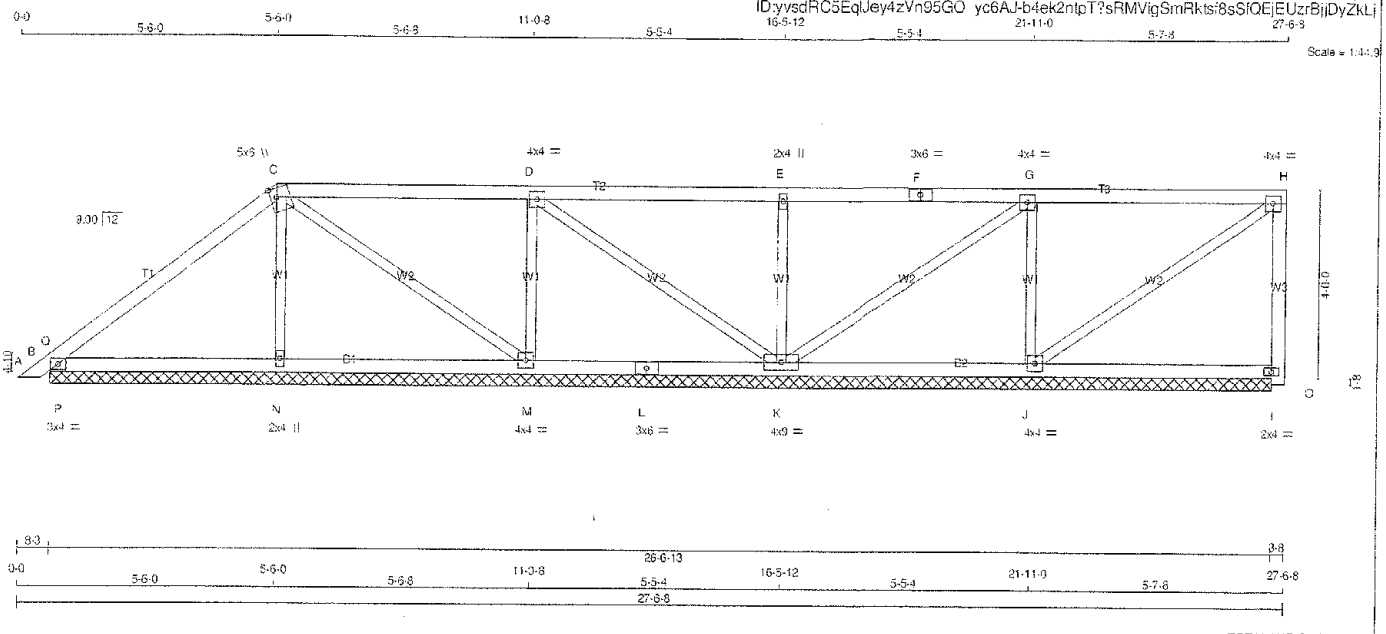

TOTAL WEIGHT = 99 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
O - H	2x4	DRY	No.2	SPF
B - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-t	MT20	3.0	4.0		
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TMVW-t	MT20	4.0	4.0		
I	BVM1-t	MT20	2.0	4.0		
J	BMWW1-t	MT20	4.0	4.0		
K	BMWWW1-t	MT20	4.0	9.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW1-t	MT20	4.0	4.0		
N	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	403	0	403	0	0	26-6-13 (13-8)6-13	
N	391	0	391	0	0	26-6-13 (13-8)6-13	
M	639	0	639	0	0	26-6-13 (13-8)6-13	
K	697	0	697	0	0	26-6-13 (13-8)6-13	
J	635	0	635	0	0	26-6-13 (13-8)6-13	
I	251	0	251	0	0	26-6-13 (13-8)6-13	

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX. MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	282	202 / 0	0 / 0	0 / 0	0 / 0	80 / 0	0 / 0
N	280	163 / 0	0 / 0	0 / 0	0 / 0	116 / 0	0 / 0
M	452	299 / 0	0 / 0	0 / 0	0 / 0	152 / 0	0 / 0
K	491	335 / 0	0 / 0	0 / 0	0 / 0	156 / 0	0 / 0
J	450	289 / 0	0 / 0	0 / 0	0 / 0	161 / 0	0 / 0
I	177	116 / 0	0 / 0	0 / 0	0 / 0	61 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, N, M, K, J, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 15	-91.8	-91.8	0.02 (1)	10.00
B-Q	-34 / 61	-91.8	-91.8	0.14 (1)	6.25
Q-C	-176 / 0	-91.8	-91.8	0.27 (1)	6.25
C-D	-42 / 0	-91.8	-91.8	0.41 (1)	6.25
D-E	0 / 78	-91.8	-91.8	0.42 (1)	10.00
E-F	0 / 78	-91.8	-91.8	0.43 (1)	10.00
F-G	0 / 78	-91.8	-91.8	0.43 (1)	10.00
G-H	-8 / 0	-91.8	-91.8	0.41 (1)	10.00
O-I	0 / 0	0.0	0.0	0.00 (1)	10.00
I-H	-209 / 0	0.0	0.0	0.06 (1)	7.81
B-P	0 / 130	-18.5	-18.5	0.23 (1)	10.00
P-N	0 / 130	-18.5	-18.5	0.23 (1)	10.00
N-M	0 / 124	-18.5	-18.5	0.14 (1)	10.00
M-L	0 / 42	-18.5	-18.5	0.11 (4)	10.00
L-K	0 / 42	-18.5	-18.5	0.11 (4)	10.00
K-J	0 / 8	-18.5	-18.5	0.14 (4)	10.00
J-I	0 / 0	-18.5	-18.5	0.14 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N-C	-236 / 0	0.06 (1)
C-M	-98 / 0	0.08 (1)
M-D	-496 / 0	0.12 (1)
D-K	-145 / 0	0.11 (1)
K-E	-458 / 0	0.11 (1)
K-G	-104 / 0	0.08 (1)
J-G	-524 / 0	0.13 (1)
J-H	0 / 9	0.00 (1)
P-Q	-563 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.43/1.00 (E-G:1) , BC=0.23/1.00 (B-P:1) , WB=0.13/1.00 (G-J:1) , SSI=0.44/1.00 (B-P:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (C) (INPUT = 0.90)
JSI METAL= 0.24 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
09/26/19
H. J. G. ALVES
100090924
PROVINCE OF ONTARIO

Structural component only
DWG# T-1924937

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405684	P9	2	1	GREENYORK HOMES TRUSS DESC	

Tamarack Roof Truss, Burlington

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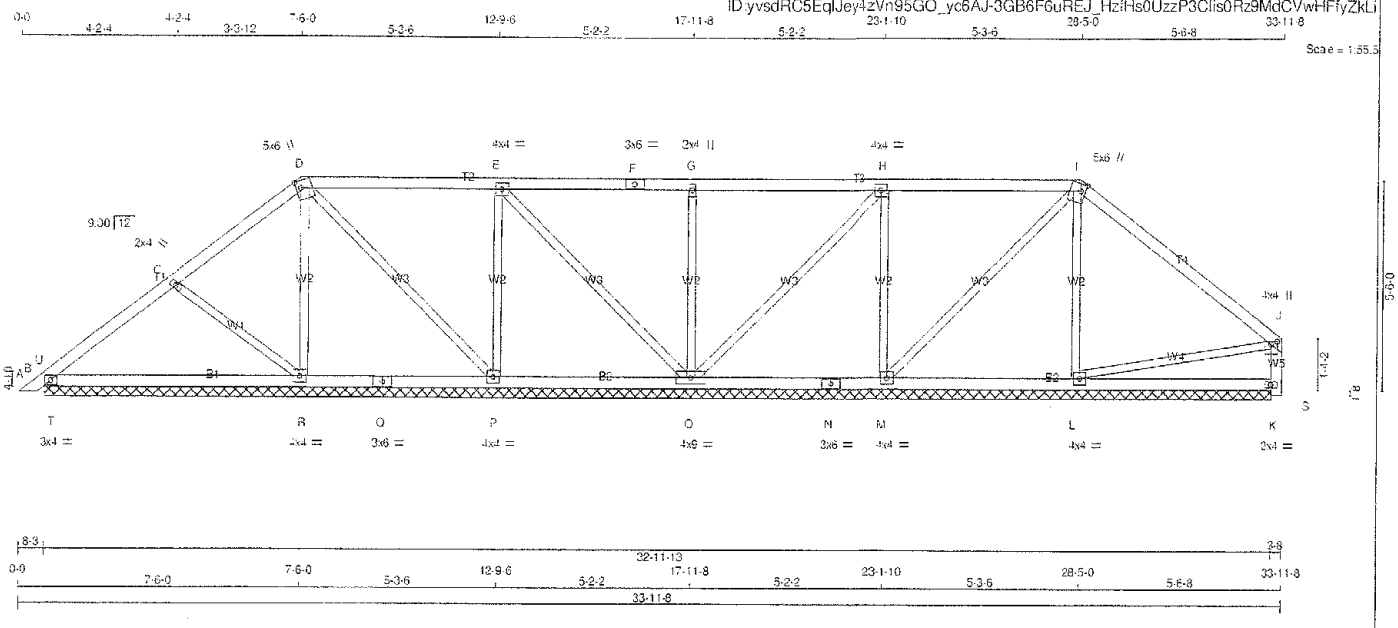

TOTAL WEIGHT = 2 X 132 = 265 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
I - J	2x4	DRY	No.2	SPF
S - J	2x4	DRY	No.2	SPF
B - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB14	MT20	3.0	4.0		
C	TMW+w	MT20	2.0	4.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW+m	MT20	5.0	6.0	2.25	1.50
J	TMVW+p	MT20	4.0	4.0	1.00	2.00
K	BVM1-t	MT20	2.0	4.0		
L, M, P, R						
L	BMWW1-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW1-t	MT20	4.0	9.0		
Q	BS-t	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	428	0	428	0	0	32-11-13	11-32-12-13
R	597	0	597	0	0	32-11-13	11-32-12-13
P	557	0	557	0	0	32-11-13	11-32-12-13
O	775	0	775	0	0	32-11-13	11-32-12-13
M	590	0	590	0	0	32-11-13	11-32-12-13
L	407	0	407	0	0	32-11-13	11-32-12-13
K	369	0	369	0	0	32-11-13	11-32-12-13

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX. MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	301	207 / 0	0 / 0	0 / 0	0 / 0	94 / 0	0 / 0
R	425	284 / 0	0 / 0	0 / 0	0 / 0	161 / 0	0 / 0
P	393	262 / 0	0 / 0	0 / 0	0 / 0	131 / 0	0 / 0
O	545	376 / 0	0 / 0	0 / 0	0 / 0	168 / 0	0 / 0
M	416	279 / 0	0 / 0	0 / 0	0 / 0	137 / 0	0 / 0
L	293	152 / 0	0 / 0	0 / 0	0 / 0	130 / 0	0 / 0
K	259	182 / 0	0 / 0	0 / 0	0 / 0	77 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, R, P, O, M, L, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 15	-91.8	-91.8	0.02 (1)	10.00	
B- U	-467 / 0	-91.8	-91.8	0.11 (4)	6.25	
U- C	-303 / 0	-91.8	-91.8	0.18 (1)	6.25	
C- D	-31 / 0	-91.8	-91.8	0.18 (1)	6.25	
D- E	0 / 31	-91.8	-91.8	0.37 (1)	10.00	
E- F	0 / 138	-91.8	-91.8	0.39 (1)	10.00	
F- G	0 / 138	-91.8	-91.8	0.39 (1)	10.00	
G- H	0 / 138	-91.8	-91.8	0.39 (1)	10.00	
H- I	0 / 9	-91.8	-91.8	0.37 (1)	10.00	
I- J	-98 / 0	-91.8	-91.8	0.48 (1)	6.25	
S- K	0 / 0	0.0	0.0	0.00 (1)	10.00	
K- J	-328 / 0	0.0	0.0	0.03 (1)	7.81	
B- T	0 / 269	-18.5	-18.5	0.10 (4)	10.00	
T- R	0 / 269	-18.5	-18.5	0.18 (4)	10.00	
R- Q	0 / 4	-18.5	-18.5	0.16 (4)	10.00	
Q- F	0 / 4	-18.5	-18.5	0.16 (4)	10.00	
P- O	-31 / 0	-18.5	-18.5	0.10 (4)	6.25	
O- N	-9 / 0	-18.5	-18.5	0.10 (4)	10.00	
N- M	-9 / 0	-18.5	-18.5	0.10 (4)	10.00	
M- L	0 / 73	-18.5	-18.5	0.14 (4)	10.00	
L- K	0 / 0	-18.5	-18.5	0.14 (4)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- R	-328 / 0	0.11 (1)
R- D	-273 / 0	0.12 (1)
D- P	-44 / 0	0.05 (1)
P- E	-442 / 0	0.20 (1)
E- O	-153 / 0	0.15 (1)
O- G	-437 / 0	0.20 (1)
O- H	-183 / 0	0.19 (1)
M- H	-420 / 0	0.19 (1)
M- I	-115 / 0	0.12 (1)
L- I	-306 / 0	0.14 (1)
L- J	0 / 80	0.02 (1)
T- U	0 / 210	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.48/1.00 (I-J:1) , BC=0.18/1.00 (R-T:4) , WB=0.20/1.00 (E-P:1) , SSI=0.23/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.64 (L) (INPUT = 0.90)
JSI METAL= 0.29 (K) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO
09/26/19

Structural component only
DWG# T-1924938

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405684	P10	1	3	GREENYORK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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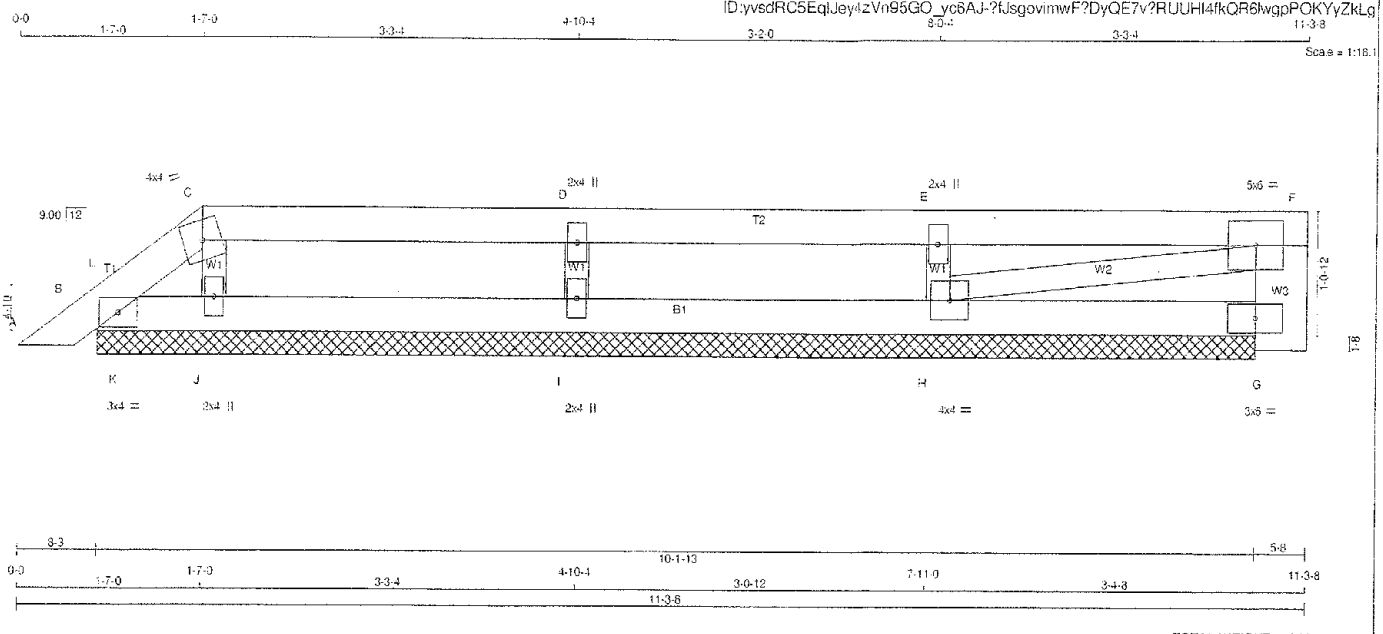


TOTAL WEIGHT = 3 X 30 = 90 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
G - F	2x6	DRY	No.2	SPF
B - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	TOP
C-F	1	12	TOP
F-G	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
B-G	1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1+	MT20	3.0	4.0		
C	TTW+m	MT20	4.0	4.0		
D	TMW+w	MT20	2.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TMVW+	MT20	5.0	6.0		
G	BVM1+	MT20	3.0	6.0		
H	BMWW1+	MT20	4.0	4.0		
I	BMW1+w	MT20	2.0	4.0		
J	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	110	0	110	0	0	10-1-13	10-1-13
G	145	0	145	0	0	10-1-13	10-1-13
J	196	0	196	0	0	10-1-13	10-1-13
H	392	0	392	0	0	10-1-13	10-1-13
I	381	0	381	0	0	10-1-13	10-1-13

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	75	63 / 0	0 / 0	0 / 0	0 / 0	12 0	0 / 0
G	103	68 / 0	0 / 0	0 / 0	0 / 0	35 0	0 / 0
J	141	81 / 0	0 / 0	0 / 0	0 / 0	60 0	0 / 0
H	277	181 / 0	0 / 0	0 / 0	0 / 0	96 0	0 / 0
I	269	181 / 0	0 / 0	0 / 0	0 / 0	88 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, G, J, H, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

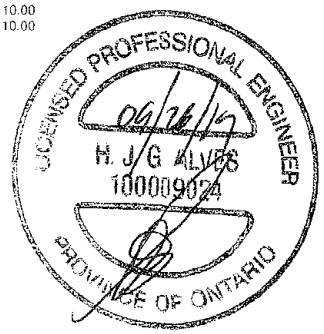
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 15	-91.8	-91.8	0.01 (1)	10.00	
B-L	-25 / 0	-91.8	-91.8	0.00 (4)	6.25	
L-C	-27 / 0	-91.8	-91.8	0.00 (1)	6.25	
C-D	-2 / 0	-91.8	-91.8	0.04 (1)	10.00	
D-E	-2 / 0	-91.8	-91.8	0.04 (1)	10.00	
E-F	-2 / 0	-91.8	-91.8	0.04 (1)	10.00	
G-F	-121 / 0	0.0	0.0	0.00 (1)	7.81	
B-K	0 / 20	-18.5	-18.5	0.00 (1)	10.00	
K-J	0 / 20	-18.5	-18.5	0.01 (4)	10.00	
J-I	0 / 2	-18.5	-18.5	0.01 (4)	10.00	
I-H	0 / 2	-18.5	-18.5	0.02 (4)	10.00	
H-G	0 / 0	-18.5	-18.5	0.02 (4)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J-C	-133 / 0	0.01 (1)
H-E	-324 / 0	0.02 (1)
H-F	0 / 2	0.00 (1)
I-D	-324 / 0	0.02 (1)
K-L	-45 / 0	0.00 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.04/1.00 (C-D:1) , BC=0.02/1.00 (G-H:4) , WB=0.02/1.00 (D-I:1) , SSI=0.05/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.06 (H) (INPUT = 0.90)
JSI METAL= 0.02 (G) (INPUT = 1.00)

Structural component only
DWG# T-1924939

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405684	P11	1	1	GREENYORK HOMES TRUSS DESC	

Tamarack Roof Truss, Burlington

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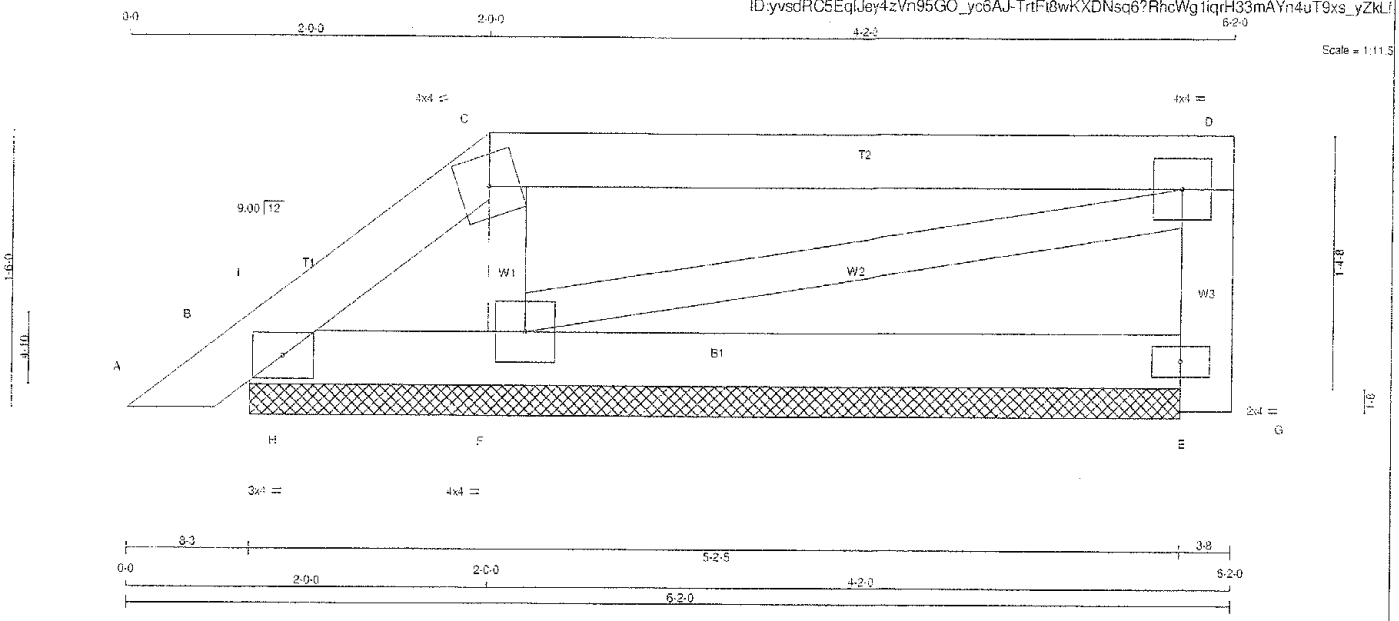

TOTAL WEIGHT = 19 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
G - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB1-t	MT20	3.0	4.0	
C	TTW-m	MT20	4.0	4.0	
D	TMVW-t	MT20	4.0	4.0	
E	BVM1-t	MT20	2.0	4.0	
F	BMWW1-t	MT20	4.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	129	0	129	0	0	5-2-5	5-2-5
E	223	0	223	0	0	5-2-5	5-2-5
F	307	0	307	0	0	5-2-5	5-2-5

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX. MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	86	77 / 0	0 / 0	0 / 0	0 / 0	11 / 0	0 / 0
E	157	108 / 0	0 / 0	0 / 0	0 / 0	49 / 0	0 / 0
F	220	126 / 0	0 / 0	0 / 0	0 / 0	94 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	(LC)	MAX. UNBRAC LENGTH
A-B	0 / 15	-91.8	-91.8	0.02	(1)		10.00
B-I	-20 / 11	-91.8	-91.8	0.02	(4)		6.25
I-C	-40 / 0	-91.8	-91.8	0.01	(4)		6.25
C-D	-8 / 0	-91.8	-91.8	0.27	(1)		10.00
G-E	0 / 0	0.0	0.0	0.00	(1)		10.00
E-D	-193 / 0	0.0	0.0	0.02	(1)		7.81
B-H	0 / 28	-18.5	-18.5	0.03	(1)		10.00
H-F	0 / 28	-18.5	-18.5	0.07	(4)		10.00
F-E	0 / 0	-18.5	-18.5	0.07	(4)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI	(LC)
F-C	212 / 0	0.03	(1)
F-D	0 / 8	0.00	(1)
H-I	-89 / 0	0.00	(1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00:12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD= EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.27/1.00 (C-D:1) , BC=0.07/1.00 (F-H:4) , WB=0.03/1.00 (C-F:1) , SSI=0.15/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP/DRY (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (E) (INPUT = 0.90)
JSI METAL= 0.17 (E) (INPUT = 1.00)

Structural component only
DWG# T-1924940

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENYORK HOMES	DRWG NO.
405684	P12	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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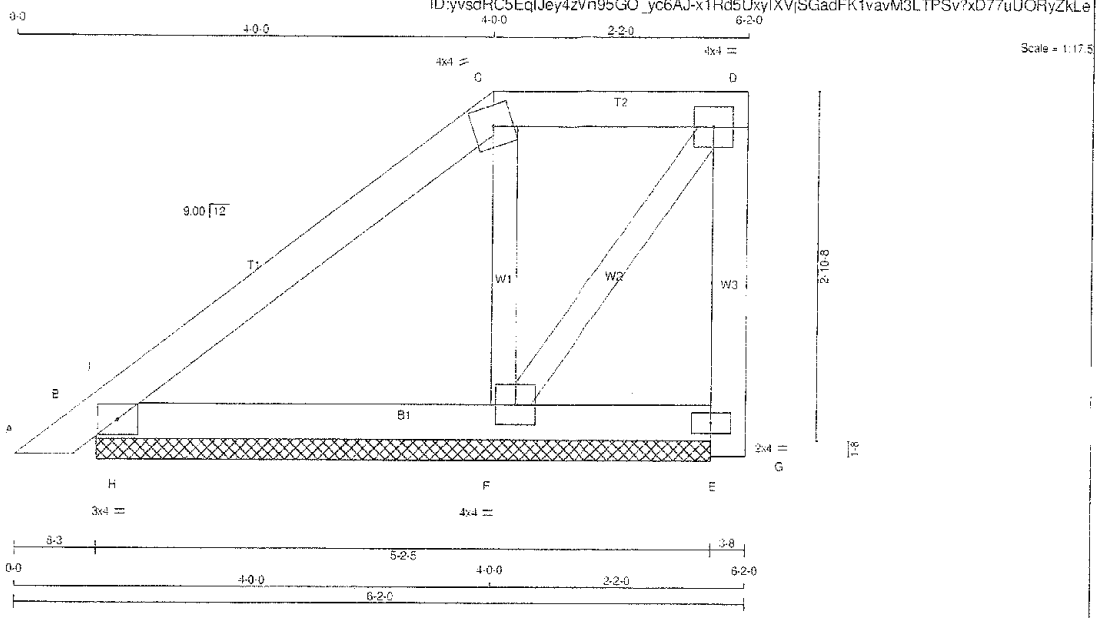


TOTAL WEIGHT = 21 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
G - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW-m	MT20	4.0	4.0		
D	TMVW-r	MT20	4.0	4.0		
E	BVM1-I	MT20	2.0	4.0		
F	BMWW1-I	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
B	253	0	253	0	0	IN-SX 5-2-5	IN-SX 5-2-5
E	131	0	131	0	0	5-2-5	5-2-5
F	275	0	275	0	0	5-2-5	5-2-5

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	178	125 / 0	0 / 0	0 / 0	0 / 0	52 / 0	0 / 0
E	92	65 / 0	0 / 0	0 / 0	0 / 0	27 / 0	0 / 0
F	196	121 / 0	0 / 0	0 / 0	0 / 0	75 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	(LC)	MAX. UNBRAC LENGTH
A-B	0 / 15	-91.8	-91.8	0.02	(1)		10.00
B-I	-17 / 39	-91.8	-91.8	0.05	(1)		6.25
I-C	-51 / 0	-91.8	-91.8	0.12	(1)		6.25
C-D	-27 / 0	-91.8	-91.8	0.07	(1)		6.25
G-E	0 / 0	0.0	0.0	0.00	(1)		10.00
E-D	-133 / 0	0.0	0.0	0.02	(1)		7.81
B-H	0 / 35	-18.5	-18.5	0.11	(1)		10.00
H-F	0 / 35	-18.5	-18.5	0.11	(1)		10.00
F-E	0 / 0	-18.5	-18.5	0.06	(1)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI	(LC)
F-C	-202 / 0	0.03	(1)
F-D	0 / 44	0.01	(1)
H-I	-283 / 0	0.00	(1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	5.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	38.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF SCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.12/1.00 (C-I:1) , BC=0.11/1.00 (F-H:1) , WB=0.03/1.00 (C-F:1) , SSI=0.22/1.00 (B-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

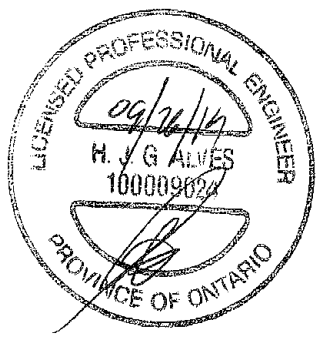
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (B) (INPUT = 0.90)
JSI METAL= 0.12 (E) (INPUT = 1.00)



Structural component only
DWG# T-1924941

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENYORK HOMES	DRWG NO.
405684	P13	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.300 S May 10 2019 MiTek Industries, Inc. Thu Sep 26 15:23:02 2019 Page 1

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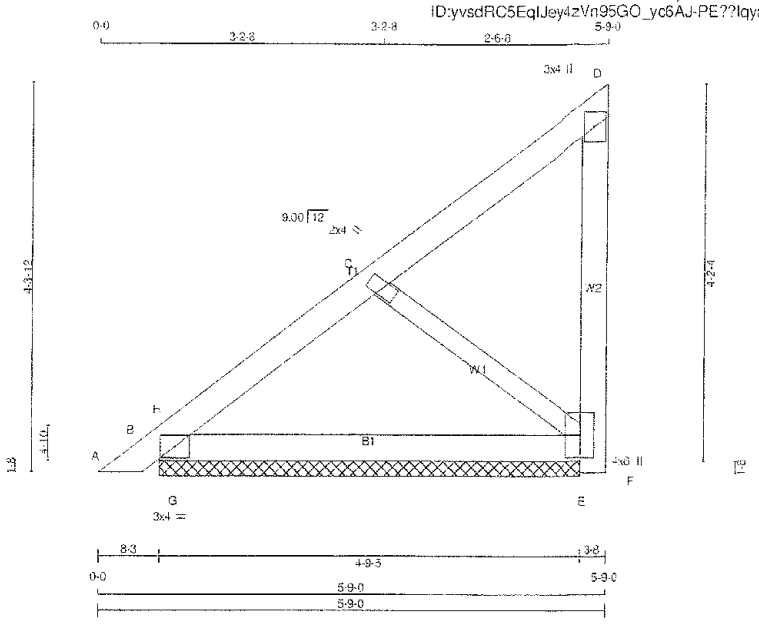


TOTAL WEIGHT = 2 X 20 = 41 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB1-I	MT20	3.0	4.0	
C	TMW+w	MT20	2.0	4.0	
D	TMV+p	MT20	3.0	4.0	
E	BVMW1+p	MT20	4.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	334	0	334	0	0	4-9-5	4-9-5
E	279	0	279	0	0	4-9-5	4-9-5

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	235	150 / 0	0 / 0	0 / 0	0 / 0	75 / 0	0 / 0
E	198	130 / 0	0 / 0	0 / 0	0 / 0	68 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FR-TO	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	(LC)	MAX. UNBRAC LENGTH
A-B	0 / 15		-91.8	-91.8	0.02	(1)		10.00
B-H	-373 / 0		-91.8	-91.8	0.09	(4)		6.25
H-C	-224 / 0		-91.8	-91.8	0.10	(1)		6.25
C-D	-17 / 0		-91.8	-91.8	0.10	(1)		6.25
F-E	0 / 0		0.0	0.0	0.00	(1)		10.00
E-D	-92 / 0		0.0	0.0	0.03	(1)		7.81
B-G	0 / 204		-18.5	-18.5	0.08	(4)		10.00
G-E	0 / 204		-19.5	-18.5	0.11	(4)		10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI	(LC)
C-E	-251 / 0	0.05	(1)
G-H	0 / 175	0.00	(1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD; EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.10/1.00 (C-H:1) , BC=0.11/1.00 (E-G:4) , WB=0.05/1.00 (C-E:1) , SSI=0.14/1.00 (B-G:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

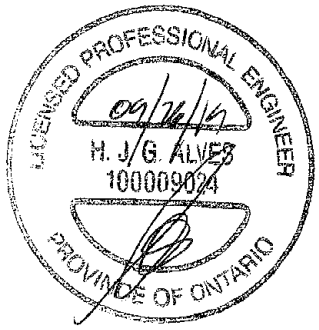
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)



Structural component only
DWG# T-1924942

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405684	P14	4	1	GREENYORK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.300 S May 10 2019 MiTek Industries, Inc. Thu Sep 26 15:23:04 2019 Page 1
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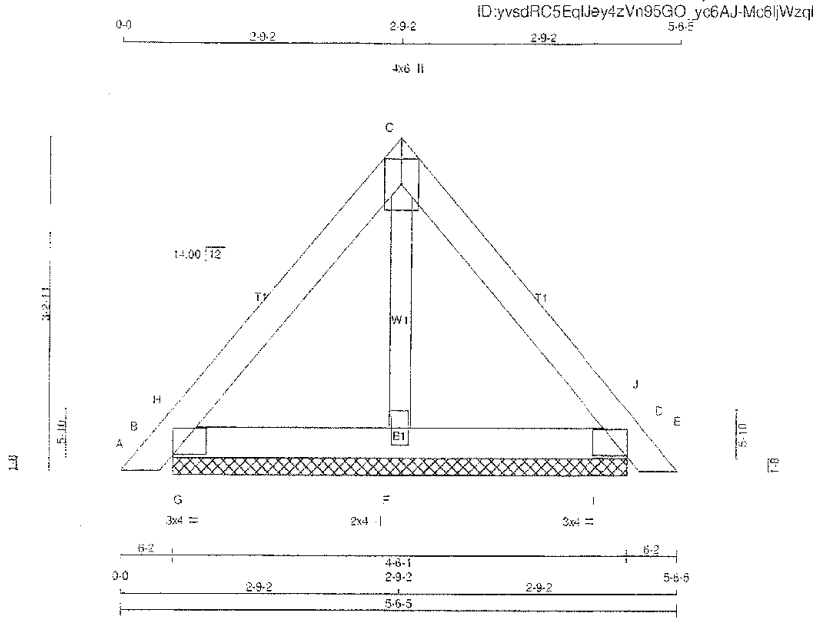


TOTAL WEIGHT = 4 X 17 = 67 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB14	MT20	3.0	4.0	1.50	2.50
C	TTW+p	MT20	4.0	6.0		
D	TMB14	MT20	3.0	4.0	1.50	2.50
F	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	210	0	210	0	0	4-6-1	4-6-1
D	210	0	210	0	0	4-6-1	4-6-1
F	149	0	149	0	0	4-6-1	4-6-1

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	147	107 / 0	0 / 0	0 / 0	0 / 0	40 / 0	0 / 0
D	147	107 / 0	0 / 0	0 / 0	0 / 0	40 / 0	0 / 0
F	108	58 / 0	0 / 0	0 / 0	0 / 0	50 / 0	0 / 0

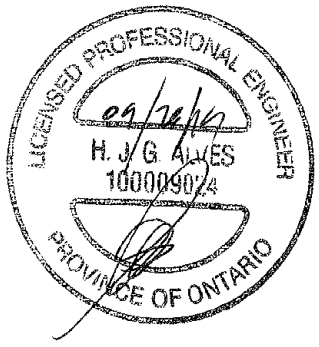
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 12	-91.8	91.8	0.01	(1)		10.00	F-C	-47 / 0	0.01 (1)
B-H	-14 / 8	-91.8	-91.8	0.03	(1)		6.25	G-H	-214 / 0	0.00 (1)
H-C	-83 / 0	91.8	-91.8	0.05	(1)		6.25	I-J	-214 / 0	0.00 (1)
C-J	-98 / 0	91.8	-91.8	0.05	(1)		6.25			
J-D	-14 / 8	91.8	-91.8	0.03	(1)		6.25			
D-E	0 / 12	-91.8	-91.8	0.01	(1)		10.00			
B-G	0 / 52	-18.5	-18.5	0.06	(1)		10.00			
G-F	0 / 52	-18.5	-18.5	0.06	(1)		10.00			
F-I	0 / 52	-18.5	-18.5	0.06	(1)		10.00			
I-D	0 / 52	-18.5	-18.5	0.06	(1)		10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.05/1.00 (C-J:1) , BC=0.06/1.00 (D-I:1) , WB=0.01/1.00 (C-F:1) , SSI=0.18/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

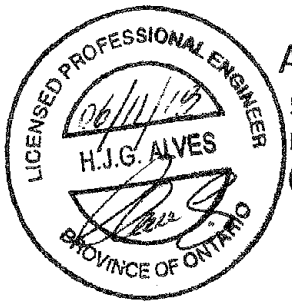
PLATE	GRIP(DRY: (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	518	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.26 (D) (INPUT = 0.90)
JSI METAL = 0.05 (D) (INPUT = 1.00)

Structural component only
DWG# T-1924943



Alves Engineering Services Inc.
5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components

2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.

3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.

4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.

5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.

2- Lumber is to be the sizes and grade specified on the truss drawing.

3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.

4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings

5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.

6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)

7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.

8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1800213 Feb 09, 2018

TECHNICAL BULLETIN

HUS/LJS – Double Shear Joist Hangers

SIMPSON Strong-Tie

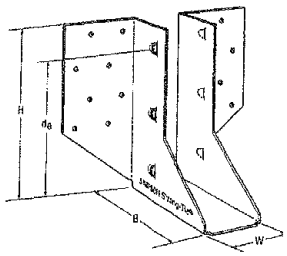
All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

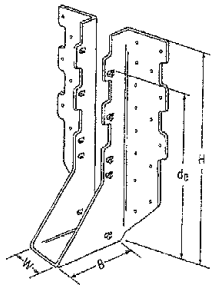
Finish: G90 galvanized

Design:

- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.



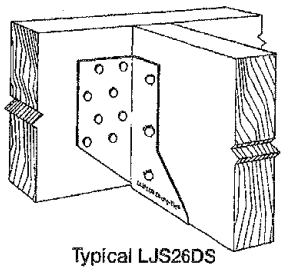
LJS26DS



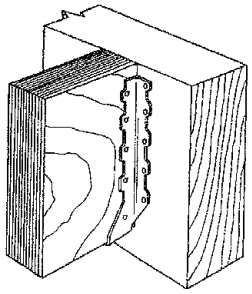
HUS210
(HUS26, HUS28, similar)

Installation:

- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications



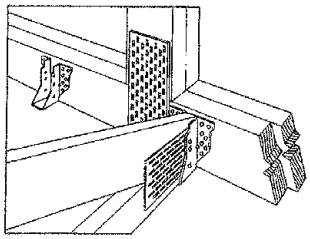
Typical LJS26DS Installation



Typical HUS Installation

Options:

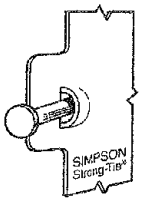
- See current catalogue for options



Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

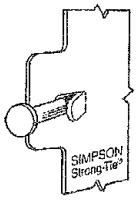
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
								lb.	lb.	lb.	lb.
LJS26DS	18	1 9/16	5	3½	4⅝	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1⅝	5⅜	3	3 15/16	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1⅝	7 3/32	3	6 3/32	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1⅝	9 3/32	3	7 31/32	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1 13/16	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

1. d_e is the distance from the seat of the hanger to the highest joist nail.

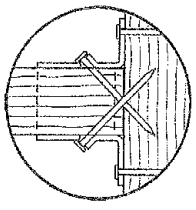


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

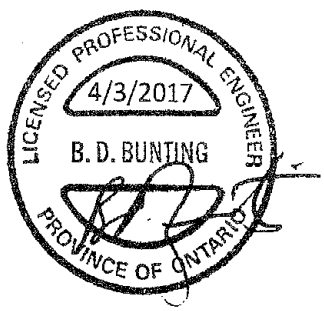
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2019, and reflects information available as of March 1, 2017. This information is updated periodically and should not be relied upon after June 30, 2019. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

T-SPECHUS17 3/17 exp. 6/19

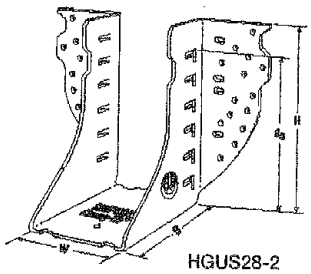
(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.



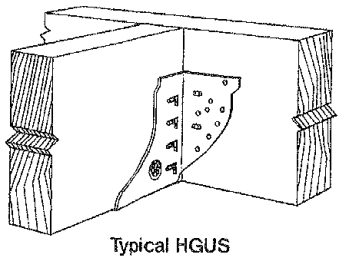
HGUS28-2

Material: 12 gauge

Finish: G90 galvanized

Design:

- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.



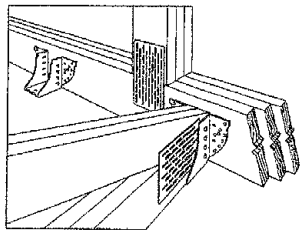
Typical HGUS Installation

Installation:

- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

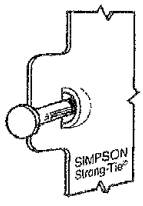
- See current catalogue for options



Typical HGUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)

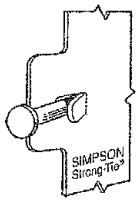
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
HGUS26	12	1⅝	5⅜	5	4 5/32	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3 5/16	5 7/16	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4 15/16	5½	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6 9/16	5 7/16	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1⅝	7⅛	5	6⅛	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3 5/16	7 3/16	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4 15/16	7¼	4	6⅜	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6 9/16	7 3/16	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210-2	12	3 5/16	9 3/16	4	8⅛	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-3	12	4 15/16	9¼	4	8⅜	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6 9/16	9 3/16	4	8⅛	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6 9/16	10⅜	4	10⅛	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6 9/16	12⅝	4	11⅛	(66) 16d	(22) 16d	10130	16400	7195	11645

1. d_e is the distance from the seat of the hanger to the highest joist nail.

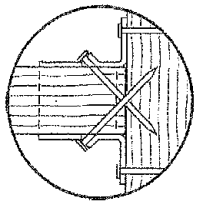


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

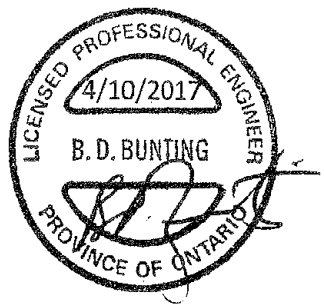
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2019, and reflects information available as of March 1, 2017. This information is updated periodically and should not be relied upon after June 30, 2019. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com

T-SPECHGUS17 3/17 exp. 6/19

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

H – Seismic and Hurricane Ties

The H connector series provides wind and seismic ties for trusses and rafters.

MATERIAL: 18 gauge **FINISH:** G90 galvanized

DESIGN: • Factored resistances are in accordance with CSA O86-14

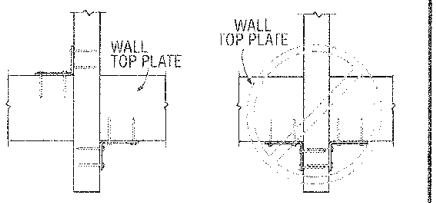
- Factored resistances have been increased 15%. No further increase is permitted.

INSTALLATION: • Use all specified fasteners

- Nails: 8d = 0.131" dia. x 2½" long common wire, 8d x 1½ = 0.131" x 1½" long, 10d x 1½" = 0.146" x 1½" long
- H1 can be installed with flanges facing outwards
- Hurricane ties do not replace solid blocking

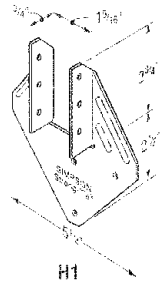
Factored resistances for more than one direction for a single connection cannot be added together. A factored load which can be divided into components in the directions given must be evaluated as follows: Factored Shear/Resisting Shear + Factored Tension/Resisting Tension ≤ 1.0.

Hurricane Tie Installations to achieve Twice the Load (Top View)

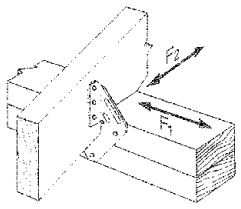


Install diagonally across from each other for minimum 2x truss.

Nailing into both sides of a single ply 2x truss may cause the wood to split.

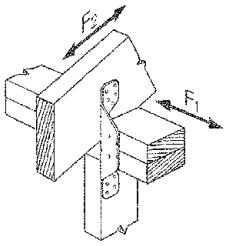


H1

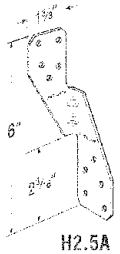


H1 Installation

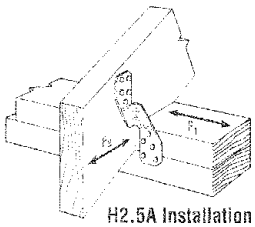
H2A



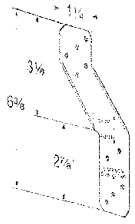
H2A Installation



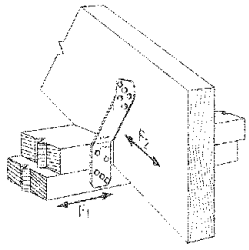
H2.5A



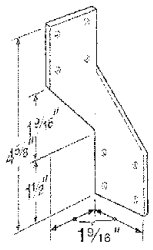
H2.5A Installation



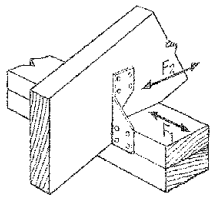
H2.5T



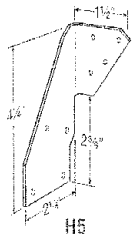
H2.5T Installation
(Nails into both top plates)



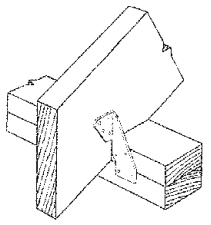
H3



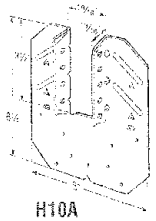
H3 Installation



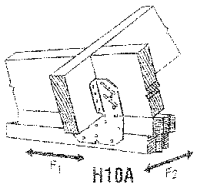
H5



H5 Installation
(Nails into both top plates)



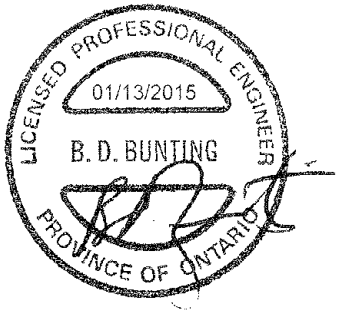
H10A



H10A Installation

Model No.	Ga	Fasteners			Factored Resistance (lbs)					
					D.Fir-L			S-P-F		
		To Rafter	To Plates	To Studs	Uplift	Lateral F_1	Lateral F_2	Uplift	Lateral F_1	Lateral F_2
					(K_D = 1.15)			(K_D = 1.15)		
H1	18	6-8dx1½	4-8d	–	740	685	300	680	485	215
H2A	18	5-8dx1½	2-8dx1½	5-8dx1½	830	220	75	590	155	55
H2.5A	18	5-8d	5-8d	–	805	160	160	755	160	160
H2.5T	18	5-8d	5-8d	–	835	175	240	740	160	210
H3	18	4-8d	4-8d	–	740	180	265	615	125	190
H5	18	4-8d	4-8d	–	685	180	305	500	130	215
H10A	18	9-10dx1½	9-10dx1½	–	1735	795	410	1505	565	290

1. Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.
2. Factored resistances are for one anchor. A minimum rafter thickness of 2½" must be used when framing anchors are installed on each side of the joist and on the same side of the plate.
3. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.
4. Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a Continuous Load Path, connections must be on same side of the wall.



LIMIT STATES DESIGN

This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2015. This information is updated periodically and should not be relied upon after December 31, 2016; contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.

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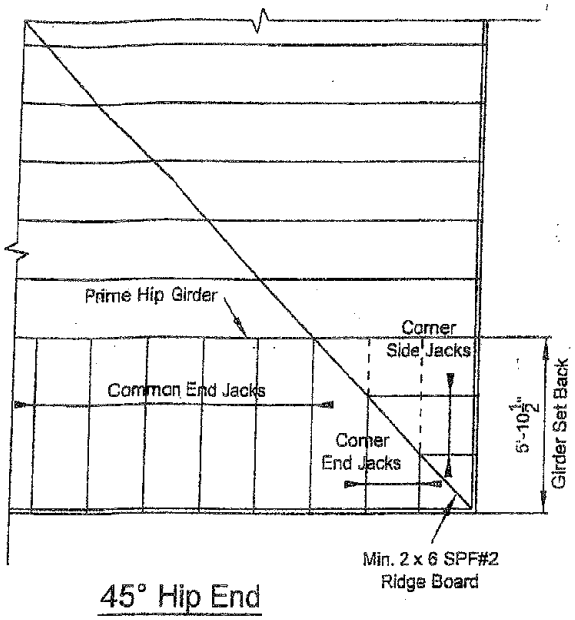

45° Hip End

LUMBER SPECIFICATION

TOP CHORD	:	2 x 4 SPF#2
BOTTOM CHORD	:	2 x 4 SPF#2
WEBS	:	2 x 3 SPF#2

UNLESS OTHERWISE SHOWN

DESIGN LOAD

TOP CHORD SNOW LOAD	:	40.5 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD	:	50.5 P.S.F

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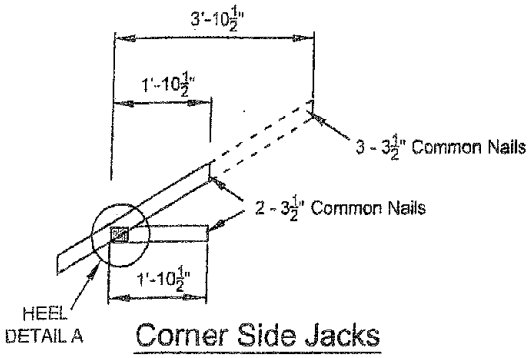
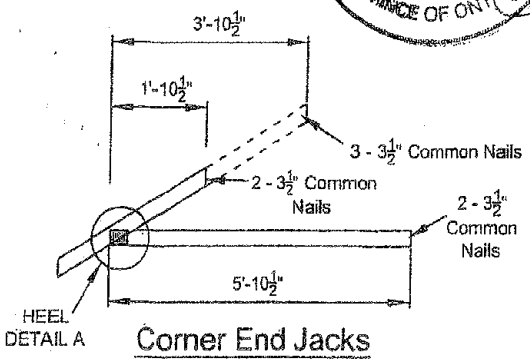
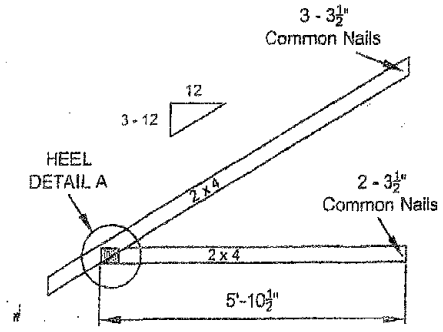


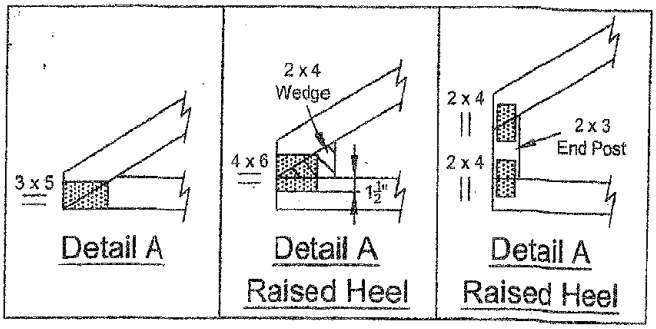
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800216

ONTARIO WOOD TRUSS FABRICATORS ASSOCIATION

TECH-NOTES

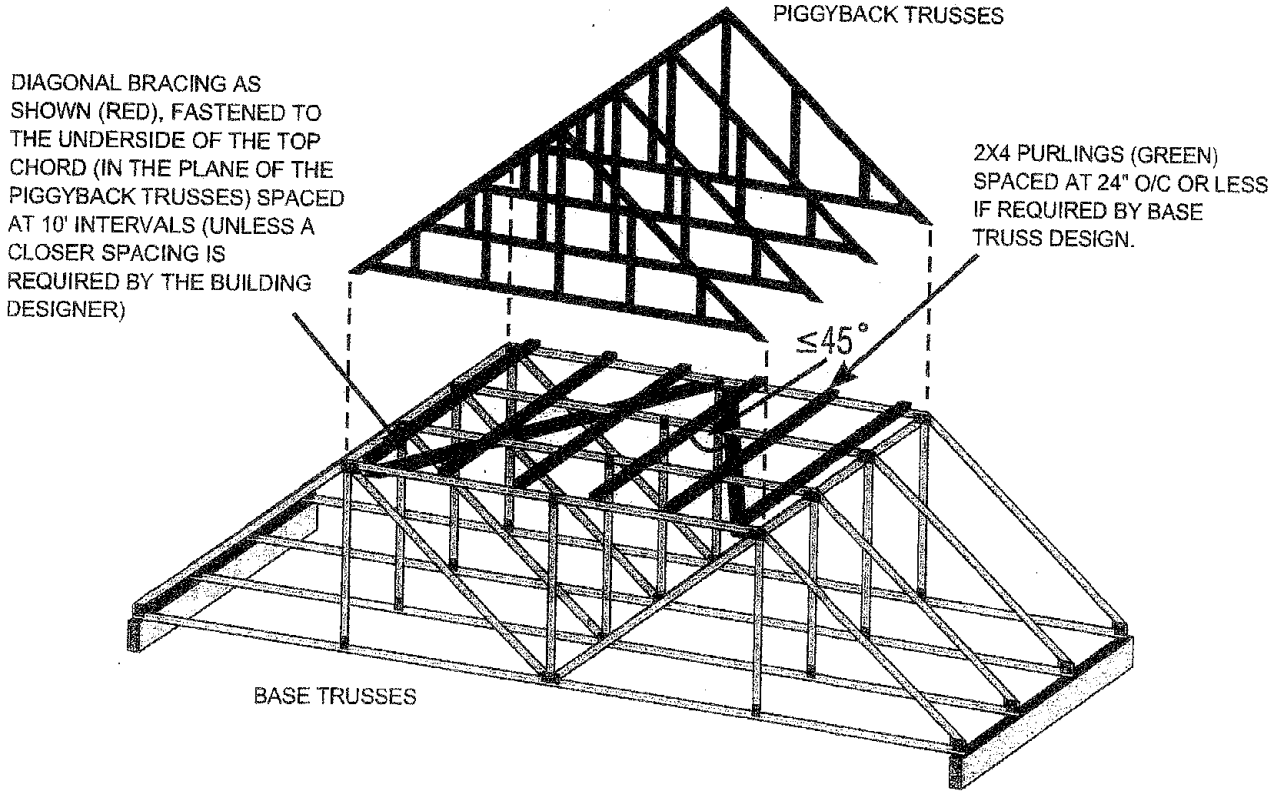
TN 15-001
Piggyback Bracing

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:

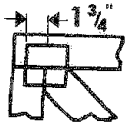


NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

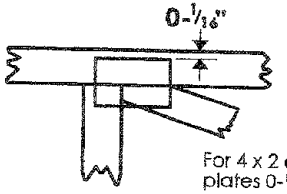
SKETCH FROM BCSI-CANADA 2013

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

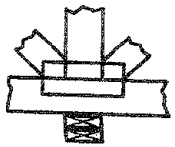
4 X 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

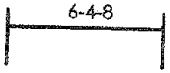
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses

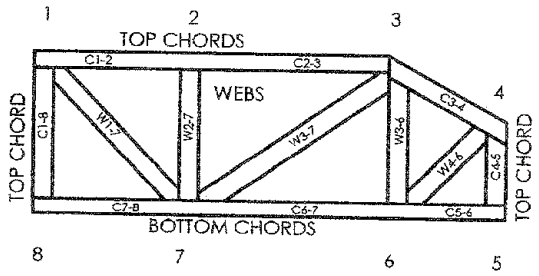
DSB-89: Design Standard for Bracing.

BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

MiTek®

POWER TO PERFORM.™

MiTek Engineering Reference Sheet: Mii-7473C rev. 10-'08

⚠ General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.