ASPHALT SHINGLES
12" FINISH O.H
R.T.M.C.
2x6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:

HGUS26-2 -(XX)
HGUS28-2 -(##)
LUS26-2 -(VV)
LUS24 -(O)
LJS26DS -(V)

ALL B-2-2X10
(FLUSH)

DENOTE:

H- 12" RAISED
CEILING

H2- 24" HIGHER PLATE

8/12 PITCHES (TYP.)
UNLESS NOTED

T-180485

DENOTES:
CONVENTIONAL
FRAMING

UNIT2203
A -LOT12
300330

UNIT2202
A -LOT11
300332

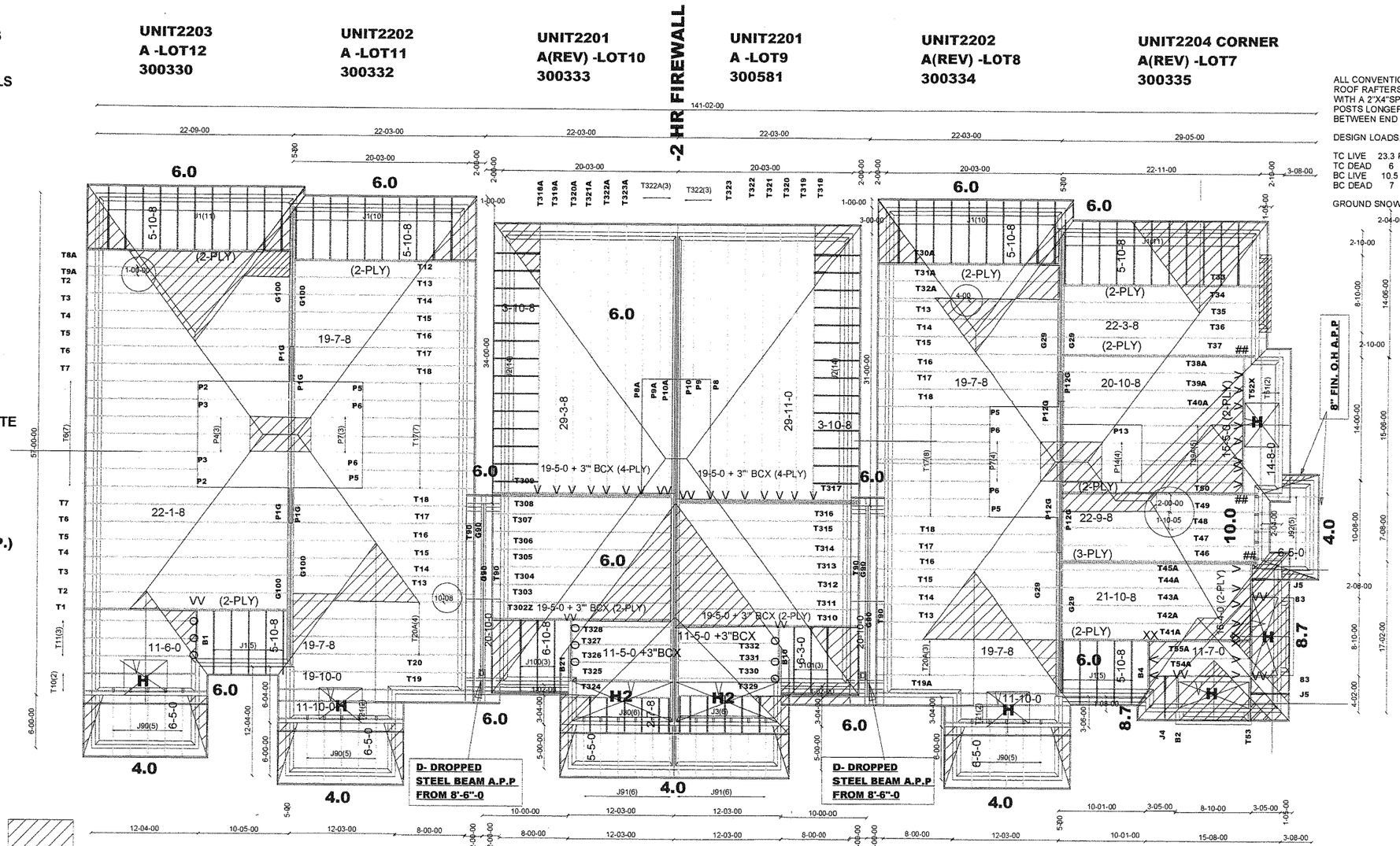
UNIT2201
A(REV) -LOT10
300333

-2 HR FIREWALL

UNIT2201
A -LOT9
300581

UNIT2202
A(REV) -LOT8
300334

UNIT2204 CORNER
A(REV) -LOT7
300335



ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c.
WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.
POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE
BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN LOADS:

TC LIVE 23.3 PSF
TC DEAD 6 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

GROUND SNOW LOAD 27.2 PSF

Unit 7, DWG# T-1901167, T-1901271 to T-1901295, T-1901223, T-1800216, T1800218

LICENSED PROFESSIONAL ENGINEER
04/15/19
H. J. G ALVES
100009024
PROVINCE OF ONTARIO

DWG NO. TAM T1908348
STRUCTURAL
COMPONENT ONLY

Unit 12, DWG# T-1901143 to T-1901154, T-1901211 to T-1901214 , T-1800216, T-1800218

Unit 11, DWG# T-1901143, T-1901155 to T-1901162, T-1901164 to T-1901166, T-1901171 to T-1901172 , T-1901211, T-1901215, T-1901216, T-1901224, T-1800216, T1800218

M12127

Unit 10, DWG# T-1901171 to T-1901172,T-1901174 to T-1901181, T-1901191, T-1901193, T-1901195, T-1901197, T-1901199, T-1901201 to T-1901206, T-1901218, T-1901220, T-1901222, T-1800216, T1800218

Unit 9, DWG# T-1901171 to T-1901172, T-1901182 to T-1901190, T-1901192, T-1901194 , T-1901196, T-1901198, T-1901200,T-1901207 to T-1901210 , T-1901217, T-1901119, T-1901221, T-1800216, T1800218

Unit 8, DWG# T-1901156 to T-1901161,T-1901163, T-1901165 to T-1901172, T-1901215 to T-1901216, T-1901223 to T-1901224, T-1800216, T1800218

TRUSS PLACEMENT PLAN.

This is a truss placement plan only, NOT a final roof framing plan. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual truss drawings for each component identified on this placement plan.

Please verify that all dimensions match the dimensions found on the job. The Building designer is responsible for the temporary/ permanent bracing of the roof and floor system and its integration into the bracing of the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsability of the building designer unless otherwise noted in this plan. Building designer to review and aprove this plan to acertain comformity to his overall structural plan.

TAMARACK LUMBER INC
ALPA LUMBER GROUP

Job Track: 41239
Layout ID: 300329
Plan Log: 98887

Builder / Location: ROYAL PINE HOMES / BRAMPTON
Project: FORESTSIDE PH.6
Date: 4/10/2019 Designer: AC

Model / Elevation: BLOCK 4 - LOTS 7 TO 12
Mitek ver 8.2.3.229

TAMARACK LUMBER INC
ALPA LUMBER GROUP

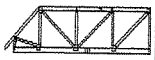
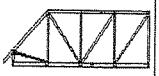
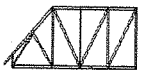
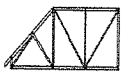
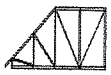
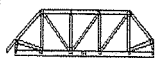
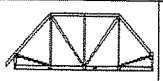
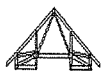
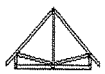
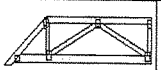
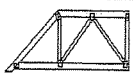
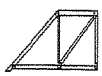
Delivery Shiplist

DATE	09/10/18
SALES REP	Mario

JOB TRACK: 41239 LAYOUT ID: 300330 LOCATION:
BUILDER: ROYAL PINE-FORESTSIDE PH.6 SUB-BUILDER:
MODEL: UNIT2203 ELEVATION: A LT.12

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	2	G100 GABLE	6.00 0.00	25-11-00	09-01-04	2 X 4	2 X 4	00-00-00 00-00-00	01-02-00 09-01-04	267.92 168.66		
	1 2 Ply	T1 HALF HIP	8.00 0.00	22-01-08	04-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-04-13 04-01-04	219.12 137.34		
	2	T2 HALF HIP	8.00 0.00	22-01-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-01-04	184.50 115.34		
	2	T3 HALF HIP	8.00 0.00	22-01-08	06-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 06-01-04	200.40 128.00		
	2	T4 HALF HIP	8.00 0.00	22-01-08	07-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 07-01-04	194.90 123.34		
	2	T5 HALF HIP	8.00 0.00	22-01-08	08-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 08-01-04	209.34 131.66		
	9	T6 PIGGYBACK	8.00 0.00	22-01-08	09-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 09-01-04	991.80 616.50		
	2	T7 HALF HIP	8.00 0.00	22-01-08	10-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 10-01-04	230.16 144.00		
	1 2 Ply	T8A HIP GIRDER	8.00 0.00	22-01-08	04-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-04-13 01-06-08	227.06 142.68		
	1	T9A HIP	8.00 0.00	22-01-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	88.47 56.50		
	2	T10 ROOF	8.00 0.00	11-06-00	05-02-13	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	119.46 76.00		
	3	T11 COMMON	8.00 0.00	11-06-00	05-02-13	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	142.89 89.49		
	2	P1G GABLE	6.00 0.00	09-01-03	04-10-12	2 X 4	2 X 4	00-00-00 00-00-00	00-00-00 04-10-12	67.48 45.00		
	2	P2 PIGGYBACK	8.00 0.00	09-09-15	01-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 01-10-08	65.22 42.00		
	2	P3 PIGGYBACK	8.00 0.00	09-09-15	02-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 02-10-08	69.32 45.66		
	3	P4 PIGGYBACK	8.00 0.00	09-09-15	03-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 03-10-08	106.05 69.00		
	16	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	268.64 170.72		
	5	J90 JACK	4.00 0.00	06-05-00	03-00-14	2 X 4	2 X 4	01-03-08 00-00-00	00-11-03 03-00-14	85.95 56.65		

TOTAL # TRUSS= **61.00** TOTAL BFT OF ALL TRUSSES= **2358.54** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3738.68** LBS.

TAMARACK LUMBER INC
ALPA LUMBER GROUP

Delivery Shiplist

DATE	09/10/18
SALES REP	Mario

JOB TRACK: 41239 **LAYOUT ID:** 300330 **LOCATION:**
BUILDER: ROYAL PINE-FORESTSIDE PH.6 **SUB-BUILDER:**
MODEL: UNIT2203 **ELEVATION:** A LT 12

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	LUS26-2	
3	Hangers	LUS24	

TOTAL # ITEMS= **4.00**

TAMARACK LUMBER INC
ALPA LUMBER GROUP

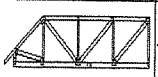
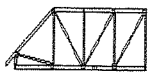
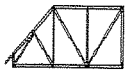
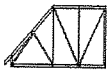
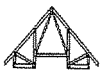
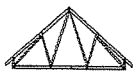
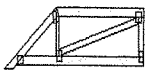
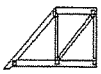
Delivery Shiplist

DATE	09/10/18
SALES REP	Mario

JOB TRACK: 41239 LAYOUT ID: 300332 LOCATION:
BUILDER: ROYAL PINE-FORESTSIDE PH.6 SUB-BUILDER:
MODEL: UNIT2202 ELEVATION: A LT 11

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	2	G100 GABLE	6.00 0.00	25-11-00	09-01-04	2 X 4	2 X 4	00-00-00 00-00-00	01-02-00 09-01-04	267.92 168.66		
	1 2 Ply	T12 HALF HIP	8.00 0.00	19-07-08	04-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-04-13 04-01-04	202.20 124.68		
	2	T13 HALF HIP	8.00 0.00	19-07-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-01-04	170.12 108.66		
	2	T14 HALF HIP	8.00 0.00	19-07-08	06-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 06-01-04	170.92 108.00		
	2	T15 HALF HIP	8.00 0.00	19-07-08	07-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 07-01-04	180.80 114.66		
	2	T16 HALF HIP	8.00 0.00	19-07-08	08-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 08-01-04	195.66 121.66		
	9	T17 PIGGYBACK	8.00 0.00	19-07-08	09-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 09-01-04	870.39 546.03		
	2	T18 HALF HIP	8.00 0.00	19-07-08	10-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 10-01-04	196.86 122.66		
	1	T19 GABLE	8.00 0.00	19-10-00	08-00-02	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	98.60 62.33		
	1	T20 COMMON	8.00 0.00	19-10-00	08-00-02	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	86.45 54.33		
	4	T20A COMMON	8.00 0.00	19-07-08	08-00-02	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	336.76 214.68		
	2	T21 ROOF	8.00 0.00	11-10-00	05-04-02	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	109.90 70.68		
	1	T90 COMMON	6.00 0.00	20-10-00	06-04-08	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	81.86 51.00		
	1	G90 COMMON	6.00 0.00	20-10-00	06-04-08	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	85.23 54.00		
	2	P1G GABLE	6.00 0.00	09-01-03	04-10-12	2 X 4	2 X 4	00-00-00 00-00-00	00-00-00 04-10-12	67.48 45.00		
	2	P5 PIGGYBACK	8.00 0.00	07-03-15	01-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 01-10-08	49.28 33.66		
	2	P6 PIGGYBACK	8.00 0.00	07-03-15	02-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 02-10-08	52.40 35.00		
	3	P7 PIGGYBACK	8.00 0.00	07-03-15	03-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 03-10-08	85.05 57.99		

TAMARACK LUMBER INC
ALPA LUMBER GROUP

Delivery Shiplist

DATE	09/10/18
SALES REP	Mario

JOB TRACK: 41239 **LAYOUT ID:** 300332 **LOCATION:**
BUILDER: ROYAL PINE-FORESTSIDE PH.6 **SUB-BUILDER:**
MODEL: UNIT2202 **ELEVATION:** A- LT. 11

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	10	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	167.90 106.70		
	5	J90 JACK	4.00 0.00	06-05-00	03-00-14	2 X 4	2 X 4	01-03-08 00-00-00	00-11-03 03-00-14	85.95 56.65		

TOTAL # TRUSS= **57.00** TOTAL BFT OF ALL TRUSSES= **2257.03** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3561.73** LBS.

TAMARACK LUMBER INC
ALPA LUMBER GROUP

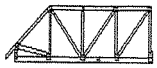
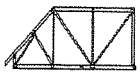
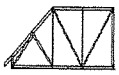
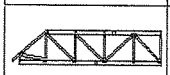
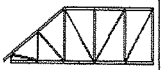
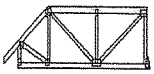
Delivery Shiplist

DATE	09/07/18
SALES REP	Mario

JOB TRACK: 41239 LAYOUT ID: 300333 LOCATION:
BUILDER: ROYAL PINE-FORESTSIDE PH.6 SUB-BUILDER:
MODEL: UNIT2201 ELEVATION: A LT. 10

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	1 2 Ply	T302Z HALF HIP	8.00 0.00	19-05-00	04-07-04	2 X 4	2 X 6	01-03-08 00-00-00	01-04-13 00-00-04	209.54 126.00		
	1	T303 HALF HIP	8.00 0.00	19-05-00	05-07-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 00-00-04	83.02 53.67		
	1	T304 HALF HIP	8.00 0.00	19-05-00	06-07-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 00-00-04	87.82 56.83		
	1	T305 HALF HIP	8.00 0.00	19-05-00	07-07-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 00-00-04	94.99 60.50		
	1	T306 HALF HIP	8.00 0.00	19-05-00	08-07-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 00-00-04	100.65 63.67		
	1	T307 HALF HIP	8.00 0.00	19-05-00	09-07-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 00-00-04	99.23 62.50		
	1	T308 HALF HIP	8.00 0.00	19-05-00	10-07-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 00-00-04	103.68 65.00		
	1 4 Ply	T309 HALF HIP	8.00 0.00	19-05-00	11-07-04	2 X 4	2 X 6	01-03-08 00-00-00	01-04-13 00-00-04	571.80 358.68		
	1	T318A HALF HIP	6.00 0.00	29-03-08	03-11-13	2 X 4	2 X 6	01-03-08 00-00-00	01-02-00 03-11-13	157.44 95.00		
	1	T319A HALF HIP	6.00 0.00	29-03-08	05-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 05-03-13	121.58 75.00		
	1	T320A HALF HIP	6.00 0.00	29-03-08	06-07-13	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 06-07-13	125.30 79.83		
	1	T321A HALF HIP	6.00 0.00	29-03-08	07-11-13	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 07-11-13	136.79 85.00		
	4	T322A HALF HIP	6.00 0.00	29-03-08	09-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 09-03-13	562.40 345.32		
	1	T323A HALF HIP	6.00 0.00	29-03-08	10-07-13	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 10-07-13	158.91 99.33		
	1	T324 HALF HIP	8.00 0.00	11-05-00	03-01-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 00-00-04	47.20 30.17		
	1	T325 HALF HIP	8.00 0.00	11-05-00	06-05-13	2 X 4	2 X 4	00-00-00 00-00-00	03-04-13 00-00-04	57.92 36.33		
	1	T326 HALF HIP	8.00 0.00	11-05-00	07-09-13	2 X 4	2 X 4	00-00-00 00-00-00	03-04-13 00-00-04	62.28 39.83		
	1	T327 HALF HIP	8.00 0.00	11-05-00	09-01-13	2 X 4	2 X 4	00-00-00 00-00-00	03-04-13 00-00-04	70.64 44.50		

TAMARACK LUMBER INC
ALPA LUMBER GROUP

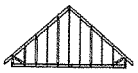
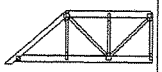
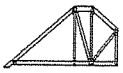
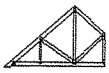
Delivery Shiplist

DATE	09/07/18
SALES REP	Mario

JOB TRACK: 41239 **LAYOUT ID:** 300333 **LOCATION:**
BUILDER: ROYAL PINE-FORESTSIDE PH.6 **SUB-BUILDER:**
MODEL: UNIT2201 **ELEVATION:** A LT.10

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T328 HALF HIP	8.00 0.00	11-05-00	10-05-13	2 X 6	2 X 4	00-00-00 00-00-00	03-04-13 00-00-04	81.47 51.17		
	1	T90 COMMON	6.00 0.00	20-10-00	06-04-08	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	81.86 51.00		
	1	G90 COMMON	6.00 0.00	20-10-00	06-04-08	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	85.23 54.00		
	1	P8A PIGGYBACK	6.00 0.00	12-00-09	02-06-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-03 02-03-00	41.19 25.33		
	1	P9A PIGGYBACK	6.00 0.00	12-00-09	03-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-03 02-04-15	43.42 28.67		
	1	P10A PIGGYBACK	6.00 0.00	12-00-09	04-04-11	2 X 4	2 X 4	00-00-00 00-00-00	00-04-03 02-04-15	44.04 28.67		
	3	J100 JACK-OPEN	6.00 0.00	06-10-08	04-07-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-07-04	57.63 36.00		
	14	J2 JACK-OPEN	8.00 0.00	03-10-08	03-11-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 03-11-13	179.34 112.00		
	6	J30 JACK-OPEN	8.00 0.00	02-07-08	03-01-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 03-01-13	58.02 36.00		
	6	J91 JACK	4.00 0.00	05-05-00	02-08-14	2 X 4	2 X 4	01-03-08 00-00-00	00-11-03 02-08-14	89.04 60.00		

TOTAL # TRUSS= **60.00** TOTAL BFT OF ALL TRUSSES= **2260.00** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3612.43** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	LUS26-2	
3	Hangers	LUS24	
9	Hangers	LJS26DS	

TOTAL # ITEMS= **13.00**

TAMARACK LUMBER INC
ALPA LUMBER GROUP

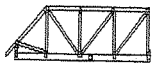
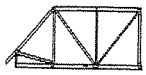
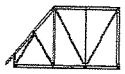
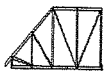
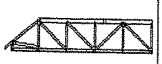
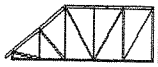
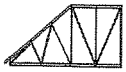
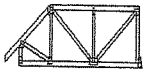
Delivery Shiplist

DATE	09/07/18
SALES REP	Mario

JOB TRACK: 41239 LAYOUT ID: 300581 LOCATION:
BUILDER: ROYAL PINE-FORESTSIDE PH.6 SUB-BUILDER:
MODEL: UNIT2201 ELEVATION: A1 LT9

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T310 HALF HIP	8.00 0.00	19-05-00	04-03-08	2 X 4	2 X 6	01-03-08 00-00-00	01-04-13 00-00-04	204.90 128.00		
	1	T311 HALF HIP	8.00 0.00	19-05-00	05-03-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 00-00-04	79.84 49.33		
	1	T312 HALF HIP	8.00 0.00	19-05-00	06-03-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 00-00-04	86.29 54.67		
	1	T313 HALF HIP	8.00 0.00	19-05-00	07-03-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 00-00-04	93.28 58.83		
	1	T314 HALF HIP	8.00 0.00	19-05-00	08-03-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 00-00-04	98.85 62.17		
	1	T315 HALF HIP	8.00 0.00	19-05-00	09-03-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 00-00-04	97.64 60.67		
	1	T316 HALF HIP	8.00 0.00	19-05-00	10-03-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 00-00-04	99.14 62.00		
	1 4 Ply	T317 HALF HIP	8.00 0.00	19-05-00	11-03-08	2 X 4	2 X 6	01-03-08 00-00-00	01-04-13 00-00-04	564.24 350.68		
	1	T318 HALF HIP	6.00 0.00	29-11-00	03-11-13	2 X 4	2 X 6	01-03-08 00-00-00	01-02-00 03-11-13	160.30 97.67		
	1	T319 HALF HIP	6.00 0.00	29-11-00	05-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 05-03-13	123.36 75.00		
	1	T320 HALF HIP	6.00 0.00	29-11-00	06-07-13	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 06-07-13	127.07 79.83		
	1	T321 HALF HIP	6.00 0.00	29-11-00	07-11-13	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 07-11-13	138.49 85.67		
	4	T322 HALF HIP	6.00 0.00	29-11-00	09-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 09-03-13	569.76 348.00		
	1	T323 HALF HIP	6.00 0.00	29-11-00	10-07-13	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 10-07-13	160.65 99.33		
	1	T329 HALF HIP	8.00 0.00	11-05-00	03-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 00-00-04	47.87 30.83		
	1	T330 HALF HIP	8.00 0.00	11-05-00	06-07-13	2 X 4	2 X 4	00-00-00 00-00-00	03-04-13 00-00-04	54.55 35.67		
	1	T331 HALF HIP	8.00 0.00	11-05-00	07-11-13	2 X 4	2 X 4	00-00-00 00-00-00	03-04-13 00-00-04	62.91 40.33		
	1	T332 HALF HIP	8.00 0.00	11-05-00	09-03-13	2 X 4	2 X 4	00-00-00 00-00-00	03-04-13 00-00-04	71.37 45.00		

TAMARACK LUMBER INC
ALPA LUMBER GROUP

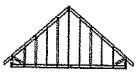
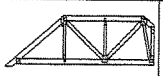
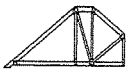
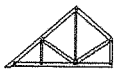
Delivery Shiplist

DATE	09/07/18
SALES REP	Mario

JOB TRACK: 41239 LAYOUT ID: 300581 LOCATION:
BUILDER: ROYAL PINE-FORESTSIDE PH.6 SUB-BUILDER:
MODEL: UNIT2201 ELEVATION: A LT9

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T90 COMMON	6.00 0.00	20-10-00	06-04-08	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	81.86 51.00		
	1	G90 COMMON	6.00 0.00	20-10-00	06-04-08	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	85.23 54.00		
	1	P8 PIGGYBACK	6.00 0.00	12-08-01	02-06-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-03 02-01-03	44.19 28.17		
	1	P9 PIGGYBACK	6.00 0.00	12-08-01	03-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-03 02-01-03	44.88 29.33		
	1	P10 PIGGYBACK	6.00 0.00	12-08-01	04-04-11	2 X 4	2 X 4	00-00-00 00-00-00	00-04-03 02-01-03	45.55 29.33		
	14	J2 JACK-OPEN	8.00 0.00	03-10-08	03-11-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 03-11-13	179.34 112.00		
	6	J3 JACK-OPEN	8.00 0.00	02-10-08	03-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 03-03-13	61.80 40.02		
	3	J101 JACK-OPEN	6.00 0.00	06-03-00	04-03-08	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 00-04-11	53.10 33.99		
	6	J91 JACK	4.00 0.00	05-05-00	02-08-14	2 X 4	2 X 4	01-03-08 00-00-00	00-11-03 02-08-14	89.04 60.00		

TOTAL # TRUSS= **59.00** TOTAL BFT OF ALL TRUSSES= **2201.52** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3525.50** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	LUS26-2	
3	Hangers	LUS24	
9	Hangers	LJS26DS	

TOTAL # ITEMS= **13.00**

TAMARACK LUMBER INC
ALPA LUMBER GROUP

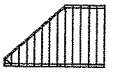
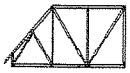
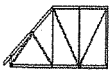
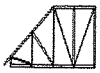
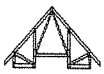
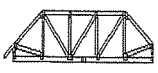
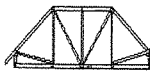
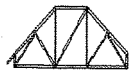
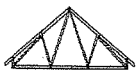
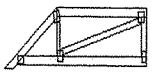
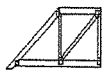
Delivery Shiplist

DATE	09/10/18
SALES REP	Mario

JOB TRACK: 41239 LAYOUT ID: 300334 LOCATION:
BUILDER: ROYAL PINE-FORESTSIDE PH.6 SUB-BUILDER:
MODEL: UNIT2202 ELEVATION: A LT 8

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	2	G29 GABLE	6.00 0.00	26-02-00	09-01-04	2 X 4	2 X 4	00-00-00 00-00-00	01-02-00 09-01-04	269.82 170.00		
	2	T13 HALF HIP	8.00 0.00	19-07-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-01-04	170.12 108.66		
	2	T14 HALF HIP	8.00 0.00	19-07-08	06-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 06-01-04	170.92 108.00		
	2	T15 HALF HIP	8.00 0.00	19-07-08	07-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 07-01-04	180.80 114.66		
	2	T16 HALF HIP	8.00 0.00	19-07-08	08-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 08-01-04	195.66 121.66		
	10	T17 PIGGYBACK	8.00 0.00	19-07-08	09-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 09-01-04	967.10 606.70		
	2	T18 HALF HIP	8.00 0.00	19-07-08	10-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 10-01-04	196.86 122.66		
	1	T19A GABLE	8.00 0.00	19-07-08	08-00-02	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	96.06 61.67		
	3	T20A COMMON	8.00 0.00	19-07-08	08-00-02	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	252.57 161.01		
	2	T21 ROOF	8.00 0.00	11-10-00	05-04-02	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	109.90 70.68		
	1 2 Ply	T30A HIP GIRDER	8.00 0.00	19-07-08	04-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-04-13 01-06-08	208.04 126.68		
	1	T31A HIP	8.00 0.00	19-07-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	81.31 52.50		
	1	T32A HIP	8.00 0.00	19-07-08	06-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	82.62 52.67		
	1	T90 COMMON	6.00 0.00	20-10-00	06-04-08	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	81.86 51.00		
	1	G90 COMMON	6.00 0.00	20-10-00	06-04-08	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	85.23 54.00		
	2	P12G GABLE	6.00 0.00	09-04-03	05-00-04	2 X 4	2 X 4	00-00-00 00-00-00	00-00-00 05-01-12	69.58 45.00		
	2	P5 PIGGYBACK	8.00 0.00	07-03-15	01-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 01-10-08	49.28 33.66		
	2	P6 PIGGYBACK	8.00 0.00	07-03-15	02-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 02-10-08	52.40 35.00		

TAMARACK LUMBER INC
ALPA LUMBER GROUP

Delivery Shiplist

DATE	09/10/18
SALES REP	Mario

JOB TRACK: 41239 LAYOUT ID: 300334 LOCATION:
BUILDER: ROYAL PINE-FORESTSIDE PH.6 SUB-BUILDER:
MODEL: UNIT2202 ELEVATION: A LT 8

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	4	P7 PIGGYBACK	8.00 0.00	07-03-15	03-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 03-10-08	113.40 77.32		
	10	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	167.90 106.70		
	5	J90 JACK	4.00 0.00	06-05-00	03-00-14	2 X 4	2 X 4	01-03-08 00-00-00	00-11-03 03-00-14	85.95 56.65		

TOTAL # TRUSS= **59.00** TOTAL BFT OF ALL TRUSSES= **2336.88** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3687.38** LBS.

TAMARACK LUMBER INC
ALPA LUMBER GROUP

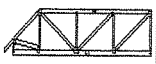
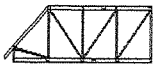
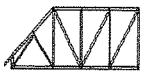
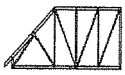
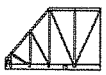
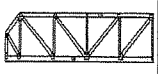
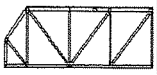
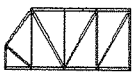
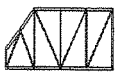
Delivery Shiplist

DATE	09/07/18
SALES REP	Mario

JOB TRACK: 41239 LAYOUT ID: 300335 LOCATION:
BUILDER: ROYAL PINE-FORESTSIDE PH.6 SUB-BUILDER:
MODEL: UNIT2204 CORNER ELEVATION: A LT 7

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	2	G29 GABLE	6.00 0.00	26-02-00	09-01-04	2 X 4	2 X 4	00-00-00 00-00-00	01-02-00 09-01-04	269.82 170.00		
	1 2 Ply	T33 HALF HIP	8.00 0.00	22-03-08	04-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-04-13 04-01-04	222.92 138.68		
	1	T34 HALF HIP	8.00 0.00	22-03-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-01-04	92.73 57.67		
	1	T35 HALF HIP	8.00 0.00	22-03-08	06-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 06-01-04	100.66 64.00		
	1	T36 HALF HIP	8.00 0.00	22-03-08	07-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 07-01-04	107.27 67.17		
	1 2 Ply	T37 HALF HIP	8.00 0.00	22-03-08	08-01-04	2 X 4	2 X 8	00-00-00 00-00-00	01-04-13 08-01-04	294.14 187.34		
	1	T38A HALF HIP	8.00 0.00	20-10-08	09-01-04	2 X 4	2 X 4	00-00-00 00-00-00	02-04-02 09-01-04	106.28 66.33		
	6	T39A PIGGYBACK	8.00 0.00	20-10-08	10-01-04	2 X 4	2 X 4	00-00-00 00-00-00	02-04-02 10-01-04	666.72 412.98		
	1	T40A HALF HIP	8.00 0.00	20-10-08	11-01-04	2 X 4	2 X 4	00-00-00 00-00-00	02-04-02 11-01-04	120.01 74.17		
	1 2 Ply	T41A HALF HIP	10.00 0.00	21-10-08	04-01-04	2 X 4	2 X 6	00-00-00 00-00-00	02-04-14 04-01-04	230.22 140.00		
	1	T42A HALF HIP	10.00 0.00	21-10-08	05-01-04	2 X 4	2 X 4	00-00-00 00-00-00	02-04-14 05-01-04	90.79 56.83		
	1	T43A HALF HIP	10.00 0.00	21-10-08	06-01-04	2 X 4	2 X 4	00-00-00 00-00-00	02-04-14 06-01-04	96.52 60.17		
	1	T44A HALF HIP	10.00 0.00	21-10-08	07-01-04	2 X 4	2 X 4	00-00-00 00-00-00	02-04-14 07-01-04	105.96 66.33		
	1	T45A HALF HIP	10.00 0.00	21-10-08	08-01-04	2 X 4	2 X 4	00-00-00 00-00-00	02-04-14 08-01-04	112.61 70.17		
	1 3 Ply	T46 HALF HIP	10.00 0.00	22-09-08	08-06-00	2 X 6	2 X	00-00-00 00-00-00	01-07-11 08-06-00	596.28 363.99		
	1	T47 HALF HIP	10.00 0.00	22-09-08	09-06-00	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 09-06-00	121.82 74.67		
	1	T48 HALF HIP	10.00 0.00	22-09-08	10-06-00	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 10-06-00	127.70 78.67		
	1	T49 HALF HIP	10.00 0.00	22-09-08	11-05-02	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 11-05-03	140.33 85.67		

TAMARACK LUMBER INC
ALPA LUMBER GROUP

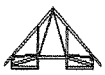
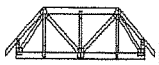
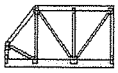
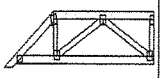
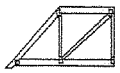
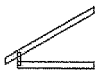
Delivery Shiplist

DATE	09/07/18
SALES REP	Mario

JOB TRACK: 41239 LAYOUT ID: 300335 LOCATION:
BUILDER: ROYAL PINE-FORESTSIDE PH.6 SUB-BUILDER:
MODEL: UNIT2204 CORNER ELEVATION: A LT. 7

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T50 HALF HIP	10.00 0.00	22-09-08	11-05-02	2 X 4	2 X 8	00-00-00 00-00-00	01-07-11 11-05-02	352.78 218.66		
	2	T51 ROOF	8.00 0.00	14-08-00	06-03-08	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	155.08 98.00		
	1 2 Ply	T52X COMMON	8.00 0.00	15-05-00	06-03-08	2 X 4	2 X 6	00-00-00 00-00-00	01-01-13 01-01-13	166.50 105.34		
	1 2 Ply	T53 HIP GIRDER	8.00 0.00	16-04-00	03-07-14	2 X 4	2 X 6	01-03-08 01-03-08	01-04-13 01-04-13	186.56 121.34		
	1	T54A HALF HIP	8.70 0.00	11-07-00	03-09-08	2 X 4	2 X 6	00-00-00 00-00-00	01-05-13 03-09-08	54.92 35.67		
	1	T55A HIP	8.70 0.00	11-07-00	05-01-08	2 X 4	2 X 4	00-00-00 00-00-00	01-05-13 03-10-01	53.30 35.00		
	2	P12G GABLE	6.00 0.00	09-04-03	05-00-04	2 X 4	2 X 4	00-00-00 00-00-00	00-00-00 05-01-12	69.58 45.00		
	1	P13 PIGGYBACK	8.00 0.00	08-05-15	01-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 01-10-08	28.56 19.67		
	4	P14 PIGGYBACK	8.00 0.00	08-05-15	02-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 02-10-08	118.60 77.32		
	16	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	268.64 170.72		
	1	J4 JACK-OPEN	8.00 0.00	03-07-00	03-09-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 03-09-08	12.07 8.00		
	2	J5 JACK-OPEN	8.70 0.00	03-00-00	03-07-14	2 X 4	2 X 4	01-05-00 00-00-00	01-05-13 03-07-14	22.14 14.66		
	5	J92 JACK	4.00 0.00	06-05-00	02-11-08	2 X 4	2 X 4	00-11-08 00-00-00	00-09-13 00-05-08	83.25 53.35		

TOTAL # TRUSS= **70.00** TOTAL BFT OF ALL TRUSSES= **3237.57** BFT. TOTAL WEIGHT OF ALL TRUSSES= **5174.76** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	HGUS28-2	
2	Hangers	HGUS26-2	
2	Hangers	LUS26-2	
16	Hangers	LJS26DS	

TOTAL # ITEMS= **23.00**

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300330	T1	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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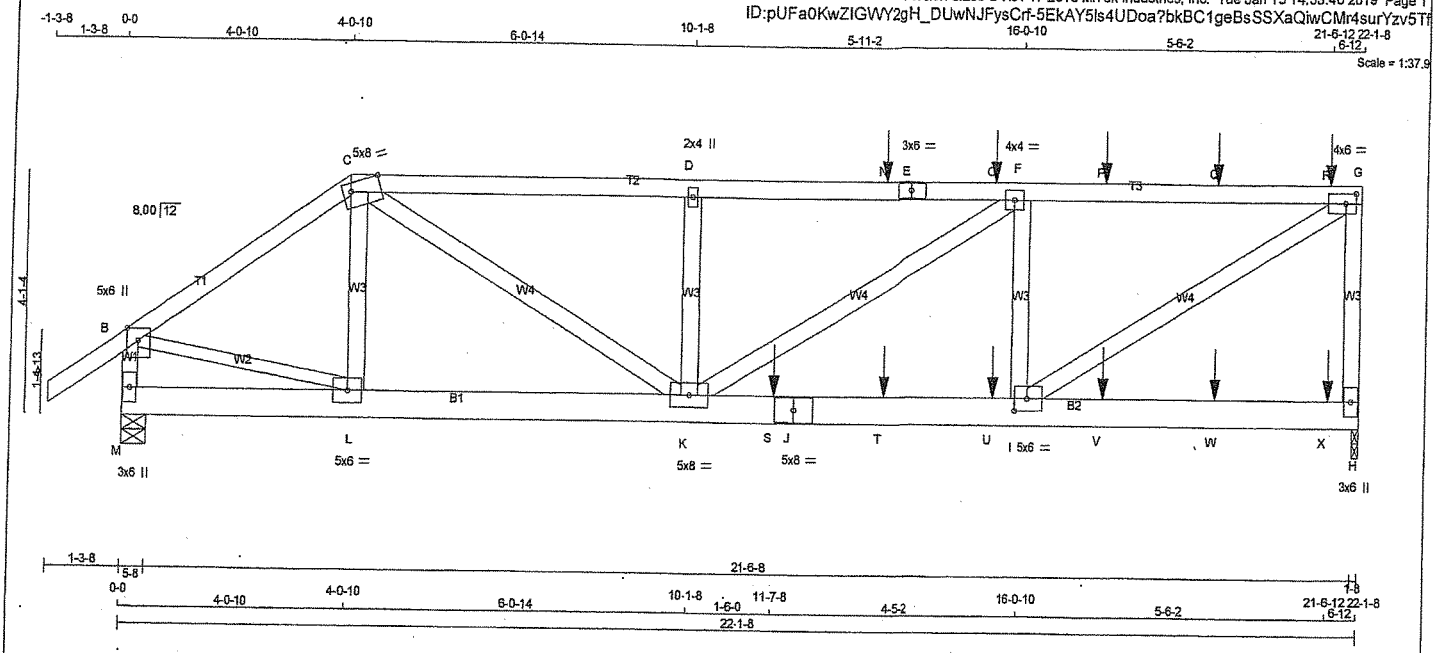
ID:pUFa0KwZIGWY2gH_DUwNJFysCrf-5EkAY5ls4UDoa?bkBC1geBsSSXaQiwCMr4surYzv5Tf



TOTAL WEIGHT = 2 X 110 = 219 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
B - L	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	TOP
C-E	1	12	SIDE(0.0)
E-G	1	12	SIDE(0.0)
G-H	1	12	TOP
M-B	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
M-J	2	12	TOP
J-H	2	12	SIOE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIOE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is In inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW-m	MT20	5.0	8.0	Edge	6.25
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW-t	MT20	4.0	6.0	2.00	2.25
H	BMV1+p	MT20	3.0	8.0		
I	BMWW-t	MT20	5.0	6.0	2.50	2.75
J	BS-t	MT20	5.0	8.0		
K	BMWWW-t	MT20	5.0	8.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	2749	0	2749	0	0	1-8	1-8
M	2307	0	2307	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	2078	1057 / 0	449 / 0	0 / 0	0 / 0	572 / 0	0 / 0
M	1732	917 / 0	347 / 0	0 / 0	0 / 0	468 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.9	-84.9	0.06 (1)		10.00
B-C	-2548 / 0	-84.9	-84.9	0.16 (1)		5.51
C-O	-4494 / 0	-84.9	-84.9	0.36 (1)		4.23
D-N	-4494 / 0	-84.9	-84.9	0.53 (1)		4.00
N-E	-4494 / 0	-84.9	-84.9	0.53 (1)		4.00
E-O	-4494 / 0	-84.9	-84.9	0.53 (1)		4.00
O-F	-4494 / 0	-84.9	-84.9	0.53 (1)		4.00
F-P	-3645 / 0	-84.9	-84.9	0.48 (1)		4.41
P-Q	-3645 / 0	-84.9	-84.9	0.48 (1)		4.41
Q-R	-3645 / 0	-84.9	-84.9	0.48 (1)		4.41
R-G	-3645 / 0	-84.9	-84.9	0.48 (1)		4.41
H-G	-2611 / 0	0.0	0.0	0.33 (1)		7.03
M-B	-2210 / 0	0.0	0.0	0.12 (1)		7.50
M-L	0 / 0	-38.5	-38.5	0.05 (1)		10.00
L-K	0 / 2100	-38.5	-38.5	0.36 (1)		10.00
K-S	0 / 3645	-38.5	-38.5	0.66 (1)		10.00
S-J	0 / 3645	-38.5	-38.5	0.66 (1)		10.00
J-T	0 / 3645	-38.5	-38.5	0.66 (1)		10.00
T-U	0 / 3645	-38.5	-38.5	0.66 (1)		10.00
U-I	0 / 3645	-38.5	-38.5	0.66 (1)		10.00
I-V	0 / 0	-38.5	-38.5	0.19 (2)		10.00
V-W	0 / 0	-38.5	-38.5	0.19 (2)		10.00
W-X	0 / 0	-38.5	-38.5	0.19 (2)		10.00
X-H	0 / 0	-38.5	-38.5	0.19 (2)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L-C	-440 / 0	0.04 (1)
K-D	0 / 2607	0.25 (1)
K-D	-563 / 0	0.05 (1)
K-F	0 / 1004	0.09 (1)
I-F	-1440 / 0	0.13 (1)
I-G	0 / 4277	0.38 (1)
B-L	0 / 3176	0.27 (1)

FACTOREO CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
N	13-6-12	-102	-102	—	FRONT	VERT	TOTAL	—	—
O	15-6-12	-102	-102	—	FRONT	VERT	TOTAL	—	—
P	17-6-12	-102	-102	—	FRONT	VERT	TOTAL	—	—
Q	19-6-12	-102	-102	—	FRONT	VERT	TOTAL	—	—
R	21-6-12	-121	-121	—	FRONT	VERT	TOTAL	—	—
S	11-7-8	-1398	-1398	—	TOP	VERT	TOTAL	—	—
T	13-6-12	-55	-70	—	FRONT	VERT	TOTAL	—	—
U	15-6-12	-55	-70	—	FRONT	VERT	TOTAL	—	—
V	17-6-12	-55	-70	—	FRONT	VERT	TOTAL	—	—
W	19-6-12	-55	-70	—	FRONT	VERT	TOTAL	—	—
X	21-6-12	-63	-81	—	FRONT	VERT	TOTAL	—	—

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.53/1.00 (D-F:1) , BC=0.66/1.00 (I-K:1) , WB=0.38/1.00 (G-I:1) , SSI=0.40/1.00 (I-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

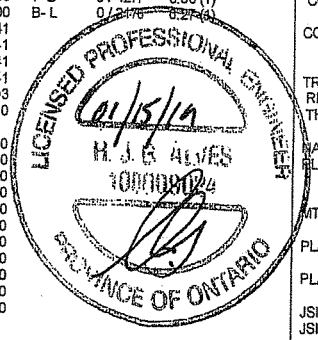
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (G) (INPUT = 0.90)
JSI METAL= 0.82 (J) (INPUT = 1.00)



DWG NO. TAM T1901144
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300330	T2	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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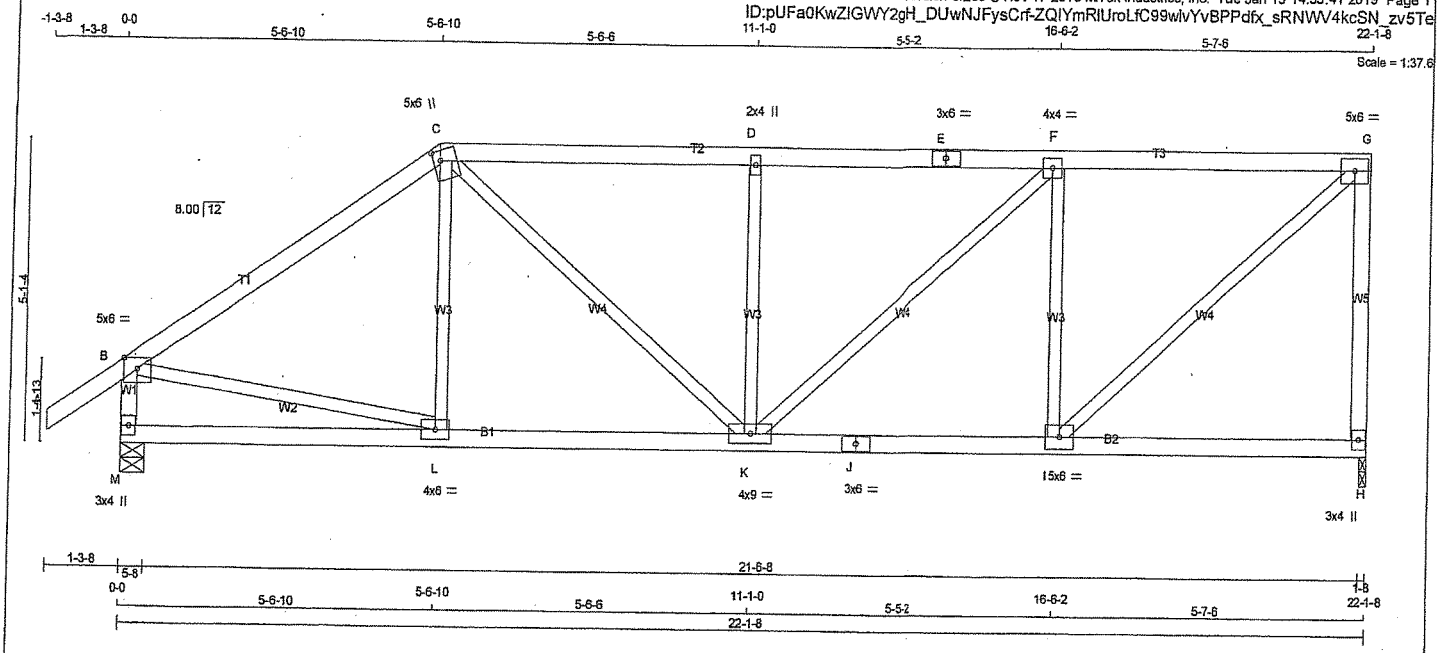

TOTAL WEIGHT = 2 X 92 = 184 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	ORY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	ORY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	ORY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	ORY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	9.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1365	0	1365	0	0	1-8	1-8
M	1481	0	1481	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1035	515 / 0	232 / 0	0 / 0	0 / 0	288 / 0	0 / 0
M	1116	579 / 0	232 / 0	0 / 0	0 / 0	304 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.71 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTOREO FORCE (LBS)	MAX CSI (LC)
A- B	0 / 32	-84.9	-84.9	0.11 (1)		10.00	L- C	0 / 188	0.04 (3)
B- C	-1457 / 0	-84.9	-84.9	0.55 (1)		4.74	B- L	0 / 1233	0.28 (1)
C- D	-1632 / 0	-84.9	-84.9	0.40 (1)		4.73	I- G	0 / 1666	0.37 (1)
D- E	-1632 / 0	-84.9	-84.9	0.42 (1)		4.71	C- K	0 / 557	0.13 (1)
E- F	-1632 / 0	-84.9	-84.9	0.42 (1)		4.71	I- F	-841 / 0	0.32 (1)
F- G	-1265 / 0	-84.9	-84.9	0.40 (1)		5.22	K- D	-502 / 0	0.19 (1)
H- G	-1278 / 0	0.0	0.0	0.56 (1)		7.13	K- F	0 / 491	0.11 (1)
M- B	-1394 / 0	0.0	0.0	0.14 (1)		6.90			
M- L	0 / 0	-38.5	-38.5	0.23 (3)		10.00			
L- K	0 / 1211	-38.5	-38.5	0.39 (2)		10.00			
K- J	0 / 1265	-38.5	-38.5	0.39 (2)		10.00			
J- I	0 / 1265	-38.5	-38.5	0.39 (2)		10.00			
I- H	0 / 0	-38.5	-38.5	0.22 (3)		10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 **IN. C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.56/1.00 (G-H:1) , BC=0.39/1.00 (I-K:2) , WB=0.37/1.00 (G-I:1) , SSI=0.22/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (B) (INPUT = 0.90)
JSI METAL= 0.47 (J) (INPUT = 1.00)

DWG NO. TAM T190 1145
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300330	T3	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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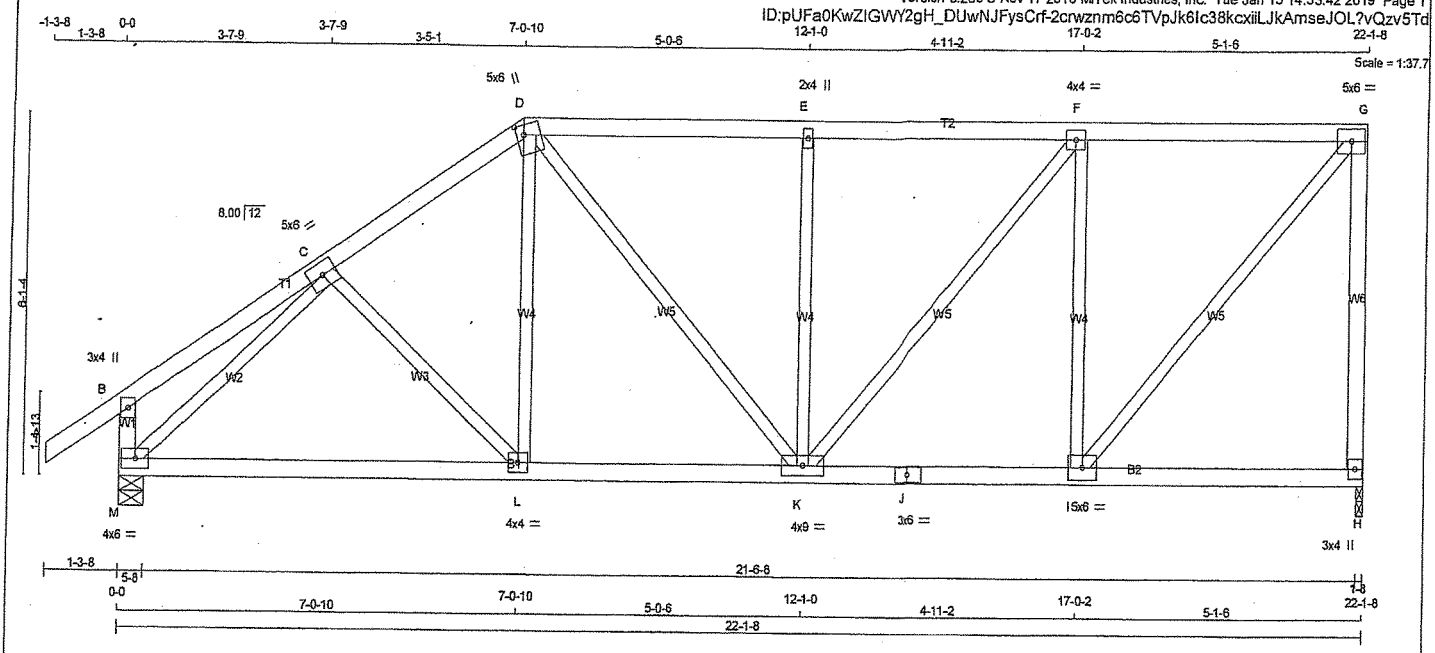


TOTAL WEIGHT = 2 X 100 = 200 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR
A - D	2x4	DRY	No.2	SPF
D - G	2x4	ORY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x4	ORY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	ORY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMV+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	9.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPDRTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1365	0	1365	0	0	1-8	1-8
M	1481	0	1481	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1035	515 / 0	232 / 0	0 / 0	0 / 0	288 / 0	0 / 0
M	1116	579 / 0	232 / 0	0 / 0	0 / 0	304 / 0	0 / 0

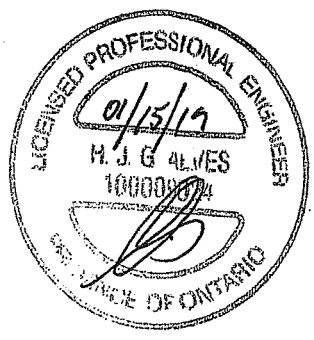
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.22 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTOREO VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTOREO FORCE (LBS)	MAX CSI (LC)
A-B	0 / 32	-84.9	-84.9	0.11 (1)	10.00	C-L	-8 / 92	0.02 (3)	
B-C	0 / 17	-84.9	-84.9	0.15 (1)	10.00	L-D	0 / 336	0.08 (2)	
C-D	-1412 / 0	-84.9	-84.9	0.18 (1)	5.29	M-C	-1625 / 0	0.62 (1)	
D-E	-1328 / 0	-84.9	-84.9	0.31 (1)	5.24	I-G	0 / 1479	0.33 (1)	
E-F	-1328 / 0	-84.9	-84.9	0.33 (1)	5.22	D-K	0 / 250	0.06 (1)	
F-G	-977 / 0	-84.9	-84.9	0.31 (1)	5.88	I-F	-883 / 0	0.52 (1)	
H-G	-1287 / 0	0.0	0.0	0.93 (1)	7.11	K-E	-456 / 0	0.27 (1)	
M-B	-234 / 0	0.0	0.0	0.02 (1)	7.81	K-F	0 / 543	0.12 (1)	
M-L	0 / 1164	-38.5	-38.5	0.47 (2)	10.00				
L-K	0 / 1164	-38.5	-38.5	0.48 (2)	10.00				
K-J	0 / 977	-38.5	-38.5	0.32 (2)	10.00				
J-I	0 / 977	-38.5	-38.5	0.32 (2)	10.00				
I-H	0 / 0	-38.5	-38.5	0.20 (3)	10.00				



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	OL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF 8CBC 2018 , OBC 2012
- CSA 088-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.93/1.00 (G-H:1) , BC=0.48/1.00 (K-L:2) , WB=0.62/1.00 (C-M:1) , SSI=0.20/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (M) (INPUT = 0.90)
JSI METAL= 0.41 (J) (INPUT = 1.00)

DWG NO. TAM T1901146
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300330	T4	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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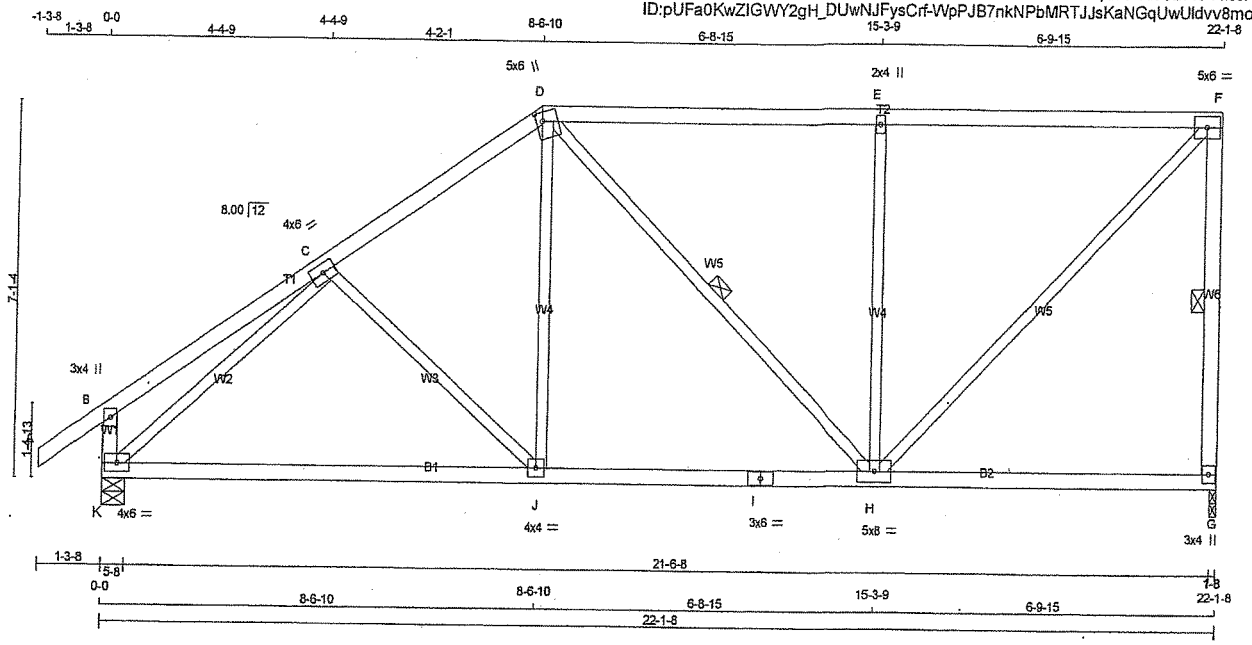


TOTAL WEIGHT = 2 X 97 = 195 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
O - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	ORY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
O	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	5.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1365	0	1365	0	0	1-8	1-8
K	1481	0	1481	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1035	515 / 0	232 / 0	0 / 0	0 / 0	288 / 0	0 / 0
K	1116	579 / 0	232 / 0	0 / 0	0 / 0	304 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H.

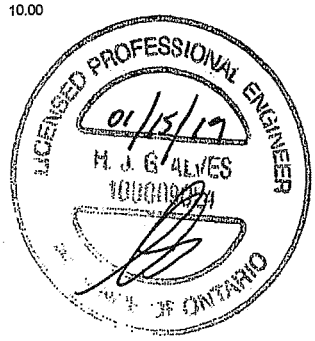
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.11 (1)		10.00
B- C	0 / 23	-84.9	-84.9	0.24 (1)		10.00
C- D	-1331 / 0	-84.9	-84.9	0.28 (1)		5.29
D- E	-1037 / 0	-84.9	-84.9	0.73 (1)		4.91
E- F	-1037 / 0	-84.9	-84.9	0.73 (1)		4.91
G- F	-1255 / 0	0.0	0.0	0.29 (1)		5.74
K- B	-257 / 0	0.0	0.0	0.03 (1)		7.81
K- J	0 / 1182	-38.5	-38.5	0.61 (2)		10.00
J- I	0 / 1093	-38.5	-38.5	0.61 (2)		10.00
I- H	0 / 1093	-38.5	-38.5	0.61 (2)		10.00
H- G	0 / 0	-38.5	-38.5	0.28 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- J	-128 / 53	0.08 (1)
J- D	0 / 470	0.11 (2)
K- C	-1606 / 0	0.90 (1)
H- F	0 / 1465	0.33 (1)
D- H	-99 / 0	0.05 (2)
H- E	-714 / 0	0.62 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.22")
ALLOWABLE DEFL.(TL)= L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/ 710 (0.37")

CSI: TC=0.73/1.00 (E-F:1) , BC=0.61/1.00 (J-K:2) , WB=0.90/1.00 (C-K:1) , SSI=0.28/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (K) (INPUT = 0.90)
JSI METAL= 0.39 (C) (INPUT = 1.00)

DWG NO. TAM T7901147
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300330	T5	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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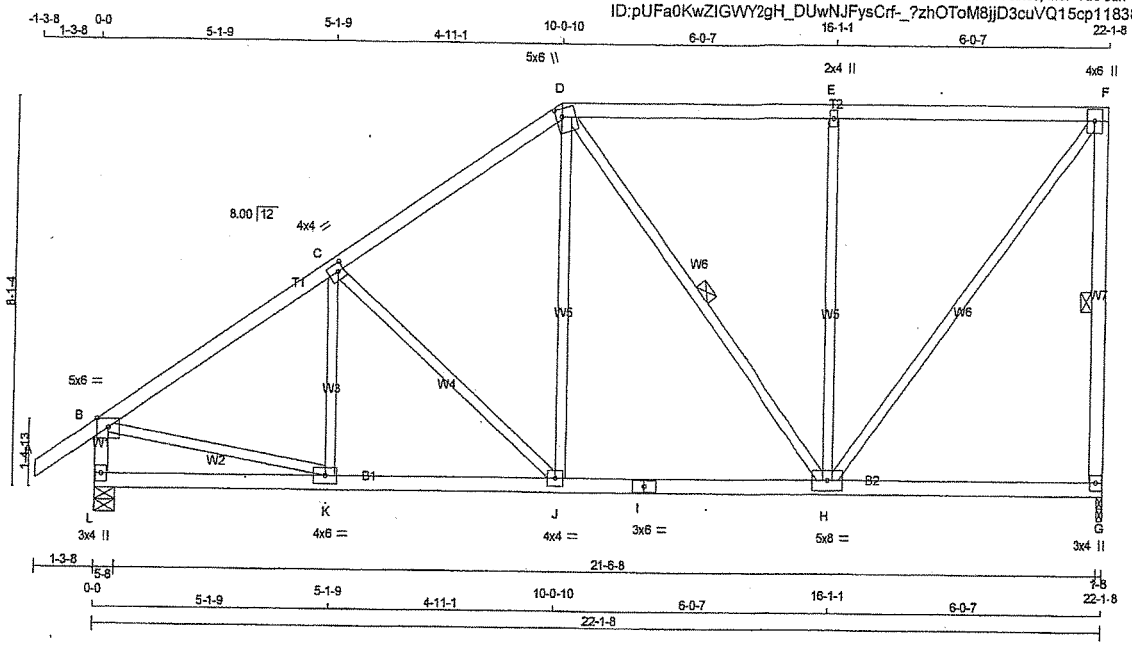


TOTAL WEIGHT = 2 X 105 = 209 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

ORY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	8.0		
I	8S-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	6.0		
L	8MV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1365	0	1385	0	0	1-8	1-8
L	1481	0	1481	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1035	515 / 0	232 / 0	0 / 0	0 / 0	288 / 0	0 / 0
L	1116	579 / 0	232 / 0	0 / 0	0 / 0	304 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.86 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

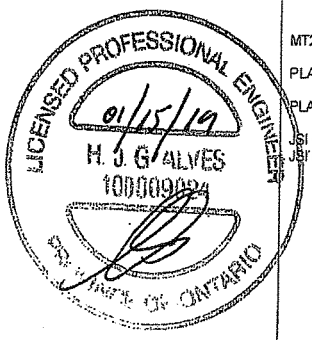
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.11 (1)	10.00
B- C	-1500 / 0	-84.9	-84.9	0.43 (1)	4.86
C- D	-1191 / 0	-84.9	-84.9	0.40 (1)	5.34
D- E	-836 / 0	-84.9	-84.9	0.55 (1)	5.76
E- F	-836 / 0	-84.9	-84.9	0.55 (1)	5.76
G- F	-1271 / 0	0.0	0.0	0.38 (1)	5.71
L- B	-1400 / 0	0.0	0.0	0.14 (1)	6.88
L- K	0 / 0	-38.5	-38.5	0.18 (3)	10.00
K- J	0 / 1271	-38.5	-38.5	0.34 (2)	10.00
J- I	0 / 972	-38.5	-38.5	0.39 (2)	10.00
I- H	0 / 972	-38.5	-38.5	0.39 (2)	10.00
H- G	0 / 0	-38.5	-38.5	0.27 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-53 / 139	0.03 (3)
C- J	-410 / 0	0.35 (1)
J- O	0 / 491	0.11 (2)
D- H	-222 / 0	0.15 (1)
H- E	-634 / 0	0.80 (1)
H- F	0 / 1362	0.31 (1)
B- K	0 / 1299	0.29 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.55/1.00 (E-F:1) , BC=0.39/1.00 (H-J:2) , WB=0.80/1.00 (E-H:1) , SSI=0.25/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SSI GRIP= 0.88 (K) (INPUT = 0.90)
SSI METAL= 0.39 (I) (INPUT = 1.00)

DWG NO. TAM T1901148
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300330	T6	9	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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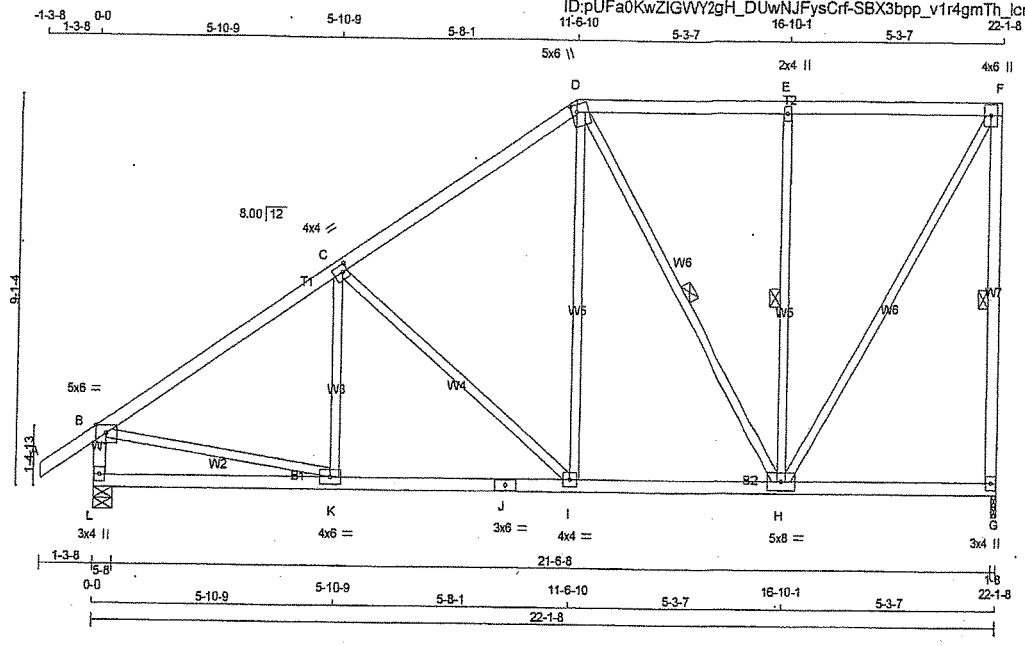


TOTAL WEIGHT = 9 X 110 = 992 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - G	2x4	ORY	No.2	SPF
ALL WEBS EXCEPT	2x3	ORY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1365	0	1365	0	0	1-8	1-8
L	1481	0	1481	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1035	515 / 0	232 / 0	0 / 0	0 / 0	288 / 0	0 / 0
L	1116	579 / 0	232 / 0	0 / 0	0 / 0	304 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

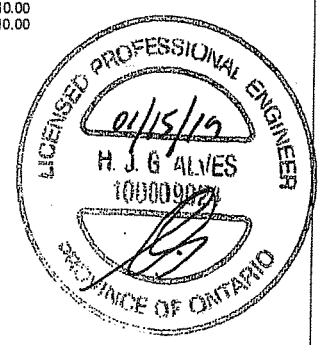
1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 32	-84.9	-84.9	0.11 (1)	10.00	K-C	0 / 207	0.05 (3)
B-C	-1497 / 0	-84.9	-84.9	0.42 (1)	4.91	C-I	-546 / 0	0.64 (1)
C-D	-1068 / 0	-84.9	-84.9	0.38 (1)	5.62	I-D	0 / 565	0.13 (2)
D-E	-670 / 0	-84.9	-84.9	0.30 (1)	6.25	D-H	-383 / 0	0.29 (1)
E-F	-670 / 0	-84.9	-84.9	0.30 (1)	6.25	H-E	-554 / 0	0.30 (1)
G-F	-1282 / 0	0.0	0.0	0.49 (1)	5.69	H-F	0 / 1296	0.29 (1)
L-B	-1390 / 0	0.0	0.0	0.14 (1)	6.90	B-K	0 / 1294	0.29 (1)
L-K	0 / 0	-38.5	-38.5	0.25 (3)	10.00			
K-J	0 / 1272	-38.5	-38.5	0.41 (2)	10.00			
J-I	0 / 1272	-38.5	-38.5	0.41 (2)	10.00			
I-H	0 / 868	-38.5	-38.5	0.31 (2)	10.00			
H-G	0 / 0	-38.5	-38.5	0.20 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.49/1.00 (F-G:1) , BC=0.41/1.00 (I-K:2) , WB=0.64/1.00 (C-I:1) , SSI=0.22/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (H) (INPUT = 0.90)
JSI METAL= 0.46 (J) (INPUT = 1.00)

DWG NO. TAM T1901149
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300330	T7	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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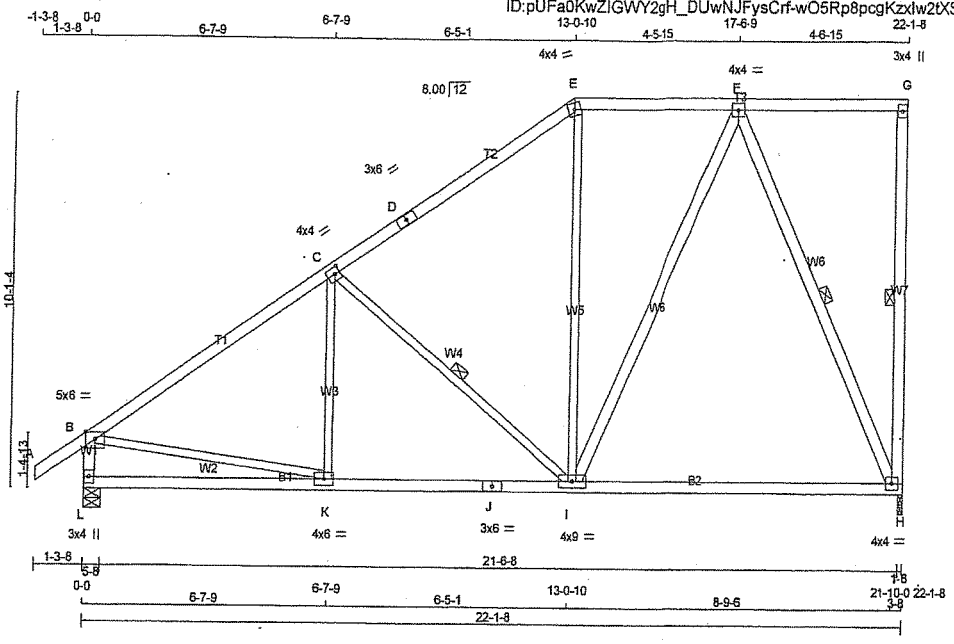

Scale = 1:57.2

TOTAL WEIGHT = 2 X 115 = 230 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	ORY	No.2	SPF
O - E	2x4	DRY	No.2	SPF
E - G	2x4	ORY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
I - F	2x4	ORY	No.2	SPF
F - H	2x4	ORY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REORD BRG
H	1365	0	1365	0	0	1-8	1-8
L	1481	0	1481	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1035	515 / 0	232 / 0	0 / 0	0 / 0	288 / 0	0 / 0
L	1116	579 / 0	232 / 0	0 / 0	0 / 0	304 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.74 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-H, C-I, F-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.11 (1)		10.00
B- C	-1472 / 0	-84.9	-84.9	0.55 (1)		4.74
C- O	-962 / 0	-84.9	-84.9	0.49 (1)		5.64
D- E	-962 / 0	-84.9	-84.9	0.49 (1)		5.64
E- F	-770 / 0	-84.9	-84.9	0.23 (1)		6.25
F- G	0 / 0	-84.9	-84.9	0.29 (1)		10.00
H- G	-148 / 0	0.0	0.0	0.07 (1)		6.25
L- B	-1372 / 0	0.0	0.0	0.14 (1)		6.94
L- K	0 / 0	-38.5	-38.5	0.27 (3)		10.00
K- J	0 / 1255	-38.5	-38.5	0.67 (2)		10.00
J- I	0 / 1255	-38.5	-38.5	0.67 (2)		10.00
I- H	0 / 502	-38.5	-38.5	0.56 (2)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	0 / 215	0.05 (3)
C- I	-637 / 0	0.32 (1)
I- E	0 / 217	0.05 (3)
B- K	0 / 1272	0.29 (1)
I- F	0 / 656	0.11 (1)
F- H	-1187 / 0	0.74 (1)

LICENSED PROFESSIONAL ENGINEER
01/15/19
H. J. G. ALVES
100009824
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	48.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.25")
ALLOWABLE DEFL.(TL)= L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/ 642 (0.41")

CSI: TC=0.55/1.00 (B-C:1) , BC=0.67/1.00 (I-K:2) , WB=0.74/1.00 (F-H:1) , SSI=0.23/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

USI GRIP= 0.81 (B) (INPUT = 0.90)
USI METAL= 0.48 (J) (INPUT = 1.00)

DWG NO. TAM T7@1/50
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300330	T8A	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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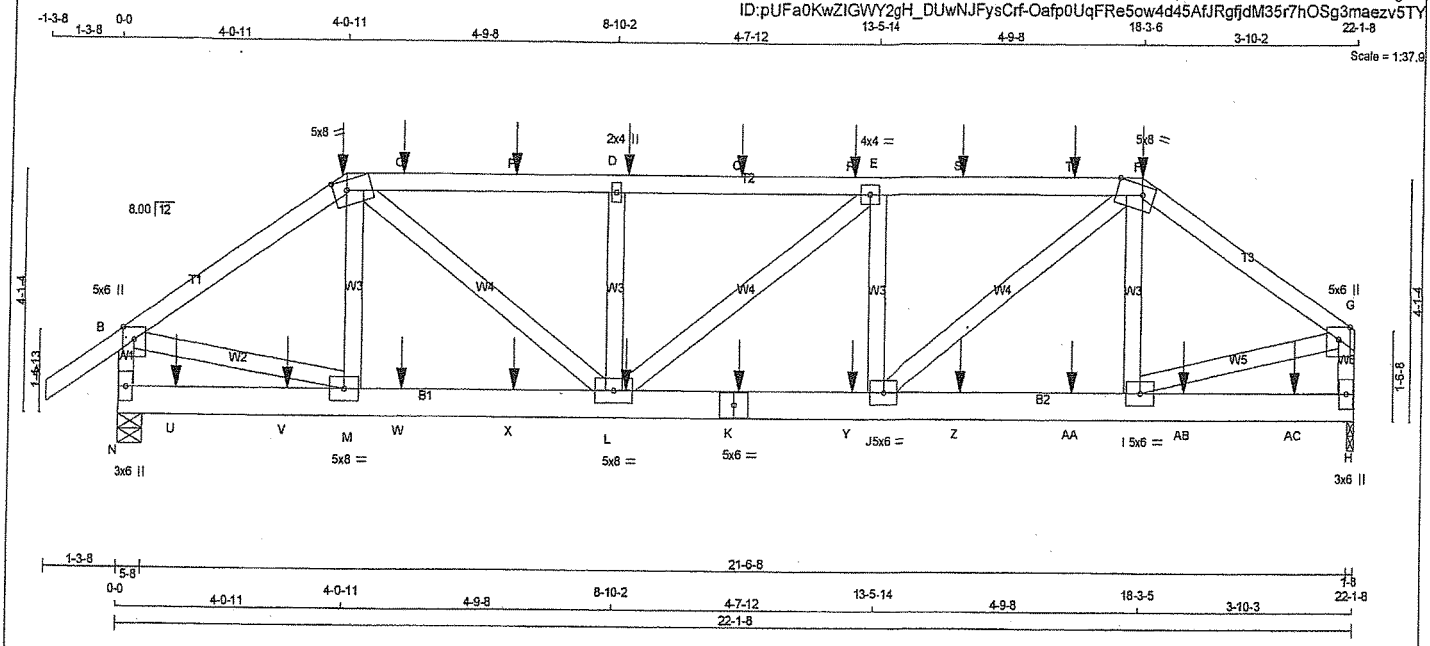


TOTAL WEIGHT = 2 X 114 = 227 lb

[M]

LUMBER

N. L G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- F	1	12	SIDE(61.0)
F- G	1	12	SIDE(61.0)
N- B	1	12	TOP
H- G	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
N- K	2	12	SIDE(183.1)
K- H	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW-m	MT20	5.0	8.0	2.00	2.75
O	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTWW-m	MT20	5.0	8.0	Edge	5.50
G	TMVW+p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	6.0		
I, J, M						
I	BMWW-t	MT20	5.0	6.0		
K	BS-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	5.0	6.0		
N	BMV1+p	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	2418	0	2418	0	0	5-8	5-8
H	2294	0	2294	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1629	927 / 0	398 / 0	0 / 0	0 / 0	504 / 0	0 / 0
H	1743	860 / 0	397 / 0	0 / 0	0 / 0	486 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.76 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

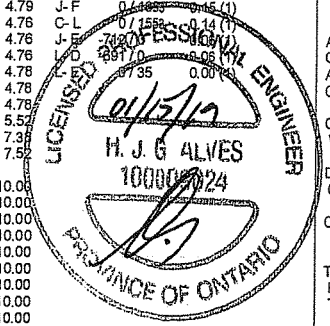
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.06 (1)	10.00
B- C	-2679 / 0	-84.9	-84.9	0.17 (1)	5.40
C- O	-3445 / 0	-84.9	-84.9	0.26 (1)	4.79
O- P	-3445 / 0	-84.9	-84.9	0.26 (1)	4.79
P- D	-3445 / 0	-84.9	-84.9	0.26 (1)	4.79
D- O	-3445 / 0	-84.9	-84.9	0.28 (1)	4.76
Q- R	-3445 / 0	-84.9	-84.9	0.28 (1)	4.76
R- E	-3445 / 0	-84.9	-84.9	0.28 (1)	4.76
E- S	-3418 / 0	-84.9	-84.9	0.28 (1)	4.78
S- T	-3418 / 0	-84.9	-84.9	0.28 (1)	4.78
T- F	-3418 / 0	-84.9	-84.9	0.28 (1)	4.78
F- G	-2555 / 0	-84.9	-84.9	0.15 (1)	5.52
N- B	-2308 / 0	0.0	0.0	0.13 (1)	7.38
H- G	-2190 / 0	0.0	0.0	0.12 (1)	7.52
N- U	0 / 0	-38.5	-38.5	0.06 (3)	10.00
U- V	0 / 0	-38.5	-38.5	0.06 (3)	10.00
V- M	0 / 0	-38.5	-38.5	0.06 (3)	10.00
M- W	0 / 2218	-38.5	-38.5	0.19 (1)	10.00
W- X	0 / 2218	-38.5	-38.5	0.19 (1)	10.00
X- L	0 / 2218	-38.5	-38.5	0.19 (1)	10.00
L- K	0 / 3418	-38.5	-38.5	0.27 (1)	10.00
K- Y	0 / 3418	-38.5	-38.5	0.27 (1)	10.00
Y- J	0 / 3418	-38.5	-38.5	0.27 (1)	10.00
J- Z	0 / 2111	-38.5	-38.5	0.17 (1)	10.00
Z-AA	0 / 2111	-38.5	-38.5	0.17 (1)	10.00
AA- I	0 / 2111	-38.5	-38.5	0.17 (1)	10.00
I-AB	0 / 0	-38.5	-38.5	0.06 (3)	10.00
AB-AC	0 / 0	-38.5	-38.5	0.06 (3)	10.00
AC- H	0 / 0	-38.5	-38.5	0.06 (3)	10.00

WEBS FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- C	-191 / 149	0.02 (1)
I- F	-264 / 94	0.03 (1)
B- M	0 / 2287	0.20 (1)
I- G	0 / 2207	0.20 (1)
J- F	0 / 1883	0.15 (1)
C- L	0 / 1558	0.14 (1)
J- E	-742 / 0	0.07 (1)
L- D	-891 / 0	0.08 (1)
L- E	-97 35	0.00 (3)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	4-0-11	-50	-56	—	FRONT	VERT	DEAD	—	—
C	4-0-11	-231	-231	—	FRONT	VERT	SNOW	—	—
D	9-0-12	-102	-102	—	BACK	VERT	TOTAL	—	—
F	18-3-6	-48	-53	—	FRONT	VERT	DEAD	—	—
F	18-3-6	-219	-219	—	FRONT	VERT	SNOW	—	—
K	11-0-12	-55	-70	—	BACK	VERT	TOTAL	—	—
L	9-0-12	-55	-70	—	BACK	VERT	TOTAL	—	—



DWG NO. TAM 1901451
STRUCTURAL COMPONENT ONLY

1/2

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.28/1.00 (E-F:1) , BC=0.27/1.00 (J-L:1) , WB=0.20/1.00 (B-M:1) , SSI=0.15/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (C) (INPUT = 0.90)
JSI METAL= 0.52 (B) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300330	T8A	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
O	5-0-12	-102	-102	—	BACK	VERT	TOTAL	—	—
P	7-0-12	-102	-102	—	BACK	VERT	TOTAL	—	—
Q	11-0-12	-102	-102	—	BACK	VERT	TOTAL	—	—
R	13-0-12	-102	-102	—	BACK	VERT	TOTAL	—	—
S	15-0-12	-102	-102	—	BACK	VERT	TOTAL	—	—
T	17-0-12	-102	-102	—	BACK	VERT	TOTAL	—	—
U	1-0-12	-55	-70	—	BACK	VERT	TOTAL	—	—
V	3-0-12	-55	-70	—	BACK	VERT	TOTAL	—	—
W	5-0-12	-55	-70	—	BACK	VERT	TOTAL	—	—
X	7-0-12	-55	-70	—	BACK	VERT	TOTAL	—	—
Y	13-0-12	-55	-70	—	BACK	VERT	TOTAL	—	—
Z	15-0-12	-55	-70	—	BACK	VERT	TOTAL	—	—
AA	17-0-12	-55	-70	—	BACK	VERT	TOTAL	—	—
AB	19-0-12	-55	-70	—	BACK	VERT	TOTAL	—	—
AC	21-0-12	-55	-70	—	BACK	VERT	TOTAL	—	—

DWG NO. TAM 7190151
STRUCTURAL
COMPONENT ONLY 2/2

JDB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300330	T9A	1	1	TRUSS DESC	

Tamarack Roof Truss, Burlington

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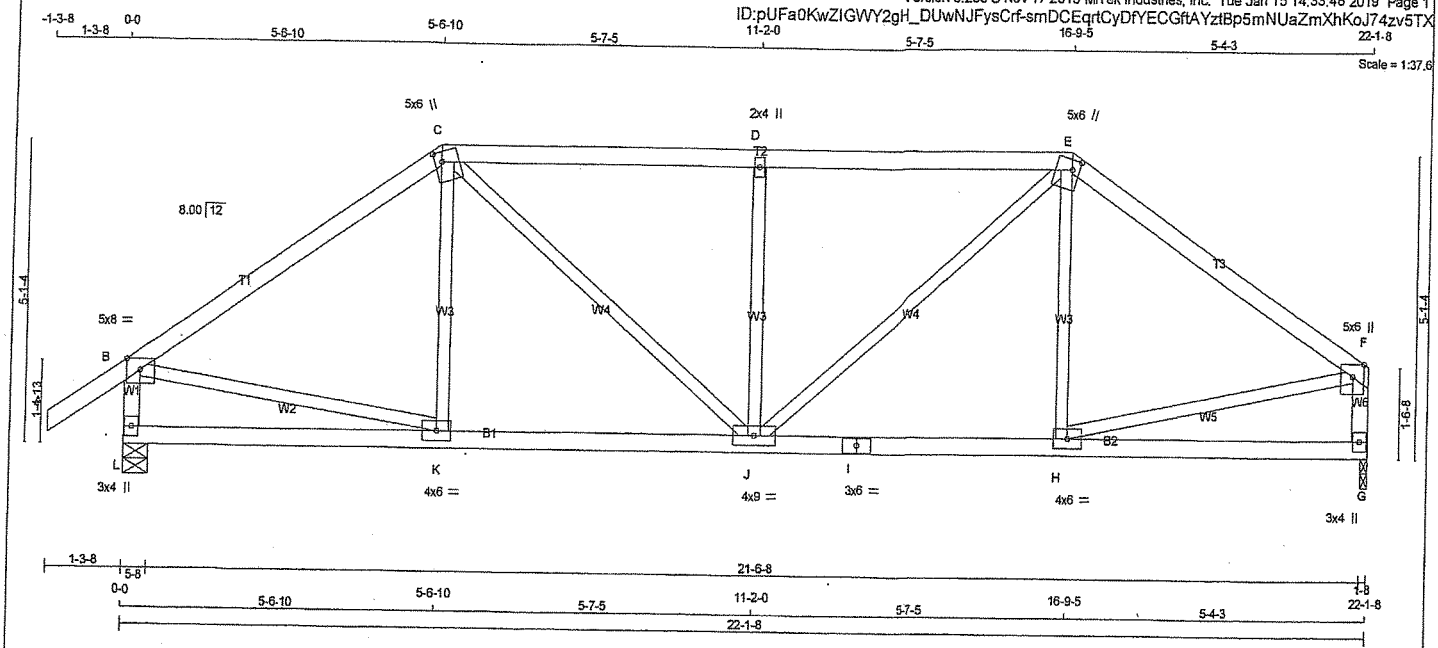

TOTAL WEIGHT = 88 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - I	2x4	ORY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	ORY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.00	1.50
F	TMVW+p	MT20	5.0	6.0	Edge	
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	9.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REORD BRG IN-SX
L	1481	0	1481	0	0	5-8	5-8
G	1365	0	1365	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1116	579 / 0	232 / 0	0 / 0	0 / 0	304 / 0	0 / 0
G	1035	515 / 0	232 / 0	0 / 0	0 / 0	288 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.52 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.11 (1)	10.00	
B- C	-1458 / 0	-84.9	-84.9	0.55 (1)	4.74	
C- D	-1649 / 0	-84.9	-84.9	0.52 (1)	4.52	
D- E	-1649 / 0	-84.9	-84.9	0.52 (1)	4.52	
E- F	-1421 / 0	-84.9	-84.9	0.50 (1)	4.85	
L- B	-1394 / 0	0.0	0.0	0.14 (1)	6.90	
G- F	-1282 / 0	0.0	0.0	0.13 (1)	7.12	
L- K	0 / 0	-38.5	-38.6	0.23 (3)	10.00	
K- J	0 / 1212	-38.5	-38.5	0.39 (2)	10.00	
J- I	0 / 1181	-38.5	-38.5	0.38 (2)	10.00	
I- H	0 / 1181	-38.5	-38.5	0.38 (2)	10.00	
H- G	0 / 0	-38.5	-38.5	0.22 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	0 / 189	0.04 (3)
C- J	0 / 576	0.13 (1)
J- D	-581 / 0	0.22 (1)
J- E	0 / 617	0.14 (1)
H- E	-23 / 165	0.04 (3)
B- K	0 / 1233	0.28 (1)
H- F	0 / 1210	0.27 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.55/1.00 (B-C:1) , BC=0.39/1.00 (J-K:2) , WB=0.28/1.00 (B-K:1) , SSI=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (F) (INPUT = 0.90)
JSI METAL= 0.57 (F) (INPUT = 1.00)

DWG NO. TAM T1901152
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300330	T10	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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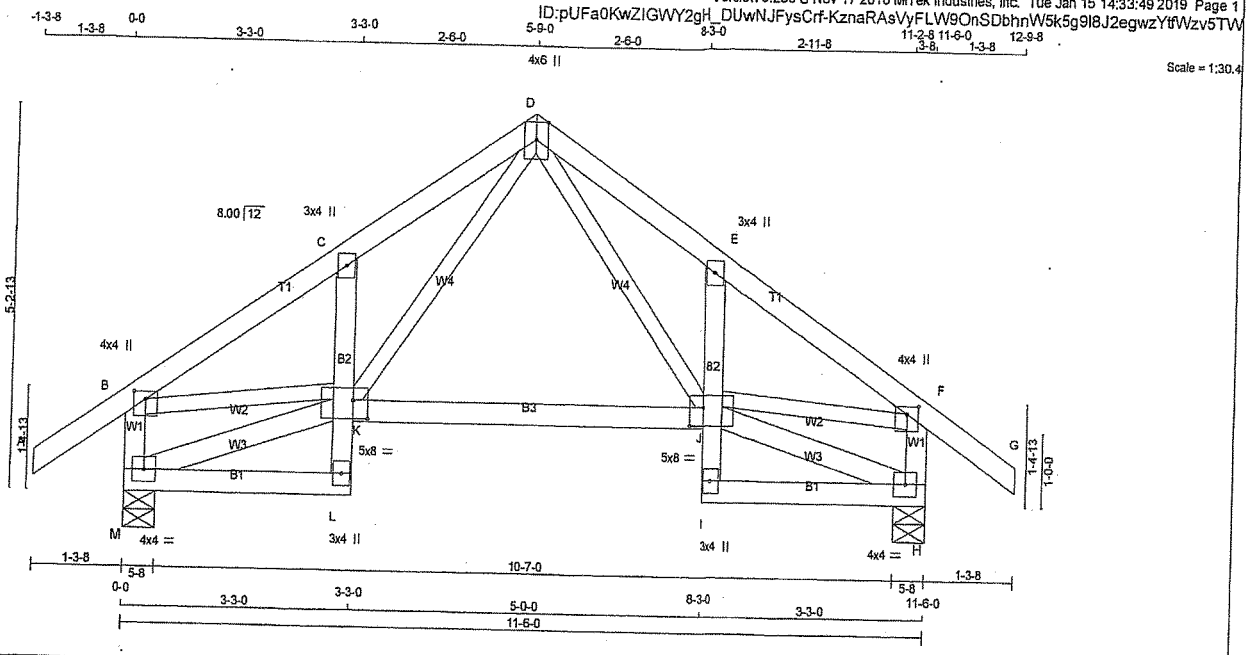

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
L - C	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
I - E	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TMV+p	MT20	3.0	4.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMV+p	MT20	3.0	4.0		
F	TMVW+p	MT20	4.0	4.0	1.25	2.00
H	BMVW1-t	MT20	4.0	4.0		
I	BMV+p	MT20	3.0	4.0		
J	BVMWWW-I	MT20	5.0	8.0	3.00	2.60
K	BVMWWW-J	MT20	5.0	8.0	3.00	2.50
L	BMV+p	MT20	3.0	4.0		
M	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	826	0	826	0	0	5-8	5-8
H	826	0	826	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	618	332 / 0	121 / 0	0 / 0	0 / 0	166 / 0	0 / 0
H	618	332 / 0	121 / 0	0 / 0	0 / 0	166 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.11 (1)		10.00
B- C	-916 / 0	-84.9	-84.9	0.09 (1)		6.25
C- D	-941 / 0	-84.9	-84.9	0.08 (1)		6.25
D- E	-941 / 0	-84.9	-84.9	0.08 (1)		6.25
E- F	-916 / 0	-84.9	-84.9	0.09 (1)		6.25
F- G	0 / 32	-84.9	-84.9	0.11 (1)		10.00
M- B	-759 / 0	0.0	0.0	0.08 (1)		7.81
H- F	-759 / 0	0.0	0.0	0.08 (1)		7.81
M- L	0 / 23	-38.5	-38.5	0.09 (3)		10.00
L- K	0 / 76	0.0	0.0	0.04 (1)		10.00
K- C	-287 / 0	0.0	0.0	0.03 (1)		7.81
K- J	0 / 487	-38.5	-38.5	0.30 (2)		10.00
I- J	0 / 76	0.0	0.0	0.04 (1)		10.00
J- E	-287 / 0	0.0	0.0	0.03 (1)		7.81
I- H	0 / 23	-38.5	-38.6	0.09 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	CSI (LC)
D- J	0 / 509	0.11 (1)
K- D	0 / 509	0.11 (1)
M- K	-24 / 0	0.00 (1)
B- K	0 / 772	0.17 (1)
J- H	-24 / 0	0.00 (1)
J- F	0 / 772	0.17 (1)

TOTAL WEIGHT = 2 X 60 = 119 lb [M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.11/1.00 (F-G:1) , BC=0.30/1.00 (J-K:2) , WB=0.17/1.00 (B-K:1) , SSI=0.11/1.00 (J-K:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (F) (INPUT = 0.90)
JSI METAL= 0.25 (B) (INPUT = 1.00)

DWG NO. TAM 190153
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300330	T11	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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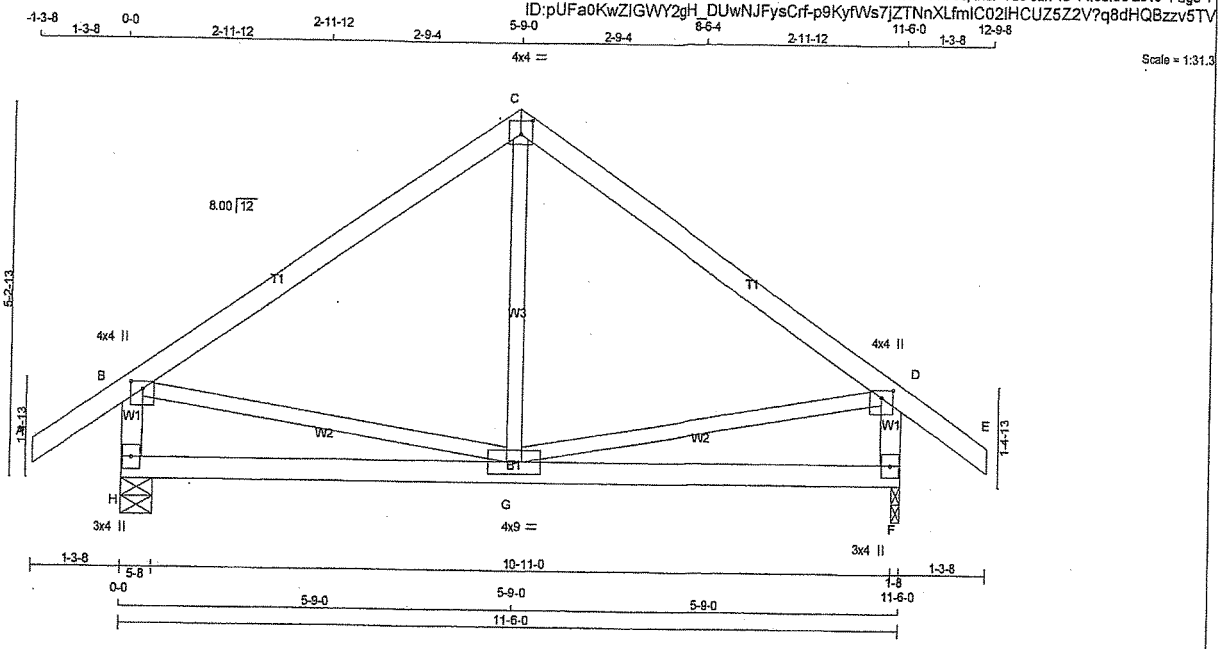


TOTAL WEIGHT = 3 X 48 = 143 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMVW+p	MT20	4.0	4.0	1.25	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	826	0	826	0	0	5-8	5-8
F	826	0	826	0	0	1-8	1-8

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	818	332 / 0	121 / 0	0 / 0	0 / 0	166 / 0	0 / 0
F	618	332 / 0	121 / 0	0 / 0	0 / 0	166 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

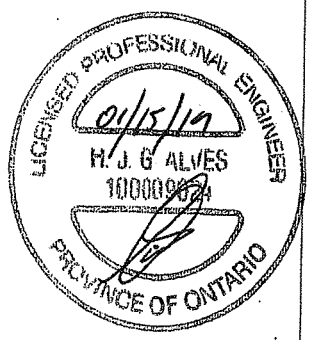
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 32	-84.9	-84.9	0.11 (1)	10.00	G-C	0 / 255	0.06 (3)
B-C	-536 / 0	-84.9	-84.9	0.36 (1)	6.25	B-G	0 / 454	0.10 (1)
C-D	-536 / 0	-84.9	-84.9	0.36 (1)	6.25	G-D	0 / 454	0.10 (1)
D-E	0 / 32	-84.9	-84.9	0.11 (1)	10.00			
H-B	-742 / 0	0.0	0.0	0.08 (1)	7.81			
F-D	-742 / 0	0.0	0.0	0.08 (1)	7.81			
H-G	0 / 0	-38.5	-38.5	0.29 (3)	10.00			
G-F	0 / 0	-38.5	-38.5	0.29 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.36/1.00 (C-D:1) , BC=0.29/1.00 (G-H:3) , WB=0.10/1.00 (B-G:1) , SSI=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.59 (D) (INPUT = 0.90)
JSI METAL= 0.17 (D) (INPUT = 1.00)

DWG NO. TAM T1901454
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300332	T12	1	2	TRUSS DESC	

Tamarack Roof Truss, Burlington

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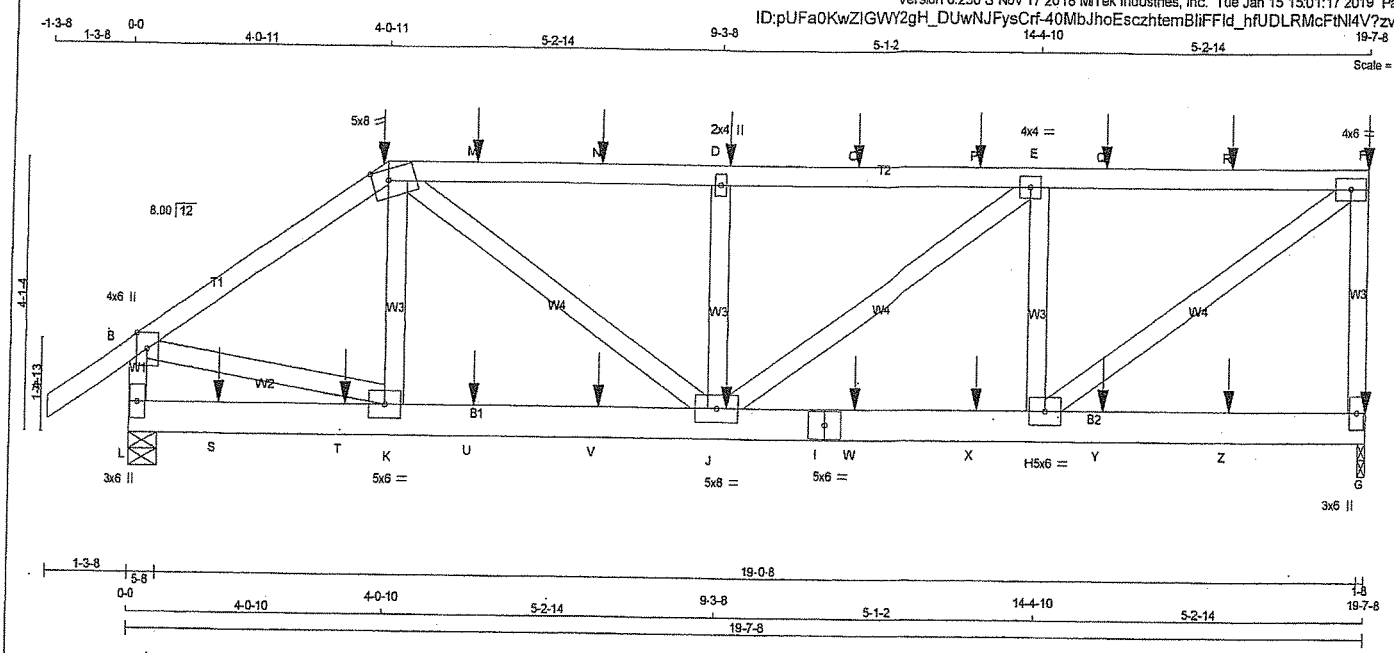


TOTAL WEIGHT = 2 X 101 = 202 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
I - G	2x6	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- F	1	12	SIDE(61.0)
F- G	1	12	TOP
L- B	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
L- I	2	12	SIDE(183.1)
I- G	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW-m	MT20	5.0	8.0	2.00	3.00
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	6.0		
H	BMWW-t	MT20	5.0	6.0		
I	BS-t	MT20	5.0	6.0		
J	BMWWW-t	MT20	5.0	8.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	2129	0	2129	0	0	1-8	1-8
L	2106	0	2106	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1620	792 / 0	374 / 0	0 / 0	0 / 0	453 / 0	0 / 0
L	1593	807 / 0	346 / 0	0 / 0	0 / 0	439 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.12 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

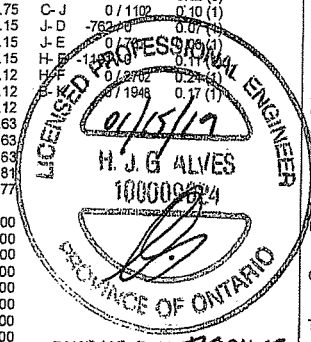
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.06 (1)	10.00	
B- C	-2282 / 0	-84.9	-84.9	0.16 (1)	5.75	
C- M	-2791 / 0	-84.9	-84.9	0.30 (1)	5.15	
M- N	-2791 / 0	-84.9	-84.9	0.30 (1)	5.15	
N- D	-2791 / 0	-84.9	-84.9	0.30 (1)	5.15	
D- O	-2791 / 0	-84.9	-84.9	0.31 (1)	5.12	
O- P	-2791 / 0	-84.9	-84.9	0.31 (1)	5.12	
P- E	-2791 / 0	-84.9	-84.9	0.31 (1)	5.12	
E- Q	-2201 / 0	-84.9	-84.9	0.30 (1)	5.63	
Q- R	-2201 / 0	-84.9	-84.9	0.30 (1)	5.63	
R- F	-2201 / 0	-84.9	-84.9	0.30 (1)	5.63	
G- F	-1939 / 0	0.0	0.0	0.24 (1)	7.81	
L- B	-2009 / 0	0.0	0.0	0.11 (1)	7.77	
L- S	0 / 0	-38.5	-38.5	0.07 (3)	10.00	
S- T	0 / 0	-38.5	-38.5	0.07 (3)	10.00	
T- K	0 / 0	-38.5	-38.5	0.07 (3)	10.00	
K- U	0 / 1892	-38.5	-38.5	0.17 (2)	10.00	
U- V	0 / 1892	-38.5	-38.5	0.17 (2)	10.00	
V- J	0 / 1892	-38.5	-38.5	0.17 (2)	10.00	
J- I	0 / 2201	-38.5	-38.5	0.20 (2)	10.00	
I- W	0 / 2201	-38.5	-38.5	0.20 (2)	10.00	
W- X	0 / 2201	-38.5	-38.5	0.20 (2)	10.00	
X- H	0 / 2201	-38.5	-38.5	0.20 (2)	10.00	
H- Y	0 / 0	-38.5	-38.5	0.08 (3)	10.00	
Y- Z	0 / 0	-38.5	-38.5	0.08 (3)	10.00	
Z- G	0 / 0	-38.5	-38.5	0.08 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-99 / 206	0.02 (3)
C- J	0 / 1102	0.10 (1)
J- D	-762 / 0	0.07 (1)
J- E	0 / 1705	0.15 (1)
H- E	-1193 / 0	0.11 (1)
H- F	0 / 2702	0.24 (1)
F- K	-97 / 1948	0.17 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	4-0-11	-50	-56	—	FRONT	VERT	DEAD	—	—
C	4-0-11	-231	-231	—	FRONT	VERT	SNOW	—	—
D	9-5-4	-102	-102	—	FRONT	VERT	TOTAL	—	—
F	19-7-8	-137	-137	—	FRONT	VERT	TOTAL	—	—
G	19-7-8	-71	-90	—	FRONT	VERT	TOTAL	—	—
J	9-5-4	-55	-70	—	FRONT	VERT	TOTAL	—	—
M	5-5-4	-102	-102	—	FRONT	VERT	TOTAL	—	—
N	7-5-4	-102	-102	—	FRONT	VERT	TOTAL	—	—
O	11-5-4	-102	-102	—	FRONT	VERT	TOTAL	—	—
P	13-5-4	-102	-102	—	FRONT	VERT	TOTAL	—	—



DWG NO. TAM T1901155
STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.31/1.00 (D-E:1) , BC=0.20/1.00 (H-J:2) , WB=0.24/1.00 (F-H:1) , SSI=0.18/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)		SHEAR (PSI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (B) (INPUT = 0.90)
JSI METAL= 0.56 (B) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300332	T12	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
Q	15-5-4	-102	-102	—	FRONT	VERT	TOTAL	—	—
R	17-5-4	-102	-102	—	FRONT	VERT	TOTAL	—	—
S	1-5-4	-55	-70	—	FRONT	VERT	TOTAL	—	—
T	3-5-4	-55	-70	—	FRONT	VERT	TOTAL	—	—
U	5-5-4	-55	-70	—	FRONT	VERT	TOTAL	—	—
V	7-5-4	-55	-70	—	FRONT	VERT	TOTAL	—	—
W	11-5-4	-55	-70	—	FRONT	VERT	TOTAL	—	—
X	13-5-4	-55	-70	—	FRONT	VERT	TOTAL	—	—
Y	15-5-4	-55	-70	—	FRONT	VERT	TOTAL	—	—
Z	17-5-4	-55	-70	—	FRONT	VERT	TOTAL	—	—

LICENSED PROFESSIONAL ENGINEER
01/15/19
H. J. G. ALVES
100093724
PROVINCE OF ONTARIO

DWG NO. TAM T1901155
STRUCTURAL
COMPONENT ONLY 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300332	T13	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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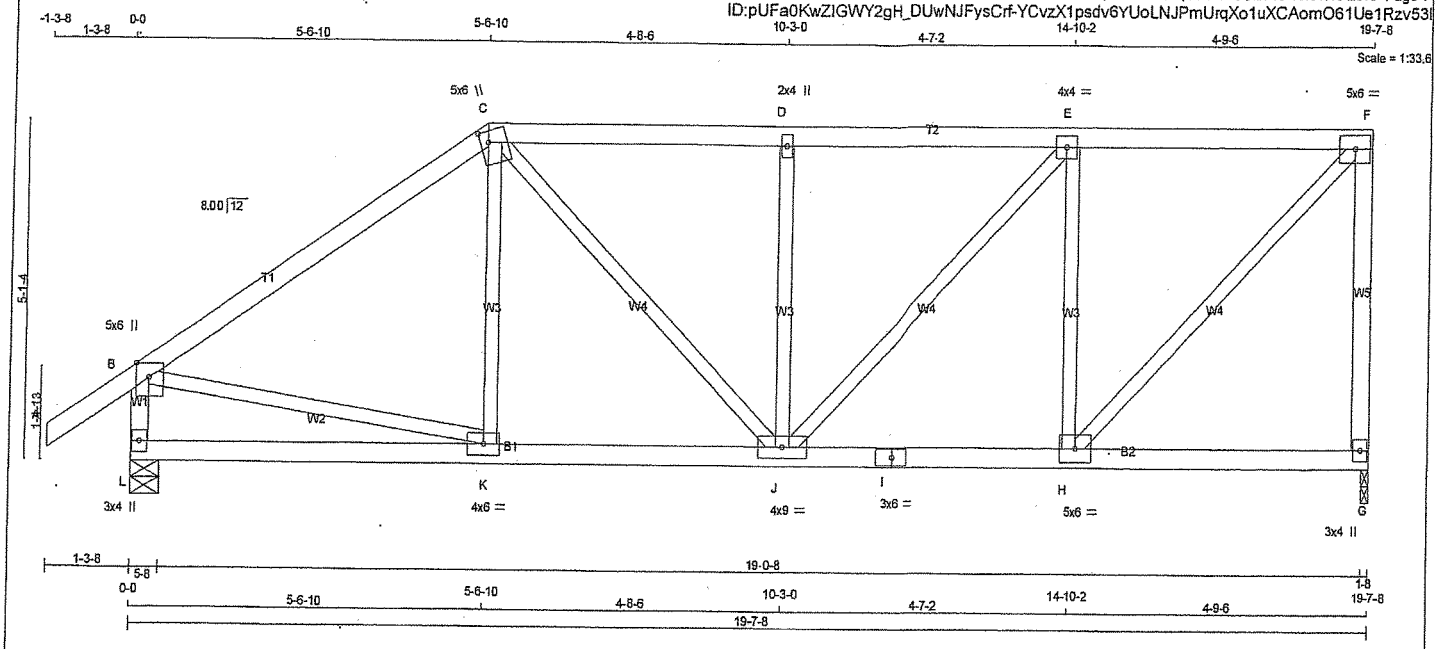


TOTAL WEIGHT = 2 X 85 = 170 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMVW-t	MT20	5.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	5.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	9.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REORD BRG IN-SX
G	1210	0	1210	0	0	1-8	1-8
L	1327	0	1327	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	918	457 / 0	206 / 0	0 / 0	0 / 0	255 / 0	0 / 0
L	999	521 / 0	206 / 0	0 / 0	0 / 0	272 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

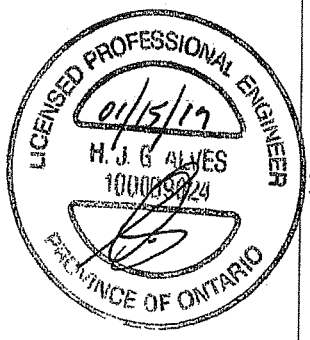
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.07 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 32	-84.9	-84.9	0.11 (1)	10.00	K- C	0 / 192	0.05 (3)
B- C	-1239 / 0	-84.9	-84.9	0.53 (1)	5.07	B- K	0 / 1049	0.24 (1)
C- D	-1275 / 0	-84.9	-84.9	0.27 (1)	5.39	H- F	0 / 1369	0.31 (1)
D- E	-1275 / 0	-84.9	-84.9	0.28 (1)	5.36	C- J	0 / 349	0.08 (1)
E- F	-965 / 0	-84.9	-84.9	0.27 (1)	5.98	H- E	-762 / 0	0.29 (1)
G- F	-1137 / 0	0.0	0.0	0.50 (1)	7.45	J- D	-424 / 0	0.16 (1)
L- B	-1238 / 0	0.0	0.0	0.13 (1)	7.22	J- E	0 / 449	0.10 (1)
L- K	0 / 0	-38.5	-38.5	0.21 (3)	10.00			
K- J	0 / 1031	-38.5	-38.5	0.35 (2)	10.00			
J- I	0 / 985	-38.5	-38.5	0.29 (2)	10.00			
I- H	0 / 965	-38.5	-38.5	0.29 (2)	10.00			
H- G	0 / 0	-38.5	-38.5	0.16 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.53/1.00 (B-C:1) , BC=0.35/1.00 (J-K:2) , WB=0.31/1.00 (F-H:1) , SSI=0.19/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (B) (INPUT = 0.90)
JSI METAL= 0.53 (B) (INPUT = 1.00)

PERMIT NO. TAM 19011156
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300332	T14	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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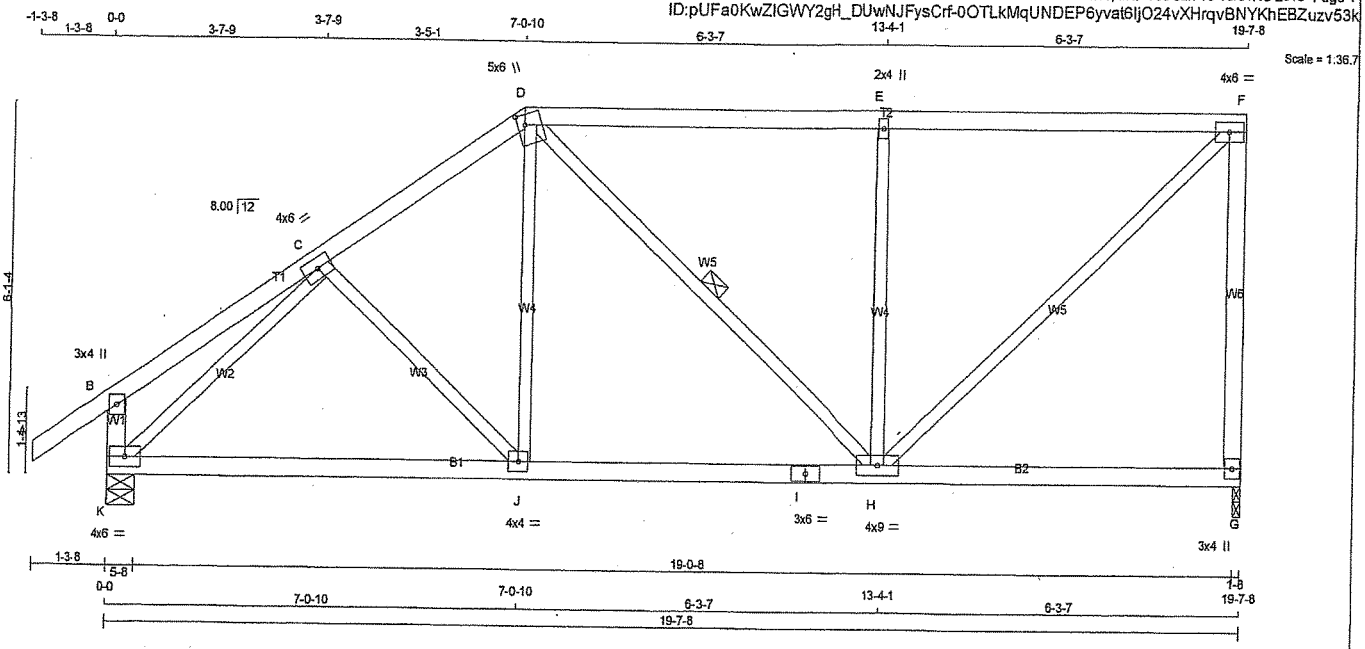


TOTAL WEIGHT = 2 X 85 = 171 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+m	MT2D	5.0	6.0	2.00	1.50
E	TMW+w	MT2D	2.0	4.0		
F	TMVW-t	MT2D	4.0	6.0		
G	BMV1+p	MT2D	3.0	4.0		
H	BMWWW-t	MT2D	4.0	9.0		
I	BS-t	MT2D	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT2D	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1210	0	1210	0	0	1-8	1-8
K	1327	0	1327	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	918	457 / 0	206 / 0	0 / 0	0 / 0	255 / 0	0 / 0
K	999	521 / 0	208 / 0	0 / 0	0 / 0	272 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.29 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-H.

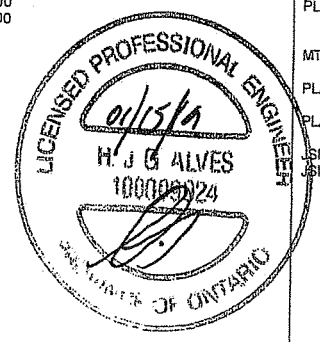
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.11 (1)	10.00
B- C	0 / 18	-84.9	-84.9	0.16 (1)	10.00
C- D	-1186 / 0	-84.9	-84.9	0.18 (1)	5.66
D- E	-985 / 0	-84.9	-84.9	0.61 (1)	5.29
E- F	-985 / 0	-84.9	-84.9	0.61 (1)	5.29
G- F	-1110 / 0	0.0	0.0	0.80 (1)	7.52
K- B	-234 / 0	0.0	0.0	0.02 (1)	7.81
K- J	0 / 1007	-38.5	-38.5	0.45 (2)	10.00
J- I	0 / 976	-38.5	-38.5	0.45 (2)	10.00
I- H	0 / 976	-38.5	-38.5	0.45 (2)	10.00
H- G	0 / 0	-38.5	-38.5	0.25 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- J	-50 / 71	0.02 (1)
J- D	0 / 364	0.08 (2)
D- H	-36 / 13	0.02 (3)
H- E	-660 / 0	0.39 (1)
H- F	0 / 1338	0.30 (1)
K- C	-1405 / 0	0.54 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.80/1.00 (F-G:1) , BC=0.45/1.00 (H-J:2) , WB=0.54/1.00 (C-K:1) , SSI=0.26/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg

SI GRIP= 0.90 (F) (INPUT = 0.90)
SI METAL= 0.34 (C) (INPUT = 1.00)

DWG NO. TAM 1190157
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300332	T15	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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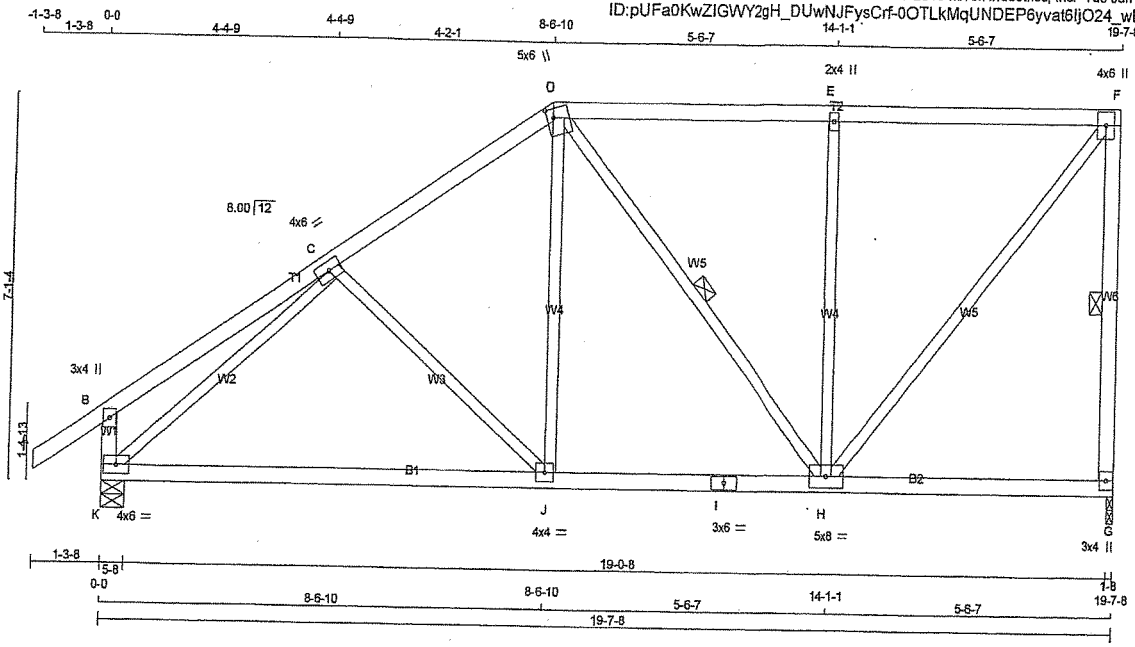

TOTAL WEIGHT = 2 X 90 = 181 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	ORY	No.2	SPF
O - F	2x4	DRY	No.2	SPF
G - F	2x4	ORY	No.2	SPF
K - B	2x4	ORY	No.2	SPF
K - I	2x4	ORY	No.2	SPF
I - G	2x4	ORY	No.2	SPF
ALL WEBS EXCEPT	2x3	ORY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
O	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT2D	2.0	4.0		
F	TMVW+p	MT2D	4.0	6.0		
G	BMV1+p	MT2D	3.0	4.0		
H	BMWWW-t	MT2D	5.0	8.0		
I	BS-t	MT2D	3.0	6.0		
J	BMWW-t	MT2D	4.0	4.0		
K	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1210	0	1210	0	0	1-8	1-8
K	1327	0	1327	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	918	457 / 0	206 / 0	0 / 0	0 / 0	255 / 0	0 / 0
K	999	521 / 0	206 / 0	0 / 0	0 / 0	272 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.70 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	1	0.11 (1)	10.00
B- C	0 / 23	-84.9	-84.9	1	0.24 (1)	10.00
C- D	-1096 / 0	-84.9	-84.9	1	0.27 (1)	5.70
D- E	-766 / 0	-84.9	-84.9	1	0.45 (1)	6.15
E- F	-766 / 0	-84.9	-84.9	1	0.46 (1)	6.15
G- F	-1116 / 0	0.0	0.0	1	0.25 (1)	6.00
K- B	-256 / 0	0.0	0.0	1	0.03 (1)	7.81
K- J	0 / 1015	-38.5	-38.5	1	0.58 (2)	10.00
J- I	0 / 899	-38.5	-38.5	1	0.57 (2)	10.00
I- H	0 / 899	-38.5	-38.5	1	0.57 (2)	10.00
H- G	0 / 0	-38.5	-38.5	1	0.21 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- J	-165 / 34	0.10 (1)
J- O	0 / 482	0.11 (2)
D- H	-218 / 0	0.11 (2)
H- E	-581 / 0	0.51 (1)
H- F	0 / 1210	0.27 (1)
K- C	-1379 / 0	0.77 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS OESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.22")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 635 (0.37")

CSI: TC=0.46/1.00 (E-F:1) , BC=0.58/1.00 (J-K:2) , WB=0.77/1.00 (C-K:1) , SSI=0.23/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

GRIP= 0.75 (F) (INPUT = 0.90)
METAL= 0.35 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
01/15/19
H J. G ALVES
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PROVINCE OF ONTARIO

DWG NO. IAM 1190 1158
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300332	T16	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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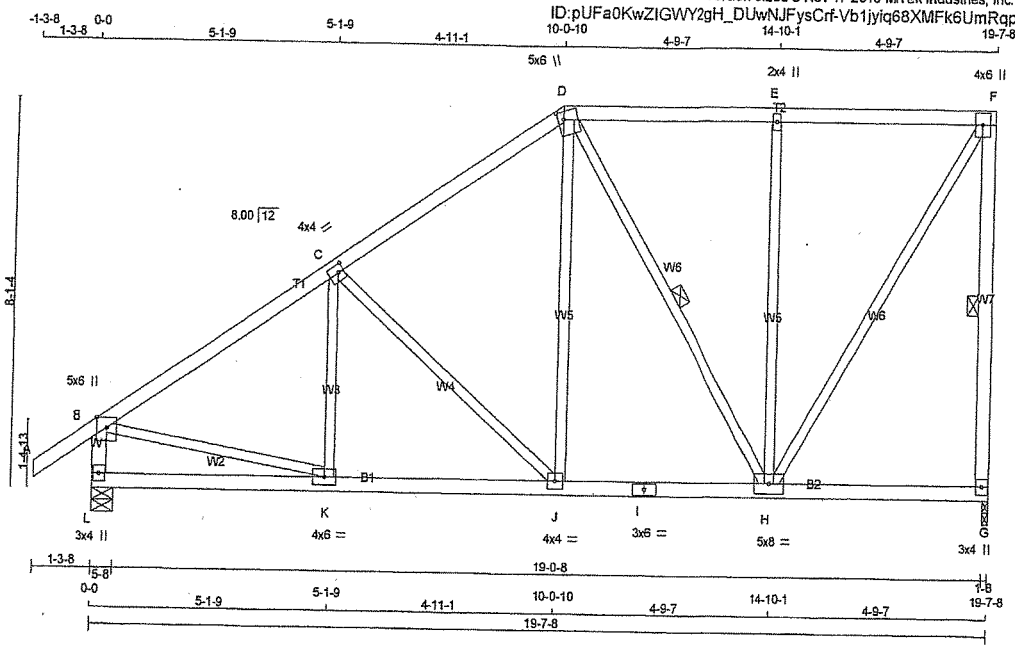


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT2D	5.0	6.0	Edge	
C	TMWW-t	MT2D	4.0	4.0	2.00	1.50
O	TTWW+m	MT2D	5.0	8.0	2.00	1.50
E	TMW+w	MT2D	2.0	4.0		
F	TMWW+p	MT2D	4.0	6.0		
G	BMV1+p	MT2D	3.0	4.0		
H	BMWWW-t	MT2D	5.0	8.0		
I	BS-t	MT2D	3.0	6.0		
J	BMWW-t	MT2D	4.0	4.0		
K	BMWW-t	MT2D	4.0	6.0		
L	BMV1+p	MT2D	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EOGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REORD BRG
G	1210	0	1210	0	0	IN-SX	IN-SX
L	1327	0	1327	0	0	1-8	1-8
						5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	918	457 / 0	206 / 0	0 / 0	0 / 0	255 / 0	0 / 0
L	999	521 / 0	206 / 0	0 / 0	0 / 0	272 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.35 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

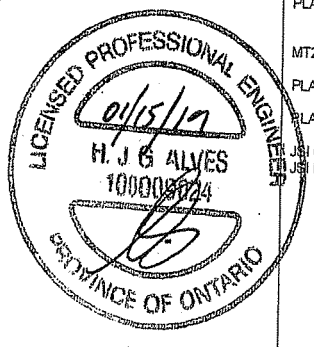
TOTAL LOAD CASES: (4)

CHORDS

MEMB FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A-B	0 / 32	-84.9	-84.9	0.11 (1)	10.00
B-C	-1291 / 0	-84.9	-84.9	0.30 (1)	5.35
C-D	-948 / 0	-84.9	-84.9	0.28 (1)	6.02
D-E	-604 / 0	-84.9	-84.9	0.25 (1)	6.25
E-F	-604 / 0	-84.9	-84.9	0.25 (1)	6.25
G-F	-1135 / 0	0.0	0.0	0.34 (1)	5.96
L-B	-1247 / 0	0.0	0.0	0.13 (1)	7.19
L-K	0 / 0	-38.5	-38.5	0.19 (3)	10.00
K-J	0 / 1097	-38.5	-38.5	0.33 (2)	10.00
J-I	0 / 770	-38.5	-38.5	0.26 (2)	10.00
I-H	0 / 770	-38.5	-38.5	0.26 (2)	10.00
H-G	0 / 0	-38.5	-38.5	0.16 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K-C	-10 / 166	0.04 (3)
C-J	-447 / 0	0.38 (1)
J-O	0 / 484	0.11 (2)
D-H	-319 / 0	0.18 (1)
H-E	-501 / 0	0.63 (1)
H-F	0 / 1149	0.28 (1)
B-K	0 / 1121	0.25 (1)



TOTAL WEIGHT = 2 X 98 = 196 lb

[M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.34/1.00 (F-G:1) , BC=0.33/1.00 (J-K:2) , WB=0.63/1.00 (E-H:1) , SSI=0.20/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1967	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (B) (INPUT = 0.90)
JSI METAL= 0.54 (B) (INPUT = 1.00)

DWG NO. TAM T1901159
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300332	T17	9	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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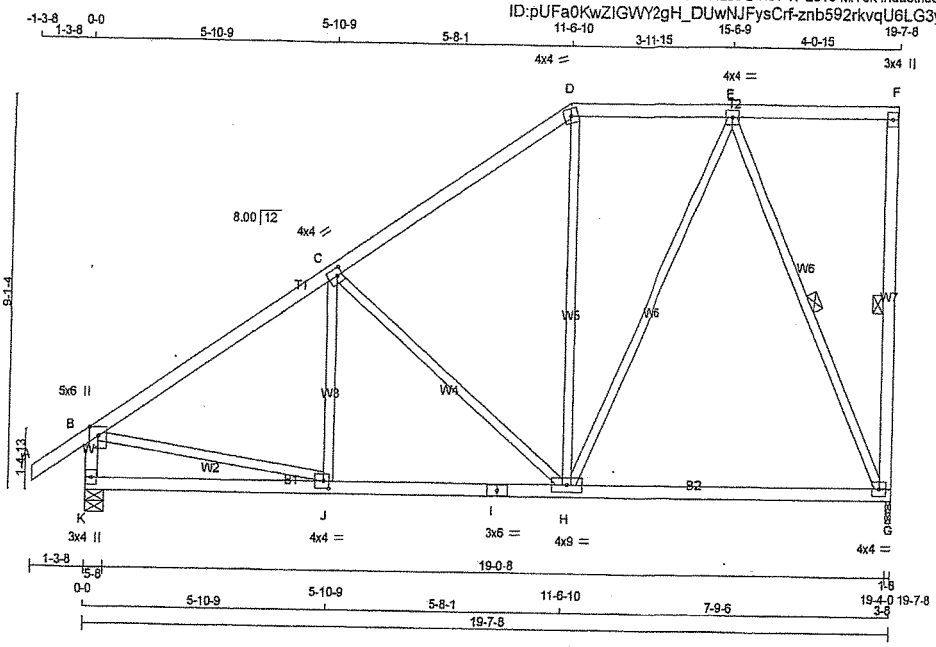


Scale = 1:51.6

TOTAL WEIGHT = 9 X 97 = 670 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0	2.00	1.50
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HDRZ	UPLIFT	INPUT BRG	REQRD BRG
G	1210	0	1210	0	0	1-8	1-8
K	1327	0	1327	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	918	457 / 0	206 / 0	0 / 0	0 / 0	255 / 0	0 / 0
K	999	521 / 0	206 / 0	0 / 0	0 / 0	272 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.24 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, E-G.

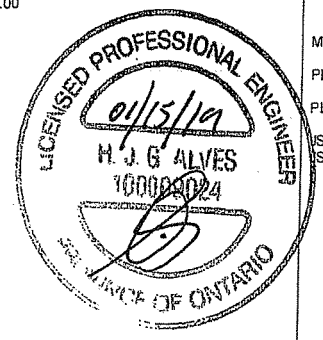
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.11 (1)	10.00
B- C	-1272 / 0	-84.9	-84.9	0.40 (1)	5.24
C- D	-843 / 0	-84.9	-84.9	0.38 (1)	6.14
D- E	-674 / 0	-84.9	-84.9	0.18 (1)	6.25
E- F	0 / 0	-84.9	-84.9	0.23 (1)	10.00
G- F	-132 / 0	0.0	0.0	0.05 (1)	6.25
K- B	-1230 / 0	0.0	0.0	0.13 (1)	7.23
K- J	0 / 0	-38.5	-38.5	0.22 (3)	10.00
J- I	0 / 1085	-38.5	-38.5	0.54 (2)	10.00
I- H	0 / 1085	-38.5	-38.5	0.54 (2)	10.00
H- G	0 / 441	-38.5	-38.5	0.44 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- C	0 / 180	0.04 (3)
C- H	-545 / 0	0.64 (1)
H- D	0 / 189	0.04 (3)
B- J	0 / 1103	0.25 (1)
H- E	0 / 579	0.13 (1)
E- G	-1050 / 0	0.70 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 893 (0.26")

CSI: TC=0.40/1.00 (B-C:1) , BC=0.54/1.00 (H-J:2) , WB=0.70/1.00 (E-G:1) , SSI=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.90 (J) (INPUT = 0.90)
SI METAL= 0.54 (B) (INPUT = 1.00)

DWG NO. TAM T1901160
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300332	T18	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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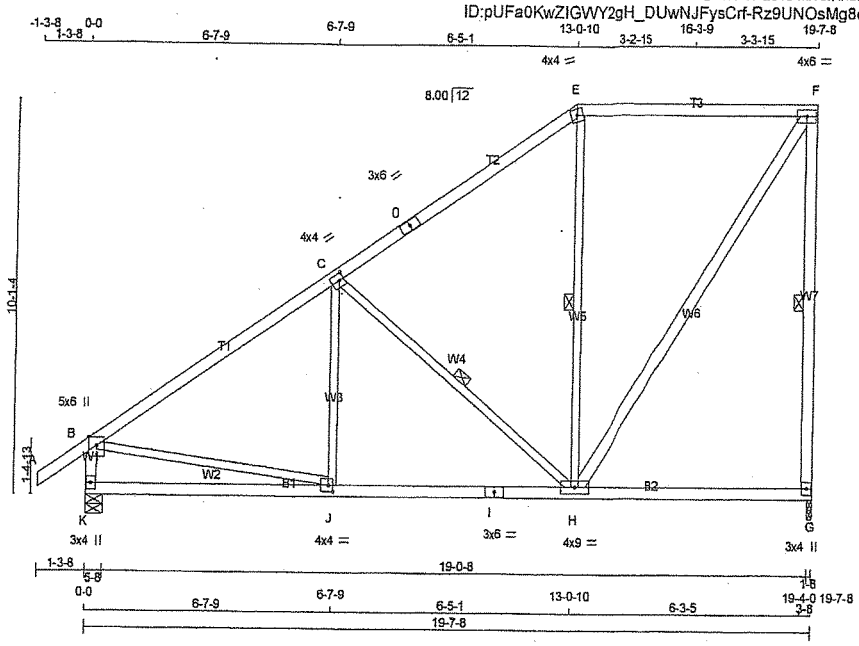


TOTAL WEIGHT = 2 X 98 = 197 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTW-m	MT20	4.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0	2.00	1.50
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1210	0	1210	0	0	1-8	1-8
K	1327	0	1327	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	918	457 / 0	208 / 0	0 / 0	0 / 0	255 / 0	0 / 0
K	999	521 / 0	206 / 0	0 / 0	0 / 0	272 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.08 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

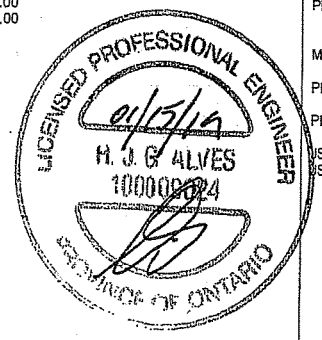
1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, C-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.9	-84.9	0.11 (1)	10.00
B-C	-1255 / 0	-84.9	-84.9	0.53 (1)	5.06
C-D	-700 / 0	-84.9	-84.9	0.48 (1)	6.25
D-E	-700 / 0	-84.9	-84.9	0.48 (1)	8.25
E-F	-549 / 0	-84.9	-84.9	0.47 (1)	6.25
G-F	-1110 / 0	0.0	0.0	0.55 (1)	6.01
K-B	-1223 / 0	0.0	0.0	0.13 (1)	7.25
K-J	0 / 0	-38.5	-38.5	0.30 (3)	10.00
J-I	0 / 1075	-38.5	-38.5	0.43 (2)	10.00
I-H	0 / 1075	-38.5	-38.5	0.43 (2)	10.00
H-G	0 / 0	-38.5	-38.5	0.29 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
J-C	0 / 264	0.06 (3)
C-H	-687 / 0	0.34 (1)
H-E	-115 / 112	0.08 (1)
B-J	0 / 1090	0.25 (1)
H-F	0 / 997	0.16 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = **24.0** IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.55/1.00 (F-G:1) , BC=0.43/1.00 (H-J:2) , WB=0.34/1.00 (C-H:1) , SSI=0.23/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SSI GRIP= 0.87 (J) (INPUT = 0.90)
SSI METAL= 0.54 (B) (INPUT = 1.00)

DWG NO. TAM 1901161
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300332	T19	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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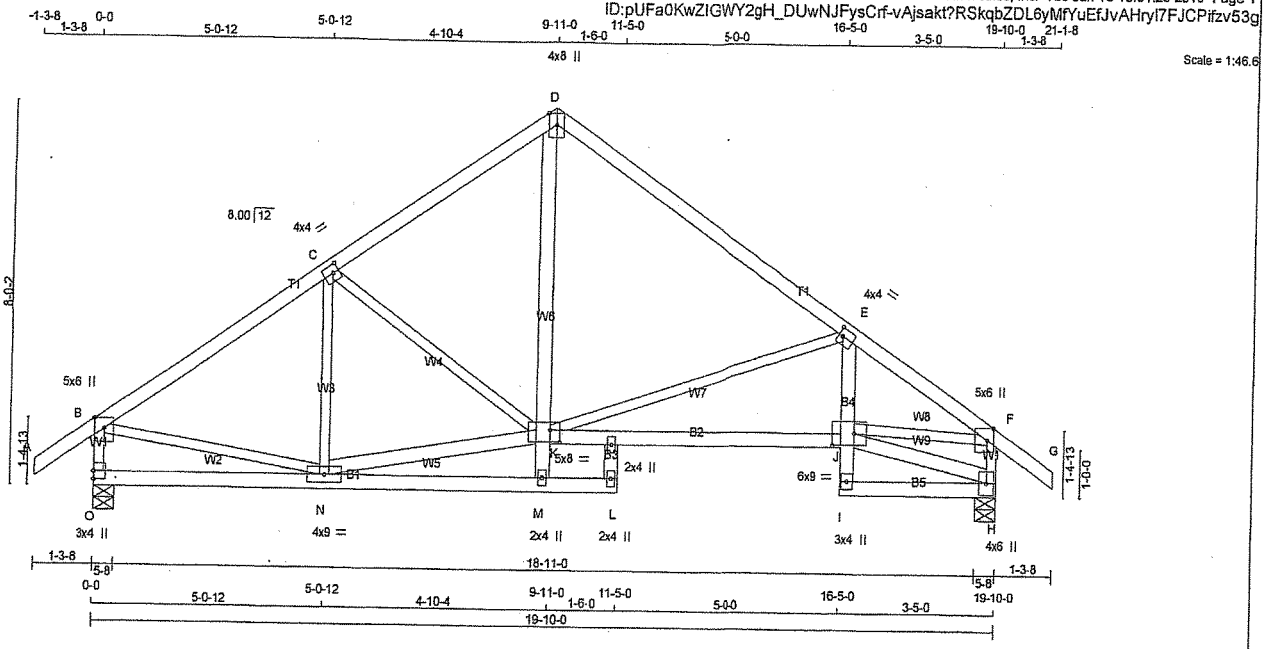

TOTAL WEIGHT = 98 lb [M][F]

LUMBER

N.L.G.A. RULES

CHOROS	SIZE		LUMBER	DESCR.
A - D	2x4	ORY	No.2	SPF
D - G	2x4	ORY	No.2	SPF
O - B	2x3	ORY	No.2	SPF
H - F	2x3	DRY	No.2	SPF
O - L	2x4	ORY	No.2	SPF
K - J	2x4	ORY	No.2	SPF
I - E	2x4	DRY	No.2	SPF
I - H	2x4	ORY	No.2	SPF
ALL WEBS EXCEPT	2x3	ORY	No.2	SPF
M - D	2x4	ORY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
N - K	2x4	ORY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT2D	5.0	6.0	Edge	
C	TMWW-t	MT2D	4.0	4.0	2.00	1.50
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMVW-t	MT2D	4.0	4.0	2.00	1.00
F	TMVW+p	MT20	5.0	6.0	Edge	1.75
H	BMVW1+p	MT20	4.0	6.0		
I	BMV+p	MT20	3.0	4.0		
J	BVMWW-I	MT20	6.0	9.0	3.00	5.75
K	BWMWWW"-I	MT20	5.0	8.0	3.00	2.50
L	NP+w	MT20	2.0	4.0		
M	BMW+w	MT2D	2.0	4.0		
N	BMWWW-t	MT20	4.0	9.0		
O	BMV1+p	MT20	3.0	4.0		
P	NP+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED. GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REORD BRG IN-SX
O	1370	0	1370	0	0	5-8	5-8
H	1369	0	1369	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1036	526 / 0	225 / 0	0 / 0	0 / 0	285 / 0	0 / 0
H	1035	526 / 0	224 / 0	0 / 0	0 / 0	285 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.46 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 10.00 FT

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.9	-84.9	0.11 (1)		10.00
B-C	-1353 / 0	-84.9	-84.9	0.29 (1)		5.27
C-D	-1171 / 0	-84.9	-84.9	0.28 (1)		5.57
D-E	-1166 / 0	-84.9	-84.9	0.49 (1)		5.27
E-F	-1918 / 0	-84.9	-84.9	0.38 (1)		4.46
F-G	0 / 32	-84.9	-84.9	0.11 (1)		10.00
O-B	-1293 / 0	0.0	0.0	0.19 (1)		6.22
H-F	-1291 / 0	0.0	0.0	0.19 (1)		6.22
O-N	0 / 0	-38.5	-38.5	0.20 (3)		10.00
N-M	0 / 35	-38.5	-38.5	0.21 (3)		10.00
M-L	0 / 0	-38.5	-38.5	0.10 (3)		10.00
K-J	0 / 1663	-38.5	-38.5	0.61 (2)		10.00
I-J	0 / 80	0.0	0.0	0.08 (1)		10.00
J-E	0 / 216	0.0	0.0	0.09 (2)		10.00
I-H	0 / 50	-38.5	-38.5	0.10 (3)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N-C	-247 / 41	0.08 (1)
M-K	0 / 170	0.05 (2)
K-D	0 / 877	0.17 (1)
B-N	0 / 1174	0.26 (1)
K-E	-756 / 0	0.74 (1)
J-H	-52 / 0	0.01 (1)
J-F	0 / 1642	0.37 (1)
N-K	0 / 1139	0.18 (1)
C-K	-244 / 0	0.14 (1)

LICENSED PROFESSIONAL ENGINEER
01/15/19
H. J. G. ALVES
100090924
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	48.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 875 (0.27")

CSI: TC=0.49/1.00 (D-E:1) , BC=0.61/1.00 (J-K:2) , WB=0.74/1.00 (E-K:1) , SSI=0.21/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.87 (F) (INPUT = 0.90)
SI METAL= 0.76 (F) (INPUT = 1.00)

DWG NO. TAM T1901162
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300334	T19A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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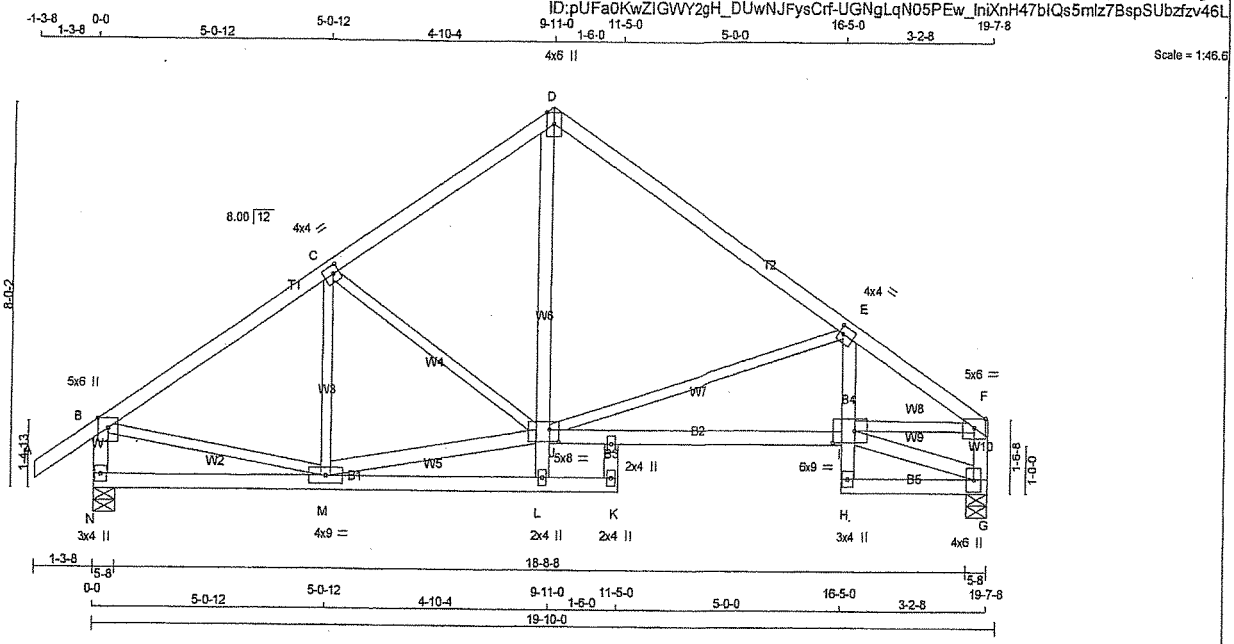


TOTAL WEIGHT = 96 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
L - D	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMVW-t	MT20	4.0	4.0	2.00	1.00
F	TMVW-p	MT20	5.0	6.0	Edge	
G	BMVW1+p	MT20	4.0	6.0		
H	BMV+p	MT20	3.0	4.0		
I	BVMWW-t	MT20	6.0	9.0	3.00	5.75
J	BWMWWW*-t	MT20	5.0	8.0	3.00	2.50
K	NP+w	MT20	2.0	4.0		
L	BMW+w	MT20	2.0	4.0		
M	BMWWW-t	MT20	4.0	9.0		
N	BMV1+p	MT20	3.0	4.0		
O	NP+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
N	1357	0	1357	0	0	5-8	5-8
G	1240	0	1240	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1026	521 / 0	222 / 0	0 / 0	0 / 0	282 / 0	0 / 0
G	945	457 / 0	222 / 0	0 / 0	0 / 0	266 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.57 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 10.00 FT

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.9	-84.9	0.11 (1)		10.00
B-C	-1335 / 0	-84.9	-84.9	0.29 (1)		5.29
C-D	-1147 / 0	-84.9	-84.9	0.28 (1)		5.62
D-E	-1143 / 0	-84.9	-84.9	0.49 (1)		5.31
E-F	-1802 / 0	-84.9	-84.9	0.38 (1)		4.57
N-B	-1280 / 0	0.0	0.0	0.13 (1)		7.13
G-F	-1167 / 0	0.0	0.0	0.12 (1)		7.38
N-M	0 / 0	-38.6	-38.5	0.20 (3)		10.00
M-L	0 / 35	-38.5	-38.5	0.20 (3)		10.00
L-K	0 / 0	-38.5	-38.5	0.10 (3)		10.00
J-I	0 / 1568	-38.5	-38.5	0.60 (2)		10.00
H-I	0 / 75	0.0	0.0	0.07 (1)		10.00
I-E	0 / 176	0.0	0.0	0.07 (2)		10.00
H-G	0 / 44	-38.5	-38.5	0.09 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M-C	-240 / 44	0.08 (1)
L-J	0 / 170	0.06 (2)
J-D	0 / 852	0.16 (1)
B-M	0 / 1159	0.26 (1)
J-E	-675 / 0	0.66 (1)
I-G	-46 / 0	0.01 (1)
I-F	0 / 1553	0.35 (1)
M-J	0 / 1124	0.18 (1)
C-J	-249 / 0	0.14 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 887 (0.27")

CSI: TC=0.49/1.00 (D-E:1) , BC=0.60/1.00 (I-J:2) , WB=0.66/1.00 (E-J:1) , SSI=0.21/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

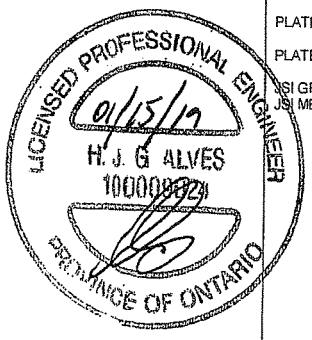
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SSI GRIP= 0.85 (B) (INPUT = 0.90)
SSI METAL= 0.56 (B) (INPUT = 1.00)



DWG NO. TAM 7901163
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300332	T20	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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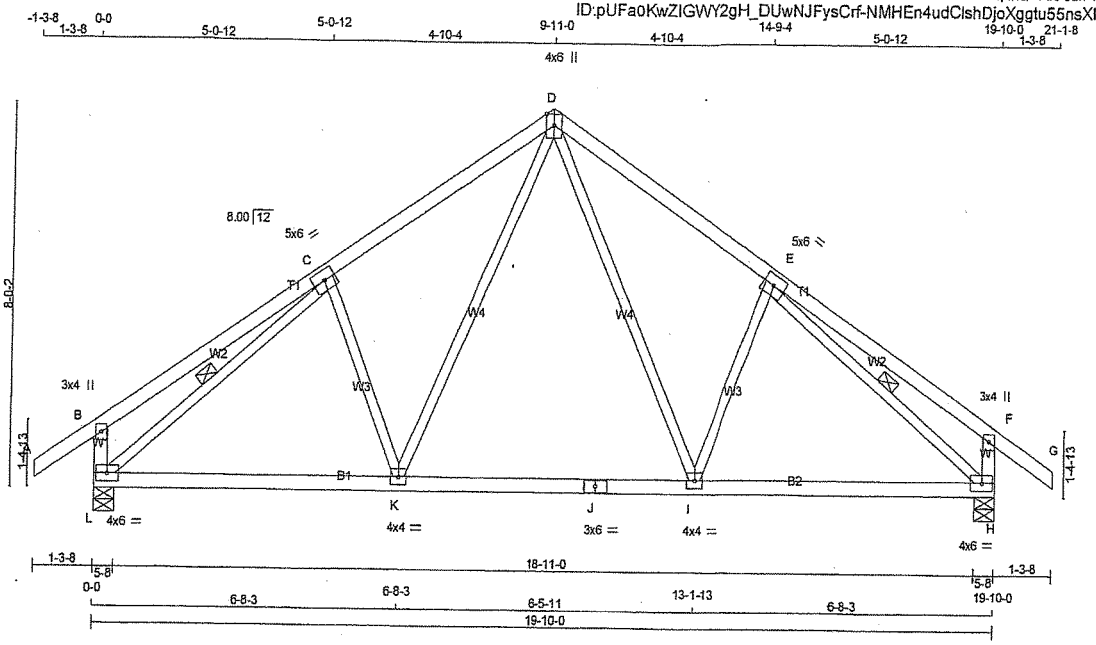


TOTAL WEIGHT = 86 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWV+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	5.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMVW1-t	MT20	4.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1340	0	1340	0	0	5-8	5-8
H	1340	0	1340	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1008	526 / 0	208 / 0	0 / 0	0 / 0	274 / 0	0 / 0
H	1008	526 / 0	208 / 0	0 / 0	0 / 0	274 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.49 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-L, E-H.

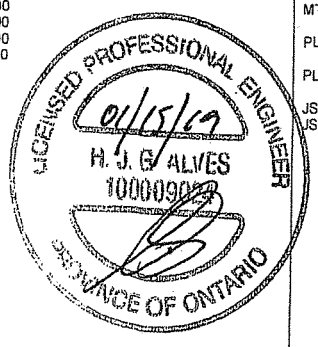
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.9	-84.9	0.11 (1)	10.00	
B-C	0 / 27	-84.9	-84.9	0.34 (1)	10.00	
C-D	-1221 / 0	-84.9	-84.9	0.28 (1)	5.49	
D-E	-1221 / 0	-84.9	-84.9	0.28 (1)	5.49	
E-F	0 / 27	-84.9	-84.9	0.34 (1)	10.00	
F-G	0 / 32	-84.9	-84.9	0.11 (1)	10.00	
L-B	-277 / 0	0.0	0.0	0.03 (1)	7.81	
H-F	-277 / 0	0.0	0.0	0.03 (1)	7.81	
L-K	0 / 1082	-38.5	-38.5	0.43 (2)	10.00	
K-J	0 / 783	-38.5	-38.5	0.39 (2)	10.00	
J-I	0 / 783	-38.5	-38.5	0.39 (2)	10.00	
I-H	0 / 1082	-38.5	-38.5	0.43 (2)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D-I	0 / 542	0.12 (1)
I-E	-240 / 23	0.09 (1)
K-D	0 / 542	0.12 (1)
C-K	-240 / 23	0.09 (1)
L-C	-1446 / 0	0.43 (1)
E-H	-1446 / 0	0.43 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.34/1.00 (E-F:1) , BC=0.43/1.00 (K-L:2) , WB=0.43/1.00 (E-H:1) , SSI=0.17/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (H) (INPUT = 0.90)
JSI METAL= 0.37 (C) (INPUT = 1.00)

DWG NO. TAM T1901164
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300332	T20A	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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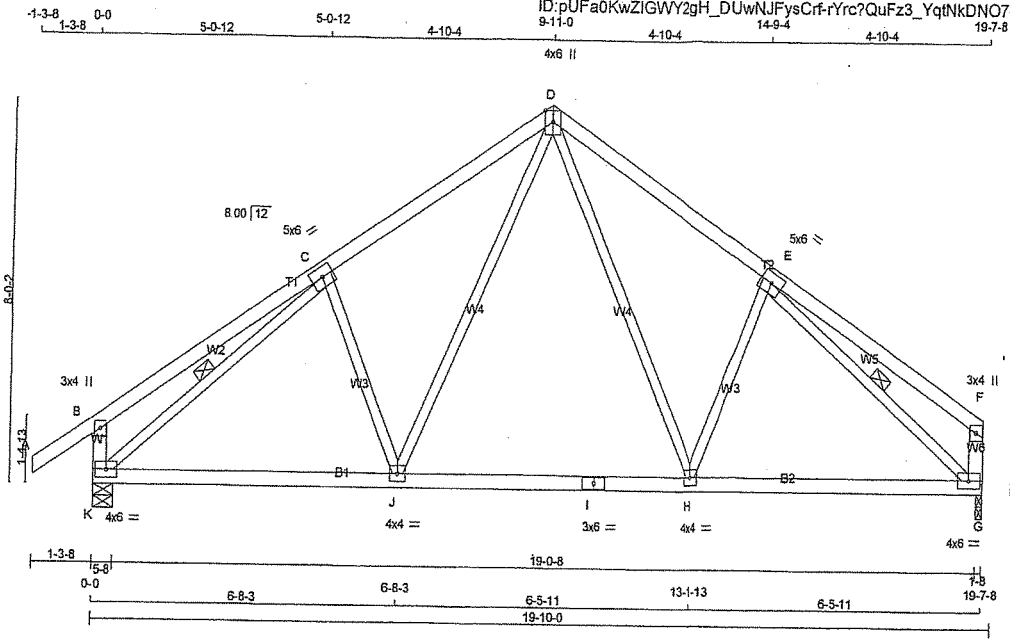

TOTAL WEIGHT = 4 X 84 = 337 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	ORY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
O	TTWW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	5.0	6.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	6.0		
H	BMWW-t	MT20	4.0	4.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
K	1327	0	1327	0	0	5-8	5-8
G	1210	0	1210	0	0	1-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	999	521 / 0	206 / 0	0 / 0	0 / 0	272 / 0	0 / 0
G	918	457 / 0	206 / 0	0 / 0	0 / 0	255 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.52 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-K, E-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.9	-84.9	0.11 (1)		10.00
B-C	0 / 27	-84.9	-84.9	0.34 (1)		10.00
C-D	-1203 / 0	-84.9	-84.9	0.28 (1)		5.52
D-E	-1180 / 0	-84.9	-84.9	0.27 (1)		5.57
E-F	0 / 27	-84.9	-84.9	0.32 (1)		10.00
K-B	-277 / 0	0.0	0.0	0.03 (1)		7.81
G-F	-151 / 0	0.0	0.0	0.02 (1)		7.81
K-J	0 / 1068	-38.5	-38.5	0.43 (2)		10.00
J-I	0 / 766	-38.5	-38.5	0.39 (2)		10.00
I-H	0 / 766	-38.5	-38.5	0.39 (2)		10.00
H-G	0 / 1036	-38.5	-38.5	0.41 (2)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D-H	0 / 500	0.11 (1)
H-E	-205 / 38	0.08 (1)
J-D	0 / 546	0.12 (1)
C-J	-242 / 22	0.09 (1)
K-C	-1427 / 0	0.42 (1)
E-G	-1411 / 0	0.40 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.34/1.00 (B-C:1) , BC=0.43/1.00 (J-K:2) , WB=0.42/1.00 (C-K:1) , SSI=0.17/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TDL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

USI GRIP= 0.77 (K) (INPUT = 0.90)
USI METAL= 0.37 (C) (INPUT = 1.00)

DWG NO. TAM T1901165
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300332	T21	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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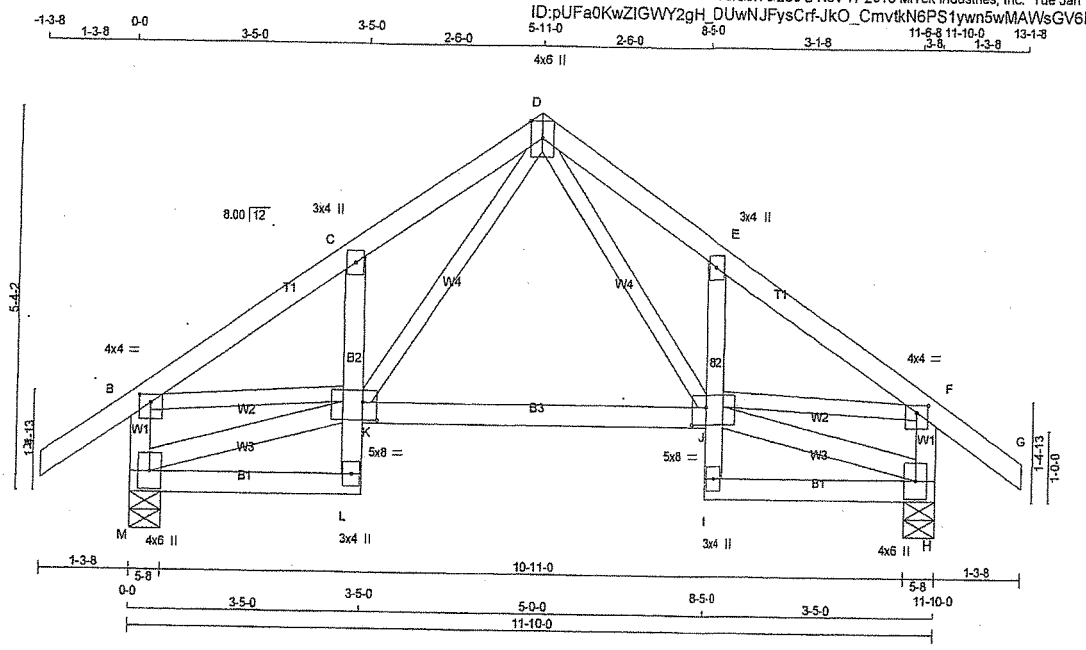

TOTAL WEIGHT = 2 X 62 = 123 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
L - C	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
I - E	2x4	DRY	No.2	SPF
I - H	2x4	ORY	No.2	SPF
ALL WEBS EXCEPT	2x3	ORY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
J - H	2x4	ORY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	4.0	1.25	2.00
C	TMV+p	MT20	3.0	4.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMV+p	MT20	3.0	4.0		
F	TMVW-p	MT20	4.0	4.0	1.25	2.00
H	BMVW1+p	MT20	4.0	6.0		
I	BMV+p	MT20	3.0	4.0		
J	BVMWWW-I	MT20	5.0	8.0	3.00	2.50
K	BVMWWW-I	MT20	5.0	8.0	3.00	2.50
L	BMV+p	MT20	3.0	4.0		
M	BMVW1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
M	846	0	846	0	0	5-8	5-8
H	846	0	846	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	634	339 / 0	124 / 0	0 / 0	0 / 0	170 / 0	0 / 0
H	634	339 / 0	124 / 0	0 / 0	0 / 0	170 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT DR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO							FR-TO		
A- B	0 / 32	-84.9	-84.9	1	0.11 (1)	10.00	D- J	0 / 522	0.12 (1)
B- C	-938 / 0	-84.9	-84.9	1	0.11 (1)	6.25	K- D	0 / 522	0.12 (1)
C- D	-963 / 0	-84.9	-84.9	1	0.08 (1)	6.25	M- K	-24 / 0	0.00 (1)
D- E	-963 / 0	-84.9	-84.9	1	0.08 (1)	6.25	J- H	-24 / 0	0.00 (1)
E- F	-938 / 0	-84.9	-84.9	1	0.11 (1)	6.25	B- K	0 / 791	0.18 (1)
F- G	0 / 32	-84.9	-84.9	1	0.11 (1)	10.00	J- F	0 / 791	0.18 (1)
M- B	-776 / 0	0.0	0.0	1	0.08 (1)	7.81			
H- F	-776 / 0	0.0	0.0	1	0.08 (1)	7.81			
M- L	0 / 23	-38.5	-38.5	1	0.10 (3)	10.00			
L- K	0 / 80	0.0	0.0	1	0.04 (1)	10.00			
K- C	-298 / 0	0.0	0.0	1	0.03 (1)	7.81			
K- J	0 / 503	-38.5	-38.5	1	0.30 (2)	10.00			
I- J	0 / 80	0.0	0.0	1	0.04 (1)	10.00			
J- E	-298 / 0	0.0	0.0	1	0.03 (1)	7.81			
I- H	0 / 23	-38.5	-38.5	1	0.10 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.39")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.39")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.11/1.00 (A-B:1) , BC=0.30/1.00 (J-K:2) , WB=0.18/1.00 (B-K:1) , SSI=0.11/1.00 (J-K:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1867	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SSI GRIP= 0.83 (F) (INPUT = 0.90)
SSI METAL= 0.29 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
01/15/19
H. J. G. ALVES
100089024
PROVINCE OF ONTARIO

DWG NO. IAM T1901166
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300334	G29	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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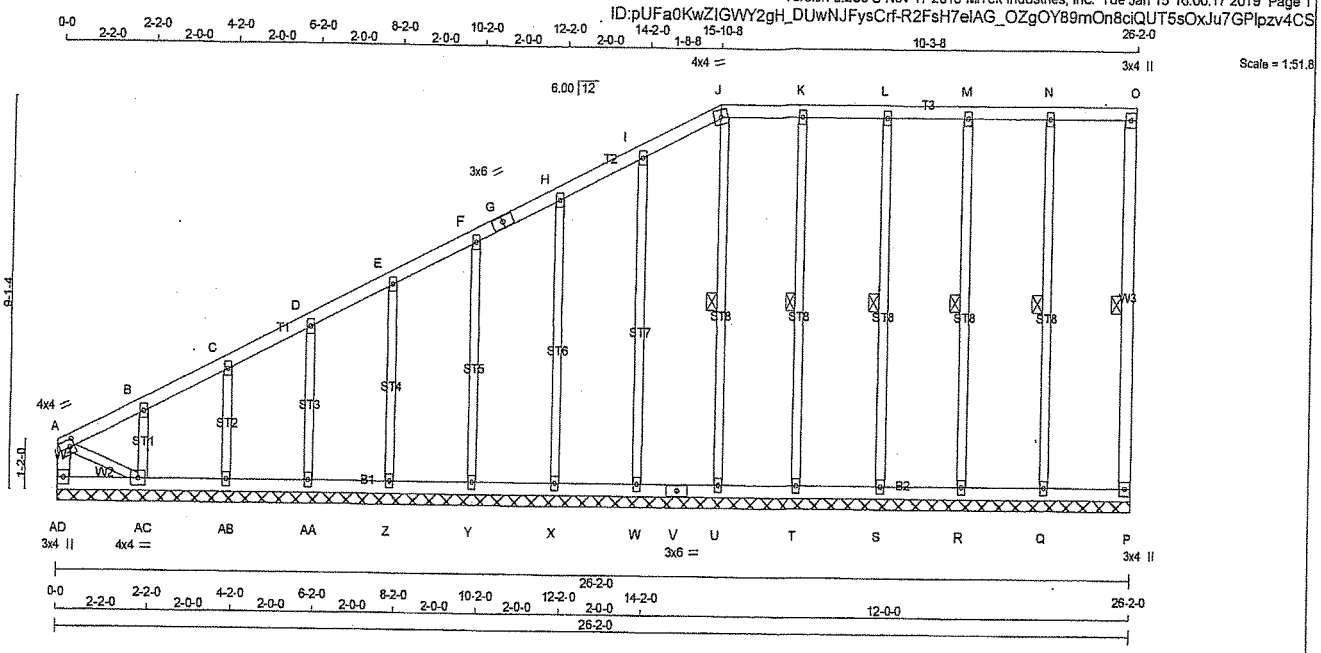

TOTAL WEIGHT = 2 X 135 = 270 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
J - O	2x4	DRY	No.2	SPF
P - O	2x4	DRY	No.2	SPF
AD- A	2x4	DRY	No.2	SPF
AD- V	2x4	DRY	No.2	SPF
V - P	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMW-t	MT20	4.0	4.0	2.00	1.25
B, C, D, E, F, H, I, K, L, M, N						
B	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
J	TTW-m	MT20	4.0	4.0		
O	TMV+p	MT20	3.0	4.0		
P	BMV1+p	MT20	3.0	4.0		
Q, R, S, T, U, W, X, Y, Z, AA, AB						
Q	BMW1+w	MT20	2.0	4.0		
V	BS-t	MT20	3.0	6.0		
AC	BMWW1-t	MT20	4.0	4.0		
AD	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF O-P, N-Q, M-R, L-S, K-T, J-U.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-17 / 0	-84.9	-84.9	0.05 (1)		6.25
B- C	-19 / 0	-84.9	-84.9	0.05 (1)		6.25
C- D	-13 / 0	-84.9	-84.9	0.04 (1)		6.25
D- E	-10 / 0	-84.9	-84.9	0.04 (1)		6.25
E- F	-7 / 0	-84.9	-84.9	0.04 (1)		10.00
F- G	-5 / 0	-84.9	-84.9	0.04 (1)		10.00
G- H	-5 / 0	-84.9	-84.9	0.04 (1)		10.00
H- I	-3 / 0	-84.9	-84.9	0.04 (1)		10.00
I- J	-9 / 0	-84.9	-84.9	0.04 (1)		10.00
J- K	0 / 0	-84.9	-84.9	0.05 (1)		10.00
K- L	0 / 0	-84.9	-84.9	0.05 (1)		10.00
L- M	0 / 0	-84.9	-84.9	0.04 (1)		10.00
M- N	0 / 0	-84.9	-84.9	0.05 (1)		10.00
N- O	0 / 0	-84.9	-84.9	0.05 (1)		10.00
P- O	-75 / 0	0.0	0.0	0.03 (1)		6.25
AD- A	-88 / 0	0.0	0.0	0.01 (1)		7.B1
AD-AC	0 / 0	-38.5	-38.5	0.03 (3)		10.00
AC-AB	0 / 16	-38.5	-38.5	0.03 (3)		10.00
AB-AA	0 / 12	-38.5	-38.5	0.02 (3)		10.00
AA- Z	0 / 9	-38.5	-38.5	0.02 (3)		10.00
Z- Y	0 / 7	-38.5	-38.5	0.02 (3)		10.00
Y- X	0 / 5	-38.5	-38.5	0.02 (3)		10.00
X- W	0 / 3	-38.5	-38.5	0.02 (3)		10.00
W- V	0 / 2	-38.5	-38.5	0.02 (3)		10.00
V- U	0 / 2	-38.5	-38.5	0.02 (3)		10.00
U- T	0 / 0	-38.5	-38.5	0.02 (3)		10.00
T- S	0 / 0	-38.5	-38.5	0.02 (3)		10.00
S- R	0 / 0	-38.5	-38.5	0.02 (3)		10.00
R- O	0 / 0	-38.5	-38.5	0.03 (3)		10.00
O- P	0 / 0	-38.5	-38.5	0.03 (3)		10.00

WEBS MEMB. FR-TO	MAX. FORCE (LBS)	MAX CSI (LC)
O- N	-195 / 0	0.11 (1)
R- M	-166 / 0	0.09 (1)
S- L	-168 / 0	0.09 (1)
T- K	-197 / 0	0.11 (1)
U- J	-136 / 0	0.07 (1)
W- I	-184 / 0	0.24 (1)
X- H	-167 / 0	0.15 (1)
Y- F	-169 / 0	0.10 (1)
Z- E	-168 / 0	0.07 (1)
AA- D	-169 / 0	0.04 (1)
AB- C	-164 / 0	0.03 (1)
AC- B	-186 / 0	0.03 (1)
A-AC	0 / 24	0.01 (1)

LICENSED PROFESSIONAL ENGINEER
01/15/19
R. J. G ALVES
100096824
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS.

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.05/1.00 (J-K:1) , BC=0.03/1.00 (P-Q:3) , WB=0.24/1.00 (I-W:1) , SSI=0.06/1.00 (N-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI (GRIP= 0.82 (A) (INPUT = 0.90)
JSI (METAL= 0.08 (B) (INPUT = 1.00)

DWG NO. T1904167
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300334	T30A	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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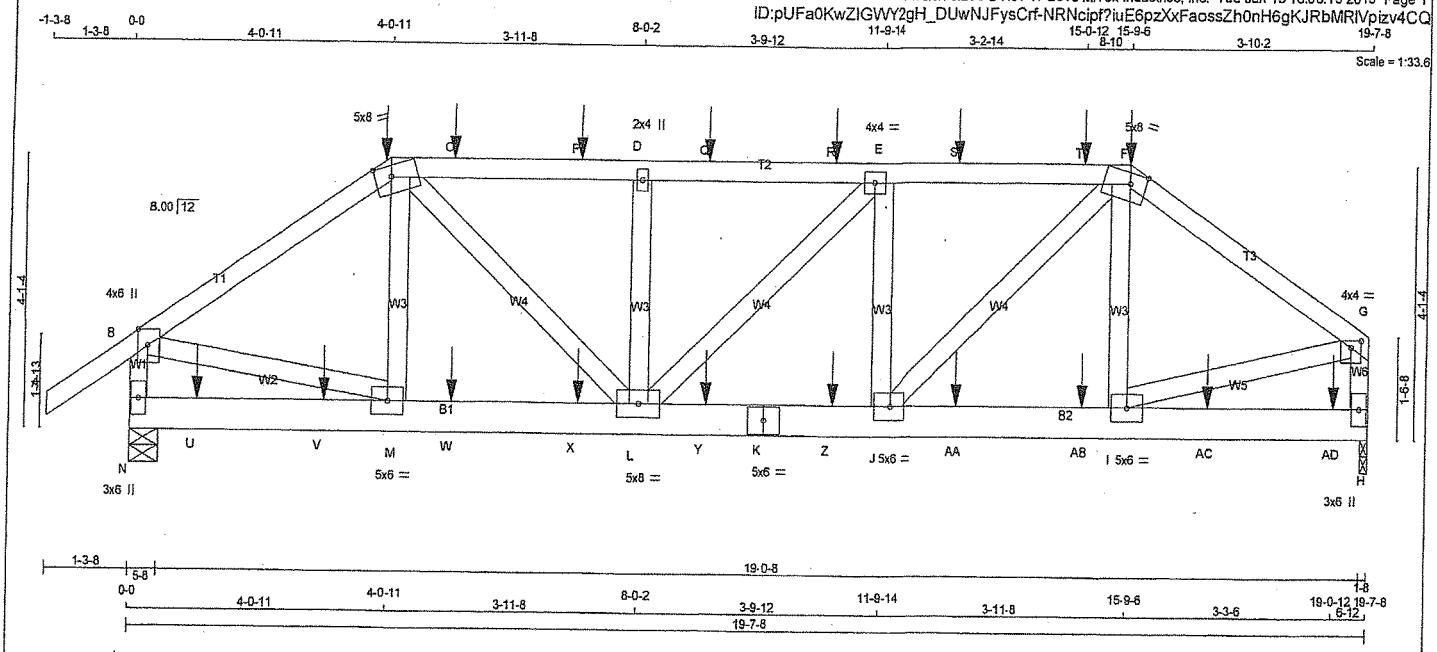

TOTAL WEIGHT = 2 X 104 = 208 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	SIDE(61.0)
C-F	1	12	SIDE(61.0)
F-G	1	12	SIDE(61.0)
N-B	1	12	TOP
H-G	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
N-K	2	12	SIDE(0.0)
K-H	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT2D	4.0	6.0	Edge	
C	TTWW-m	MT20	5.0	8.0	2.00	3.00
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTWW-m	MT20	5.0	8.0	2.00	3.00
G	TMVW-p	MT20	4.0	4.0	1.25	2.00
H	BMV1+p	MT20	3.0	6.0		
I, J, M						
I	BMWW-t	MT20	5.0	6.0		
K	BS-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	5.0	8.0		
N	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
N	2171	0	2171	0	0	5-8	5-8
H	2096	0	2096	0	0	1-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX /MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1640	837 / 0	352 / 0	0 / 0	0 / 0	451 / 0	0 / 0
H	1593	784 / 0	364 / 0	0 / 0	0 / 0	445 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.30 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)		MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO			FR-TO		
A-B	0 / 32	-84.9	-84.9	0.06 (1)	10.00	M-C	-155 / 151	0.01 (1)
B-C	-2344 / 0	-84.9	-84.9	0.16 (1)	5.69	C-L	0 / 1135	0.10 (1)
C-O	-2768 / 0	-84.9	-84.9	0.18 (1)	5.31	L-D	-564 / 0	0.05 (1)
O-P	-2768 / 0	-84.9	-84.9	0.18 (1)	5.31	L-E	0 / 12	0.00 (2)
P-D	-2768 / 0	-84.9	-84.9	0.18 (1)	5.31	J-E	-582 / 0	0.05 (1)
D-Q	-2768 / 0	-84.9	-84.9	0.18 (1)	5.30	J-F	0 / 1212	0.11 (1)
Q-R	-2768 / 0	-84.9	-84.9	0.18 (1)	5.30	I-F	-247 / 100	0.02 (1)
R-E	-2768 / 0	-84.9	-84.9	0.18 (1)	5.30	B-M	0 / 2008	0.18 (1)
E-S	-2759 / 0	-84.9	-84.9	0.18 (1)	5.31	M-G	0 / 1961	0.17 (1)
S-T	-2759 / 0	-84.9	-84.9	0.18 (1)	5.31			
T-F	-2759 / 0	-84.9	-84.9	0.18 (1)	5.31			
F-G	-2271 / 0	-84.9	-84.9	0.14 (1)	5.78			
N-B	-2055 / 0	0.0	0.0	0.11 (1)	7.79			
H-G	-1964 / 0	0.0	0.0	0.11 (1)	7.81			
N-U	0 / 0	-38.5	-38.5	0.05 (3)	10.00			
U-V	0 / 0	-38.5	-38.5	0.05 (3)	10.00			
V-M	0 / 0	-38.5	-38.5	0.05 (3)	10.00			
M-W	0 / 1941	-38.5	-38.5	0.16 (1)	10.00			
W-X	0 / 1941	-38.5	-38.5	0.16 (1)	10.00			
X-L	0 / 1941	-38.5	-38.5	0.16 (1)	10.00			
L-Y	0 / 2760	-38.5	-38.5	0.22 (1)	10.00			
Y-K	0 / 2760	-38.5	-38.5	0.22 (1)	10.00			
K-Z	0 / 2760	-38.5	-38.5	0.22 (1)	10.00			
Z-J	0 / 2760	-38.5	-38.5	0.22 (1)	10.00			
J-AA	0 / 1876	-38.5	-38.5	0.15 (1)	10.00			
AA-AB	0 / 1876	-38.5	-38.5	0.15 (1)	10.00			
AB-I	0 / 1876	-38.5	-38.5	0.15 (1)	10.00			
I-AC	0 / 0	-38.5	-38.5	0.05 (3)	10.00			
AC-AD	0 / D	-38.5	-38.5	0.05 (3)	10.00			
AD-H	0 / 0	-38.5	-38.5	0.05 (3)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	4-0-11	-50	-56	—	FRONT	VERT	DEAD	—	—
C	4-0-11	-231	-231	—	FRONT	VERT	SNOW	—	—
F	15-9-6	-48	-53	—	FRONT	VERT	DEAD	—	—
F	15-9-6	-219	-219	—	FRONT	VERT	SNOW	—	—
O	5-0-12	-102	-102	—	BACK	VERT	TOTAL	—	—
P	7-0-12	-102	-102	—	BACK	VERT	TOTAL	—	—

LICENSED PROFESSIONAL ENGINEER
01/15/19
H. J. G. ALVES
100090124
PROVINCE OF ONTARIO

DWG NO. TAM T1901168
STRUCTURAL COMPONENT ONLY

1/2

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.18/1.00 (E-F:1) , BC=0.22/1.00 (J-L:1) , WB=0.18/1.00 (B-M:1) , SSI=0.14/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)		SHEAR (PSI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (G) (INPUT = 0.90)
JSI METAL= 0.58 (B) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300334	T30A	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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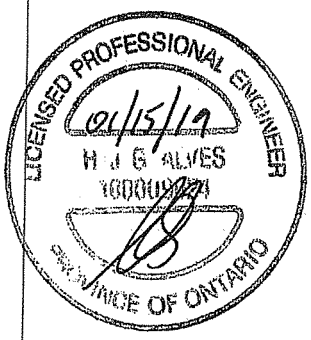
FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
O	9-0-12	-102	-102	—	BACK	VERT	TOTAL	—	—
R	11-0-12	-102	-102	—	BACK	VERT	TOTAL	—	—
S	13-0-12	-102	-102	—	BACK	VERT	TOTAL	—	—
T	15-0-12	-114	-114	—	BACK	VERT	TOTAL	—	—
U	1-0-12	-55	-70	—	BACK	VERT	TOTAL	—	—
V	3-0-12	-55	-70	—	BACK	VERT	TOTAL	—	—
W	5-0-12	-55	-70	—	BACK	VERT	TOTAL	—	—
X	7-0-12	-55	-70	—	BACK	VERT	TOTAL	—	—
Y	9-0-12	-55	-70	—	BACK	VERT	TOTAL	—	—
Z	11-0-12	-55	-70	—	BACK	VERT	TOTAL	—	—
AA	13-0-12	-55	-70	—	BACK	VERT	TOTAL	—	—
AB	15-0-12	-55	-70	—	BACK	VERT	TOTAL	—	—
AC	17-0-12	-55	-70	—	BACK	VERT	TOTAL	—	—
AD	19-0-12	-63	-81	—	BACK	VERT	TOTAL	—	—



DWG NO. TAM T1901168
STRUCTURAL
COMPONENT ONLY

JDB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300334	T31A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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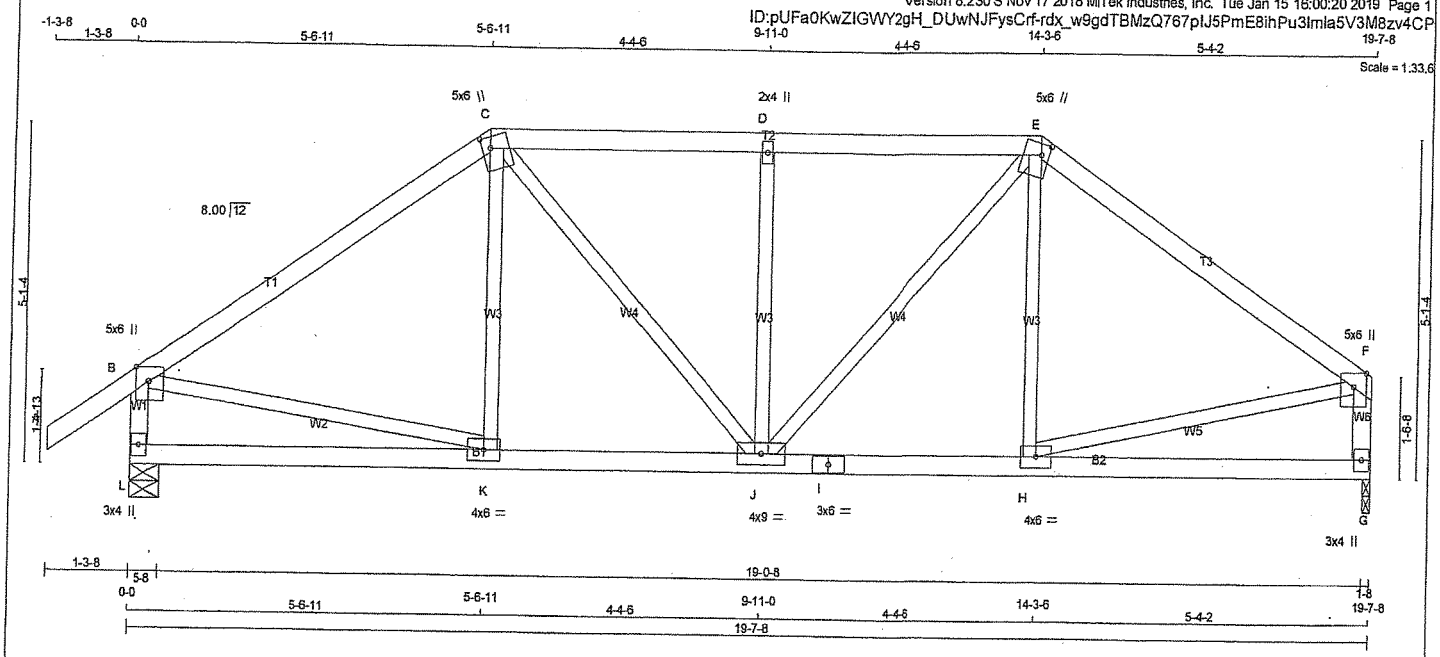


TOTAL WEIGHT = 81 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMW+w	MT2D	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.00	1.50
F	TMVW+p	MT2D	5.0	6.0	Edge	
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	9.0		
K	BMWW-t	MT2D	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1327	0	1327	0	0	5-8	5-8
G	1210	0	1210	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	999	521 / 0	206 / 0	0 / 0	0 / 0	272 / 0	0 / 0
G	918	457 / 0	206 / 0	0 / 0	0 / 0	255 / 0	0 / 0

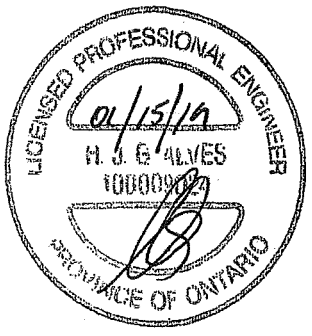
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.34 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FR-TO	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 32		-84.9	-84.9	0.11 (1)	10.00	K- C	0 / 189	0.04 (3)
B- C	-1239 / 0		-84.9	-84.9	0.37 (1)	5.34	C- J	0 / 367	0.08 (1)
C- D	-1277 / 0		-84.9	-84.9	0.22 (1)	5.47	J- D	-448 / 0	0.17 (1)
D- E	-1277 / 0		-84.9	-84.9	0.22 (1)	5.47	J- E	0 / 404	0.09 (1)
E- F	-1210 / 0		-84.9	-84.9	0.34 (1)	5.43	H- E	0 / 167	0.04 (3)
L- B	-1238 / 0		0.0	0.0	0.13 (1)	7.22	B- K	0 / 1048	0.24 (1)
G- F	-1125 / 0		0.0	0.0	0.12 (1)	7.48	H- F	0 / 1031	0.23 (1)
L- K	0 / 0		-38.5	-38.5	0.21 (3)	10.00			
K- J	0 / 1030		-38.5	-38.5	0.35 (2)	10.00			
J- I	0 / 1006		-38.5	-38.5	0.33 (2)	10.00			
I- H	0 / 1006		-38.5	-38.5	0.33 (2)	10.00			
H- G	0 / 0		-38.5	-38.5	0.20 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.85")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.37/1.00 (B-C:1) , BC=0.35/1.00 (J-K:2) , WB=0.24/1.00 (B-K:1) , SSI=0.18/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION , (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.78 (B) (INPUT = 0.90)
JSI METAL= 0.53 (B) (INPUT = 1.00)

DWG NO. TAM 7901169
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300334	T32A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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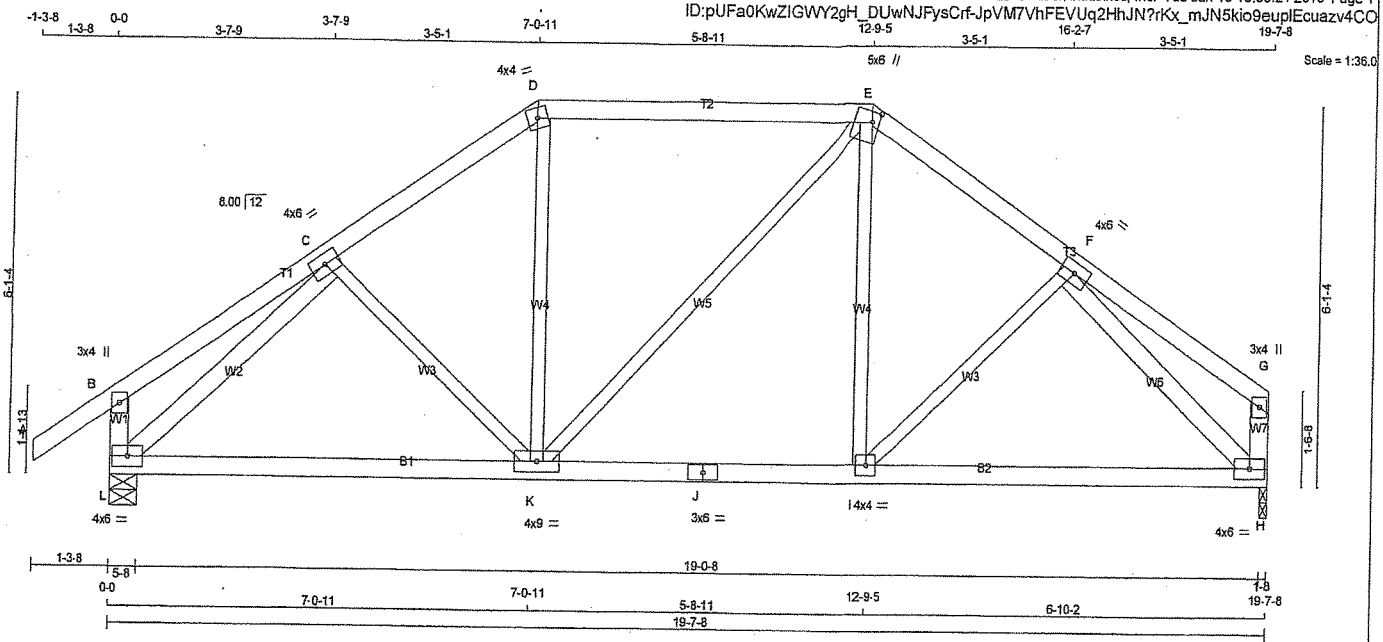


TOTAL WEIGHT = 86 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
O - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
L - C	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.00	1.50
F	TMWW-t	MT20	4.0	6.0		
G	TMV+p	MT20	3.0	4.0		
H	BMWW1-t	MT20	4.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	9.0		
L	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REORD BRG
L	1327	0	1327	0	0	5-8	5-8
H	1210	0	1210	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	999	521 / 0	206 / 0	0 / 0	0 / 0	272 / 0	0 / 0
H	918	457 / 0	206 / 0	0 / 0	0 / 0	255 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.73 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

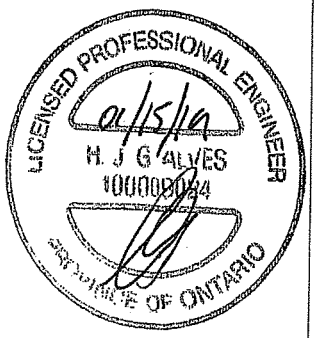
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO		FROM TO			
A- B	0 / 32	-84.9 -84.9	0.11 (1)	10.00	
B- C	0 / 18	-84.9 -84.9	0.16 (1)	10.00	
C- O	-1184 / 0	-84.9 -84.9	0.14 (1)	5.73	
D- E	-973 / 0	-84.9 -84.9	0.37 (1)	5.82	
E- F	-1166 / 0	-84.9 -84.9	0.13 (1)	5.77	
F- G	0 / 18	-84.9 -84.9	0.15 (1)	10.00	
L- B	-233 / 0	0.0 0.0	0.02 (1)	7.81	
H- G	-107 / 0	0.0 0.0	0.01 (1)	7.81	
L- K	0 / 1008	-38.5 -38.5	0.44 (2)	10.00	
K- J	0 / 959	-38.5 -38.5	0.43 (2)	10.00	
J- I	0 / 959	-38.5 -38.5	0.43 (2)	10.00	
I- H	0 / 957	-38.5 -38.5	0.42 (2)	10.00	

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
C- K	-52 / 69	0.02 (1)
K- D	0 / 327	0.07 (2)
K- E	0 / 21	0.00 (1)
I- E	0 / 302	0.07 (2)
I- F	-5 / 89	0.02 (3)
L- C	-1404 / 0	0.39 (1)
F- H	-1378 / 0	0.36 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.37/1.00 (D-E:1) , BC=0.44/1.00 (K-L:2) , WB=0.39/1.00 (C-L:1) , SSI=0.19/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (F) (INPUT = 0.90)
JSI METAL= 0.34 (C) (INPUT = 1.00)

DWG NO. TAM T1900170
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300335	T33	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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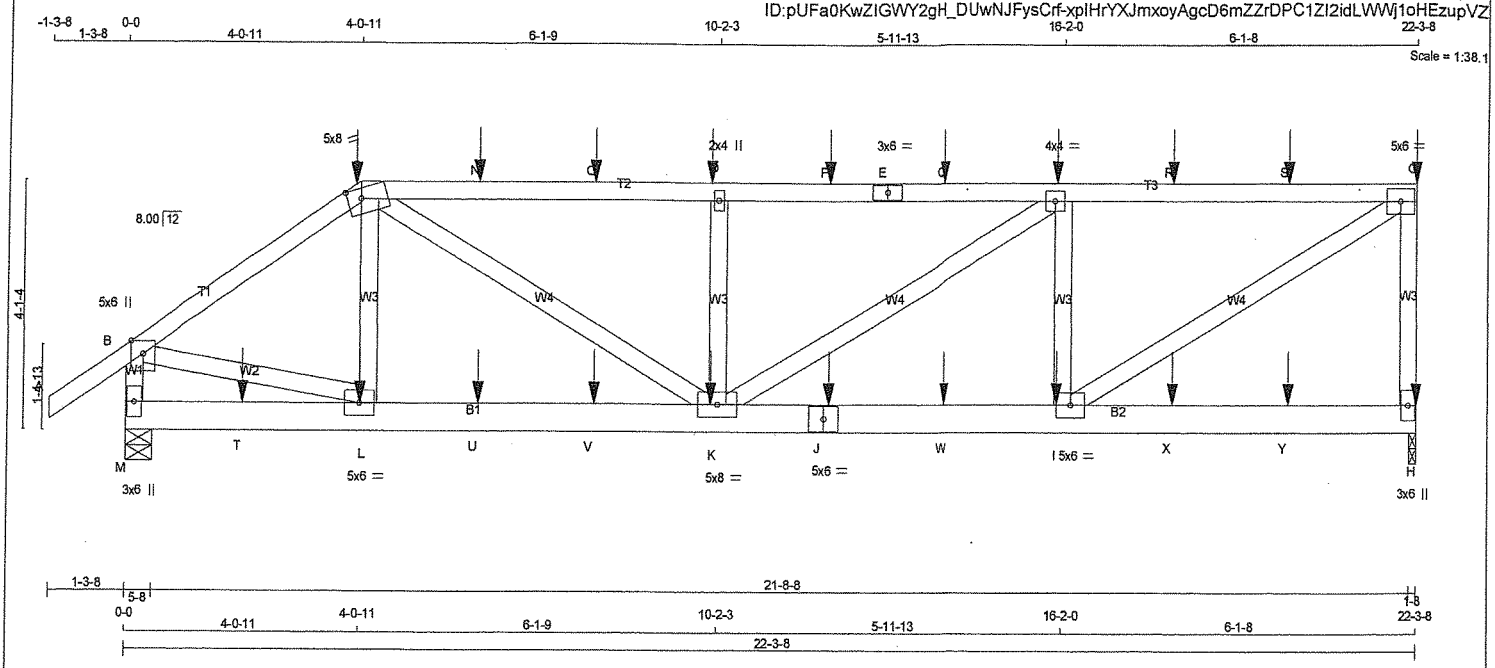


TOTAL WEIGHT = 2 X 111 = 223 lb

[M

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

OESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS.

CHOROS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHOROS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIOE(61.0)
C- E	1	12	SIDE(61.0)
E- G	1	12	SIDE(61.0)
G- H	1	12	TOP
M- B	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
M- J	2	12	SIDE(183.1)
J- H	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERREO TO EACH PLY.

SIOE - PLF SHOWN IS THE EQUIVALENT UDL APPLIEO TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST 8E APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW-m	MT20	5.0	8.0	2.00	2.75
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	6.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	5.0	8.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DWG NO. TAM T1901271
STRUCTURAL
COMPONENT ONLY 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO 8E VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTOREO GROSS REACTION VERT	HORZ	MAXIMUM FACTOREO GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	2404	0	2404	0	0	1-8	1-8
M	2424	0	2424	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINEO	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WINO	0EAD	SOIL
H	1828	895 / 0	421 / 0	0 / 0	0 / 0	512 / 0	0 / 0
M	1828	940 / 0	385 / 0	0 / 0	0 / 0	504 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.46 FT.
MAX. UNBRACED BOTTOM CHORO LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

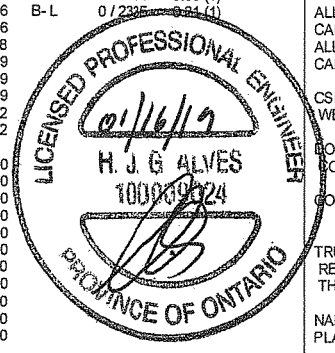
LOADING

TOTAL LOAO CASES: (4)

CHOROS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 32	-84.9	-84.9	0.06 (1)	10.00		L- C	-157 / 199	0.02 (3)
B- C	-2735 / 0	-84.9	-84.9	0.17 (1)	5.36		C- K	0 / 1578	0.14 (1)
C- N	-3615 / 0	-84.9	-84.9	0.46 (1)	4.47		K- D	-885 / 0	0.08 (1)
N- O	-3615 / 0	-84.9	-84.9	0.46 (1)	4.47		K- F	0 / 822	0.07 (1)
O- O	-3615 / 0	-84.9	-84.9	0.48 (1)	4.47		I- F	-1334 / 0	0.12 (1)
O- P	-3615 / 0	-84.9	-84.9	0.48 (1)	4.46		I- G	0 / 3416	0.30 (1)
P- E	-3615 / 0	-84.9	-84.9	0.46 (1)	4.46		B- L	0 / 2335	0.84 (1)
E- Q	-3615 / 0	-84.9	-84.9	0.46 (1)	4.46				
Q- F	-3615 / 0	-84.9	-84.9	0.46 (1)	4.48				
F- R	-2918 / 0	-84.9	-84.9	0.43 (1)	4.89				
R- S	-2918 / 0	-84.9	-84.9	0.43 (1)	4.89				
S- G	-2918 / 0	-84.9	-84.9	0.43 (1)	4.89				
H- G	-2192 / 0	0.0	0.0	0.27 (1)	7.52				
M- B	-2351 / 0	0.0	0.0	0.13 (1)	7.32				
M- T	0 / 0	-38.5	-38.5	0.08 (3)	10.00				
T- L	0 / 0	-38.5	-38.5	0.08 (3)	10.00				
L- U	0 / 2266	-38.5	-38.5	0.21 (1)	10.00				
U- V	0 / 2266	-38.5	-38.5	0.21 (1)	10.00				
V- K	0 / 2266	-38.5	-38.5	0.21 (1)	10.00				
K- J	0 / 2918	-38.5	-38.5	0.26 (2)	10.00				
J- W	0 / 2918	-38.5	-38.5	0.26 (2)	10.00				
W- I	0 / 2918	-38.5	-38.5	0.26 (2)	10.00				
I- X	0 / 0	-38.5	-38.5	0.10 (3)	10.00				
X- Y	0 / 0	-38.5	-38.5	0.10 (3)	10.00				
Y- H	0 / 0	-38.5	-38.5	0.10 (3)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	OIR.	TYPE	HEEL	CONN.
C	4-0-11	-50	-56	—	FRONT	VERT	DEAO	—	—
C	4-0-11	-110	-110	—	FRONT	VERT	TOTAL	—	—
C	4-0-11	-231	-231	—	FRONT	VERT	SNOW	—	—
D	10-0-12	-102	-102	—	FRONT	VERT	TOTAL	—	—
F	16-0-12	-102	-102	—	FRONT	VERT	TOTAL	—	—
G	22-3-8	-135	-135	—	FRONT	VERT	TOTAL	—	—
H	22-3-8	-70	-89	—	FRONT	VERT	TOTAL	—	—
I	16-0-12	-55	-70	—	FRONT	VERT	TOTAL	—	—
J	12-0-12	-55	-70	—	FRONT	VERT	TOTAL	—	—
K	10-0-12	-55	-70	—	FRONT	VERT	TOTAL	—	—
L	4-0-12	-55	-70	—	FRONT	VERT	TOTAL	—	—



DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGEO BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	OL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASEO ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADOT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS OESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS OESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.46/1.00 (D-F.1) , BC=0.26/1.00 (I-K.2) , WB=0.30/1.00 (G-I:1) , SSI=0.19/1.00 (F-G:1)

BOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

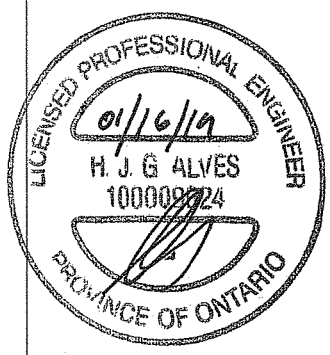
JSI GRIP= 0.87 (C) (INPUT = 0.90)
JSI METAL= 0.53 (B) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300335	T33	1	2	TRUSS DESC.	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
N	6-0-12	-102	-102	—	FRONT	VERT	TOTAL	—	—
O	8-0-12	-102	-102	—	FRONT	VERT	TOTAL	—	—
P	12-0-12	-102	-102	—	FRONT	VERT	TOTAL	—	—
Q	14-0-12	-102	-102	—	FRONT	VERT	TOTAL	—	—
R	18-0-12	-102	-102	—	FRONT	VERT	TOTAL	—	—
S	20-0-12	-102	-102	—	FRONT	VERT	TOTAL	—	—
T	2-0-12	-55	-70	—	FRONT	VERT	TOTAL	—	—
U	6-0-12	-55	-70	—	FRONT	VERT	TOTAL	—	—
V	8-0-12	-55	-70	—	FRONT	VERT	TOTAL	—	—
W	14-0-12	-55	-70	—	FRONT	VERT	TOTAL	—	—
X	18-0-12	-55	-70	—	FRONT	VERT	TOTAL	—	—
Y	20-0-12	-55	-70	—	FRONT	VERT	TOTAL	—	—



DWG NO. TAM T19012 71
STRUCTURAL
COMPONENT ONLY 3/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300335	T34	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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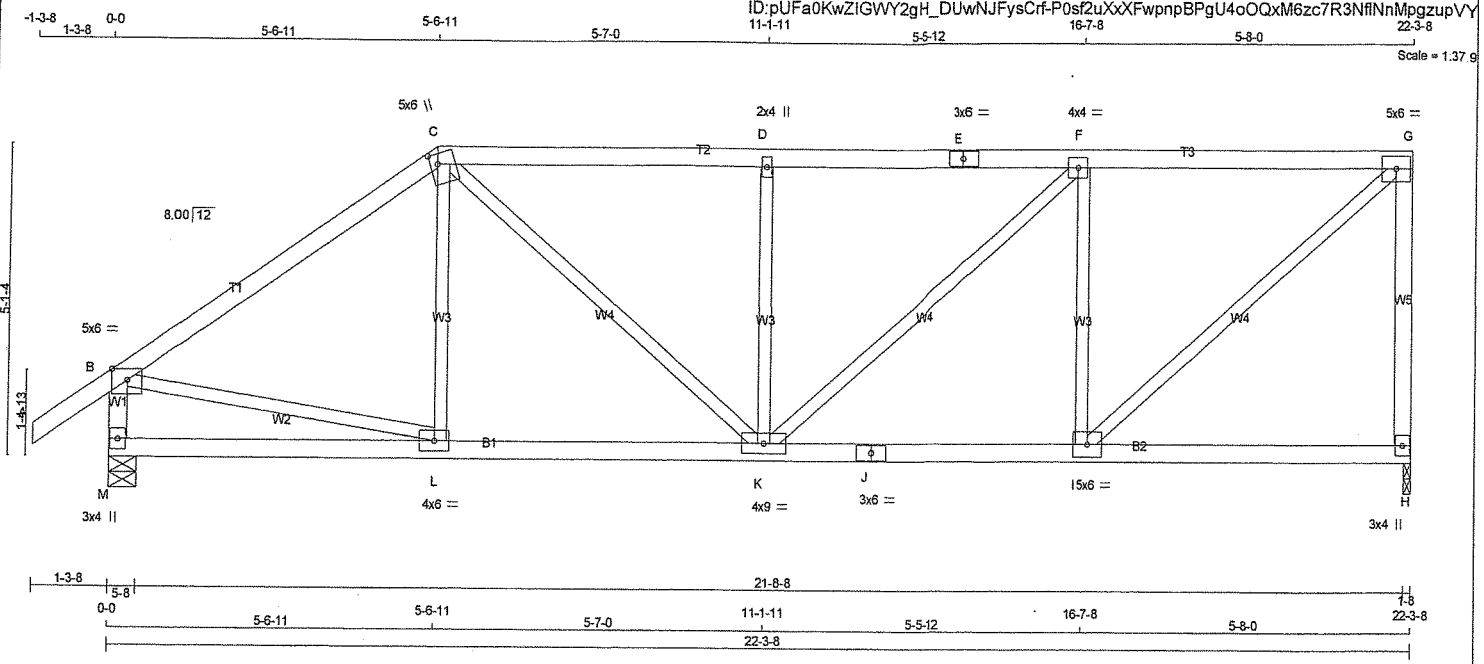


TOTAL WEIGHT = 93 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	9.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REORD BRG
H	1375	0	1375	0	0	1-8	1-8
M	1491	0	1491	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1043	519 / 0	234 / 0	0 / 0	0 / 0	290 / 0	0 / 0
M	1123	583 / 0	234 / 0	0 / 0	0 / 0	306 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

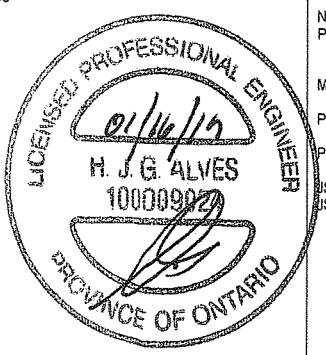
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES. (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX. CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 32	-84.9	-84.9	0.11 (1)	10.00	L-C	0 / 188	0.04 (3)
B-C	-1472 / 0	-84.9	-84.9	0.55 (1)	4.72	B-L	0 / 1245	0.28 (1)
C-D	-1657 / 0	-84.9	-84.9	0.42 (1)	4.69	I-G	0 / 1687	0.38 (1)
D-E	-1657 / 0	-84.9	-84.9	0.43 (1)	4.66	C-K	0 / 572	0.13 (1)
E-F	-1657 / 0	-84.9	-84.9	0.43 (1)	4.66	I-F	-846 / 0	0.33 (1)
F-G	-1286 / 0	-84.9	-84.9	0.41 (1)	5.18	K-D	-507 / 0	0.20 (1)
H-G	-1287 / 0	0.0	0.0	0.57 (1)	7.11	K-F	0 / 494	0.11 (1)
M-B	-1404 / 0	0.0	0.0	0.14 (1)	6.87			
M-L	0 / 0	-38.5	-38.5	0.23 (3)	10.00			
L-K	0 / 1224	-38.5	-38.5	0.39 (2)	10.00			
K-J	0 / 1286	-38.5	-38.5	0.40 (2)	10.00			
J-I	0 / 1286	-38.5	-38.5	0.40 (2)	10.00			
I-H	0 / 0	-38.5	-38.5	0.23 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.57/1.00 (G-H:1) , BC=0.40/1.00 (I-K:2) , WB=0.38/1.00 (G-I:1) , SSI=0.22/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg

SSI GRIP= 0.83 (B) (INPUT = 0.90)
SSI METAL= 0.48 (J) (INPUT = 1.00)

DWG NO. TAM T1901272
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300335	T35	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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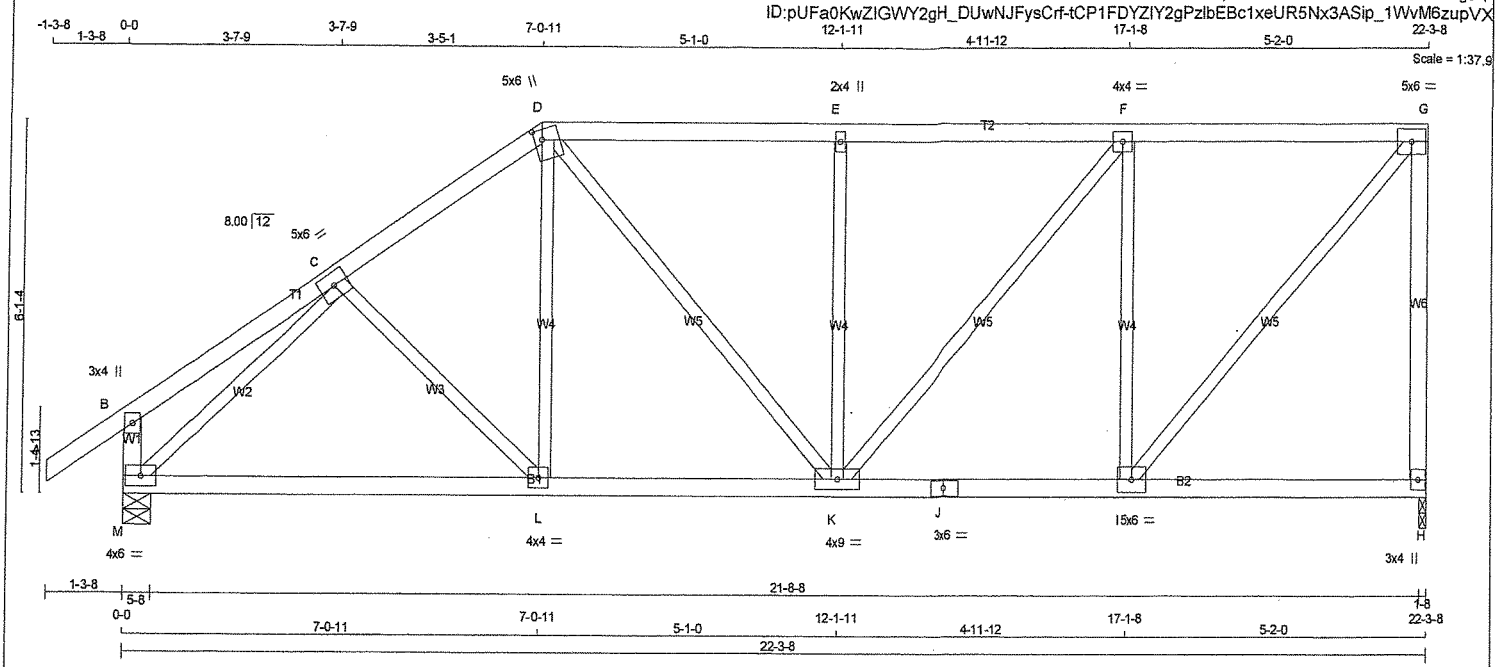


TOTAL WEIGHT = 101 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	9.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	1375	0	1375	0	0	1-8	1-8
M	1491	0	1491	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1043	519 / 0	234 / 0	0 / 0	0 / 0	290 / 0	0 / 0
M	1123	583 / 0	234 / 0	0 / 0	0 / 0	306 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.18 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 32	-84.9	-84.9	0.11 (1)	10.00	C- L	-5 / 94	0.02 (3)
B- C	0 / 17	-84.9	-84.9	0.15 (1)	10.00	L- D	0 / 335	0.08 (2)
C- D	-1428 / 0	-84.9	-84.9	0.18 (1)	5.27	M- C	-1640 / 0	0.63 (1)
D- E	-1349 / 0	-84.9	-84.9	0.32 (1)	5.20	I- G	0 / 1496	0.34 (1)
E- F	-1349 / 0	-84.9	-84.9	0.34 (1)	5.18	D- K	0 / 262	0.06 (1)
F- G	-994 / 0	-84.9	-84.9	0.32 (1)	5.83	I- F	-888 / 0	0.52 (1)
H- G	-1296 / 0	0.0	0.0	0.94 (1)	7.09	K- E	-461 / 0	0.27 (1)
M- B	-234 / 0	0.0	0.0	0.02 (1)	7.81	K- F	0 / 545	0.12 (1)
M- L	0 / 1175	-38.5	-38.5	0.48 (2)	10.00			
L- K	0 / 1176	-38.5	-38.5	0.48 (2)	10.00			
K- J	0 / 994	-38.5	-38.5	0.33 (2)	10.00			
J- I	0 / 994	-38.5	-38.5	0.33 (2)	10.00			
I- H	0 / 0	-38.5	-38.5	0.20 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH.
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.94/1.00 (G-H:1) , BC=0.48/1.00 (K-L:2) , WB=0.83/1.00 (C-M:1) , SSI=0.20/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

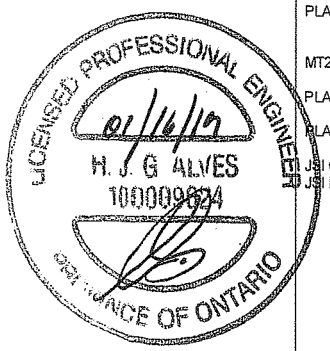
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (M) (INPUT = 0.90)
JSI METAL= 0.41 (J) (INPUT = 1.00)



DWG NO. TAM T190123
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300335	T36	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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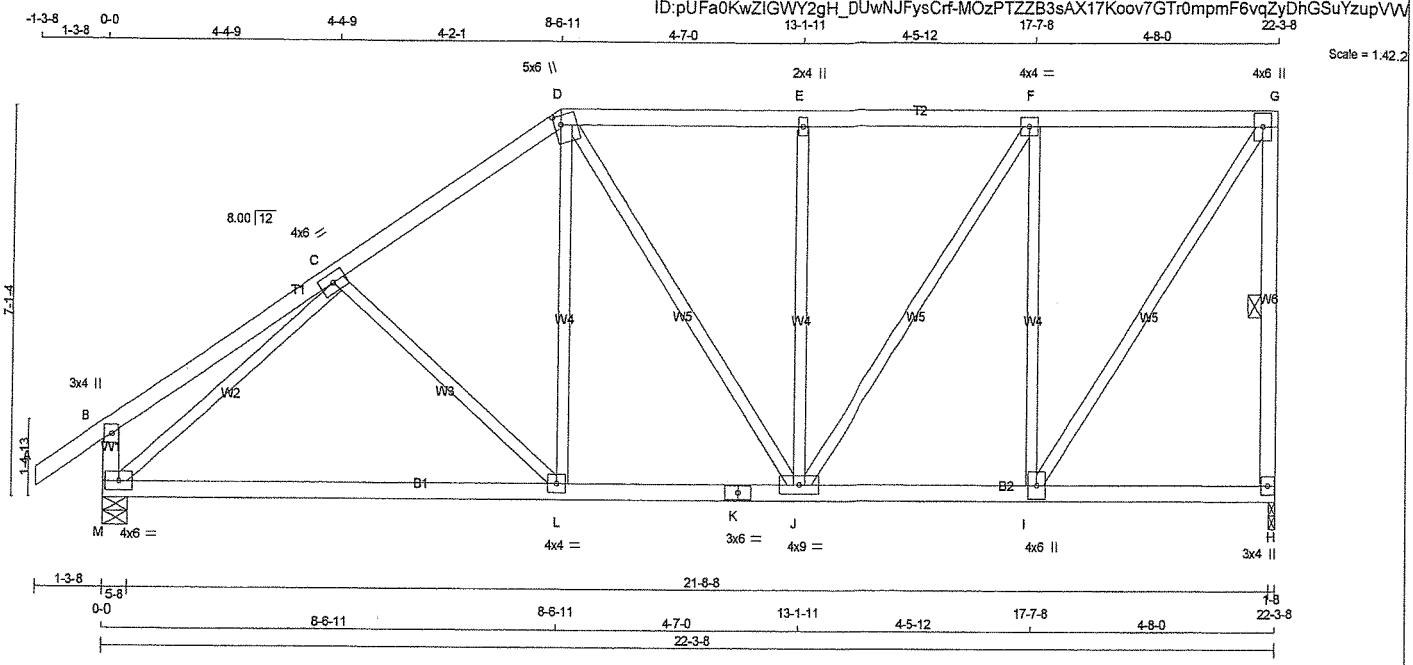


TOTAL WEIGHT = 107 lb

[M]

LUMBER

N. L. G.A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW+p	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWW+t	MT20	4.0	6.0		
J	BMWWW-t	MT20	4.0	9.0		
K	BS-t	MT20	3.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	1375	0	1375	0	0	1-8	1-8
M	1491	0	1491	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1043	519 / 0	234 / 0	0 / 0	0 / 0	290 / 0	0 / 0
M	1123	583 / 0	234 / 0	0 / 0	0 / 0	306 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.26 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

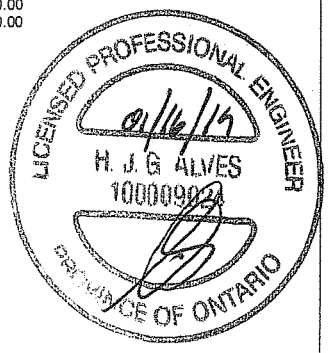
1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
A- B	0 / 32	-84.9	-84.9	0.11 (1)	10.00	C- L	-126 / 55	0.08 (1)	
B- C	0 / 23	-84.9	-84.9	0.24 (1)	10.00	L- D	0 / 470	0.11 (2)	
C- D	-1347 / 0	-84.9	-84.9	0.28 (1)	5.26	M- C	-1622 / 0	0.91 (1)	
D- E	-1112 / 0	-84.9	-84.9	0.25 (1)	5.69	I- G	0 / 1387	0.31 (1)	
E- F	-1112 / 0	-84.9	-84.9	0.26 (1)	5.68	D- J	-64 / 10	0.09 (3)	
F- G	-784 / 0	-84.9	-84.9	0.25 (1)	6.25	I- F	-926 / 0	0.81 (1)	
H- G	-1305 / 0	0.0	0.0	0.30 (1)	5.66	J- E	-415 / 0	0.36 (1)	
M- B	-257 / 0	0.0	0.0	0.03 (1)	7.81	J- F	0 / 598	0.13 (1)	
M- L	0 / 1194	-38.5	-38.5	0.62 (2)	10.00				
L- K	0 / 1107	-38.5	-38.5	0.62 (2)	10.00				
K- J	0 / 1107	-38.5	-38.5	0.62 (2)	10.00				
J- I	0 / 784	-38.5	-38.5	0.28 (2)	10.00				
I- H	0 / 0	-38.5	-38.5	0.18 (3)	10.00				



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.22")
ALLOWABLE DEFL.(TL)= L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/ 723 (0.37")

CSI: TC=0.30/1.00 (G-H:1) , BC=0.82/1.00 (L-M:2) , WB=0.91/1.00 (C-M:1) , SSI=0.21/1.00 (L-M:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (M) (INPUT = 0.90)
JSI METAL= 0.39 (C) (INPUT = 1.00)

DWG NO. TAM T7901214
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300335	T37	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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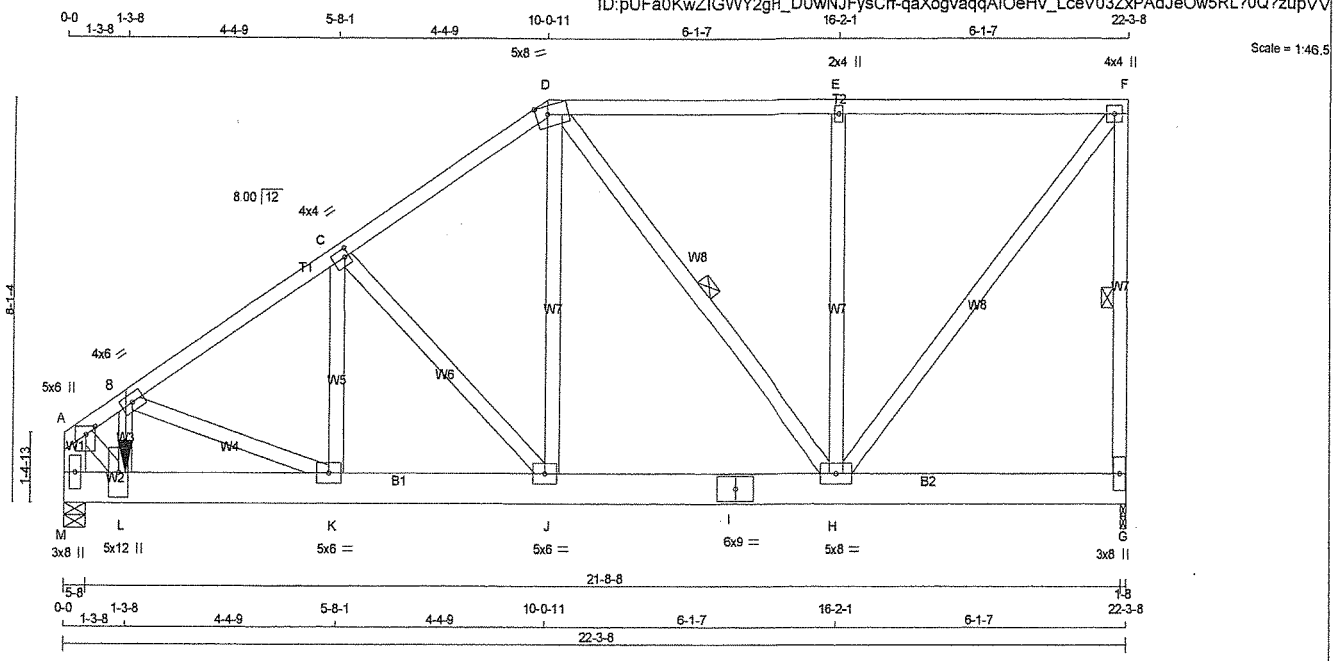

TOTAL WEIGHT = 2 X 147 = 294 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
M - A	2x6	DRY	No.2	SPF
M - I	2x8	DRY	No.2	SPF
I - G	2x8	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	TOP
D- F	1	12	TOP
F- G	1	12	TOP
M- A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
M- I	2	12	TOP
I- G	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	8	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	2.00	2.25
B	TMWW-t	MT20	4.0	6.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.00
D	TTWW-m	MT20	5.0	8.0	2.00	3.00
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	4.0		
G	BMV1+p	MT20	3.0	8.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BS-t	MT20	6.0	9.0		
J	BMWW-t	MT20	5.0	6.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMWW+t	MT20	5.0	12.0		
M	BMV1+p	MT20	3.0	8.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REORD BRG
JT	VERT	HORZ	DOWN	HDRZ	UPLIFT	IN-SX	IN-SX
G	1690	0	1690	0	0	1-8	1-8
M	6496	0	6496	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1280	642 / 0	284 / 0	0 / 0	0 / 0	355 / 0	D / 0
M	4902	2516 / 0	1043 / 0	D / 0	0 / 0	1343 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-5021 / 0	-84.9	-84.9	0.10 (1)	4.25
B- C	-2887 / 0	-84.9	-84.9	0.18 (1)	5.23
C- D	-1807 / 0	-84.9	-84.9	0.16 (1)	6.25
D- E	-1112 / 0	-84.9	-84.9	0.31 (1)	6.25
E- F	-1112 / 0	-84.9	-84.9	0.31 (1)	6.25
G- F	-1588 / 0	0.0	0.0	0.22 (1)	6.25
M- A	-5819 / 0	0.0	0.0	0.21 (1)	6.11
M- L	0 / 0	-38.5	-38.5	0.21 (1)	10.00
L- K	0 / 4187	-38.5	-38.5	0.43 (1)	10.00
K- J	0 / 2419	-38.5	-38.5	0.17 (1)	10.00
J- I	0 / 1504	-38.5	-38.5	0.11 (2)	10.00
I- H	0 / 1504	-38.5	-38.5	0.11 (2)	10.00
H- G	0 / 0	-38.5	-38.5	0.04 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FDRCE (LBS)	MAX CSI (LC)
L- B	0 / 1673	0.15 (1)
B- K	-1910 / 0	0.25 (1)
K- C	0 / 1144	0.10 (1)
C- J	-1351 / 0	0.36 (1)
J- D	0 / 1139	0.10 (1)
A- L	0 / 5148	0.46 (1)
H- F	0 / 1781	0.16 (1)
D- H	-627 / 0	0.15 (1)
H- E	-643 / 0	0.26 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	REEL	CONN.
L	1-3-8	-5436	-5436	—	TOP	VERT	TOTAL		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS DF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2D11, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.31/1.00 (E-F:1) , BC=0.43/1.00 (K-L:1) , WB=0.46/1.00 (A-L:1) , SSI=0.20/1.00 (L-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (F) (INPUT = 0.90)
JSI METAL= 0.75 (L) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
01/16/19
H. J. G. ALVES
100009824
PROVINCE OF ONTARIO

DWG NO. TAM T190275
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300335	T38A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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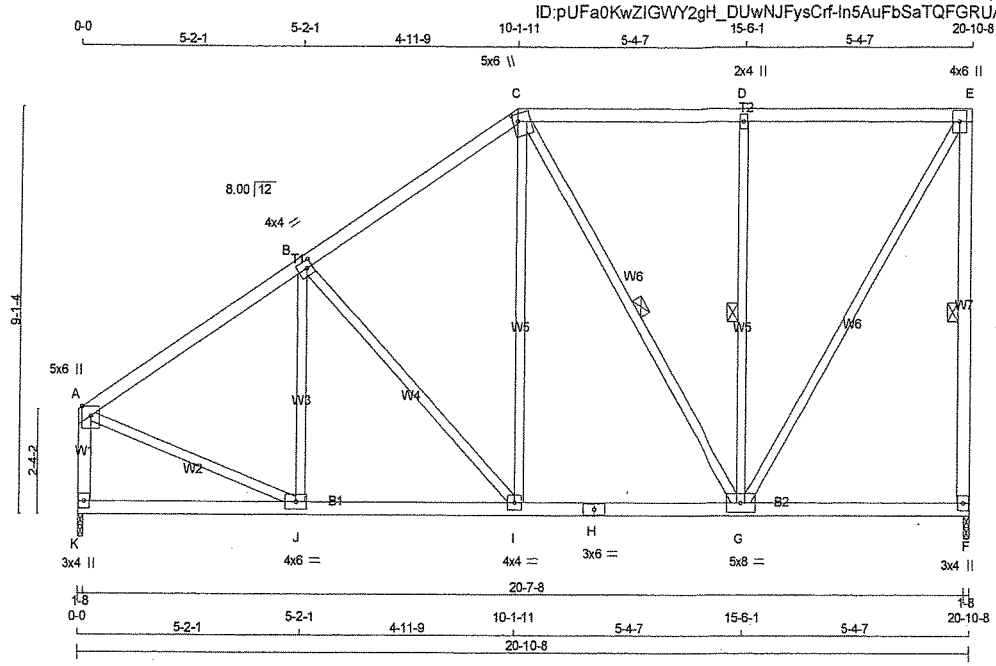


TOTAL WEIGHT = 106 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	Edge	
B	TMWW-t	MT20	4.0	4.0	2.00	1.50
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW+p	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	8.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	1288	0	1288	0	0	1-8	1-8
K	1288	0	1288	0	0	1-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	977	486 / 0	219 / 0	0 / 0	0 / 0	271 / 0	0 / 0
K	977	486 / 0	219 / 0	0 / 0	0 / 0	271 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.40 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-F, C-G, D-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

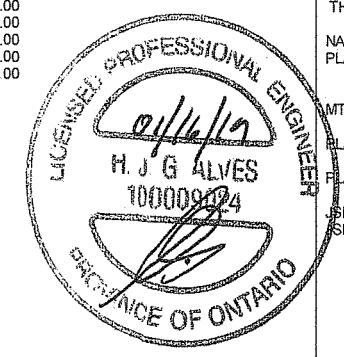
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	-1151 / 0	-84.9	-84.9	0.41 (1)		5.40
B- C	-953 / 0	-84.9	-84.9	0.40 (1)		5.80
C- D	-631 / 0	-84.9	-84.9	0.42 (1)		6.25
D- E	-631 / 0	-84.9	-84.9	0.42 (1)		6.25
F- E	-1204 / 0	0.0	0.0	0.46 (1)		5.83
K- A	-1207 / 0	0.0	0.0	0.15 (1)		7.29
K- J	0 / 0	-38.5	-38.5	0.19 (3)		10.00
J- I	0 / 981	-38.5	-38.5	0.32 (2)		10.00
I- H	0 / 772	-38.5	-38.5	0.31 (2)		10.00
H- G	0 / 772	-38.5	-38.5	0.31 (2)		10.00
G- F	0 / 0	-38.5	-38.5	0.21 (3)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- B	-162 / 87	0.08 (1)
B- I	-315 / 0	0.35 (1)
I- C	0 / 437	0.10 (2)
C- G	-272 / 0	0.21 (1)
G- D	-563 / 0	0.30 (1)
G- E	0 / 1207	0.27 (1)
A- J	0 / 1054	0.24 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.46/1.00 (E-F:1) , BC=0.32/1.00 (I-J:2) , WB=0.35/1.00 (B-I:1) , SSI=0.22/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.81 (G) (INPUT = 0.90)
SI METAL= 0.47 (A) (INPUT = 1.00)

DWG NO. TAM T4901276
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300335	T39A	6	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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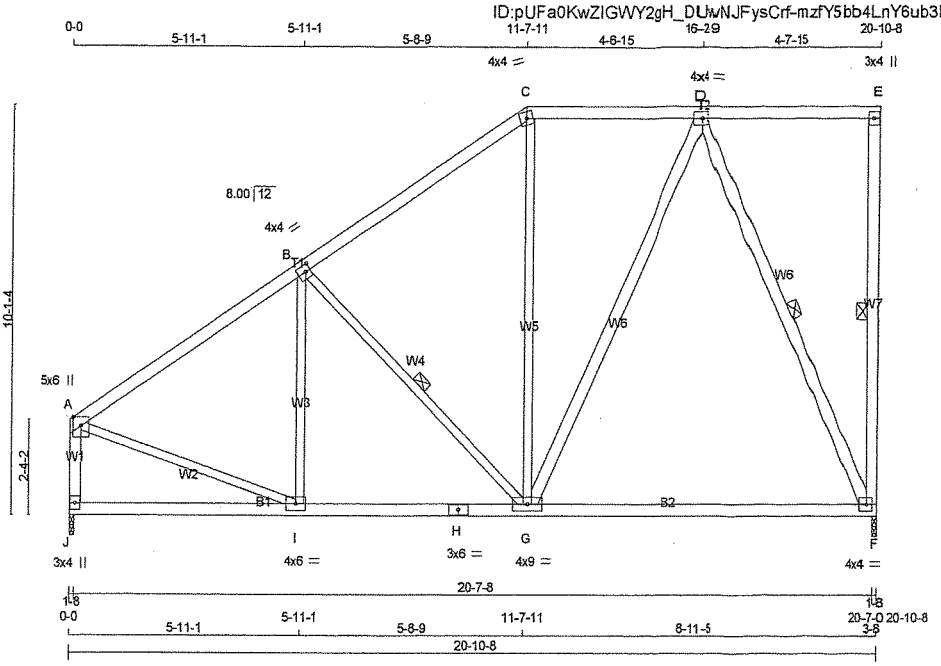


TOTAL WEIGHT = 6 X 111 = 667 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - A	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
G - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	Edge	
B	TMWW-t	MT20	4.0	4.0	2.00	1.50
C	TTW-m	MT20	4.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TMV+p	MT20	3.0	4.0		
F	BMVW1-t	MT20	4.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	6.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQR0 BRG IN-SX
F	1288	0	1288	0	0	1-8	1-8
J	1288	0	1288	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SDIL
F	977	486 / 0	219 / 0	0 / 0	0 / 0	271 / 0	0 / 0
J	977	486 / 0	219 / 0	0 / 0	0 / 0	271 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.43 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-F, B-G, D-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATE0 IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

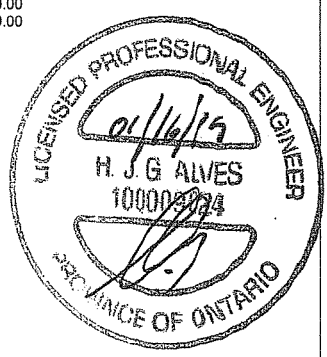
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1154 / 0	-84.9	-84.9	0.40 (1)	5.43
B- C	-878 / 0	-84.9	-84.9	0.38 (1)	6.03
C- D	-703 / 0	-84.9	-84.9	0.24 (1)	6.25
D- E	0 / 0	-84.9	-84.9	0.30 (1)	10.00
F- E	-150 / 0	0.0	0.0	0.07 (1)	6.25
J- A	-1188 / 0	0.0	0.0	0.15 (1)	7.32
J- I	0 / 0	-38.5	-38.5	0.23 (3)	10.00
I- H	0 / 987	-38.5	-38.5	0.64 (2)	10.00
H- G	0 / 987	-38.5	-38.5	0.64 (2)	10.00
G- F	0 / 472	-38.5	-38.5	0.57 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- B	-132 / 91	0.08 (1)
B- G	-410 / 0	0.19 (1)
G- C	0 / 198	0.05 (3)
A- I	0 / 1044	0.23 (1)
G- D	0 / 560	0.09 (2)
D- F	-1099 / 0	0.69 (1)



DESIGN CRITERIA

SPECIFIED LOA0S:

TOP CH.	LL	=	23.3	PSF
	0L	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS B.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOA0

ALLOWABLE 0EFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 948 (0.26")
ALLOWABLE 0EFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 567 (0.44")

CSI: TC=0.40/1.00 (A-B:1) , BC=0.64/1.00 (G-I:2) , WB=0.69/1.00 (D-F:1) , SSI=0.22/1.00 (F-G:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(0RY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1667	788	1987	1656

PLATE PLACEMENT TDL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (H) (INPUT = 0.90)
JSI METAL= 0.55 (H) (INPUT = 1.00)

DWG NO. TAM T39A1219
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300335	T40A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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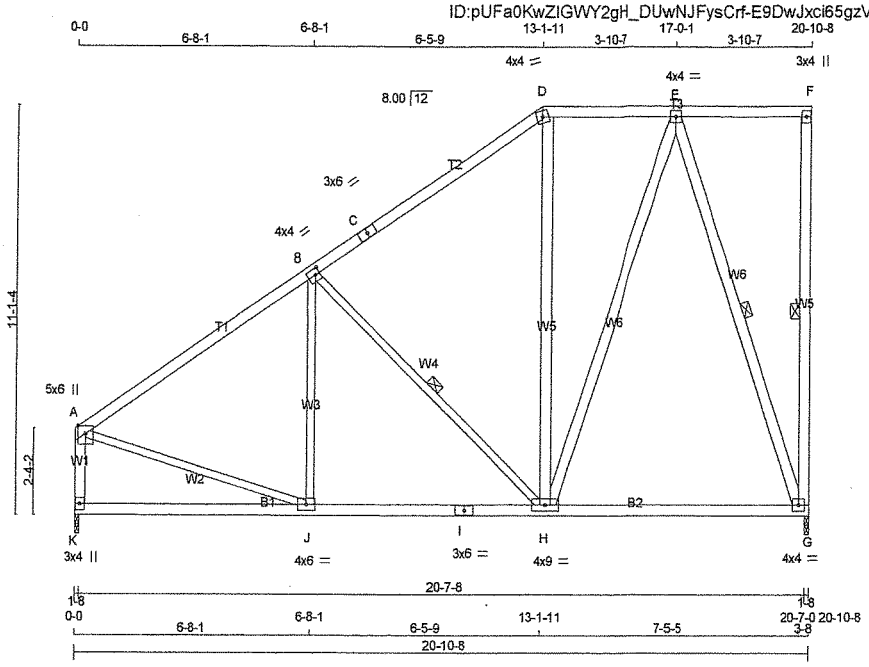


Scale = 1:62.9

TOTAL WEIGHT = 120 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
J - B	2x3	DRY	No.2	SPF
B - H	2x3	DRY	No.2	SPF
A - J	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	Edge	
B	TMWW-t	MT20	4.0	4.0	2.00	1.50
C	TS-t	MT20	3.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQR0 BRG
G	1288	0	1288	0	0	1-8	1-8
K	1288	0	1288	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	0EAD	SOIL
G	977	486 / 0	219 / 0	0 / 0	0 / 0	271 / 0	0 / 0
K	977	486 / 0	219 / 0	0 / 0	0 / 0	271 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.21 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, B-H, E-G

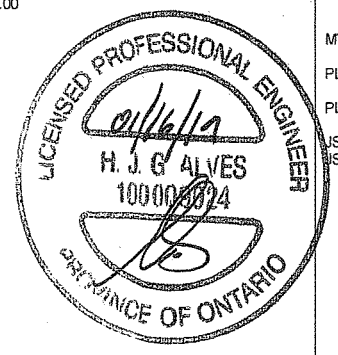
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATE0 IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	-1160 / 0	-84.9	-84.9	0.53 (1)	5.21
B-C	-752 / 0	-84.9	-84.9	0.49 (1)	6.18
C-D	-752 / 0	-84.9	-84.9	0.49 (1)	6.18
D-E	-593 / 0	-84.9	-84.9	0.17 (1)	6.25
E-F	0 / 0	-84.9	-84.9	0.21 (1)	10.00
G-F	-124 / 0	0.0	0.0	0.08 (1)	6.25
K-A	-1182 / 0	0.0	0.0	0.15 (1)	7.34
K-J	0 / 0	-38.5	-38.5	0.28 (3)	10.00
J-I	0 / 996	-38.5	-38.5	0.50 (2)	10.00
I-H	0 / 998	-38.5	-38.5	0.50 (2)	10.00
H-G	0 / 374	-38.5	-38.5	0.41 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J-B	-32 / 190	0.05 (3)
B-H	-565 / 0	0.34 (1)
H-D	0 / 155	0.03 (3)
A-J	0 / 1041	0.23 (1)
H-E	0 / 675	0.11 (1)
E-G	-1109 / 0	0.81 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.53/1.00 (A-B:1) , BC=0.50/1.00 (H-J:2) , WB=0.81/1.00 (E-G:1) , SSI=0.23/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.75 (E) (INPUT = 0.90)
SI METAL= 0.48 (A) (INPUT = 1.00)

DWG NO. TAM T1901128
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300335	T41A	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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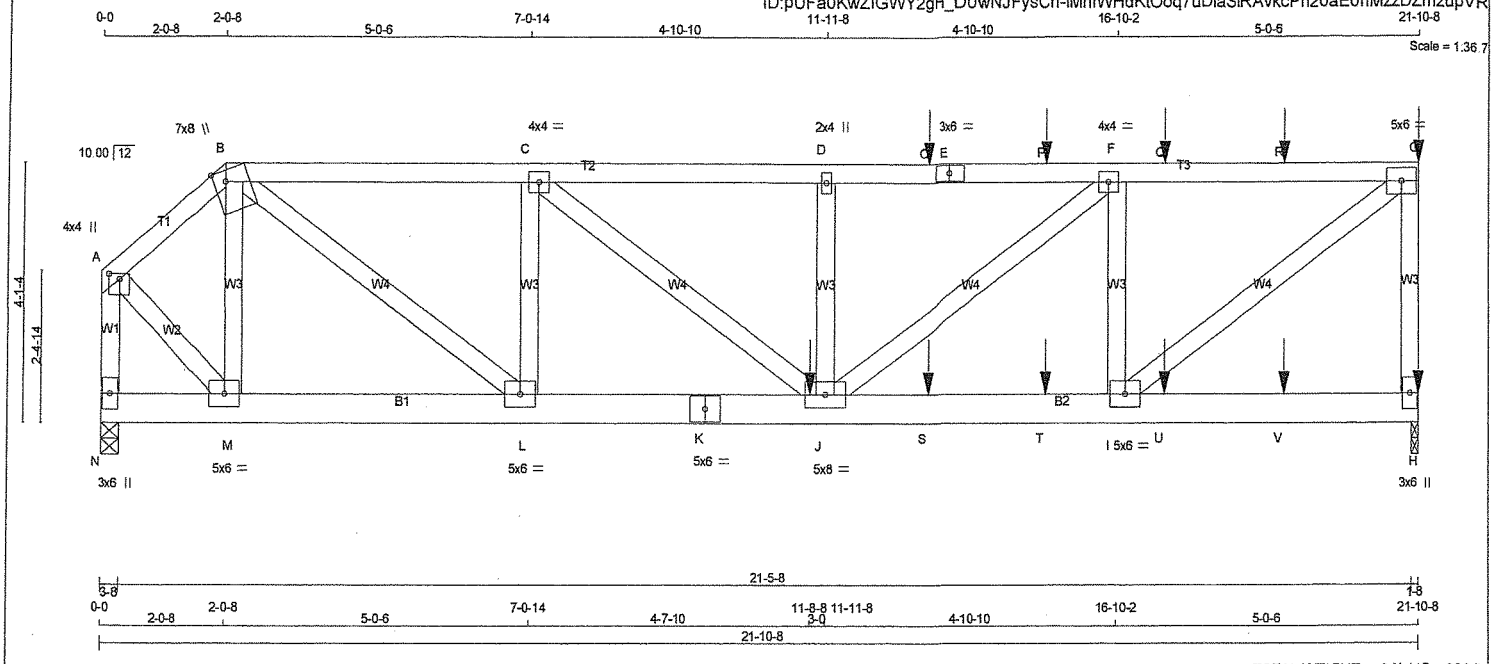

TOTAL WEIGHT = 2 X 115 = 230 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
N - A	2x4	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- B	1	12	TOP
B- E	1	12	SIDE(0.0)
E- G	1	12	SIDE(61.0)
G- H	1	12	TOP
N- A	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
N- K	2	12	TOP
K- H	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TTWW+m	MT20	7.0	8.0	Edge	2.25
C	TMWW-t	MT20	4.0	4.0		
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	6.0		
I, L, M						
J	BMWW-t	MT20	5.0	6.0		
J	BMWWW-t	MT20	5.0	8.0		
K	BS-t	MT20	5.0	6.0		
N	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	2695	0	2695	0	0	1-8	1-8
N	2049	0	2049	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	2042	1023 / 0	453 / 0	0 / 0	0 / 0	566 / 0	0 / 0
N	1550	786 / 0	336 / 0	0 / 0	0 / 0	427 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.27 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1464 / 0	-84.9	-84.9	0.04 (1)	6.25
B- C	-3121 / 0	-84.9	-84.9	0.21 (1)	5.05
C- D	-4233 / 0	-84.9	-84.9	0.25 (1)	4.43
D- O	-4233 / 0	-84.9	-84.9	0.37 (1)	4.27
O- E	-4233 / 0	-84.9	-84.9	0.37 (1)	4.27
E- P	-4233 / 0	-84.9	-84.9	0.37 (1)	4.27
P- F	-4233 / 0	-84.9	-84.9	0.37 (1)	4.27
F- Q	-2918 / 0	-84.9	-84.9	0.32 (1)	5.02
Q- R	-2918 / 0	-84.9	-84.9	0.32 (1)	5.02
R- G	-2918 / 0	-84.9	-84.9	0.32 (1)	5.02
H- G	-2513 / 0	0.0	0.0	0.31 (1)	7.14
N- A	-2048 / 0	0.0	0.0	0.13 (1)	7.72
N- M	0 / 0	-38.5	-38.5	0.04 (2)	10.00
M- L	0 / 1086	-38.5	-38.5	0.10 (1)	10.00
L- K	0 / 3122	-38.5	-38.5	0.25 (1)	10.00
K- J	0 / 3122	-38.5	-38.5	0.25 (1)	10.00
J- S	0 / 2918	-38.5	-38.5	0.25 (1)	10.00
S- T	0 / 2918	-38.5	-38.5	0.25 (1)	10.00
T- I	0 / 2918	-38.5	-38.5	0.25 (1)	10.00
I- U	0 / 0	-38.5	-38.5	0.08 (3)	10.00
U- V	0 / 0	-38.5	-38.5	0.08 (3)	10.00
V- H	0 / 0	-38.5	-38.5	0.08 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- B	-829 / 0	0.08 (1)
B- L	0 / 2528	0.22 (1)
L- C	-1307 / 0	0.12 (1)
C- J	0 / 1398	0.12 (1)
J- D	-453 / 0	0.04 (1)
J- F	0 / 1655	0.15 (1)
I- F	-1781 / 0	0.16 (1)
I- G	0 / 3632	0.32 (1)
A- M	0 / 1312	0.13 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	21-10-8	-135	-135	---	BACK	VERT	TOTAL	---	---
H	21-10-8	-70	-89	---	BACK	VERT	TOTAL	---	---
J	11-8-8	-1213	-1213	---	TOP	VERT	TOTAL	---	---
O	13-7-12	-102	-102	---	BACK	VERT	TOTAL	---	---
P	15-7-12	-102	-102	---	BACK	VERT	TOTAL	---	---
Q	17-7-12	-102	-102	---	BACK	VERT	TOTAL	---	---
R	19-7-12	-102	-102	---	BACK	VERT	TOTAL	---	---
S	13-7-12	-55	-70	---	BACK	VERT	TOTAL	---	---
T	15-7-12	-55	-70	---	BACK	VERT	TOTAL	---	---
U	17-7-12	-55	-70	---	BACK	VERT	TOTAL	---	---
V	19-7-12	-55	-70	---	BACK	VERT	TOTAL	---	---

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.37/1.00 (D-F:1) , BC=0.25/1.00 (I-J:1) , WB=0.32/1.00 (G-I:1) , SSI=0.18/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (F) (INPUT = 0.90)
JSI METAL= 0.38 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
01/16/19
H. J. G. ALVES
100090024
PROVINCE OF ONTARIO

DWG NO. TAM T1901279
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300335	T42A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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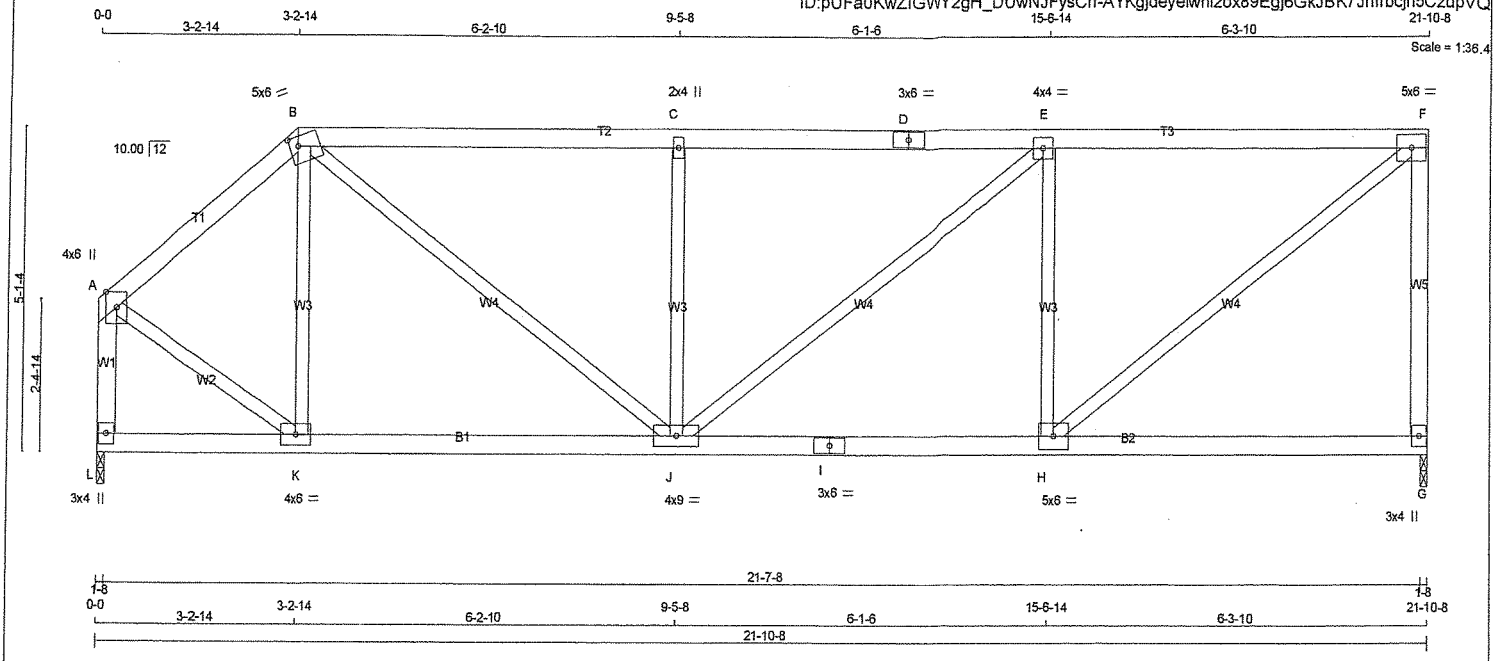


TOTAL WEIGHT = 91 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - A	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B	TTWW-m	MT20	5.0	6.0	1.75	1.75
C	TMW+w	MT20	2.0	4.0		
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMVW-t	MT20	5.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	5.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	9.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1349	0	1349	0	0	1-8	1-8
L	1349	0	1349	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1023	509 / 0	230 / 0	0 / 0	0 / 0	284 / 0	0 / 0
L	1023	509 / 0	230 / 0	0 / 0	0 / 0	284 / 0	0 / 0

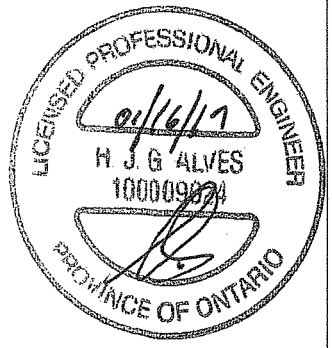
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-1054 / 0	-84.9	-84.9	0.17 (1)	5.93	K- B	-288 / 0	0.11 (1)
B- C	-1590 / 0	-84.9	-84.9	0.52 (1)	4.61	A- K	0 / 949	0.21 (1)
C- D	-1591 / 0	-84.9	-84.9	0.54 (1)	4.58	H- F	0 / 1705	0.38 (1)
D- E	-1591 / 0	-84.9	-84.9	0.54 (1)	4.58	B- J	0 / 997	0.22 (1)
E- F	-1355 / 0	-84.9	-84.9	0.52 (1)	4.91	H- E	-765 / 0	0.29 (1)
G- F	-1251 / 0	0.0	0.0	0.55 (1)	7.19	J- C	-565 / 0	0.22 (1)
L- A	-1315 / 0	0.0	0.0	0.17 (1)	7.05	J- E	0 / 300	0.07 (1)
L- K	0 / 0	-38.5	-38.5	0.18 (3)	10.00			
K- J	0 / 801	-38.5	-38.5	0.30 (2)	10.00			
J- I	0 / 1355	-38.5	-38.5	0.45 (2)	10.00			
I- H	0 / 1355	-38.5	-38.5	0.45 (2)	10.00			
H- G	0 / 0	-38.5	-38.5	0.28 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL (TL)= L/360 (0.73")
CALCULATED VERT. DEFL (TL) = L/ 999 (0.12")

CSI: TC=0.55/1.00 (F-G:1) , BC=0.45/1.00 (H-J:2) , WB=0.38/1.00 (F-H:1) , SSI=0.25/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (A) (INPUT = 0.90)
JSI METAL= 0.52 (I) (INPUT = 1.00)

DWG NO. TAM T1901280
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300335	T43A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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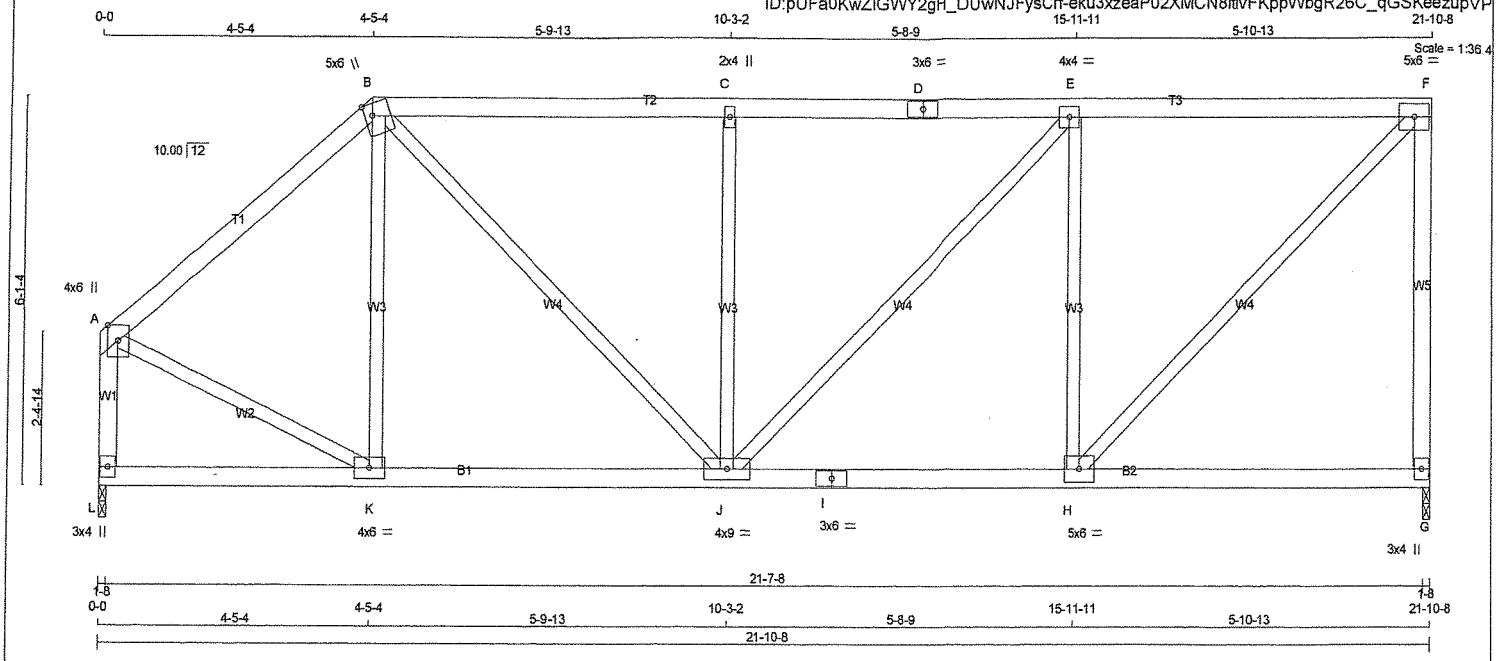

TOTAL WEIGHT = 97 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - A	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B	TTWW+m	MT20	5.0	6.0	2.25	1.50
C	TMW+w	MT20	2.0	4.0		
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMVW-t	MT20	5.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	5.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	9.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1349	0	1349	0	0	1-8	1-8
L	1349	0	1349	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1023	509 / 0	230 / 0	0 / 0	0 / 0	284 / 0	0 / 0
L	1023	509 / 0	230 / 0	0 / 0	0 / 0	284 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-1106 / 0	-84.9	-84.9	0.33 (1)	5.61	K- B	-162 / 77	0.09 (1)
B- C	-1326 / 0	-84.9	-84.9	0.43 (1)	5.08	A- K	0 / 931	0.21 (1)
C- D	-1326 / 0	-84.9	-84.9	0.45 (1)	5.05	H- F	0 / 1504	0.34 (1)
D- E	-1326 / 0	-84.9	-84.9	0.45 (1)	5.05	B- J	0 / 679	0.15 (1)
E- F	-1071 / 0	-84.9	-84.9	0.43 (1)	5.50	H- E	-801 / 0	0.47 (1)
G- F	-1258 / 0	0.0	0.0	0.91 (1)	7.17	J- C	-529 / 0	0.31 (1)
L- A	-1285 / 0	0.0	0.0	0.16 (1)	7.11	J- E	0 / 363	0.08 (1)
L- K	0 / 0	-38.5	-38.5	0.19 (3)	10.00			
K- J	0 / 845	-38.5	-38.5	0.30 (2)	10.00			
J- I	0 / 1071	-38.5	-38.5	0.39 (2)	10.00			
I- H	0 / 1071	-38.5	-38.5	0.39 (2)	10.00			
H- G	0 / 0	-38.5	-38.5	0.25 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.91/1.00 (F-G:1) , BC=0.39/1.00 (H-J:2) , WB=0.47/1.00 (E-H:1) , SSI=0.23/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (A) (INPUT = 0.90)
JSI METAL= 0.51 (A) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
01/16/19
H. J. G ALVES
100009284
PROVINCE OF ONTARIO

DWG NO. TAM 1790 1281
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300335	T44A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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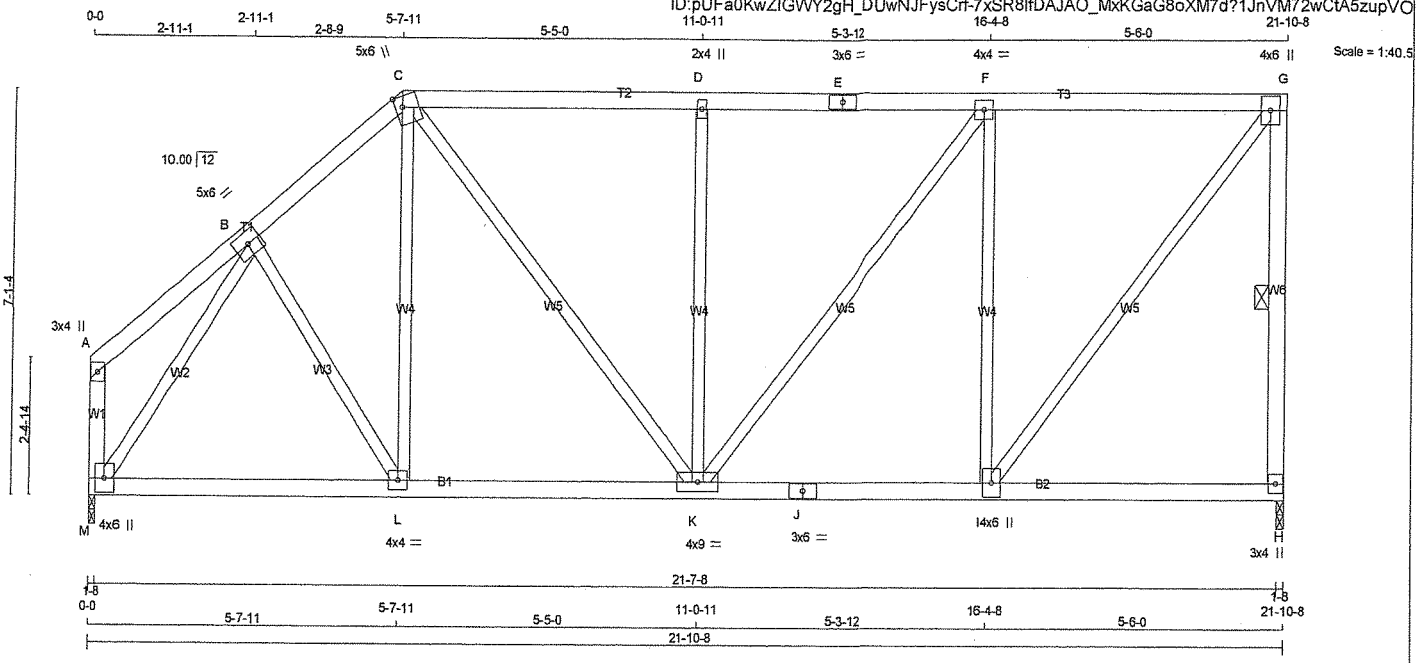


TOTAL WEIGHT = 106 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - A	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	5.0	6.0		
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW+p	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWW+t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	9.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMVW1+p	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1349	0	1349	0	0	1-8	1-8
M	1349	0	1349	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1023	509 / 0	230 / 0	0 / 0	0 / 0	284 / 0	0 / 0
M	1023	509 / 0	230 / 0	0 / 0	0 / 0	284 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.49 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

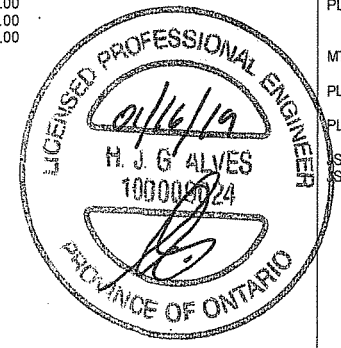
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATE0 IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES. (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 16	-84.9	-84.9	0.09 (1)	10.00	B- L	0 / 210	0.05 (2)
B- C	-1127 / 0	-84.9	-84.9	0.11 (1)	5.86	L- C	0 / 173	0.04 (3)
C- D	-1126 / 0	-84.9	-84.9	0.36 (1)	5.51	M- B	-1382 / 0	0.81 (1)
D- E	-1127 / 0	-84.9	-84.9	0.37 (1)	5.49	I- G	0 / 1382	0.31 (1)
E- F	-1127 / 0	-84.9	-84.9	0.37 (1)	5.49	C- K	0 / 440	0.10 (1)
F- G	-869 / 0	-84.9	-84.9	0.36 (1)	6.06	I- F	-836 / 0	0.73 (1)
H- G	-1265 / 0	0.0	0.0	0.29 (1)	5.73	K- D	-492 / 0	0.43 (1)
M- A	-94 / 0	0.0	0.0	0.01 (1)	7.81	K- F	0 / 419	0.09 (1)
M- L	0 / 740	-38.5	-38.5	0.32 (2)	10.00			
L- K	0 / 852	-38.5	-38.5	0.34 (2)	10.00			
K- J	0 / 869	-38.5	-38.5	0.33 (2)	10.00			
J- I	0 / 869	-38.5	-38.5	0.33 (2)	10.00			
I- H	0 / 0	-38.5	-38.5	0.22 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.37/1.00 (D-F:1) , BC=0.34/1.00 (K-L:2) , WB=0.73/1.00 (F-I:1) , SSI=0.22/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.86 (I) (INPUT = 0.90)
SI METAL= 0.39 (I) (INPUT = 1.00)

DWG NO. TAM T1901282
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300335	T45A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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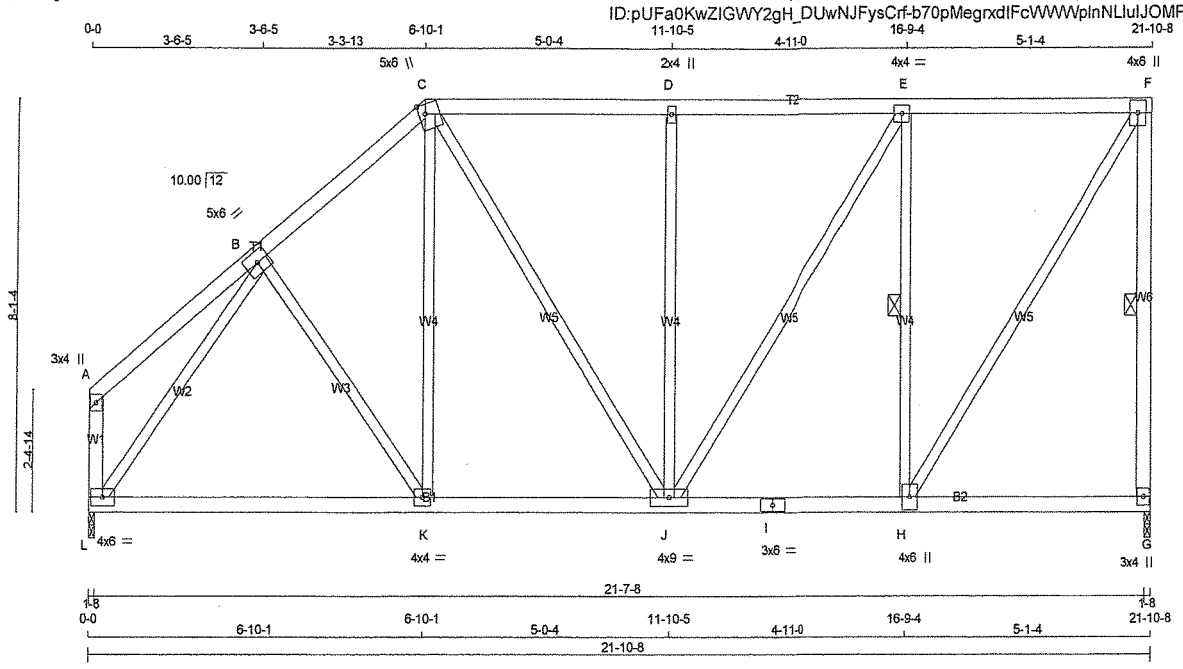


TOTAL WEIGHT = 113 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - A	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	5.0	6.0		
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMV+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWW+t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	9.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1349	0	1349	0	0	1-8	1-8
L	1349	0	1349	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1023	509 / 0	230 / 0	0 / 0	0 / 0	284 / 0	0 / 0
L	1023	509 / 0	230 / 0	0 / 0	0 / 0	284 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.81 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

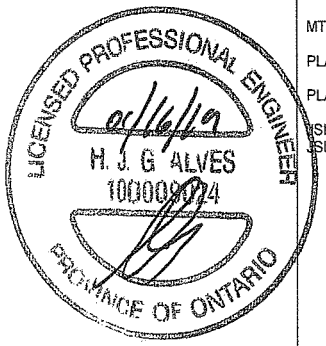
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 21	-84.9	-84.9	0.15 (1)	10.00
B- C	-1110 / 0	-84.9	-84.9	0.17 (1)	5.81
C- D	-968 / 0	-84.9	-84.9	0.30 (1)	5.93
D- E	-968 / 0	-84.9	-84.9	0.31 (1)	5.90
E- F	-716 / 0	-84.9	-84.9	0.30 (1)	6.25
G- F	-1272 / 0	0.0	0.0	0.38 (1)	5.71
L- A	-112 / 0	0.0	0.0	0.01 (1)	7.81
L- K	0 / 781	-38.5	-38.5	0.41 (2)	10.00
K- J	0 / 838	-38.5	-38.5	0.42 (2)	10.00
J- I	0 / 716	-38.5	-38.5	0.29 (2)	10.00
I- H	0 / 716	-38.5	-38.5	0.29 (2)	10.00
H- G	0 / 0	-38.5	-38.5	0.19 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- K	0 / 128	0.03 (3)
K- C	0 / 257	0.06 (3)
L- B	-1372 / 0	0.82 (1)
H- F	0 / 1309	0.29 (1)
C- J	0 / 241	0.05 (1)
H- E	-869 / 0	0.36 (1)
J- D	-455 / 0	0.58 (1)
J- E	0 / 474	0.11 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.38/1.00 (F-G:1) , BC=0.42/1.00 (J-K:2) , WB=0.82/1.00 (B-L:1) , SSI=0.20/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	165B

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SSI GRIP= 0.88 (L) (INPUT = 0.90)
SSI METAL= 0.33 (I) (INPUT = 1.00)

DWG NO. TAM T1901283
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300335	T46	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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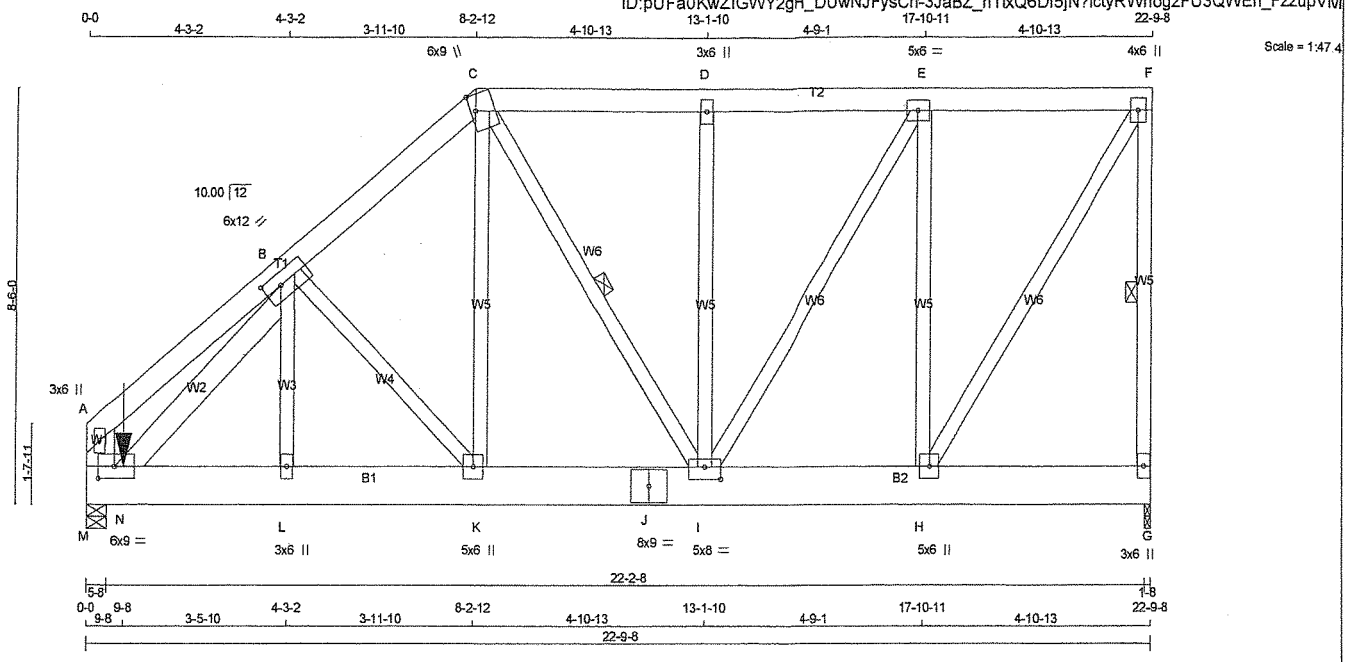


TOTAL WEIGHT = 3 X 199 = 596 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
M - A	2x8	DRY	No.2	SPF
M - J	2x10	DRY	No.2	SPF
J - G	2x10	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
M - B	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	TOP
C- F	2	12	TOP
F- G	1	12	TOP
M- A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
M- J	4	4	SIDE(1922.8)
J- G	3	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	
2x6	2	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	6.0		
B	TMWWW-t	MT20	6.0	12.0	2.75	4.25
C	TTWW+m	MT20	6.0	9.0	4.00	1.25
D	TMW+w	MT20	3.0	6.0		
E	TMWW-t	MT20	5.0	6.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	6.0		
H	BMWW+t	MT20	5.0	6.0		
I	BMWWW-t	MT20	5.0	8.0	3.00	4.00
J	BS-t	MT20	8.0	9.0		
K	BMWW+t	MT20	5.0	6.0		
L	BMW+w	MT20	3.0	6.0		
M	BMVW1-t	MT20	6.0	9.0	3.00	4.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1659	0	1659	0	0	1-8	1-8
M	8443	0	8443	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1257	631 / 0	278 / 0	0 / 0	0 / 0	348 / 0	0 / 0
M	6357	3305 / 0	1320 / 0	0 / 0	0 / 0	1732 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, C-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES. (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-2 / 14	-84.9	-84.9	0.03 (1)	10.00	
B- C	-1817 / 0	-84.9	-84.9	0.03 (1)	6.25	
C- D	-1325 / 0	-84.9	-84.9	0.04 (1)	6.25	
D- E	-1325 / 0	-84.9	-84.9	0.04 (1)	6.25	
E- F	-866 / 0	-84.9	-84.9	0.04 (1)	6.25	
G- F	-1565 / 0	0.0	0.0	0.14 (1)	6.25	
M- A	-155 / 0	0.0	0.0	0.00 (1)	7.81	
M- N	0 / 2339	-38.5	-38.5	0.52 (1)	10.00	
N- L	0 / 2339	-38.5	-38.5	0.52 (1)	10.00	
L- K	0 / 2379	-38.5	-38.5	0.16 (1)	10.00	
K- J	0 / 1397	-38.5	-38.5	0.06 (1)	10.00	
J- I	0 / 1397	-38.5	-38.5	0.06 (1)	10.00	
I- H	0 / 866	-38.5	-38.5	0.03 (1)	10.00	
H- G	0 / 0	-38.5	-38.5	0.01 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- B	0 / 1986	0.11 (1)
B- K	-1453 / 0	0.20 (1)
K- C	0 / 943	0.05 (1)
C- I	-135 / 0	0.02 (1)
I- D	-433 / 0	0.12 (1)
I- E	0 / 889	0.05 (1)
H- E	-1209 / 0	0.33 (1)
H- F	0 / 1640	0.09 (1)
M- B	-3413 / 0	0.25 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
N	9-8	-7290	-7290	—	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF	
	DL =	6.0	PSF	
BOT CH.	LL =	10.5	PSF	
	DL =	7.0	PSF	
TOTAL LOAD	=	46.8	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.76")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.76")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.14/1.00 (F-G:1) , BC=0.52/1.00 (L-M:1) , WB=0.33/1.00 (E-H:1) , SSI=0.88/1.00 (L-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

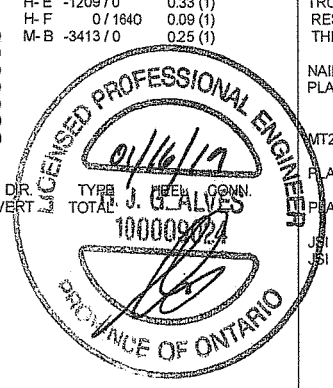
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.70 (M) (INPUT = 0.90)
JSI METAL= 0.41 (M) (INPUT = 1.00)



DWG NO. TAM 1900184
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300335	T47	1	1	TRUSS DESC.	

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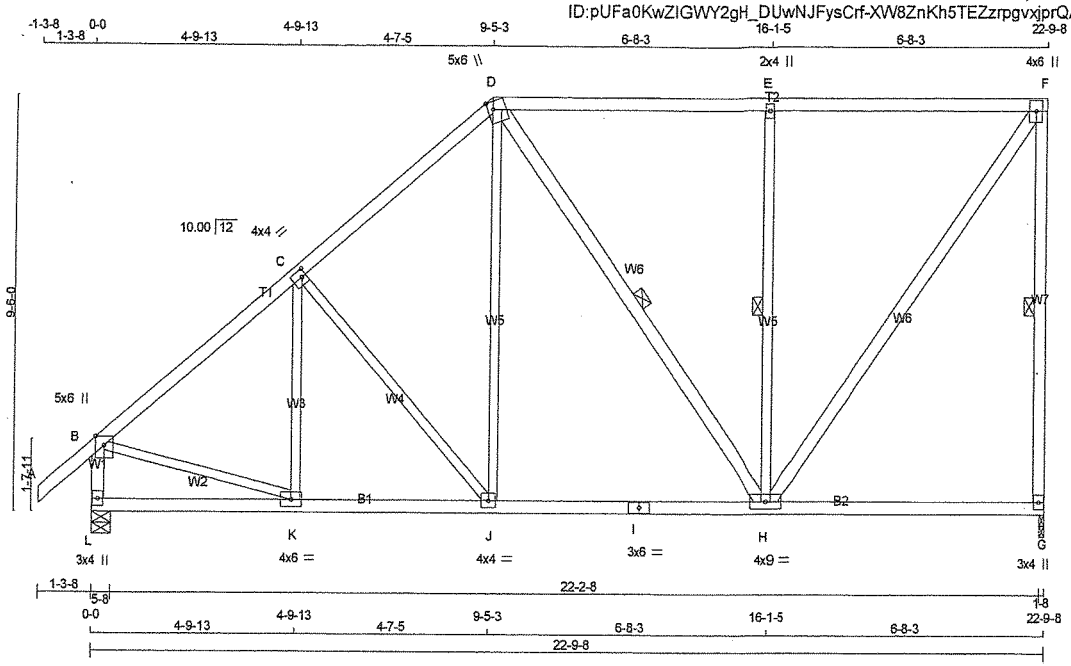


TOTAL WEIGHT = 122 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1406	0	1406	0	0	1-8	1-8
L	1523	0	1523	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1086	531 / 0	239 / 0	0 / 0	0 / 0	296 / 0	0 / 0
L	1147	595 / 0	239 / 0	0 / 0	0 / 0	313 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.15 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H, E-H.

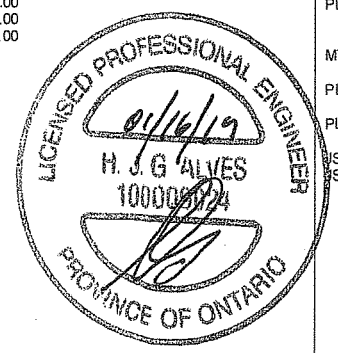
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 38	-84.9	-84.9	0.12 (1)	10.00
B- C	-1350 / 0	-84.9	-84.9	0.37 (1)	5.15
C- D	-1143 / 0	-84.9	-84.9	0.35 (1)	5.50
D- E	-793 / 0	-84.9	-84.9	0.69 (1)	5.53
E- F	-794 / 0	-84.9	-84.9	0.69 (1)	5.53
G- F	-1303 / 0	0.0	0.0	0.56 (1)	5.66
L- B	-1446 / 0	0.0	0.0	0.15 (1)	6.80
L- K	0 / 0	-38.5	-38.5	0.15 (3)	10.00
K- J	0 / 1060	-38.5	-38.5	0.31 (2)	10.00
J- I	0 / 857	-38.5	-38.5	0.44 (2)	10.00
I- H	0 / 857	-38.5	-38.5	0.44 (2)	10.00
H- G	0 / 0	-38.5	-38.5	0.34 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-91 / 105	0.04 (1)
C- J	-314 / 0	0.30 (1)
J- D	0 / 466	0.10 (2)
D- H	-109 / 0	0.08 (1)
H- E	-703 / 0	0.42 (1)
H- F	0 / 1346	0.22 (1)
B- K	0 / 1099	0.25 (1)



DESIGN CRITERIA

SPECIFIED LOADS.

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23 3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.76")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.76")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.69/1.00 (E-F:1) , BC=0.44/1.00 (H-J:2) , WB=0.42/1.00 (E-H.1) , SSI=0.28/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg

SI GRIP= 0.89 (H) (INPUT = 0.90)
SI METAL= 0.52 (B) (INPUT = 1.00)

DWG NO. TAM 97901285
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300335	T48	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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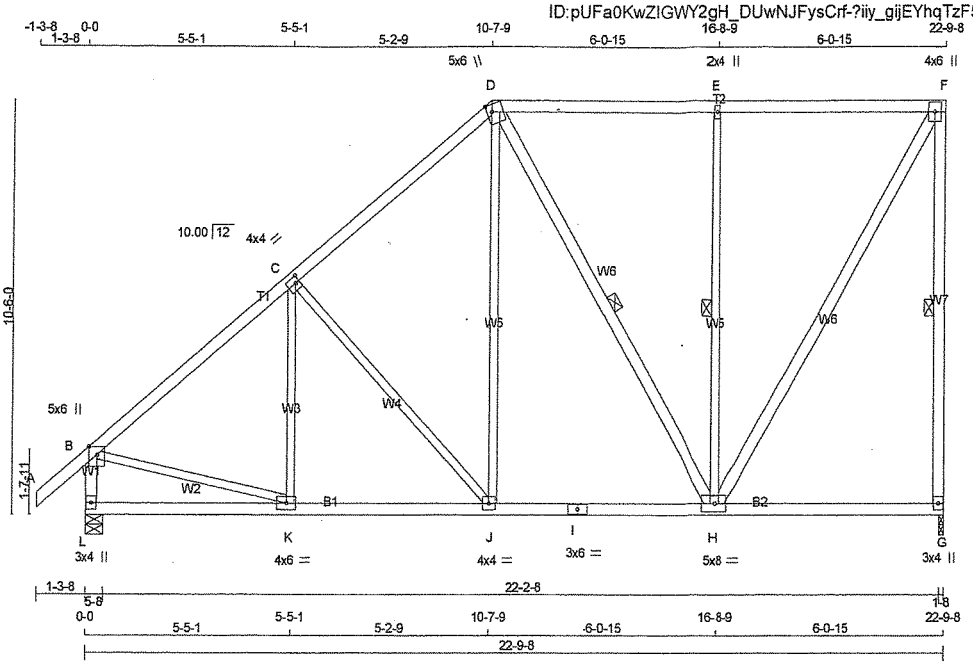


TOTAL WEIGHT = 128 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
O	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1406	0	1406	0	0	1-8	1-8
L	1523	0	1523	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1066	531 / 0	239 / 0	0 / 0	0 / 0	296 / 0	0 / 0
L	1147	595 / 0	239 / 0	0 / 0	0 / 0	313 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.99 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

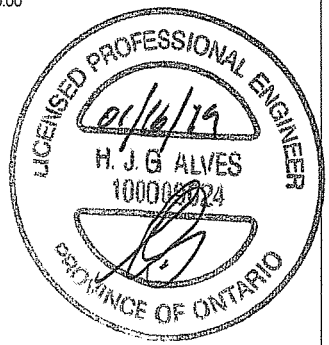
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATE0 IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 38	-84.9	-84.9	0.12 (1)	10.00	K- C	-26 / 164	0.04 (3)
B- C	-1353 / 0	-84.9	-84.9	0.49 (1)	4.99	C- J	-413 / 0	0.53 (1)
C- D	-1062 / 0	-84.9	-84.9	0.46 (1)	5.49	J- D	0 / 525	0.12 (2)
D- E	-668 / 0	-84.9	-84.9	0.55 (1)	6.24	D- H	-243 / 0	0.19 (1)
E- F	-668 / 0	-84.9	-84.9	0.55 (1)	6.24	H- E	-639 / 0	0.49 (1)
G- F	-1312 / 0	0.0	0.0	0.72 (1)	5.65	H- F	0 / 1300	0.21 (1)
L- B	-1439 / 0	0.0	0.0	0.15 (1)	6.81	B- K	0 / 1096	0.25 (1)
L- K	0 / 0	-38.5	-38.5	0.20 (3)	10.00			
K- J	0 / 1065	-38.5	-38.5	0.34 (2)	10.00			
J- I	0 / 792	-38.5	-38.5	0.37 (2)	10.00			
I- H	0 / 792	-38.5	-38.5	0.37 (2)	10.00			
H- G	0 / 0	-38.5	-38.5	0.28 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.76")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.76")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.72/1.00 (F-G:1) , BC=0.37/1.00 (H-J:2) , WB=0.53/1.00 (C-J:1) , SSI=0.25/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (K) (INPUT = 0.90)
JSI METAL= 0.52 (B) (INPUT = 1.00)

DWG NO. TAM T1901286
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300335	T49	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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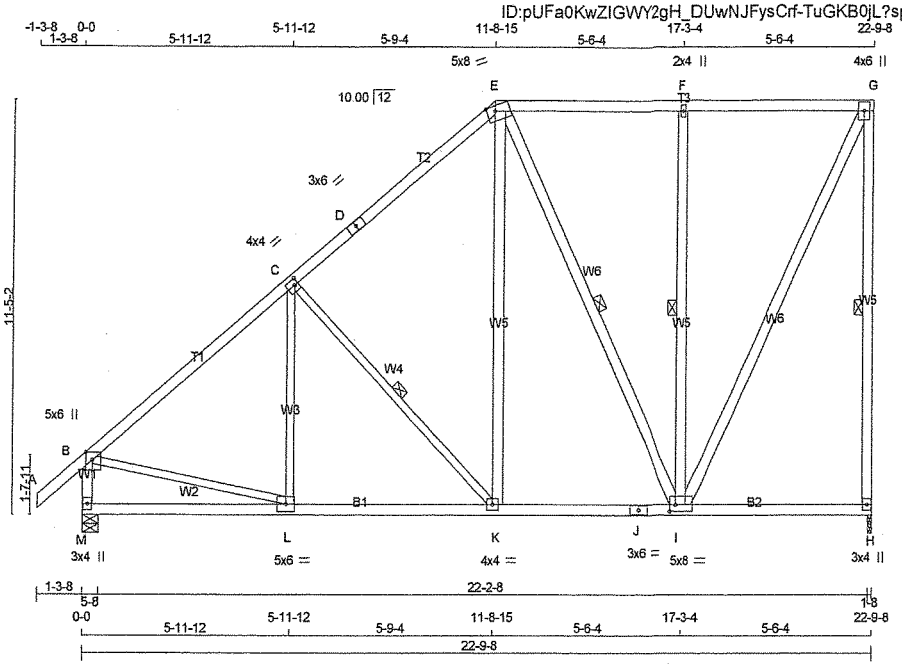


TOTAL WEIGHT = 140 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
L - C	2x3	DRY	No.2	SPF
C - K	2x3	DRY	No.2	SPF
B - L	2x3	DRY	No.2	SPF

DRY. SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.50	2.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	5.0	8.0	Edge	3.00
F	TMW+w	MT20	2.0	4.D		
G	TMVW+p	MT20	4.0	6.D		
H	BMV1+p	MT20	3.0	4.D		
I	BMWWW-t	MT20	5.0	8.0	2.25	2.00
J	BS-t	MT20	3.0	6.D		
K	BMWW-t	MT20	4.0	4.D		
L	BMWW-t	MT20	5.0	6.D		
M	BMV1+p	MT20	3.0	4.D		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REORD BRG
H	1406	0	1406	0	D	1-8	1-8
M	1523	0	1523	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1066	531 / 0	239 / 0	0 / 0	0 / 0	296 / 0	0 / 0
M	1147	595 / 0	239 / 0	0 / 0	0 / 0	313 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.08 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-H, C-K, E-I, F-I.

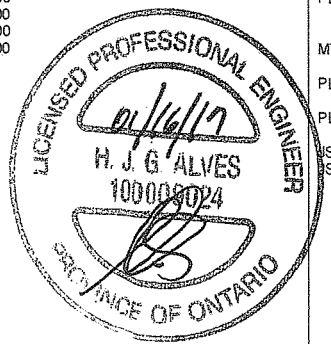
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATE0 IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES. (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT LOAD (PLF) FROM	TO	LC	CSI	MAX. UNBRAC LENGTH
A- B	0 / 38	-84.9	-84.9	0.12 (1)		10.00
B- C	-1348 / 0	-84.9	-84.9	0.44 (1)		5.08
C- D	-981 / 0	-84.9	-84.9	0.40 (1)		5.77
D- E	-981 / 0	-84.9	-84.9	0.40 (1)		5.77
E- F	-566 / 0	-84.9	-84.9	0.33 (1)		6.25
F- G	-566 / 0	-84.9	-84.9	0.33 (1)		6.25
H- G	-1320 / 0	0.0	0.0	0.91 (1)		5.63
M- B	-1431 / 0	0.0	0.0	0.15 (1)		6.82
M- L	0 / 0	-38.5	-38.5	0.26 (3)		10.00
L- K	0 / 1065	-38.5	-38.5	0.40 (2)		10.00
K- J	0 / 729	-38.5	-38.5	0.31 (2)		10.00
J- I	0 / 729	-38.5	-38.5	0.31 (2)		10.00
I- H	0 / 0	-38.5	-38.5	0.22 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- C	0 / 214	0.05 (3)
C- K	-501 / 0	0.25 (1)
K- E	0 / 581	0.09 (2)
E- I	-365 / 0	0.33 (1)
I- F	-580 / 0	0.40 (1)
I- G	0 / 1275	0.20 (1)
B- L	0 / 1090	0.25 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.76")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/380 (0.76")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.91/1.00 (G-H:1) , BC=0.40/1.00 (K-L:2) , WB=0.40/1.00 (F-I:1) , SSI=0.23/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.70 (G) (INPUT = 0.90)
SI METAL= 0.30 (I) (INPUT = 1.00)

DWG NO. TAM T1901287
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300335	T50	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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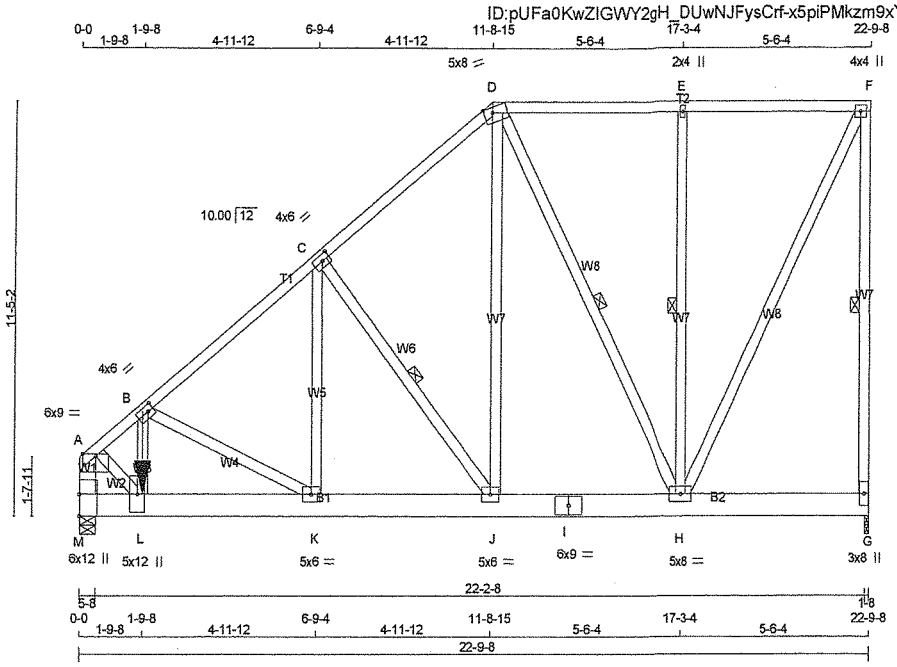

TOTAL WEIGHT = 2 X 176 = 351 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
M - A	2x6	DRY	No.2	SPF
M - I	2x8	DRY	No.2	SPF
I - G	2x8	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	TOP
D- F	1	12	TOP
F- G	1	12	TOP
M- A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
M- I	2	12	TOP
I- G	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	6.0	9.0	Edge	
B	TMWW-t	MT20	4.0	6.0	2.00	2.00
C	TMWW-t	MT20	4.0	6.0	2.00	2.75
D	TTWW-m	MT20	5.0	8.0	Edge	3.00
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	4.0		
G	BMV1+p	MT20	3.0	8.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BS-t	MT20	6.0	9.0		
J	BMWW-t	MT20	5.0	6.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMWW+t	MT20	5.0	12.0		
M	BMV1+t	MT20	6.0	12.0	7.25	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1907	0	1907	0	0	1-8	1-8
M	7286	0	7286	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1444	726 / 0	319 / 0	0 / 0	0 / 0	400 / 0	0 / 0
M	5498	2823 / 0	1169 / 0	0 / 0	0 / 0	1506 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, C-J, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	-5951 / 0	-84.9	-84.9	0.16 (1)		3.89
B- C	-2882 / 0	-84.9	-84.9	0.25 (1)		5.18
C- O	-1618 / 0	-84.9	-84.9	0.20 (1)		6.25
D- E	-824 / 0	-84.9	-84.9	0.24 (1)		6.25
E- F	-824 / 0	-84.9	-84.9	0.24 (1)		6.25
G- F	-1816 / 0	0.0	0.0	0.59 (1)		6.25
M- A	-6806 / 0	0.0	0.0	0.25 (1)		5.71
M- L	0 / 0	-38.5	-38.5	0.20 (1)		10.00
L- K	0 / 4590	-38.5	-38.5	0.44 (1)		10.00
K- J	0 / 2230	-38.5	-38.5	0.16 (1)		10.00
J- I	0 / 1237	-38.5	-38.5	0.09 (1)		10.00
I- H	0 / 1237	-38.5	-38.5	0.09 (1)		10.00
H- G	0 / 0	-38.5	-38.5	0.04 (3)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- B	0 / 2928	0.26 (1)
B- K	-2871 / 0	0.50 (1)
K- C	0 / 1678	0.15 (1)
C- J	-1682 / 0	0.29 (1)
J- D	0 / 1495	0.13 (1)
D- H	-918 / 0	0.39 (1)
H- E	-579 / 0	0.19 (1)
H- F	0 / 1834	0.16 (1)
A- L	0 / 5452	0.48 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE	TRIB.	CONN. TOTAL
L	1-9-8	-6382	-6382	---	TOP	VERT			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.76")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.76")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.59/1.00 (F-G:1) , BC=0.44/1.00 (K-L:1) , WB=0.50/1.00 (B-K:1) , SSI=0.14/1.00 (L-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SSI GRIP= 0.90 (F) (INPUT = 0.90)
SSI METAL= 0.83 (L) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
01/16/19
J. G. ALVES
100009024
PROVINCE OF ONTARIO

DWG NO. TAM T190128
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300335	T51	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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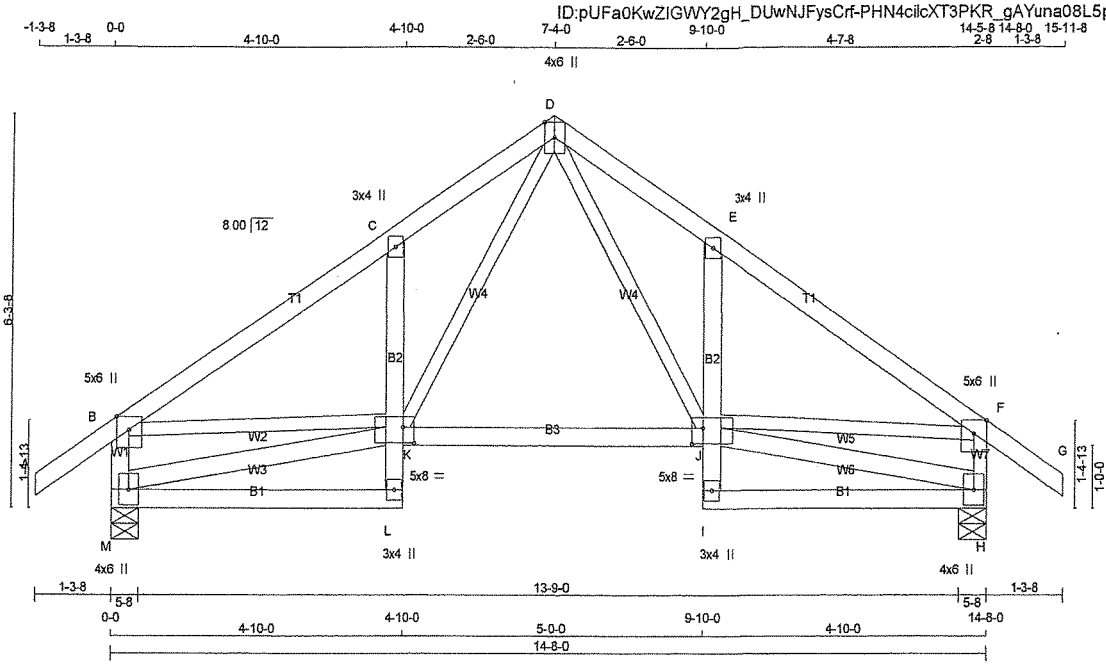


TOTAL WEIGHT = 2 X 77 = 155 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
H - F	2x3	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
L - C	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
I - E	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMV+p	MT20	3.0	4.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMV+p	MT20	3.0	4.0		
F	TMVW+p	MT20	5.0	6.0	Edge	
H	BMVW1+p	MT20	4.0	6.0		
I	BMV+p	MT20	3.0	4.0		
J	BVMWWW-I	MT20	5.0	8.0	3.00	2.25
K	BVMWWW-I	MT20	5.0	8.0	3.00	2.25
L	BMV+p	MT20	3.0	4.0		
M	BMVW1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REORD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
M	1021	0	1021	0	0	5-8	5-8
H	1021	0	1021	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	767	405 / 0	154 / 0	0 / 0	0 / 0	207 / 0	0 / 0
H	767	405 / 0	154 / 0	0 / 0	0 / 0	207 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.73 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

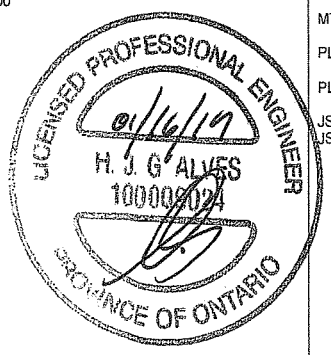
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	1	0.11 (1)	10.00
B- C	-1119 / 0	-84.9	-84.9	1	0.23 (1)	5.74
C- D	-1162 / 0	-84.9	-84.9	1	0.17 (1)	5.73
D- E	-1162 / 0	-84.9	-84.9	1	0.17 (1)	5.73
E- F	-1119 / 0	-84.9	-84.9	1	0.23 (1)	5.74
F- G	0 / 32	-84.9	-84.9	1	0.11 (1)	10.00
M- B	-926 / 0	0.0	0.0	1	0.10 (1)	7.81
H- F	-926 / 0	0.0	0.0	1	0.13 (1)	7.09
M- L	0 / 21	-38.5	-38.5	3	0.20 (3)	10.00
L- K	0 / 115	0.0	0.0	2	0.04 (2)	10.00
K- C	-395 / 0	0.0	0.0	1	0.03 (1)	7.81
K- J	0 / 640	-38.5	-38.5	2	0.32 (2)	10.00
I- J	0 / 115	0.0	0.0	2	0.04 (2)	10.00
J- E	-395 / 0	0.0	0.0	1	0.03 (1)	7.81
I- H	0 / 21	-38.5	-38.5	3	0.20 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D- J	0 / 642	0.14 (1)
K- D	0 / 642	0.14 (1)
M- K	-21 / 0	0.01 (1)
J- H	-22 / 0	0.01 (1)
B- K	0 / 946	0.21 (1)
J- F	0 / 946	0.21 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 088-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.49")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.49")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.23/1.00 (B-C:1) , BC=0.32/1.00 (J-K:2) , WB=0.21/1.00 (B-K:1) , SSI=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (F) (INPUT = 0.90)
JSI METAL= 0.48 (B) (INPUT = 1.00)

DWG NO. TAM T1901289
STRUCTURAL
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300335	T52X	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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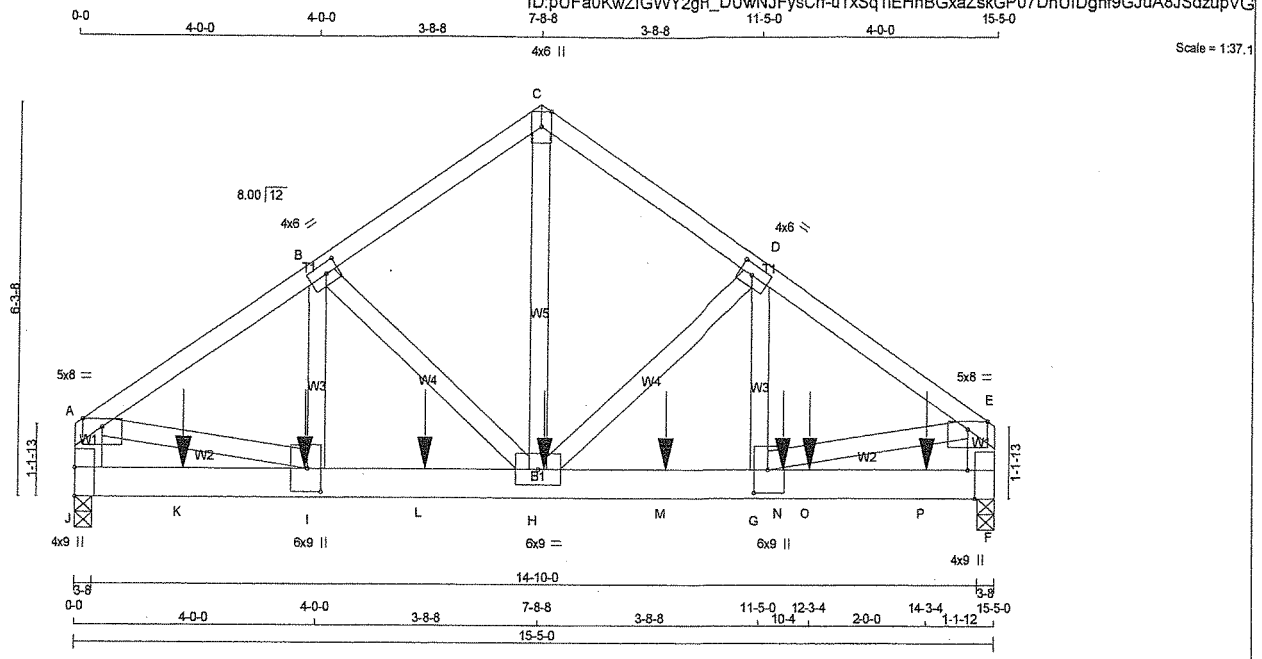

TOTAL WEIGHT = 2 X 83 = 167 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
J - A	2x6	DRY	No.2	SPF
F - E	2x6	DRY	No.2	SPF
J - F	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

OESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- E	1	12	TOP
J- A	2	12	TOP
F- E	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
J- F	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
H- C	1	6	SIDE(142.8)
B- I	1	6	SIDE(142.8)
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACEO ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST 8E APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	8.0	Edge	
B	TMWW-t	MT20	4.0	6.0	2.00	2.50
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMWW-t	MT20	4.0	6.0	2.00	2.50
E	TMVW-p	MT20	5.0	8.0	Edge	
F	BMV1+t	MT20	4.0	9.0	Edge	1.50
G	BMWW+t	MT20	6.0	9.0	4.50	2.75
H	BMWWW-t	MT20	6.0	9.0		
I	BMWW+t	MT20	6.0	9.0	4.50	2.75
J	BMV1+t	MT20	4.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATDR TD BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	5474	0	5474	0	0	3-8	3-8
F	6420	0	6420	0	0	3-8	3-8

UNFACTORED REACTIDNS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	0EAD	SOIL
J	4132	2119 / 0	880 / 0	0 / 0	0 / 0	1132 / 0	0 / 0
F	4844	2487 / 0	1030 / 0	0 / 0	0 / 0	1327 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.44 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING OIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-6611 / 0	-84.9	-84.9	0.29 (1)	3.63	H- C	0 / 5304	0.47 (1)
B- C	-5038 / 0	-84.9	-84.9	0.18 (1)	4.18	H- O	-2529 / 0	0.37 (1)
C- D	-5037 / 0	-84.9	-84.9	0.19 (1)	4.17	G- D	0 / 2587	0.23 (1)
D- E	-7252 / 0	-84.9	-84.9	0.33 (1)	3.44	B- H	-1805 / 0	0.27 (1)
J- A	-4851 / 0	0.0	0.0	0.17 (1)	6.58	I- B	0 / 1738	0.15 (1)
F- E	-5307 / 0	0.0	0.0	0.19 (1)	6.35	A- I	0 / 5605	0.50 (1)
						G- E	0 / 6147	0.54 (1)
J- K	0 / 0	-38.5	-38.5	0.39 (1)	10.00			
K- I	0 / 0	-38.5	-38.5	0.39 (1)	10.00			
I- L	0 / 5510	-38.5	-38.5	0.63 (1)	10.00			
L- H	0 / 5510	-38.5	-38.5	0.63 (1)	10.00			
H- M	0 / 6043	-38.5	-38.5	0.75 (1)	10.00			
M- G	0 / 6043	-38.5	-38.5	0.75 (1)	10.00			
G- N	0 / 0	-38.5	-38.5	0.46 (1)	10.00			
N- O	0 / 0	-38.5	-38.5	0.46 (1)	10.00			
O- P	0 / 0	-38.5	-38.5	0.46 (1)	10.00			
P- F	0 / 0	-38.5	-38.5	0.46 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN
H	7-9-12	-1249	-1249	---	FRONT	VERT	TOTAL	---	---
I	3-9-12	-1249	-1249	---	FRONT	VERT	TOTAL	---	---
K	1-9-12	-1249	-1249	---	TOP	VERT	TOTAL	---	---
L	5-9-12	-1249	-1249	---	TOP	VERT	TOTAL	---	---
M	9-9-12	-1249	-1249	---	FRONT	VERT	TOTAL	---	---
N	11-9-12	-1249	-1249	---	FRONT	VERT	TOTAL	---	---
O	12-3-4	-1249	-1249	---	FRONT	VERT	TOTAL	---	---
P	14-3-4	-1249	-1249	---	FRONT	VERT	TOTAL	---	---

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8 4 P.S.F. RAIN LOAD) EQUALS 23 3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.51")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.51")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.33/1.00 (D-E:1) , BC=0.75/1.00 (G-H:1) , WB=0.54/1.00 (E-G:1) , SSI=0.92/1.00 (F-G 1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.91 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
01/16/19
H. J. G. ALVES
100[illegible]024
PROVINCE OF ONTARIO

DWG NO. TAM 190129O
STRUCTURAL

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300335	T53	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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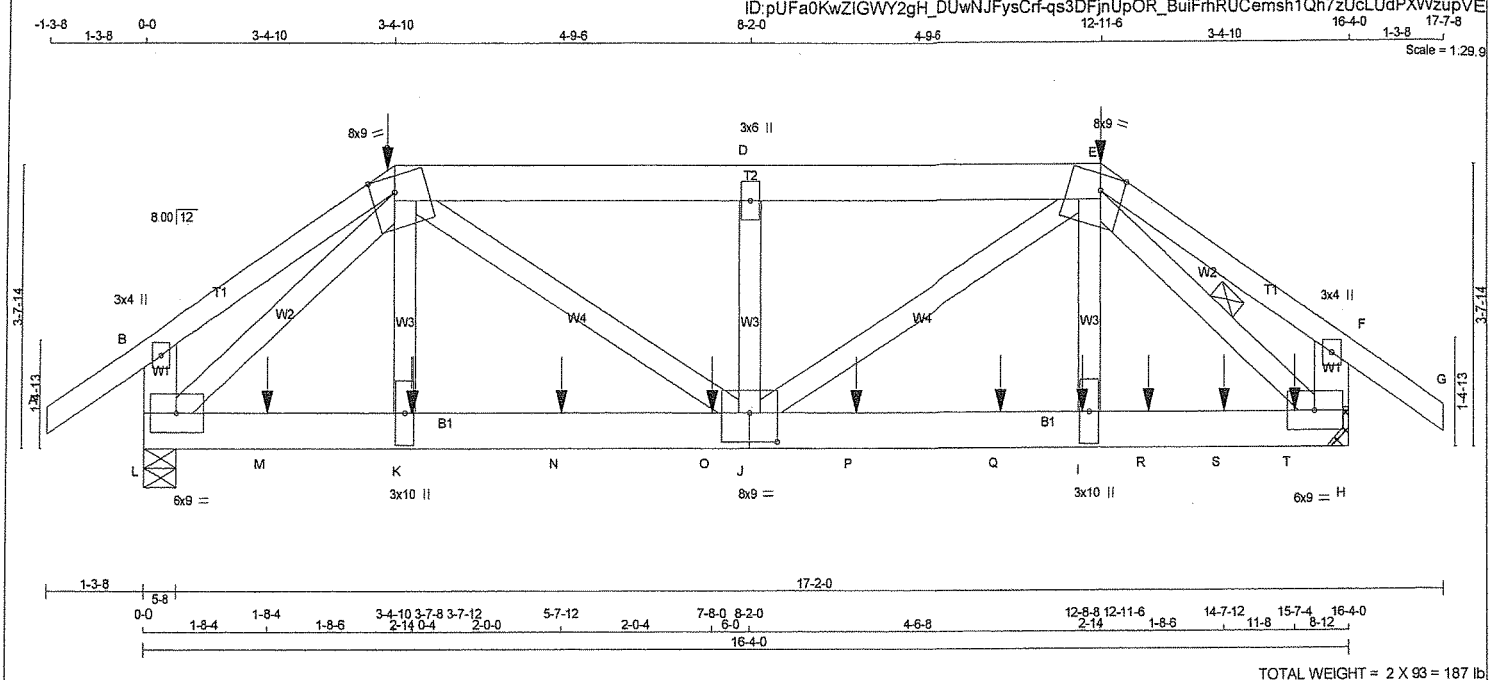


TOTAL WEIGHT = 2 X 93 = 187 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	0RY	No.2	SPF
C - E	2x6	0RY	No.2	SPF
E - G	2x4	0RY	No.2	SPF
L - B	2x6	0RY	No.2	SPF
H - F	2x6	0RY	No.2	SPF
L - J	2x6	0RY	2100F 1.8E	SPF
J - H	2x6	0RY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	0RY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS.

CHORDS #ROWS	SURFACE SPACING (IN)		LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
E- G	1	12	SIDE(61.0)
C- E	2	12	SIDE(61.0)
L- B	2	12	TOP
H- F	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
L- J	2	12	SIDE(132.2)
J- H	2	12	SIDE(188.0)
WEBS : (0.122"X3") SPIRAL NAILS			
K- C	1	6	SIDE(29.3)
I- E	1	6	SIDE(70.7)
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TTWWW-m	MT20	8.0	9.0	Edge	3.75
D	TMW+w	MT20	3.0	6.0		
E	TTWWW-m	MT20	8.0	9.0	Edge	3.75
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	6.0	9.0		
I	BMW+w	MT20	3.0	10.0		
J	BSWWW-I	MT20	8.0	9.0	4.50	4.50
K	BMW+w	MT20	3.0	10.0		
L	BMVW1-t	MT20	6.0	9.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD

DWG NO. TAM T90121 91
STRUCTURAL COMPONENT ONLY
1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQR0 BRG
L	5979	0	5979	0	0	5-8	5-8
H	7325	0	7325	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	0EA0	SOIL
L	4496	2355 / 0	920 / 0	0 / 0	0 / 0	1221 / 0	0 / 0
H	5513	2874 / 0	1138 / 0	0 / 0	0 / 0	1500 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.94 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATE0 IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.06 (1)	10.00
B- C	0 / 0	-84.9	-84.9	0.09 (1)	10.00
C- D	-8920 / 0	-84.9	-84.9	0.21 (1)	3.94
D- E	-8920 / 0	-84.9	-84.9	0.21 (1)	3.94
E- F	0 / 0	-84.9	-84.9	0.09 (1)	10.00
F- G	0 / 32	-84.9	-84.9	0.06 (1)	10.00
L- B	-260 / 0	0.0	0.0	0.01 (1)	7.81
H- F	-260 / 0	0.0	0.0	0.01 (1)	7.81
L- M	0 / 5735	-239.0	-239.0	0.28 (1)	10.00
M- K	0 / 5735	-239.0	-239.0	0.28 (1)	10.00
K- N	0 / 5870	-60.3	-60.3	0.37 (1)	10.00
N- O	0 / 5870	-60.3	-60.3	0.37 (1)	10.00
O- J	0 / 5870	-60.3	-60.3	0.37 (1)	10.00
J- P	0 / 6509	-60.3	-60.3	0.47 (1)	10.00
P- O	0 / 6509	-60.3	-60.3	0.47 (1)	10.00
Q- I	0 / 6509	-60.3	-60.3	0.47 (1)	10.00
I- R	0 / 6328	-38.5	-38.5	0.44 (1)	10.00
R- S	0 / 6328	-38.5	-38.5	0.44 (1)	10.00
S- T	0 / 6328	-38.5	-38.5	0.44 (1)	10.00
T- H	0 / 6328	-38.5	-38.5	0.44 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	0 / 3009	0.27 (1)
C- J	0 / 3679	0.33 (1)
J- D	-492 / 0	0.04 (1)
J- E	0 / 2908	0.26 (1)
I- E	0 / 4007	0.35 (1)
L- C	-7922 / 0	0.95 (1)
E- H	-8742 / 0	0.84 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-4-10	-18	-20	—	FRONT	VERT	DEAD	—	—
C	3-4-10	-80	-80	—	FRONT	VERT	SNOW	—	—
E	12-11-6	-18	-20	—	FRONT	VERT	DEAD	—	—
E	12-11-6	-80	-80	—	FRONT	VERT	SNOW	—	—
I	12-8-8	-441	-441	—	FRONT	VERT	TOTAL	—	—
K	3-7-8	-441	-441	—	FRONT	VERT	TOTAL	—	—
K	3-7-12	1095	1095	—	TOP	VERT	TOTAL	—	—

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

GIRDER TYPE: CStdGirder
START DISTANCE = 3-7-8
START SPAN CARRIED = 3-0-0
END DISTANCE = 12-8-8
END SPAN CARRIED = 3-0-0
END WALL WIDTH = 3-8
APPLIED TO FRONT SIDE OF BOTTOM CHORD
- ADDT'L LOA0S BASED ON 55 % OF GSL.

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 8-6-0
END DISTANCE = 3-4-10
END SPAN CARRIED = 8-6-0
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHDRD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TD ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

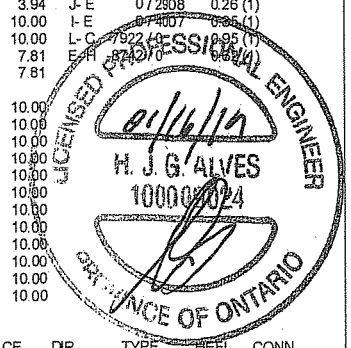
ALLOWABLE DEFL.(LL)= L/360 (0.54")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.54")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.21/1.00 (D-E:1) , BC=0.47/1.00 (I-J:1) , WB=0.95/1.00 (C-L:1) , SSI=0.70/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF



FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
M	1-8-4	-19	-24	—	FRONT	VERT	TOTAL	—	—
N	5-7-12	-676	-676	—	TOP	VERT	TOTAL	—	—
O	7-8-0	-2011	-2011	—	TOP	VERT	TOTAL	—	—
P	9-7-4	-1311	-1311	—	TOP	VERT	TOTAL	—	—
Q	11-7-4	-1311	-1311	—	TOP	VERT	TOTAL	—	—
R	13-7-4	-1311	-1311	—	TOP	VERT	TOTAL	—	—
S	14-7-12	-19	-24	—	FRONT	VERT	TOTAL	—	—
T	15-7-4	-1316	-1316	—	TOP	VERT	TOTAL	—	—

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.70 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
01/16/19
H. J. G. ALVES
100009824
PROVINCE OF ONTARIO

DWG NO. TAM T1901291
STRUCTURAL
COMPONENT ONLY 1/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300335	T54A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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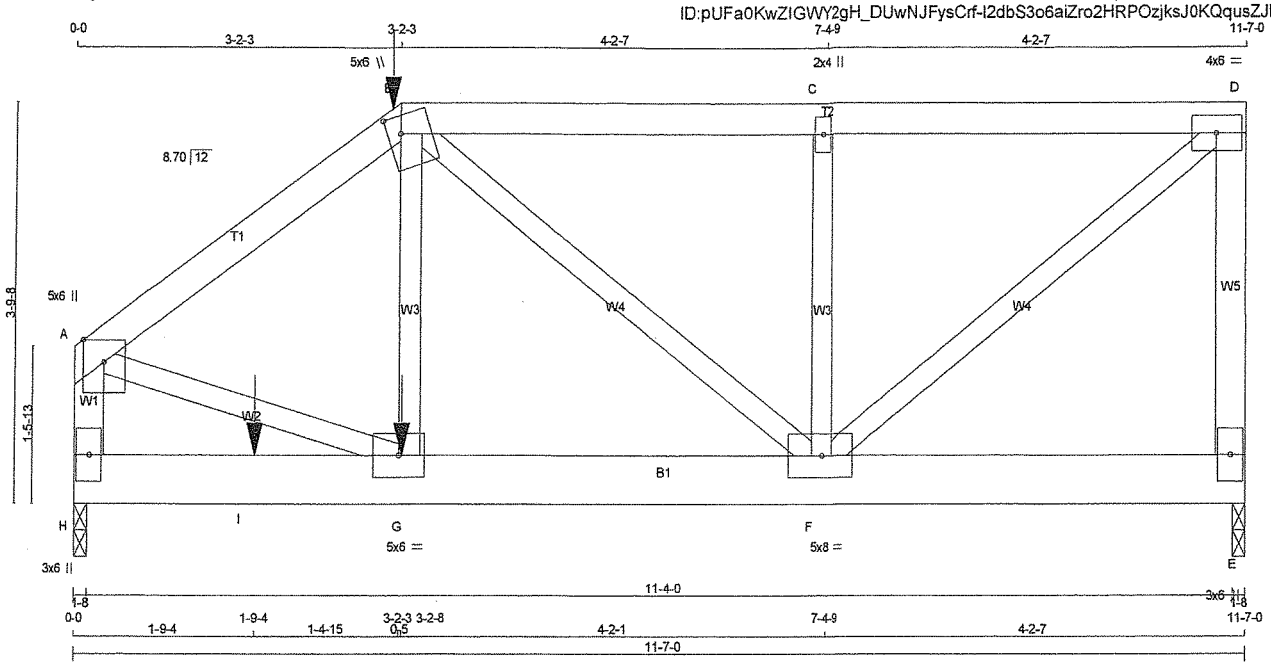


TOTAL WEIGHT = 55 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
H - E	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	Edge	
B	TTWW+m	MT20	5.0	6.0	2.00	1.50
C	TMW+w	MT20	2.0	4.0		
D	TMVW-t	MT20	4.0	6.0		
E	BMV1+p	MT20	3.0	6.0		
F	BMWWW-t	MT20	5.0	8.0		
G	BMWW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	1133	0	1133	0	0	1-8	1-8
H	1319	0	1319	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	857	436 / 0	185 / 0	0 / 0	0 / 0	236 / 0	0 / 0
H	995	513 / 0	209 / 0	0 / 0	0 / 0	272 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.41 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES. (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-1308 / 0	-84.9	-84.9	0.19 (1)	5.41	G- B	0 / 411	0.10 (2)
B- C	-1045 / 0	-84.9	-84.9	0.28 (1)	5.76	B- F	-31 / 0	0.01 (1)
C- D	-1045 / 0	-84.9	-84.9	0.28 (1)	5.76	F- C	-433 / 0	0.10 (1)
E- D	-981 / 0	0.0	0.0	0.21 (1)	7.81	F- D	0 / 1341	0.33 (1)
H- A	-1252 / 0	0.0	0.0	0.14 (1)	7.15	A- G	0 / 1111	0.28 (1)
H- I	0 / 0	-38.5	-38.5	0.05 (2)	10.00			
I- G	0 / 0	-38.5	-38.5	0.05 (2)	10.00			
G- F	0 / 1069	-83.5	-83.5	0.22 (1)	10.00			
F- E	0 / 0	-83.5	-83.5	0.11 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
B	3-2-3	-21	-24	---	FRONT	VERT	DEAD	---	---
B	3-2-3	-96	-98	---	FRONT	VERT	SNOW	---	---
G	3-2-8	-503	-503	---	TOP	VERT	TOTAL	---	---
I	1-9-4	-30	-39	---	FRONT	VERT	TOTAL	---	---

DWG NO. TAM 18901292
STRUCTURAL
COMPONENT ONLY
4/2

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS.

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

GIRDER TYPE: CStdGirder
START DISTANCE = 3-2-8
START SPAN CARRIED = 3-7-0
END DISTANCE = 11-7-0
END SPAN CARRIED = 3-7-0
END WALL WIDTH = 1-8
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.39")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.39")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.28/1.00 (C-D:1) , BC=0.22/1.00 (F-G:1) , WB=0.33/1.00 (D-F:1) , SSI=0.19/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1658

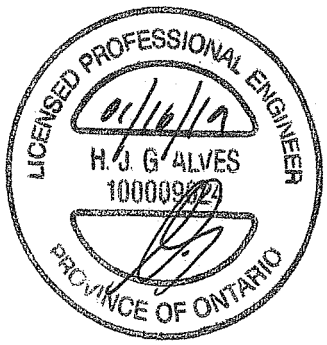
CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300335	T54A	1	1	TRUSS DESC.	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (D) (INPUT = 0.90)
JSI METAL= 0.50 (A) (INPUT = 1.00)



DWG NO. TAM 1901292
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300335	T55A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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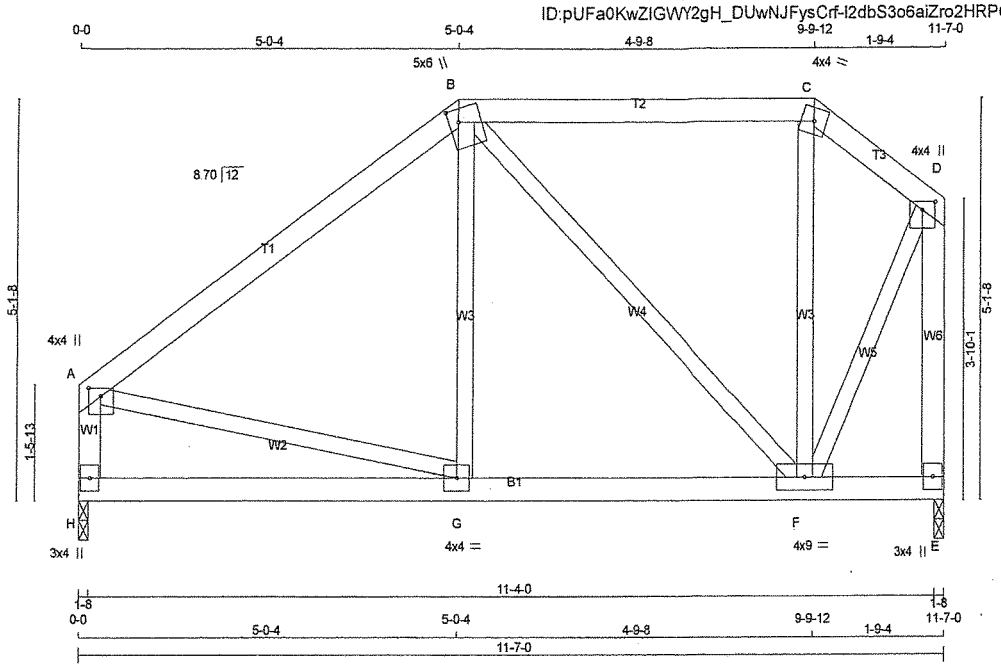


Scale = 1:29.6

TOTAL WEIGHT = 53 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
B	TTWW+m	MT20	5.0	6.0	2.00	1.50
C	TTW-m	MT20	4.0	4.0		
D	TMVW+p	MT20	4.0	4.0	1.25	2.00
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	9.0		
G	BMWW-t	MT20	4.0	4.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	714	0	714	0	0	1-8	1-8
E	714	0	714	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	542	270 / 0	122 / 0	0 / 0	0 / 0	151 / 0	0 / 0
E	542	270 / 0	122 / 0	0 / 0	0 / 0	151 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, E

BRACING

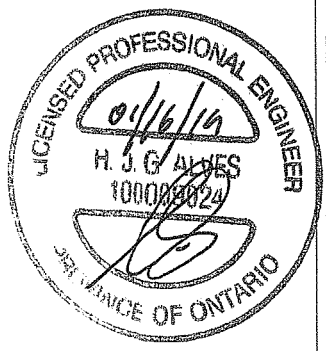
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	-547 / 0	-84.9	-84.9	0.28	(1)	6.25	G-B	0 / 226	0.05 (3)
B-C	-236 / 0	-84.9	-84.9	0.25	(1)	6.25	B-F	-299 / 0	0.24 (1)
C-D	-298 / 0	-84.9	-84.9	0.03	(1)	6.25	F-C	-103 / 46	0.04 (1)
H-A	-635 / 0	0.0	0.0	0.07	(1)	7.81	A-G	0 / 453	0.10 (1)
E-D	-697 / 0	0.0	0.0	0.17	(1)	7.81	F-D	0 / 507	0.11 (1)
H-G	0 / 0	-38.5	-38.5	0.19	(3)	10.00			
G-F	0 / 445	-38.5	-38.5	0.24	(2)	10.00			
F-E	0 / 0	-38.5	-38.5	0.07	(3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.39")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.39")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.28/1.00 (A-B:1) , BC=0.24/1.00 (F-G:2) , WB=0.24/1.00 (B-F:1) , SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.63 (F) (INPUT = 0.90)
JSI METAL= 0.17 (G) (INPUT = 1.00)

DWG NO. TAM 1901293
STRUCTURAL

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300332	T90	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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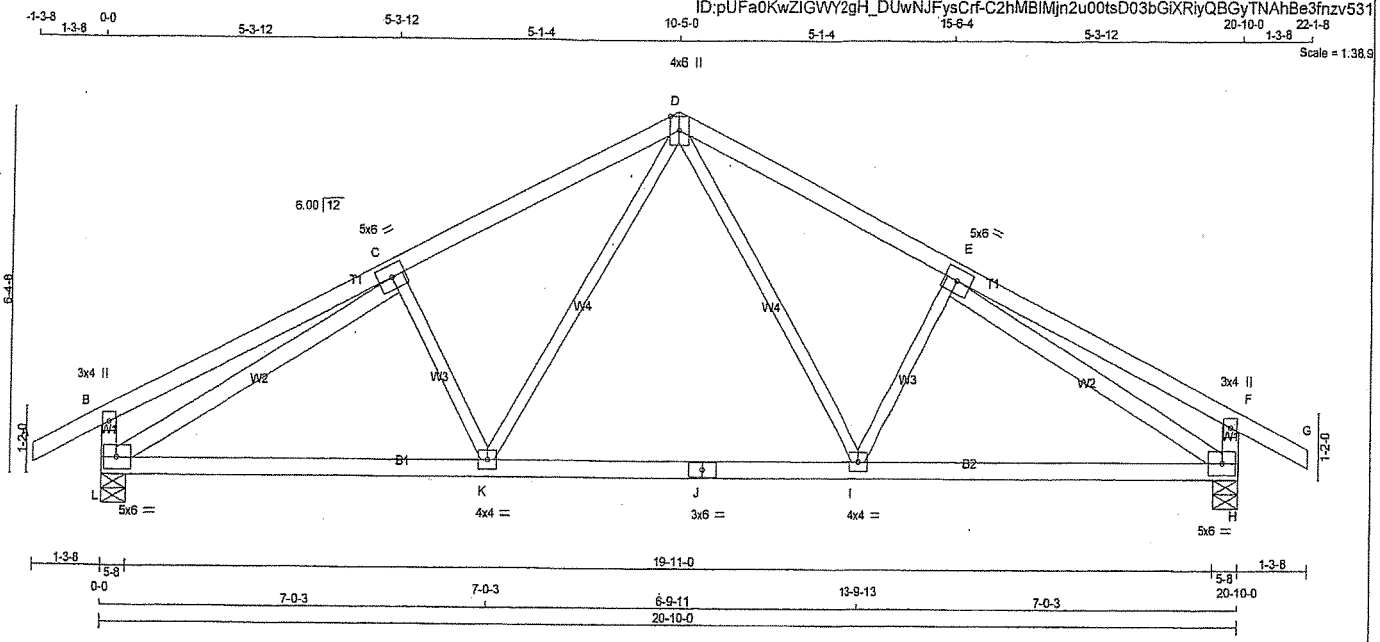


TOTAL WEIGHT = 86 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
L - C	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	5.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	5.0	6.0		
I	BMWW-t	MT2D	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMVW1-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1400	0	1400	0	0	5-8	5-8
H	1400	0	1400	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM LIVE	WIND	0EA0	SOIL
L	1054	548 / 0	219 / 0	0 / 0	0 / 0	287 / 0	0 / 0
H	1054	548 / 0	219 / 0	0 / 0	0 / 0	287 / 0	0 / 0

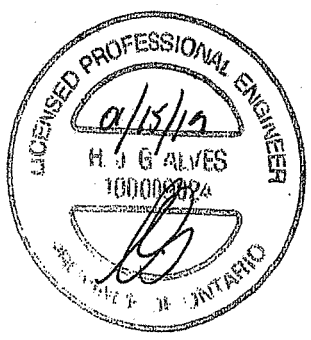
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.93 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORE0 FORCE (LBS)	MAX CSI (LC)
A- B	0 / 26	-84.9	-84.9	0.11 (1)	10.00	D- I	0 / 601	D.14 (1)
B- C	0 / 23	-84.9	-84.9	0.37 (1)	10.00	I- E	-264 / 27	0.07 (1)
C- D	-1576 / 0	-84.9	-84.9	0.32 (1)	4.93	K- D	0 / 601	0.14 (1)
D- E	-1576 / 0	-84.9	-84.9	0.32 (1)	4.93	C- K	-264 / 27	0.07 (1)
E- F	0 / 23	-84.9	-84.9	0.37 (1)	10.00	L- C	-1816 / 0	0.87 (1)
F- G	0 / 28	-84.9	-84.9	0.11 (1)	10.00	E- H	-1816 / 0	0.87 (1)
L- B	-284 / 0	0.0	0.0	0.03 (1)	7.81			
H- F	-284 / 0	0.0	0.0	0.03 (1)	7.81			
L- K	0 / 1512	-38.5	-38.5	0.52 (2)	10.00			
K- J	0 / 1095	-38.5	-38.5	0.46 (2)	10.00			
J- I	0 / 1095	-38.5	-38.5	0.46 (2)	10.00			
I- H	0 / 1512	-38.5	-38.5	0.52 (2)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOA0

ALLOWABLE DEFL.(LL)= L/360 (0.69")
CALCULATED VERT. 0EFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.69")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.37/1.00 (E-F:1) , BC=0.52/1.00 (H-I:2) , WB=0.87/1.00 (C-L:1) , SSI=0.19/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (E) (INPUT = 0.90)
JSI METAL= 0.45 (C) (INPUT = 1.00)

DWG NO. TAM 1190491
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300332	G90	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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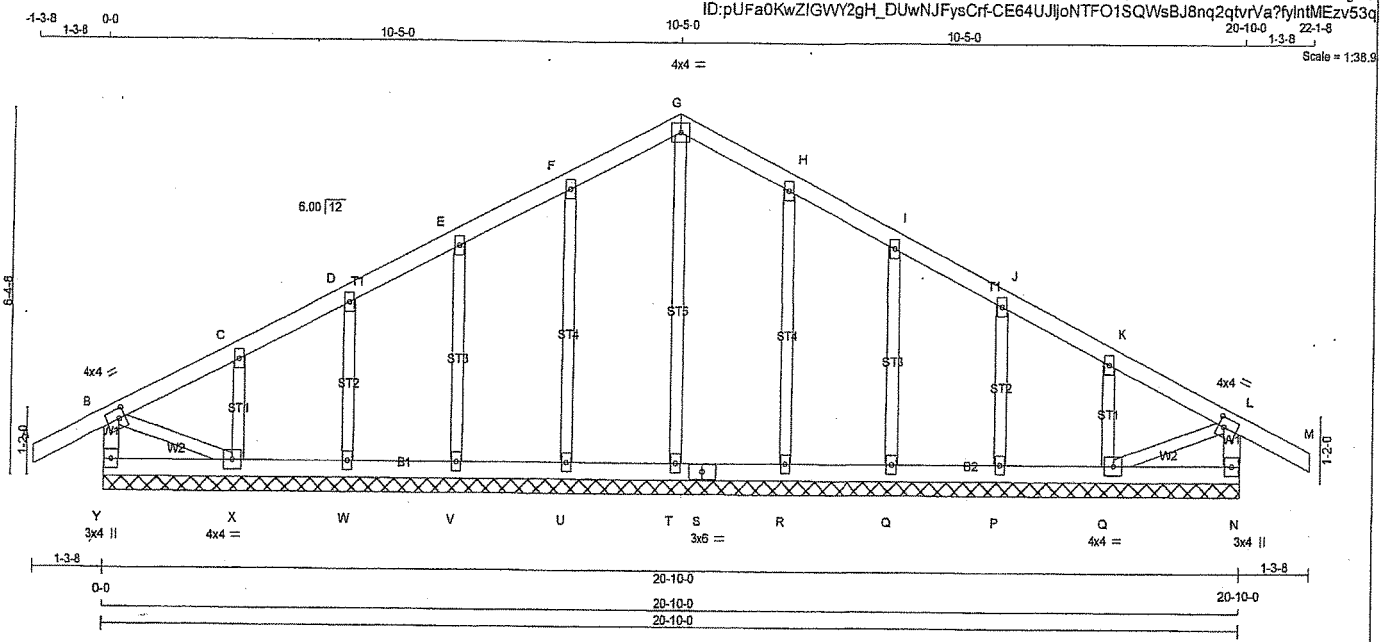


TOTAL WEIGHT = 85 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
Y - B	2x4	DRY	No.2	SPF
A - G	2x4	DRY	No.2	SPF
G - M	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
Y - S	2x4	DRY	No.2	SPF
S - N	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C, D, E, F, H, I, J, K						
C	TMW+w	MT20	2.0	4.0		
G	TTW-p	MT20	4.0	4.0		
L	TMVW-t	MT20	4.0	4.0	2.00	1.25
N	BMV1+p	MT20	3.0	4.0		
O	BMWW1-t	MT20	4.0	4.0		
P, O, R, T, U, V, W						
P	BMW1+w	MT20	2.0	4.0		
S	BS-t	MT20	3.0	6.0		
X	BMWW1-t	MT20	4.0	4.0		
Y	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

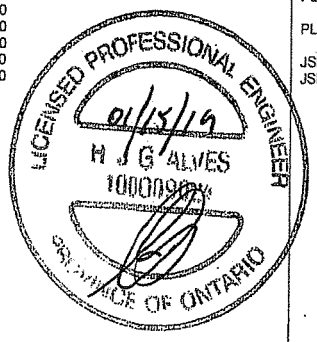
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
Y- B	-216 / 0	0.0	0.0	0.02 (1)		7.81
A- B	0 / 26	-84.9	-84.9	0.11 (1)		10.00
B- C	-18 / 0	-84.9	-84.9	0.06 (1)		6.25
C- D	-23 / 0	-84.9	-84.9	0.06 (1)		6.25
D- E	-14 / 0	-84.9	-84.9	0.04 (1)		6.25
E- F	-10 / 0	-84.9	-84.9	0.05 (1)		6.25
F- G	-17 / 0	-84.9	-84.9	0.05 (1)		6.25
G- H	-17 / 0	-84.9	-84.9	0.05 (1)		6.25
H- I	-10 / 0	-84.9	-84.9	0.05 (1)		6.25
I- J	-14 / 0	-84.9	-84.9	0.04 (1)		6.25
J- K	-23 / 0	-84.9	-84.9	0.06 (1)		6.25
K- L	-18 / 0	-84.9	-84.9	0.06 (1)		6.25
L- M	0 / 26	-84.9	-84.9	0.11 (1)		10.00
N- L	-216 / 0	0.0	0.0	0.02 (1)		7.81
Y- X	0 / 0	-38.5	-38.5	0.04 (3)		10.00
X- W	0 / 17	-38.5	-38.5	0.04 (3)		10.00
W- V	0 / 13	-38.5	-38.5	0.03 (3)		10.00
V- U	0 / 11	-38.5	-38.5	0.03 (3)		10.00
U- T	0 / 8	-38.5	-38.5	0.02 (3)		10.00
T- S	0 / 8	-38.5	-38.5	0.02 (3)		10.00
S- R	0 / 8	-38.5	-38.5	0.02 (3)		10.00
R- O	0 / 11	-38.5	-38.5	0.03 (3)		10.00
Q- P	0 / 13	-38.5	-38.5	0.03 (3)		10.00
P- O	0 / 17	-38.5	-38.5	0.04 (3)		10.00
O- N	0 / 0	-38.5	-38.5	0.04 (3)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- G	-133 / 0	0.09 (1)
U- F	-192 / 0	0.08 (1)
V- E	-166 / 0	0.05 (1)
W- D	-157 / 0	0.03 (1)
X- C	-210 / 0	0.03 (1)
R- H	-192 / 0	0.08 (1)
O- I	-166 / 0	0.05 (1)
P- J	-157 / 0	0.03 (1)
O- K	-210 / 0	0.03 (1)
B- X	0 / 25	0.01 (1)
O- L	0 / 25	0.01 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11/1.00 (A-B:1) , BC=0.04/1.00 (O-P:3) , WB=0.09/1.00 (G-T:1) , SSI=0.08/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (B) (INPUT = 0.90)
JSI METAL= 0.09 (C) (INPUT = 1.00)

DWG NO. TAM 190172
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300330	G100	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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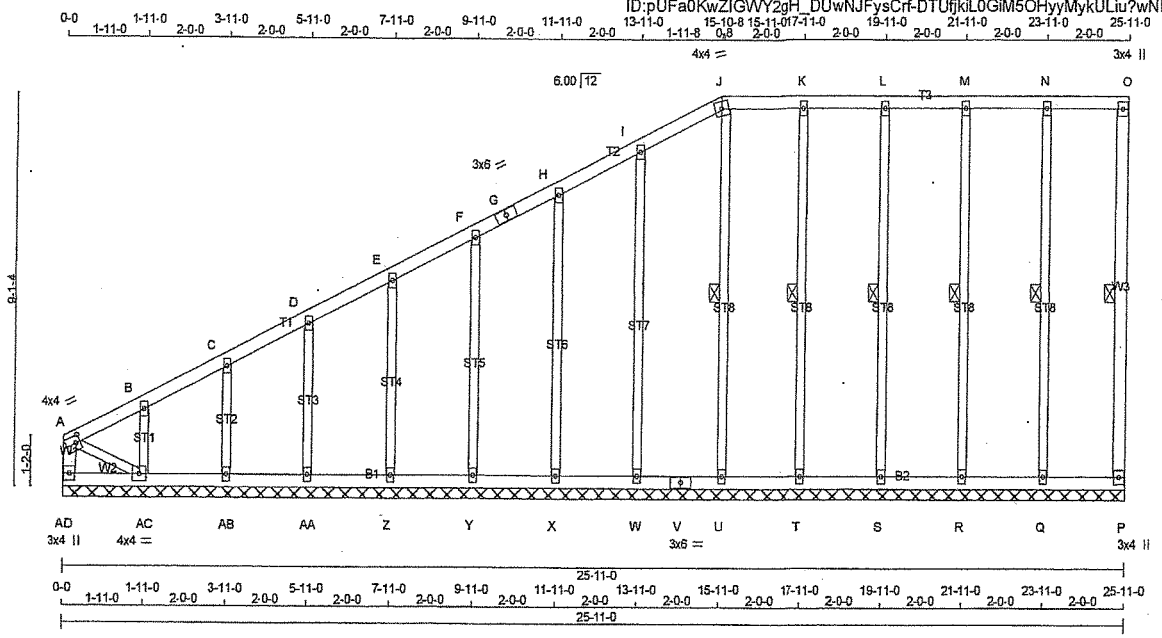


TOTAL WEIGHT = 2 X 134 = 268 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
J - O	2x4	DRY	No.2	SPF
P - O	2x4	DRY	No.2	SPF
AD- A	2x4	ORY	No.2	SPF
AD- V	2x4	DRY	No.2	SPF
V - P	2x4	ORY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

ORY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0	2.00	1.25
B, C, D, E, F, H, I, K, L, M, N						
B	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
J	TTW-m	MT20	4.0	4.0		
O	TMV+p	MT20	3.0	4.0		
P	BMV1+p	MT20	3.0	4.0		
Q, R, S, T, U, W, X, Y, Z, AA, AB						
O	BMW1+w	MT20	2.0	4.0		
V	BS-t	MT20	3.0	6.0		
AC	BMWW1-t	MT20	4.0	4.0		
AD	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF O-P, N-O, M-R, L-S, K-T, J-U.

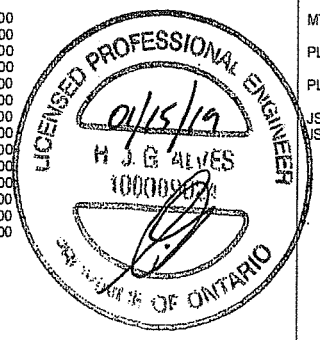
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-17 / 0	-84.9	-84.9	0.04 (1)		6.25
B- C	-19 / 0	-84.9	-84.9	0.04 (1)		6.25
C- D	-13 / 0	-84.9	-84.9	0.04 (1)		6.25
D- E	-10 / 0	-84.9	-84.9	0.04 (1)		6.25
E- F	-7 / 0	-84.9	-84.9	0.04 (1)		10.00
F- G	-6 / 0	-84.9	-84.9	0.04 (1)		10.00
G- H	-6 / 0	-84.9	-84.9	0.04 (1)		10.00
H- I	-2 / 0	-84.9	-84.9	0.05 (1)		10.00
I- J	-9 / 0	-84.9	-84.9	0.05 (1)		10.00
J- K	0 / 0	-84.9	-84.9	0.05 (1)		10.00
K- L	0 / 0	-84.9	-84.9	0.05 (1)		10.00
L- M	0 / 0	-84.9	-84.9	0.04 (1)		10.00
M- N	0 / 0	-84.9	-84.9	0.04 (1)		10.00
N- O	0 / 0	-84.9	-84.9	0.04 (1)		10.00
P- O	-70 / 0	0.0	0.0	0.03 (1)		6.25
AD- A	-83 / 0	0.0	0.0	0.01 (1)		7.81
AD-AC	0 / 0	-38.5	-38.5	0.03 (3)		10.00
AC-AB	0 / 16	-38.5	-38.5	0.03 (3)		10.00
AB-AA	0 / 12	-38.5	-38.5	0.02 (3)		10.00
AA- Z	0 / 9	-38.5	-38.5	0.02 (3)		10.00
Z- Y	0 / 7	-38.5	-38.5	0.02 (3)		10.00
Y- X	0 / 5	-38.5	-38.5	0.02 (3)		10.00
X- W	0 / 3	-38.5	-38.5	0.02 (3)		10.00
W- V	0 / 2	-38.5	-38.5	0.02 (3)		10.00
V- U	0 / 2	-38.5	-38.5	0.02 (3)		10.00
U- T	0 / 0	-38.5	-38.5	0.02 (3)		10.00
T- S	0 / 0	-38.5	-38.5	0.02 (3)		10.00
S- R	0 / 0	-38.5	-38.5	0.02 (3)		10.00
R- Q	0 / 0	-38.5	-38.5	0.03 (3)		10.00
O- P	0 / 0	-38.5	-38.5	0.03 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- N	-186 / 0	0.10 (1)
R- M	-169 / 0	0.09 (1)
S- L	-168 / 0	0.09 (1)
T- K	-191 / 0	0.10 (1)
U- J	-138 / 0	0.07 (1)
W- I	-190 / 0	0.24 (1)
X- H	-165 / 0	0.15 (1)
Y- F	-169 / 0	0.10 (1)
Z- E	-168 / 0	0.07 (1)
AA- D	-169 / 0	0.04 (1)
AB- C	-165 / 0	0.03 (1)
AC- B	-180 / 0	0.03 (1)
A-AC	0 / 25	0.01 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.05/1.00 (I-J:1) , BC=0.03/1.00 (O-R:3) , WB=0.24/1.00 (I-W:1) , SSI=0.08/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (A) (INPUT = 0.90)
JSI METAL= 0.08 (I) (INPUT = 1.00)

DWG NO. TAM 7190473
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300333	T302Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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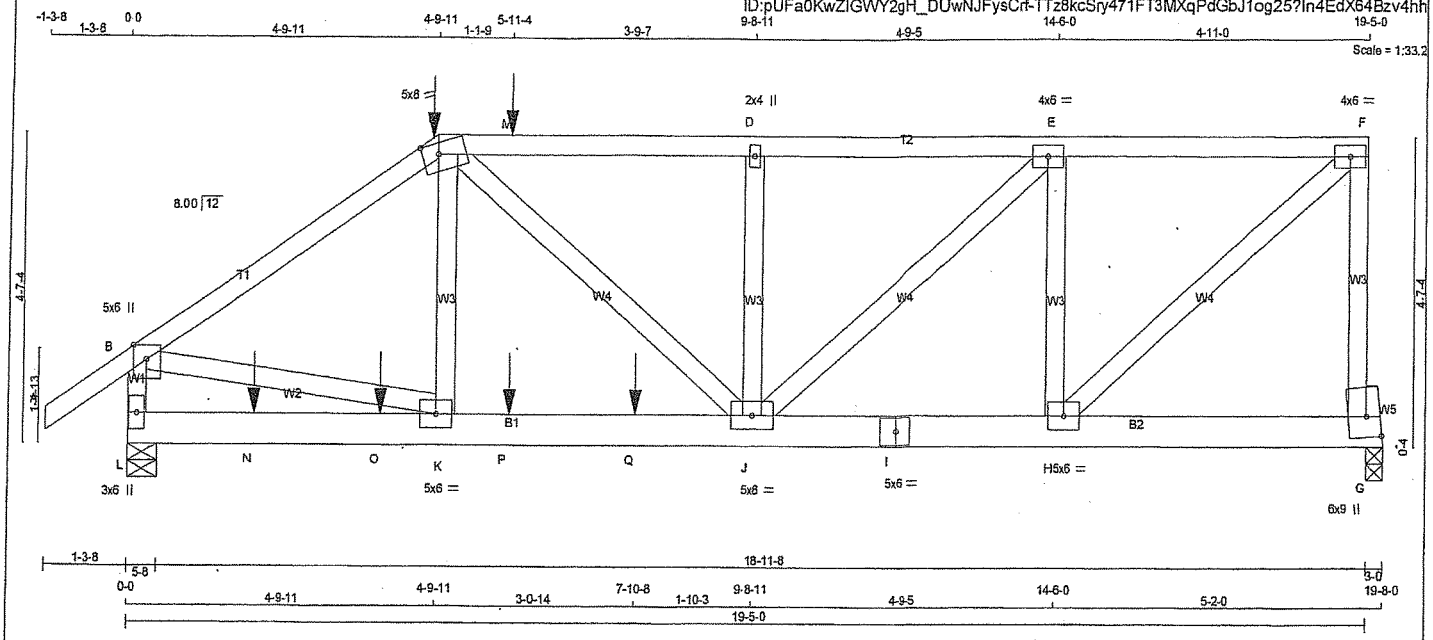


TOTAL WEIGHT = 2 X 105 = 210 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
I - G	2x6	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF

ORY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FDLLOWS:

CHOROS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- F	1	12	SIDE(61.0)
F- G	1	12	TOP
L- B	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
L- I	2	12	SIDE(0.0)
I- G	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOAOED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIEO ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW-m	MT20	5.0	8.0	2.00	3.00
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	6.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMVW1m	MT20	6.0	9.0	2.50	3.75
H	BMWW-t	MT20	5.0	6.0		
I	BS-t	MT20	5.0	6.0		
J	BMWWW-t	MT20	5.0	8.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EOGE OF CHORO.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO 8E VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTOREO GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	2044	0	2044	0	0	3-0	3-0
L	2848	0	2848	0	0	5-8	5-8

UNFACTORED REACTIDNS

JT	1ST LCASE COMBINEO	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	0EAO	SOIL
G	1541	795 / 0	324 / 0	0 / 0	0 / 0	421 / 0	0 / 0
L	2141	1123 / 0	437 / 0	0 / 0	0 / 0	581 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.75 FT.
MAX. UNBRAOED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIEO.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

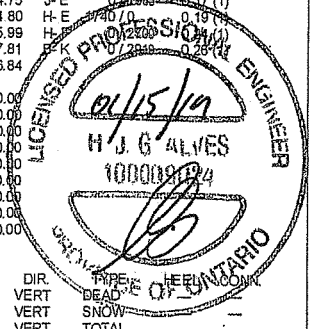
LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	FORCE (LBS)	FACTOREO VERT LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	FORCE (LBS)	MAX. FACTORED MAX CSI (LC)
A- B	0 / 32	-84.9	-84.9	0.06 (1)	10.00	K- C	0 / 577	0.05 (2)
B- C	-3444 / 0	-84.9	-84.9	0.26 (1)	4.80	C- J	0 / 831	0.07 (1)
C- M	-3510 / 0	-84.9	-84.9	0.26 (1)	4.75	J- D	-479 / 0	0.05 (1)
M- D	-3510 / 0	-84.9	-84.9	0.26 (1)	4.75	J- E	0 / 1983	0.17 (1)
D- E	-3510 / 0	-84.9	-84.9	0.22 (1)	4.80	H- E	-1740 / 0	0.19 (1)
E- F	-2047 / 0	-84.9	-84.9	0.16 (1)	5.99	H- F	0 / 2209	0.24 (1)
G- F	-1931 / 0	0.0	0.0	0.31 (1)	7.81	K- H	0 / 2849	0.26 (1)
L- B	-2813 / 0	0.0	0.0	0.16 (1)	6.84			
L- N	0 / 0	-38.5	-38.5	0.21 (1)	10.00			
N- O	0 / 0	-38.5	-38.5	0.21 (1)	10.00			
O- K	0 / 0	-38.5	-38.5	0.21 (1)	10.00			
K- P	0 / 2879	-38.5	-38.5	0.69 (1)	10.00			
P- Q	0 / 2879	-38.5	-38.5	0.69 (1)	10.00			
Q- J	0 / 2879	-38.5	-38.5	0.69 (1)	10.00			
J- I	0 / 2047	-38.5	-38.5	0.36 (1)	10.00			
I- H	0 / 2047	-38.5	-38.5	0.36 (1)	10.00			
H- G	0 / 0	-38.5	-38.5	0.06 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN
C	4-9-11	-71	-80	—	FRONT	VERT	DEAD	—	—
C	4-9-11	-330	-330	—	FRDNT	VERT	SNOW	—	—
M	5-11-4	-134	-134	—	FRONT	VERT	TOTAL	—	—
N	1-11-4	-70	-89	—	FRONT	VERT	TOTAL	—	—
O	3-11-4	-70	-89	—	FRDNT	VERT	TOTAL	—	—
P	5-11-4	-70	-89	—	FRONT	VERT	TOTAL	—	—
Q	7-10-8	-1635	-1635	—	TOP	VERT	TOTAL	—	—



DWG NO. TAM T1901174
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGEO BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIDNS WERE MADE

SPECIFIED LOADS:

TDP	CH.	LL =	23.3	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	10.5	PSF
		DL =	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLDPE OF 6.00/12

*** NON STANDARO GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIEO TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLDWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. OEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.31/1.00 (F-G:1) , BC=0.69/1.00 (J-K:1) , WB=0.26/1.00 (B-K:1) , SSI=0.37/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANIDN LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Oeg.

JSI GRIP= 0.82 (B) (INPUT = 0.90)
JSI METAL= 0.67 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300333	T303	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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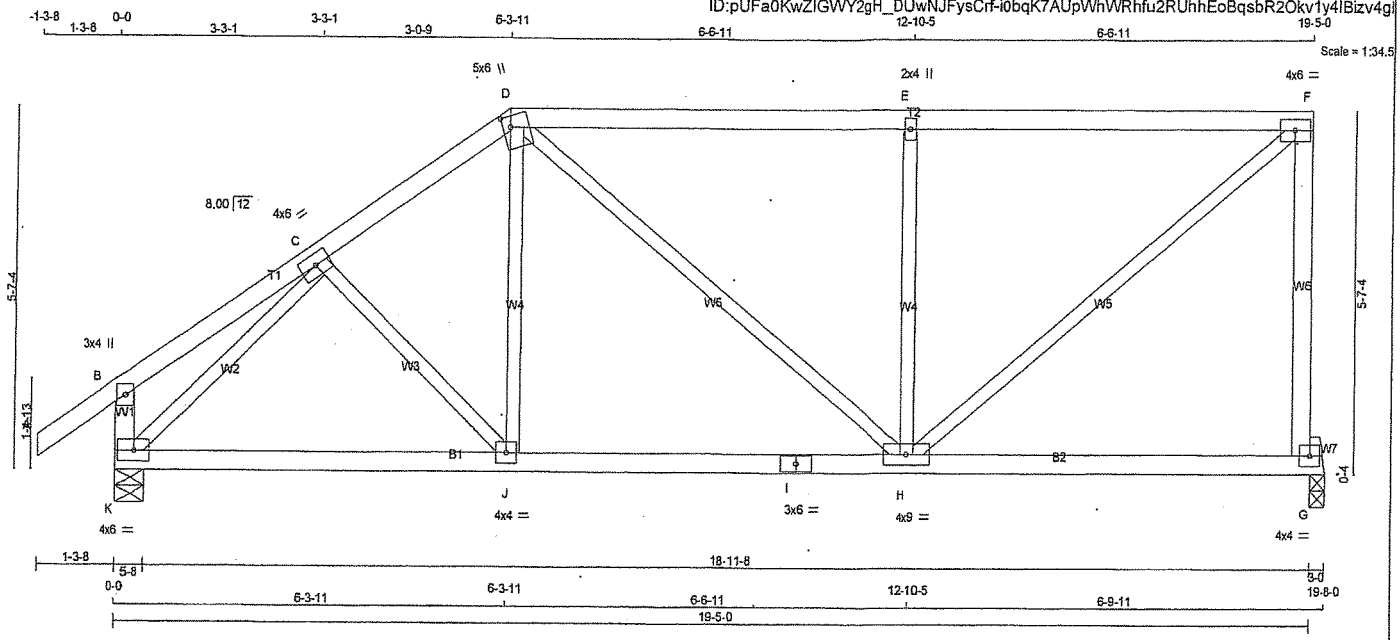


TOTAL WEIGHT = 83 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	ORY	No.2	SPF
D - F	2x4	ORY	No.2	SPF
G - F	2x4	ORY	No.2	SPF
K - B	2x4	ORY	No.2	SPF
K - I	2x4	ORY	No.2	SPF
I - G	2x4	ORY	No.2	SPF
ALL WEBS EXCEPT	2x3	ORY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1198	0	1198	0	0	3-0	3-0
K	1314	0	1314	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	908	452 / 0	204 / 0	0 / 0	0 / 0	252 / 0	0 / 0
K	989	516 / 0	204 / 0	0 / 0	0 / 0	269 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.94 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

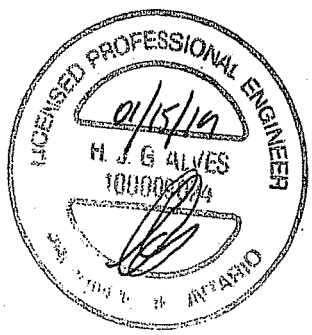
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.11 (1)	10.00
B- C	0 / 15	-84.9	-84.9	0.12 (1)	10.00
C- D	-1205 / 0	-84.9	-84.9	0.14 (1)	5.68
D- E	-1094 / 0	-84.9	-84.9	0.68 (1)	4.94
E- F	-1094 / 0	-84.9	-84.9	0.68 (1)	4.94
G- F	-1094 / 0	0.0	0.0	0.62 (1)	7.56
K- B	-223 / 0	0.0	0.0	0.02 (1)	7.81
K- J	0 / 981	-38.5	-38.5	0.40 (2)	10.00
J- I	0 / 993	-38.5	-38.5	0.42 (2)	10.00
I- H	0 / 993	-38.5	-38.5	0.42 (2)	10.00
H- G	0 / 0	-38.5	-38.5	0.29 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- J	0 / 90	0.02 (3)
J- D	0 / 308	0.07 (2)
D- H	0 / 130	0.03 (1)
H- E	-689 / 0	0.33 (1)
H- F	0 / 1405	0.32 (1)
K- C	-1395 / 0	0.44 (1)



DESIGN CRITERIA

SPECIFIED LOADS.

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.68/1.00 (E-F:1) , BC=0.42/1.00 (H-J:2) , WB=0.44/1.00 (C-K:1) , SSI=0.27/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (F) (INPUT = 0.90)
JSI METAL= 0.34 (C) (INPUT = 1.00)

DWG NO. TAM 11901/75
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300333	T304	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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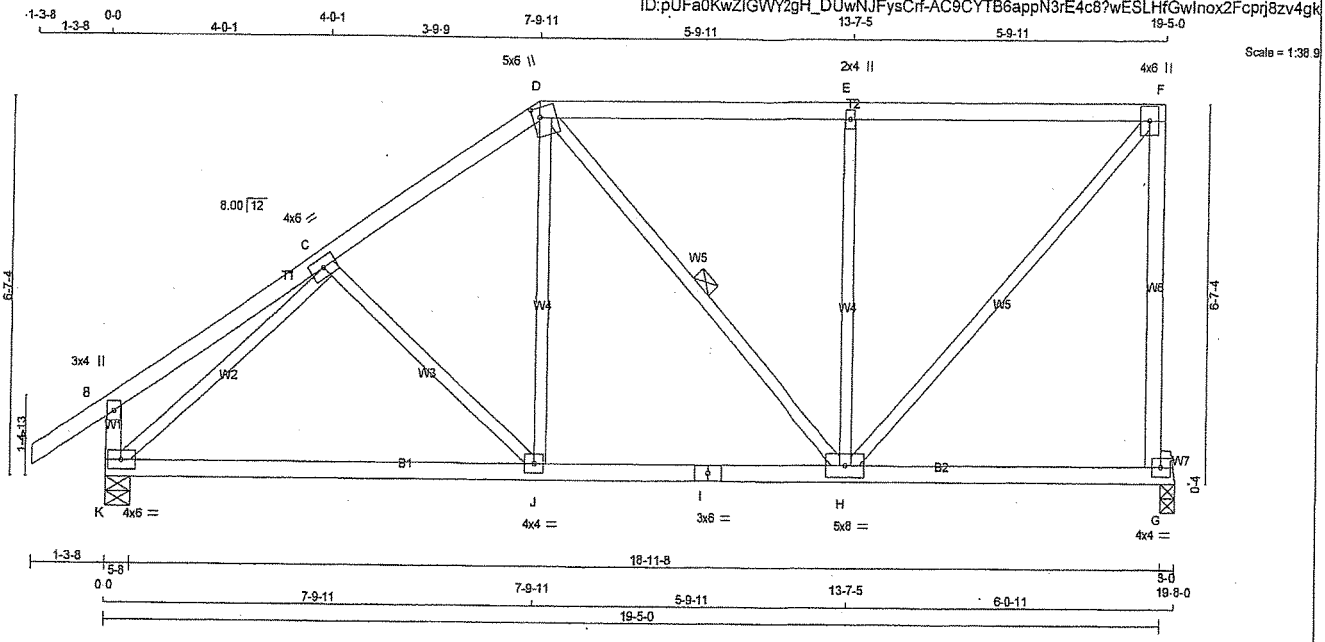

TOTAL WEIGHT = 88 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMV+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REORD BRG IN-SX
G	1198	0	1198	0	0	3-0	3-0
K	1314	0	1314	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	908	452 / 0	204 / 0	0 / 0	0 / 0	252 / 0	0 / 0
K	989	516 / 0	204 / 0	0 / 0	0 / 0	269 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.71 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.9	-84.9	0.11 (1)		10.00
B-C	0 / 20	-84.9	-84.9	0.20 (1)		10.00
C-D	-1124 / 0	-84.9	-84.9	0.22 (1)		5.71
D-E	-845 / 0	-84.9	-84.9	0.51 (1)		5.83
E-F	-846 / 0	-84.9	-84.9	0.51 (1)		5.83
G-F	-1102 / 0	0.0	0.0	1.00 (1)		7.54
K-B	-245 / 0	0.0	0.0	0.03 (1)		7.81
K-J	0 / 1000	-38.5	-38.5	0.51 (2)		10.00
J-I	0 / 923	-38.5	-38.5	0.50 (2)		10.00
I-H	0 / 923	-38.5	-38.5	0.50 (2)		10.00
H-G	0 / 0	-38.5	-38.5	0.21 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-J	-112 / 50	0.06 (1)
J-D	0 / 423	0.10 (2)
D-H	-128 / 0	0.06 (2)
H-E	-610 / 0	0.44 (1)
H-F	0 / 1244	0.28 (1)
K-C	-1375 / 0	0.64 (1)

LICENSED PROFESSIONAL ENGINEER
01/15/19
H. J. G. ALVES
100099924
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 883 (0.26")

CSI: TC=1.00/1.00 (F-G:1) , BC=0.51/1.00 (J-K:2) , WB=0.64/1.00 (C-K:1) , SSI=0.24/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (F) (INPUT = 0.90)
JSI METAL= 0.38 (F) (INPUT = 1.00)

DWG NO. TAM T1901176
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300333	T305	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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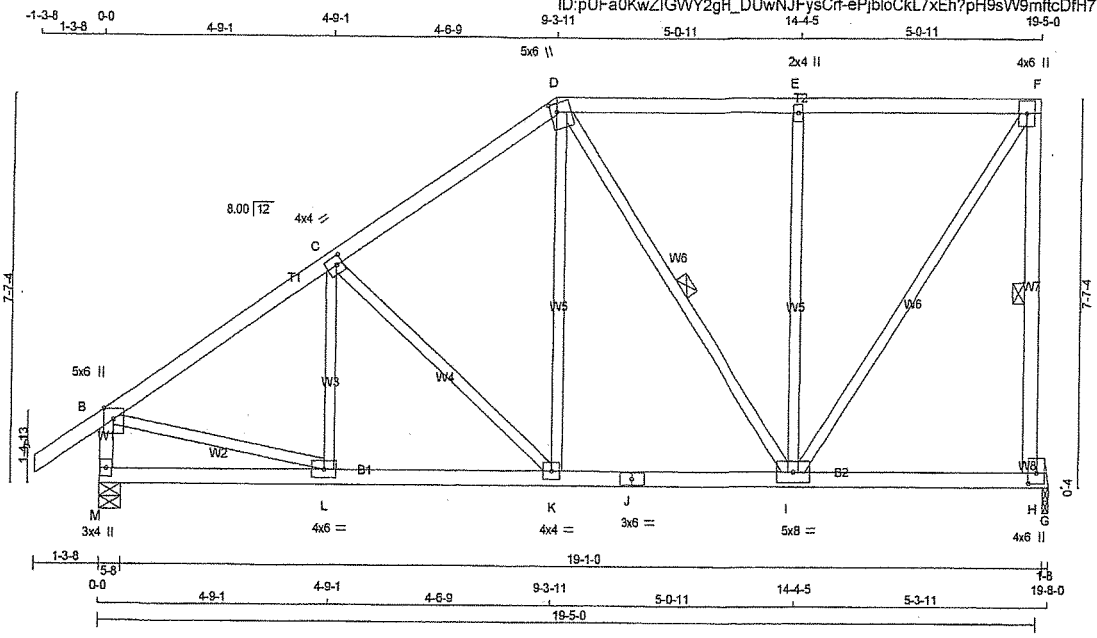


TOTAL WEIGHT = 95 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - B	2x4	ORY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY- SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
H	BMVW+p	MT20	4.0	6.0	2.50	2.00
I	BMWWW-t	MT20	5.0	8.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
M	1329	0	1329	0	0	5-8	5-8
G	1196	0	1196	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1000	522 / 0	207 / 0	0 / 0	0 / 0	272 / 0	0 / 0
G	909	446 / 0	207 / 0	0 / 0	0 / 0	256 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.26 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

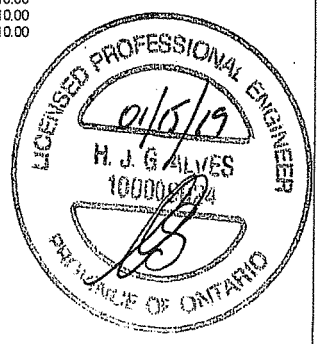
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-H, D-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 32	-84.9	-84.9	0.11 (1)	10.00	L- C	-42 / 135	0.03 (3)
B- C	-1296 / 0	-84.9	-84.9	0.35 (1)	5.26	C- K	-385 / 0	0.27 (1)
C- D	-1008 / 0	-84.9	-84.9	0.33 (1)	5.79	K- D	0 / 424	0.10 (2)
D- E	-705 / 0	-84.9	-84.9	0.37 (1)	6.25	D- I	-205 / 0	0.11 (1)
E- F	-705 / 0	-84.9	-84.9	0.37 (1)	6.25	I- E	-529 / 0	0.56 (1)
H- F	-1177 / 0	0.0	0.0	0.31 (1)	5.88	I- F	0 / 1233	0.28 (1)
M- B	-1255 / 0	0.0	0.0	0.13 (1)	7.17	B- L	0 / 1127	0.25 (1)
M- L	0 / 0	-38.5	-38.5	0.16 (3)	10.00			
L- K	0 / 1099	-38.5	-38.5	0.31 (2)	10.00			
K- J	0 / 821	-38.5	-38.5	0.39 (2)	10.00			
J- I	0 / 821	-38.5	-38.5	0.39 (2)	10.00			
I- H	0 / 0	-38.5	-38.5	0.40 (1)	10.00			
H- G	0 / 0	-53.5	-53.5	0.17 (1)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 088-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.37/1.00 (E-F:1) , BC=0.40/1.00 (H-I:1) , WB=0.56/1.00 (E-I:1) , SSI=0.47/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (H) (INPUT = 0.90)
JSI METAL= 0.56 (H) (INPUT = 1.00)

DWG NO. TAM 1904177
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300333	T306	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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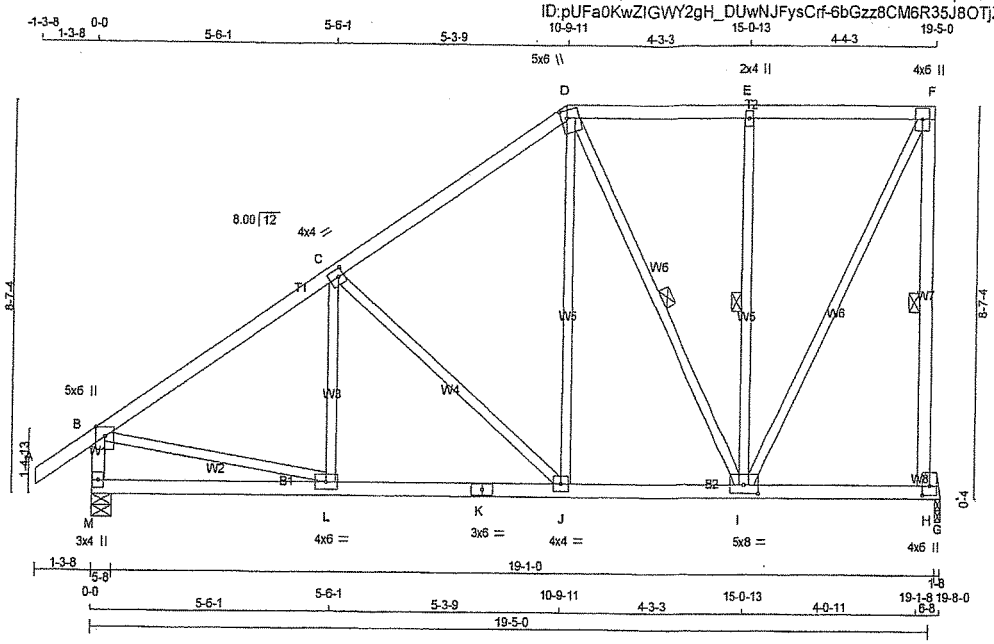


TOTAL WEIGHT = 101 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	ORY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	ORY	No.2	SPF

ORY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
O	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
H	BMVW+p	MT20	4.0	6.0	2.50	2.00
I	BMWWW-t	MT20	5.0	8.0	2.25	4.00
J	BMWW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INOICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1329	0	1329	0	0	5-8	5-8
G	1196	0	1196	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1000	522 / 0	207 / 0	0 / 0	0 / 0	272 / 0	0 / 0
G	909	446 / 0	207 / 0	0 / 0	0 / 0	256 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.06 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

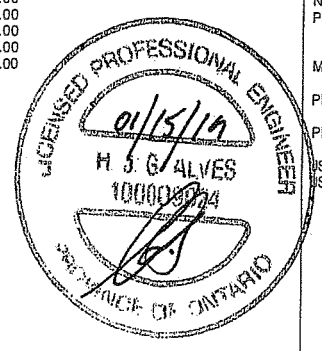
1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-H, D-I, E-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.9	-84.9	0.11 (1)	10.00
B-C	-1292 / 0	-84.9	-84.9	0.49 (1)	5.06
C-D	-887 / 0	-84.9	-84.9	0.45 (1)	5.87
D-E	-553 / 0	-84.9	-84.9	0.26 (1)	6.25
E-F	-553 / 0	-84.9	-84.9	0.26 (1)	6.25
H-F	-1200 / 0	0.0	0.0	0.41 (1)	5.84
M-B	-1245 / 0	0.0	0.0	0.13 (1)	7.20
M-L	0 / 0	-38.5	-38.5	0.22 (3)	10.00
L-K	0 / 1100	-38.5	-38.5	0.37 (2)	10.00
K-J	0 / 1100	-38.5	-38.5	0.37 (2)	10.00
J-I	0 / 718	-38.5	-38.5	0.31 (2)	10.00
I-H	0 / 0	-38.5	-38.5	0.40 (1)	10.00
H-G	0 / 0	-53.5	-53.5	0.17 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L-C	0 / 200	0.05 (3)
C-J	-517 / 0	0.52 (1)
J-D	0 / 498	0.11 (2)
B-L	0 / 1121	0.25 (1)
I-F	0 / 1193	0.27 (1)
D-I	-360 / 0	0.22 (1)
I-E	-448 / 0	0.21 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIOENTIAL OR SMALL BUILOING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAO

ALLOWABLE DEFL.(LL)= L/380 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.49/1.00 (B-C:1) , BC=0.40/1.00 (H-I:1) , WB=0.52/1.00 (C-J:1) , SSI=0.47/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TDL. = 5.0 Deg.

USI GRIP= 0.88 (H) (INPUT = 0.90)
USI METAL= 0.56 (H) (INPUT = 1.00)

DWG NO. TAM 190118
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300333	T307	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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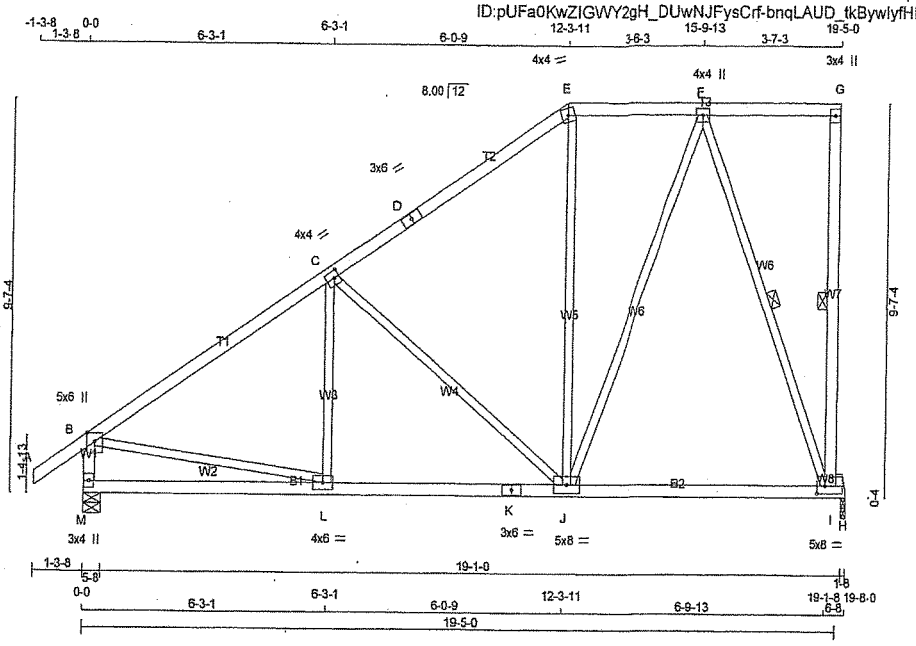

TOTAL WEIGHT = 99 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTW-m	MT20	4.0	4.0		
F	TMWW+t	MT20	4.0	4.0		
G	TMV+p	MT20	3.0	4.0		
I	BMVWW-t	MT20	5.0	8.0	2.25	2.50
J	BMWWW-t	MT20	5.0	8.0		
K	BS-t	MT20	3.0	6.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1329	0	1329	0	0	5-8	5-8
H	1196	0	1196	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1000	522 / 0	207 / 0	0 / 0	0 / 0	272 / 0	0 / 0
H	909	446 / 0	207 / 0	0 / 0	0 / 0	256 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.82 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-I, F-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.11 (1)	10.00	
B- C	-1264 / 0	-84.9	-84.9	0.66 (1)	4.82	
C- D	-785 / 0	-84.9	-84.9	0.59 (1)	5.83	
D- E	-785 / 0	-84.9	-84.9	0.59 (1)	5.83	
E- F	-624 / 0	-84.9	-84.9	0.18 (1)	6.25	
F- G	0 / 0	-84.9	-84.9	0.17 (1)	10.00	
I- G	-117 / 0	0.0	0.0	0.05 (1)	6.25	
M- B	-1226 / 0	0.0	0.0	0.13 (1)	7.24	
M- L	0 / 0	-38.5	-38.5	0.24 (3)	10.00	
L- K	0 / 1081	-38.5	-38.5	0.56 (2)	10.00	
K- J	0 / 1081	-38.5	-38.5	0.56 (2)	10.00	
J- I	0 / 390	-38.5	-38.5	0.53 (2)	10.00	
I- H	0 / 0	-53.5	-53.5	0.17 (1)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- C	0 / 217	0.05 (3)
C- J	-603 / 0	0.83 (1)
J- E	0 / 169	0.04 (3)
B- L	0 / 1097	0.25 (1)
J- F	0 / 680	0.15 (1)
F- I	-1082 / 0	0.77 (1)

LICENSED PROFESSIONAL ENGINEER
01/15/19
N. J. G. ALVES
100009924
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 792 (0.30")

CSI: TC=0.66/1.00 (B-C:1) , BC=0.56/1.00 (J-L:2) , WB=0.83/1.00 (C-J:1) , SSI=0.47/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)		SECTION (PLI)	
		MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.88 (I) (INPUT = 0.90)
SI METAL= 0.55 (K) (INPUT = 1.00)

DWG NO. TAM T190179
STRUCTURAL
COMPONENT ONLY

JDB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300333	T308	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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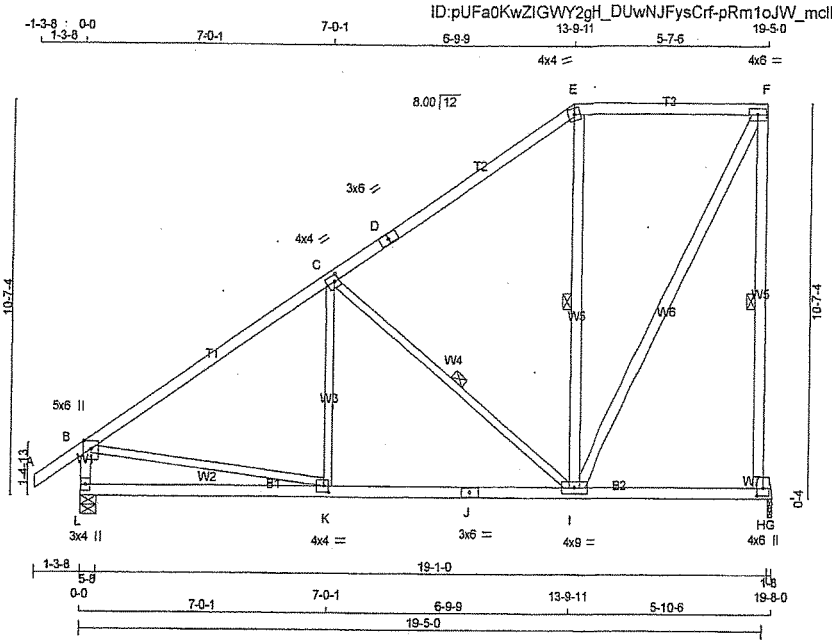


TOTAL WEIGHT = 104 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	ORY	No.2	SPF
D - E	2x4	ORY	No.2	SPF
E - F	2x4	ORY	No.2	SPF
H - F	2x4	ORY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - J	2x4	ORY	No.2	SPF
J - G	2x4	ORY	No.2	SPF
H - G	2x4	ORY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
I - E	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
O	TS-t	MT20	3.0	6.0		
E	TTW-m	MT20	4.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
H	BMVW+p	MT20	4.0	6.0	2.50	2.00
I	BMWWW-t	MT20	4.0	9.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0	2.00	1.50
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1329	0	1329	0	0	5-8	5-8
G	1196	0	1196	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1000	522 / 0	207 / 0	0 / 0	0 / 0	272 / 0	0 / 0
G	909	446 / 0	207 / 0	0 / 0	0 / 0	256 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.44 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

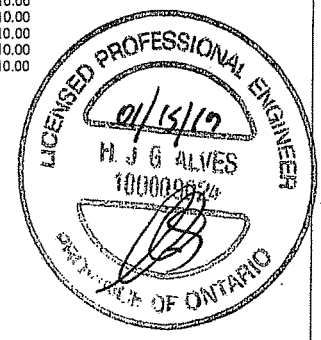
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-H, C-I, E-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 32	-84.9	-84.9	0.11 (1)	10.00	K- C	0 / 295	0.07 (3)
B- C	-1244 / 0	-84.9	-84.9	0.87 (1)	4.44	C- I	-743 / 0	0.42 (1)
C- D	-637 / 0	-84.9	-84.9	0.75 (1)	5.85	I- E	-123 / 97	0.07 (1)
D- E	-637 / 0	-84.9	-84.9	0.75 (1)	5.85	I- F	0 / 1055	0.17 (1)
E- F	-494 / 0	-84.9	-84.9	0.47 (1)	6.25	B- K	0 / 1080	0.24 (1)
H- F	-1169 / 0	0.0	0.0	0.66 (1)	5.90			
L- B	-1219 / 0	0.0	0.0	0.13 (1)	7.26			
L- K	0 / 0	-38.5	-38.5	0.34 (3)	10.00			
K- J	0 / 1068	-38.5	-38.5	0.47 (2)	10.00			
J- I	0 / 1068	-38.5	-38.5	0.47 (2)	10.00			
I- H	0 / 0	-38.5	-38.5	0.41 (1)	10.00			
H- G	0 / 0	-53.5	-53.5	0.17 (1)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS.

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.87/1.00 (B-C:1) , BC=0.47/1.00 (I-K:2) , WB=0.42/1.00 (C-I:1) , SSI=0.47/1.00 (G-H:1)

DDL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (H) (INPUT = 0.90)
JSI METAL= 0.56 (H) (INPUT = 1.00)

DWG NO. TAM T1901180
STRUCTURAL
[illegible] ONLY

JDB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300333	T309	1	4	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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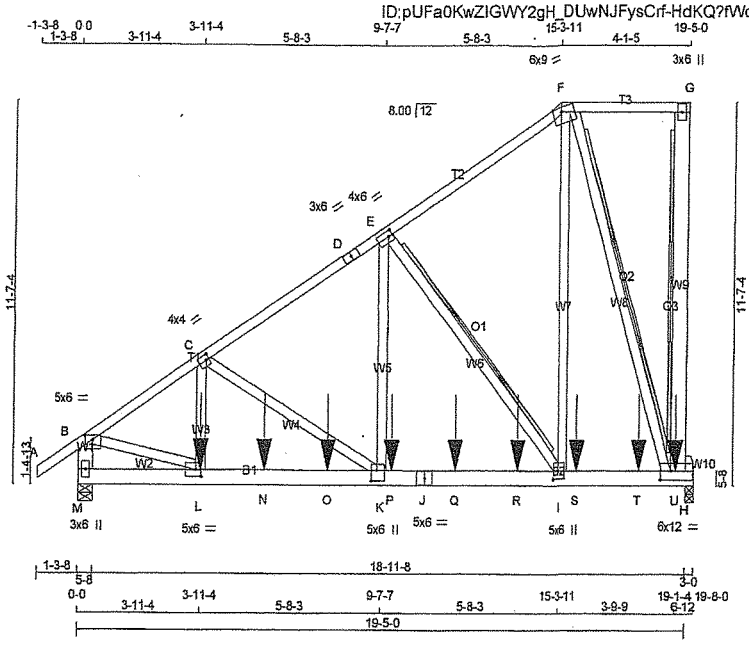


TOTAL WEIGHT = 4 X 143 = 572 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
H - G	2x6	DRY	No.2	SPF
M - B	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 4 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	TOP
D- F	1	12	TOP
F- G	1	12	TOP
G- H	2	12	SIDE(56.2)
M- B	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
M- J	2	12	TOP
J- H	2	5	SIDE(694.4)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

IN ADDITION, PRE-DRILL ONE 0.56" DIAM. HOLE IN EACH CHORD PANEL AND INSTALL 0.50" DIAM. ASTM A307 BOLTS WITH WASHERS, BOTH SIDES, FOR OTHER BOLT TYPES SEE CSA086 3.3.2.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TDP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.00
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	6.0	2.00	1.50
F	TTWW-m	MT20	6.0	9.0	1.75	3.75
G	TMV+p	MT20	3.0	6.0		
H	BMVWW1-t	MT20	6.0	12.0	3.25	4.00
I	BMWW+t	MT20	5.0	6.0	3.00	2.25
J	BS-t	MT20	5.0	6.0		
K	BMWW+t	MT20	5.0	6.0	3.75	2.00
L	BMWW-t	MT20	5.0	6.0	2.50	2.00
M	BMV1+p	MT20	3.0	8.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	11094	0	11094	0	0	3-0	3-0
M	8210	0	8210	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	8385	4265 / 0	1813 / 0	0 / 0	0 / 0	2308 / 0	0 / 0
M	6197	3175 / 0	1323 / 0	0 / 0	0 / 0	1700 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.83 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x8 DRY SPF No.2 T-BRACE AT G-H, E-I, F-H

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 32	-84.9	-84.9	0.04 (1)	10.00	L- C	0 / 2142	0.09 (1)
B- C	-10558 / 0	-84.9	-84.9	0.23 (1)	3.83	C- K	-2318 / 0	0.34 (1)
C- D	-8207 / 0	-84.9	-84.9	0.38 (1)	4.22	K- E	0 / 6996	0.28 (1)
D- E	-8207 / 0	-84.9	-84.9	0.38 (1)	4.22	E- I	-6310 / 0	0.34 (1)
E- F	-3564 / 0	-84.9	-84.9	0.18 (1)	5.93	I- F	0 / 3675	0.15 (1)
F- G	0 / 0	-84.9	-84.9	0.09 (1)	10.00	F- H	-8919 / 0	0.72 (1)
H- G	-174 / 0	0.0	0.0	0.01 (1)	7.81	B- L	-9007 / 0	0.58 (1)
M- B	-8348 / 0	0.0	0.0	0.15 (1)	6.74			
M- L	0 / 0	-38.5	-38.5	0.18 (1)	10.00			
L- N	0 / 8797	-38.5	-38.5	0.86 (1)	10.00			
N- O	0 / 8797	-38.5	-38.5	0.86 (1)	10.00			
O- K	0 / 8797	-38.5	-38.5	0.86 (1)	10.00			
K- P	0 / 6842	-38.5	-38.5	0.79 (1)	10.00			
P- J	0 / 6842	-38.5	-38.5	0.79 (1)	10.00			
J- Q	0 / 8842	-38.5	-38.5	0.79 (1)	10.00			
Q- R	0 / 6842	-38.5	-38.5	0.79 (1)	10.00			
R- I	0 / 6842	-38.5	-38.5	0.79 (1)	10.00			
I- S	0 / 3070	-38.5	-38.5	0.51 (1)	10.00			
S- T	0 / 3070	-38.5	-38.5	0.51 (1)	10.00			
T- U	0 / 3070	-38.5	-38.5	0.51 (1)	10.00			
U- H	0 / 3070	-38.5	-38.5	0.51 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE	HEEL	CONN.
L	3-11-4	-2633	-2633	—	TOP	VERT	TOTAL	—	—
N	5-11-4	-1768	-1768	—	TOP	VERT	TOTAL	—	—
O	7-11-4	-1768	-1768	—	TOP	VERT	TOTAL	—	—
P	9-11-4	-1768	-1768	—	TOP	VERT	TOTAL	—	—
Q	11-11-4	-1768	-1768	—	BACK	VERT	TOTAL	—	—
R	13-11-4	-1768	-1768	—	TOP	VERT	TOTAL	—	—

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.38/1.00 (C-E:1) , BC=0.86/1.00 (K-L:1) , WB=0.72/1.00 (F-H:1) , SSI=0.58/1.00 (I-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

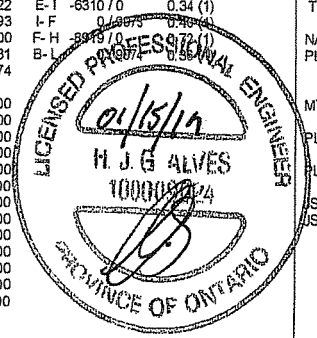
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.90 (B) (INPUT = 0.90)
SI METAL= 0.55 (L) (INPUT = 1.00)



DWG NO. TAM T1901181
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300333	T309	1	4	TRUSS DESC.	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
S	15-11-4	-1768	-1768	—	BACK	VERT	TOTAL	—	—
T	17-11-4	-1768	-1768	—	BACK	VERT	TOTAL	—	—
U	19-1-4	-1781	-1781	—	BACK	VERT	TOTAL	—	—

DWG NO. TAM 7790181
STRUCTURAL
COPY IS NOT ONLY

JDB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300581	T310	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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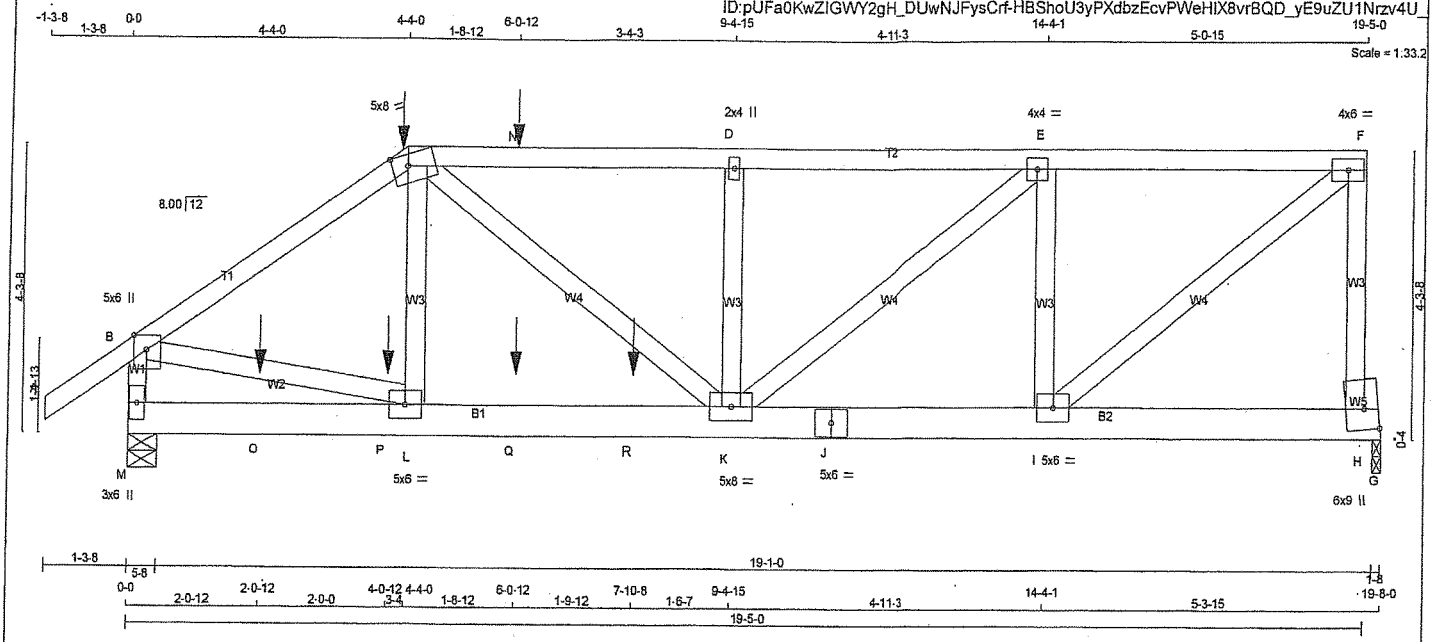


TOTAL WEIGHT = 2 X 102 = 205 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - G	2x6	DRY	No.2	SPF
H - G	2x6	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	SIDE(81.0)
C-F	1	12	SIDE(61.0)
F-H	1	12	TOP
M-B	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
M-J	2	12	SIDE(0.0)
J-G	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOAOED FROM THE TOP AND MUST BE PLACED DN TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIOE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIEO ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW-m	MT20	5.0	8.0	2.00	3.00
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
H	BMVWm	MT20	6.0	9.0	Edge	3.75
I	BMVW-t	MT20	5.0	6.0		
J	BS-t	MT20	5.0	6.0		
K	BMVWW-t	MT20	5.0	8.0		
L	BMVW-t	MT20	5.0	8.0		
M	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPDRTS AND LOADINGS SPECIFIED BY FABRICATDR TD BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTOREO GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	2552	0	2552	0	0	5-8	5-8
G	1816	0	1816	0	0	1-8	1-8

UNFACTDRED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WINO	DEAD	SOIL
M	1912	1033 / 0	385 / 0	0 / 0	0 / 0	494 / 0	0 / 0
G	1370	706 / 0	293 / 0	0 / 0	0 / 0	370 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.83 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JDINTS MUST BE LATERALLY RESTRAINED.

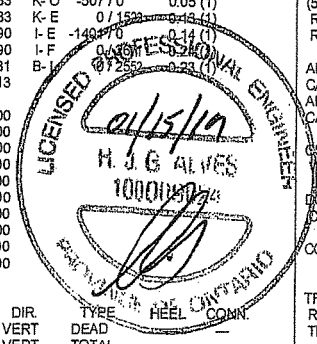
LOADING

TOTAL LOAO CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTDRED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 32	-84.9	-84.9	0.06 (1)	10.00	L-C	0 / 335	0.03 (3)
B-C	-2999 / 0	-84.9	-84.9	0.20 (1)	5.14	C-K	0 / 1020	0.09 (1)
C-N	-3306 / 0	-84.9	-84.9	0.29 (1)	4.83	K-O	-507 / 0	0.05 (1)
N-D	-3306 / 0	-84.9	-84.9	0.29 (1)	4.83	K-E	0 / 1532	0.13 (1)
D-E	-3306 / 0	-84.9	-84.9	0.23 (1)	4.90	I-E	-1403 / 0	0.14 (1)
E-F	-2115 / 0	-84.9	-84.9	0.17 (1)	5.90	I-F	0 / 1001	0.24 (1)
H-F	-1809 / 0	0.0	0.0	0.25 (1)	7.81	B-I	0 / 2552	0.23 (1)
M-B	-2524 / 0	0.0	0.0	0.14 (1)	7.13			
M-O	0 / 0	-38.5	-38.5	0.15 (2)	10.00			
O-P	0 / 0	-38.5	-38.5	0.15 (2)	10.00			
P-L	0 / 0	-38.5	-38.5	0.15 (2)	10.00			
L-O	0 / 2498	-38.5	-38.5	0.51 (1)	10.00			
Q-R	0 / 2498	-38.5	-38.5	0.51 (1)	10.00			
R-K	0 / 2498	-38.5	-38.5	0.51 (1)	10.00			
K-J	0 / 2115	-38.5	-38.5	0.29 (1)	10.00			
J-I	0 / 2115	-38.5	-38.5	0.29 (1)	10.00			
I-H	0 / 0	-38.5	-38.5	0.17 (1)	10.00			
H-G	0 / 0	-53.5	-53.5	0.07 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	COMN.
C	4-4-1	-58	-64	—	FRONT	VERT	DEAD	—	—
C	4-4-0	-120	-120	—	BACK	VERT	TOTAL	—	—
C	4-4-1	-266	-266	—	FRONT	VERT	SNOW	—	—
N	6-0-12	-114	-114	—	BACK	VERT	TOTAL	—	—
O	2-0-12	-46	-62	—	BACK	VERT	TOTAL	—	—
P	4-0-12	-51	-67	—	BACK	VERT	TOTAL	—	—
O	6-0-12	-46	-62	—	BACK	VERT	TOTAL	—	—
R	7-10-8	-1145	-1145	—	TOP	VERT	TOTAL	—	—



DWG NO. TAM T1901B2
STRUCTURAL
CONFORMANT ONLY

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FRDM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLDPE OF 6.00/12

*** NDN STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

GSI: TC=0.29/1.00 (C-O:1) , BC=0.51/1.00 (K-L:1) , WB=0.24/1.00 (F-I:1) , SSI=0.30/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TDL. = 5.0 Deg.

JSI GRIP= 0.77 (F) (INPUT = 0.90)
JSI METAL= 0.58 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300581	T311	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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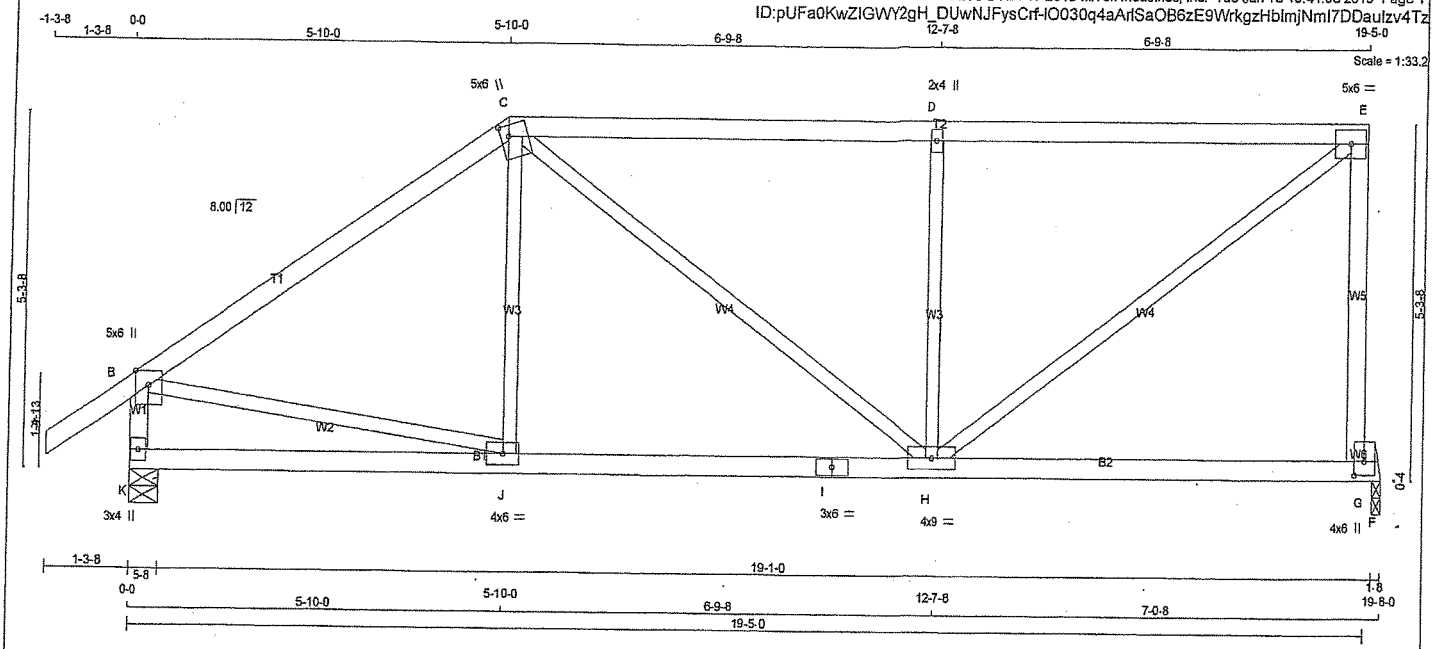


TOTAL WEIGHT = 80 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	5.0	6.0		
G	BMVW+p	MT20	4.0	6.0	2.50	2.00
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	1329	0	1329	0	0	5-8	5-8
F	1196	0	1196	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	1000	522 / 0	207 / 0	0 / 0	0 / 0	272 / 0	0 / 0
F	909	448 / 0	207 / 0	0 / 0	0 / 0	256 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, F

BRACING

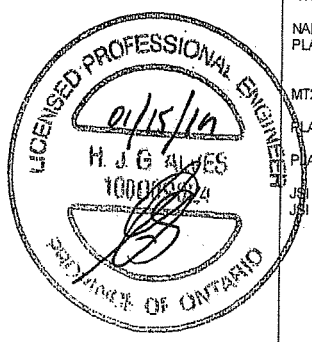
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.55 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 32	-84.9	-84.9	1	0.11 (1)	10.00	J- C	0 / 222	0.05 (3)
B- C	-1231 / 0	-84.9	-84.9	1	0.59 (1)	4.98	C- H	0 / 273	0.06 (1)
C- D	-1244 / 0	-84.9	-84.9	1	0.75 (1)	4.55	H- D	-713 / 0	0.30 (1)
D- E	-1244 / 0	-84.9	-84.9	1	0.75 (1)	4.55	H- E	0 / 1541	0.35 (1)
G- E	-1131 / 0	0.0	0.0	1	0.55 (1)	7.46	B- J	0 / 1040	0.23 (1)
K- B	-1235 / 0	0.0	0.0	1	0.13 (1)	7.22			
K- J	0 / 0	-38.5	-38.5	1	0.24 (3)	10.00			
J- I	0 / 1025	-38.5	-38.5	1	0.55 (2)	10.00			
I- H	0 / 1025	-38.5	-38.5	1	0.55 (2)	10.00			
H- G	0 / 0	-38.5	-38.5	1	0.46 (2)	10.00			
G- F	0 / 0	-53.5	-53.5	1	0.17 (1)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 835 (0.28")

CSI: TC=0.75/1.00 (D-E:1) , BC=0.55/1.00 (H-J:2) , WB=0.35/1.00 (E-H:1) , SSI=0.47/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (G) (INPUT = 0.90)
JSI METAL= 0.56 (G) (INPUT = 1.00)

DWG NO. TAM 7190183
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300581	T312	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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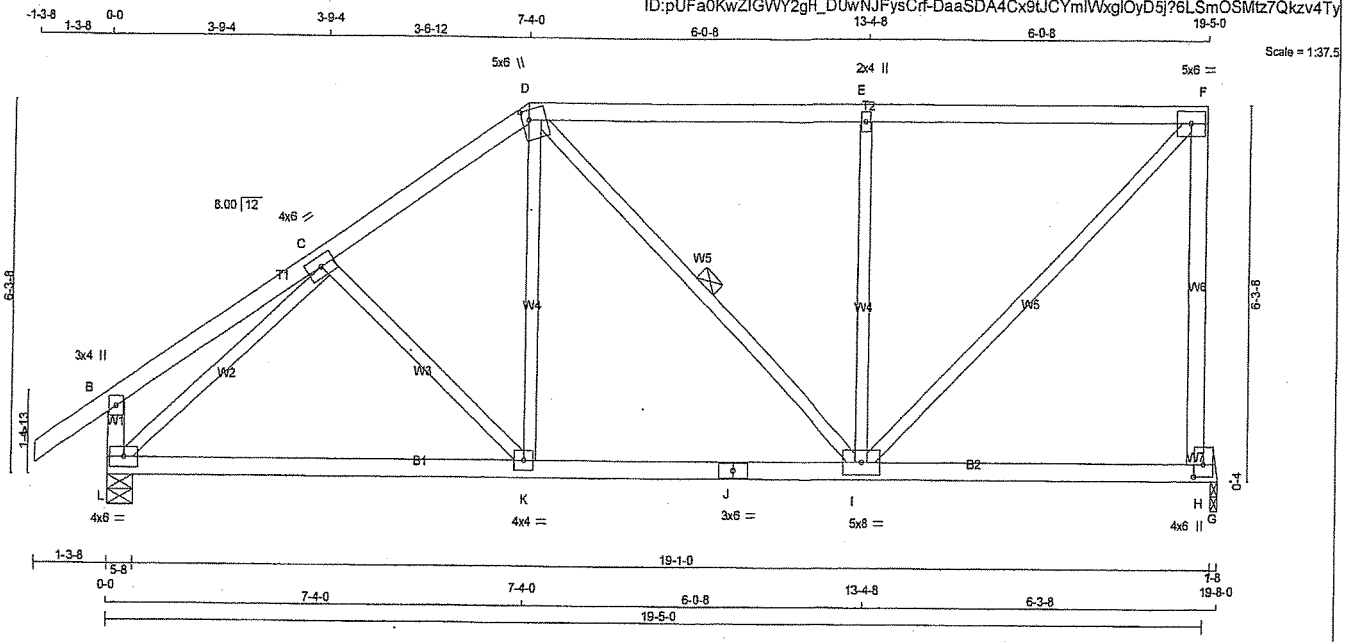


TOTAL WEIGHT = 86 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	ORY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
H - F	2x4	ORY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - G	2x4	ORY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	5.0	6.0		
H	BMVW+p	MT20	4.0	6.0	2.50	2.00
I	BMWWW-t	MT20	5.0	8.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQR0 BRG IN-SX
L	1329	0	1329	0	0	5-8	5-8
G	1196	0	1196	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1000	522 / 0	207 / 0	0 / 0	0 / 0	272 / 0	0 / 0
G	909	446 / 0	207 / 0	0 / 0	0 / 0	256 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.46 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-I

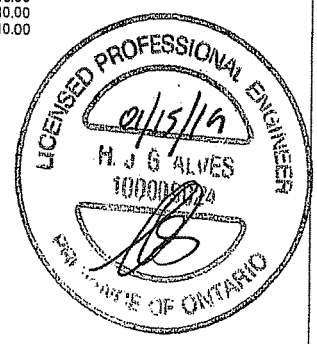
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.11 (1)	10.00	
B- C	0 / 19	-84.9	-84.9	0.17 (1)	10.00	
C- D	-1171 / 0	-84.9	-84.9	0.19 (1)	5.66	
D- E	-962 / 0	-84.9	-84.9	0.56 (1)	5.46	
E- F	-963 / 0	-84.9	-84.9	0.56 (1)	5.46	
H- F	-1146 / 0	0.0	0.0	0.90 (1)	7.43	
L- B	-238 / 0	0.0	0.0	0.02 (1)	7.81	
L- K	0 / 1010	-38.5	-38.5	0.46 (2)	10.00	
K- J	0 / 962	-38.5	-38.5	0.45 (2)	10.00	
J- I	0 / 962	-38.5	-38.5	0.45 (2)	10.00	
I- H	0 / 0	-38.5	-38.5	0.43 (2)	10.00	
H- G	0 / 0	-53.5	-53.5	0.17 (1)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- K	-72 / 63	0.03 (1)
K- D	0 / 368	0.08 (2)
D- I	-45 / 0	0.02 (3)
I- E	-634 / 0	0.40 (1)
I- F	0 / 1352	0.30 (1)
L- C	-1401 / 0	0.58 (1)



DESIGN CRITERIA

SPECIFIED LOADS.

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.90/1.00 (F-H:1) , BC=0.48/1.00 (K-L:2) , WB=0.58/1.00 (C-L:1) , SSI=0.47/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (H) (INPUT = 0.90)
JSI METAL= 0.56 (H) (INPUT = 1.00)

DWG NO. TAM T1901184

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300581	T313	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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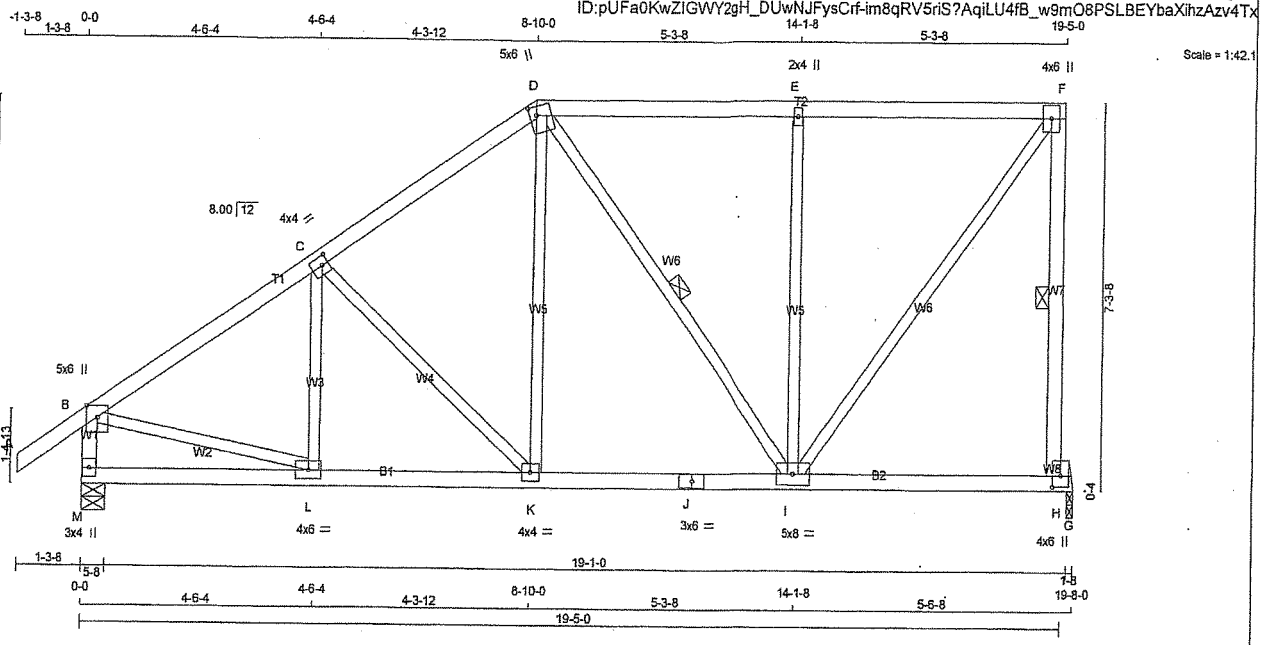


TOTAL WEIGHT = 93 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
H	BMVW+p	MT20	4.0	6.0	2.50	2.00
I	BMWWW-t	MT20	5.0	8.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
M	1329	0	1329	0	0	5-8	5-8
G	1196	0	1196	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1000	522 / 0	207 / 0	0 / 0	0 / 0	272 / 0	0 / 0
G	909	446 / 0	207 / 0	0 / 0	0 / 0	256 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.31 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-H, D-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.11 (1)	10.00
B- C	-1294 / 0	-84.9	-84.9	0.31 (1)	5.31
C- D	-1043 / 0	-84.9	-84.9	0.29 (1)	5.77
D- E	-760 / 0	-84.9	-84.9	0.41 (1)	6.25
E- F	-760 / 0	-84.9	-84.9	0.41 (1)	6.25
H- F	-1171 / 0	0.0	0.0	0.28 (1)	5.90
M- B	-1258 / 0	0.0	0.0	0.13 (1)	7.17
M- L	0 / 0	-38.5	-38.5	0.14 (3)	10.00
L- K	0 / 1096	-38.5	-38.5	0.29 (2)	10.00
K- J	0 / 852	-38.5	-38.5	0.41 (2)	10.00
J- I	0 / 852	-38.5	-38.5	0.41 (2)	10.00
I- H	0 / 0	-38.5	-38.5	0.41 (1)	10.00
H- G	0 / 0	-53.5	-53.5	0.17 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- C	-66 / 113	0.03 (3)
C- K	-342 / 0	0.22 (1)
K- D	0 / 400	0.09 (2)
D- I	-152 / 0	0.08 (1)
I- E	-554 / 0	0.52 (1)
I- F	0 / 1254	0.28 (1)
B- L	0 / 1127	0.25 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	8.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.41/1.00 (E-F:1) , BC=0.41/1.00 (I-K:2) , WB=0.52/1.00 (E-I:1) , SSI=0.47/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

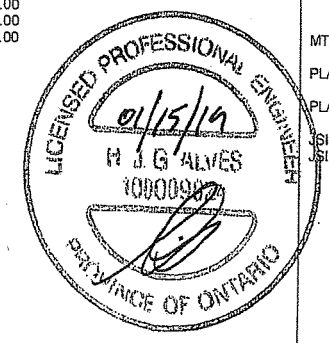
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.88 (H) (INPUT = 0.90)
SI METAL= 0.56 (H) (INPUT = 1.00)



DWG NO. TAM T1901185
STRUCTURAL
CONSENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300581	T314	1	1	TRUSS DESC.	

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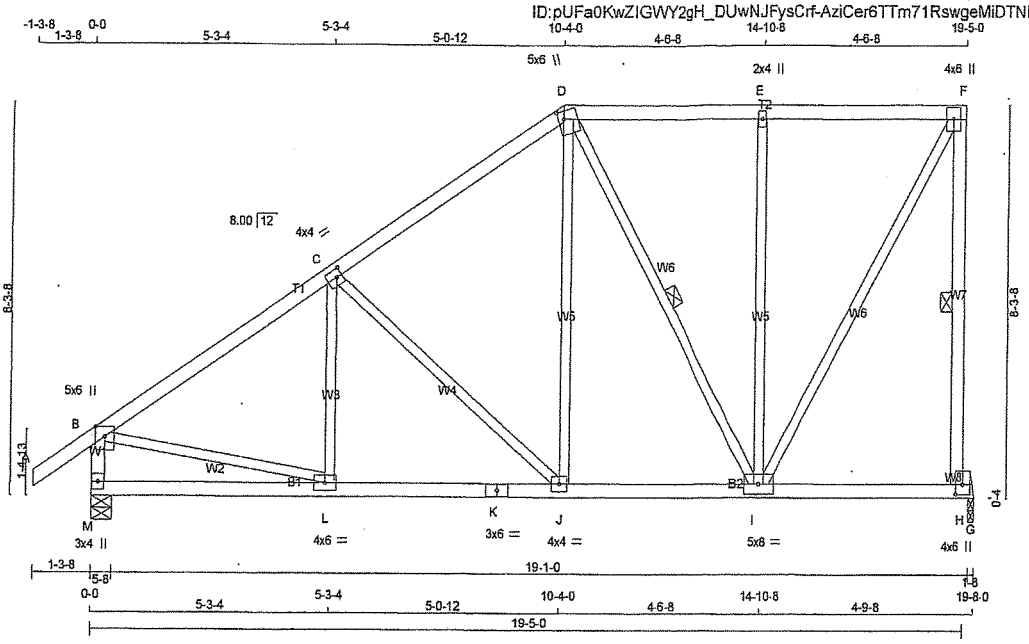


TOTAL WEIGHT = 99 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT2D	5.0	6.0	Edge	
C	TMWW-t	MT2D	4.0	4.0	2.00	1.50
D	TTWW+m	MT2D	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
H	BMVW+p	MT2D	4.0	6.0	2.50	2.00
I	BMWWW-t	MT2D	5.0	8.0		
J	BMWW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMWW-t	MT2D	4.0	6.0		
M	BMV1+p	MT2D	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
M	1329	0	1329	0	0	5-8	5-8
G	1196	0	1196	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1000	522 / 0	207 / 0	0 / 0	0 / 0	272 / 0	0 / 0
G	909	446 / 0	207 / 0	0 / 0	0 / 0	256 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.13 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-H, D-I.

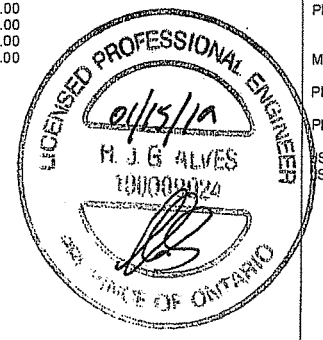
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES. (4)

CHORDS MEMB.	MAX FACTORED FORCE (LBS)	FR-TO	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32		-84.9	-84.9	0.11 (1)		10.00
B- C	-1295 / 0		-84.9	-84.9	0.44 (1)		5.13
C- D	-926 / 0		-84.9	-84.9	0.41 (1)		5.85
D- E	-597 / 0		-84.9	-84.9	0.29 (1)		6.25
E- F	-597 / 0		-84.9	-84.9	0.29 (1)		6.25
H- F	-1192 / 0		0.0	0.0	0.37 (1)		5.85
M- B	-1248 / 0		0.0	0.0	0.13 (1)		7.19
M- L	0 / 0		-38.5	-38.5	0.20 (3)		10.00
L- K	0 / 1101		-38.5	-38.5	0.35 (2)		10.00
K- J	0 / 1101		-38.5	-38.5	0.35 (2)		10.00
J- I	0 / 751		-38.5	-38.5	0.34 (2)		10.00
I- H	0 / 0		-38.5	-38.5	0.40 (1)		10.00
H-G	0 / 0		-53.5	-53.5	0.17 (1)		10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- C	0 / 181	0.04 (3)
C- J	-477 / 0	0.43 (1)
J- D	0 / 475	0.11 (2)
D- I	-313 / 0	0.18 (1)
I- E	-474 / 0	0.64 (1)
I- F	0 / 1201	0.27 (1)
B- L	0 / 1124	0.25 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.44/1.00 (B-C:1) , BC=0.40/1.00 (H-I:1) , WB=0.64/1.00 (E-I:1) , SSI=0.47/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.88 (H) (INPUT = 0.90)
SI METAL= 0.56 (H) (INPUT = 1.00)

DWG NO. TAM 1190186

STRUCTURAL COMPONENT

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300581	T315	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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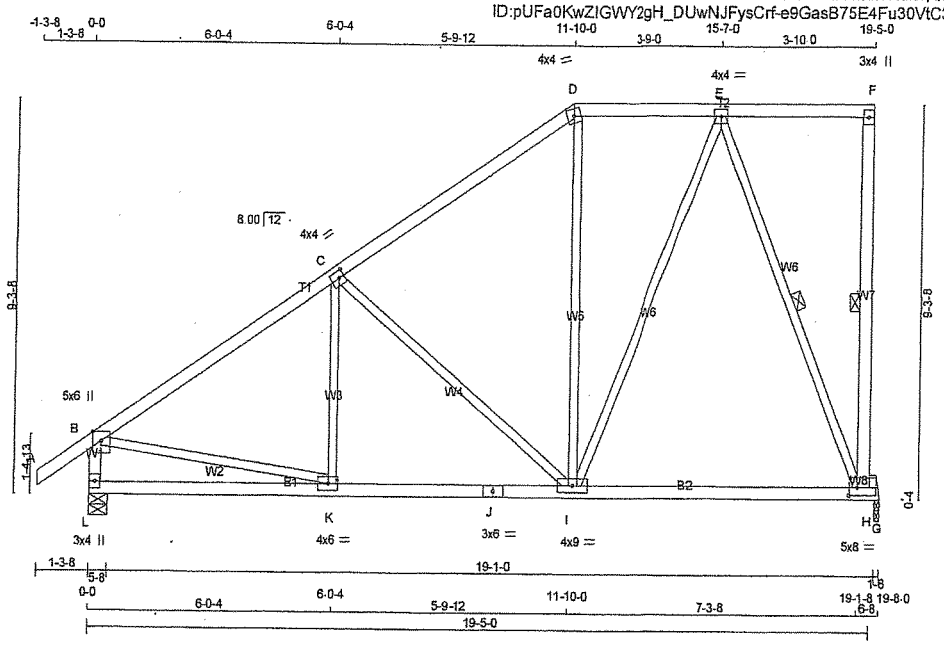


TOTAL WEIGHT = 98 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVWW-t	MT20	5.0	8.0	2.25	2.50
I	BMWWW-t	MT20	4.0	9.0		
J	BS-t	MT20	3.0	8.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQR0 BRG IN-SX
L	1329	0	1329	0	0	5-8	5-8
G	1196	0	1196	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1000	522 / 0	207 / 0	0 / 0	0 / 0	272 / 0	0 / 0
G	909	446 / 0	207 / 0	0 / 0	0 / 0	256 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.92 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

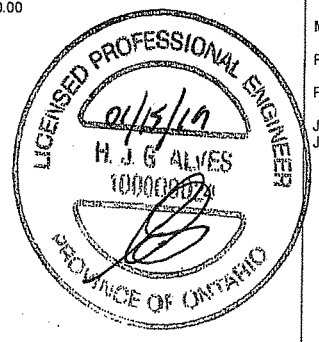
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.11 (1)	10.00
B- C	-1268 / 0	-84.9	-84.9	0.60 (1)	4.92
C- D	-831 / 0	-84.9	-84.9	0.55 (1)	5.82
D- E	-663 / 0	-84.9	-84.9	0.21 (1)	6.25
E- F	0 / 0	-84.9	-84.9	0.20 (1)	10.00
H- F	-124 / 0	0.0	0.0	0.05 (1)	6.25
L- B	-1228 / 0	0.0	0.0	0.13 (1)	7.23
L- K	0 / 0	-38.5	-38.5	0.23 (3)	10.00
K- J	0 / 1083	-38.5	-38.5	0.60 (2)	10.00
J- I	0 / 1083	-38.5	-38.5	0.60 (2)	10.00
I- H	0 / 422	-38.5	-38.5	0.57 (2)	10.00
H- G	0 / 0	-53.5	-53.5	0.17 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	0 / 189	0.04 (3)
C- I	-556 / 0	0.70 (1)
I- D	0 / 184	0.04 (3)
B- K	0 / 1100	0.25 (1)
I- E	0 / 642	0.14 (1)
E- H	-1078 / 0	0.73 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.21")
ALLOWABLE DEFL.(TL)= L/380 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 655 (0.36")

CSI: TC=0.60/1.00 (B-C:1) , BC=0.60/1.00 (I-K:2) , WB=0.73/1.00 (E-H:1) , SSI=0.47/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PU) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (H) (INPUT = 0.90)
JSI METAL= 0.54 (B) (INPUT = 1.00)

DWG NO. TAM T1904B7
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300581	T316	1	1	TRUSS DESC	

Tamarack Roof Truss, Burlington

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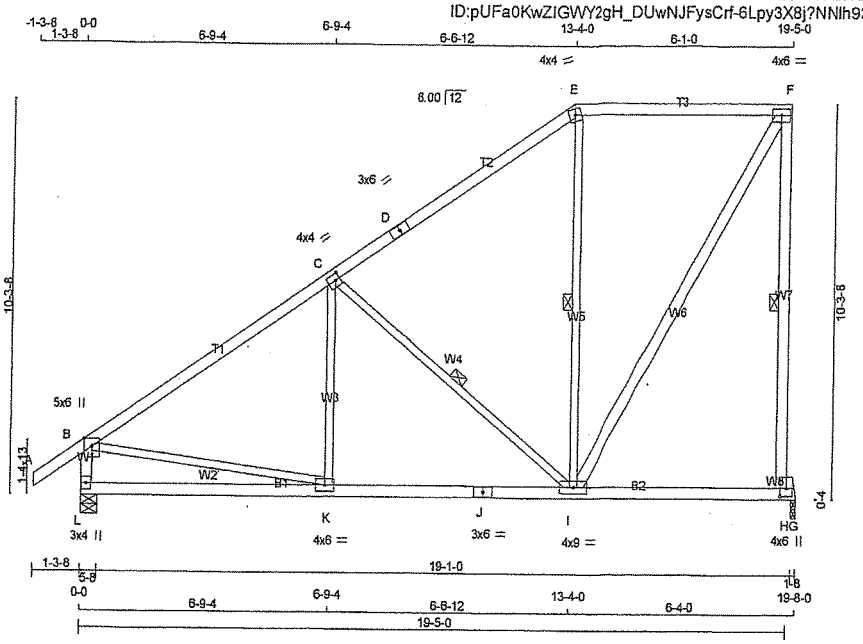


TOTAL WEIGHT = 99 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X	
B	TMVW+p	MT20	5.0	6.0	Edge		
C	TMWW-t	MT20	4.0	4.0	2.00	1.50	
D	TS-t	MT20	3.0	6.0			
E	TTW-m	MT20	4.0	4.0			
F	TMVW-t	MT20	4.0	6.0			
H	BMVW+p	MT20	4.0	6.0	2.50	2.00	
I	BMWWW-t	MT20	4.0	9.0			
J	BS-t	MT20	3.0	6.0			
K	BMWW-t	MT20	4.0	6.0			
L	BMV1+p	MT20	3.0	4.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
L	1329	0	1329	0	0	5-8	5-8
G	1196	0	1196	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1000	522 / 0	207 / 0	0 / 0	0 / 0	272 / 0	0 / 0
G	909	446 / 0	207 / 0	0 / 0	0 / 0	256 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-H, C-I, E-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

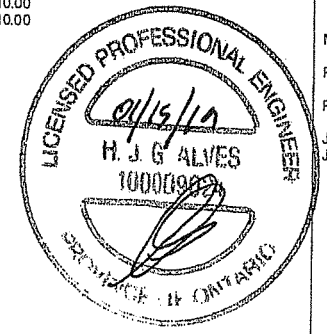
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.11 (1)	10.00
B- C	-1251 / 0	-84.9	-84.9	0.80 (1)	4.58
C- D	-885 / 0	-84.9	-84.9	0.70 (1)	5.85
D- E	-685 / 0	-84.9	-84.9	0.70 (1)	5.85
E- F	-536 / 0	-84.9	-84.9	0.56 (1)	6.25
H- F	-1153 / 0	0.0	0.0	0.60 (1)	5.93
L- B	-1221 / 0	0.0	0.0	0.13 (1)	7.25
L- K	0 / 0	-38.5	-38.5	0.31 (3)	10.00
K- J	0 / 1073	-38.5	-38.5	0.50 (2)	10.00
J- I	0 / 1073	-38.5	-38.5	0.50 (2)	10.00
I- H	0 / 0	-38.5	-38.5	0.42 (1)	10.00
H- G	0 / 0	-53.5	-53.5	0.17 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	0 / 271	0.06 (3)
C- I	-699 / 0	0.36 (1)
I- E	-108 / 110	0.08 (1)
I- F	0 / 1045	0.17 (1)
B- K	0 / 1086	0.24 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.80/1.00 (B-C:1) , BC=0.50/1.00 (I-K:2) , WB=0.36/1.00 (C-I:1) , SSI=0.47/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (H) (INPUT = 0.90)
JSI METAL= 0.56 (H) (INPUT = 1.00)

DWG NO. TAM T1901188
STRUCTURAL
CONSTRUCTION ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300581	T317	1	4	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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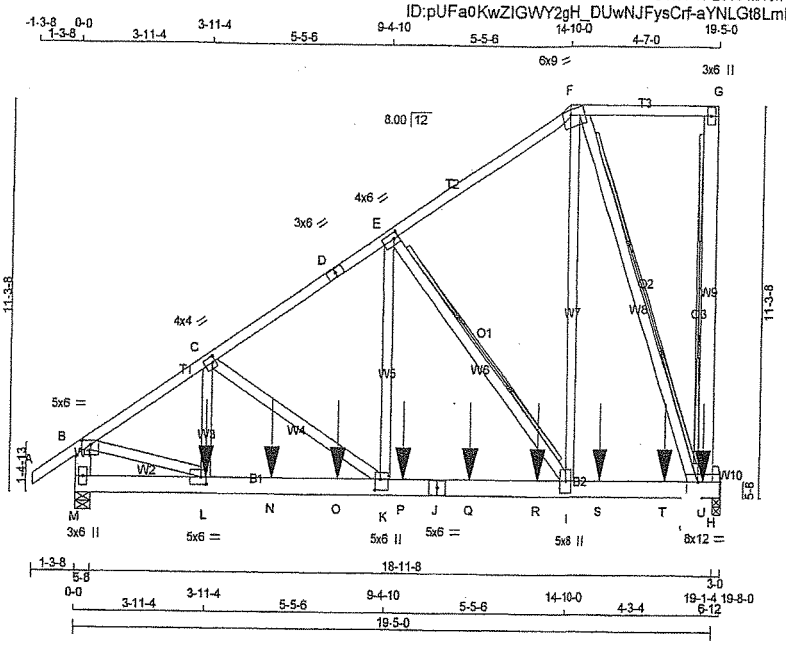

TOTAL WEIGHT = 4 X 141 = 565 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
H - G	2x6	DRY	No.2	SPF
M - B	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 4 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-D	1	12	TOP
D-F	1	12	TOP
F-G	1	12	TOP
G-H	2	12	SIDE(71.3)
M-B	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
M-J	2	12	TOP
J-H	2	5	SIDE(711.7)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

IN ADDITION, PRE-DRILL ONE 0.56" DIAM. HOLE IN EACH CHORD PANEL AND INSTALL 0.50" DIAM. ASTM A307 BOLTS WITH WASHERS, BOTH SIDES. FOR OTHER BOLT TYPES SEE CSA086 3.3.2.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.00
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	6.0	2.00	1.75
F	TTWW-m	MT20	6.0	9.0	1.75	3.50
G	TMV+p	MT20	3.0	6.0		
H	BMVWW1-t	MT20	8.0	12.0	5.25	4.00
I	BMWW+t	MT20	5.0	8.0		
J	BS-t	MT20	5.0	6.0		
K	BMWW+t	MT20	5.0	6.0	3.75	2.00
L	BMWW-t	MT20	5.0	6.0	2.50	2.00
M	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REORD BRG IN-SX
H	11302	0	11302	0	0	3-0	3-0
M	8331	0	8331	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM LIVE	WIND	DEAD	SOIL
H	8543	4343 / 0	1848 / 0	0 / 0	0 / 0	2351 / 0	0 / 0
M	6289	3221 / 0	1343 / 0	0 / 0	0 / 0	1725 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.81 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x8 DRY SPF No.2 T-BRACE AT G-H, E-I, F-H

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO			
A-B	0 / 32	-84.9	-84.9	0.04 (1)	10.00	L-C	0 / 2064		0.08 (1)
B-C	-10687 / 0	-84.9	-84.9	0.24 (1)	3.81	C-K	-2219 / 0		0.30 (1)
C-D	-8485 / 0	-84.9	-84.9	0.34 (1)	4.18	K-E	0 / 6767		0.27 (1)
D-E	-8485 / 0	-84.9	-84.9	0.34 (1)	4.18	E-I	-6094 / 0		0.31 (1)
E-F	-4035 / 0	-84.9	-84.9	0.17 (1)	5.68	I-F	0 / 10104		0.41 (1)
F-G	0 / 0	-84.9	-84.9	0.11 (1)	10.00	F-H	-8984 / 0		0.71 (1)
H-G	-194 / 0	0.0	0.0	0.01 (1)	7.81	B-I	0 / 2637		0.37 (1)
M-B	-8448 / 0	0.0	0.0	0.15 (1)	6.71				
M-L	0 / 0	-38.5	-38.5	0.16 (1)	10.00				
L-N	0 / 8903	-38.5	-38.5	0.83 (1)	10.00				
N-O	0 / 8903	-38.5	-38.5	0.83 (1)	10.00				
O-K	0 / 8903	-38.5	-38.5	0.83 (1)	10.00				
K-P	0 / 7055	-38.5	-38.5	0.77 (1)	10.00				
P-J	0 / 7055	-38.5	-38.5	0.77 (1)	10.00				
J-Q	0 / 7055	-38.5	-38.5	0.77 (1)	10.00				
O-R	0 / 7055	-38.5	-38.5	0.77 (1)	10.00				
R-I	0 / 7055	-38.5	-38.5	0.77 (1)	10.00				
I-S	0 / 3468	-38.5	-38.5	0.59 (1)	10.00				
S-T	0 / 3468	-38.5	-38.5	0.59 (1)	10.00				
T-U	0 / 3468	-38.5	-38.5	0.59 (1)	10.00				
U-H	0 / 3468	-38.5	-38.5	0.59 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
L	3-11-4	-2654	-2654	—	TOP	VERT	TOTAL	—	—
N	5-11-4	-1807	-1807	—	TOP	VERT	TOTAL	—	—
O	7-11-4	-1807	-1807	—	TOP	VERT	TOTAL	—	—
P	9-11-4	-1807	-1807	—	TOP	VERT	TOTAL	—	—

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.85")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.34/1.00 (C-E:1) , BC=0.83/1.00 (K-L:1) , WB=0.71/1.00 (F-H:1) , SSI=0.52/1.00 (I-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (E) (INPUT = 0.90)
JSI METAL= 0.56 (L) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
01/15/19
H. J. G. ALVES
100088724
PROVINCE OF ONTARIO

DWG NO. T317 01901189
STRUCTURAL
CHORD JOINT ONLY

1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300581	T317	1	4	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
Q	11-11-4	-1807	-1807	—	FRONT	VERT	TOTAL	—	—
R	13-11-4	-1807	-1807	—	TOP	VERT	TOTAL	—	—
S	15-11-4	-1807	-1807	—	FRONT	VERT	TOTAL	—	—
T	17-11-4	-1807	-1807	—	FRONT	VERT	TOTAL	—	—
U	19-1-4	-1820	-1820	—	FRONT	VERT	TOTAL	—	—

LICENSED PROFESSIONAL ENGINEER
1/15/19
H. J. G. ALVES
100[illegible]24
PROVINCE OF ONTARIO

DWG NO. T300581
STRUCTURAL
COPY ONLY
2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300581	T318	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Tue Jan 15 15:41:43 2019 Page 1
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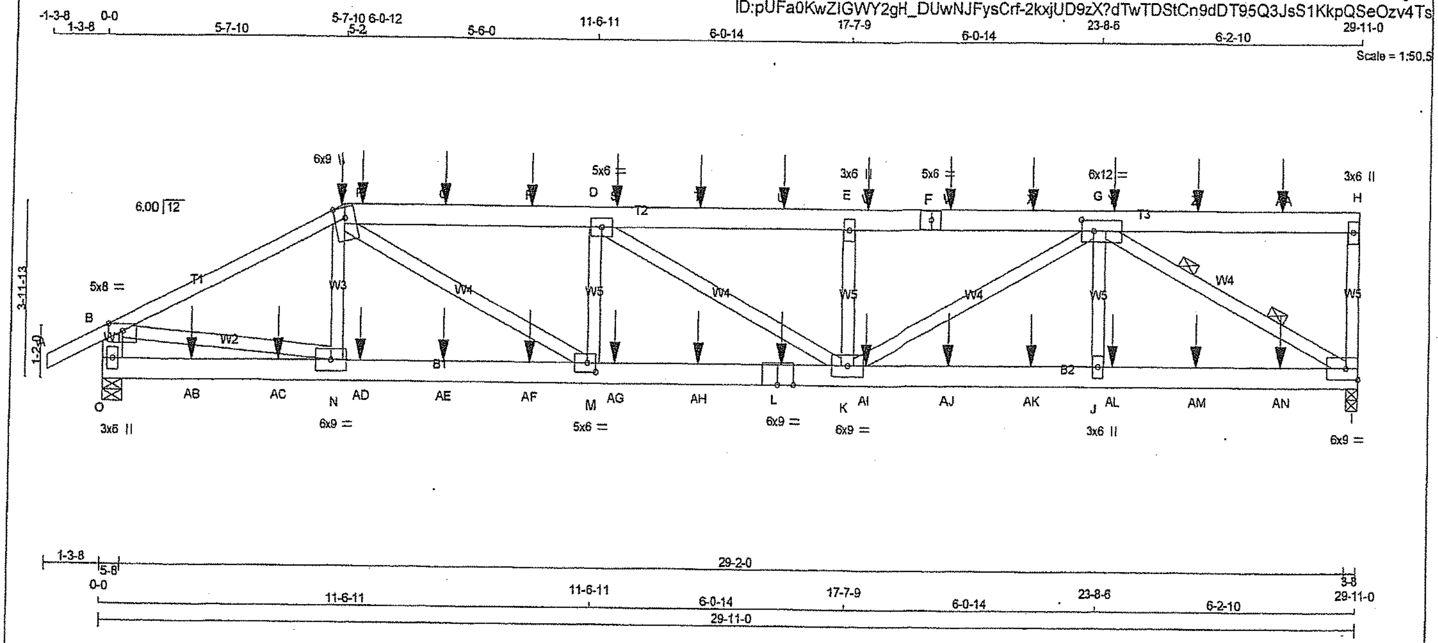


TOTAL WEIGHT = 160 lb
[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	1650F 1.5E	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - B	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	6.0	9.0	Edge	
D	TMWW-t	MT20	5.0	6.0		
E	TMW+w	MT20	3.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMWWW-t	MT20	6.0	12.0	3.00	3.75
H	TMV+p	MT20	3.0	6.0		
I	BMVW1-t	MT20	6.0	9.0		Edge
J	BMW+w	MT20	3.0	6.0		
K	BMWWW-t	MT20	6.0	9.0		
L	BS-t	MT20	6.0	9.0		
M	BMWW-t	MT20	5.0	6.0	2.50	2.50
N	BMWW-t	MT20	6.0	9.0		
O	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	2692	0	2692	0	0	3-8	3-8
O	2828	0	2828	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	2040	1022 / 0	453 / 0	0 / 0	0 / 0	565 / 0	0 / 0
O	2133	1098 / 0	451 / 0	0 / 0	0 / 0	584 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.14 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2 LATERAL BRACE(S) AT 1/ 3 LENGTH OF G-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 26	-84.9	-84.9	0.10 (1)	10.00
B-C	-4142 / 0	-84.9	-84.9	0.72 (1)	3.35
C-P	-5747 / 0	-84.9	-84.9	0.56 (1)	3.19
P-Q	-5747 / 0	-84.9	-84.9	0.58 (1)	3.19
Q-R	-5747 / 0	-84.9	-84.9	0.56 (1)	3.19
R-D	-5747 / 0	-84.9	-84.9	0.56 (1)	3.19
D-S	-5766 / 0	-84.9	-84.9	0.55 (1)	3.20
S-T	-5766 / 0	-84.9	-84.9	0.55 (1)	3.20
T-U	-5766 / 0	-84.9	-84.9	0.55 (1)	3.20
U-E	-5766 / 0	-84.9	-84.9	0.55 (1)	3.20
E-V	-5766 / 0	-84.9	-84.9	0.59 (1)	3.14
V-F	-5766 / 0	-84.9	-84.9	0.59 (1)	3.14
F-W	-5766 / 0	-84.9	-84.9	0.59 (1)	3.14
W-X	-5766 / 0	-84.9	-84.9	0.59 (1)	3.14
X-G	-5766 / 0	-84.9	-84.9	0.59 (1)	3.14
G-Y	0 / 0	-84.9	-84.9	0.31 (1)	10.00
Y-Z	0 / 0	-84.9	-84.9	0.31 (1)	10.00
Z-AA	0 / 0	-84.9	-84.9	0.31 (1)	10.00
AA-H	0 / 0	-84.9	-84.9	0.31 (1)	10.00
I-H	-286 / 0	0.0	0.0	0.06 (1)	7.81
O-B	-2713 / 0	0.0	0.0	0.19 (1)	6.29
O-AB	0 / 0	-38.5	-38.5	0.16 (3)	10.00
AB-AC	0 / 0	-38.5	-38.5	0.16 (3)	10.00
AC-N	0 / 0	-38.5	-38.5	0.16 (3)	10.00
N-AD	0 / 3720	-38.5	-38.5	0.60 (1)	10.00
AD-AE	0 / 3720	-38.5	-38.5	0.60 (1)	10.00
AE-AF	0 / 3720	-38.5	-38.5	0.60 (1)	10.00
AF-M	0 / 3720	-38.5	-38.5	0.60 (1)	10.00
M-AG	0 / 5747	-38.5	-38.5	0.87 (1)	10.00
AG-AH	0 / 5747	-38.5	-38.5	0.87 (1)	10.00
AH-L	0 / 5747	-38.5	-38.5	0.87 (1)	10.00
L-K	0 / 5747	-38.5	-38.5	0.87 (1)	10.00
K-AI	0 / 4000	-38.5	-38.5	0.63 (1)	10.00
AI-AJ	0 / 4000	-38.5	-38.5	0.63 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N-C	-171 / 188	0.03 (3)
B-N	0 / 3750	0.66 (1)
G-I	-4598 / 0	0.70 (1)
C-M	0 / 2364	0.42 (1)
J-G	0 / 493	0.09 (3)
M-D	-877 / 0	0.14 (1)
K-G	0 / 2043	0.36 (1)
D-K	0 / 47	0.01 (3)
K-E	-706 / 0	0.12 (1)

LICENSED PROFESSIONAL ENGINEER
01/15/19
H. J. G. ALVES
100009[illegible]4
PROVINCE OF ONTARIO

DWG NO. TMA 77901P10
STRUCTURAL
COMPONENT ONLY 1/2

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA O86-09, CSA O86-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.24")
ALLOWABLE DEFL.(TL)= L/360 (1.00")
CALCULATED VERT. DEFL.(TL) = L/ 870 (0.41")

CSI: TC=0.72/1.00 (B-C:1) , BC=0.87/1.00 (K-M:1) , WB=0.70/1.00 (G-I:1) , SSI=0.32/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	786	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (K) (INPUT = 0.90)
JSI METAL= 0.90 (L) (INPUT = 1.00)

CONTINUED ON PAGE 2

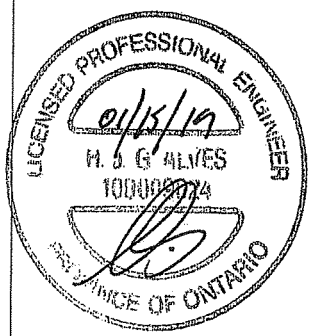
LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
AJ-AK	0 / 4000	-38.5	-38.5	0.63 (1)	10.00			
AK- J	0 / 4000	-38.5	-38.5	0.63 (1)	10.00			
J-AL	0 / 4000	-38.5	-38.5	0.67 (1)	10.00			
AL-AM	0 / 4000	-38.5	-38.5	0.67 (1)	10.00			
AM-AN	0 / 4000	-38.5	-38.5	0.67 (1)	10.00			
AN- I	0 / 4000	-38.5	-38.5	0.87 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-7-10	-41	-46	—	FRONT	VERT	DEAD	—	—
C	5-7-10	-189	-189	—	FRONT	VERT	SNOW	—	—
L	16-0-12	-36	-46	—	BACK	VERT	TOTAL	—	—
P	6-0-12	-104	-104	—	BACK	VERT	TOTAL	—	—
Q	8-0-12	-80	-80	—	BACK	VERT	TOTAL	—	—
R	10-0-12	-80	-80	—	BACK	VERT	TOTAL	—	—
S	12-0-12	-80	-80	—	BACK	VERT	TOTAL	—	—
T	14-0-12	-80	-80	—	BACK	VERT	TOTAL	—	—
U	16-0-12	-80	-80	—	BACK	VERT	TOTAL	—	—
V	18-0-12	-80	-80	—	BACK	VERT	TOTAL	—	—
W	20-0-12	-80	-80	—	BACK	VERT	TOTAL	—	—
X	22-0-12	-80	-80	—	BACK	VERT	TOTAL	—	—
Y	24-0-12	-80	-80	—	BACK	VERT	TOTAL	—	—
Z	26-0-12	-80	-80	—	BACK	VERT	TOTAL	—	—
AA	28-0-12	-80	-80	—	BACK	VERT	TOTAL	—	—
AB	2-0-12	-36	-46	—	BACK	VERT	TOTAL	—	—
AC	4-0-12	-36	-46	—	BACK	VERT	TOTAL	—	—
AD	6-0-12	-36	-46	—	BACK	VERT	TOTAL	—	—
AE	8-0-12	-36	-46	—	BACK	VERT	TOTAL	—	—
AF	10-0-12	-36	-46	—	BACK	VERT	TOTAL	—	—
AG	12-0-12	-36	-46	—	BACK	VERT	TOTAL	—	—
AH	14-0-12	-36	-46	—	BACK	VERT	TOTAL	—	—
AI	18-0-12	-36	-46	—	BACK	VERT	TOTAL	—	—
AJ	20-0-12	-36	-46	—	BACK	VERT	TOTAL	—	—
AK	22-0-12	-36	-46	—	BACK	VERT	TOTAL	—	—
AL	24-0-12	-36	-46	—	BACK	VERT	TOTAL	—	—
AM	26-0-12	-36	-46	—	BACK	VERT	TOTAL	—	—
AN	28-0-12	-36	-46	—	BACK	VERT	TOTAL	—	—



DWG NO. TAM T1901190
STRUCTURAL
COMPONENT ONLY 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300333	T318A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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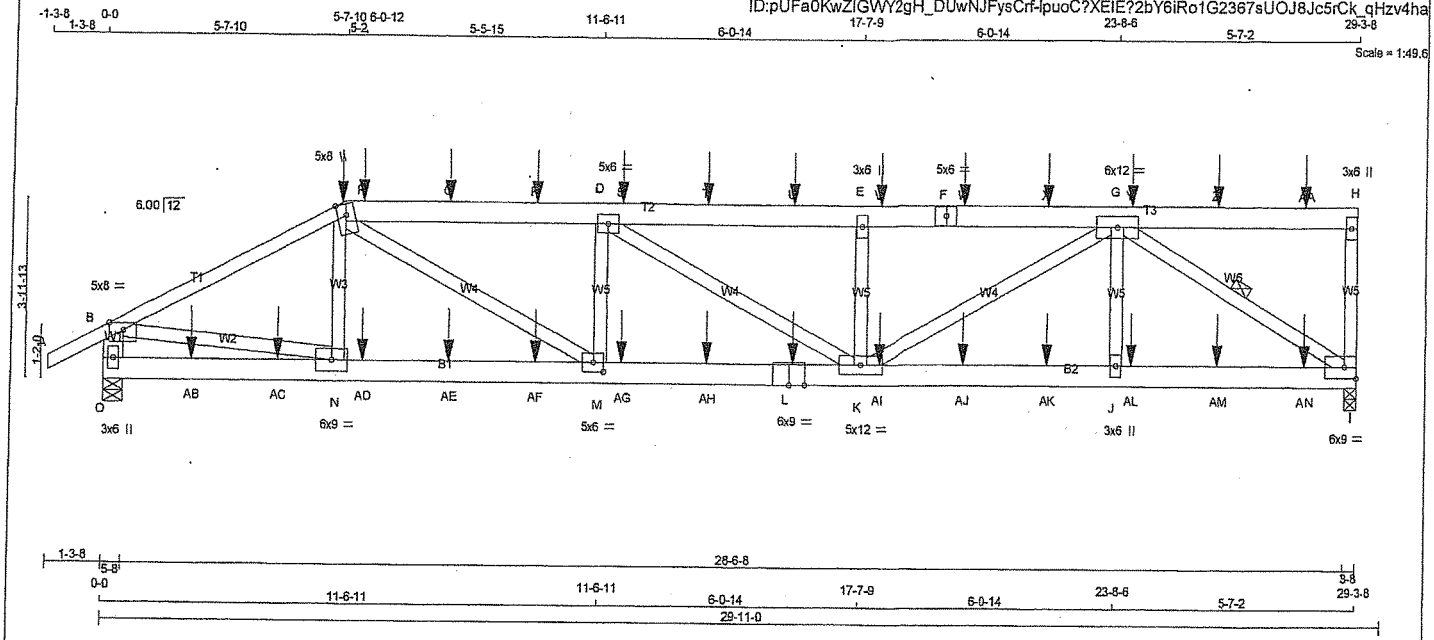


TOTAL WEIGHT = 157 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	1650F 1.5E	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - B	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	5.0	8.0	Edge	
O	TMWW-t	MT20	5.0	6.0		
E	TMW+w	MT20	3.0	6.0		
F	TS-t	MT20	5.0	8.0		
G	TMWWW-t	MT20	6.0	12.0		
H	TMV+p	MT20	3.0	6.0		
I	BMVW1-t	MT20	6.0	9.0		Edge
J	BMW+w	MT20	3.0	6.0		
K	BMWWW-t	MT20	5.0	12.0		
L	BS-t	MT20	6.0	9.0		
M	BMWW-t	MT20	5.0	6.0	2.50	2.75
N	BMWW-t	MT20	6.0	9.0		
O	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	2672	0	2672	0	0	3-8	3-8
O	2771	0	2771	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	2025	1014 / 0	449 / 0	0 / 0	0 / 0	561 / 0	0 / 0
O	2090	1076 / 0	442 / 0	0 / 0	0 / 0	572 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

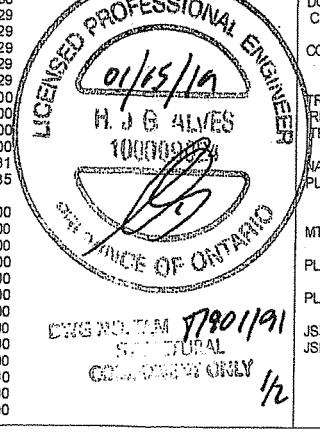
LOADING
TOTAL LOAD CASES. (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 26	-84.9	-84.9	0.10 (1)		10.00
B- C	-4043 / 0	-84.9	-84.9	0.71 (1)		3.39
C- P	-5561 / 0	-84.9	-84.9	0.54 (1)		3.25
P- O	-5561 / 0	-84.9	-84.9	0.54 (1)		3.25
Q- R	-5561 / 0	-84.9	-84.9	0.54 (1)		3.25
R- D	-5561 / 0	-84.9	-84.9	0.54 (1)		3.25
D- S	-5489 / 0	-84.9	-84.9	0.53 (1)		3.30
S- T	-5489 / 0	-84.9	-84.9	0.53 (1)		3.30
T- U	-5489 / 0	-84.9	-84.9	0.53 (1)		3.30
U- E	-5489 / 0	-84.9	-84.9	0.53 (1)		3.30
E- V	-5489 / 0	-84.9	-84.9	0.53 (1)		3.29
V- F	-5489 / 0	-84.9	-84.9	0.53 (1)		3.29
F- W	-5489 / 0	-84.9	-84.9	0.53 (1)		3.29
W- X	-5489 / 0	-84.9	-84.9	0.53 (1)		3.29
X- G	-5489 / 0	-84.9	-84.9	0.53 (1)		3.29
G- Y	0 / 0	-84.9	-84.9	0.28 (1)		10.00
Y- Z	0 / 0	-84.9	-84.9	0.28 (1)		10.00
Z-AA	0 / 0	-84.9	-84.9	0.28 (1)		10.00
AA- H	0 / 0	-84.9	-84.9	0.28 (1)		10.00
I- H	-270 / 0	0.0	0.0	0.06 (1)		7.81
O- B	-2657 / 0	0.0	0.0	0.19 (1)		6.35
O-AB	0 / 0	-38.5	-38.5	0.16 (3)		10.00
AB-AC	0 / 0	-38.5	-38.5	0.16 (3)		10.00
AC- N	0 / 0	-38.5	-38.5	0.16 (3)		10.00
N-AD	0 / 3631	-38.5	-38.5	0.58 (1)		10.00
AD-AE	0 / 3631	-38.5	-38.5	0.58 (1)		10.00
AE-AF	0 / 3631	-38.5	-38.5	0.58 (1)		10.00
AF- M	0 / 3631	-38.5	-38.5	0.58 (1)		10.00
M-AG	0 / 5561	-38.5	-38.5	0.85 (1)		10.00
AG-AH	0 / 5561	-38.5	-38.5	0.85 (1)		10.00
AH- L	0 / 5561	-38.5	-38.6	0.85 (1)		10.00
L- K	0 / 5561	-38.5	-38.5	0.85 (1)		10.00
K-AI	0 / 3603	-38.5	-38.5	0.58 (1)		10.00
AI-AJ	0 / 3603	-38.5	-38.5	0.58 (1)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- C	-158 / 195	0.04 (3)
B- N	0 / 3660	0.65 (1)
G- I	-4260 / 0	0.89 (1)
C- M	0 / 2252	0.40 (1)
J- G	0 / 465	0.09 (3)
M- D	-820 / 0	0.13 (1)
K- G	0 / 2181	0.39 (1)
D- K	-84 / 0	0.05 (1)
K- E	-718 / 0	0.12 (3)



DWG NO. T318A 1/15/01/19
STRUCTURAL
GIRDER ONLY
1/2

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.98")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.22")
ALLOWABLE DEFL.(TL)= L/360 (0.98")
CALCULATED VERT. DEFL.(TL) = L/ 924 (0.38")

CSI: TC=0.71/1.00 (B-C:1) , BC=0.85/1.00 (K-M:1) , WB=0.89/1.00 (G-I:1) , SSI=0.29/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (G) (INPUT = 0.90)
JSI METAL= 0.87 (L) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300333	T318A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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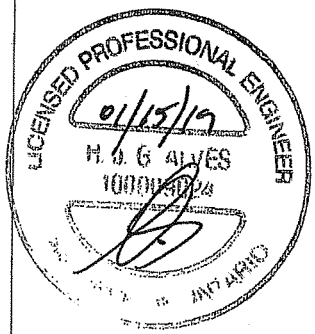
LOADING
TOTAL LOAD CASES. (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
AJ-AK	0 / 3603	-38.5	-38.5	0.58 (1)	10.00			
AK- J	0 / 3603	-38.5	-38.5	0.58 (1)	10.00			
J-AL	0 / 3603	-38.5	-38.5	0.58 (1)	10.00			
AL-AM	0 / 3603	-38.5	-38.5	0.58 (1)	10.00			
AM-AN	0 / 3603	-38.5	-38.5	0.58 (1)	10.00			
AN- I	0 / 3603	-38.5	-38.5	0.58 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-7-10	-41	-46	—	FRONT	VERT	DEAD	—	—
C	5-7-10	-189	-189	—	FRONT	VERT	SNOW	—	—
L	16-0-12	-36	-46	—	FRONT	VERT	TOTAL	—	—
P	6-0-12	-104	-104	—	FRONT	VERT	TOTAL	—	—
Q	8-0-12	-80	-80	—	FRONT	VERT	TOTAL	—	—
R	10-0-12	-80	-80	—	FRONT	VERT	TOTAL	—	—
S	12-0-12	-80	-80	—	FRONT	VERT	TOTAL	—	—
T	14-0-12	-80	-80	—	FRONT	VERT	TOTAL	—	—
U	16-0-12	-80	-80	—	FRONT	VERT	TOTAL	—	—
V	18-0-12	-80	-80	—	FRONT	VERT	TOTAL	—	—
W	20-0-12	-80	-80	—	FRONT	VERT	TOTAL	—	—
X	22-0-12	-80	-80	—	FRONT	VERT	TOTAL	—	—
Y	24-0-12	-80	-80	—	FRONT	VERT	TOTAL	—	—
Z	26-0-12	-80	-80	—	FRONT	VERT	TOTAL	—	—
AA	28-0-12	-80	-80	—	FRONT	VERT	TOTAL	—	—
AB	2-0-12	-36	-46	—	FRONT	VERT	TOTAL	—	—
AC	4-0-12	-36	-46	—	FRONT	VERT	TOTAL	—	—
AD	6-0-12	-36	-46	—	FRONT	VERT	TOTAL	—	—
AE	8-0-12	-36	-46	—	FRONT	VERT	TOTAL	—	—
AF	10-0-12	-36	-46	—	FRONT	VERT	TOTAL	—	—
AG	12-0-12	-36	-46	—	FRONT	VERT	TOTAL	—	—
AH	14-0-12	-36	-46	—	FRONT	VERT	TOTAL	—	—
AI	18-0-12	-36	-46	—	FRONT	VERT	TOTAL	—	—
AJ	20-0-12	-36	-46	—	FRONT	VERT	TOTAL	—	—
AK	22-0-12	-36	-46	—	FRONT	VERT	TOTAL	—	—
AL	24-0-12	-36	-46	—	FRONT	VERT	TOTAL	—	—
AM	26-0-12	-36	-46	—	FRONT	VERT	TOTAL	—	—
AN	28-0-12	-36	-46	—	FRONT	VERT	TOTAL	—	—



DWG NO. T.M. T7901191
STRUCTURAL
COMPONENT ONLY 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300581	T319	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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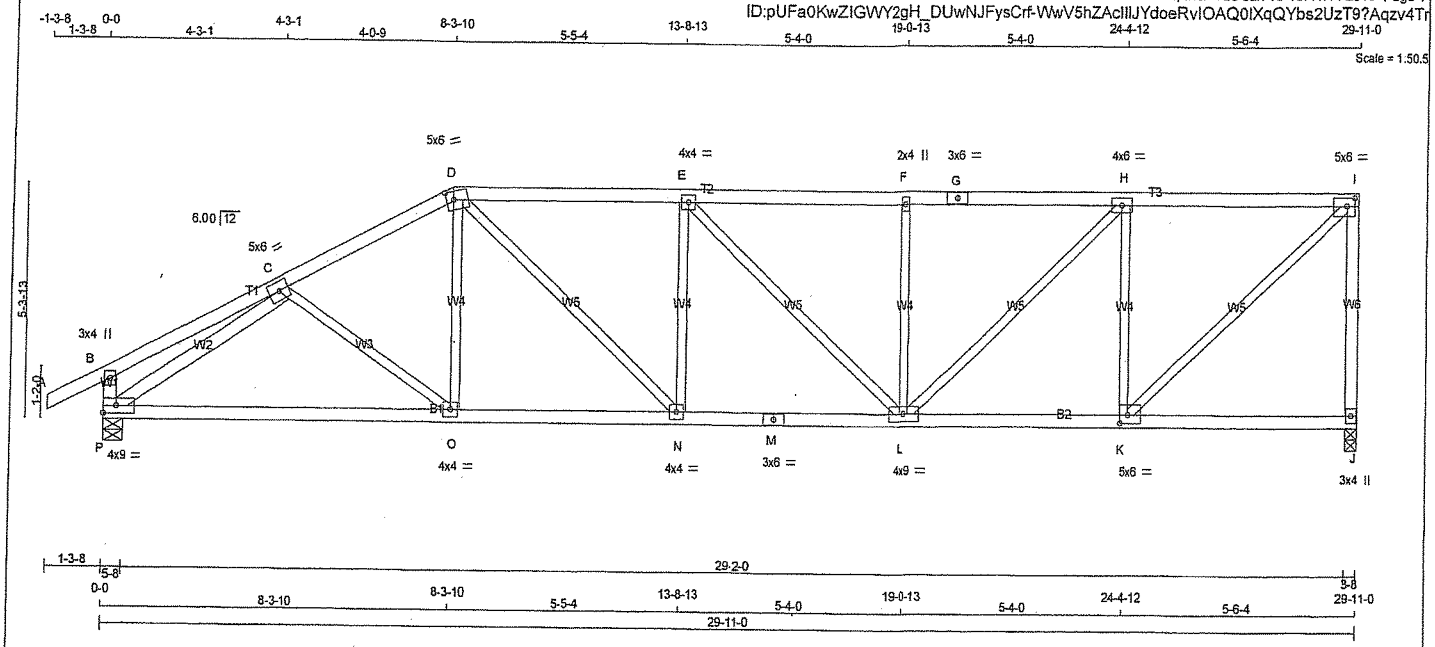

TOTAL WEIGHT = 125 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
P - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	6.0		
I	TMVW-t	MT20	5.0	6.0	2.25	2.25
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0	2.25	2.25
L	BMWWW-t	MT20	4.0	9.0		
M	BS-t	MT20	3.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMVW1-t	MT20	4.0	9.0		Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1845	0	1845	0	0	3-8	3-8
P	1960	0	1960	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
		MAX./MIN. COMPONENT REACTIONS					
J	1400	697 / 0	314 / 0	0 / 0	0 / 0	389 / 0	0 / 0
P	1479	760 / 0	314 / 0	0 / 0	0 / 0	405 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, P

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.67 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 26	-84.9	-84.9	0.11 (1)	10.00
B-C	0 / 16	-84.9	-84.9	0.21 (1)	10.00
C-D	-2534 / 0	-84.9	-84.9	0.31 (1)	4.06
D-E	-2786 / 0	-84.9	-84.9	0.53 (1)	3.67
E-F	-2585 / 0	-84.9	-84.9	0.51 (1)	3.81
F-G	-2585 / 0	-84.9	-84.9	0.52 (1)	3.80
G-H	-2585 / 0	-84.9	-84.9	0.52 (1)	3.80
H-I	-1725 / 0	-84.9	-84.9	0.44 (1)	4.58
J-I	-1759 / 0	0.0	0.0	0.87 (1)	6.29
P-B	-256 / 0	0.0	0.0	0.03 (1)	7.81
P-O	0 / 2227	-38.5	-38.5	0.73 (2)	10.00
O-N	0 / 2256	-38.5	-38.5	0.75 (2)	10.00
N-M	0 / 2788	-38.5	-38.5	0.55 (1)	10.00
M-L	0 / 2786	-38.5	-38.5	0.55 (1)	10.00
L-K	0 / 1725	-38.5	-38.5	0.44 (2)	10.00
K-J	0 / 0	-38.5	-38.5	0.21 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-O	0 / 151	0.04 (3)
O-D	0 / 387	0.08 (2)
P-C	-2725 / 0	0.84 (1)
K-I	0 / 2334	0.53 (1)
D-N	0 / 721	0.16 (1)
K-H	-1337 / 0	0.56 (1)
N-E	-327 / 0	0.14 (1)
L-H	0 / 1182	0.27 (1)
E-L	-276 / 0	0.29 (1)
L-F	-415 / 0	0.17 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.22")
ALLOWABLE DEFL.(TL)= L/360 (1.00")
CALCULATED VERT. DEFL.(TL) = L/ 953 (0.38")

CSI: TC=0.87/1.00 (I-J:1) , BC=0.75/1.00 (N-O:2) , WB=0.84/1.00 (C-P:1) , SSI=0.22/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (K) (INPUT = 0.90)
JSI METAL= 0.99 (M) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
01/15/19
H J G ALVES
100009684
PROVINCE OF ONTARIO

DWG NO. TAM 11901/92
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300333	T319A	1	1	TRUSS DESC	

Tamarack Roof Truss, Burlington

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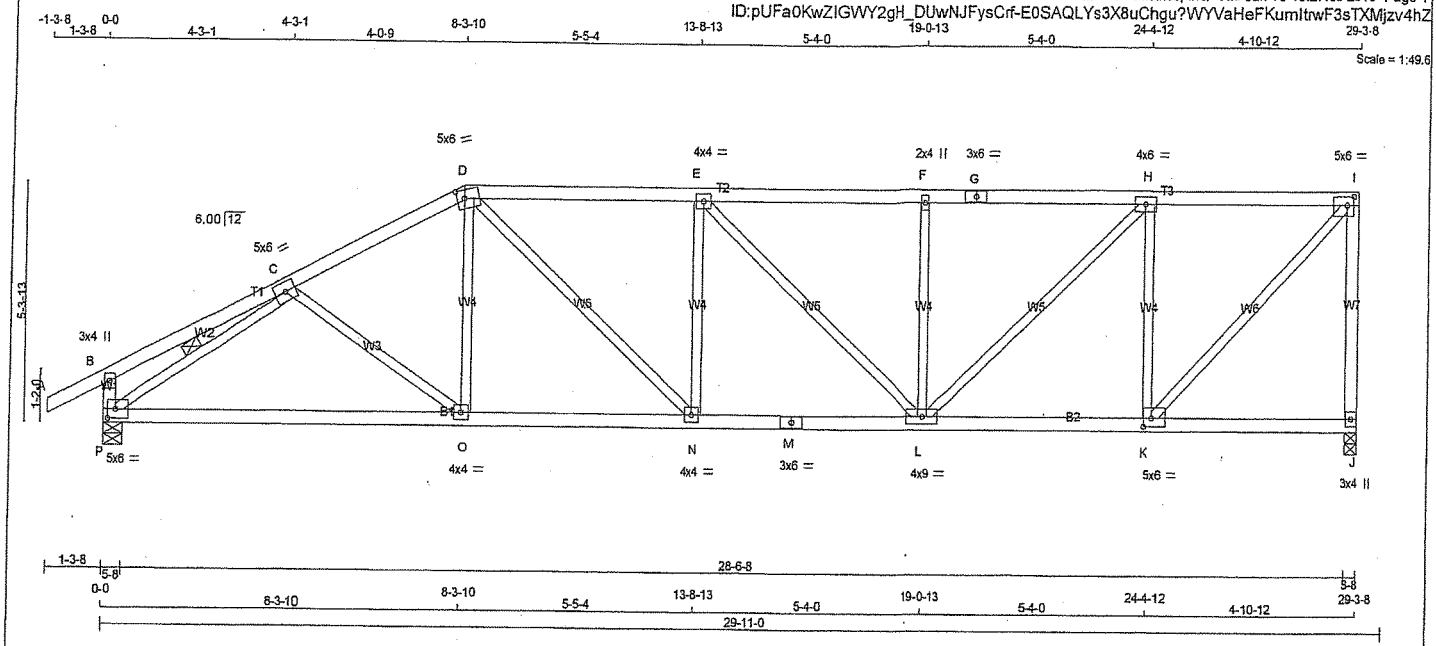


TOTAL WEIGHT = 122 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
O	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	6.0		
I	TMVW-t	MT20	5.0	6.0	2.25	2.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0	2.25	2.25
L	BMWWW-t	MT20	4.0	9.0		
M	BS-t	MT20	3.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMVW1-t	MT20	5.0	6.0	2.50	2.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
J	1807	0	1807	0	0	3-8	3-8
P	1922	0	1922	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	1370	662 / 0	308 / 0	0 / 0	0 / 0	381 / 0	0 / 0
P	1450	745 / 0	308 / 0	0 / 0	0 / 0	397 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, P

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-P.

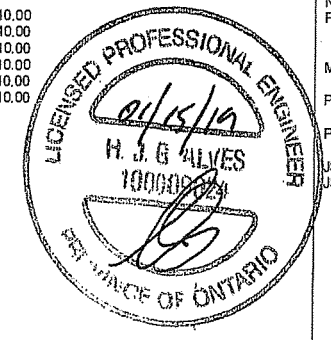
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 26	-84.9	-84.9	0.11 (1)	10.00
B-C	0 / 16	-84.9	-84.9	0.20 (1)	10.00
C-D	-2462 / 0	-84.9	-84.9	0.30 (1)	4.12
D-E	-2680 / 0	-84.9	-84.9	0.52 (1)	3.75
E-F	-2442 / 0	-84.9	-84.9	0.49 (1)	3.93
F-G	-2442 / 0	-84.9	-84.9	0.44 (1)	3.99
G-H	-2442 / 0	-84.9	-84.9	0.44 (1)	3.99
H-I	-1528 / 0	-84.9	-84.9	0.36 (1)	4.91
J-I	-1732 / 0	0.0	0.0	0.85 (1)	6.33
P-B	-257 / 0	0.0	0.0	0.03 (1)	7.81
P-O	0 / 2170	-38.5	-38.5	0.72 (2)	10.00
O-N	0 / 2192	-38.5	-38.5	0.74 (2)	10.00
N-M	0 / 2680	-38.5	-38.5	0.54 (1)	10.00
M-L	0 / 2680	-38.5	-38.5	0.54 (1)	10.00
L-K	0 / 1528	-38.5	-38.5	0.38 (2)	10.00
K-J	0 / 0	-38.5	-38.5	0.18 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-O	0 / 147	0.03 (3)
O-D	0 / 371	0.08 (2)
P-C	-2656 / 0	0.56 (1)
K-I	0 / 2190	0.49 (1)
D-N	0 / 664	0.15 (1)
K-H	-1351 / 0	0.57 (1)
N-E	-290 / 15	0.12 (1)
L-H	0 / 1257	0.28 (1)
E-L	-327 / 0	0.34 (1)
L-F	-426 / 0	0.18 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.98")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.22")
ALLOWABLE DEFL.(TL)= L/360 (0.98")
CALCULATED VERT. DEFL.(TL) = L/ 932 (0.38")

CSI: TC=0.85/1.00 (I-J:1) , BC=0.74/1.00 (N-O:2) , WB=0.57/1.00 (H-K:1) , SSI=0.22/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (I) (INPUT = 0.90)
JSI METAL= 0.95 (M) (INPUT = 1.00)

DWG NO. TAM T1901193
STRUCTURAL
COPY FOR SPOT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300581	T320	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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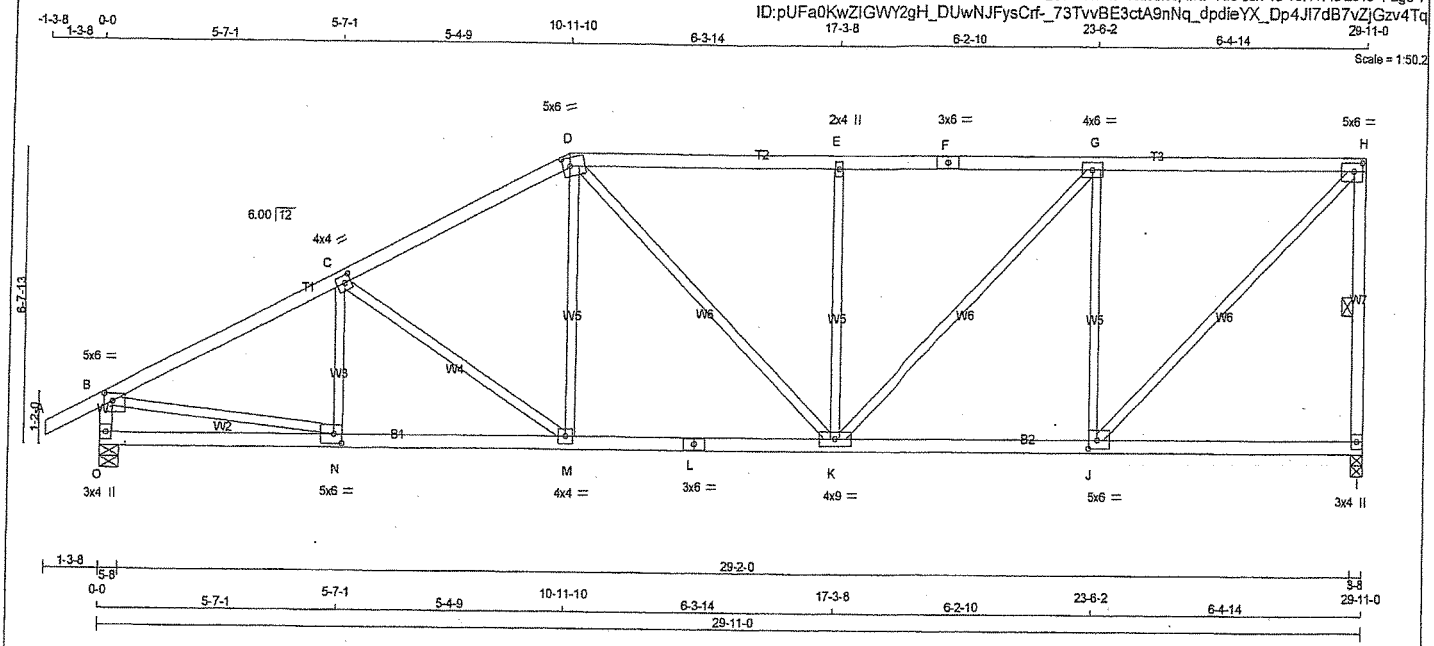


TOTAL WEIGHT = 127 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	2.00	2.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	6.0		
H	TMVW-t	MT20	5.0	6.0	2.25	2.50
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	5.0	6.0	2.25	2.50
K	BMWWW-t	MT20	4.0	9.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMWW-t	MT20	5.0	6.0	2.50	2.25
O	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN,	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	1845	0	1845	0	0	3-8	3-8
O	1960	0	1960	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1400	697 / 0	314 / 0	0 / 0	0 / 0	389 / 0	0 / 0
O	1479	760 / 0	314 / 0	0 / 0	0 / 0	405 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.67 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF H-I.

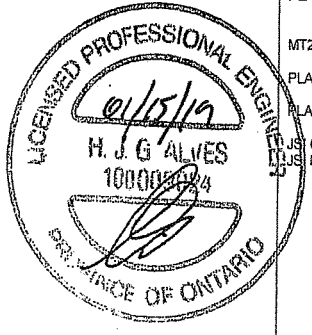
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 26	-84.9	-84.9	0.11 (1)	10.00
B- C	-2661 / 0	-84.9	-84.9	0.62 (1)	3.67
C- D	-2295 / 0	-84.9	-84.9	0.56 (1)	3.95
D- E	-2184 / 0	-84.9	-84.9	0.61 (1)	3.97
E- F	-2184 / 0	-84.9	-84.9	0.63 (1)	3.94
F- G	-2184 / 0	-84.9	-84.9	0.63 (1)	3.94
G- H	-1537 / 0	-84.9	-84.9	0.56 (1)	4.62
I- H	-1746 / 0	0.0	0.0	0.35 (1)	5.04
O- B	-1869 / 0	0.0	0.0	0.19 (1)	6.13
O- N	0 / 0	-38.5	-38.5	0.20 (3)	10.00
N- M	0 / 2401	-38.5	-38.5	0.54 (1)	10.00
M- L	0 / 2038	-38.5	-38.5	0.48 (2)	10.00
L- K	0 / 2038	-38.5	-38.5	0.48 (2)	10.00
K- J	0 / 1537	-38.5	-38.5	0.49 (2)	10.00
J- I	0 / 0	-38.5	-38.5	0.29 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- C	-143 / 106	0.03 (1)
C- M	-446 / 0	0.35 (1)
M- D	0 / 505	0.11 (2)
B- N	0 / 2429	0.55 (1)
J- H	0 / 2166	0.49 (1)
D- K	0 / 207	0.05 (1)
J- G	-1251 / 0	0.91 (1)
K- E	-578 / 0	0.42 (1)
K- G	0 / 925	0.21 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (1.00")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI. TC=0.63/1.00 (E-G:1) , BC=0.54/1.00 (M-N:1) , WB=0.91/1.00 (G-J:1) , SSI=0.25/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JS GRIP= 0.89 (H) (INPUT = 0.90)
JS METAL= 0.78 (L) (INPUT = 1.00)

DWG NO. TAM 1790194

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300333	T320A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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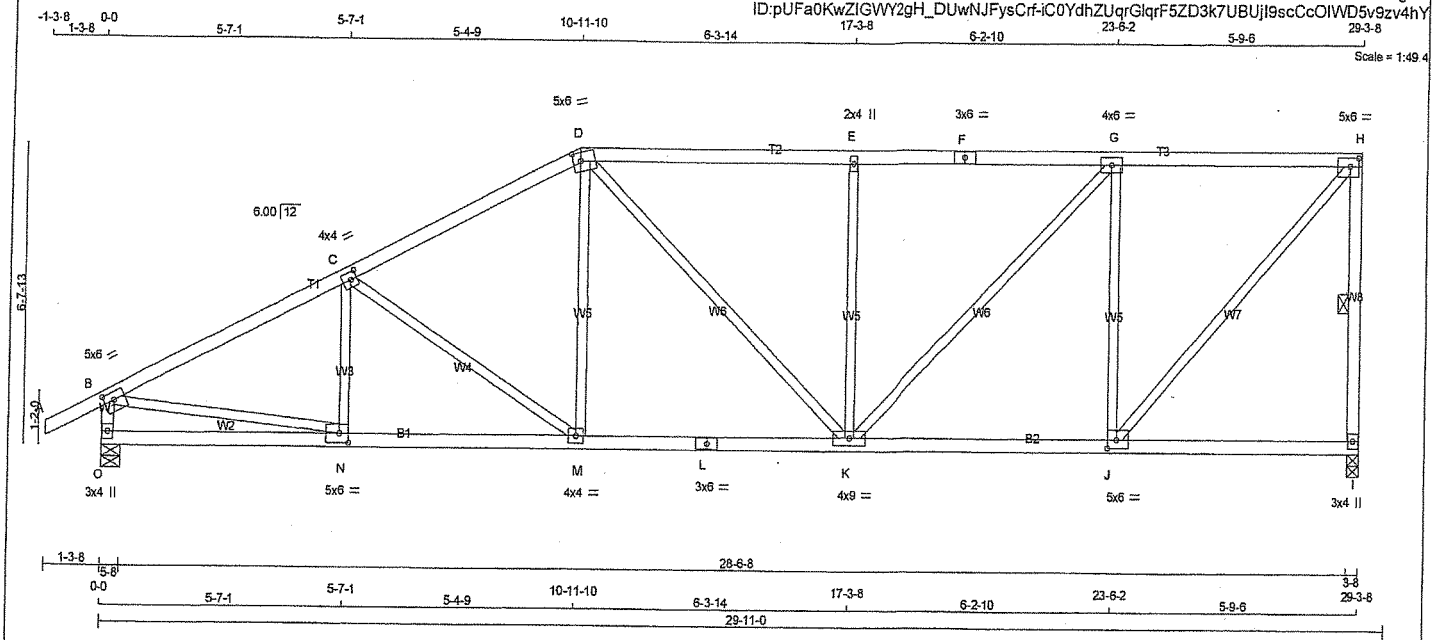


TOTAL WEIGHT = 125 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	ORY	No.2	SPF
D - F	2x4	ORY	No.2	SPF
F - H	2x4	ORY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
O - L	2x4	ORY	No.2	SPF
L - I	2x4	ORY	No.2	SPF
ALL WEBS EXCEPT	2x3	ORY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.00	2.75
C	TMWW-t	MT2D	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT2D	3.0	6.0		
G	TMWW-t	MT2D	4.0	6.0		
H	TMVW-t	MT2D	5.0	6.0	2.25	2.50
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	5.0	6.0	2.25	2.50
K	BMWWW-t	MT2D	4.0	9.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT2D	4.0	4.0		
N	BMWW-t	MT20	5.0	6.0	2.50	2.50
O	BMV1+p	MT2D	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REORD BRG
I	1807	0	1807	0	0	3-8	3-8
O	1922	0	1922	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1370	682 / 0	308 / D	0 / 0	0 / 0	381 / 0	0 / 0
O	1450	745 / 0	308 / 0	0 / 0	0 / 0	397 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.72 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF H-I.

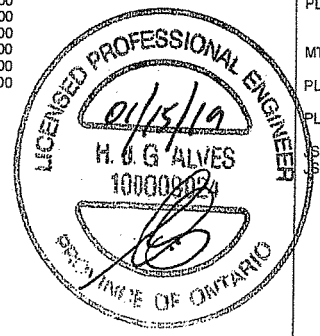
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 26	-84.9	-84.9	0.11 (1)	10.00
B-C	-2595 / 0	-84.9	-84.9	0.61 (1)	3.72
C-D	-2221 / 0	-84.9	-84.9	0.55 (1)	4.01
D-E	-2082 / 0	-84.9	-84.9	0.62 (1)	4.03
E-F	-2082 / 0	-84.9	-84.9	0.61 (1)	4.03
F-G	-2082 / 0	-84.9	-84.9	0.61 (1)	4.03
G-H	-1384 / 0	-84.9	-84.9	0.46 (1)	4.95
I-H	-1719 / 0	0.0	0.0	0.35 (1)	5.08
O-B	-1831 / 0	0.0	0.0	0.19 (1)	6.19
O-N	0 / 0	-38.5	-38.5	0.20 (3)	10.00
N-M	0 / 2342	-38.5	-38.5	0.52 (1)	10.00
M-L	0 / 1971	-38.5	-38.5	0.48 (2)	10.00
L-K	0 / 1971	-38.5	-38.5	0.48 (2)	10.00
K-J	0 / 1384	-38.5	-38.5	0.44 (2)	10.00
J-I	0 / 0	-38.5	-38.5	0.25 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N-C	-133 / 111	0.03 (1)
C-M	-456 / 0	0.36 (1)
M-D	0 / 508	0.11 (2)
B-N	0 / 2370	0.53 (1)
J-H	0 / 2058	0.46 (1)
D-K	0 / 157	0.04 (1)
J-G	-1265 / 0	0.92 (1)
K-E	-589 / 0	0.43 (1)
K-G	0 / 998	0.22 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.98")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.98")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.62/1.00 (D-E:1) , BC=0.52/1.00 (M-N:1) , WB=0.92/1.00 (G-J:1) , SSI=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.90 (J) (INPUT = 0.90)
SI METAL= 0.76 (L) (INPUT = 1.00)

DWG NO. T320A 17901195

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300581	T321	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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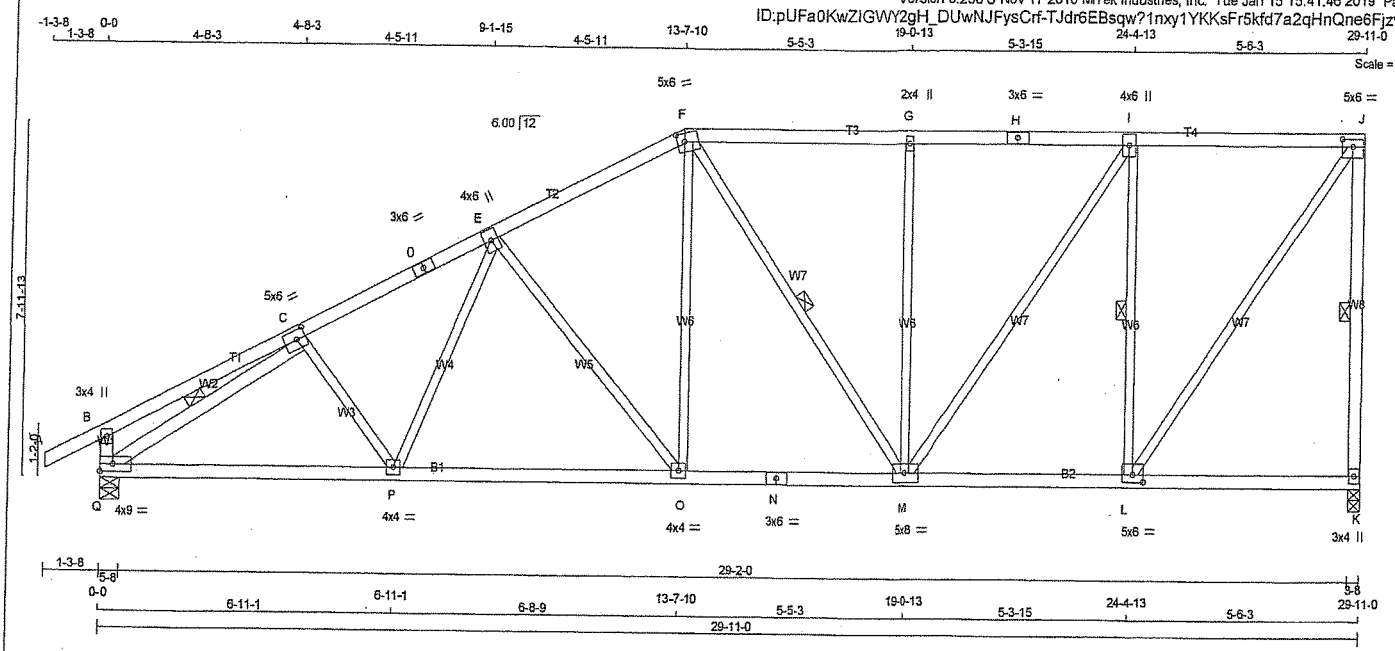

TOTAL WEIGHT = 140 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
Q - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.50
O	TS-t	MT20	3.0	6.0		
E	TMWW+t	MT20	4.0	6.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMW+w	MT20	2.0	4.0		
H	TS-t	MT20	3.0	6.0		
I	TMWW+t	MT20	4.0	6.0		
J	TMVW-t	MT20	5.0	6.0	2.00	3.00
K	BMV1+p	MT20	3.0	4.0		
L	BMWW-t	MT20	5.0	6.0	2.00	3.00
M	BMWWW-t	MT20	5.0	8.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	4.0	4.0		
O	BMVW1-t	MT20	4.0	9.0		Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTOREO GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REORD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
K	1845	0	1845	0	0	3-8	3-8
Q	1960	0	1960	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
K	1400	697 / 0	314 / 0	0 / 0	0 / 0	389 / 0	0 / 0
Q	1479	760 / 0	314 / 0	0 / 0	0 / 0	405 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, Q

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.01 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF J-K, C-Q, F-M, I-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 26	-84.9	-84.9	0.11 (1)		10.00
B- C	0 / 15	-84.9	-84.9	0.21 (1)		10.00
C- D	-2568 / 0	-84.9	-84.9	0.35 (1)		4.01
D- E	-2568 / 0	-84.9	-84.9	0.35 (1)		4.01
E- F	-2018 / 0	-84.9	-84.9	0.31 (1)		4.47
F- G	-1698 / 0	-84.9	-84.9	0.40 (1)		4.67
G- H	-1698 / 0	-84.9	-84.9	0.41 (1)		4.64
H- I	-1698 / 0	-84.9	-84.9	0.41 (1)		4.64
I- J	-1126 / 0	-84.9	-84.9	0.37 (1)		5.49
K- J	-1760 / 0	0.0	0.0	0.51 (1)		5.03
O- B	-276 / 0	0.0	0.0	0.03 (1)		7.81
O- P	0 / 2291	-38.5	-38.5	0.65 (2)		10.00
P- O	0 / 2181	-38.5	-38.5	0.63 (2)		10.00
O- N	0 / 1798	-38.5	-38.5	0.45 (2)		10.00
N- M	0 / 1798	-38.5	-38.5	0.45 (2)		10.00
M- L	0 / 1126	-38.5	-38.5	0.37 (2)		10.00
L- K	0 / 0	-38.5	-38.5	0.22 (3)		10.00

WEBS

MEMB. FR-TO	MAX. FACTOREO FORCE (LBS)	MAX. CSI (LC)
C- P	0 / 132	0.03 (3)
P- E	0 / 352	0.08 (2)
E- O	-608 / 0	0.59 (1)
O- F	0 / 696	0.16 (1)
O- C	-2761 / 0	0.48 (1)
L- J	0 / 1932	0.43 (1)
F- M	-173 / 0	0.11 (1)
L- I	-1331 / 0	0.53 (1)
M- G	-495 / 0	0.60 (1)
M- I	0 / 1003	0.23 (1)

LICENSED PROFESSIONAL ENGINEER
01/15/19
H. J. G. ALVES
100089220
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL	=	23.3 PSF
		DL	=	6.0 PSF
BOT	CH.	LL	=	10.5 PSF
		DL	=	7.0 PSF
TOTAL LOAD			=	46.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (1.00")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.51/1.00 (J-K:1) , BC=0.65/1.00 (P-Q:2) , WB=0.60/1.00 (G-M:1) , SSI=0.22/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SSI GRIP= 0.90 (C) (INPUT = 0.90)
SSI METAL= 0.69 (C) (INPUT = 1.00)

ENGINEERING BY 7190196
STRUCTURAL
COMPONENTS ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300333	T321A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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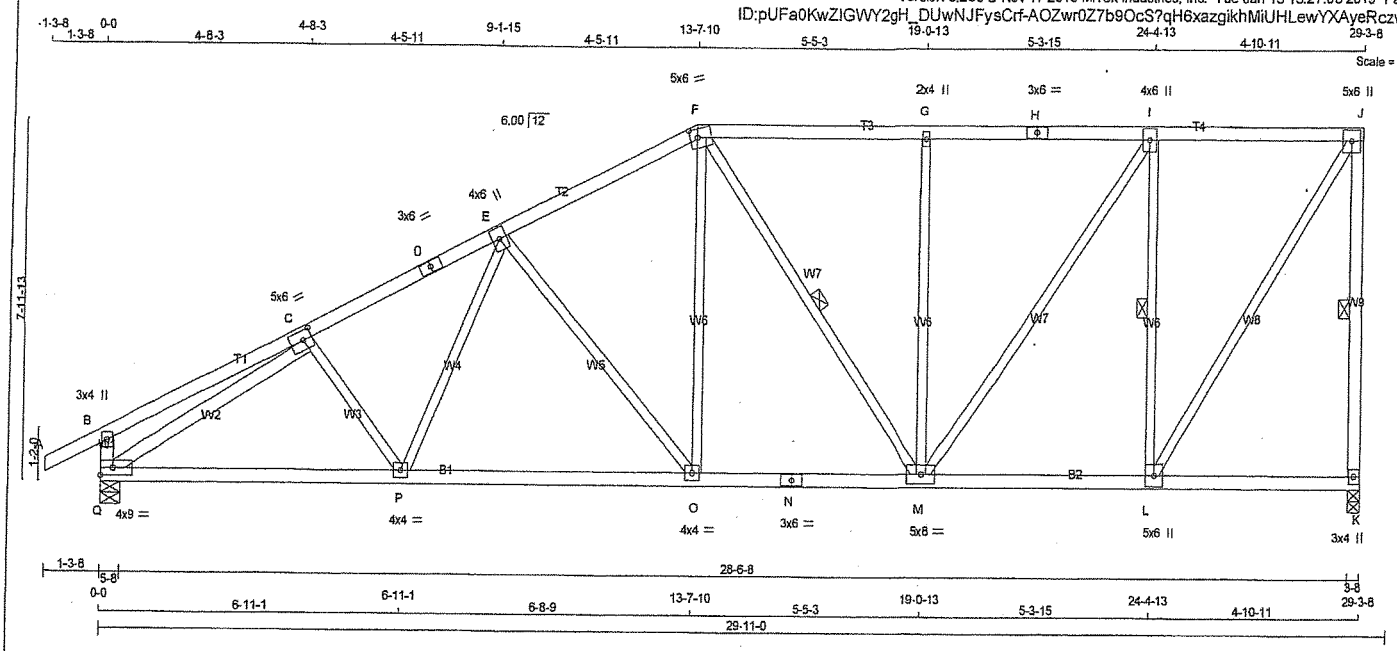


TOTAL WEIGHT = 138 lb [M]

LUMBER

N L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
Q - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.50
D	TS-t	MT20	3.0	6.0		
E	TMWW+t	MT20	4.0	6.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMW+w	MT20	2.0	4.0		
H	TS-t	MT20	3.0	6.0		
I	TMWW+t	MT20	4.0	6.0		
J	TMVW+p	MT20	5.0	6.0		
K	BMV1+p	MT20	3.0	4.0		
L	BMWW+t	MT20	5.0	6.0		
M	BMWWW-t	MT20	5.0	8.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	4.0	4.0		
O	BMVW1-t	MT20	4.0	9.0		Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
K	1807	0	1807	0	0	3-8	3-8
Q	1922	0	1922	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	1370	682 / 0	308 / 0	0 / 0	0 / 0	381 / 0	0 / 0
O	1450	745 / 0	308 / 0	0 / 0	0 / 0	397 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.06 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF J-K, F-M, I-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

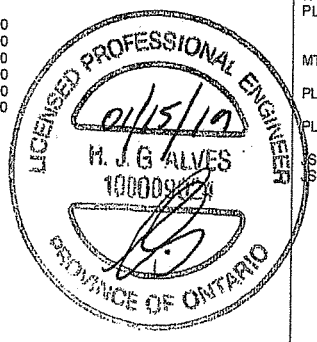
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 26	-84.9	-84.9	0.11 (1)	10.00
B- C	0 / 15	-84.9	-84.9	0.21 (1)	10.00
C- D	-2499 / 0	-84.9	-84.9	0.35 (1)	4.06
D- E	-2499 / 0	-84.9	-84.9	0.35 (1)	4.06
E- F	-1941 / 0	-84.9	-84.9	0.30 (1)	4.55
F- G	-1604 / 0	-84.9	-84.9	0.40 (1)	4.75
G- H	-1604 / 0	-84.9	-84.9	0.40 (1)	4.75
H- I	-1604 / 0	-84.9	-84.9	0.40 (1)	4.75
I- J	-996 / 0	-84.9	-84.9	0.30 (1)	5.85
K- J	-1733 / 0	0.0	0.0	0.50 (1)	5.06
O- B	-276 / 0	0.0	0.0	0.03 (1)	7.81
O- P	0 / 2236	-38.5	-38.5	0.64 (2)	10.00
P- O	0 / 2116	-38.5	-38.5	0.82 (2)	10.00
O- N	0 / 1729	-38.5	-38.5	0.44 (2)	10.00
N- M	0 / 1729	-38.5	-38.5	0.44 (2)	10.00
M- L	0 / 996	-38.5	-38.5	0.32 (2)	10.00
L- K	0 / 0	-38.5	-38.5	0.19 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- P	0 / 127	0.03 (3)
P- E	0 / 360	0.08 (2)
E- O	-614 / 0	0.60 (1)
O- F	0 / 699	0.16 (1)
Q- C	-2713 / 0	0.98 (1)
L- J	0 / 1857	0.42 (1)
F- M	-217 / 0	0.13 (1)
L- I	-1345 / 0	0.54 (1)
M- G	-507 / 0	0.61 (1)
M- I	0 / 1067	0.24 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, N8CC 2010, N8CC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.98")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.98")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.50/1.00 (J-K:1) , BC=0.64/1.00 (P-Q:2) , WB=0.98/1.00 (C-Q:1) , SSI=0.21/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.89 (J) (INPUT = 0.90)
SI METAL= 0.67 (C) (INPUT = 1.00)

ENGINEERED FOR 1190497 STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300581	T322	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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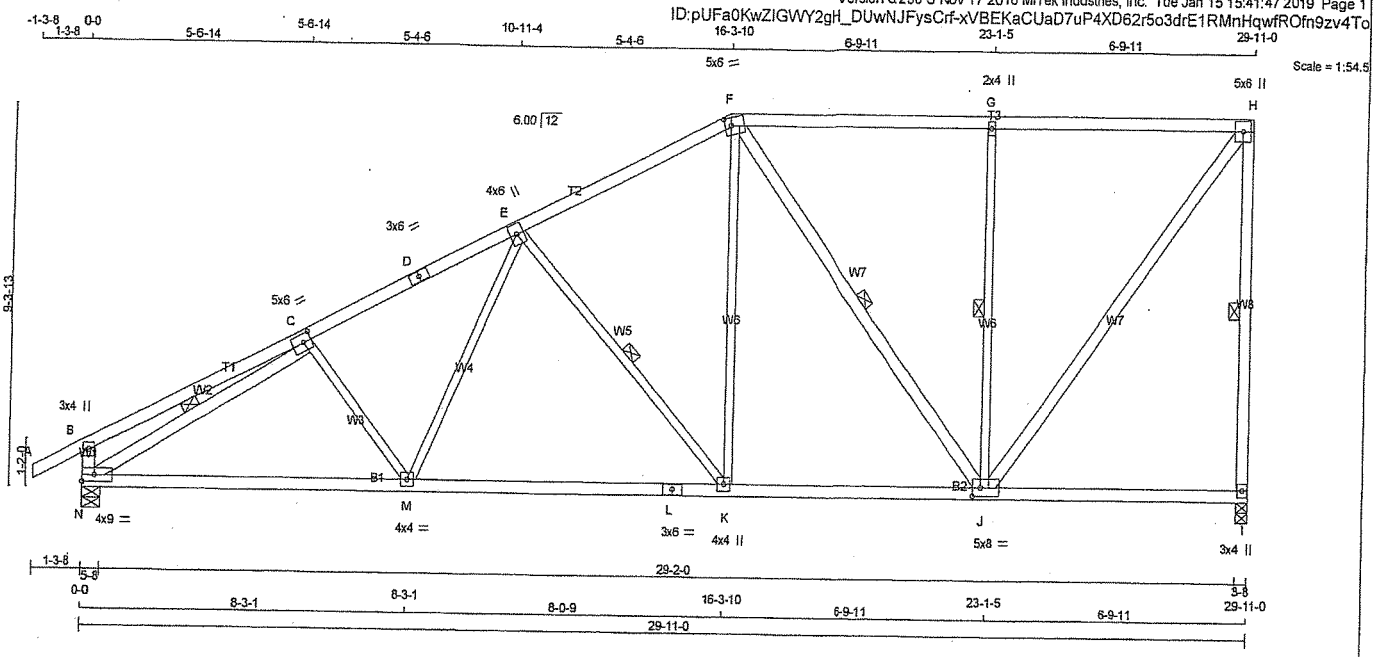


TOTAL WEIGHT = 4 X 144 = 578 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
F - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
N - C	2x4	ORY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.50
D	TS-t	MT20	3.0	6.0		
E	TMWW+t	MT20	4.0	6.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMW+w	MT20	2.0	4.0		
H	TMVW+p	MT20	5.0	6.0		
I	BMV1+p	MT20	3.0	4.0		
J	BMWWW-t	MT20	5.0	8.0	2.50	2.25
K	BMWW+t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMVW1-t	MT20	4.0	9.0		Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
I	1845	0	1845	0	0	3-8	3-8
N	1960	0	1960	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1400	697 / 0	314 / 0	0 / 0	0 / 0	389 / 0	0 / 0
N	1479	760 / 0	314 / 0	0 / 0	0 / 0	405 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

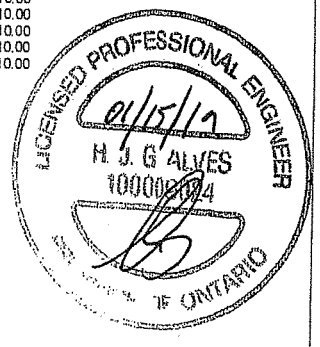
1 LATERAL BRACE(S) AT 1/2 LENGTH OF H-I, E-K, F-J, G-J, C-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 26	-84.9	-84.9	0.11 (1)	10.00	
B- C	0 / 19	-84.9	-84.9	0.32 (1)	10.00	
C- D	-2511 / 0	-84.9	-84.9	0.37 (1)	4.05	
D- E	-2511 / 0	-84.9	-84.9	0.37 (1)	4.05	
E- F	-1727 / 0	-84.9	-84.9	0.33 (1)	4.75	
F- G	-1152 / 0	-84.9	-84.9	0.53 (1)	5.19	
G- H	-1152 / 0	-84.9	-84.9	0.53 (1)	5.19	
I- H	-1739 / 0	0.0	0.0	0.71 (1)	5.05	
N- B	-305 / 0	0.0	0.0	0.03 (1)	7.81	
N- M	0 / 2325	-38.5	-38.5	0.80 (2)	10.00	
M- L	0 / 2055	-38.5	-38.5	0.77 (2)	10.00	
L- K	0 / 2055	-38.5	-38.5	0.77 (2)	10.00	
K- J	0 / 1538	-38.5	-38.5	0.53 (2)	10.00	
J- I	0 / 0	-38.5	-38.5	0.33 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- M	-127 / 87	0.04 (1)
M- E	0 / 533	0.12 (2)
E- K	-808 / 0	0.39 (1)
K- F	0 / 877	0.20 (1)
F- J	-640 / 0	0.44 (1)
J- G	-717 / 0	0.41 (1)
J- H	0 / 1906	0.31 (1)
N- C	-2782 / 0	0.58 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.19")
ALLOWABLE DEFL.(TL)= L/360 (1.00")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.32")

CSI: TC=0.71/1.00 (H-I:1) , BC=0.80/1.00 (M-N:2) , WB=0.58/1.00 (C-N:1) , SSI=0.28/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (M) (INPUT = 0.90)
JSI METAL= 0.88 (C) (INPUT = 1.00)

EWC ROOF TRUSS 7201198
STRUCTURAL
CONSULTANTS

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300333	T322A	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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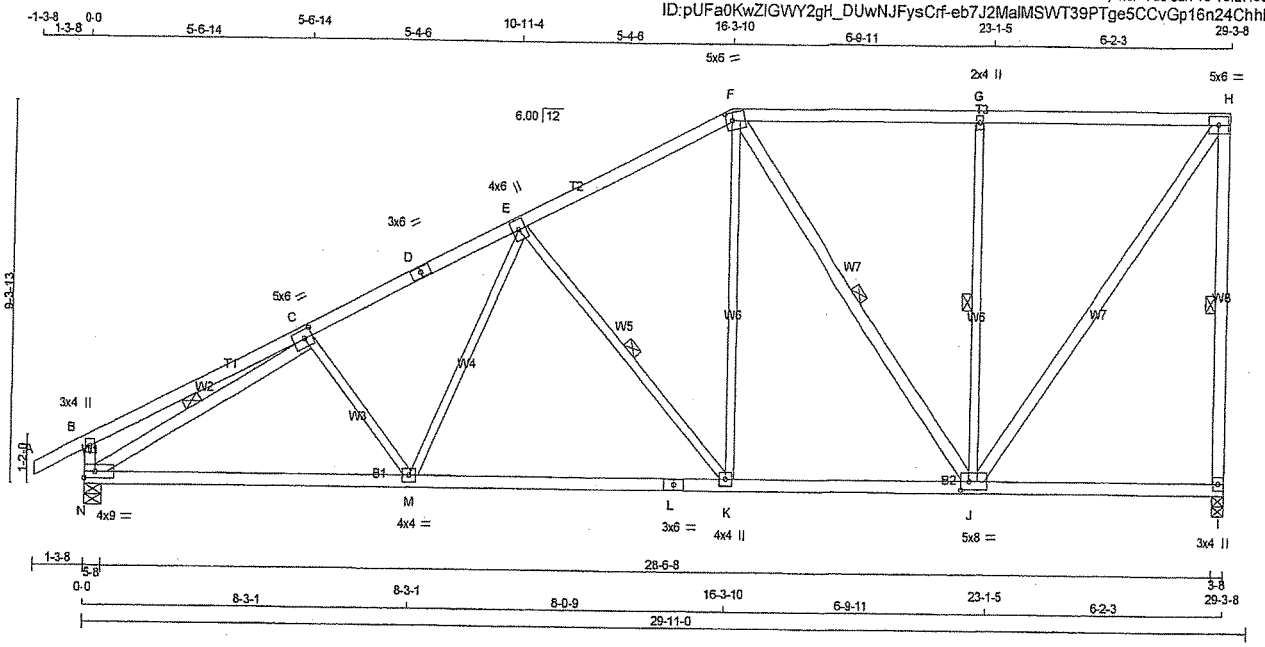


TOTAL WEIGHT = 4 X 143 = 570 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
F - J	2x4	DRY	No.2	SPF
J - H	2x4	ORY	No.2	SPF
N - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT2D	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.50
D	TS-t	MT20	3.0	6.0		
E	TMWW+t	MT20	4.0	6.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMW+w	MT20	2.0	4.0		
H	TMVW-t	MT20	5.0	6.0		
I	BMV1+p	MT2D	3.0	4.0		
J	BMWWW-t	MT20	5.0	8.0	2.50	2.50
K	BMWW+t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMVW1-t	MT20	4.0	9.0		Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REORD BRG IN-SX
I	1807	0	1807	0	0	3-8	3-8
N	1922	0	1922	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS .SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1370	682 / 0	308 / 0	0 / 0	0 / 0	381 / 0	0 / 0
N	1450	745 / 0	308 / 0	0 / 0	0 / 0	397 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.11 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF H-I, E-K, F-J, G-J, C-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

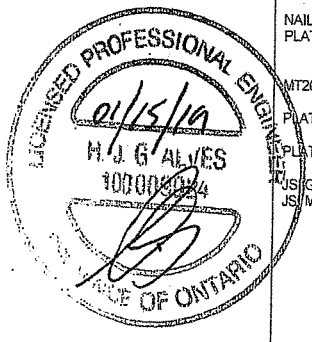
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 26	-84.9	-84.9	1 (1)	0.11	10.00
B- C	0 / 19	-84.9	-84.9	1 (1)	0.32	10.00
C- D	-2440 / 0	-84.9	-84.9	1 (1)	0.36	4.11
D- E	-2440 / 0	-84.9	-84.9	1 (1)	0.36	4.11
E- F	-1849 / 0	-84.9	-84.9	1 (1)	0.32	4.84
F- G	-1082 / 0	-84.9	-84.9	1 (1)	0.48	5.41
G- H	-1082 / 0	-84.9	-84.9	1 (1)	0.48	5.41
I- H	-1705 / 0	0.0	0.0	1 (1)	0.69	5.09
N- B	-305 / 0	0.0	0.0	1 (1)	0.03	7.81
N- M	0 / 2267	-38.5	-38.5	2 (2)	0.80	10.00
M- L	0 / 1988	-38.5	-38.5	2 (2)	0.76	10.00
L- K	0 / 1988	-38.5	-38.5	2 (2)	0.76	10.00
K- J	0 / 1466	-38.5	-38.5	2 (2)	0.49	10.00
J- I	0 / 0	-38.5	-38.5	3 (3)	0.29	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI	LC
C- M	-135 / 83	0.05	1 (1)
M- E	0 / 541	0.12	2 (2)
E- K	-813 / 0	0.39	1 (1)
K- F	0 / 876	0.20	1 (1)
F- J	-659 / 0	0.44	1 (1)
J- G	-684 / 0	0.39	1 (1)
J- H	0 / 1845	0.30	1 (1)
N- C	-2713 / 0	0.57	1 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.98")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.19")
ALLOWABLE DEFL.(TL)= L/360 (0.98")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.32")

CSI: TC=0.69/1.00 (H-I:1) , BC=0.80/1.00 (M-N:2) , WB=0.57/1.00 (C-N:1) , SSI=0.27/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JS GRIP= 0.89 (M) (INPUT = 0.90)
JS METAL= 0.67 (C) (INPUT = 1.00)

DWG NO. FOR 7901199
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300581	T323	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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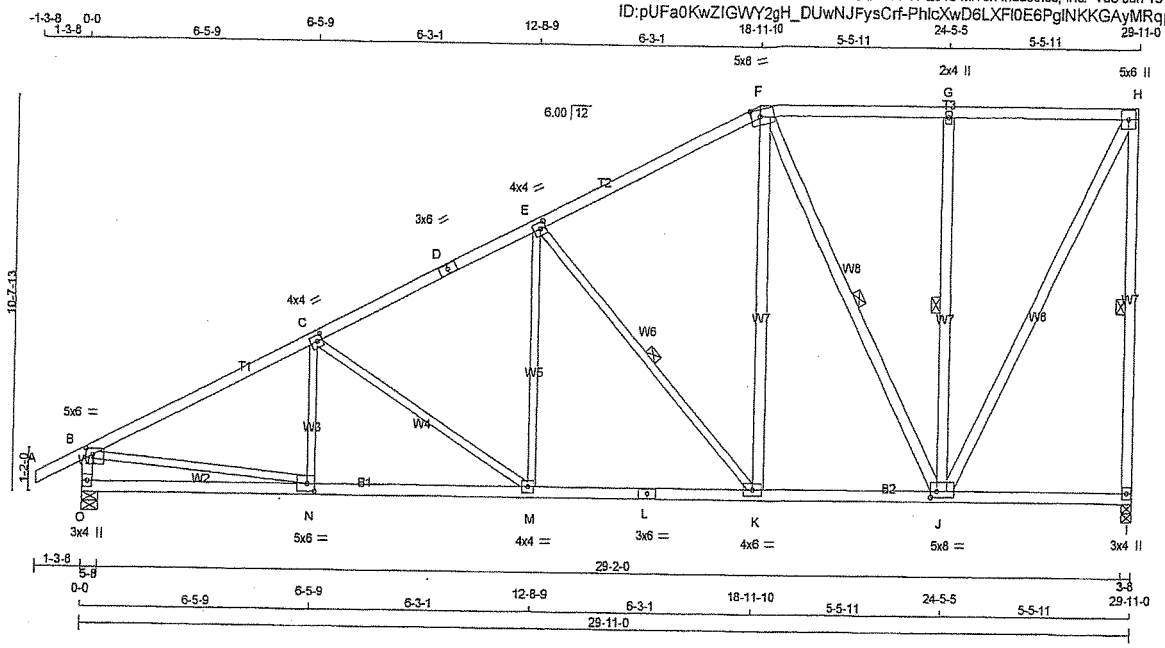


TOTAL WEIGHT = 161 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
K - F	2x4	DRY	No.2	SPF
F - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	2.00	2.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TTWW-m	MT20	5.0	8.0	2.25	3.00
G	TMW+w	MT20	2.0	4.0		
H	TMVW+p	MT20	5.0	6.0		
I	BMV1+p	MT20	3.0	4.0		
J	BMWWW-t	MT20	5.0	8.0	2.00	2.00
K	BMWW-t	MT20	4.0	6.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMWW-t	MT20	5.0	6.0	2.50	2.50
O	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER.

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HDRZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	1845	0	1845	0	0	3-8	3-8
O	1960	0	1960	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1400	697 / 0	314 / 0	0 / 0	0 / 0	389 / 0	0 / 0
O	1479	760 / 0	314 / 0	0 / 0	0 / 0	405 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.72 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

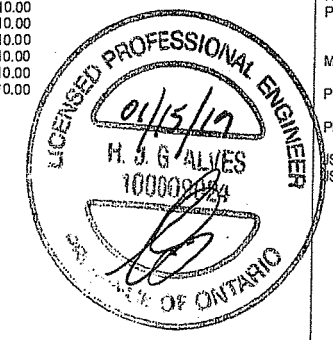
1 LATERAL BRACE(S) AT 1/2 LENGTH OF H-I, E-K, F-J, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 28	-84.9	-84.9	0.11 (1)	10.00
B-C	-2660 / 0	-84.9	-84.9	0.59 (1)	3.72
C-D	-2159 / 0	-84.9	-84.9	0.45 (1)	4.22
D-E	-2159 / 0	-84.9	-84.9	0.45 (1)	4.22
E-F	-1405 / 0	-84.9	-84.9	0.43 (1)	5.00
F-G	-836 / 0	-84.9	-84.9	0.33 (1)	6.22
G-H	-837 / 0	-84.9	-84.9	0.33 (1)	6.22
I-H	-1759 / 0	0.0	0.0	1.00 (1)	5.03
O-B	-1857 / 0	0.0	0.0	0.19 (1)	6.15
O-N	0 / 0	-38.5	-38.5	0.28 (3)	10.00
N-M	0 / 2400	-38.5	-38.5	0.60 (2)	10.00
M-L	0 / 1931	-38.5	-38.5	0.49 (2)	10.00
L-K	0 / 1931	-38.5	-38.5	0.49 (2)	10.00
K-J	0 / 1249	-38.5	-38.5	0.37 (2)	10.00
J-I	0 / 0	-38.5	-38.5	0.20 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N-C	-49 / 183	0.04 (3)
C-M	-560 / 0	0.62 (1)
M-E	0 / 583	0.13 (2)
E-K	-1049 / 0	0.69 (1)
K-F	0 / 1009	0.16 (1)
F-J	-883 / 0	0.67 (1)
J-G	-576 / 0	0.33 (1)
J-H	0 / 1791	0.29 (1)
B-N	0 / 2421	0.54 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN PLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (1.00")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=1.00/1.00 (H-I:1) , BC=0.60/1.00 (M-N:2) , WB=0.69/1.00 (E-K:1) , SSI=0.23/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg

SI GRIP= 0.90 (B) (INPUT = 0.90)
SI METAL= 0.77 (L) (INPUT = 1.00)

DWG NO. TAM 1901200
STRUCTURAL
CONFIGURATION ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300333	T323A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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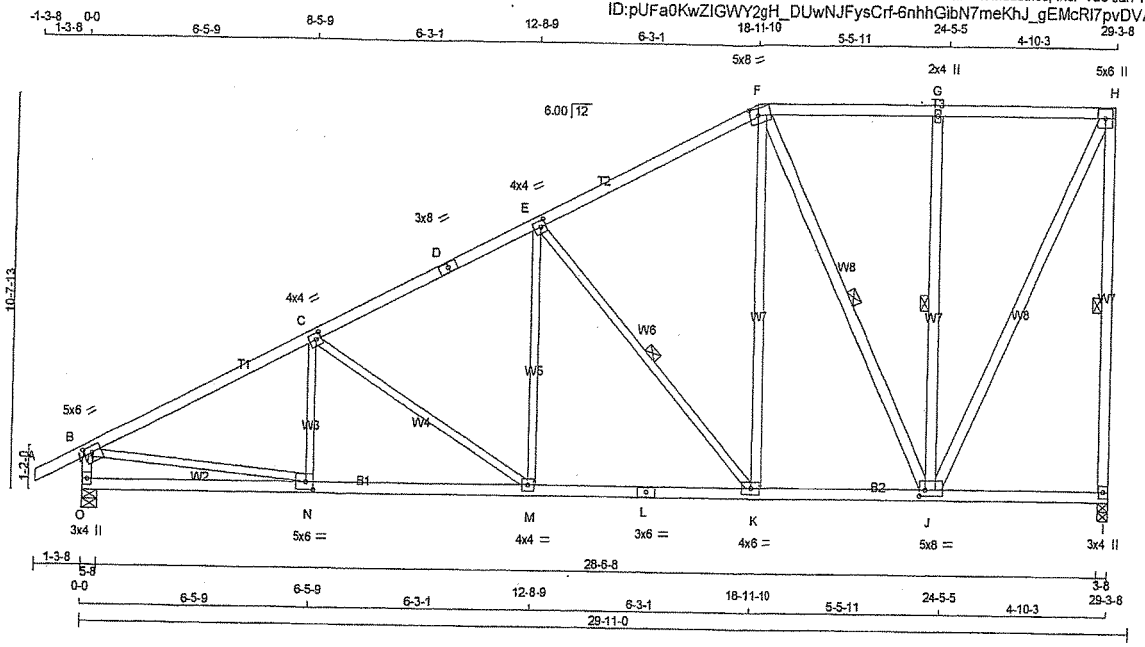


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
O - L	2x4	ORY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
K - F	2x4	ORY	No.2	SPF
F - J	2x4	DRY	No.2	SPF
J - G	2x4	ORY	No.2	SPF
J - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.00	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
O	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TTWW-m	MT20	5.0	8.0	2.25	3.00
G	TMW+w	MT20	2.0	4.0		
H	TMVW+p	MT20	5.0	6.0		
I	BMV1+p	MT20	3.0	4.0		
J	BMWWW-t	MT20	5.0	8.0	2.00	2.00
K	BMWW-t	MT20	4.0	6.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMWW-t	MT20	5.0	6.0	2.50	2.50
O	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	1807	0	1807	0	0	3-8	3-8
O	1922	0	1922	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1370	682 / 0	308 / 0	0 / 0	0 / 0	381 / 0	0 / 0
O	1450	745 / 0	308 / 0	0 / 0	0 / 0	397 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF H-I, E-K, F-J, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

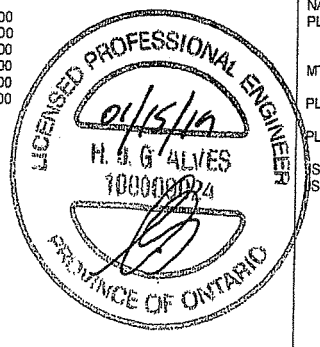
TOTAL LOAD CASES: (4)

CHORDS

MEMB FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 26	-84.9	-84.9	0.11 (1)	10.00
B- C	-2592 / 0	-84.9	-84.9	0.58 (1)	3.77
C- D	-2083 / 0	-84.9	-84.9	0.45 (1)	4.29
D- E	-2083 / 0	-84.9	-84.9	0.45 (1)	4.29
E- F	-1325 / 0	-84.9	-84.9	0.42 (1)	5.13
F- G	-776 / 0	-84.9	-84.9	0.29 (1)	6.25
G- H	-777 / 0	-84.9	-84.9	0.29 (1)	6.25
I- H	-1725 / 0	0.0	0.0	0.99 (1)	5.07
O- B	-1819 / 0	0.0	0.0	0.18 (1)	6.20
O- N	0 / 0	-38.5	-38.5	0.28 (3)	10.00
N- M	0 / 2339	-38.5	-38.5	0.59 (2)	10.00
M- L	0 / 1863	-38.5	-38.5	0.48 (2)	10.00
L- K	0 / 1863	-38.5	-38.5	0.48 (2)	10.00
K- J	0 / 1178	-38.5	-38.5	0.35 (2)	10.00
J- I	0 / 0	-38.5	-38.5	0.18 (3)	10.00

WEBS

MEMB FR-TO	MAX. FACTOREO FORCE (LBS)	MAX CSI (LC)
N- C	-41 / 187	0.04 (3)
C- M	-568 / 0	0.63 (1)
M- E	0 / 588	0.13 (2)
E- K	-1054 / 0	0.69 (1)
K- F	0 / 1008	0.16 (1)
F- J	-900 / 0	0.66 (1)
J- G	-542 / 0	0.31 (1)
J- H	0 / 1741	0.28 (1)
B- N	0 / 2359	0.53 (1)



TOTAL WEIGHT = 159 lb

[M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI. TC=0.99/1.00 (H-I:1) , BC=0.59/1.00 (M-N:2) , WB=0.69/1.00 (E-K:1) , SSI=0.23/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.88 (B) (INPUT = 0.90)
SI METAL= 0.75 (L) (INPUT = 1.00)

DWG NO. TAM 7901201
STRUCTURAL
ROOF TRUSS ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300333	T324	1	1	TRUSS OESC.	

Tamarack Roof Truss, Burlington

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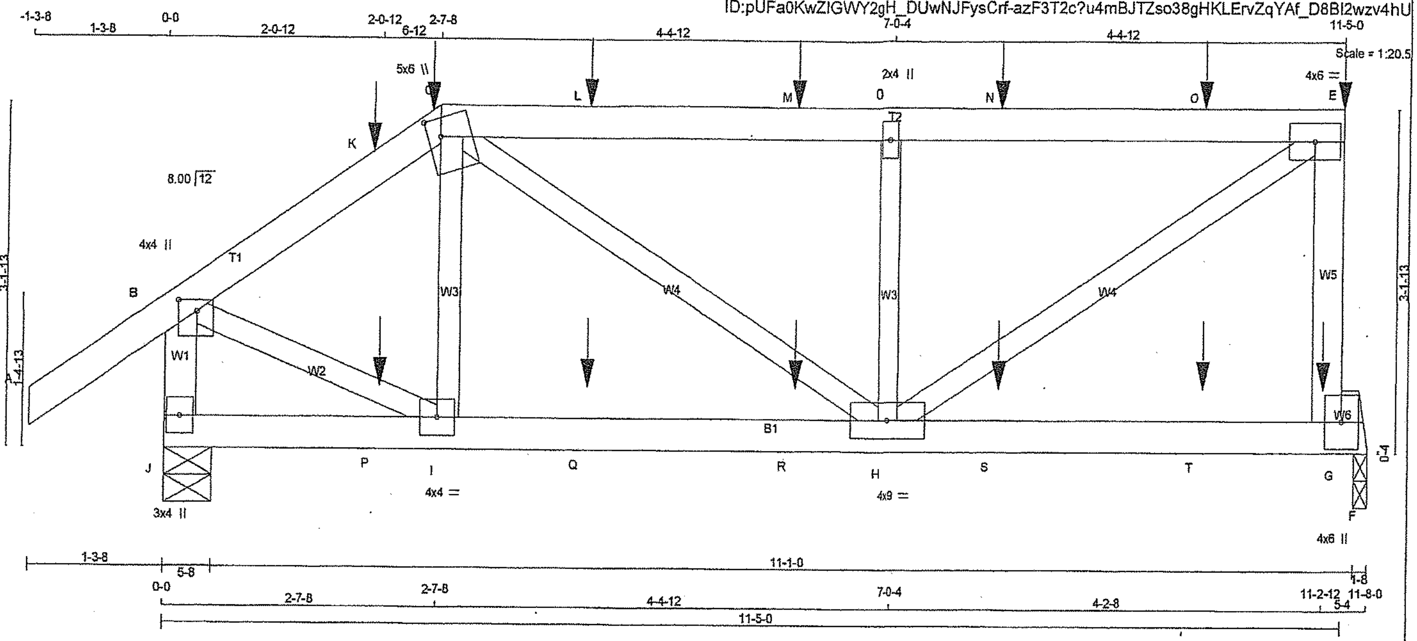

TOTAL WEIGHT = 47 lb
[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
J - F	2x4	DRY	No.2	SPF
G - F	2x4	ORY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
O	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
G	BMVW+p	MT20	4.0	6.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

LICENSED PROFESSIONAL ENGINEER
01/15/19
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TD 8E VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REORD BRG IN-SX
J	980	0	980	0	0	5-8	5-8
F	879	0	879	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	0EAD	SOIL
J	731	406 / 0	139 / 0	0 / 0	0 / 0	186 / 0	0 / 0
F	665	340 / 0	152 / 0	0 / 0	0 / 0	173 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.83 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.13 (1)	10.00
B- K	-781 / 0	-84.9	-84.9	0.14 (1)	6.25
K- C	-781 / 0	-84.9	-84.9	0.14 (1)	6.25
C- L	-952 / 0	-84.9	-84.9	0.35 (1)	5.83
L- M	-952 / 0	-84.9	-84.9	0.35 (1)	5.83
M- D	-952 / 0	-84.9	-84.9	0.35 (1)	5.83
D- N	-952 / 0	-84.9	-84.9	0.35 (1)	5.83
N- O	-952 / 0	-84.9	-84.9	0.35 (1)	5.83
O- E	-952 / 0	-84.9	-84.9	0.35 (1)	5.83
G- E	-838 / 0	0.0	0.0	0.14 (1)	7.81
J- B	-936 / 0	0.0	0.D	0.11 (1)	7.81
J- P	0 / 0	-38.5	-38.5	0.08 (3)	10.00
P- I	0 / 0	-38.5	-38.5	0.08 (3)	10.00
I- O	0 / 650	-38.5	-38.5	0.33 (2)	10.00
Q- R	0 / 650	-38.5	-38.5	0.33 (2)	10.00
R- H	0 / 650	-38.5	-38.5	0.33 (2)	10.00
H- S	0 / 0	-38.5	-38.5	0.33 (1)	10.00
S- T	0 / 0	-38.5	-38.5	0.33 (1)	10.00
T- G	0 / 0	-38.5	-38.5	0.33 (1)	10.00
G- F	0 / 0	-53.5	-53.5	0.14 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- C	-127 / 56	0.03 (1)
C- H	0 / 361	0.09 (1)
H- D	-527 / 0	0.10 (1)
H- E	0 / 1132	0.28 (1)
B- I	0 / 704	0.17 (1)

DWG NO. 11901202
STRUCTURAL COMPONENT ONLY

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-7-8	-11	-13	—	FRONT	VERT	DEAD	—	—
C	2-7-8	-51	-51	—	FRONT	VERT	SNOW	—	—
E	11-5-0	-61	-61	—	FRONT	VERT	TOTAL	—	—
G	11-2-12	-14	-20	—	FRONT	VERT	TOTAL	—	—
K	2-0-12	-45	-45	—	FRONT	VERT	TOTAL	—	—
L	4-0-12	-27	-27	—	FRONT	VERT	TOTAL	—	—
M	6-0-12	-27	-27	—	FRONT	VERT	TOTAL	—	—
N	8-0-12	-27	-27	—	FRONT	VERT	TOTAL	—	—
O	10-0-12	-27	-27	—	FRONT	VERT	TOTAL	—	—
P	2-0-12	-7	-10	—	FRONT	VERT	TOTAL	—	—
O	4-0-12	-7	-10	—	FRONT	VERT	TOTAL	—	—
R	8-0-12	-7	-10	—	FRONT	VERT	TOTAL	—	—
S	8-0-12	-7	-10	—	FRONT	VERT	TOTAL	—	—
T	10-0-12	-7	-10	—	FRONT	VERT	TOTAL	—	—

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MOOIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP	CH.	LL	=	23.3	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	10.5	PSF
		DL	=	7.0	PSF
TOTAL	LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TD ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REOUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EOUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.39")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.39")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.35/1.00 (D-E:1) , BC=0.33/1.00 (G-H:1) , WB=0.28/1.00 (E-H:1) , SSI=0.38/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR OUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.D Deg.

JSI GRIP= 0.81 (I) (INPUT = 0.90)
JSI METAL= 0.41 (G) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300333	T325	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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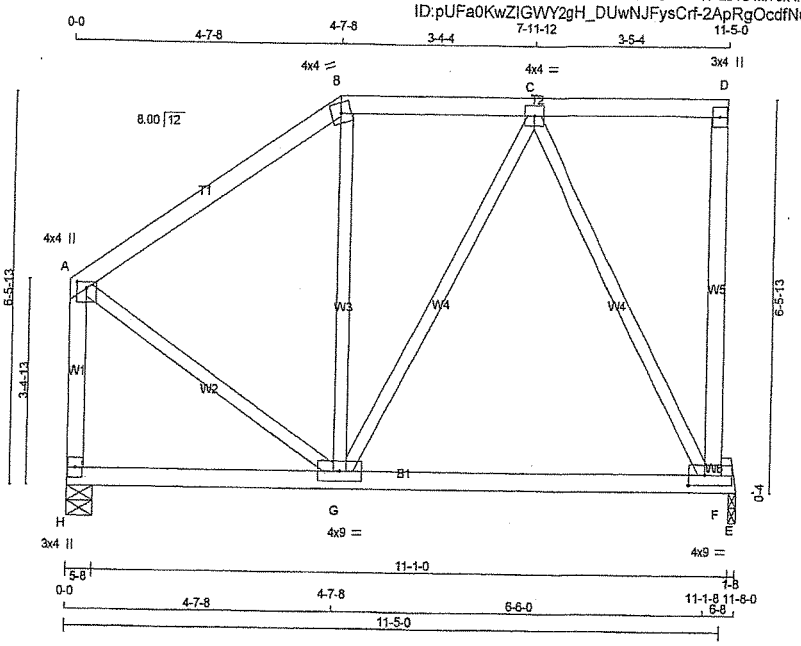


Scale = 1:36.9

LUMBER

N.L.G.A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
B	TTW-m	MT20	4.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TMV+p	MT20	3.0	4.0		
F	BMVWW-t	MT20	4.0	9.0	2.00	3.25
G	BMWWW-t	MT20	4.0	9.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REORD BRG IN-SX
H	719	0	719	0	0	5-8	5-8
E	702	0	702	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	546	272 / 0	123 / 0	0 / 0	0 / 0	152 / 0	0 / 0
E	534	260 / 0	123 / 0	0 / 0	0 / 0	152 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, E

BRACING

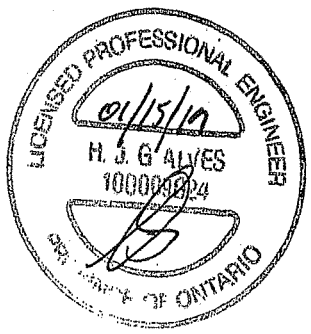
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-435 / 0	-84.9	-84.9	0.32 (1)	6.25	G- B	-63 / 84	0.04 (1)
B- C	-360 / 0	-84.9	-84.9	0.16 (1)	6.25	A- G	0 / 432	0.10 (1)
C- D	0 / 0	-84.9	-84.9	0.16 (1)	10.00	G- C	0 / 200	0.04 (2)
F- D	-111 / 0	0.0	0.0	0.10 (1)	7.81	C- F	-580 / 0	0.54 (1)
H- A	-674 / 0	0.0	0.0	0.13 (1)	7.81			
H- G	0 / 0	-38.5	-38.5	0.35 (2)	10.00			
G- F	0 / 281	-38.5	-38.5	0.40 (2)	10.00			
F- E	0 / 0	-53.5	-53.5	0.10 (1)	10.00			



TOTAL WEIGHT = 58 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.39")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.39")
CALCULATED VERT. DEFL.(TL) = L/ 718 (0.20")

CSI: TC=0.32/1.00 (A-B:1) , BC=0.40/1.00 (F-G:2) , WB=0.54/1.00 (C-F:1) , SSI=0.27/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.66 (F) (INPUT = 0.90)
JSI METAL= 0.16 (F) (INPUT = 1.00)

DWG NO. T325 7901203

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300333	T326	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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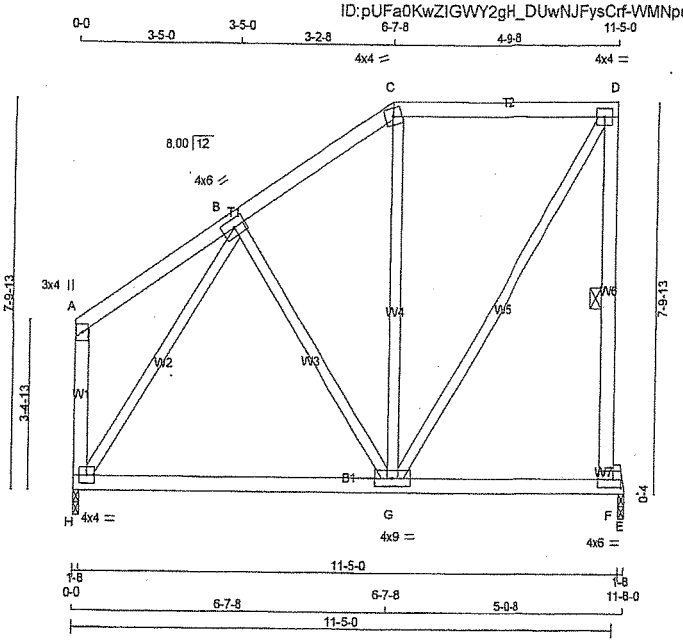


TOTAL WEIGHT = 62 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	6.0		
C	TTW-m	MT20	4.0	4.0		
D	TMVW-t	MT20	4.0	4.0		
F	BMVW-t	MT20	4.0	6.0	2.00	2.25
G	BMWWW-t	MT20	4.0	9.0		
H	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	702	0	702	0	0	1-8	1-8
H	719	0	719	0	0	1-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	534	260 / 0	123 / 0	0 / 0	0 / 0	152 / 0	0 / 0
H	546	272 / 0	123 / 0	0 / 0	0 / 0	152 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

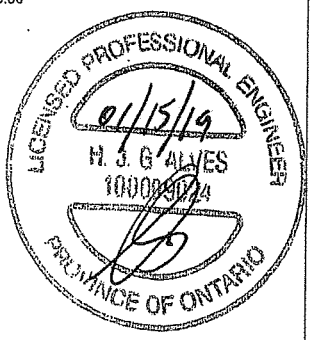
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 19	-84.9	-84.9	0.15 (1)	10.00	B-G	-41 / 58	0.03 (1)
B-C	-377 / 0	-84.9	-84.9	0.15 (1)	6.25	G-C	-113 / 59	0.13 (1)
C-D	-296 / 0	-84.9	-84.9	0.34 (1)	6.25	G-D	0 / 561	0.13 (1)
F-D	-678 / 0	0.0	0.0	0.19 (1)	6.25	H-B	-605 / 0	0.40 (1)
H-A	-106 / 0	0.0	0.0	0.02 (1)	7.81			
H-G	0 / 320	-38.5	-38.5	0.38 (2)	10.00			
G-F	0 / 0	-38.5	-38.5	0.33 (2)	10.00			
F-E	0 / 0	-53.5	-53.5	0.10 (1)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.39")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.39")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.34/1.00 (C-D:1) , BC=0.38/1.00 (G-H:2) , WB=0.40/1.00 (B-H:1) , SSI=0.27/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (F) (INPUT = 0.90)
JSI METAL= 0.22 (F) (INPUT = 1.00)

DWG NO. TAM T90/2044
SEE CENTRAL
CONFIDENTIALLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300333	T327	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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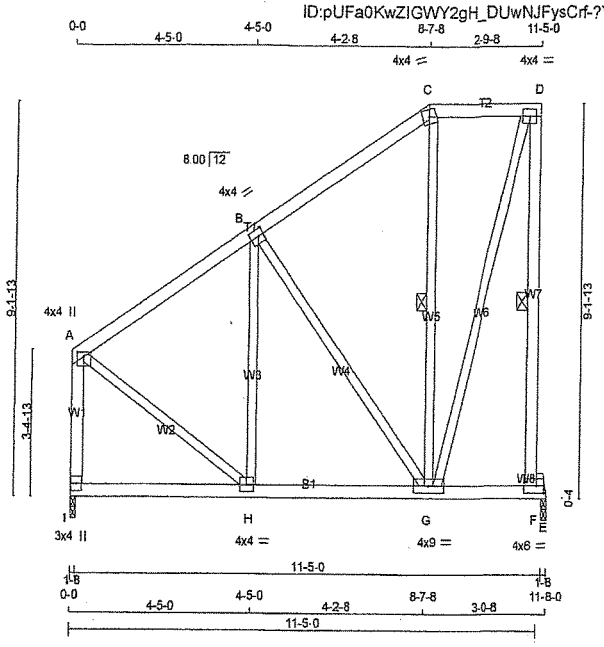


TOTAL WEIGHT = 71 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
I - A	2x4	DRY	No.2	SPF
I - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
B	TMWW-t	MT20	4.0	4.0	2.00	1.50
C	TTW-m	MT20	4.0	4.0		
D	TMVW-t	MT20	4.0	4.0		
F	BMVW-t	MT20	4.0	6.0	2.00	2.25
G	BMWWW-t	MT20	4.0	9.0		
H	BMWW-t	MT20	4.0	4.0		
I	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	719	0	719	0	0	1-8	1-8
E	702	0	702	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	546	272 / 0	123 / 0	0 / 0	0 / 0	152 / 0	0 / 0
E	534	260 / 0	123 / 0	0 / 0	0 / 0	152 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-F, C-G.

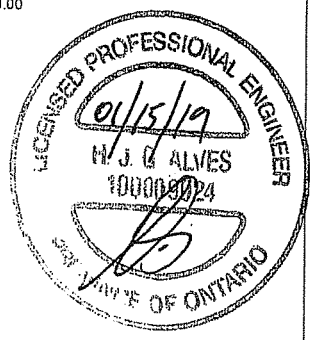
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-422 / 0	-84.9	-84.9	0.27 (1)	6.25
B- C	-250 / 0	-84.9	-84.9	0.27 (1)	6.25
C- D	-185 / 0	-84.9	-84.9	0.11 (1)	6.25
F- D	-726 / 0	0.0	0.0	0.28 (1)	6.25
I- A	-650 / 0	0.0	0.0	0.13 (1)	7.81
I- H	0 / 0	-38.5	-38.5	0.14 (3)	10.00
H- G	0 / 371	-38.5	-38.5	0.18 (2)	10.00
G- F	0 / 0	-38.5	-38.5	0.24 (1)	10.00
F- E	0 / 0	-53.5	-53.5	0.10 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- B	-74 / 109	0.05 (1)
B- G	-318 / 0	0.34 (1)
G- C	-127 / 22	0.07 (1)
G- D	0 / 636	0.14 (1)
A- H	0 / 453	0.10 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.39")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.39")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.28/1.00 (D-F:1) , BC=0.24/1.00 (F-G:1) , WB=0.34/1.00 (B-G:1) , SSI=0.27/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 CDMP=1.10 SHEAR=1.10 TENS= 1.10

CDMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (D) (INPUT = 0.90)
JSI METAL= 0.22 (F) (INPUT = 1.00)

T1901205
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300333	T328	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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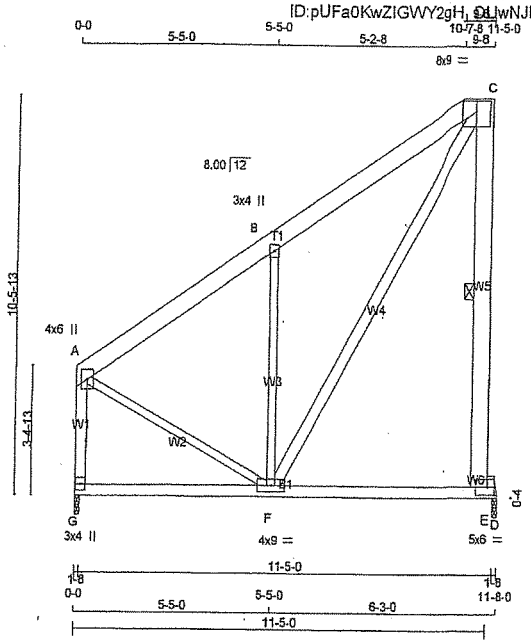


TOTAL WEIGHT = 81 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
E - C	2x6	DRY	No.2	SPF
G - A	2x4	DRY	No.2	SPF
G - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
F - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0		
B	TMW+w	MT20	3.0	4.0		
C	TVMWm	MT20	8.0	9.0	3.25	4.50
E	BMVW-t	MT20	5.0	6.0	2.50	2.25
F	BMWWW-t	MT20	4.0	9.0		
G	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	719	0	719	0	0	1-8	1-8
D	699	0	699	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	546	272 / 0	123 / 0	0 / 0	0 / 0	152 / 0	0 / 0
D	531	260 / 0	123 / 0	0 / 0	0 / 0	149 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

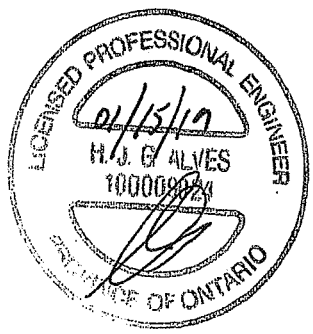
1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-447 / 0	-84.9	-84.9	0.21 (1)		6.25	F- B	-587 / 0	0.46 (1)
B- C	-494 / 0	-84.9	-84.9	0.21 (1)		6.25	A- F	0 / 456	0.10 (1)
E- C	-635 / 0	0.0	0.0	0.24 (1)		6.25	F- C	0 / 787	0.13 (1)
G- A	-651 / 0	0.0	0.0	0.12 (1)		7.81			
G- F	0 / 0	-38.5	-38.5	0.32 (2)		10.00			
F- E	0 / 0	-38.5	-38.5	0.32 (2)		10.00			
E- D	0 / 0	-38.5	-38.5	0.10 (1)		10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.39")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.39")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.24/1.00 (C-E:1) , BC=0.32/1.00 (E-F:2) , WB=0.46/1.00 (B-F:1) , SSI=0.27/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.85 (E) (INPUT = 0.90)
JSI METAL= 0.26 (A) (INPUT = 1.00)

T901206

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300581	T329	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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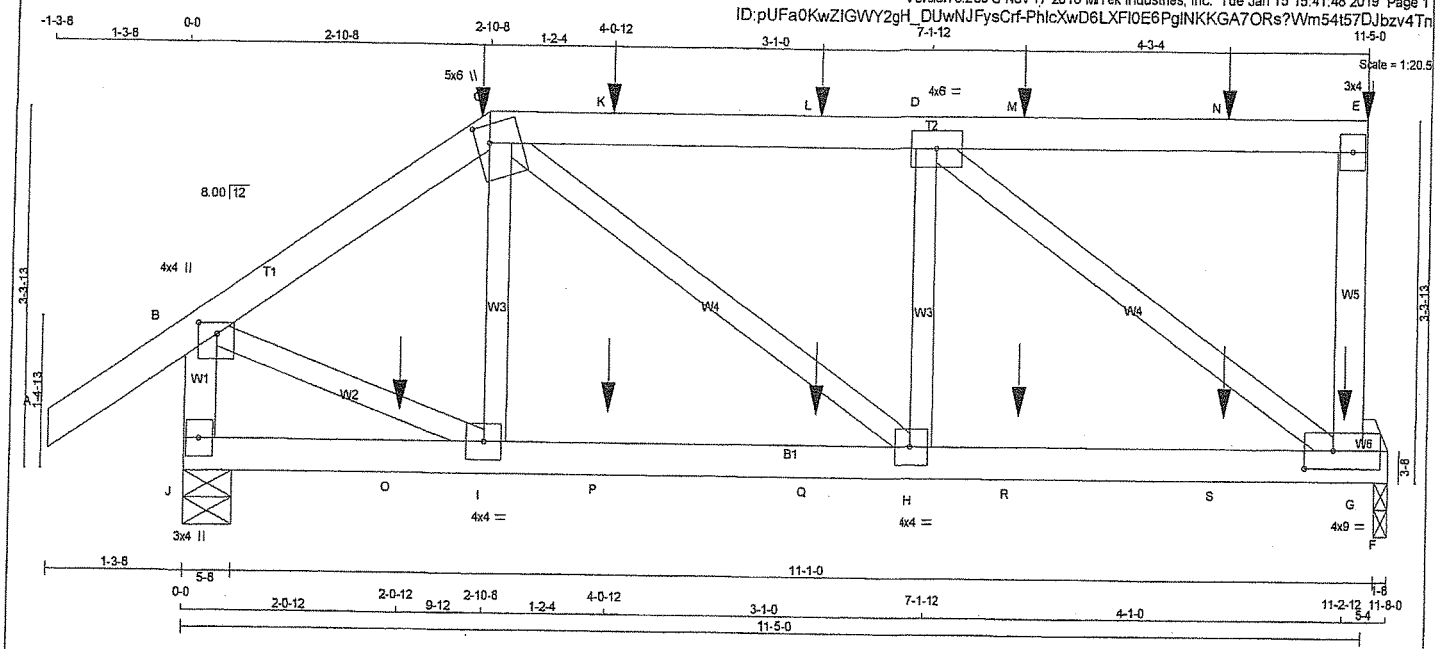


LUMBER

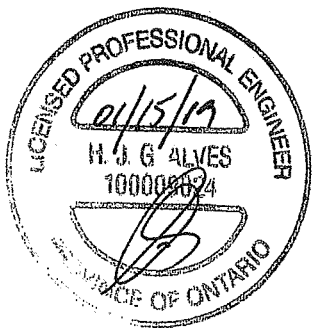
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
J - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMWW-t	MT20	4.0	6.0		
E	TMV+p	MT20	3.0	4.0		
G	BMVWW-t	MT20	4.0	9.0	2.00	3.25
H	BMWW-t	MT20	4.0	4.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	924	0	924	0	0	5-8	5-8
F	820	0	820	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	893	370 / 0	136 / 0	0 / 0	0 / 0	186 / 0	0 / 0
F	626	301 / 0	148 / 0	0 / 0	0 / 0	177 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.22 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 32	-84.9	-84.9	0.13 (1)	10.00	I- C	-81 / 77	0.02 (3)
B- C	-740 / 0	-84.9	-84.9	0.14 (1)	6.25	C- H	0 / 275	0.07 (1)
C- K	-835 / 0	-84.9	-84.9	0.30 (1)	6.22	H- D	0 / 230	0.06 (3)
K- L	-835 / 0	-84.9	-84.9	0.30 (1)	6.22	D- G	-1021 / 0	0.45 (1)
L- D	-835 / 0	-84.9	-84.9	0.30 (1)	6.22	B- I	0 / 654	0.16 (1)
D- M	0 / 0	-84.9	-84.9	0.28 (1)	10.00			
M- N	0 / 0	-84.9	-84.9	0.28 (1)	10.00			
N- E	0 / 0	-84.9	-84.9	0.28 (1)	10.00			
G- E	-187 / 0	0.0	0.0	0.03 (1)	7.81			
J- B	-874 / 0	0.0	0.0	0.10 (1)	7.81			
J- O	0 / 0	-38.5	-38.5	0.08 (3)	10.00			
O- I	0 / 0	-38.5	-38.5	0.08 (3)	10.00			
I- P	0 / 612	-38.5	-38.5	0.32 (2)	10.00			
P- O	0 / 612	-38.5	-38.5	0.32 (2)	10.00			
Q- H	0 / 612	-38.5	-38.5	0.32 (2)	10.00			
H- R	0 / 836	-38.5	-38.5	0.46 (1)	10.00			
R- S	0 / 836	-38.5	-38.5	0.46 (1)	10.00			
S- G	0 / 836	-38.5	-38.5	0.46 (1)	10.00			
G- F	0 / 0	-38.5	-38.5	0.13 (1)	10.00			

DWG NO. TM T1901207
STRUCTURAL
COMPONENTS ONLY

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-10-8	-13	-14	—	FRONT	VERT	DEAD	—	—
C	2-10-8	-56	-56	—	FRONT	VERT	SNOW	—	—
E	11-5-0	-41	-41	—	BACK	VERT	TOTAL	—	—
G	11-2-12	-21	-27	—	BACK	VERT	TOTAL	—	—
K	4-0-12	-7	-7	—	BACK	VERT	TOTAL	—	—
L	6-0-12	-7	-7	—	BACK	VERT	TOTAL	—	—
M	8-0-12	-7	-7	—	BACK	VERT	TOTAL	—	—
N	10-0-12	-7	-7	—	BACK	VERT	TOTAL	—	—
O	2-0-12	-10	-13	—	BACK	VERT	TOTAL	—	—
P	4-0-12	-10	-13	—	BACK	VERT	TOTAL	—	—
Q	6-0-12	-10	-13	—	BACK	VERT	TOTAL	—	—
R	8-0-12	-10	-13	—	BACK	VERT	TOTAL	—	—
S	10-0-12	-10	-13	—	BACK	VERT	TOTAL	—	—

TOTAL WEIGHT = 48 lb

[M]

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 088-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.39")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.39")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.30/1.00 (C-D:1) , BC=0.46/1.00 (G-H:1) , WB=0.45/1.00 (D-G:1) , SSI=0.35/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (I) (INPUT = 0.90)
JSI METAL= 0.23 (I) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300581	T330	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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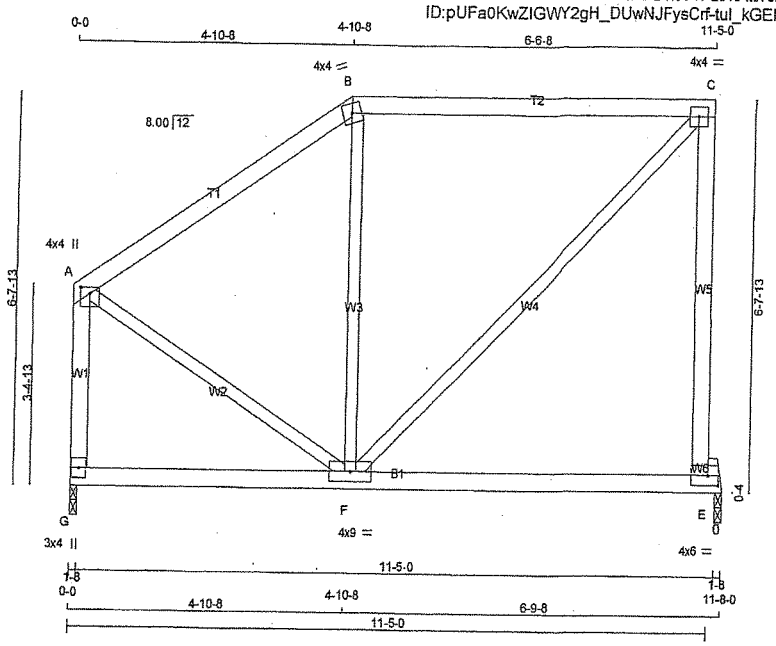

Scale = 1:37.9

TOTAL WEIGHT = 55 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
E - C	2x4	DRY	No.2	SPF
G - A	2x4	DRY	No.2	SPF
G - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
B	TTW-m	MT20	4.0	4.0		
C	TMVW-t	MT20	4.0	4.0		
E	BMVW-t	MT20	4.0	6.0	2.00	2.25
F	BMWWW-t	MT20	4.0	9.0		
G	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	719	0	719	0	0	1-8	1-8
D	699	0	699	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	546	272 / 0	123 / 0	0 / 0	0 / 0	152 / 0	0 / 0
D	531	260 / 0	123 / 0	0 / 0	0 / 0	149 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES. (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-428 / 0	-84.9	-84.9	0.36 (1)	6.25	F- B	-247 / 50	0.18 (1)
B- C	-352 / 0	-84.9	-84.9	0.64 (1)	6.25	F- C	0 / 494	0.11 (1)
E- C	-624 / 0	0.0	0.0	0.58 (1)	7.81	A- F	0 / 418	0.09 (1)
G- A	-665 / 0	0.0	0.0	0.13 (1)	7.81			
G- F	0 / 0	-38.5	-38.5	0.34 (2)	10.00			
F- E	0 / 0	-38.5	-38.5	0.34 (2)	10.00			
E- D	0 / 0	-38.5	-38.5	0.10 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.39")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.39")
CALCULATED VERT. DEFL.(TL) = L/ 816 (0.17")

CSI: TC=0.64/1.00 (B-C:1) , BC=0.34/1.00 (E-F:2) , WB=0.18/1.00 (B-F:1) , SSI=0.27/1.00 (D-E 1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (E) (INPUT = 0.90)
JSI METAL= 0.22 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
01/15/19
M. J. G ALVES
PROVINCE OF ONTARIO

DWG NO. T1901208
STRUCTURAL
COMPONENTS ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300581	T331	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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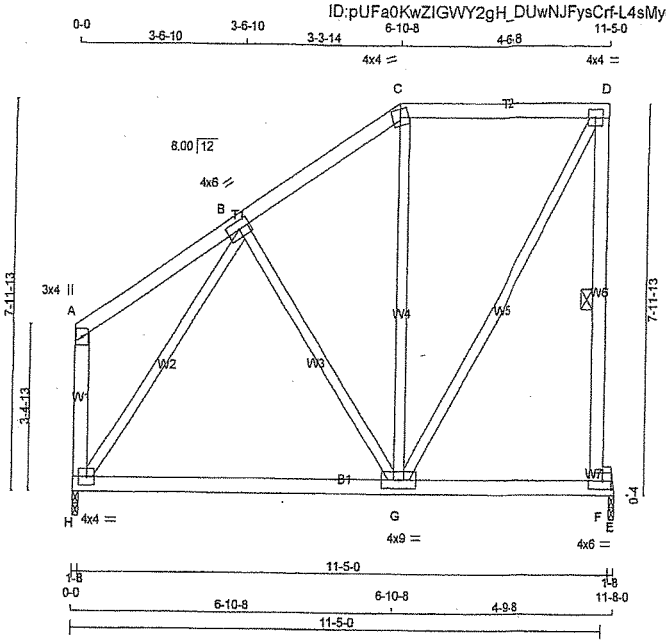

TOTAL WEIGHT = 63 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	6.0		
C	TTW-m	MT20	4.0	4.0		
D	TMVW-t	MT20	4.0	4.0		
F	BMVW-t	MT20	4.0	6.0	2.00	2.25
G	BMWWW-t	MT20	4.0	9.0		
H	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	699	0	699	0	0	1-8	1-8
H	719	0	719	0	0	1-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	531	260 / 0	123 / 0	0 / 0	0 / 0	149 / 0	0 / 0
H	546	272 / 0	123 / 0	0 / 0	0 / 0	152 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 19	-84.9	-84.9	0.17 (1)		10.00
B- C	-367 / 0	-84.9	-84.9	0.17 (1)		6.25
C- D	-287 / 0	-84.9	-84.9	0.30 (1)		6.25
F- D	-689 / 0	0.0	0.0	0.20 (1)		6.25
H- A	-111 / 0	0.0	0.0	0.02 (1)		7.81
H- G	0 / 322	-38.5	-38.5	0.39 (2)		10.00
G- F	0 / 0	-38.5	-38.5	0.34 (3)		10.00
F- E	0 / 0	-38.5	-38.5	0.10 (1)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- C	-112 / 57	0.14 (1)
G- D	0 / 574	0.13 (1)
B- G	-61 / 51	0.05 (1)
H- B	-598 / 0	0.42 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.39")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.39")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.30/1.00 (C-D:1) , BC=0.39/1.00 (G-H:2) , WB=0.42/1.00 (B-H:1) , SSI=0.27/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (F) (INPUT = 0.90)
JSI METAL= 0.22 (F) (INPUT = 1.00)

T901209

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300581	T332	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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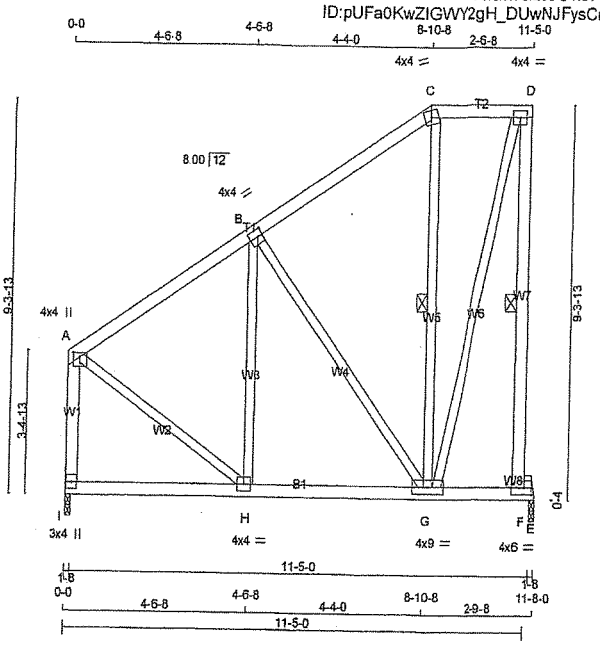


TOTAL WEIGHT = 71 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
I - A	2x4	DRY	No.2	SPF
I - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
B	TMWW-t	MT20	4.0	4.0	2.00	1.50
C	TTW-m	MT20	4.0	4.0		
D	TMVW-t	MT20	4.0	4.0		
F	BMVW-t	MT20	4.0	6.0	2.00	2.25
G	BMWWW-t	MT20	4.0	9.0		
H	BMWW-t	MT20	4.0	4.0		
I	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	719	0	719	0	0	1-8	1-8
E	699	0	699	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	546	272 / 0	123 / 0	0 / 0	0 / 0	152 / 0	0 / 0
E	531	260 / 0	123 / 0	0 / 0	0 / 0	149 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

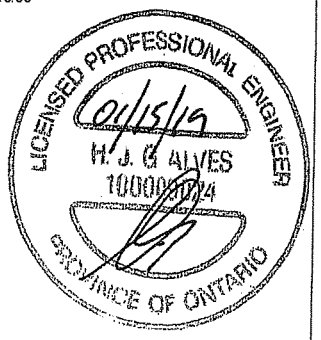
1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-F, C-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	-423 / 0	-84.9	-84.9	0.29	(1)	6.25	H-B	-59 / 121	0.04 (1)
B-C	-233 / 0	-84.9	-84.9	0.28	(1)	6.25	B-G	-342 / 0	0.39 (1)
C-D	-170 / 0	-84.9	-84.9	0.09	(1)	6.25	G-C	-130 / 18	0.07 (1)
F-D	-738 / 0	0.0	0.0	0.30	(1)	6.25	G-D	0 / 653	0.15 (1)
I-A	-649 / 0	0.0	0.0	0.13	(1)	7.81	A-H	0 / 450	0.10 (1)
I-H	0 / 0	-38.5	-38.5	0.15	(3)	10.00			
H-G	0 / 373	-38.5	-38.5	0.19	(2)	10.00			
G-F	0 / 0	-38.5	-38.5	0.24	(1)	10.00			
F-E	0 / 0	-38.5	-38.5	0.10	(1)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/380 (0.39")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.39")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.30/1.00 (D-F:1) , BC=0.24/1.00 (F-G:1) , WB=0.39/1.00 (B-G:1) , SSI=0.27/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (D) (INPUT = 0.90)
JSI METAL= 0.22 (F) (INPUT = 1.00)

DESIGNED FOR TAM T1901210
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300330	P1G	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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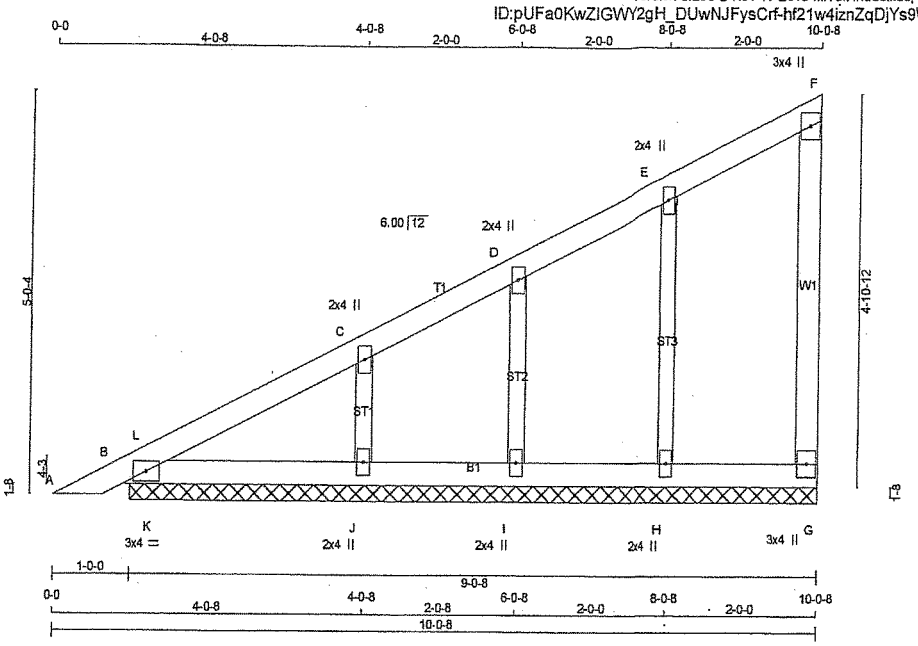


TOTAL WEIGHT = 2 X 34 = 67 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - F	2x4	ORY	No.2	SPF
G - F	2x4	ORY	No.2	SPF
B - G	2x4	ORY	No.2	SPF
ALL WEBS	2x3	ORY	No.2	SPF
ALL GABLE WEBS	2x3	ORY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB1-I	MT20	3.0	4.0	
C, D, E					
C	TMW+w	MT2D	2.0	4.0	
F	TMV+p	MT2D	3.0	4.0	
G	BMV1+p	MT20	3.0	4.0	
H, I, J					
H	BMW1+w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

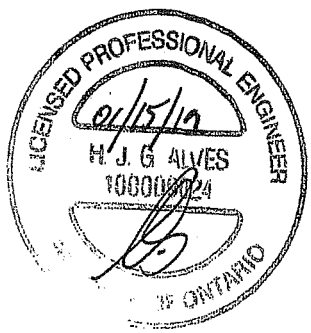
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 15	-84.9	-84.9	0.05 (1)	10.00
B- L	-37 / 0	-84.9	-84.9	0.03 (3)	6.25
L- C	-7 / 3	-84.9	-84.9	0.08 (1)	10.00
C- D	-18 / 0	-84.9	-84.9	0.08 (1)	6.25
D- E	0 / 0	-84.9	-84.9	0.05 (1)	10.00
E- F	-8 / 0	-84.9	-84.9	0.05 (1)	10.00
G- F	-70 / 0	0.0	0.0	0.03 (1)	7.81
B- K	0 / 17	-38.5	-38.5	0.06 (1)	10.00
K- J	0 / 17	-38.5	-38.5	0.07 (1)	10.00
J- I	0 / 7	-38.5	-38.5	0.06 (2)	10.00
I- H	0 / 4	-38.5	-38.5	0.03 (3)	10.00
H- G	0 / 0	-38.5	-38.5	0.03 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- E	-197 / 0	0.05 (1)
I- D	-138 / 0	0.02 (1)
J- C	-241 / 0	0.04 (1)
K- L	-74 / 46	0.00 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.08/1.00 (C-D:1) , BC=0.07/1.00 (J-K:1) , WB=0.05/1.00 (E-H:1) , SSI=0.10/1.00 (C-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.15 (B) (INPUT = 0.90)
JSI METAL= 0.10 (C) (INPUT = 1.00)

TAM 1/30/2019

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300330	P2	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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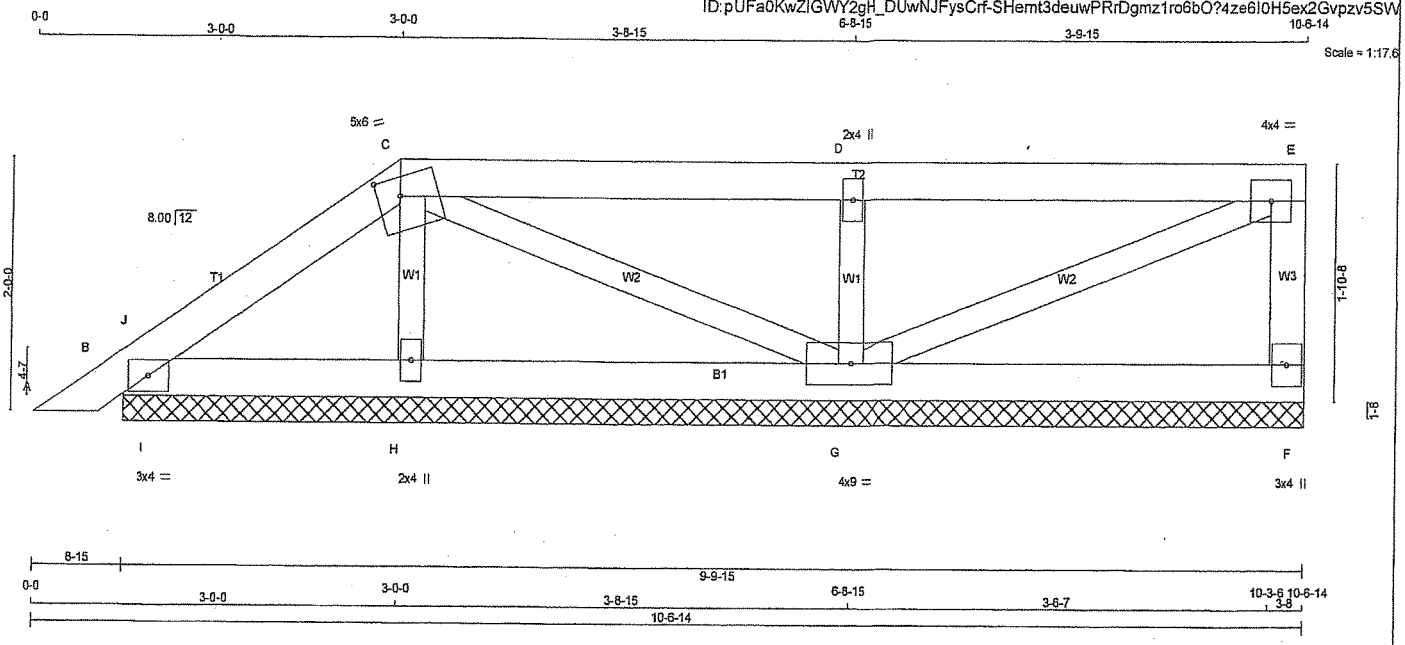

TOTAL WEIGHT = 2 X 33 = 67 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	OESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW-m	MT20	5.0	6.0	1.75	2.25
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	4.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW1-t	MT20	4.0	9.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	181	0	181	0	0	9-9-15	9-9-15
B	209	0	209	0	0	9-9-15	9-9-15
H	300	0	300	0	0	9-9-15	9-9-15
G	579	0	579	0	0	9-9-15	9-9-15

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	137	67 / 0	32 / 0	0 / 0	0 / 0	39 / 0	0 / 0
B	152	96 / 0	19 / 0	0 / 0	0 / 0	37 / 0	0 / 0
H	233	100 / 0	64 / 0	0 / 0	0 / 0	69 / 0	0 / 0
G	436	225 / 0	92 / 0	0 / 0	0 / 0	119 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 14	-84.9	-84.9	0.03 (1)	10.00	H- C	-161 / 0	0.02 (1)
B- J	-42 / 0	-84.9	-84.9	0.01 (3)	6.25	G- E	-2 / 0	0.00 (1)
J- C	-48 / 0	-84.9	-84.9	0.05 (1)	6.25	C- G	-32 / 0	0.01 (1)
C- D	0 / 2	-84.9	-84.9	0.21 (1)	10.00	G- D	-401 / 0	0.06 (1)
D- E	0 / 2	-84.9	-84.9	0.21 (1)	10.00	I- J	-105 / 6	0.00 (1)
F- E	-122 / 0	0.0	0.0	0.01 (1)	7.81			
B- I	0 / 39	-38.5	-38.5	0.05 (1)	10.00			
I- H	0 / 39	-38.5	-38.5	0.06 (2)	10.00			
H- G	0 / 28	-38.5	-38.5	0.11 (3)	10.00			
G- F	0 / 0	-38.5	-38.5	0.11 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.21/1.00 (C-D:1) , BC=0.11/1.00 (G-H:3) , WB=0.06/1.00 (D-G:1) , SSI=0.18/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PU) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (D) (INPUT = 0.90)
JSI METAL= 0.08 (D) (INPUT = 1.00)

DESIGN FOR STRUCTURAL COMPONENT ONLY

01901212

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300330	P3	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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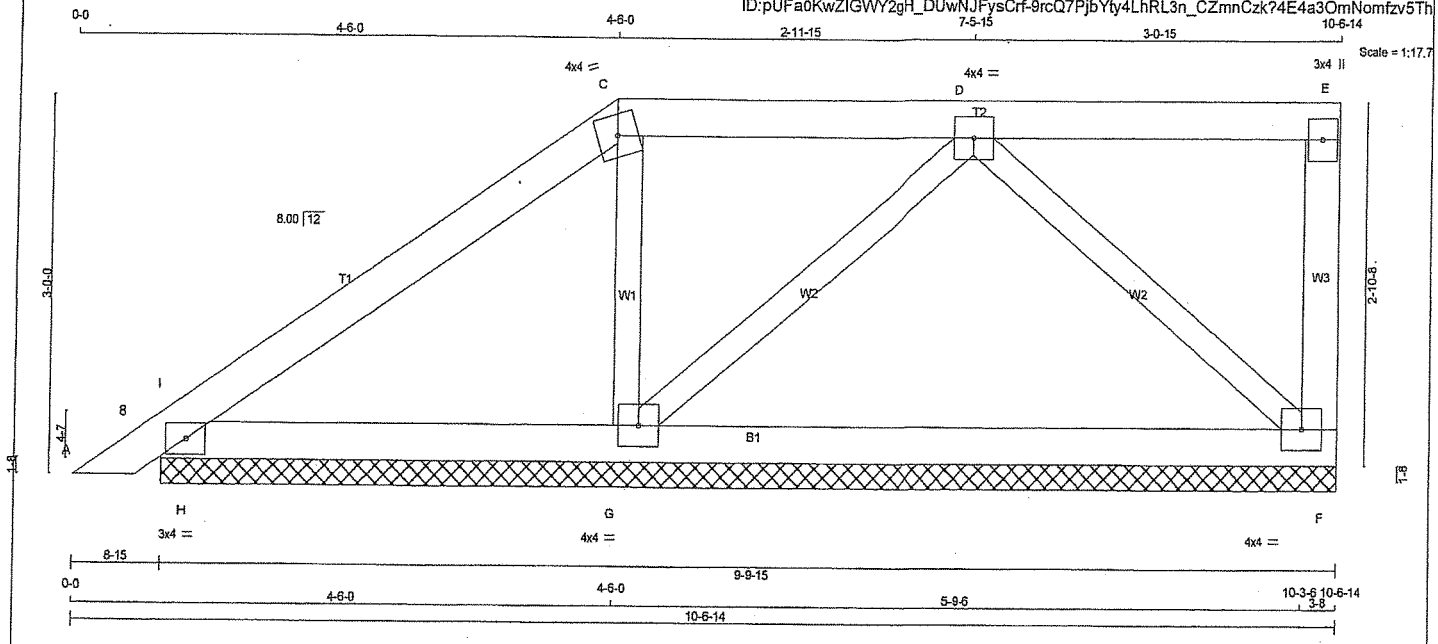


TOTAL WEIGHT = 2 X 35 = 69 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB1-I	MT20	3.0	4.0	
C	TTW-m	MT20	4.0	4.0	
D	TMWW-t	MT20	4.0	4.0	
E	TMV+p	MT20	3.0	4.0	
F	BMVW1-t	MT20	4.0	4.0	
G	BMWW1-t	MT20	4.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	355	0	355	0	0	9-9-15	9-9-15
B	274	0	274	0	0	9-9-15	9-9-15
G	639	0	639	0	0	9-9-15	9-9-15

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	268	144 / 0	51 / 0	0 / 0	0 / 0	71 / 0	0 / 0
B	198	128 / 0	22 / 0	0 / 0	0 / 0	48 / 0	0 / 0
G	494	217 / 0	133 / 0	0 / 0	0 / 0	145 / 0	0 / 0

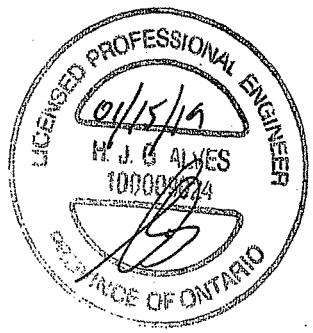
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 14	-84.9	-84.9	0.03 (1)	10.00	G- C	-208 / 0	0.04 (1)
B- I	0 / 49	-84.9	-84.9	0.07 (1)	10.00	G- D	-230 / 0	0.06 (1)
I- C	-47 / 0	-84.9	-84.9	0.14 (1)	6.25	D- F	-257 / 0	0.07 (1)
C- D	-25 / 0	-84.9	-84.9	0.13 (1)	6.25	H- I	-312 / 0	0.00 (1)
D- E	0 / 0	-84.9	-84.9	0.13 (1)	10.00			
F- E	-100 / 0	0.0	0.0	0.01 (1)	7.81			
B- H	0 / 33	-38.5	-38.5	0.14 (1)	10.00			
H- G	0 / 33	-38.5	-38.5	0.25 (2)	10.00			
G- F	0 / 197	-38.5	-38.5	0.27 (2)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (C-I:1) , BC=0.27/1.00 (F-G:2) , WB=0.07/1.00 (D-F:1) , SSI=0.25/1.00 (B-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (F) (INPUT = 0.90)
JSI METAL= 0.07 (F) (INPUT = 1.00)

DOES NOT TAM 719 012/3
FOR STRUCTURAL
CONFORMANCE ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300330	P4	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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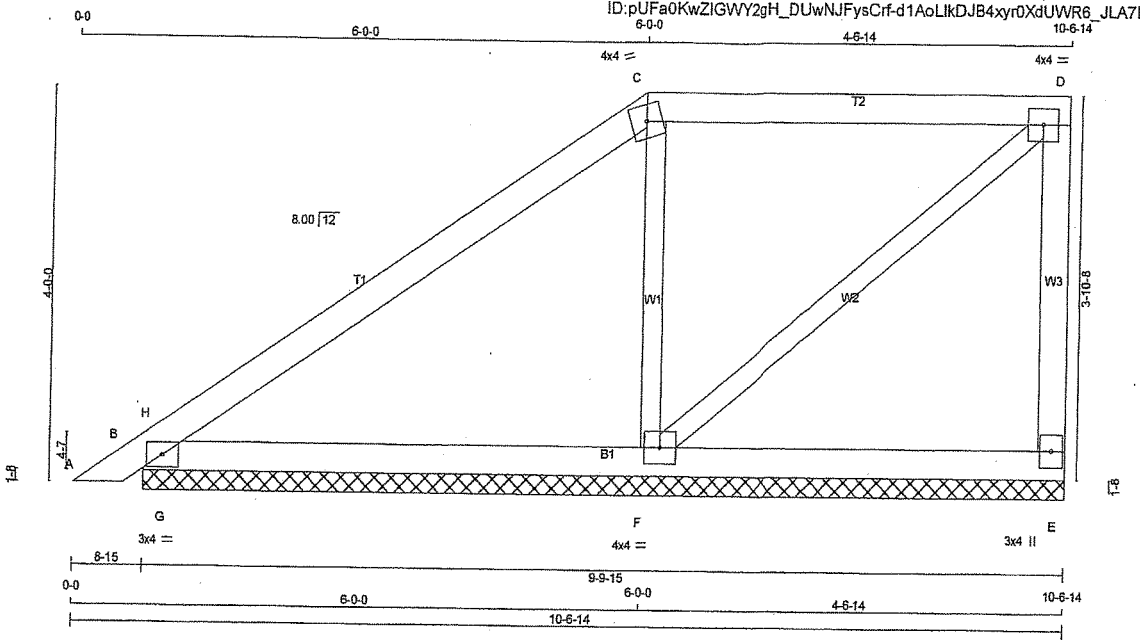


TOTAL WEIGHT = 3 X 35 = 106 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB1-t	MT20	3.0	4.0	
C	TTW-m	MT20	4.0	4.0	
D	TMVW-t	MT20	4.0	4.0	
E	BMV1+p	MT20	3.0	4.0	
F	BMVW1-t	MT20	4.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	307	0	307	0	0	9-9-15	9-9-15
B	413	0	413	0	0	9-9-15	9-9-15
F	548	0	548	0	0	9-9-15	9-9-15

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	228	128 / 0	41 / 0	0 / 0	0 / 0	60 / 0	0 / 0
B	306	178 / 0	51 / 0	0 / 0	0 / 0	79 / 0	0 / 0
F	424	185 / 0	115 / 0	0 / 0	0 / 0	124 / 0	0 / 0

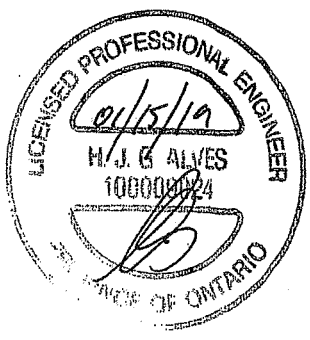
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 14	-84.9	-84.9	0.03 (1)	10.00	F- C	-330 / 0	0.08 (1)
B- H	-106 / 45	-84.9	-84.9	0.11 (1)	6.25	F- D	0 / 101	0.02 (1)
H- C	-115 / 0	-84.9	-84.9	0.31 (1)	6.25	G- H	-473 / 72	0.00 (1)
C- D	-79 / 0	-84.9	-84.9	0.30 (1)	6.25			
E- D	-257 / 0	0.0	0.0	0.06 (1)	7.81			
B- G	0 / 88	-38.5	-38.5	0.23 (1)	10.00			
G- F	0 / 88	-38.5	-38.5	0.25 (1)	10.00			
F- E	0 / 0	-38.5	-38.5	0.24 (2)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.31/1.00 (C-H:1) , BC=0.25/1.00 (F-G:1) , WB=0.08/1.00 (C-F:1) , SSI=0.41/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.31 (B) (INPUT = 0.90)
JSI METAL= 0.08 (C) (INPUT = 1.00)

Date: 190214
STRUCTURAL

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300334	P5	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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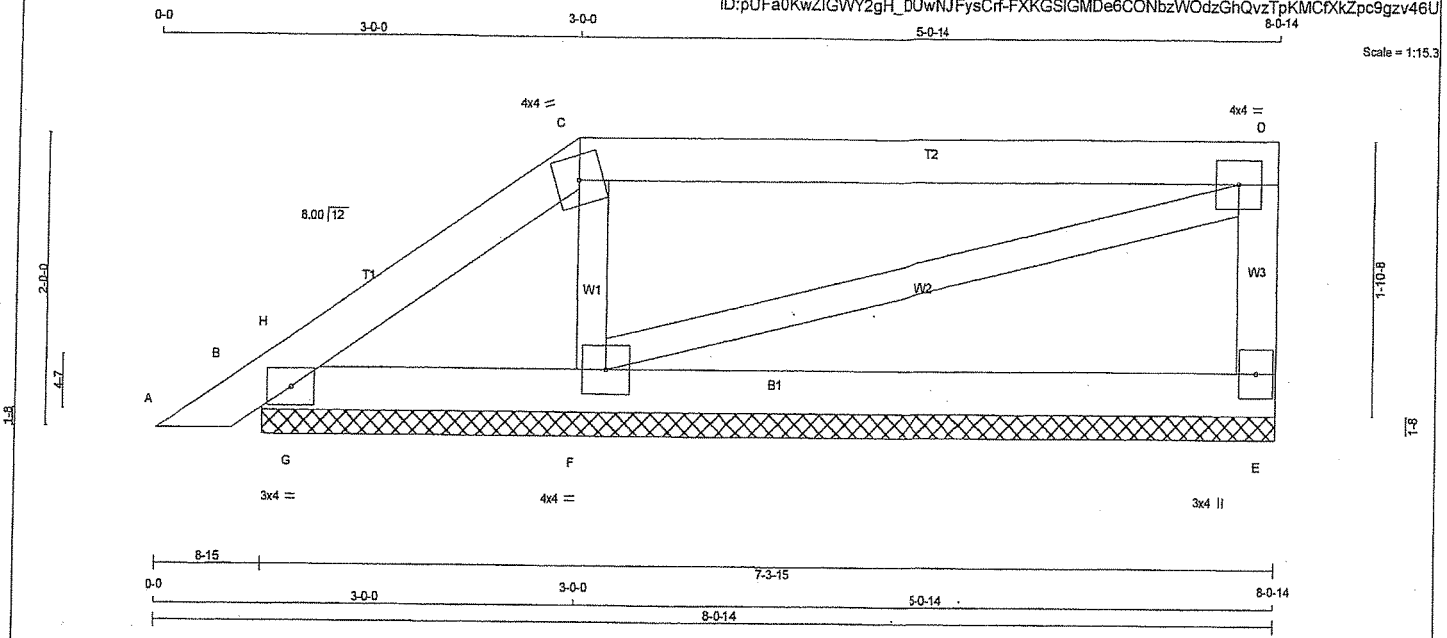


TOTAL WEIGHT = 2 X 25 = 49 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	ORY	No.2	SPF
E - D	2x4	ORY	No.2	SPF
B - E	2x4	ORY	No.2	SPF
ALL WEBS	2x3	ORY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB1-I	MT20	3.0	4.0	
C	TTW-m	MT20	4.0	4.0	
D	TMVW-t	MT20	4.0	4.0	
E	BMV1+p	MT20	3.0	4.0	
F	BMWW1-t	MT20	4.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	295	0	295	0	0	7-3-15	7-3-15
B	179	0	179	0	0	7-3-15	7-3-15
F	486	0	486	0	0	7-3-15	7-3-15

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	221	120 / 0	42 / 0	0 / 0	0 / 0	59 / 0	0 / 0
B	125	95 / 0	3 / 0	0 / 0	0 / 0	26 / 0	0 / 0
F	379	157 / 0	109 / 0	0 / 0	0 / 0	113 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 14	-84.9	-84.9	3	0.03 (1)	10.00	F-C	-268 / 0	0.04 (1)
B-H	0 / 26	-84.9	-84.9	3	0.03 (2)	10.00	F-D	0 / 14	0.00 (1)
H-C	-43 / 0	-84.9	-84.9	3	0.04 (1)	6.25	G-H	-164 / 0	0.00 (1)
C-D	-13 / 0	-84.9	-84.9	3	0.37 (1)	6.25			
E-D	-219 / 0	0.0	0.0	3	0.02 (1)	7.81			
B-G	0 / 31	-38.5	-38.5	3	0.06 (1)	10.00			
G-F	0 / 31	-38.5	-38.5	3	0.17 (3)	10.00			
F-E	0 / 0	-38.5	-38.5	3	0.17 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.37/1.00 (C-D:1) , BC=0.17/1.00 (F-G:3) , WB=0.04/1.00 (C-F:1) , SSI=0.17/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

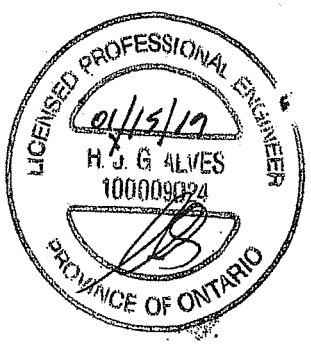
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (C) (INPUT = 0.90)
JSI METAL= 0.04 (F) (INPUT = 1.00)



DWG NO. TAM T90 1215
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300334	P6	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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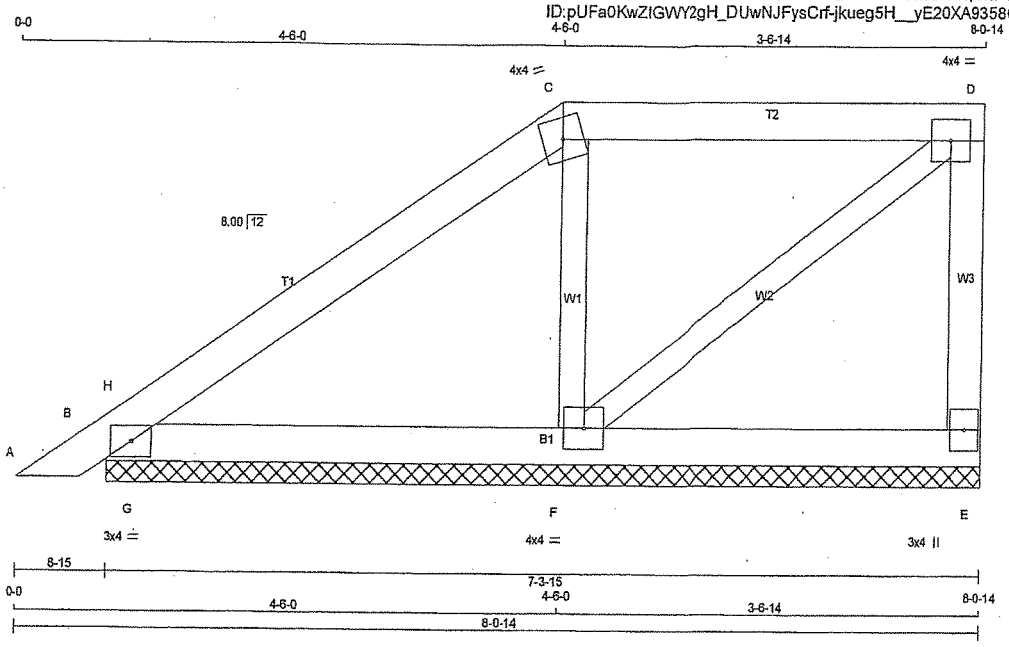


TOTAL WEIGHT = 2 X 26 = 52 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB1-I	MT20	3.0	4.0	
C	TTW-m	MT20	4.0	4.0	
D	TMVW-t	MT20	4.0	4.0	
E	BMV1+p	MT20	3.0	4.0	
F	BMWW1-t	MT20	4.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	225	0	225	0	0	7-3-15	7-3-15
B	304	0	304	0	0	7-3-15	7-3-15
F	431	0	431	0	0	7-3-15	7-3-15

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	167	93 / 0	30 / 0	0 / 0	0 / 0	44 / 0	0 / 0
B	224	132 / 0	35 / 0	0 / 0	0 / 0	57 / 0	0 / 0
F	333	147 / 0	89 / 0	0 / 0	0 / 0	97 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

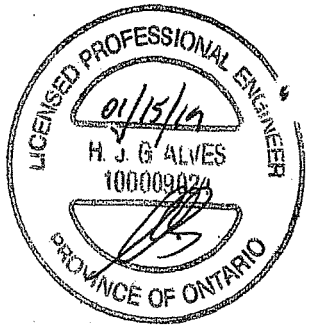
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 14	-84.9	-84.9	0.03 (1)	10.00
B- H	-59 / 0	-84.9	-84.9	0.04 (1)	6.25
H- C	-67 / 0	-84.9	-84.9	0.15 (1)	6.25
C- D	-42 / 0	-84.9	-84.9	0.18 (1)	6.25
E- D	-182 / 0	0.0	0.0	0.03 (1)	7.81
B- G	0 / 52	-38.5	-38.5	0.12 (1)	10.00
G- F	0 / 52	-38.5	-38.5	0.14 (2)	10.00
F- E	0 / 0	-38.5	-38.5	0.13 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	-255 / 0	0.04 (1)
F- D	0 / 52	0.01 (1)
G- H	-253 / 36	0.00 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = **24.0** IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.18/1.00 (C-D:1) , BC=0.14/1.00 (F-G:2) , WB=0.04/1.00 (C-F:1) , SSI=0.22/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (B) (INPUT = 0.90)
JSI METAL= 0.06 (C) (INPUT = 1.00)

DWG NO. TAM 7190/216
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300334	P7	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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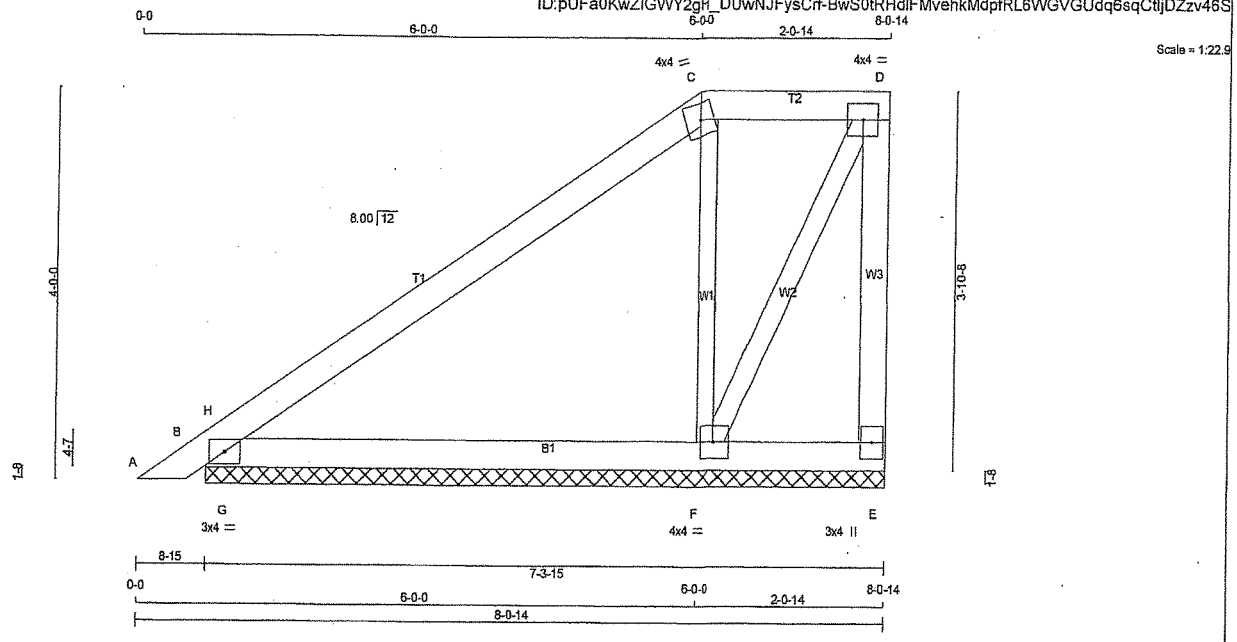


TOTAL WEIGHT = 4 X 28 = 113 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB1-I	MT20	3.0	4.0	
C	TTW-m	MT20	4.0	4.0	
D	TMVW-t	MT20	4.0	4.0	
E	BMV1+p	MT20	3.0	4.0	
F	BMWW1-t	MT20	4.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	129	0	129	0	0	7-3-15	7-3-15
B	390	0	390	0	0	7-3-15	7-3-15
F	442	0	442	0	0	7-3-15	7-3-15

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	92	63 / 0	7 / 0	0 / 0	0 / 0	21 / 0	0 / 0
B	290	162 / 0	51 / 0	0 / 0	0 / 0	76 / 0	0 / 0
F	343	147 / 0	95 / 0	0 / 0	0 / 0	101 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B, F

BRACING

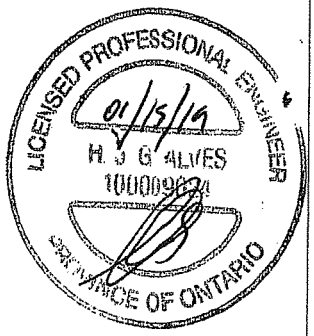
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)		LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTDRED FORCE (LBS)	MAX CSI (LC)
FR-TO		FRDM	TO			FR-TO		
A- B	0 / 14	-84.9	-84.9	0.03 (1)	10.00	F- C	-247 / 0	0.06 (1)
B- H	-103 / 88	-84.9	-84.9	0.12 (1)	6.25	F- D	0 / 96	0.02 (1)
H- C	-73 / 0	-84.9	-84.9	0.31 (1)	6.25	G- H	-476 / 88	0.00 (1)
C- D	-46 / 0	-84.9	-84.9	0.08 (1)	6.25			
E- D	-172 / 0	0.0	0.0	0.04 (1)	7.81			
B- G	0 / 54	-38.5	-38.5	0.22 (1)	10.00			
G- F	0 / 54	-38.5	-38.5	0.24 (1)	10.00			
F- E	0 / 0	-38.5	-38.5	0.22 (2)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.31/1.00 (C-H:1) , BC=0.24/1.00 (F-G:1) , WB=0.06/1.00 (C-F:1) , SSI=0.41/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTDR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)		SHEAR (PSI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.29 (B) (INPUT = 0.90)
JSI METAL= 0.10 (C) (INPUT = 1.00)

DWG NO. TAM 7901224
STRUCTURAL
[illegible] ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300581	P8	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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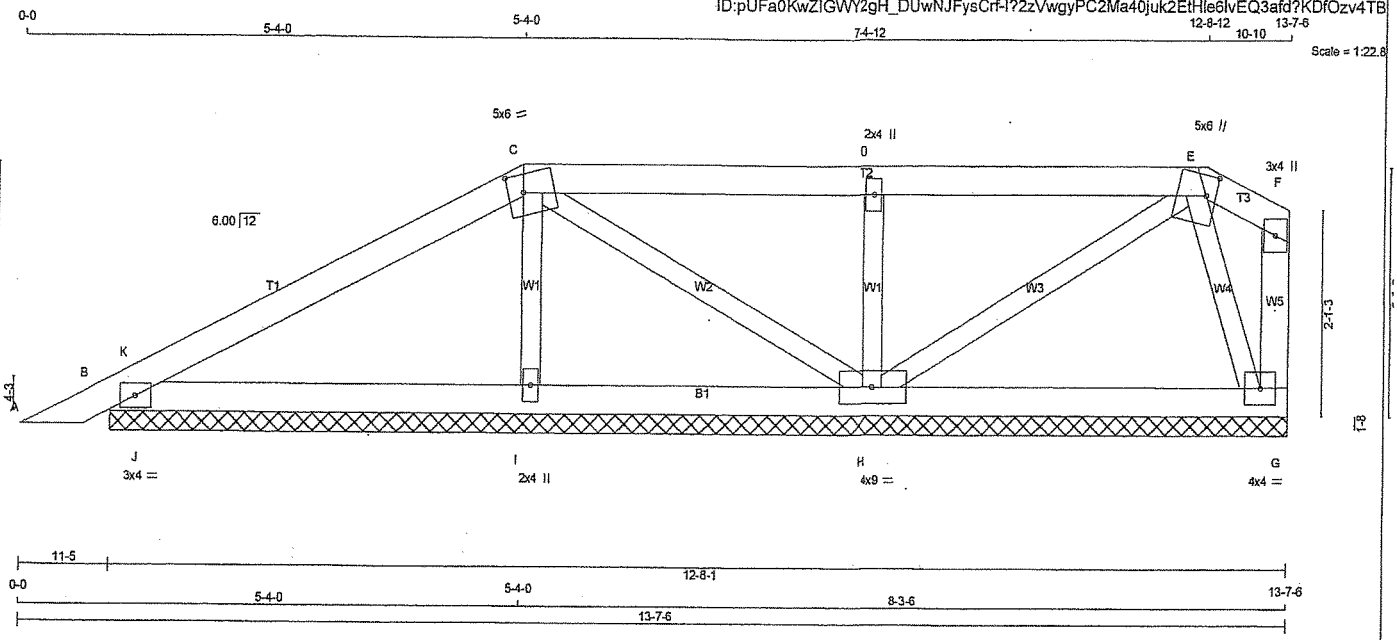


TOTAL WEIGHT = 44 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
B - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW-m	MT20	5.0	6.0	2.25	2.00
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.50	1.25
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BMWWW1-t	MT20	4.0	9.0		
I	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	239	0	239	0	0	12-8-1	12-8-1
B	370	0	370	0	0	12-8-1	12-8-1
I	416	0	416	0	0	12-8-1	12-8-1
H	613	0	613	0	0	12-8-1	12-8-1

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	181	91 / 0	40 / 0	0 / 0	0 / 0	50 / 0	0 / 0
B	273	159 / 0	44 / 0	0 / 0	0 / 0	70 / 0	0 / 0
I	321	143 / 0	85 / 0	0 / 0	0 / 0	93 / 0	0 / 0
H	462	238 / 0	97 / 0	0 / 0	0 / 0	126 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, B, I, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

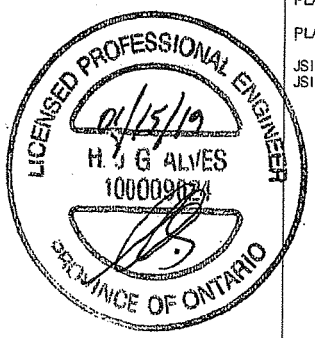
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 15	-84.9	-84.9	0.05 (1)	10.00
B- K	-84 / 0	-84.9	-84.9	0.05 (3)	6.25
K- C	-98 / 0	-84.9	-84.9	0.21 (1)	6.25
C- D	-13 / 0	-84.9	-84.9	0.19 (1)	6.25
D- E	-13 / 0	-84.9	-84.9	0.19 (1)	6.25
E- F	0 / 0	-84.9	-84.9	0.01 (1)	10.00
G- F	-40 / 0	0.0	0.0	0.00 (1)	7.81
B- J	0 / 85	-38.5	-38.5	0.17 (1)	10.00
J- I	0 / 85	-38.5	-38.5	0.18 (1)	10.00
I- H	0 / 76	-38.5	-38.5	0.15 (2)	10.00
H- G	0 / 53	-38.5	-38.5	0.13 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- C	-207 / 0	0.03 (1)
C- H	-72 / 0	0.02 (1)
H- D	-389 / 0	0.06 (1)
E- G	-136 / 0	0.02 (1)
H- E	-46 / 0	0.01 (1)
J- K	-240 / 63	0.00 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.21/1.00 (C-K:1) , BC=0.18/1.00 (I-J:1) , WB=0.06/1.00 (D-H:1) , SSI=0.22/1.00 (B-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (B) (INPUT = 0.90)
JSI METAL= 0.08 (D) (INPUT = 1.00)

NG NO.TAM 17901217
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300333	P8A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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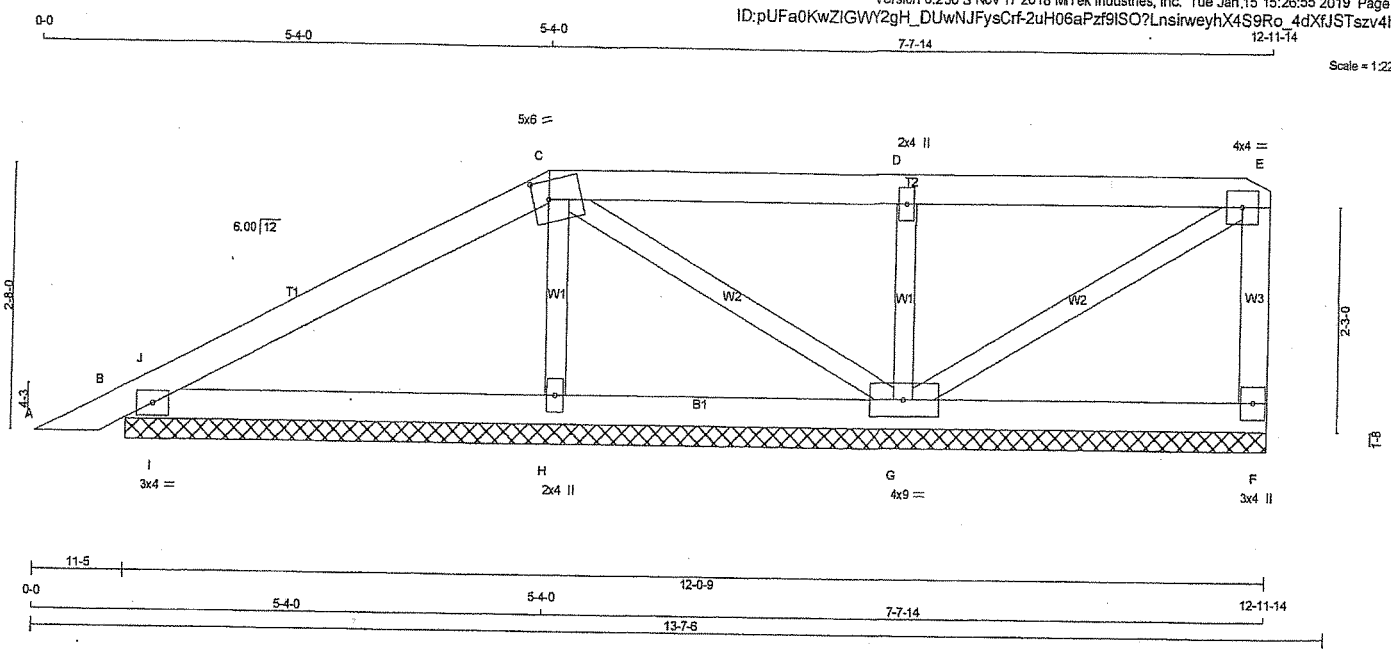


TOTAL WEIGHT = 41 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW-m	MT20	5.0	6.0	2.25	2.00
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	4.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW1-t	MT20	4.0	9.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	189	0	189	0	0	12-0-9	12-0-9
B	367	0	367	0	0	12-0-9	12-0-9
H	419	0	419	0	0	12-0-9	12-0-9
G	586	0	586	0	0	12-0-9	12-0-9

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	144	69 / 0	34 / 0	0 / 0	0 / 0	41 / 0	0 / 0
B	270	158 / 0	43 / 0	0 / 0	0 / 0	69 / 0	0 / 0
H	325	141 / 0	88 / 0	0 / 0	0 / 0	95 / 0	0 / 0
G	440	234 / 0	87 / 0	0 / 0	0 / 0	118 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, H, G

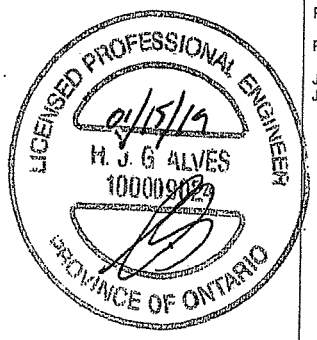
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 15	-84.9	-84.9	0.05 (1)	10.00	
B- J	-77 / 0	-84.9	-84.9	0.05 (3)	6.25	
J- C	-92 / 0	-84.9	-84.9	0.21 (1)	6.25	
C- D	-5 / 0	-84.9	-84.9	0.21 (1)	10.00	
D- E	-5 / 0	-84.9	-84.9	0.21 (1)	10.00	
F- E	-126 / 0	0.0	0.0	0.02 (1)	7.81	
B- I	0 / 80	-38.5	-38.5	0.17 (1)	10.00	
I- H	0 / 80	-38.5	-38.5	0.18 (1)	10.00	
H- G	0 / 71	-38.5	-38.5	0.15 (2)	10.00	
G- F	0 / 0	-38.5	-38.5	0.10 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-203 / 0	0.03 (1)
C- G	-79 / 0	0.02 (1)
G- D	-405 / 0	0.06 (1)
G- E	0 / 6	0.00 (3)
I- J	-243 / 60	0.00 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.21/1.00 (C-J:1) , BC=0.18/1.00 (H-I:1) , WB=0.08/1.00 (D-G:1) , SSI=0.22/1.00 (B-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.26 (B) (INPUT = 0.90)
JSI METAL= 0.09 (D) (INPUT = 1.00)

TAM 01/15/19
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300581	P9	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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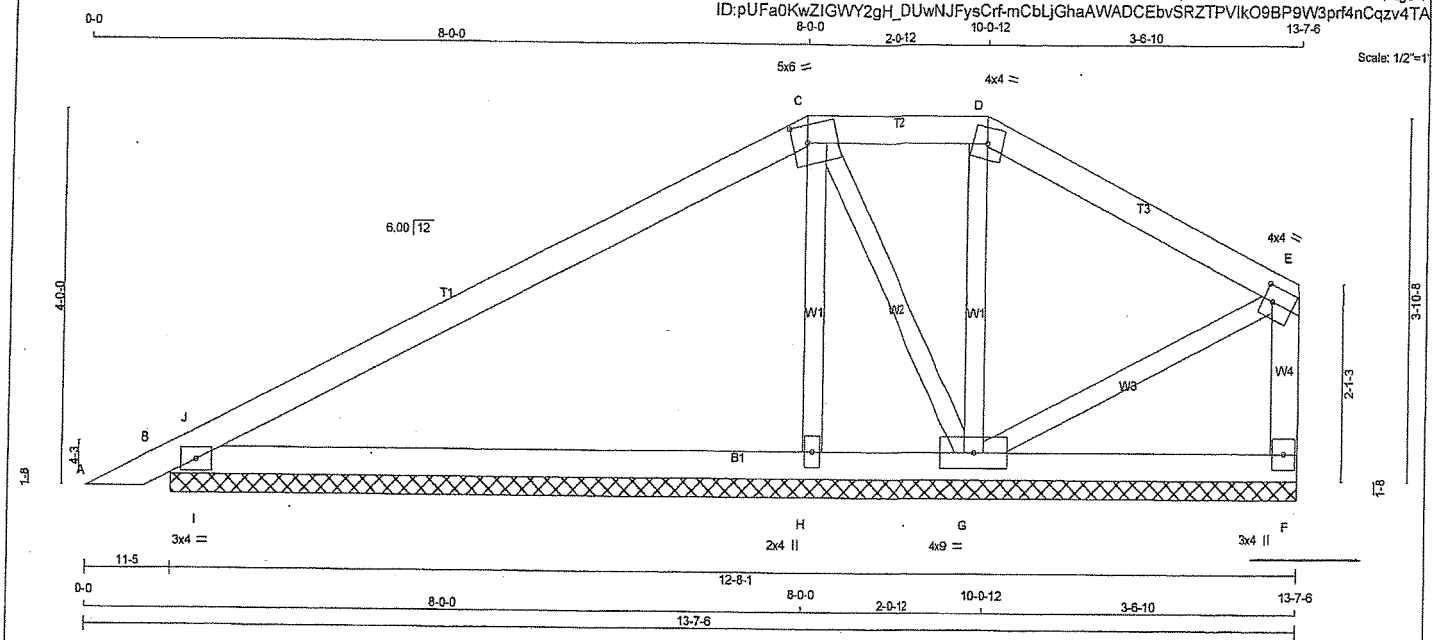

TOTAL WEIGHT = 45 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW-m	MT20	5.0	6.0	2.25	2.00
D	TTW-m	MT20	4.0	4.0		
E	TMVW-t	MT20	4.0	4.0	2.00	1.25
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW1-t	MT20	4.0	9.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	309	0	309	0	0	12-8-1	12-8-1
B	556	0	556	0	0	12-8-1	12-8-1
H	525	0	525	0	0	12-8-1	12-8-1
G	247	0	247	0	0	12-8-1	12-8-1

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	232	121 / 0	48 / 0	0 / 0	0 / 0	63 / 0	0 / 0
B	413	233 / 0	72 / 0	0 / 0	0 / 0	108 / 0	0 / 0
H	415	156 / 0	132 / 0	0 / 0	0 / 0	128 / 0	0 / 0
G	177	121 / 0	15 / 0	0 / 0	0 / 0	41 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 15	-84.9	-84.9	0.05 (1)	10.00	
B- J	-159 / 0	-84.9	-84.9	0.18 (1)	8.25	
J- C	-199 / 0	-84.9	-84.9	0.56 (1)	6.25	
C- D	-82 / 0	-84.9	-84.9	0.06 (1)	6.25	
D- E	-98 / 0	-84.9	-84.9	0.18 (1)	6.25	
F- E	-238 / 0	0.0	0.0	0.03 (1)	7.81	
B- I	0 / 170	-38.5	-38.5	0.40 (1)	10.00	
I- H	0 / 170	-38.5	-38.5	0.44 (2)	10.00	
H- G	0 / 166	-38.5	-38.5	0.44 (2)	10.00	
G- F	0 / 0	-38.5	-38.5	0.10 (2)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-111 / 1	0.03 (1)
C- G	-175 / 0	0.05 (1)
G- D	-194 / 0	0.05 (1)
G- E	0 / 98	0.02 (1)
I- J	-626 / 127	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.56/1.00 (C-J:1) , BC=0.44/1.00 (H-I:2) , WB=0.05/1.00 (C-G:1) , SSI=0.54/1.00 (B-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.42 (B) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
01/15/19
H. J. G. ALVES
100089924
PROVINCE OF ONTARIO

J. TAM 01/15/19
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300333	P9A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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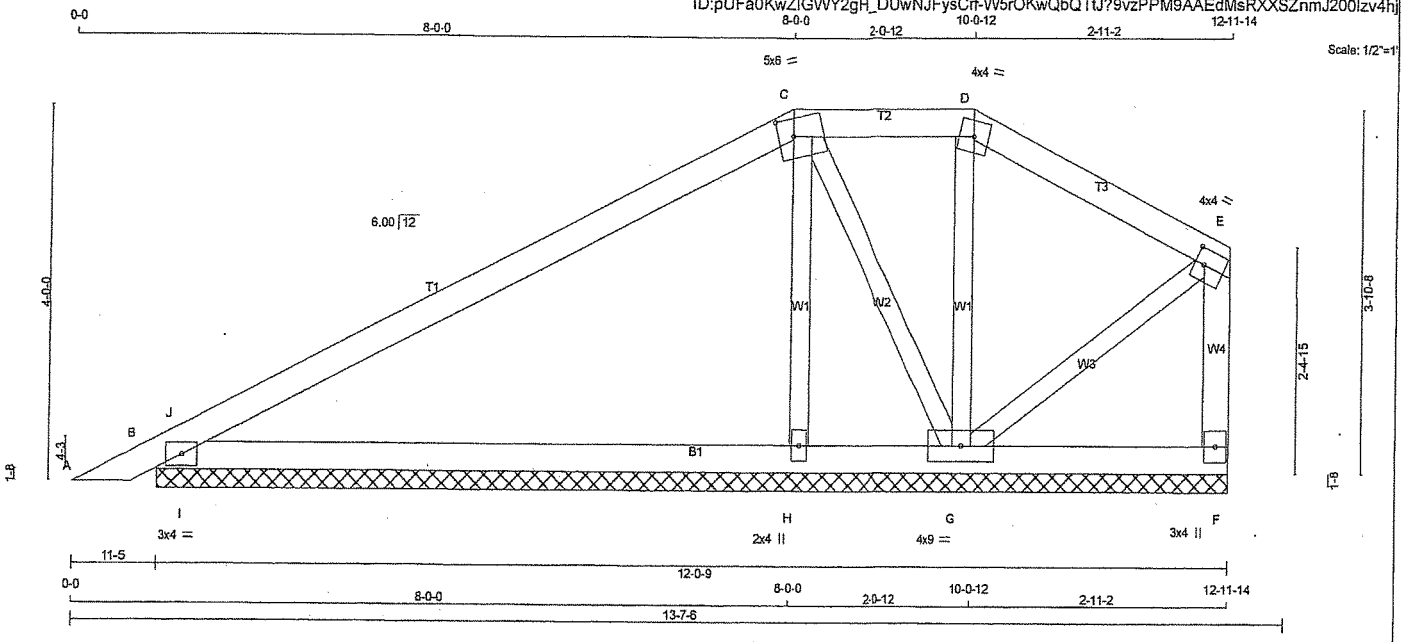

TOTAL WEIGHT = 43 lb [M]

LUMBER

N. L. G. A. RULES

CHOROS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

ORY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-t	MT20	3.0	4.0		
C	TTWW-m	MT20	5.0	6.0	2.25	2.00
O	TTW-m	MT20	4.0	4.0		
E	TMVW-t	MT20	4.0	4.0	2.00	1.25
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW1-t	MT20	4.0	9.0		
H	BMW1+w	MT20	2.0	4.0		

OIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO 8E VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REORD BRG IN-SX
F	287	0	287	0	0	12-0-9	12-0-9
B	554	0	554	0	0	12-0-9	12-0-9
H	537	0	537	0	0	12-0-9	12-0-9
G	183	0	183	0	0	12-0-9	12-0-9

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	217	111 / 0	46 / 0	0 / 0	0 / 0	59 / 0	0 / 0
B	411	232 / 0	71 / 0	0 / 0	0 / 0	107 / 0	0 / 0
H	426	156 / 0	138 / 0	0 / 0	0 / 0	132 / 0	0 / 0
G	125	103 / 0	0 / -2	0 / 0	0 / 0	25 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, 8, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING OIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST 8E LATERALLY RESTRAINED.

LDADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 15	-84.9	-84.9	0.05 (1)	10.00
B- J	-157 / 0	-84.9	-84.9	0.18 (1)	6.25
J- C	-194 / 0	-84.9	-84.9	0.56 (1)	6.25
C- D	-76 / 0	-84.9	-84.9	0.06 (1)	6.25
D- E	-90 / 0	-84.9	-84.9	0.12 (1)	6.25
F- E	-221 / 0	0.0	0.0	0.03 (1)	7.81
B- I	0 / 166	-38.5	-38.5	0.40 (1)	10.00
I- H	0 / 165	-38.5	-38.5	0.44 (2)	10.00
H- G	0 / 162	-38.5	-38.5	0.44 (2)	10.00
G- F	0 / 0	-38.5	-38.5	0.09 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-109 / 2	0.03 (1)
C- G	-180 / 0	0.05 (1)
G- D	-171 / 0	0.04 (1)
G- E	0 / 98	0.02 (1)
I- J	-628 / 125	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9.OF BC8C 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.56/1.00 (C-J:1) , BC=0.44/1.00 (H-I:2) , WB=0.05/1.00 (C-G:1) , SSI=0.54/1.00 (B-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.42 (B) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

DWG NO. TAM T190 1220
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300581	P10	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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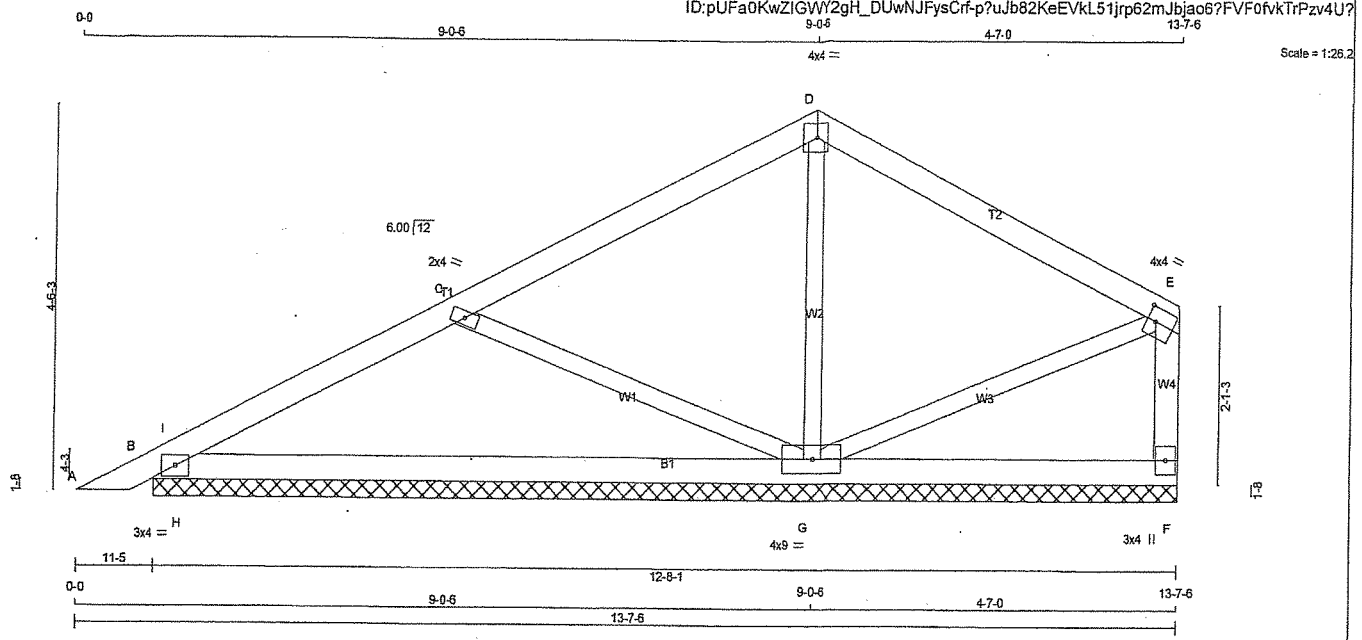


TOTAL WEIGHT = 44 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-t	MT20	3.0	4.0		
C	TMW+w	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	4.0		
E	TMVW-t	MT20	4.0	4.0	2.00	1.25
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW1-t	MT20	4.0	9.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	154	0	154	0	0	12-8-1	12-8-1
B	500	0	500	0	0	12-8-1	12-8-1
G	983	0	983	0	0	12-8-1	12-8-1

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	116	59 / 0	25 / 0	0 / 0	0 / 0	32 / 0	0 / 0
B	374	202 / 0	72 / 0	0 / 0	0 / 0	100 / 0	0 / 0
G	746	370 / 0	169 / 0	0 / 0	0 / 0	208 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

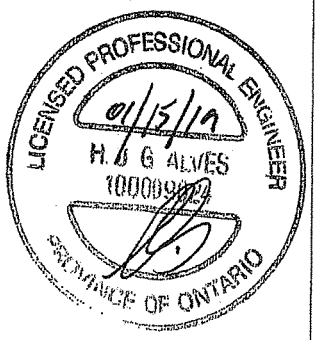
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 15	-84.9	-84.9	0.05 (1)	10.00
B- I	-671 / 0	-84.9	-84.9	0.28 (3)	6.25
I- C	-413 / 0	-84.9	-84.9	0.28 (3)	6.25
C- D	0 / 89	-84.9	-84.9	0.28 (1)	10.00
D- E	0 / 109	-84.9	-84.9	0.32 (1)	10.00
F- E	-107 / 0	0.0	0.0	0.01 (1)	7.81
B- H	0 / 411	-38.5	-38.5	0.23 (3)	10.00
H- G	0 / 411	-38.5	-38.5	0.38 (2)	10.00
G- F	0 / 0	-38.5	-38.5	0.33 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- G	-556 / 0	0.22 (1)
G- D	-432 / 0	0.12 (1)
G- E	-105 / 0	0.04 (1)
H- I	0 / 542	0.00 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI. TC=0.32/1.00 (D-E:1) , BC=0.38/1.00 (G-H:2) , WB=0.22/1.00 (C-G:1) , SSI=0.34/1.00 (B-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.43 (C) (INPUT = 0.90)
JSI METAL= 0.29 (C) (INPUT = 1.00)

DWG NO. TAM T1901221
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300333	P10A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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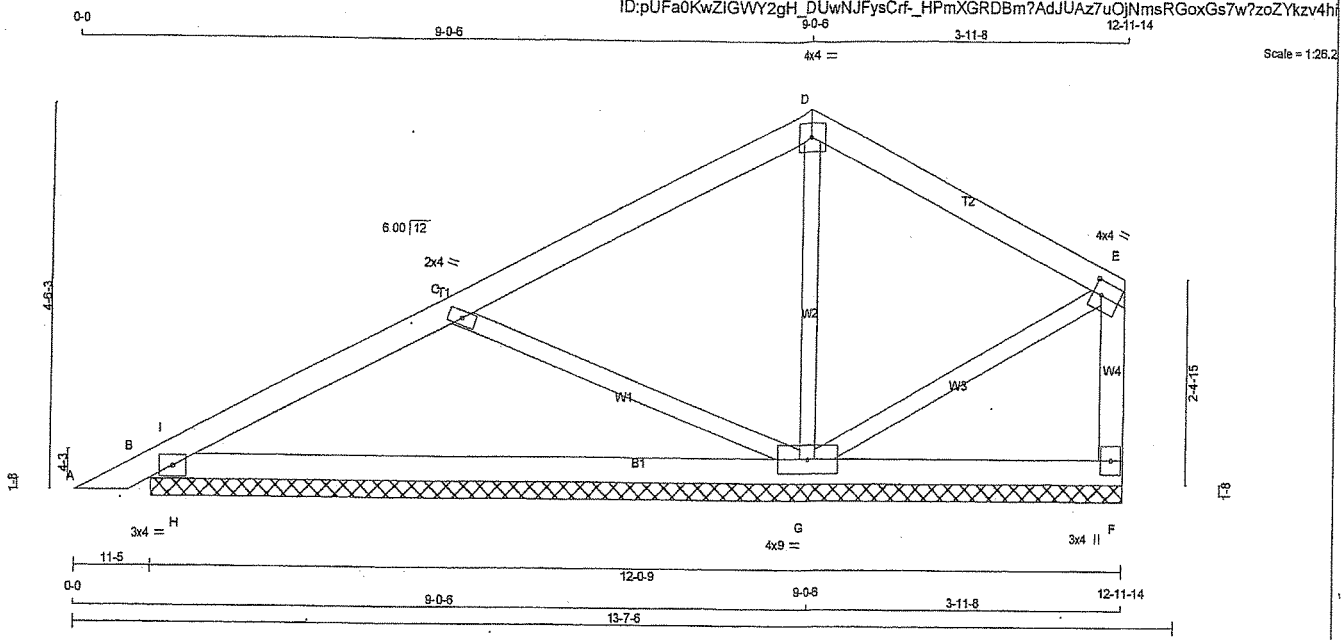


TOTAL WEIGHT = 43 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TMW+w	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	4.0		
E	TMVW-t	MT20	4.0	4.0	2.00	1.25
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW1-t	MT20	4.0	9.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	91	0	91	0	0	12-0-9	12-0-9
B	498	0	498	0	0	12-0-9	12-0-9
G	971	0	971	0	0	12-0-9	12-0-9

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	69	34 / 0	16 / 0	0 / 0	0 / 0	19 / 0	0 / 0
B	373	201 / 0	72 / 0	0 / 0	0 / 0	100 / 0	0 / 0
G	736	367 / 0	165 / 0	0 / 0	0 / 0	205 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, G

BRACING

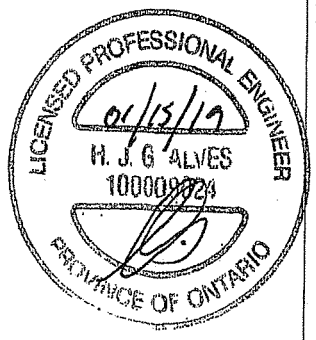
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 15	-84.9	-84.9	0.05 (1)	10.00	C- G	-557 / 0	0.22 (1)
B- I	-668 / 0	-84.9	-84.9	0.28 (3)	6.25	G- D	-410 / 0	0.12 (1)
I- C	-408 / 0	-84.9	-84.9	0.28 (3)	6.25	G- E	-116 / 0	0.04 (1)
C- D	0 / 95	-84.9	-84.9	0.29 (1)	10.00	H- I	0 / 548	0.00 (1)
D- E	0 / 115	-84.9	-84.9	0.24 (1)	10.00			
F- E	-62 / 0	0.0	0.0	0.01 (1)	7.81			
B- H	0 / 407	-38.5	-38.5	0.23 (3)	10.00			
H- G	0 / 407	-38.5	-38.5	0.37 (2)	10.00			
G- F	0 / 0	-38.5	-38.5	0.32 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.29/1.00 (C-D:1) , BC=0.37/1.00 (G-H:2) , WB=0.22/1.00 (C-G:1) , SSI=0.34/1.00 (B-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.43 (C) (INPUT = 0.90)
JSI METAL= 0.29 (C) (INPUT = 1.00)

TAM 1901222
STRUCTURAL
COMPONENT ONLY

JDB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300334	P12G	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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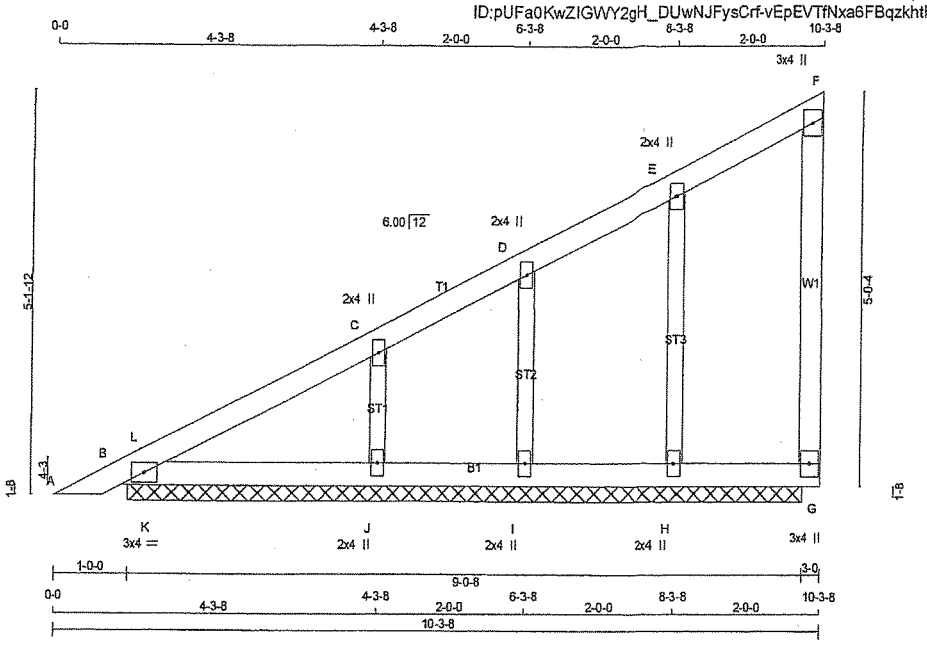


TOTAL WEIGHT = 2 X 35 = 70 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
B - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB1-I	MT20	3.0	4.0	
C, D, E					
C	TMW+w	MT20	2.0	4.0	
F	TMV+p	MT20	3.0	4.0	
G	BMV1+p	MT20	3.0	4.0	
H, I, J					
H	BMW1+w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 15	-84.9	-84.9	0.05 (1)	10.00	H- E	-199 / 0		0.05 (1)
B- L	-38 / 0	-84.9	-84.9	0.04 (3)	6.25	I- D	-130 / 0		0.02 (1)
L- C	-6 / 3	-84.9	-84.9	0.10 (1)	10.00	J- C	-258 / 0		0.04 (1)
C- D	-20 / 0	-84.9	-84.9	0.10 (1)	6.25	K- L	-85 / 53		0.00 (1)
D- E	0 / 0	-84.9	-84.9	0.05 (1)	10.00				
E- F	-8 / 0	-84.9	-84.9	0.05 (1)	10.00				
G- F	-70 / 0	0.0	0.0	0.03 (1)	7.81				
B- K	0 / 17	-38.5	-38.5	0.07 (1)	10.00				
K- J	0 / 17	-38.5	-38.5	0.08 (2)	10.00				
J- I	0 / 7	-38.5	-38.5	0.08 (2)	10.00				
I- H	0 / 4	-38.5	-38.5	0.03 (3)	10.00				
H- G	0 / 0	-38.5	-38.5	0.03 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.10/1.00 (C-D:1) , BC=0.08/1.00 (J-K:2) , WB=0.05/1.00 (E-H:1) , SSI=0.11/1.00 (C-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

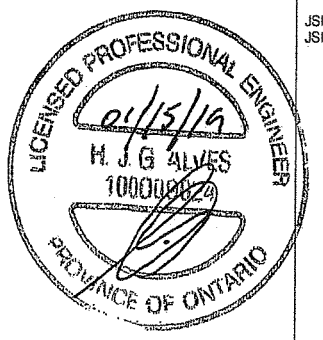
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.16 (B) (INPUT = 0.90)
JSI METAL= 0.11 (C) (INPUT = 1.00)



...O. TAM 7190123
STRUCTURAL
...SIGN ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300335	P13	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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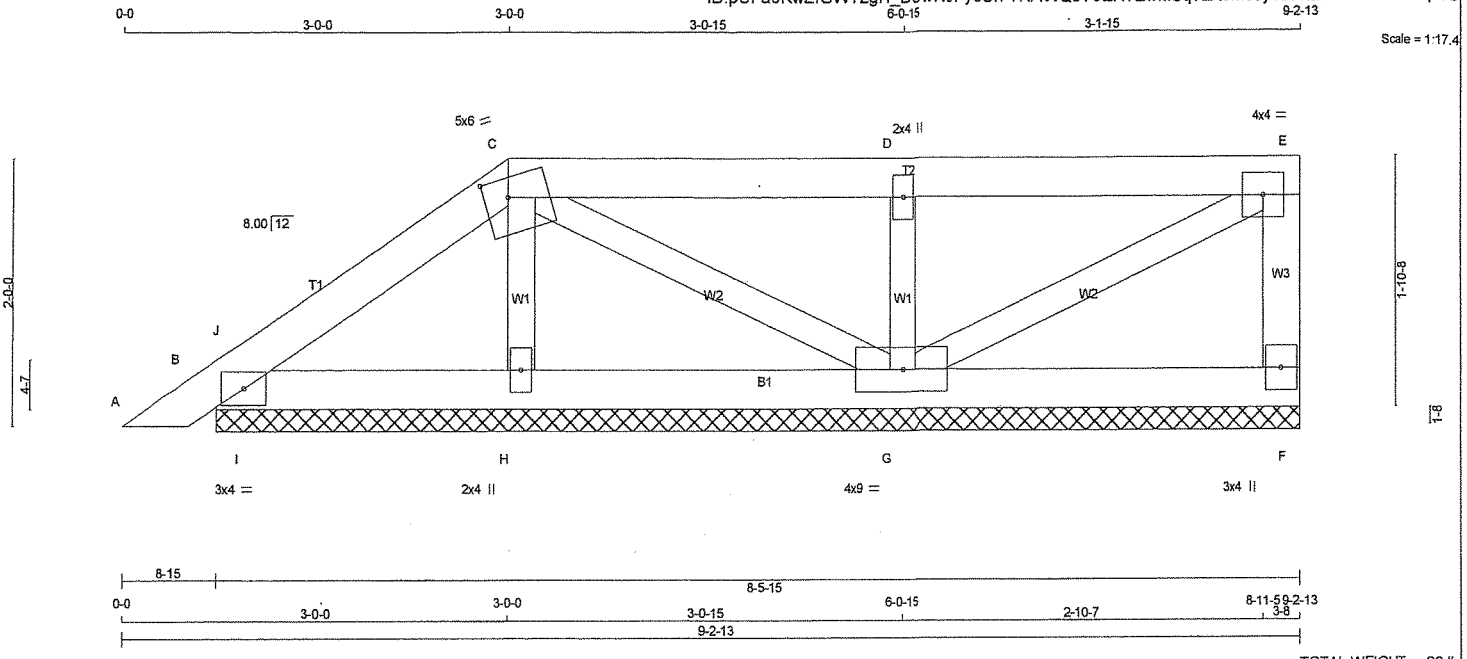


TOTAL WEIGHT = 29 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-t	MT20	3.0	4.0		
C	TTWW-m	MT20	5.0	6.0	1.75	2.25
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	4.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW1-t	MT20	4.0	9.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	149	0	149	0	0	8-5-15	8-5-15
B	211	0	211	0	0	8-5-15	8-5-15
H	263	0	263	0	0	8-5-15	8-5-15
G	480	0	480	0	0	8-5-15	8-5-15

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	114	55 / 0	27 / 0	0 / 0	0 / 0	32 / 0	0 / 0
B	154	95 / 0	21 / 0	0 / 0	0 / 0	38 / 0	0 / 0
H	204	88 / 0	56 / 0	0 / 0	0 / 0	60 / 0	0 / 0
G	361	188 / 0	75 / 0	0 / 0	0 / 0	98 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO		FR-TO			
A-B	0 / 14	-84.9 -84.9	0.03 (1)	10.00	H-C	-137 / 0	0.02 (1)
B-J	-45 / 0	-84.9 -84.9	0.01 (3)	6.25	G-E	-3 / 0	0.00 (1)
J-C	-47 / 0	-84.9 -84.9	0.05 (1)	6.25	C-G	-36 / 0	0.01 (1)
C-D	0 / 3	-84.9 -84.9	0.14 (1)	10.00	G-D	-330 / 0	0.05 (1)
D-E	0 / 3	-84.9 -84.9	0.14 (1)	10.00	I-J	-98 / 15	0.00 (1)
F-E	-100 / 0	0.0 0.0	0.01 (1)	7.81			
B-I	0 / 38	-38.5 -38.5	0.05 (1)	10.00			
I-H	0 / 38	-38.5 -38.5	0.05 (1)	10.00			
H-G	0 / 29	-38.5 -38.5	0.07 (3)	10.00			
G-F	0 / 0	-38.5 -38.5	0.07 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (C-D:1) , BC=0.07/1.00 (G-H:3) , WB=0.05/1.00 (D-G:1) , SSI=0.13/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (D) (INPUT = 0.90)
JSI METAL= 0.07 (D) (INPUT = 1.00)

DWG NO. TAM 1901254

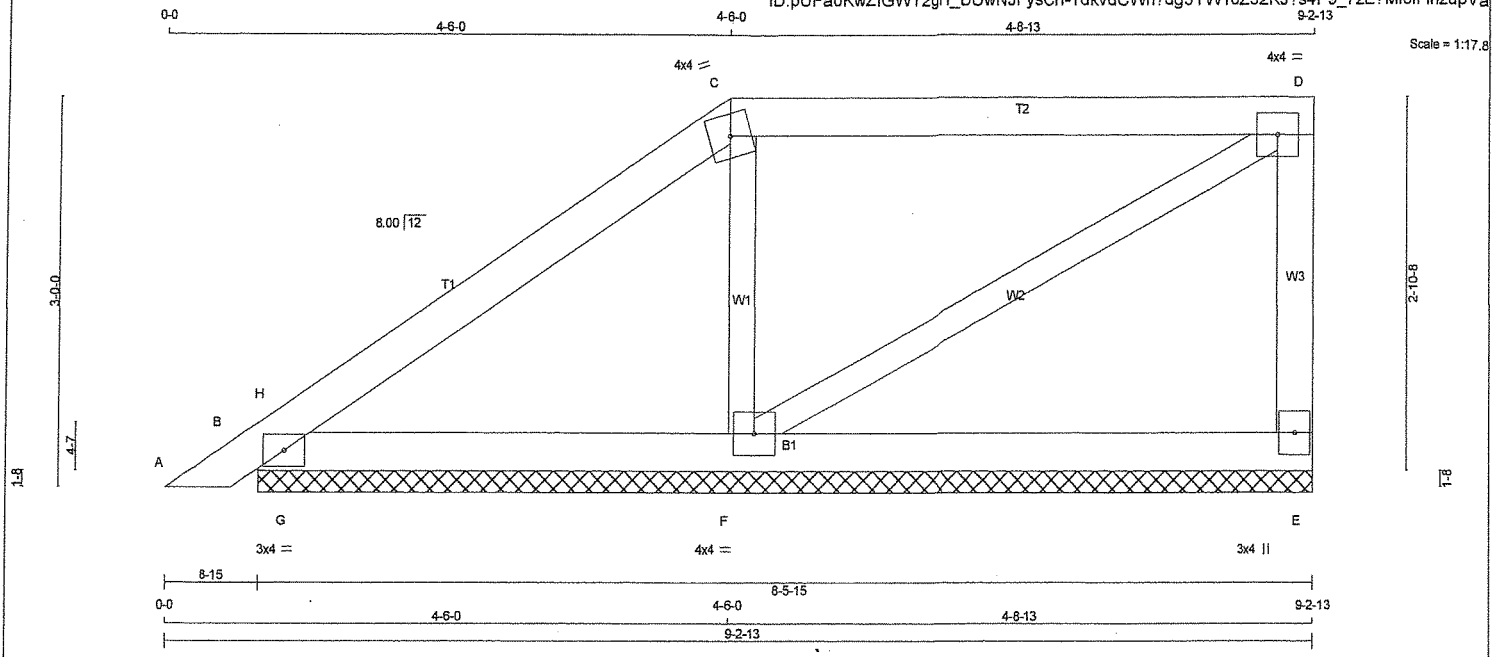
STRUCTURAL

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300335	P14	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Wed Jan 16 11:00:25 2019 Page 1

ID:pUFa0KwZIGWY2gH_DUwNJFysCrf-TdkvdCWh?dg5YW10Z32KJ?s4P9_7zE?MI3IFInzupVa



TOTAL WEIGHT = 4 X 30 = 119 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB1-I	MT20	3.0	4.0	
C	TTW-m	MT20	4.0	4.0	
D	TMVW-t	MT20	4.0	4.0	
E	BMV1+p	MT20	3.0	4.0	
F	BMWW1-t	MT20	4.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	290	0	290	0	0	8-5-15	8-5-15
B	300	0	300	0	0	8-5-15	8-5-15
F	514	0	514	0	0	8-5-15	8-5-15

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	216	119 / 0	40 / 0	0 / 0	0 / 0	57 / 0	0 / 0
B	219	134 / 0	30 / 0	0 / 0	0 / 0	55 / 0	0 / 0
F	398	173 / 0	108 / 0	0 / 0	0 / 0	117 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B, F

BRACING

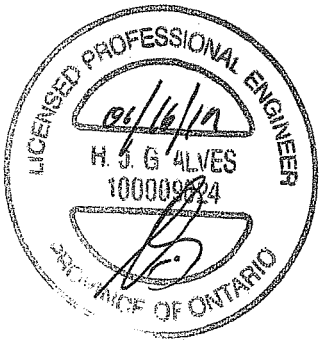
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 14	-84.9	-84.9	0.03 (1)	10.00	F- C	-301 / 0	0.05 (1)
B- H	-39 / 1	-84.9	-84.9	0.05 (1)	6.25	F- D	0 / 48	0.01 (1)
H- C	-71 / 0	-84.9	-84.9	0.15 (1)	6.25	G- H	-273 / 7	0.00 (1)
C- D	-42 / 0	-84.9	-84.9	0.32 (1)	6.25			
E- D	-224 / 0	0.0	0.0	0.03 (1)	7.81			
B- G	0 / 54	-38.5	-38.5	0.13 (1)	10.00			
G- F	0 / 54	-38.5	-38.5	0.18 (2)	10.00			
F- E	0 / 0	-38.5	-38.5	0.17 (2)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.32/1.00 (C-D:1) , BC=0.18/1.00 (F-G:2) , WB=0.05/1.00 (C-F:1) , SSI=0.23/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

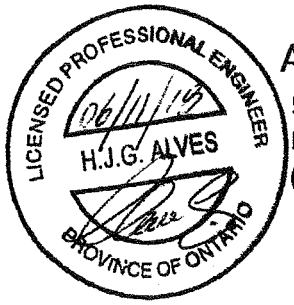
PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (B) (INPUT = 0.90)
JSI METAL= 0.06 (C) (INPUT = 1.00)

DWG NO. TAM 1190129S
STRUCTURAL



Alves Engineering Services Inc.
5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components

2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.

3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.

4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.

5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.

2- Lumber is to be the sizes and grade specified on the truss drawing.

3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.

4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings

5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.

6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)

7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.

8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1800213 Feb 09, 2018

ONTARIO WOOD TRUSS
FABRICATORS ASSOCIATION

TECH-NOTES

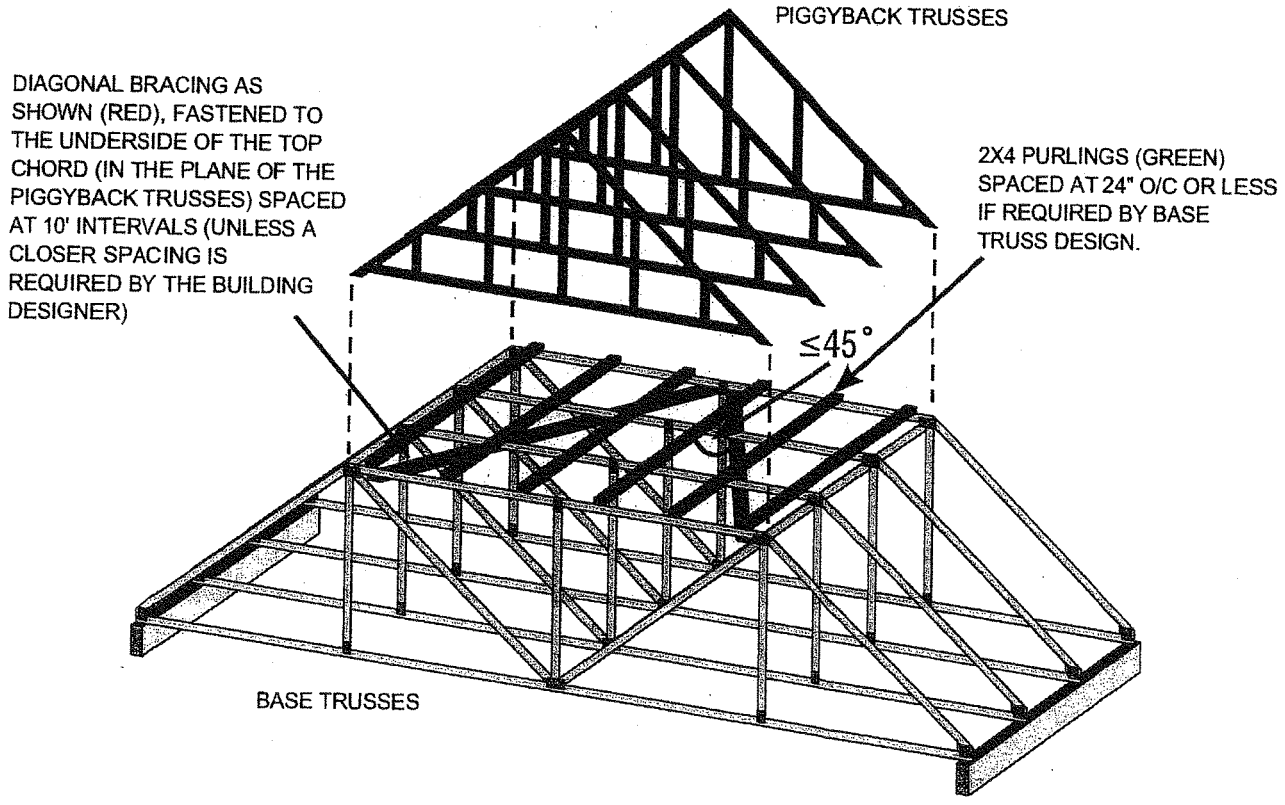
TN 15-001
Piggyback Bracing

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

TECHNICAL BULLETIN

LUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

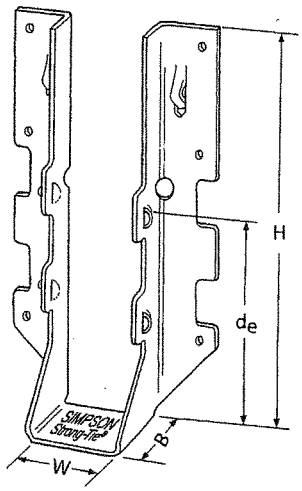
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

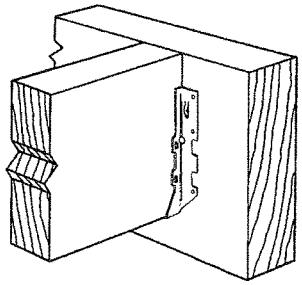
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified

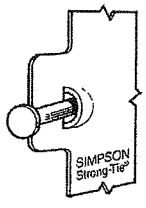


LUS28


Typical LUS Installation

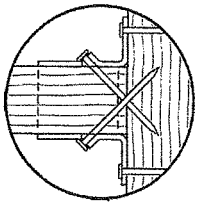
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LUS24	18	1 9/16	3 1/8	1 3/4	1 15/16	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 13/16	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1 9/16	4 3/4	1 3/4	3 5/8	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 7/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4 5/8	4 3/16	2	3 1/4	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 9/16	6 5/8	1 3/4	3 3/4	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4 5/8	6 1/4	2	3 1/4	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 9/16	7 13/16	1 3/4	3 7/8	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4 5/8	8 3/16	2	5 1/4	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580


Double Shear Nailing Top View.

LIMIT STATES DESIGN

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HUS/LJS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

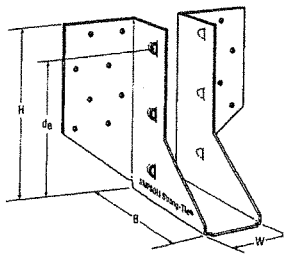
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

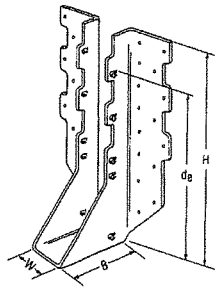
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

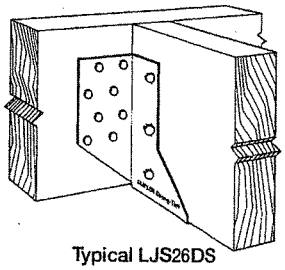
- See current catalogue for options



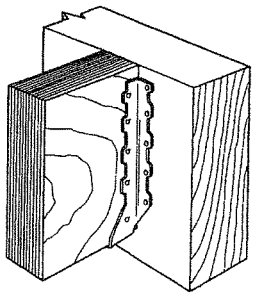
LJS26DS



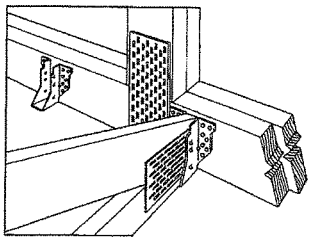
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS Installation



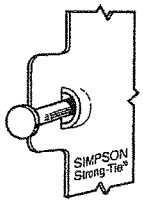
Typical HUS Installation



Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

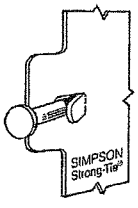
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
								lb.	lb.	lb.	lb.
LJS26DS	18	1 9/16	5	3½	4⅝	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1⅝	5⅜	3	3 15/16	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1⅝	7 3/32	3	6 3/32	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1⅝	9 3/32	3	7 31/32	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1 13/16	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

1. d_e is the distance from the seat of the hanger to the highest joist nail.

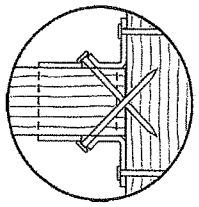


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

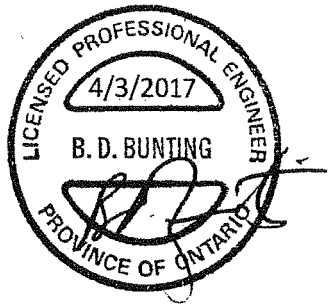
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2019, and reflects information available as of March 1, 2017. This information is updated periodically and should not be relied upon after June 30, 2019. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

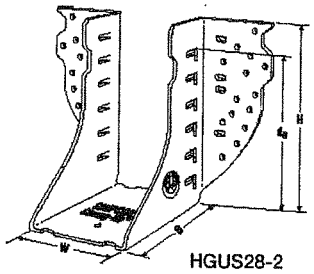
(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.



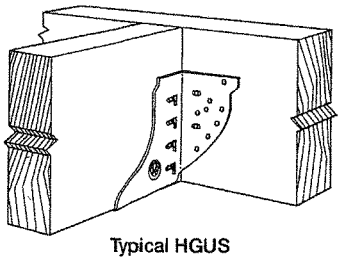
HGUS28-2

Material: 12 gauge

Finish: G90 galvanized

Design:

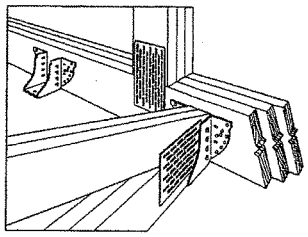
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.



Typical HGUS Installation

Installation:

- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications



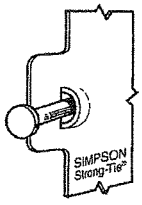
Typical HGUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)

Options:

- See current catalogue for options

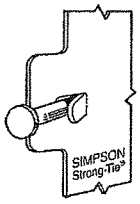
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
HGUS26	12	1⅝	5⅜	5	4 5/32	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3 5/16	5 7/16	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4 15/16	5½	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6 5/16	5 7/16	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1⅝	7⅛	5	6⅛	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3 5/16	7 5/16	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4 15/16	7¼	4	6⅜	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6 5/16	7 5/16	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210-2	12	3 5/16	9 5/16	4	8⅛	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-3	12	4 15/16	9¼	4	8⅜	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6 5/16	9 5/16	4	8⅛	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6 5/16	10⅝	4	10⅛	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6 5/16	12⅝	4	11⅛	(66) 16d	(22) 16d	10130	16400	7195	11645

1. d_e is the distance from the seat of the hanger to the highest joist nail.

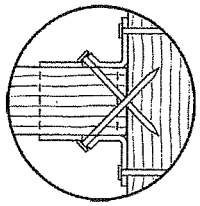


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

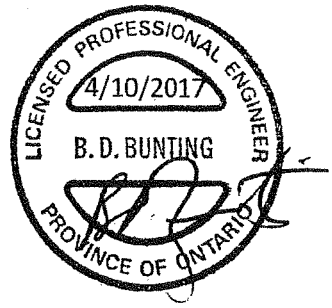
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2019, and reflects information available as of March 1, 2017. This information is updated periodically and should not be relied upon after June 30, 2019. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

T-SPECHGUS17 3/17 exp. 6/19

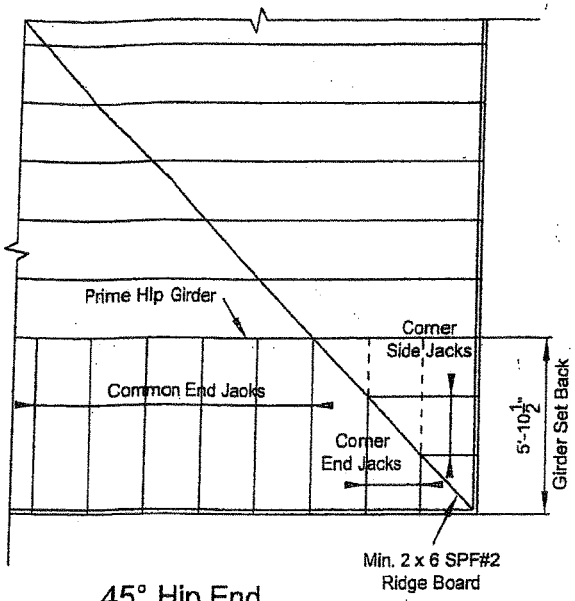
(800) 999-5099
strongtie.com

LUMBER SPECIFICATION

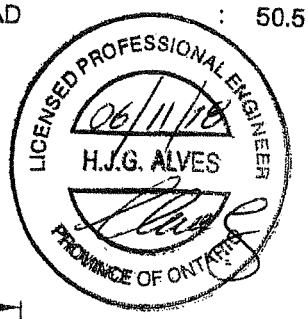
TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

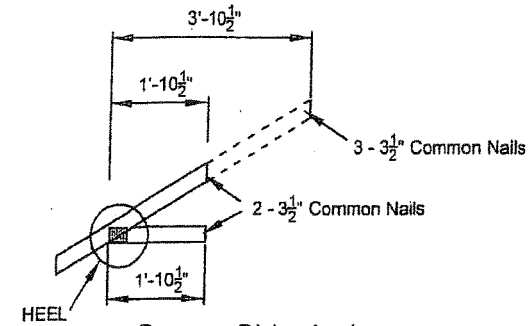
DESIGN LOAD

TOP CHORD SNOW LOAD	:	40.5 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD	:	50.5 P.S.F

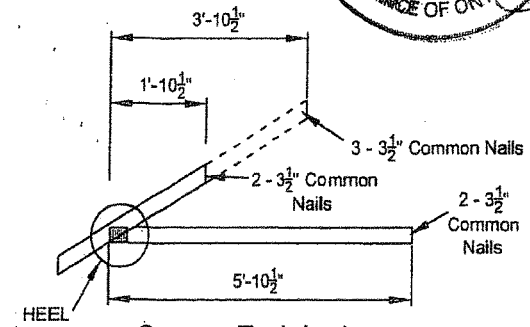


45° Hip End

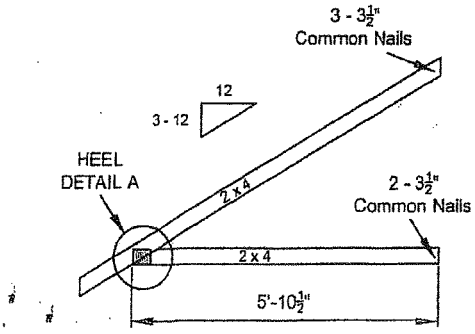




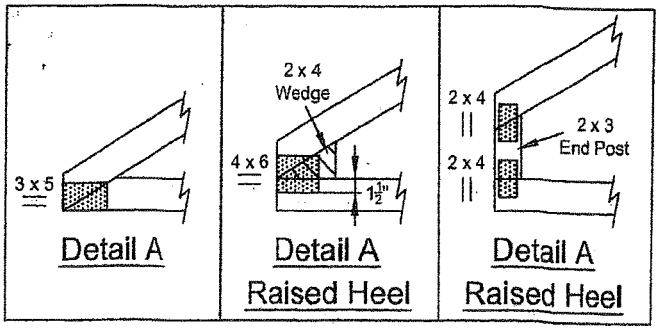
Corner Side Jacks



Corner End Jacks

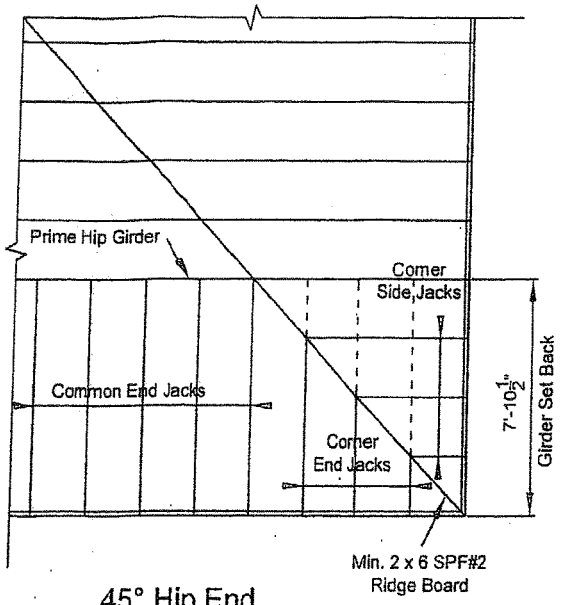


Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800216



45° Hip End

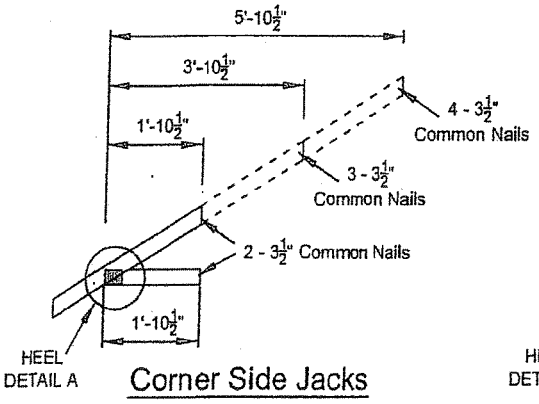
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

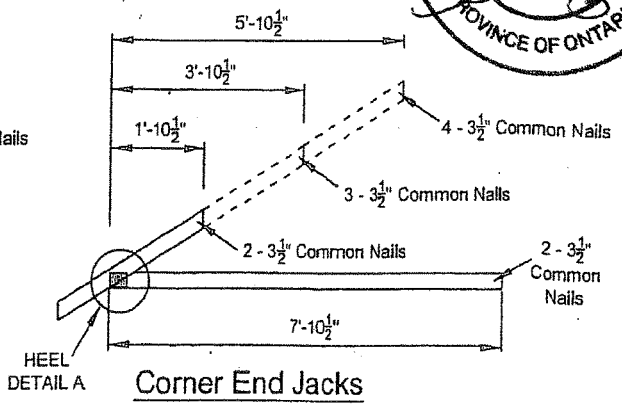
DESIGN LOAD

TOP CHORD SNOW LOAD	:	40.5 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD		50.5 P.S.F

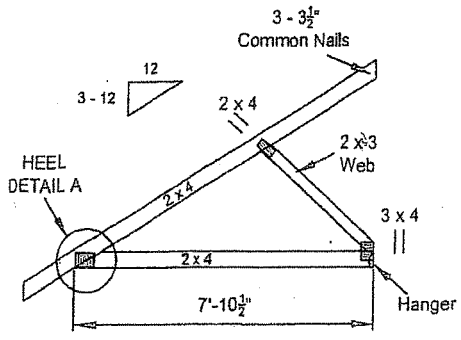
LICENSED PROFESSIONAL ENGINEER
06/11/2018
H.J.G. ALVES
PROVINCE OF ONTARIO



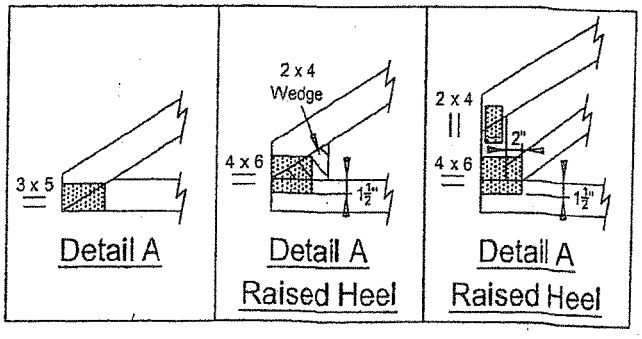
Corner Side Jacks



Corner End Jacks



Common End Jacks

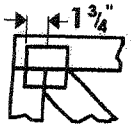


NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

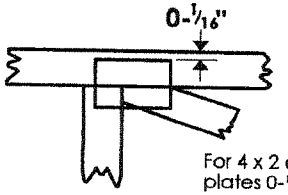
T-1800217

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

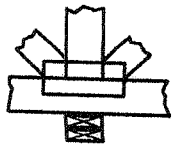
4 X 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

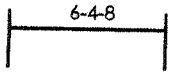


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

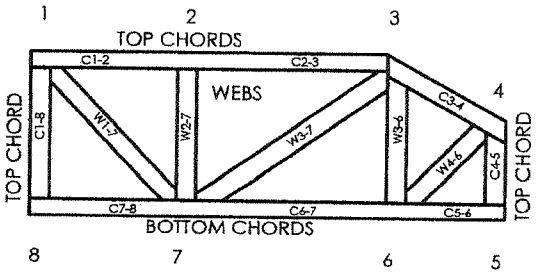
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

MiTek®

POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MII-7473C rev. 10-'08

⚠ General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.