

ASPHALT SHINGLES
12" FINISH O.H
R.T.M.C.
2x6 EXTERIOR WALLS
2X6 FASCIA BOARD

UNIT1804 END
A -LOT1
402597

UNIT1801
A (REV)-LOT2
402598

UNIT1802
A -LOT3
402598

UNIT1804 END
A(REV) -LOT4
402597

HARDWARE:

LUS24 -(O)
LJS26DS -(V)

DENOTE:

C- 10/12 CATH. CEILING

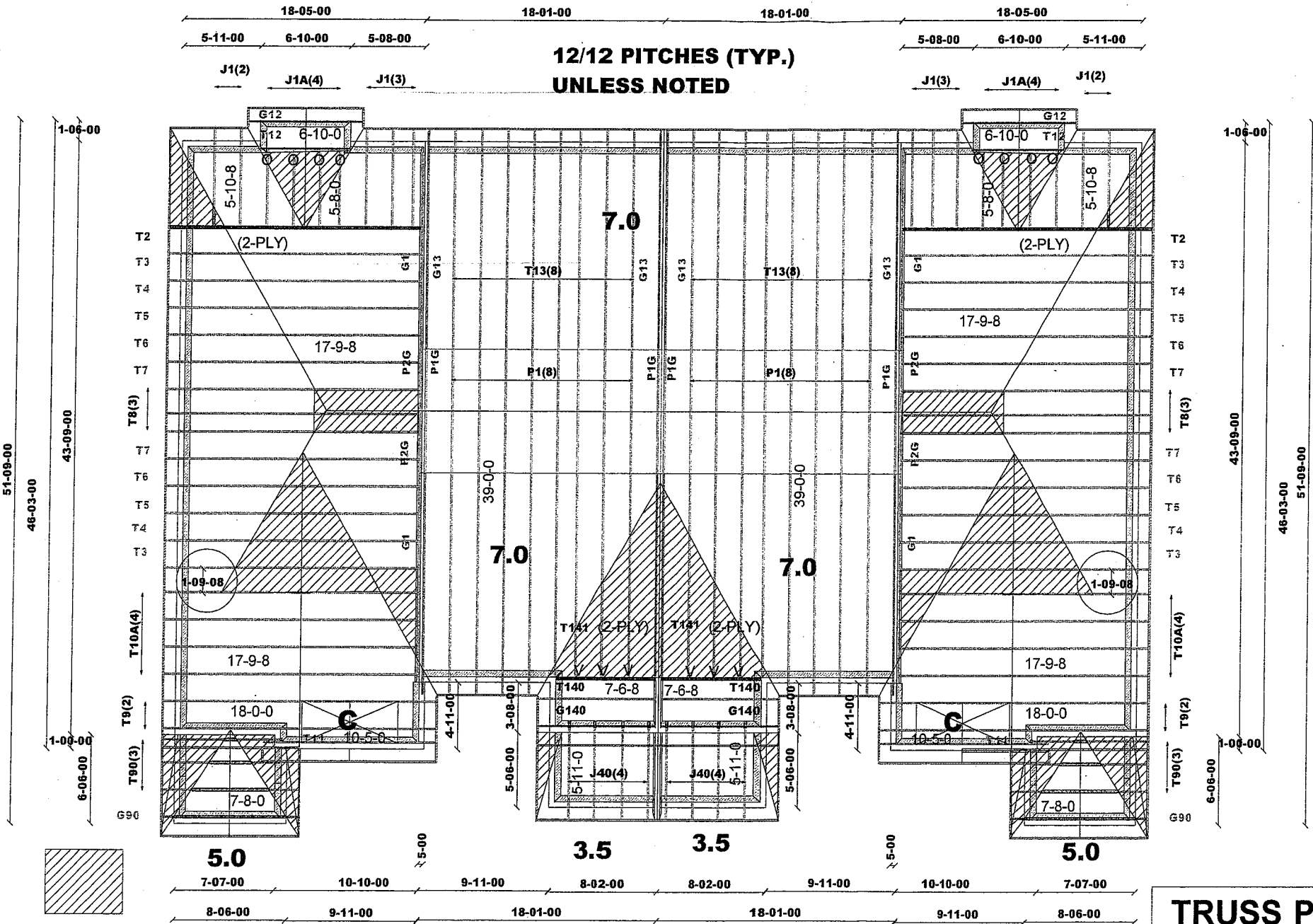
T-180485

DENOTES:
CONVENTIONAL
FRAMING

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"x4"SPF@24"o.c.
WITH A 2"x4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.
POSTS LONGER THAN 6' TO BE Laterally BRACED SO THAT THE DISTANCE
BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

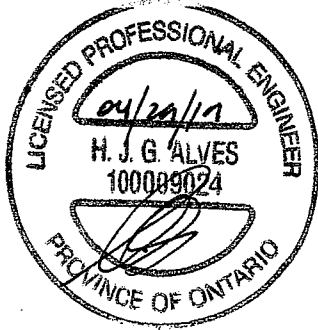
DESIGN LOADS:

SNOW LOAD 23.3 PSF
TC DEAD 15 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF



Lot # 1 or lot 4, DWG# , T-1900034,
T-1900035, T-1900039, T-1900040,
T-1900042, T-1900044 to T-
1900047, T-1900049, T-1900051, T-
1900052, T-1900056, T-1900082, T-
1909771, T-1909772, T-1800216 ,
T1800218

Lot 2 or 3 DWG#, T-1900060, T-
1900063, T-1900064, T-1900066, T-
1901090 to T-1901092, T-1800216,
T-1800218



DWG NO. TAM T1909778
STRUCTURAL
COMPONENT ONLY

TRUSS PLACEMENT PLAN.

This is a truss placement plan only, NOT a final roof framing plan.
These trusses are designed as individual building components to
be incorporated into the building design at the specification of the
building designer. See individual truss drawings for each
component identified on this placement plan.

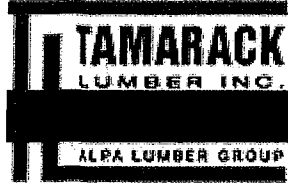
Please verify that all dimensions match the dimensions found on
the job. The Building designer is responsible for the temporary/
permanent bracing of the roof and floor system and its integration
into the bracing of the overall structure. The design of the truss
support structure including headers, beams, walls, and columns is
the responsibility of the building designer unless otherwise noted
in this plan. Building designer to review and approve this plan to
ascertain conformity to his overall structural plan.



Job Track: 50530
Layout ID: 200911
Plan Log: 402596

Builder / Location:
ROYAL PHIME HOMES / BRAMPTON
Project: FORESTSIDE PH.6
Date: 4/22/2019 Designer: S.V

Model / Elevation: BLOCK 8 / LOTS 1 TO 4 (REVISED)
Mitek ver 8.2.3.229
THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE
REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER
THAN THE MANUFACTURE OF TRUSSES BY
TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED
FOR ANY OTHER PURPOSE.

	DELIVERY SHIPLIST			
	Lumber Yard: TAMARACK LUMBER		Job Track: 50530	
	Builder: ROYAL PINE HOMES		PlanLog: 200911	
	Project: FORESTSIDE PH. 6		Layout ID: 402597	
	Location: BRAMPTON		Ref #	
	Model: UNIT 1804		Page: 1 of 2	
	Lot #:		Date: 04/22/2019	
Elevation: EL:A LOT 1 OR LOT 4		Designer:		
		Sales Rep: Mario DiCano		

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	G1 GABLE	7 /12	19-06-00	10-00-00	2 x 4		1-03-07 10-00-00	221.47 140.00		
	1 2-ply	T2 Half Hip Girder	12 /12	17-09-08	4-08-09	2 x 4 2 x 6	1-03-08	1-10-08 4-08-09	199 125.33		
	2	T3 Half Hip	12 /12	17-09-08	5-10-09	2 x 4	1-03-08	1-10-08 5-10-09	156.36 98.00		
	2	T4 Half Hip	12 /12	17-09-08	7-00-09	2 x 4	1-03-08	1-10-08 7-00-09	166.94 104.67		
	2	T5 Half Hip	12 /12	17-09-08	8-02-09	2 x 4	1-03-08	1-10-08 8-02-09	185.34 117.33		
	2	T6 Half Hip	12 /12	17-09-08	9-04-09	2 x 4	1-03-08	1-10-08 9-04-09	210.83 132.00		
	2	T7 Half Hip	12 /12	17-09-08	10-06-09	2 x 4	1-03-08	1-10-08 10-06-09	208.65 127.67		
	3	T8 Half Hip	12 /12	17-09-08	11-08-09	2 x 4	1-03-08	1-10-08 11-08-09	350.76 216.00		
	2	T9 Roof Special	12 /12 10 /12	18-00-00	10-10-08	2 x 4	1-03-08 1-03-08	1-10-08 1-10-08	204.82 129.33		
	4	T10A Common	12 /12	17-09-08	10-10-08	2 x 4	1-03-08	1-10-08 2-01-00	381.53 238.67		
	1	T11 Roof Special	12 /12 10 /12	10-05-00	7-01-00	2 x 4	1-03-08 1-03-08	1-10-08 1-10-08	55.81 36.33		
	1	T12 Common Girder	12 /12	6-10-00	4-10-08	2 x 4	10-08 10-08	1-05-08 1-05-08	33.29 23.17		
	1	G12 GABLE	12 /12	6-10-00	4-10-08	2 x 4	10-08 10-08	1-05-08 1-05-08	34.17 24.17		
	3	T90 Common	5 /12	7-08-00	2-05-07	2 x 4	1-03-08 1-03-08	6-00 6-00	73.47 47.00		

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: FORESTSIDE PH. 6
 Location: BRAMPTON
 Model: UNIT 1801 OR UNIT 1802
 Lot #:
 Elevation: EL:A LOT2 OR LOT3

Job Track: 50530
 PlanLog: 200911
 Layout ID: 402598
 Ref #
 Page: 1 of 1
 Date: 04/22/2019
 Designer:
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	8	T13 Piggyback Base	7 /12	39-00-00	10-00-00	2 x 4 2 x 6	1-03-08 1-03-08	1-03-07 1-03-07	1640.08 1016.00		
	2	G13 GABLE	7 /12	39-00-00	10-00-00	2 x 4 2 x 6	1-03-08 1-03-08	1-03-07 1-03-07	453.81 284.67		
	1	T140 Monopitch	12 /12	7-06-08	9-05-00	2 x 4	1-03-08	1-10-08 9-05-00	46.85 29.83		
	1 2-ply	T141 Monopitch Girder	12 /12	7-06-08	9-05-00	2 x 4 2 x 6		1-10-08 9-05-00	113.13 67.67		
	1	G140 GABLE	12 /12	7-06-08	9-05-00	2 x 4	1-03-08	1-10-08 9-05-00	48.07 29.83		
	8	P1 Piggyback	7 /12	9-01-08	2-07-15	2 x 4			176.91 114.67		
	2	P1G GABLE	7 /12	9-01-08	2-07-15	2 x 4			46.84 30.67		
	4	J40 Monopitch	3.5 /12	5-11-00	2-04-14	2 x 4	1-03-08	3-14 2-00-09	61.62 37.33		

TOTAL # TRUSS= 28

TOTAL BFT OF ALL TRUSSES= 1610.67

BFT.

TOTAL WEIGHT OF ALL TRSSES 2587.31 LBS

HARDWARE

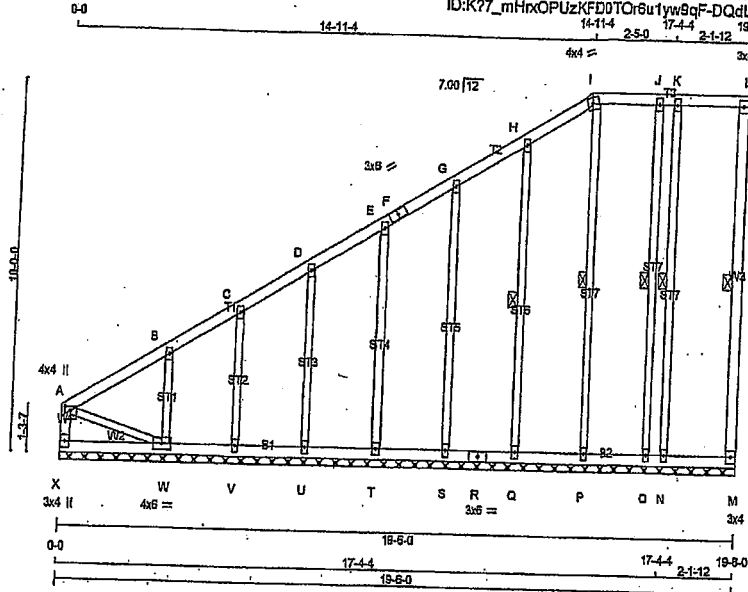
QTY	TYPE	MODEL	LENGTH
3	Hardware	LJS26DS	

TOTAL NUMBER OF ITEMS= 3

JOB NAME 300536	TRUSS NAME G1	QUANTITY 1	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
--------------------	------------------	---------------	----------	--------------------------	----------

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Thu Jan 3 11:24:25 2019 Page 1
ID:K77_mHxOPUzKF00TO6u1yW8qF-DQdLeXz03qnt2xqVQxS7VyKeuxMjGa6zLQewizz5N4



Scale = 1:58.7

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	
CHORDS			
A - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
I - L	2x4	DRY	No.2
M - L	2x4	DRY	No.2
X - A	2x4	DRY	No.2
X - R	2x4	DRY	No.2
R - M	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

GABLE STUDS SPACED AT 2-0-0 C.C.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW+p	MT20	4.0	4.0	1.25 2.00
B, C, D, E, G, H, J, K					
B	TMVW+w	MT20	2.0	4.0	
F	TS+	MT20	3.0	6.0	
I	TTW-m	MT20	4.0	4.0	
L	TMV+p	MT20	3.0	4.0	
M	BMV1+p	MT20	3.0	4.0	
N, O, P, Q, S, T, U, V					
N	BMV1+w	MT20	2.0	4.0	
R	BS+	MT20	3.0	6.0	
W	BMVW1+t	MT20	4.0	6.0	
X	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF L-M, K-N, H-Q, I-P, J-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	-10/0	-84.9 -84.9 0.09 (1)	6.25	N-K	-121/0	0.08 (1)	
B-C	-28/0	-84.9 -84.9 0.09 (1)	6.25	W-B	-254/0	0.05 (1)	
C-D	-8/0	-84.9 -84.9 0.04 (1)	10.00	V-C	-138/0	0.04 (1)	
D-E	-8/0	-84.9 -84.9 0.04 (1)	10.00	U-D	-174/0	0.08 (1)	
E-F	-6/0	-84.9 -84.9 0.04 (1)	10.00	T-E	-168/0	0.12 (1)	
F-G	-6/0	-84.9 -84.9 0.04 (1)	10.00	S-G	-168/0	0.18 (1)	
G-H	-2/0	-84.9 -84.9 0.04 (1)	10.00	Q-H	-187/0	0.10 (1)	
H-I	-10/0	-84.9 -84.9 0.04 (1)	6.25	P-I	-135/0	0.09 (1)	
I-J	0/0	-84.9 -84.9 0.04 (1)	10.00	O-J	-125/0	0.08 (1)	
J-K	0/0	-84.9 -84.9 0.04 (1)	10.00	A-W	0/20	0.00 (1)	
K-L	0/0	-84.9 -84.9 0.04 (1)	10.00				
M-L	-72/0	0.0 0.0 0.03 (1)	6.25				
X-A	-120/0	0.0 0.0 0.01 (1)	7.81				
X-W	0/0	-38.5 -38.5 0.06 (3)	10.00				
W-V	0/12	-38.5 -38.5 0.06 (3)	10.00				
V-U	0/9	-38.5 -38.5 0.03 (3)	10.00				
U-T	0/7	-38.5 -38.5 0.03 (3)	10.00				
T-S	0/5	-38.5 -38.5 0.02 (3)	10.00				
S-R	0/3	-38.5 -38.5 0.02 (3)	10.00				
R-Q	0/3	-38.5 -38.5 0.02 (3)	10.00				
Q-P	0/1	-38.5 -38.5 0.02 (3)	10.00				
P-O	0/0	-38.5 -38.5 0.02 (3)	10.00				
O-N	0/0	-38.5 -38.5 0.03 (3)	10.00				
N-M	0/0	-38.5 -38.5 0.03 (3)	10.00				

TOTAL WEIGHT = 111 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN PLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(65 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.09/1.00 (B-C:1), BC=0.08/1.00 (V-W:3)
WB=0.18/1.00 (G-S:1), SSI=0.10/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

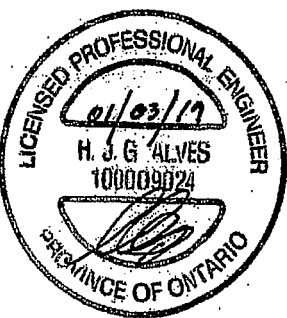
NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	618	354	1687 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.60 (I) (INPUT = 0.60)
JSI METAL= 0.12 (B) (INPUT = 1.00)

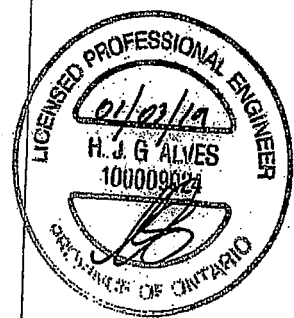
DWG NO. TAM 1900034
STRUCTURAL
COMPONENT ONLY



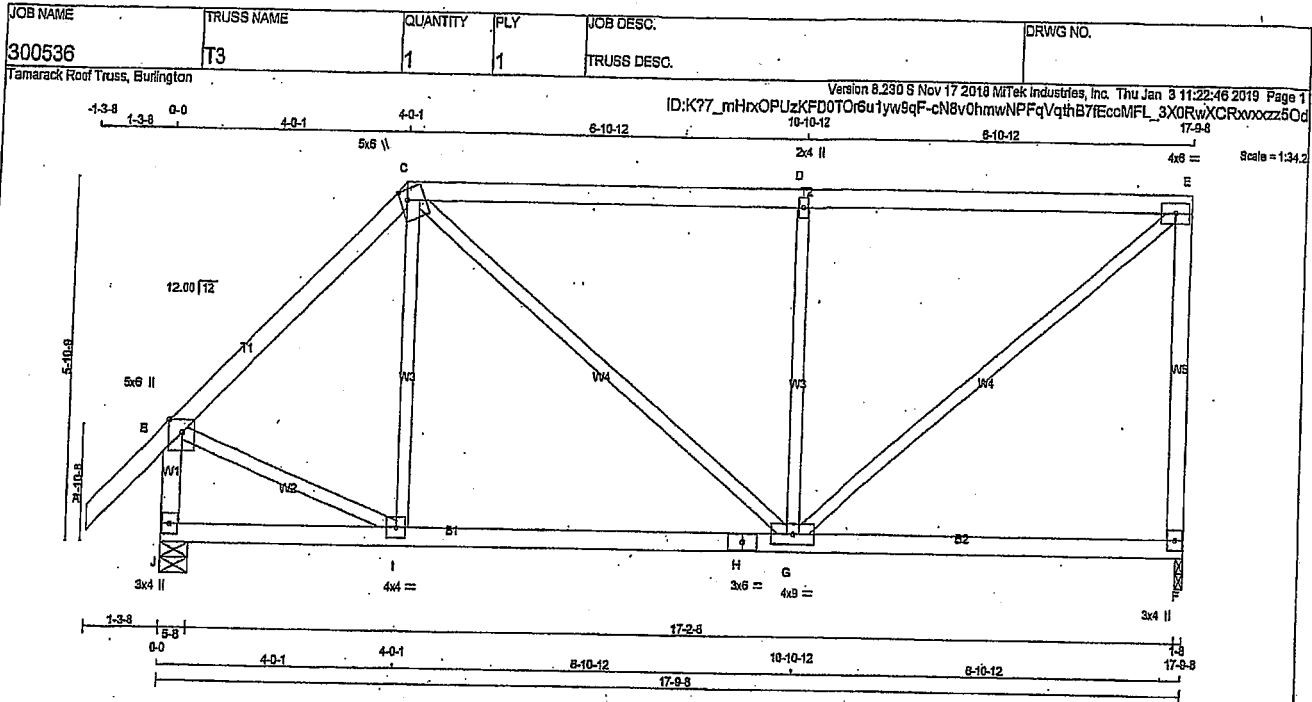
JOB NAME 300536 Tamarack Roof Truss, Burlington	TRUSS NAME T2	QUANTITY 1	PLY 2	JOB DESC. TRUSS DESC.	DRWG NO.
---	------------------	---------------	----------	--------------------------	----------

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Thu Jan 3 11:22:45 2019 Page 2
 ID:K77 mHrxOPuzKFD0TOr6u1yw9qF-8BaXpLlc67ztlVeQ873PpAban?H17O nCLPVzz8Oe

FACTORED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
R	13-1-4	-164	-164	---	BACK	VERT	TOTAL	---	---
S	15-1-4	-164	-164	---	BACK	VERT	TOTAL	---	---
T	17-1-4	-178	-178	---	BACK	VERT	TOTAL	---	---
U	1-11-4	-75	-95	---	BACK	VERT	TOTAL	---	---
V	4-1-4	-75	-95	---	BACK	VERT	TOTAL	---	---
W	8-1-4	-71	-90	---	BACK	VERT	TOTAL	---	---
X	7-1-4	-71	-90	---	BACK	VERT	TOTAL	---	---
Y	11-1-4	-71	-90	---	BACK	VERT	TOTAL	---	---
Z	13-1-4	-75	-95	---	BACK	VERT	TOTAL	---	---
AA	15-1-4	-75	-95	---	BACK	VERT	TOTAL	---	---
AB	17-1-4	-81	-103	---	BACK	VERT	TOTAL	---	---



DWG NO. TAM 71900039
 STRUCTURAL
 COMPONENT ONLY 2/2



TOTAL WEIGHT = 78 lb

LUMBER

N.L.G.A. RULE	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
F - H	2x4	DRY	No.2
J - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
H - F	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 EXCEPT

DRY: SEASONED LUMBER:

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMVW+m	MT20	2.0	4.0		
E	TMVW+t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMVWW+t	MT20	4.0	9.0		
H	BS+t	MT20	3.0	6.0		
I	BMVW+t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	UP	IN-SX
F	1097	0	0	1-8
J	1216	0	0	5-8

UNFACTORED REACTIONS

JT	1ST LOSE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	632	414/0	187/0	0/0	0/0	0/0	231/0	0/0
J	914	479/0	187/0	0/0	0/0	0/0	248/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.02 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0/42	-84.9 -84.9	0.12 (1)	10.00	I-C	-13/154	0.04 (3)
B-C	-800/0	-84.9 -84.9	0.28 (1)	8.16	C-G	0/410	0.09 (1)
C-D	-952/0	-84.9 -84.9	0.75 (1)	5.02	G-D	-723/0	0.39 (1)
D-E	-952/0	-84.9 -84.9	0.75 (1)	5.02	G-E	0/1223	0.28 (1)
E-F	-991/0	0.0 0.0	0.84 (1)	7.81	B-I	0/676	0.15 (1)
J-B	-1162/0	0.0 0.0	0.13 (1)	7.39			
J-I	0/0	-38.5 -38.5	0.19 (3)	10.00			
I-H	0/834	-38.5 -38.5	0.42 (2)	10.00			
H-G	0/834	-38.5 -38.5	0.42 (2)	10.00			
G-F	0/0	-38.5 -38.5	0.36 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3 PSF
DL = 6.0 PSF	
BOT CH.	LL = 10.5 PSF
DL = 7.0 PSF	
TOTAL LOAD	= 46.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2010, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.59")
CALCULATED VERT. DEFL. (LL) = L/899 (0.09")
ALLOWABLE DEFL. (TL) = L/360 (0.59")
CALCULATED VERT. DEFL. (TL) = L/899 (0.14")

CS: TC=0.75/1.00 (D-E-1), BC=0.42/1.00 (G-I-2),
WB=0.39/1.00 (D-G-1), SS=0.28/1.00 (D-E-1)
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

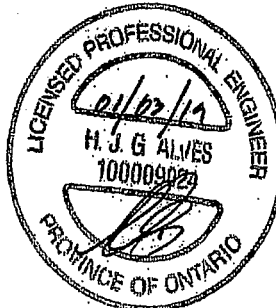
NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PL)
MT20	618 354 1657	788 1597 1658

PLATE PLACEMENT TOL. = 0.250 inches

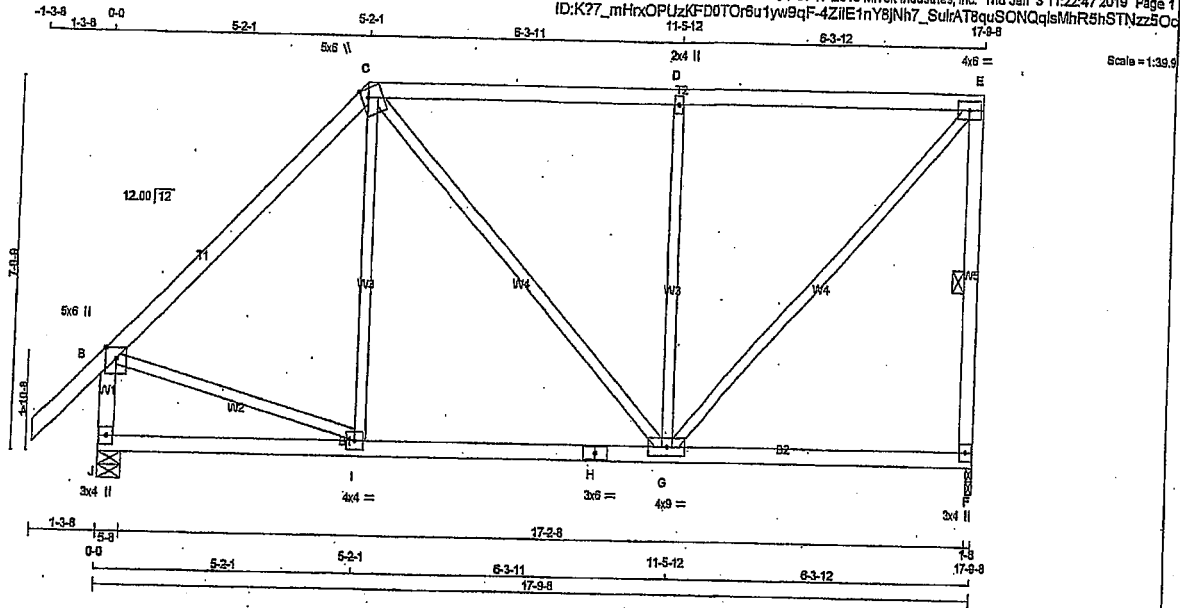
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.79 (I) (INPUT = 0.80)
JSI METAL = 0.44 (I) (INPUT = 1.00)



DRWG NO. TAM 71900040
STRUCTURAL
COMPONENT ONLY

JOB NAME 300536	TRUSS NAME T4	QUANTITY 1	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington					



TOTAL WEIGHT = 83 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
J - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
H - F	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY, SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMVW+m	MT20	5.0	6.0	2.00	1.50
D	TMVW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMVWV-t	MT20	4.0	6.0		
H	BS-t	MT20	3.0	6.0		
I	BMVWV-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
F	1097	0	1097	0	0
J	1216	0	1216	0	0

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	832	414 / 0	187 / 0	0 / 0	0 / 0	231 / 0	0 / 0
J	914	479 / 0	187 / 0	0 / 0	0 / 0	248 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 42	-84.9	-84.9 0.12 (1)	10.00	I-C	0 / 204	0.05 (3)
B-C	-875 / 0	-84.9	-84.9 0.44 (1)	5.94	C-G	0 / 189	0.04 (1)
C-D	-748 / 0	-84.9	-84.9 0.80 (1)	5.88	G-D	-592 / 0	0.57 (1)
D-E	-746 / 0	-84.9	-84.9 0.80 (1)	5.88	G-E	0 / 1090	0.25 (1)
E-F	-989 / 0	0.0	0.0 0.22 (1)	6.25	B-I	0 / 842	0.14 (1)
J-B	-1136 / 0	0.0	0.0 0.13 (1)	7.45			
J-I	0 / 0	-38.5	-38.5 0.21 (3)	10.00			
I-H	0 / 618	-38.5	-38.5 0.38 (2)	10.00			
H-G	0 / 618	-38.5	-38.5 0.36 (2)	10.00			
G-F	0 / 0	-38.5	-38.5 0.29 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 23.3 PSF
DL	= 6.0 PSF
BOT CH. LL	= 10.5 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 46.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2010, CBC 2012
- CSA 086-09, CSA 086-14
- TFC 2011, TFC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL)	= L/360 (0.59")
CALCULATED VERT. DEFL. (LL)	= L/999 (0.08")
ALLOWABLE DEFL. (TL)	= L/360 (0.59")
CALCULATED VERT. DEFL. (TL)	= L/999 (0.11")

CS1: TO=0.60/1.00 (D-E-1), BC=0.36/1.00 (G-1-2), WB=0.57/1.00 (D-G-1), SS=0.26/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

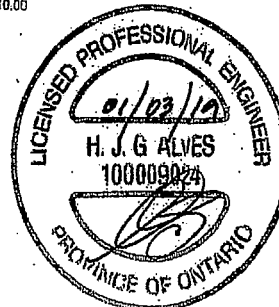
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1657
	788	1987	1656

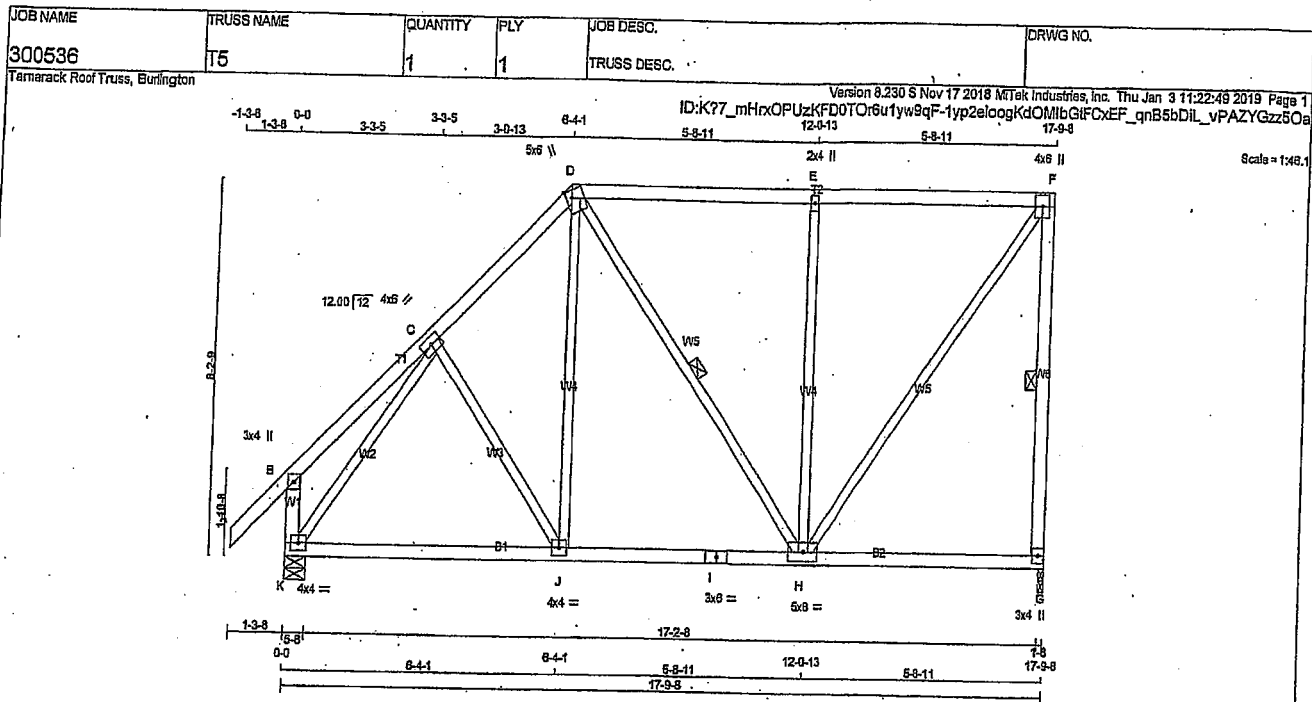
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.61 (G) (INPUT = 0.90)
JSI METAL= 0.33 (B) (INPUT = 1.00)



DRWG NO. TAM 71900042
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES	SIZE	DRY	No.2
A - D	2x4	DRY	No.2
B - E	2x4	DRY	No.2
C - F	2x4	DRY	No.2
D - G	2x4	DRY	No.2
E - H	2x4	DRY	No.2
F - I	2x4	DRY	No.2
G - J	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY, SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVAp	MT20	3.0	4.0		
B	TMVWt	MT20	4.0	6.0		
C	TMVWt	MT20	5.0	6.0	2.00	1.50
D	TMVWt	MT20	2.0	4.0		
E	TMVWt	MT20	4.0	6.0		
F	TMVWt	MT20	3.0	4.0		
G	BMVWt	MT20	5.0	8.0		
H	BMVWt	MT20	3.0	6.0		
I	BS+	MT20	4.0	4.0		
J	BMVWt	MT20	4.0	4.0		
K	BMVWt	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER.

BEARINGS

FACTORED	MAXIMUM FACTORED	INPUT	RECORD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	UPLIFT
G	1097	0	1-8
K	1216	0	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN	COMPONENT REACTIONS	DEAD	SOIL
JT	COMBINED	SNOW	LIVE	PERM. LIVE
G	832	4147/0	187/0	0/0
K	914	479/0	187/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. FURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO				FR-TO			
A-B	0/42	-84.9	-84.9 0.12 (1)	C-J	-29/57	0.02 (1)	
B-C	0/21	-84.9	-84.9 0.13 (1)	J-D	0/322	0.07 (2)	
C-D	-849/0	-84.9	-84.9 0.14 (1)	D-H	-24/16	0.02 (3)	
D-E	-588/0	-84.9	-84.9 0.48 (1)	H-E	-600/0	0.79 (1)	
E-F	-588/0	-84.9	-84.9 0.48 (1)	H-F	0/1013	0.23 (1)	
G-F	-1008/0	0.0	0.0 0.31 (1)	K-C	-1076/0	0.58 (1)	
K-B	-221/0	0.0	0.0 0.02 (1)				
K-J	0/600	-38.5	-38.5 0.34 (2)				
J-I	0/587	-38.5	-38.5 0.34 (2)				
I-H	0/587	-38.5	-38.5 0.34 (2)				
H-G	0/0	-38.5	-38.5 0.21 (3)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/999 (0.12")

CSI: TC=0.48/1.00 (E-F-1), BC=0.34/1.00 (H-J-2), WB=0.79/1.00 (E-H-1), SSF=0.24/1.00 (E-F-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 6.0 Deg.

JSI GRIP= 0.82 (K) (INPUT= 0.90)
JSI METAL= 0.27 (F) (INPUT= 1.00)



DRWG NO. TAM 11900044
STRUCTURAL
COMPONENT ONLY

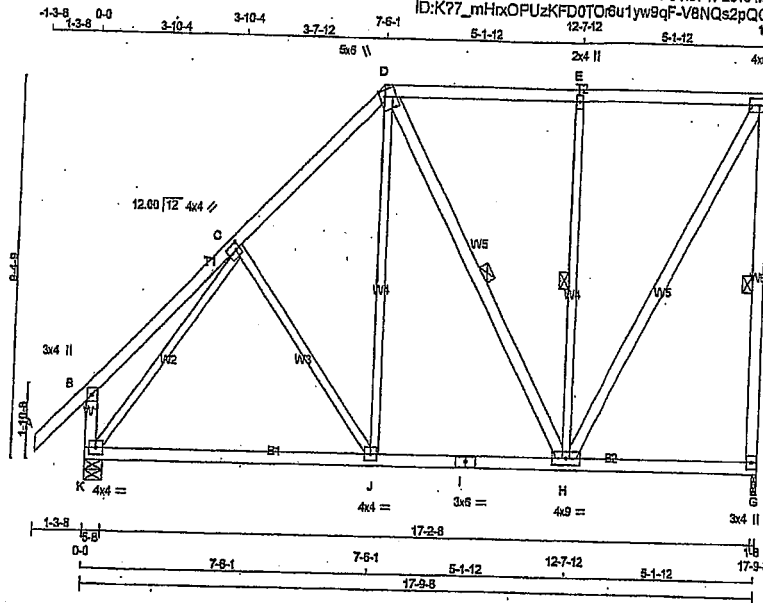
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300536	T6	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:K77_mHxOPUzKFD0T0r6u1yw9qF-V8NQs2pQq6IF_RATQzArMSW0bQUyA7773y64izz5OZ

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Thu Jan 3 11:22:50 2019 Page 1

Scale = 1:52.0



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4 DRY	No.2	SPF		
D - F	2x4 DRY	No.2	SPF		
G - F	2x4 DRY	No.2	SPF		
K - B	2x4 DRY	No.2	SPF		
K - I	2x4 DRY	No.2	SPF		
I - G	2x4 DRY	No.2	SPF		
ALL WEBS	2x3 DRY	No.2	SPF		
EXCEPT					
D - H	2x4 DRY	No.2	SPF		
H - F	2x4 DRY	No.2	SPF		

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW+P	MT20	3.0	4.0		
C	TMW+H	MT20	4.0	4.0	2.00	1.25
D	TMW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+H	MT20	2.0	4.0		
F	TMW+H	MT20	4.0	6.0		
G	BMV+P	MT20	3.0	4.0		
H	BMW+H	MT20	4.0	9.0		
I	BS+H	MT20	3.0	6.0		
J	BMW+H	MT20	4.0	4.0		
K	BMV+H	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	UPLIFT	IN-SX
G	1097	0	0	1-8
K	1216	0	0	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	832	414/0	197/0	0/0	0/0	231/0	0/0
K	914	479/0	197/0	0/0	0/0	248/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. FURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	WEBS
	MAX. FACTORED	MAX. FACTORED
	FORCE	FORCE
	(LBS)	(LBS)
FR-TO	FROM	TO
A-B	0/42	-84.9 -84.9 0.12 (1)
B-C	0/26	-84.9 -84.9 0.19 (1)
C-D	-798/0	-84.9 -84.9 0.20 (1)
D-E	-481/0	-84.9 -84.9 0.38 (1)
E-F	-481/0	-84.9 -84.9 0.38 (1)
G-F	-1011/0	0.0 0.0 0.42 (1)
K-B	-239/0	0.0 0.0 0.03 (1)
K-J	0/807	-38.5 -38.5 0.42 (2)
J-I	0/549	-38.5 -38.5 0.42 (2)
I-H	0/549	-38.5 -38.5 0.42 (2)
H-G	0/0	-38.5 -38.5 0.17 (3)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3 PSF
DL = 6.0 PSF	
BOT CH.	LL = 10.5 PSF
DL = 7.0 PSF	
TOTAL LOAD	= 46.8 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.69")
CALCULATED VERT. DEFL.(LL) = L/999 (0.13")
ALLOWABLE DEFL.(TL) = L/360 (0.69")
CALCULATED VERT. DEFL.(TL) = L/957 (0.22")

CSI: TC=0.42/1.00 (F-G:1), BC=0.42/1.00 (J-K:2),
WB=0.76/1.00 (C-K:1), SS=0.21/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

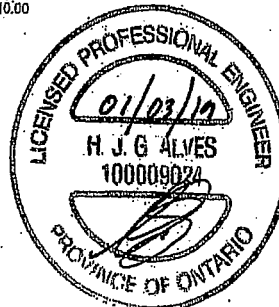
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1687 788	1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (C) (INPUT = 0.80)
JSI METAL= 0.39 (C) (INPUT = 1.00)



DRWG NO. TAM T1900045
STRUCTURAL
COMPONENT ONLY

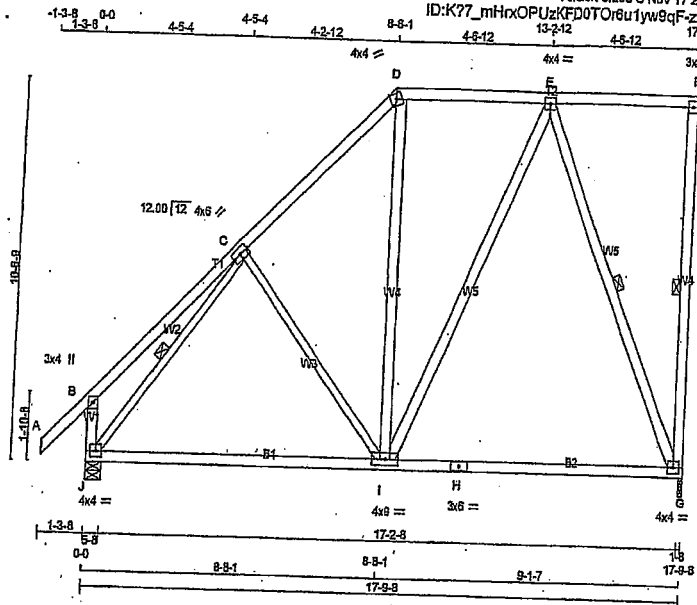
JOB NAME 300536	TRUSS NAME T7	QUANTITY 1	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
---------------------------	-------------------------	----------------------	-----------------	---------------------------------	----------

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Thu Jan 3 11:22:51 2019 Page 1

ID:K77_mHxOPuzKF0TOr6u1yw9qF-zLxp3Qq2Byu6bbf_gFPJg3D2?hnhfXGMjfgc8zz5OY

Scale = 1:58.4



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
G - F	2x4	DRY	No.2
J - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
H - G	2x4	DRY	No.2

ALL WEBS EXCEPT	2x4	DRY	No.2
C - I	2x3	DRY	No.2
J - C	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVWV-t	MT20	4.0	6.0		
D	TTW-m	MT20	4.0	4.0		Edge
E	TMVWV-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVWV-t	MT20	4.0	4.0		
H	BS-4	MT20	3.0	6.0		
I	BMVWV-t	MT20	4.0	4.0		
J	BMVWV-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

DESCR.	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
JT	1097	0	1097	0	1-8	1-8
G	1216	0	1216	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	832	414/0	187/0	0/0	0/0	231/0	0/0
J	914	479/0	187/0	0/0	0/0	248/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, E-G, C-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED				WEBS			
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 (LC)	MAX UNBRACED LENGTH	MEMB.	MAX. FORCE (LBS)	FACTORED CS1 (LC)		
FR-TO		FROM			FR-TO				
A-B	0/42	-84.9	-84.9	0.12 (1)	10.00	C-1	-172/14 0.18 (1)		
B-C	0/31	-84.9	-84.9	0.28 (1)	10.00	I-D	0/260 0.04 (2)		
C-D	-753/0	-84.9	-84.9	0.28 (1)	6.25	I-E	0/397 0.08 (2)		
D-E	-512/0	-84.9	-84.9	0.30 (1)	6.25	E-G	-999/0 0.62 (1)		
E-F	0/0	-84.9	-84.9	0.29 (1)	10.00	J-C	-1034/0 0.98 (1)		
G-F	-148/0	0.0	0.0	0.08 (1)	6.25				
J-B	-256/0	0.0	0.0	0.03 (1)	7.81				
J-I	0/614	-38.5	-38.5	0.74 (2)	10.00				
I-H	0/365	-38.5	-38.5	0.72 (3)	10.00				
H-G	0/365	-38.5	-38.5	0.72 (3)	10.00				

TOTAL WEIGHT = 108 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
	DL = 8.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2010, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

65% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/999 (0.18")
ALLOWABLE DEFL.(TL) = L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/693 (0.31")

CS1: TC=0.30/1.00 (D-E-1), BC=0.74/1.00 (I-J-2), WB=0.62/1.00 (E-G-1), SS=0.22/1.00 (G-L-3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

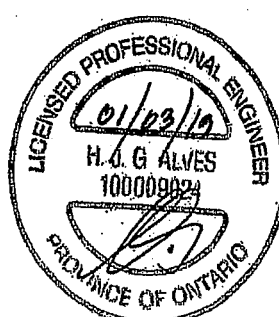
NAIL VALUES	PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1667

PLATE PLACEMENT TOL = 0.250 Inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.80 (J) (INPUT = 0.80)

JSI METAL = 0.26 (C) (INPUT = 1.00)

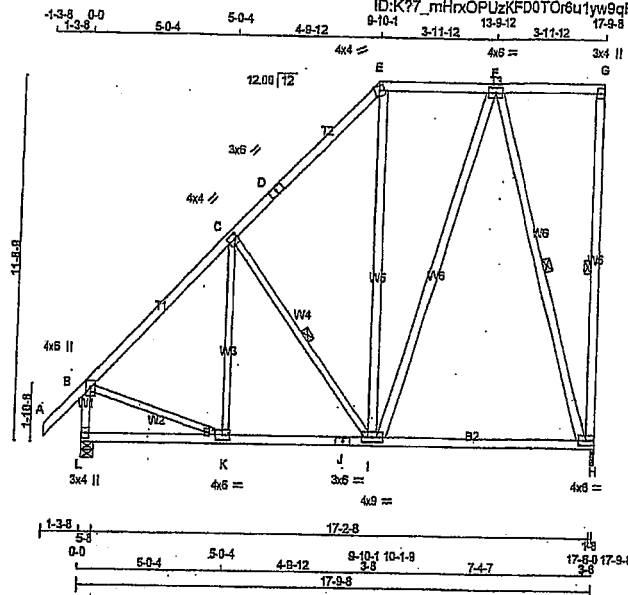


DWG NO. TAM **T1900046**
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300536	T8	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Thu Jan 3 11:22:51 2019 Page 1
ID:K77_mHrxOPuzKF0D0r6u1yw9qF-zLxo3Qq2Byu6bbf_gFPJg3DJ7iAhdNGMjgc8zz5OY



Scale = 1:67.7

LUMBER			
N.L.G.A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
H - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS EXCEPT	2x4	DRY	No.2
K - C	2x3	DRY	No.2
C - I	2x3	DRY	No.2
B - K	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table in inches)				
JT TYPE	PLATES	W	LEN	Y X
B	TMWW+p	MT20	4.0	6.0 2.75 2.00
C	TMWW-t	MT20	4.0	4.0 2.00 1.00
D	TS-t	MT20	3.0	6.0
E	TTW-m	MT20	4.0	4.0 Edge
F	TMWW-t	MT20	4.0	6.0
G	TMWW-p	MT20	3.0	4.0
H	BMWW-t	MT20	4.0	6.0
I	BMWWW-t	MT20	4.0	9.0
J	BS-t	MT20	3.0	6.0
K	BMWW-t	MT20	4.0	6.0
L	BMV1+p	MT20	3.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	VERT	GROSS REACTION	GROSS REACTION	DOWN	HORIZ	UPLIFT	IN-SX	IN-SX	IN-SX
H	1097	0	1097	0	0	1-8	1-8	1-8	1-8
L	1216	0	1216	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS		1ST LOOSE		MAX/MIN. COMPONENT REACTIONS		SNOW		LIVE		PERM. LIVE		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	SNOW	LIVE	PERM. LIVE	WIND
H	832	414/0	187/0	0/0	0/0	231/0	0/0	832	414/0	187/0	0/0	0/0	231/0	832	414/0	187/0	0/0
L	914	479/0	187/0	0/0	0/0	248/0	0/0	914	479/0	187/0	0/0	0/0	248/0	914	479/0	187/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.18 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H, C-I, F-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX. FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX	LC1 MAX	UNBRAC	MEMB.	FORCE (LBS)	MAX	CS1 (LC)
FR-TO		FROM	TO	LENGTH	FR-TO				
A-B	0/42	-84.9	-84.9	0.12 (1)	10.00	K-C	-24/122	0.03 (3)	
B-C	-887/0	-84.9	-84.9	0.29 (1)	6.18	C-I	-358/0	0.18 (1)	
C-D	-857/0	-84.9	-84.9	0.27 (1)	6.25	I-E	0/195	0.03 (3)	
D-E	-857/0	-84.9	-84.9	0.27 (1)	6.25	B-K	0/651	0.15 (1)	
E-F	-439/0	-84.9	-84.9	0.17 (1)	6.25	F-H	0/457	0.07 (2)	
F-G	0/0	-84.9	-84.9	0.22 (1)	10.00				
G-H	-128/0	0.0	0.0	0.09 (1)	6.25				
L-B	-1131/0	0.0	0.0	0.13 (1)	7.46				
L-K	0/0	-38.5	-38.5	0.17 (3)	10.00				
K-J	0/652	-38.5	-38.5	0.46 (2)	10.00				
J-I	0/652	-38.5	-38.5	0.46 (2)	10.00				
I-H	0/295	-38.5	-38.5	0.41 (3)	10.00				

TOTAL WEIGHT = 117 lb

[MIF]

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.59")
CALCULATED VERT. DEFL. (LL) = L/599 (0.14")
ALLOWABLE DEFL. (TL) = L/360 (0.59")
CALCULATED VERT. DEFL. (TL) = L/896 (0.24")

CS1: TC=0.29/1.00 (B-C:1), BC=0.46/1.00 (I-K:2), WB=0.78/1.00 (F-H:1), SS=0.19/1.00 (H-K:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

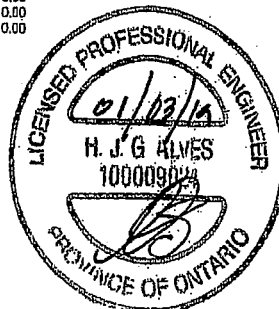
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1657 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.81 (J) (INPUT = 0.90)
JSI METAL=0.52 (J) (INPUT = 1.00)



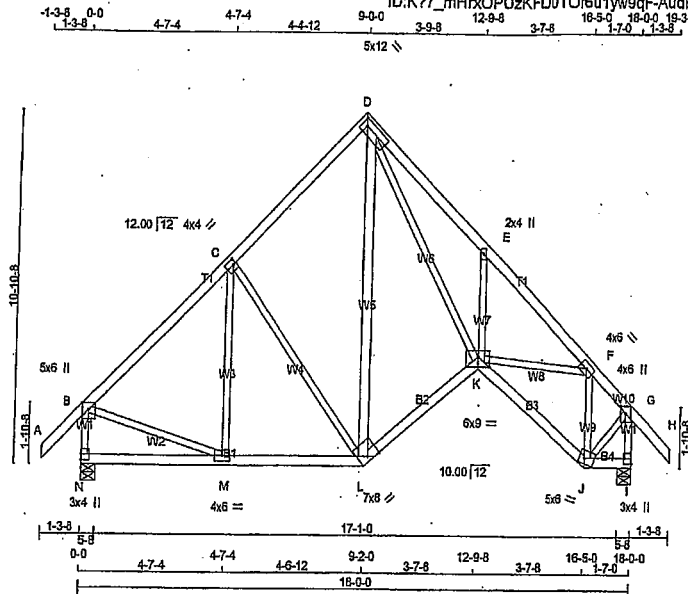
DRWG NO. TAM T1400047
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
259671	T9	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MTEK Industries, Inc. Thu Jan 3 13:33:22 2019 Page 1

ID:K77_mHrxOPUzKFD0TOn6u1yw9qF-AudFd4b9_1V_b3wjEqEoB4uVqjVO7Pw5Z7YzIzz3UB



Scale = 1/8\"/>

TOTAL WEIGHT = 2 X 101 = 203 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - H	2x4	DRY	No.2	SPF	
N - B	2x3	DRY	No.2	SPF	
I - G	2x3	DRY	No.2	SPF	
N - L	2x4	DRY	No.2	SPF	
L - K	2x4	DRY	No.2	SPF	
K - J	2x4	DRY	No.2	SPF	
J - I	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT	L - D	2x4	DRY	No.2	SPF

DRY, SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMVW-t	MT20	4.0	4.0	2.00	1.00
D	TTWW-t	MT20	5.0	12.0	2.25	2.00
E	TMVW-w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	TMVW+p	MT20	4.0	6.0	2.75	2.00
I	BMV1+p	MT20	3.0	4.0	2.00	Edge
J	BBWV-w	MT20	5.0	6.0	2.00	2.00
K	BBWV-w	MT20	6.0	9.0	Edge	4.50
L	BBWV-w	MT20	7.0	8.0	Edge	4.50
M	BMV1-t	MT20	4.0	6.0		
N	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
N	1224	0	1224	0
I	1233	0	1233	0

UNFACTORED REACTIONS

1ST LCASE	MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
N	921	482/0	189/0	0/0	0/0	250/0
I	927	487/0	189/0	0/0	0/0	251/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.04 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0/42	-84.9	-84.9 0.12 (1)	10.00	M-C	-5/163	0.04 (3)
B-C	-910/0	-84.9	-84.9 0.24 (1)	6.19	C-L	-336/0	0.39 (1)
C-D	-683/0	-84.9	-84.9 0.23 (1)	6.25	L-D	0/110	0.02 (3)
D-E	-1609/0	-84.9	-84.9 0.20 (1)	5.04	D-K	0/1461	0.33 (1)
E-F	-1606/0	-84.9	-84.9 0.15 (1)	5.10	K-E	-348/0	0.09 (1)
F-G	-744/0	-84.9	-84.9 0.09 (1)	6.25	K-F	0/689	0.15 (1)
G-H	0/42	-84.9	-84.9 0.12 (1)	10.00	J-F	-771/0	0.15 (1)
N-B	-1155/0	0.0	0.0 0.18 (1)	6.50	B-M	0/702	0.16 (1)
I-G	-1203/0	0.0	0.0 0.18 (1)	6.40	J-G	0/706	0.16 (1)
N-M	0/0	-38.5	-38.5 0.18 (3)	10.00			
M-L	0/666	-38.5	-38.5 0.26 (2)	10.00			
L-K	0/609	-38.5	-38.5 0.20 (2)	10.00			
K-J	0/596	-38.5	-38.5 0.20 (2)	10.00			
J-I	0/0	-38.5	-38.5 0.02 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2018, CBC 2012
- CSA 088-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.60")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.60")
CALCULATED VERT. DEFL.(TL) = L/999 (0.10")

CSI: TC=0.24/1.00 (B-C-1), BC=0.28/1.00 (L-M-2), WB=0.39/1.00 (C-L-1), SSI=0.13/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

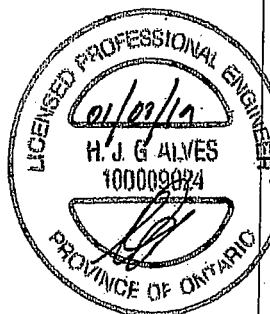
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

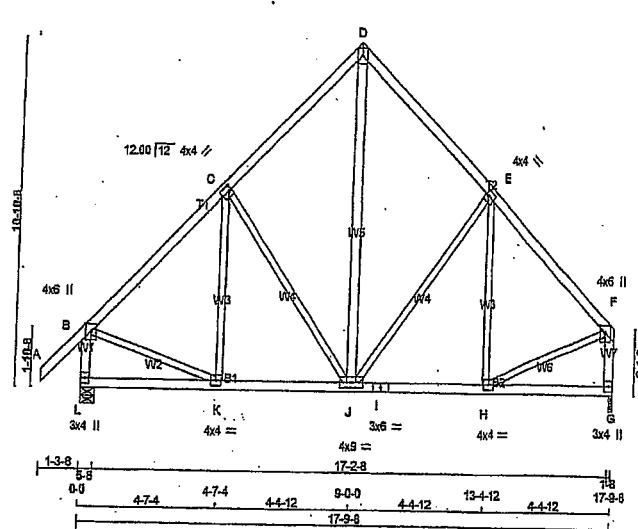
JS GRIP= 0.88 (D) (INPUT = 0.90)
JS METAL= 0.34 (B) (INPUT = 1.00)



DRWG NO. TAM 71900092
STRUCTURAL
COMPONENT ONLY

JOB NAME 300536 Tamarack Roof Truss, Burlington	TRUSS NAME T10A	QUANTITY 3	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
---	--------------------	---------------	----------	--------------------------	----------

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Thu Jan 3 11:22:53 2019 Page 1
 ID:K77_mHxOPUzKFD0TOr6u1yw9qF-vj3ZU4rJZ8qrvv265HtO58adoVC9cyZp18mh1zz5OW
 Scale = 1/85.7



LUMBER

N.L.G.A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
L - B	2x4	DRY	No.2
G - F	2x4	DRY	No.2
L - I	2x4	DRY	No.2
I - G	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
J - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	6.0	2.75	2.00
C TMVW-t	MT20	4.0	4.0	2.00	1.00
D TTW+p	MT20	4.0	6.0		
E TMVW-t	MT20	4.0	4.0	2.00	1.00
F TMVW+p	MT20	4.0	6.0	2.75	2.00
G BMV1+p	MT20	3.0	4.0		
H BMVW-t	MT20	4.0	4.0		
I BS-t	MT20	3.0	6.0		
J BMVW-t	MT20	4.0	6.0		
K BMVW-t	MT20	4.0	4.0		
L BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
L	1216	0	1216	0	5-8	5-8
G	1097	0	1097	0	1-8	1-8

UNFACTORED REACTIONS							
JT	1ST LOASE		MAX/MIN COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	914	479/0	187/0	0/0	0/0	248/0	0/0
G	832	414/0	187/0	0/0	0/0	231/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.22 FT.
 MAX. UNRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 MAX (PLF) CSI (LC)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD LC1 MAX (PLF) CSI (LC)
FR-TO		FROM TO	FR-TO	FROM TO
A-B	0/42	-84.9 -84.9 0.12 (1)	10.00	K-C
B-C	-898/0	-84.9 -84.9 0.23 (1)	6.22	C-J
C-D	-898/0	-84.9 -84.9 0.23 (1)	6.25	J-D
D-E	-895/0	-84.9 -84.9 0.21 (1)	6.25	E-E
E-F	-869/0	-84.9 -84.9 0.22 (1)	6.25	H-E
L-B	-1144/0	0.0 0.0 0.13 (1)	7.43	B-K
G-F	-1029/0	0.0 0.0 0.12 (1)	7.74	H-F
L-K	0/0	-38.5 -38.5 0.15 (3)	10.00	
K-J	0/657	-38.5 -38.5 0.23 (2)	10.00	
J-I	0/637	-38.5 -38.5 0.22 (2)	10.00	
I-H	0/637	-38.5 -38.5 0.22 (2)	10.00	
H-G	0/0	-38.5 -38.5 0.14 (3)	10.00	

TOTAL WEIGHT = 3 X 95 = 285 lb

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 23.3 PSF
 DL = 6.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 46.8 PSF

SPACING = 240 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF CBC 2018, CBC 2012
 - CSA 088-09, CSA 088-14
 - TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL (LL) = L/360 (0.59")
 CALCULATED VERT. DEFL (LL) = L/999 (0.02")
 ALLOWABLE DEFL (TL) = L/360 (0.59")
 CALCULATED VERT. DEFL (TL) = L/999 (0.04")

CSI: TO=0.23/1.00 (B-C:1), BO=0.23/1.00 (J-K:2), WB=0.37/1.00 (C-E:1), SS=0.13/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

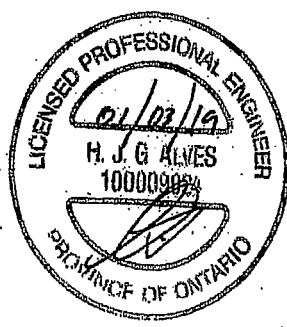
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL) (PL)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1657 788 1987 1856

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.85 (K) (INPUT = 0.90)
 JSI METAL= 0.42 (B) (INPUT = 1.00)

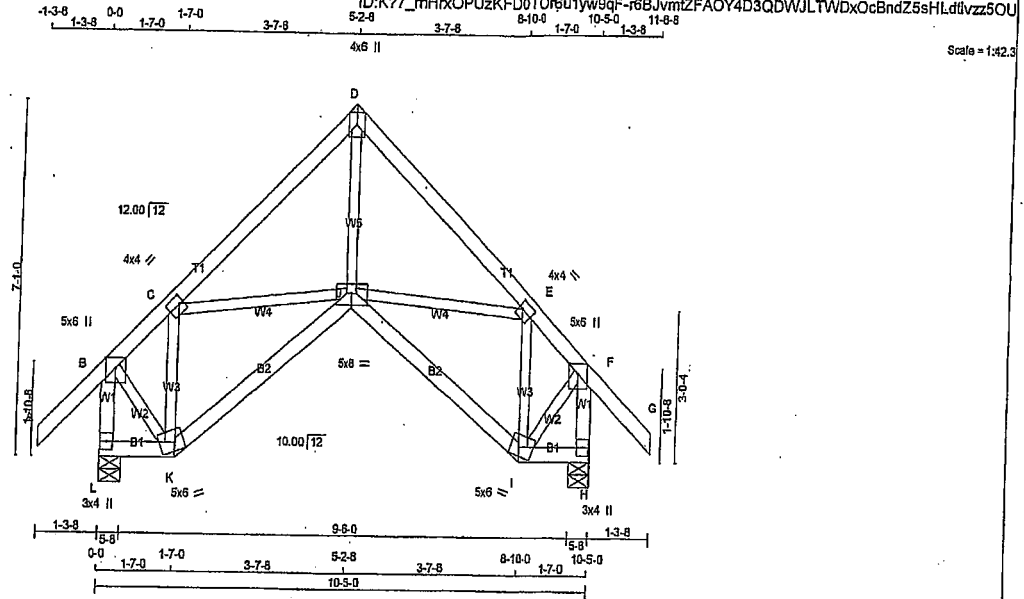


DRWG NO. TAM T1900049
 STRUCTURAL
 COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300536	T11	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 Mitex Industries, Inc. Thu Jan 3 11:22:55 2019 Page 1
ID:K77_mHxOPUzKF0T0r6u1yw9qF-r6BJvmtZFA0Y4D3QDWJLTWDXOcBndZ5sHLdtivzz5OU



Scale = 1:42.3

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	5.0	6.0	Edge	
C TMVW-t	MT20	4.0	4.0	2.00	1.00
D TTW+p	MT20	4.0	6.0		
E TMVW-t	MT20	4.0	4.0	2.00	1.00
F TMVW+p	MT20	5.0	6.0	Edge	
H BMV1+p	MT20	3.0	4.0		
I BBVW+m	MT20	5.0	6.0	2.00	2.25
J BBVW-w	MT20	5.0	8.0	2.75	4.00
K BBVW-m	MT20	5.0	6.0	2.00	2.25
L BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	UP
L	781	0	781	0
H	781	0	781	0

UNFACTORED REACTIONS

1ST CASE	MAX/MIN	COMPONENT REACTIONS	WIND	DEAD	SOIL
JT	COMBINED	SNOW	LIVE	PERM	LIVE
L	569	308 / 0	109 / 0	0 / 0	152 / 0
H	569	308 / 0	109 / 0	0 / 0	152 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)
FR-TO									
A-B	0 / 42	-84.9	-84.9	0.12 (1)	10.00	J-D	0 / 584	0.13 (1)	
B-C	-402 / 0	-84.9	-84.9	0.11 (1)	6.25	J-E	0 / 172	0.04 (1)	
C-D	-632 / 0	-84.9	-84.9	0.15 (1)	6.25	I-E	-396 / 0	0.08 (1)	
D-E	-632 / 0	-84.9	-84.9	0.15 (1)	6.25	C-J	0 / 172	0.04 (1)	
E-F	-402 / 0	-84.9	-84.9	0.11 (1)	6.25	K-C	-396 / 0	0.08 (1)	
F-G	0 / 42	-84.9	-84.9	0.12 (1)	10.00	B-K	0 / 400	0.09 (1)	
L-B	-730 / 0	0.0	0.0	0.08 (1)	7.81	I-F	0 / 400	0.09 (1)	
H-F	-730 / 0	0.0	0.0	0.08 (1)	7.81				
L-K	0 / 0	-38.5	-38.5	0.02 (3)	10.00				
K-J	0 / 342	-38.5	-38.5	0.16 (2)	10.00				
J-I	0 / 342	-38.5	-38.5	0.16 (2)	10.00				
I-H	0 / 0	-38.5	-38.5	0.02 (3)	10.00				

TOTAL WEIGHT = 56 lb

[M/F]

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.16/1.00 (D-E-1), BC=0.16/1.00 (J-K-2), WB=0.13/1.00 (D-J-1), SS=0.10/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

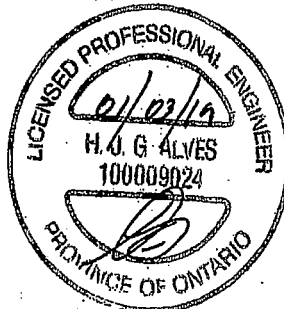
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 768 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.53 (I) (INPUT = 0.90)
JSI METAL= 0.16 (B) (INPUT = 1.00)



DRWG NO. 1AM T1900057
STRUCTURAL
COMPONENT ONLY



DWG NO. TAM T1900052
STRUCTURAL
COMPONENT ONLY

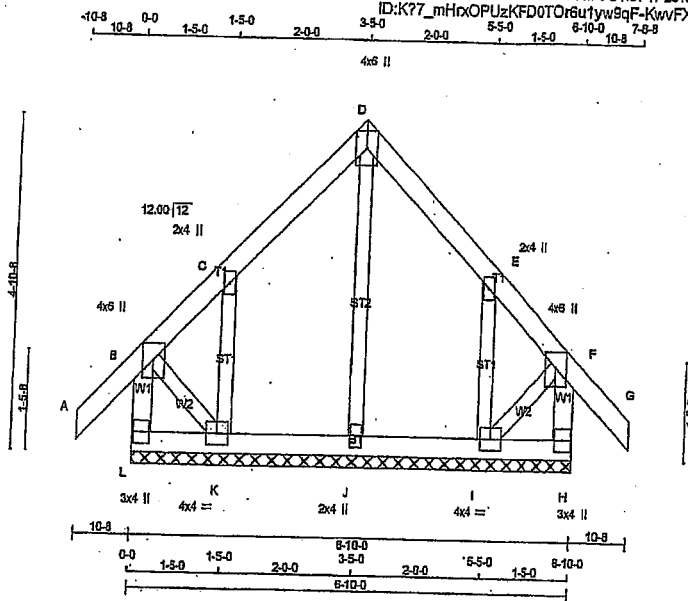
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300536	G12	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Thu Jan 3 11:24:38 2019 Page 1

ID:K77_mHxOPUzKFD0TOr6u1yw9qF-KwWFX_7A7qMx2RDf9V8E_Zs7NyGB30zs4qsRzz5M

Scale = 1:30.0



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
L - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
L - H	2x4	DRY	No.2

ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

GABLE STUDS SPACED AT 2-0-0 O.C.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVV+p	MT20	4.0	6.0	2.75	2.00
C	TMW+w	MT20	2.0	4.0		
D	TTW+p	MT20	4.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TMVV+p	MT20	4.0	6.0	2.75	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMVV1-t	MT20	4.0	4.0		
J	BMV1+w	MT20	2.0	4.0		
K	BMVV1-t	MT20	4.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	FACTORED	MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	FACTORED
FR-TO				FR-TO			
A-B	0/29	-84.9	-84.9 0.06 (1)	J-D	-118/0	0.04 (1)	
B-C	-40/0	-84.9	-84.9 0.06 (1)	K-C	-142/0	0.02 (1)	
C-D	-33/0	-84.9	-84.9 0.06 (1)	E-E	-142/0	0.02 (1)	
D-E	-33/0	-84.9	-84.9 0.06 (1)	B-K	0/29	0.01 (1)	
E-F	-40/0	-84.9	-84.9 0.06 (1)	I-F	0/29	0.01 (1)	
F-G	0/29	-84.9	-84.9 0.06 (1)				
L-B	-190/0	0.0	0.0 0.02 (1)				
H-F	-190/0	0.0	0.0 0.02 (1)				
L-K	0/0	-38.5	-38.5 0.02 (3)				
K-J	0/17	-38.5	-38.5 0.03 (3)				
J-I	0/17	-38.5	-38.5 0.03 (3)				
I-H	0/0	-38.5	-38.5 0.02 (3)				

TOTAL WEIGHT = 34 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
BOT CH.	LL	=	6.0	PSF
	LL	=	10.5	PSF
	LL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2010, NBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.06/1.00 (A-B-1), BC=0.03/1.00 (I-J-3), WB=0.04/1.00 (D-J-1), SS=0.08/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

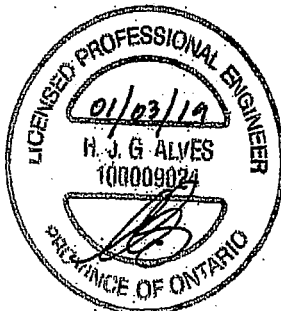
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 788	1987 1656

PLATE PLACEMENT TOL. = 0.250 Inches

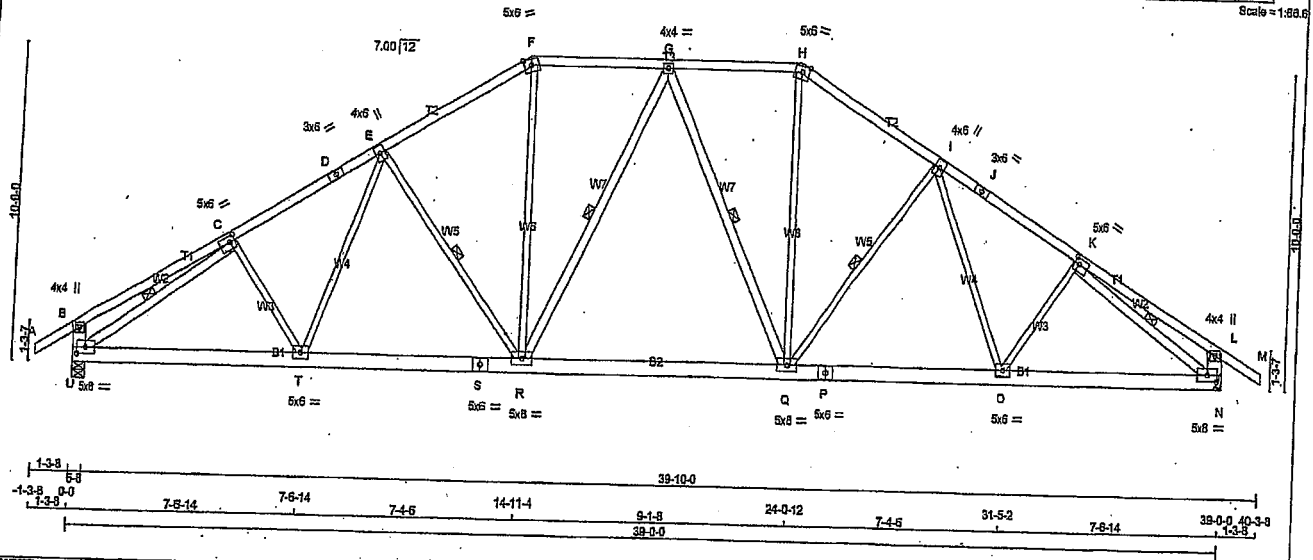
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (F) (INPUT = 0.80)
JSI METAL= 0.07 (C) (INPUT = 1.00)



DRWG NO. TAM T1900035
STRUCTURAL
COMPONENT ONLY

JOB NAME 300539	TRUSS NAME T13	QUANTITY 6	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington					
Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Thu Jan 3 12:17:49 2019 Page 1					
ID:K77_mHxOPuzKF0T0r8u1yw9qF-PpDaF5kmWGOAGRWBJPoz9mPRLZQDZkqZAN_zz4b0					
Scale = 1:80.0					



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4 DRY	No.2	SPF
D - F	2x4 DRY	No.2	SPF
F - H	2x4 DRY	No.2	SPF
H - J	2x4 DRY	No.2	SPF
J - M	2x4 DRY	No.2	SPF
U - B	2x6 DRY	No.2	SPF
N - L	2x6 DRY	No.2	SPF
U - S	2x6 DRY	No.2	SPF
S - P	2x6 DRY	No.2	SPF
P - N	2x6 DRY	No.2	SPF
ALL WEBS EXCEPT			
R - G	2x4 DRY	No.2	SPF
G - Q	2x4 DRY	No.2	SPF
U - C	2x4 DRY	No.2	SPF
K - N	2x4 DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+P	MT20	4.0	4.0
C	TMVW+T	MT20	5.0	6.0 2.25 2.00
D	TS+I	MT20	3.0	6.0
E	TMVW+T	MT20	4.0	6.0
F	TTW+M	MT20	5.0	6.0 Edge
G	TMVW+T	MT20	4.0	4.0
H	TTW+M	MT20	5.0	6.0 Edge
I	TMVW+T	MT20	4.0	6.0
J	TS+I	MT20	3.0	6.0
K	TMVW+T	MT20	5.0	8.0 2.25 2.00
L	TMV+P	MT20	4.0	4.0
N	BMVW+T	MT20	5.0	8.0 2.50 3.75
O	BMVW+T	MT20	5.0	6.0
P	BS+I	MT20	5.0	6.0
Q	BMVW+T	MT20	5.0	8.0
R	BMVW+T	MT20	5.0	8.0
S	BS+I	MT20	5.0	6.0
T	BMVW+T	MT20	5.0	6.0
U	BMVW+T	MT20	5.0	8.0 2.50 3.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	JT	GROSS REACTION	BRG	BRG	BRG	BRG
U	2521 0	2521 0	0 0	5-8	5-8	5-8	5-8
N	2521 0	2521 0	0 0	MECHANICAL	MECHANICAL	MECHANICAL	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT N. MINIMUM BEARING LENGTH AT JOINT N = 3-8.

UNFACTORED REACTIONS

1ST CASE		MAX/MIN. COMPONENT REACTIONS		WIND		DEAD		SOIL	
JT	COMBINED	JT	COMBINED	WIND	WIND	DEAD	DEAD	SOIL	SOIL
U	1905 872/0	1905 872/0	410/0	0/0	0/0	523/0	523/0	0/0	0/0
N	1905 872/0	1905 872/0	410/0	0/0	0/0	523/0	523/0	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.64 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-R, G-R, G-Q, I-Q, C-U, K-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED CSI (L/C)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (L/C)	UNBRACED LENGTH (FT)
FR-TO				FR-TO			
A-B	0/29	-84.9	-84.9 0.11 (1)	C-T	0/147	0.03 (3)	10.00
B-C	0/19	-84.9	-84.9 0.26 (1)	T-E	0/330	0.07 (2)	10.00
C-D	-3216/0	-84.9	-84.9 0.38 (1)	E-R	-598/0	0.28 (1)	3.64
D-E	-3216/0	-84.9	-84.9 0.38 (1)	R-F	0/1036	0.23 (1)	3.64
E-F	-2711/0	-84.9	-84.9 0.35 (1)	F-G	-255/0	0.15 (1)	3.94
F-G	-2333/0	-84.9	-84.9 0.28 (1)	G-Q	-255/0	0.15 (1)	4.25
G-H	-2333/0	-84.9	-84.9 0.28 (1)	Q-H	0/1036	0.23 (1)	4.25
H-I	-2711/0	-84.9	-84.9 0.35 (1)	I-O	-598/0	0.28 (1)	3.64
I-J	-3216/0	-84.9	-84.9 0.39 (1)	O-K	0/330	0.07 (2)	3.64
J-K	-3216/0	-84.9	-84.9 0.39 (1)	K-N	-3472/0	0.81 (1)	3.64
K-L	0/19	-84.9	-84.9 0.26 (1)	U-C	-3472/0	0.81 (1)	10.00
L-M	0/29	-84.9	-84.9 0.11 (1)	C-K	0/147	0.03 (3)	10.00
U-B	-280/0	0.0	0.0 0.02 (1)	U-C	-3472/0	0.81 (1)	10.00
N-L	-280/0	0.0	0.0 0.02 (1)				7.81
U-T	0/2763	-38.5	-38.5 0.47 (1)				10.00
T-S	0/2681	-38.5	-38.5 0.47 (2)				10.00
S-R	0/2681	-38.5	-38.5 0.47 (2)				10.00
R-Q	0/2440	-38.5	-38.5 0.44 (2)				10.00
Q-P	0/2681	-38.5	-38.5 0.47 (2)				10.00
P-O	0/2681	-38.5	-38.5 0.47 (2)				10.00
O-N	0/2763	-38.5	-38.5 0.47 (1)				10.00

TOTAL WEIGHT = 6 X 209 = 1252 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 48.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2010, CBC 2012
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(65 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.30")
CALCULATED VERT. DEFL. (LL) = L/989 (0.15")
ALLOWABLE DEFL. (TL) = L/360 (1.30")
CALCULATED VERT. DEFL. (TL) = L/989 (0.26")

CSI: TC=0.39/1.00 (I-C-1), BC=0.47/1.00 (O-Q-2),
WB=0.64/1.00 (K-N-1), SS=0.19/1.00 (G-H-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

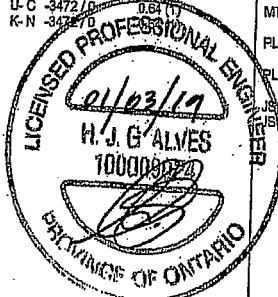
NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN
MT20 618 354 1887 788 1887 1656

PLATE PLACEMENT TOL. = 0.250 inches

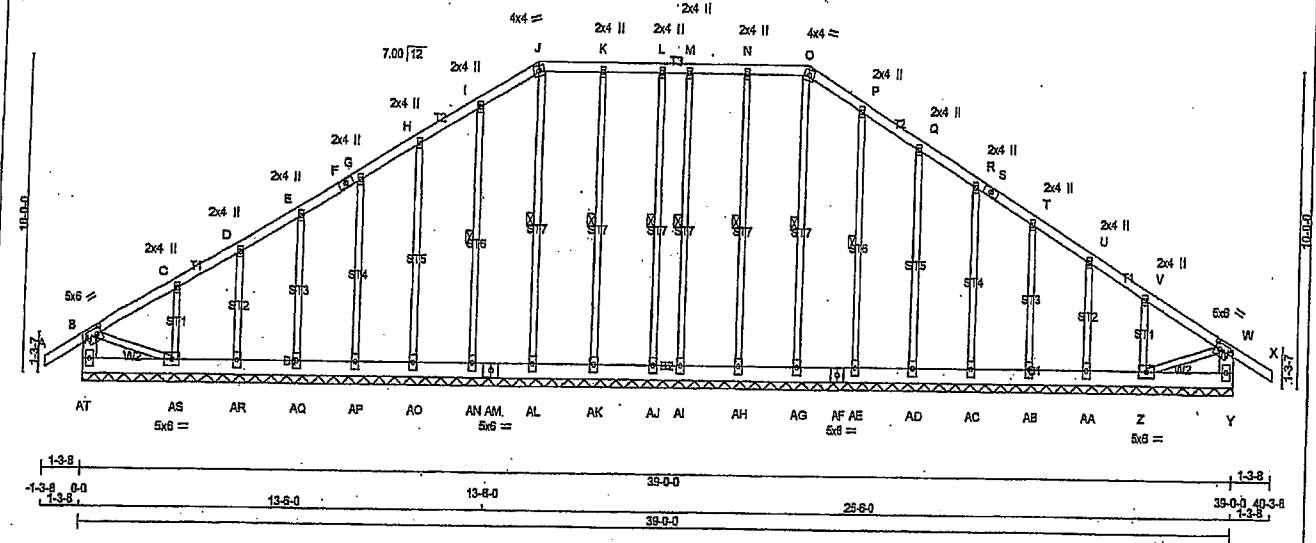
PLATE ROTATION TOL. = 5.0 Deg.

GRIP = 0.90 (K) (INPUT = 0.90)
METAL = 0.86 (C) (INPUT = 1.00)



DRWG NO. TAM T19 000 66
STRUCTURAL
COLLUSION

JOB NAME TRUSS NAME QUANTITY PLY JOB DESC. DRWG NO. 300539 G13 2 1 TRUSS DESC. Tamarack Roof Truss, Burlington Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Thu Jan 3 12:18:35 2019 Page 1 ID:K77_mHxOPUzKFD0TOr6u1yw9qF-2g?zvqldgOBenVOLT_EarsZk7_9B88JUJ_6_Kzz4a1 39-0-0 40-3-8 1-3-8 Scale = 1:50.0



LUMBER				N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.	CHORDS	SIZE	LUMBER	DESCR.
A - F	2x4	DRY	No.2	SPF			
F - J	2x4	DRY	No.2	SPF			
J - O	2x4	DRY	No.2	SPF			
O - S	2x4	DRY	No.2	SPF			
S - X	2x4	DRY	No.2	SPF			
AT - B	2x6	DRY	No.2	SPF			
Y - W	2x6	DRY	No.2	SPF			
AT - AM	2x6	DRY	No.2	SPF			
AM - AF	2x6	DRY	No.2	SPF			
AF - Y	2x6	DRY	No.2	SPF			
ALL WEBS	2x3	DRY	No.2	SPF			
ALL GABLE WEBS	2x3	DRY	No.2	SPF			
DRY: SEASONED LUMBER.							
GABLE STUDS SPACED AT 2'-0" OC.							

PLATES (table is in inches)				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER			
JT TYPE	PLATES	W	LEN Y X	BEARINGS			
B TMWV-t	MT20	5.0	6.0	2.00	THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.		
C, D, E, G, H, I, K, L, M, N, P, Q, R, T, U, V	MT20	2.0	4.0		THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.		
C TMWV-w	MT20	2.0	4.0		BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)		
F TS-t	MT20	3.0	6.0		BRACING		
J TTW-m	MT20	4.0	4.0		TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.		
O TTW-m	MT20	3.0	6.0		MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.		
S TS-t	MT20	3.0	6.0		ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.		
W TMWV-t	MT20	5.0	6.0	2.50 2.00	1 LATERAL BRACE(S) AT 1/2 LENGTH OF L-AJ, K-AK, J-AL, I-AN, N-AH, M-AI, O-AG, P-AE.		
Y BMV1-p	MT20	3.0	6.0		END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW		
Z BMWV1-t	MT20	5.0	6.0		LOADING		
AA, AB, AC, AD, AE, AG, AH, AI, AJ, AK, AL, AN, AO, AP, AQ, AR	MT20	3.0	6.0		TOTAL LOAD CASES: (4)		
AA BMW1-w	MT20	3.0	6.0		CHORDS		
AF BS-t	MT20	5.0	6.0		MEMB. FORCE VERT. LOAD LC1 MAX. FACTORED		
AM BS-t	MT20	5.0	6.0		(LBS)		
AS BMWV1-t	MT20	5.0	6.0		FR-TO FROM TO LENGTH FR-TO		
AT BMV1-p	MT20	3.0	6.0		A-B 0/29 -84.9 -84.9 0.11 (1) 10.00 AJ-L -117/0 0.08 (1)		
					B-C -26/0 -84.9 -84.9 0.09 (1) 6.25 AK-K -198/0 0.13 (1)		
					C-D -41/0 -84.9 -84.9 0.09 (1) 6.25 AL-J -131/0 0.09 (1)		
					D-E -23/0 -84.9 -84.9 0.04 (1) 6.25 AN-I -188/0 0.09 (1)		
					E-F -24/0 -84.9 -84.9 0.04 (1) 6.25 AO-H -168/0 0.17 (1)		
					F-G -24/0 -84.9 -84.9 0.04 (1) 6.25 AP-G -198/0 0.11 (1)		
					G-H -21/0 -84.9 -84.9 0.04 (1) 6.25 AR-O -174/0 0.07 (1)		
					H-I -18/0 -84.9 -84.9 0.04 (1) 6.25 AS-C -254/0 0.04 (1)		
					I-J -25/0 -84.9 -84.9 0.05 (1) 6.25 AH-N -198/0 0.13 (1)		
					J-K -13/0 -84.9 -84.9 0.05 (1) 6.25 AI-M -117/0 0.08 (1)		
					K-L -13/0 -84.9 -84.9 0.03 (1) 6.25 AG-O -131/0 0.09 (1)		
					L-M -13/0 -84.9 -84.9 0.05 (1) 6.25 AE-P -188/0 0.09 (1)		
					M-N -13/0 -84.9 -84.9 0.05 (1) 6.25 AD-Q -168/0 0.17 (1)		
					N-O -13/0 -84.9 -84.9 0.04 (1) 6.25 AO-R -168/0 0.11 (1)		
					O-P -25/0 -84.9 -84.9 0.04 (1) 6.25 AB-T -174/0 0.07 (1)		
					P-Q -18/0 -84.9 -84.9 0.04 (1) 6.25 AA-U -189/0 0.04 (1)		
					Q-R -21/0 -84.9 -84.9 0.04 (1) 6.25 Z-V -254/0 0.04 (1)		
					R-S -24/0 -84.9 -84.9 0.04 (1) 6.25 B-AS 0/34 0.01 (1)		
					S-T -24/0 -84.9 -84.9 0.04 (1) 6.25 Z-W 0/34 0.01 (1)		
					T-U -23/0 -84.9 -84.9 0.09 (1) 6.25		
					U-V -41/0 -84.9 -84.9 0.09 (1) 6.25		
					V-W -26/0 -84.9 -84.9 0.11 (1) 10.00		
					W-X 0/29 -84.9 -84.9 0.02 (1) 7.81		
					AT-B -247/0 0.0 0.0 0.02 (1) 7.81		
					Y-W -247/0 0.0 0.0 0.02 (1) 7.81		
					AT-AS 0/0 -38.5 -38.5 0.03 (3) 10.00		
					AS-AR 0/25 -38.5 -38.5 0.03 (2) 10.00		
					AR-AQ 0/22 -38.5 -38.5 0.01 (2) 10.00		
					AQ-AP 0/20 -38.5 -38.5 0.01 (2) 10.00		
					AP-AO 0/18 -38.5 -38.5 0.01 (2) 10.00		
					AO-AN 0/16 -38.5 -38.5 0.01 (2) 10.00		
					AN-AM 0/15 -38.5 -38.5 0.01 (2) 10.00		
					AM-AL 0/15 -38.5 -38.5 0.01 (2) 10.00		
					AL-AK 0/13 -38.5 -38.5 0.01 (2) 10.00		
					AK-AJ 0/13 -38.5 -38.5 0.01 (2) 10.00		
					AJ-AI 0/13 -38.5 -38.5 0.01 (2) 10.00		
					AI-AH 0/13 -38.5 -38.5 0.01 (2) 10.00		
					AH-AG 0/13 -38.5 -38.5 0.01 (2) 10.00		
					AG-AF 0/15 -38.5 -38.5 0.01 (2) 10.00		
					AF-AE 0/15 -38.5 -38.5 0.01 (2) 10.00		
					AE-AD 0/16 -38.5 -38.5 0.01 (2) 10.00		
					AD-AC 0/18 -38.5 -38.5 0.01 (2) 10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CS: TC=0.11/1.00 (A-B-1), BC=0.03/1.00 (Z-AA-2), WB=0.17/1.00 (Q-AD-1), SS=0.10/1.00 (V-W-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

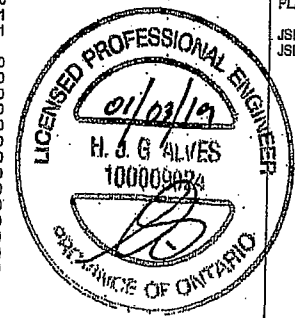
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 615 354 1657 788 1987 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.60 (J) (INPUT = 0.90)
JSI METAL= 0.12 (C) (INPUT = 1.00)

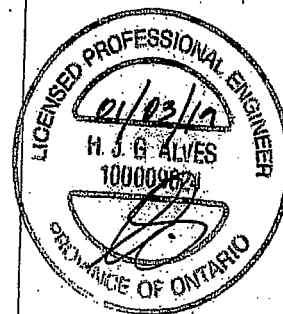


DRWG NO. TAM T1400060
STRUCTURAL
COMPANY ONLY 1/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300539	G13	2	1	TRUSS DESC.	
Tamarack Roof Truss, Burlington				Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Thu Jan 3 12:18:35 2019 Page 2 ID:K?7_mHxOPUzKFD0TOr6u1vw9aF-2a?zvaldgOBenVOLT EarsZK7 9BB8JUJ Ic Kzz4a	

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO			FR-TO		
AC-AB	0/20	-38.5 -38.5	0.01 (2)	10.00			
AB-AA	0/22	-38.5 -38.5	0.01 (2)	10.00			
AA-Z	0/25	-38.5 -38.5	0.03 (2)	10.00			
Z-Y	0/0	-38.5 -38.5	0.03 (3)	10.00			



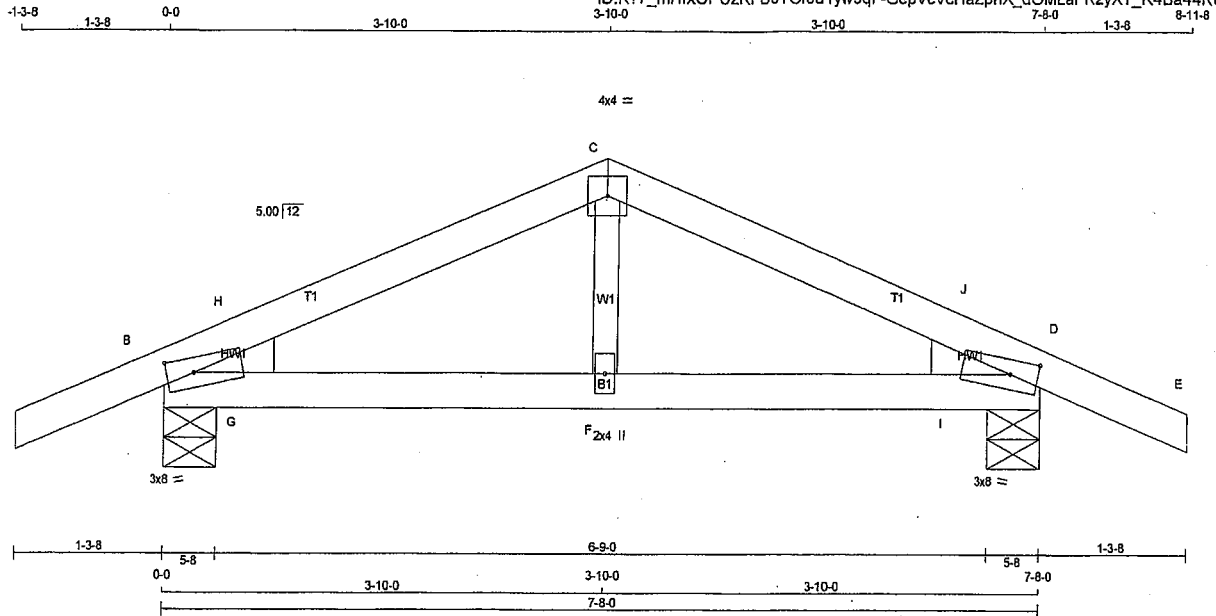
ENGINEER TAM T1900060
STRUCTURAL
CORPORATION ONLY 42

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES-BRAMPTON	DRWG NO.
402477	T90	9	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Mon Apr 29 15:03:40 2019 Page 1

ID:K77_mHxQPuzKFD0TOr6u1yw9qF-GepVcVcHaZpnX_dOMLaPK2yXT_K4Ba44RUPqPEzLqtn



Scale = 1:18.5

TOTAL WEIGHT = 9 X 24 = 220 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
B - D	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
	DRY: SEASONED LUMBER.				

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMBH1-m	MT20	3.0	8.0	1.50	2.75
C	TTW-p	MT20	4.0	4.0		
D	TMBH1-m	MT20	3.0	8.0	1.50	2.75
F	BMW+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD	HEEL
	GROSS REACTION	GROSS REACTION	BRG	BRG	WEDGE
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
B	587	0	587	0	0
D	587	0	587	0	0

UNFACTORED REACTIONS

1ST LOASE	MAX/MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	438	241 / 0	81 / 0	0 / 0	0 / 0	118 / 0	0 / 0
D	438	241 / 0	81 / 0	0 / 0	0 / 0	118 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX	MEMB.	MAX. FACTORED
MEMB.	FORCE	VERT. LOAD	LC1	MAX	MAX	MEMB.	FORCE	MAX
	(LBS)	(PLF)	(LBS)	(LBS)	(LBS)		(LBS)	(LBS)
FR-TO		FROM	TO	LENGTH	FR-TO			
A-B	0 / 15	-84.9	-84.9	0.11 (1)	10.00	F-C	0 / 229	0.05 (2)
B-H	-649 / 0	-84.9	-84.9	0.03 (1)	6.25	G-H	-53 / 70	0.00 (1)
H-C	-598 / 0	-84.9	-84.9	0.12 (1)	6.25	I-J	-53 / 70	0.00 (1)
C-J	-598 / 0	-84.9	-84.9	0.12 (1)	6.25			
J-D	-649 / 0	-84.9	-84.9	0.03 (1)	6.25			
D-E	0 / 15	-84.9	-84.9	0.11 (1)	10.00			
B-G	0 / 549	-38.5	-38.5	0.17 (1)	10.00			
G-F	0 / 549	-38.5	-38.5	0.20 (1)	10.00			
F-I	0 / 549	-38.5	-38.5	0.20 (1)	10.00			
I-D	0 / 549	-38.5	-38.5	0.17 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.26")
CALCULATED VERT. DEFL. (LL) = L/999 (0.01")
ALLOWABLE DEFL. (TL) = L/360 (0.26")
CALCULATED VERT. DEFL. (TL) = L/999 (0.02")

CSI: TC=0.12/1.00 (C-H:1), BC=0.20/1.00 (F-I:1), WB=0.05/1.00 (C-F:2), SSI=0.10/1.00 (C-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

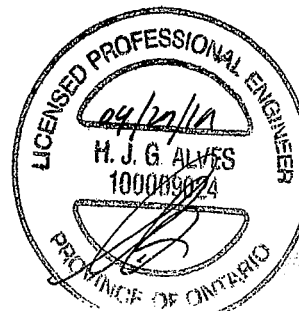
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)
MAX	MIN	MAX	MIN
MT20	618	354	1667
		788	1987
			1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.34 (B) (INPUT = 0.90)
JSI METAL= 0.12 (B) (INPUT = 1.00)

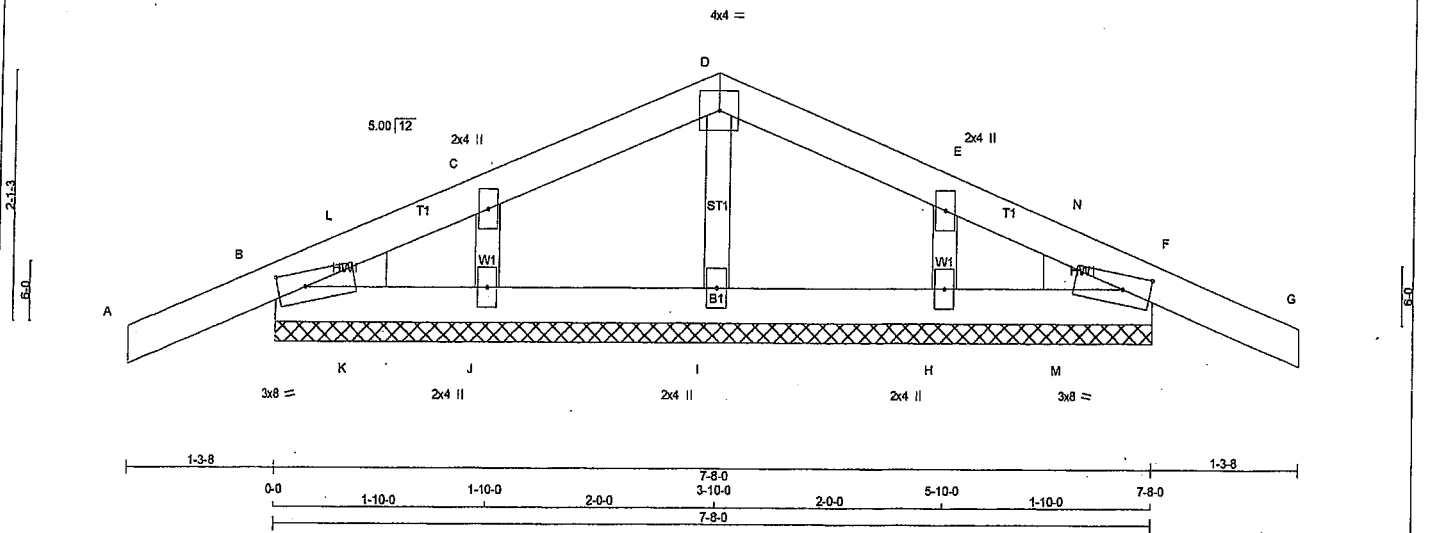


DRWG NO. TAM 19009772
STRUCTURAL
COMPONENT ONLY

JOB NAME 402477	TRUSS NAME G90	QUANTITY 3	PLY 1	JOB DESC. ROYAL PINE HOMES-BRAMPTON	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.		Version 8.230 S Nov 17 2018 Mittek Industries, Inc. Mon Apr 29 15:03:39 2019 Page 1	

ID:K77_mHxOPUzKFD0TOr6u1yw9qF-orF7P9cfpFhwvq2Coe2AnqPMtb1US7FxQqfHtozLqto

Scale = 1:18.5



TOTAL WEIGHT = 3 X 26 = 77 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				
GABLE STUDS SPACED AT 2-0-0 OC.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-m	MT20	3.0	8.0	1.50	2.75
C	TMW+w	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TMBH1-m	MT20	3.0	8.0	1.50	2.75
H, I, J						
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

HEEL
WEDGE
2x4 L
2x4 R
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0/15	-84.9 -84.9	0.11 (1)	10.00	I-D	-152/0	0.02 (1)
B-L	-85/0	-84.9 -84.9	0.10 (1)	6.25	J-C	-173/0	0.02 (1)
L-C	-1/10	-84.9 -84.9	0.04 (1)	10.00	H-E	-173/0	0.02 (1)
C-D	-5/4	-84.9 -84.9	0.04 (1)	10.00	K-L	0/66	0.00 (1)
D-E	-5/4	-84.9 -84.9	0.04 (1)	10.00	M-N	0/66	0.00 (1)
E-N	-1/10	-84.9 -84.9	0.04 (1)	10.00			
N-F	-85/0	-84.9 -84.9	0.10 (1)	6.25			
F-G	0/15	-84.9 -84.9	0.11 (1)	10.00			
B-K	0/6	-38.5 -38.5	0.02 (1)	10.00			
K-J	0/6	-38.5 -38.5	0.02 (3)	10.00			
J-I	-8/3	-38.5 -38.5	0.03 (2)	10.00			
I-H	-8/3	-38.5 -38.5	0.03 (2)	10.00			
H-M	0/6	-38.5 -38.5	0.02 (3)	10.00			
M-F	0/6	-38.5 -38.5	0.02 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF
DL = 6.0 PSF

BOT CH. LL = 10.5 PSF
DL = 7.0 PSF

TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11/1.00 (A-B:1), BC=0.03/1.00 (H-I:2), WB=0.02/1.00 (C-J:1), SSI=0.09/1.00 (B-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

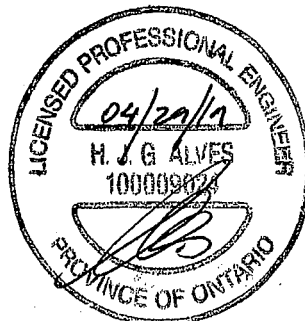
NAIL VALUES

PLATE GRIP(DRY) SHEAR	SECTION
(PSI)	(PLI)
MAX MIN	MAX MIN
MT20	618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.10 (E) (INPUT = 0.90)
JSI METAL= 0.06 (E) (INPUT = 1.00)



DRWG NO. 1AM 1700972
STRUCTURAL
COMPONENT ONLY

JOB NAME 300526	TRUSS NAME T140	QUANTITY 2	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
---------------------------	---------------------------	----------------------	-----------------	--------------------------	----------

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MITek Industries, Inc. Tue Jan 15 10:45:27 2019 Page 1
ID:4_RMDqbk1OGI3fXnIRj7uyvEbu-Xwrfh_1joJ1gHh2XsdAdEVuoJ2v0Bs8DOPkizv8pc
Scale = 1:52.5

L.G.A. RULES	SIZE	LUMBER	DESCR.
- D	2x4	DRY No.2	SPF
- D	2x4	DRY No.2	SPF
- B	2x4	DRY No.2	SPF
- E	2x4	DRY No.2	SPF
L.WEBS	2x3	DRY No.2	SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				
<u>BEARINGS</u>				
JT	VERT	HORZ	GROSS REACTION	MAXIMUM FACTORED BRG
E	465	0	465	0
G	584	0	584	0

UNFACTORED REACTIONS							
JT	1ST CASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	353	178 / 0	79 / 0	0 / 0	0 / 0	98 / 0	0 / 0
G	435	241 / 0	79 / 0	0 / 0	0 / 0	115 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MAX UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX LC1 (LC)
A-B	0 / 42	-84.9	-84.9 0.12 (1)	10.00	F-C	0 / 186	0.04 (3)
B-C	-263 / 0	-84.9	-84.9 0.16 (1)	6.25	C-E	-354 / 0	0.26 (1)
C-D	-26 / 0	-84.9	-84.9 0.16 (1)	6.25	B-F	0 / 221	0.05 (1)
D-E	-121 / 0	0.0	0.0 0.05 (1)	6.25			
E-G	-527 / 0	0.0	0.0 0.06 (1)	7.81			
G-F	0 / 0	-38.5	-38.5 0.12 (3)	10.00			
F-E	0 / 205	-38.5	-38.5 0.14 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015.

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012
- CSA 085-09, CSA 089-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = 1/360 (0.25")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.01")
ALLOWABLE DEFL.(TL) = 1/360 (0.25")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.01")

CSI: TC=0.16/1.00 (B-C:1), BC=0.14/1.00 (E-F:2), WB=0.28/1.00 (C-E:1), SS=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE-LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLJ)
MT20 618 354 1687 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (B) (INPUT = 0.90)
JSI METAL= 0.14 (B) (INPUT = 1.00)

DRWG NO. TAM **77901091**
STRUCTURAL
FOR QUOTE ONLY

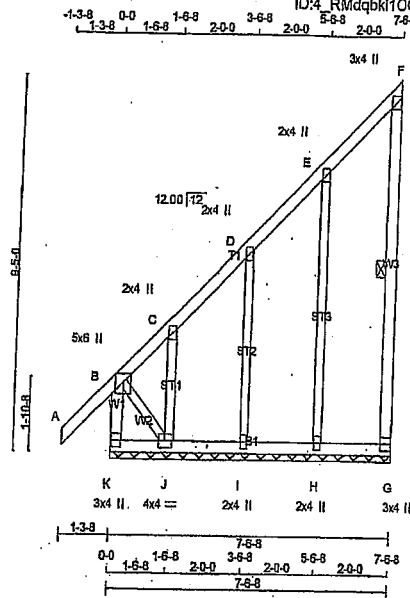
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300526	G140	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Tue Jan 15 10:45:26 2019 Page 1

ID:4_RMdqbK10G13fXnfRj7uyvEbu-2kHHUe0510vpXlrzDL0ePIMjP7XAc9jwZtGCGzv8pd

Scale = 1:53.2



LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
A - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - G	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2
ALL GABLE WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 24-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW+p	MT20	5.0	6.0	Edge	
C, D, E						
C	TMW+w	MT20	2.0	4.0		
F	TMW+p	MT20	3.0	4.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMV1+w	MT20	2.0	4.0		
I	BMV1+w	MT20	2.0	4.0		
J	BMVW1-t	MT20	4.0	4.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.
1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MAX. FACTORED		FACTORED		MAX. FACTORED		MAX. FACTORED		
MEMB.	FORCE	VERT. LOAD	LC1 MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PLF)	CS1 (LC)	UNBRAC		(LBS)	CS1 (LC)	
FR-TO		FROM TO		LENGTH	FR-TO			
A-B	0/42	-84.9	-84.9	0.12 (1)	10.00	H-E	-189/0	0.18 (1)
B-C	-49/0	-84.9	-84.9	0.11 (1)	6.25	I-D	-175/0	0.08 (1)
C-D	-2/0	-84.9	-84.9	0.04 (1)	10.00	J-C	-82/0	0.02 (1)
D-E	-3/0	-84.9	-84.9	0.05 (1)	10.00	B-J	0/13	0.00 (1)
E-F	-11/0	-84.9	-84.9	0.05 (1)	6.25			
F-G	-73/0	0.0	0.0	0.03 (1)	6.25			
K-B	-249/0	0.0	0.0	0.03 (1)	7.81			
K-J	0/0	-38.5	-38.5	0.02 (3)	10.00			
J-I	0/6	-38.5	-38.5	0.02 (3)	10.00			
I-H	0/3	-38.5	-38.5	0.03 (3)	10.00			
H-G	0/0	-38.5	-38.5	0.03 (3)	10.00			

TOTAL WEIGHT = 2 X 48 = 96 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN. O.C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(65 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CS1: TC=0.12/1.00 (A-B-1), BC=0.03/1.00 (H-I-3), WB=0.18/1.00 (E-H-1), SS=0.07/1.00 (A-B-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

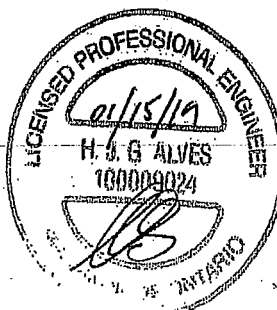
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 768 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (E) (INPUT = 0.90)
JSI METAL= 0.10 (E) (INPUT = 1.00)



DRWG NO. TAM 11901092
STRUCTURAL
COMPONENT ONLY

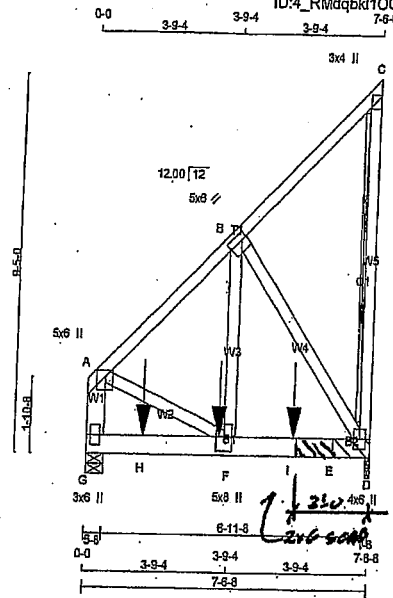
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300526	T141	2	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Tue Jan 15 10:45:28 2019 Page 1

ID: 4_RMdqbkl10G13fXnfrJ7uyvEbu-76P1vK2LZd9XxsE5dOsJqnhlCV0elQONt8NH9zv8pb

Scale = 1:52.8



TOTAL WEIGHT = 4 X 57 = 228 lb

LUMBER					
N. L. G. A. RULES					
CHORDS		SIZE	LUMBER		DESCR.
A - C	2x4	DRY	No.2	SPF	
D - C	2x4	DRY	No.2	SPF	
G - A	2x6	DRY	No.2	SPF	
G - D	2x6	DRY	No.2	SPF	
ALL WEBS		2x4	DRY	No.2	SPF
FRCFR					

ALL WEBS 2x4 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	1	12
C - D	1	12
G - A	2	12
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
G - D	2	12
WEBS : (0.122"x3") SPIRAL NAILS		
B - F	1	6
2x4	1	6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLYS FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
A	TMVW+P	MT20	5.0	6.0	1.75	2.75
B	TMVW+P	MT20	6.0	6.0	2.00	1.50
C	TMV+P	MT20	3.0	4.0		
D	BMVW+P	MT20	4.0	6.0		
F	BMVW+P	MT20	5.0	8.0	4.25	2.50
G	BMV+P	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	UP/LT	IN-SX
D	3997	0	0	1-8
G	4382	0	0	5-8

UNFACTORED REACTIONS	1ST LOASE	MAX/MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
D	3013	1558/0 632/0 0/0 0/0 823/0 0/0
G	3302	1708/0 692/0 0/0 0/0 902/0 0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, G

2x6 DRY SPF No.2 BEARING BLOCK 24" LONG AT JT. D ATTACHED TO FRONT SIDE WITH 3 ROWS OF (0.122"x3") SPIRAL NAILS SPACED 3" C.C. 22 NAILS TOTAL.

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.13 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT C-D

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	MAX. FACTORED	WEBS	MAX. FACTORED	FACTORED	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	MEMB.	FORCE (LBS)	MAX. FACTORED (LBS)	MAX. FACTORED (LC)
FR-TO		FROM TO		FR-TO			
A-B	-3103/0	-84.9	-84.9 0.15 (1)	5.13	F-B	0/4879	0.43 (1)
B-C	-24/0	-84.9	-84.9 0.10 (1)	8.25	B-D	-3772/0	0.97 (1)
C-D	-125/0	0.0	0.0 0.04 (1)	7.81	A-F	0/2367	0.21 (1)
G-A	-3181/0	0.0	0.0 0.12 (1)	7.75			
G-H	0/0	-38.5	-38.5 0.64 (1)	10.00			
H-F	0/0	-38.5	-38.5 0.64 (1)	10.00			
F-I	0/2212	-38.5	-38.5 0.73 (1)	10.00			
I-E	0/2212	-38.5	-38.5 0.73 (1)	10.00			
E-D	0/2212	-38.5	-38.5 0.73 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)	JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	3-5-12	-2483	-2483			BACK	VERT	TOTAL		
H	1-5-12	-2483	-2483			BACK	VERT	TOTAL		
I	5-5-12	-2483	-2483			BACK	VERT	TOTAL		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 088-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.25")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.25")
CALCULATED VERT. DEFL.(TL) = L/999 (0.08")

CSI: TC=0.15/1.00 (A-B:1), BC=0.73/1.00 (D-F:1),
WB=0.97/1.00 (B-D:1), SS=0.60/1.00 (D-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

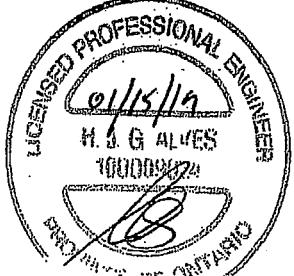
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1657 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (B) (INPUT = 0.90)
JSI METAL= 0.53 (F) (INPUT = 1.00)



DRWG NO. TAM 1701090
STRUCTURAL
COMPANION ONLY

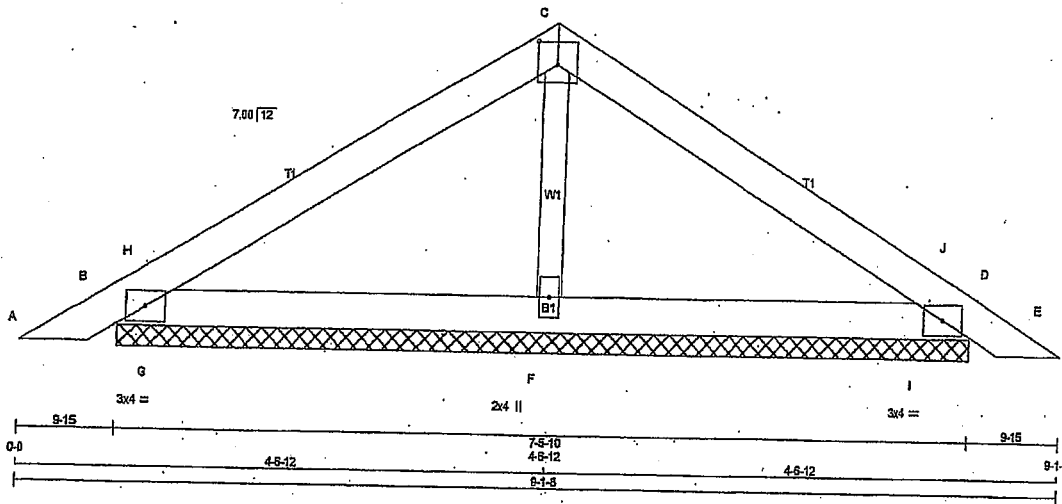
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300539	P1	6	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 8 Nov 17 2018 MITek Industries, Inc. Thu Jan 3 12:17:47 2019 Page 1

ID:K77_mHxOPUzKFD0TO6u1yw9qF-SR5qqPJW_e8S17J74uNKukaWjdsyTmQHW44J6zz4b2

0-0 45-12 45-12 45-12 9-18
4x4 =
Scale = 1:17.2



TOTAL WEIGHT = 6 X 22 = 132 lb

LUMBER

N.L.G.A. RULES

CHORDS	SIZE	DRY	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
B - D	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

DESCR.
SPF
SPF
SPF
SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
B	322	0	322	0	0	7-5-10	7-5-10
D	322	0	322	0	0	7-5-10	7-5-10
F	404	0	404	0	0	7-5-10	7-5-10

UNFACTORED REACTIONS

1ST CASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	237	141/0	36/0	0/0	0/0	60/0	0/0
D	237	141/0	36/0	0/0	0/0	60/0	0/0
F	313	136/0	65/0	0/0	0/0	92/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD		MAX. FACTORED UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
		FROM	TO					
FR-TO						FR-TO		
A-B	0/15	-84.9	-84.9	0.04 (1)	10.00	F-C	-156/0	0.02 (1)
B-H	-75/0	-84.9	-84.9	0.04 (1)	8.25	G-H	-246/24	0.00 (1)
H-C	-116/0	-84.9	-84.9	0.15 (1)	6.25	I-J	-246/24	0.00 (1)
C-J	-116/0	-84.9	-84.9	0.15 (1)	8.25			
J-D	-75/0	-84.9	-84.9	0.04 (1)	6.25			
D-E	0/15	-84.9	-84.9	0.04 (1)	10.00			
B-G	0/97	-38.5	-38.5	0.14 (1)	10.00			
G-F	0/97	-38.5	-38.5	0.16 (1)	10.00			
F-I	0/97	-38.5	-38.5	0.16 (1)	10.00			
I-D	0/97	-38.5	-38.5	0.14 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.15/1.00 (C-J:1), BC=0.16/1.00 (F-I:1), WB=0.02/1.00 (C-F:1), SS=0.21/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

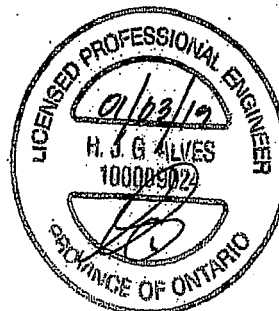
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (D) (INPUT = 0.90)
JSI METAL= 0.05 (D) (INPUT = 1.00)



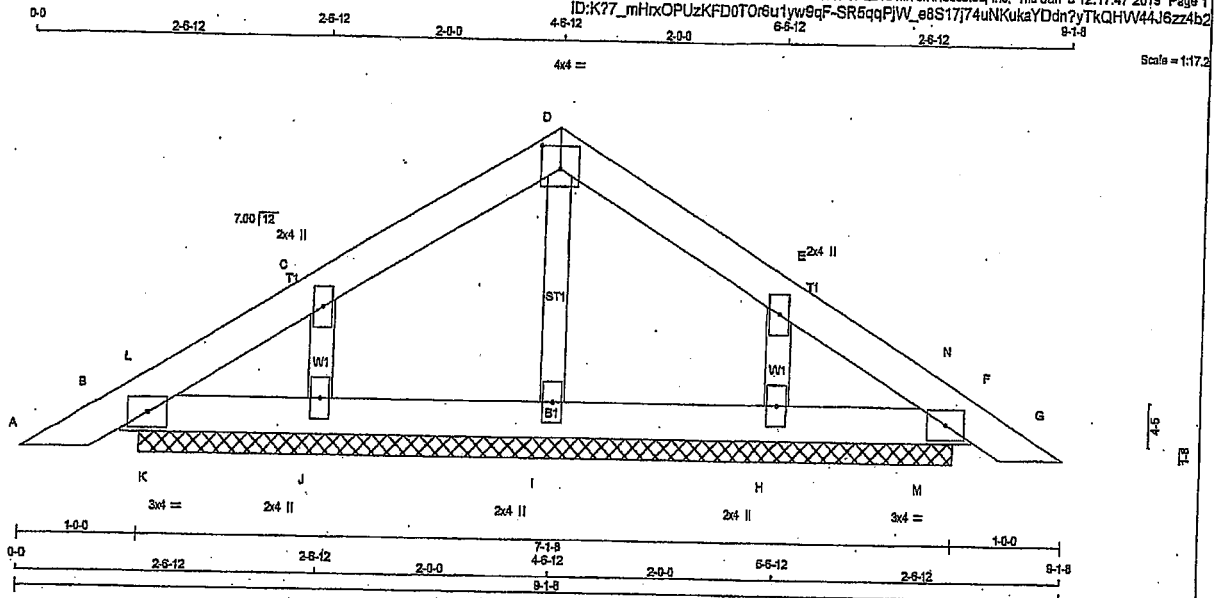
ENGR. NO. TAM 71900063
STRUCTURAL
CONSULTANT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300539	P1G	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Thu Jan 3 12:17:47 2019 Page 1

ID:K77_mHixOPuzKFD0T0r6u1yw8qF-SR5qqPJW_e8S17174uNKukaYDdn?yTKQHw44J6zz4b2



Scale = 1:17.2

LUMBER	N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS				
A - D	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

GABLE STUDS SPACED AT 2'-0" OC.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMB1-I	MT20	3.0	4.0		
C TMW1-w	MT20	2.0	4.0		
D TTW-p	MT20	4.0	4.0	2.25	2.00
E TMW1-w	MT20	2.0	4.0		
F TMB1-I	MT20	3.0	4.0		
H, I, J					
H BMW1-w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	LC2 MAX	UNBRACED LENGTH PR-TO	WEBS MEMB. MAX. FACTORED FORCE (LBS)	MAX CS1 (L.C)
FR-TO							
A-B	0/15	-84.9	-84.9	0.04 (1)	10.00	I-D	-118/0
B-L	-53/0	-84.9	-84.9	0.01 (2)	6.25	J-C	-188/0
L-C	-21/0	-84.9	-84.9	0.05 (1)	6.25	H-E	-188/0
C-D	-31/0	-84.9	-84.9	0.05 (1)	6.25	K-L	-11/17
D-E	-31/0	-84.9	-84.9	0.05 (1)	6.25	M-N	-11/17
E-N	-21/0	-84.9	-84.9	0.05 (1)	6.25		
N-F	-53/0	-84.9	-84.9	0.01 (2)	6.25		
F-G	0/15	-84.9	-84.9	0.04 (1)	10.00		
B-K	0/33	-38.5	-38.5	0.02 (1)	10.00		
K-J	0/33	-38.5	-38.5	0.03 (2)	10.00		
J-I	0/19	-38.5	-38.5	0.03 (2)	10.00		
I-H	0/19	-38.5	-38.5	0.03 (2)	10.00		
H-M	0/33	-38.5	-38.5	0.03 (2)	10.00		
M-F	0/33	-38.5	-38.5	0.02 (1)	10.00		

TOTAL WEIGHT = 2 X 23 = 47 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF

DL = 6.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CS1: TC=0.05/1.00 (D-E:1), SC=0.03/1.00 (J-K:2), WB=0.03/1.00 (E-H:1), SS=0.07/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

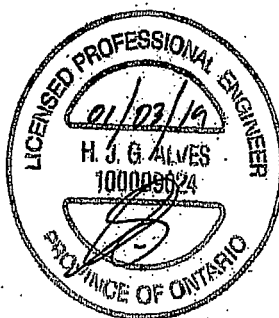
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(FBI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.12 (C) (INPUT = 0.90)
JSI METAL= 0.09 (C) (INPUT = 1.00)



DRWG NO. TAM 71902064
STRUCTURAL
DESIGNER

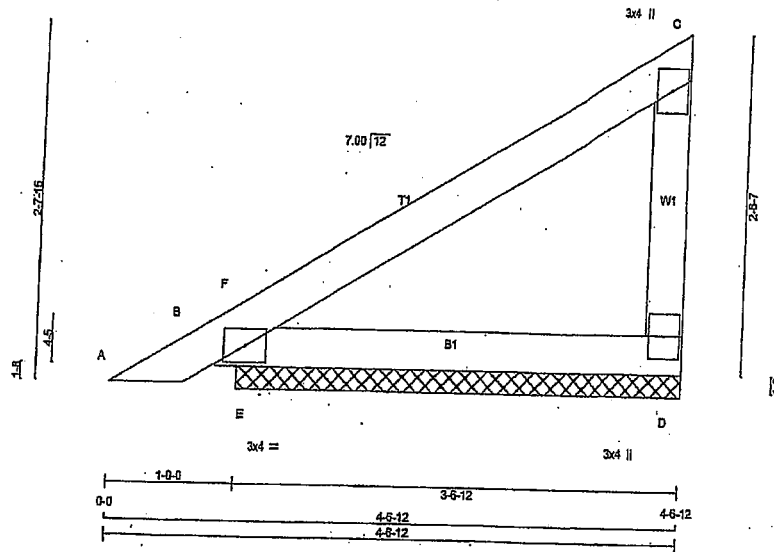
JOB NAME 300536	TRUSS NAME P2G	QUANTITY 1	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
--------------------	-------------------	---------------	----------	--------------------------	----------

Tamarack Roof Truss, Burlington

Version 6.230 S Nov 17 2018 MITek Industries, Inc. Thu Jan 3 11:24:57 2019 Page 1

ID:K77_mHxOPuzKFD0T0r6u1yw9qF-HaYRWTL5WfmEjzOn8VyQFHnRnqDnNpKJAK1rzz5Ma

Scale = 1:15.7



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
D - C	2x4	DRY	No.2
B - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 24-0 OC.

PLATES (table is in inches)				
PLATE	TYPE	W	LEN	Y
B	TMB1-1	MT20	3.0	4.0
C	TMB1-1	MT20	3.0	4.0
D	TMB1-1	MT20	3.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	MEMB.
A-B	0/15	-84.9	0.04 (1)	10.00	E-F	-165/86	0.00 (1)
B-F	-60/4	-84.9	0.05 (3)	6.25			
F-C	0/5	-84.9	0.16 (1)	10.00			
D-C	-143/0	0.0	0.02 (1)	7.81			
B-E	0/0	-38.5	0.10 (1)	10.00			
E-D	0/0	-38.5	0.13 (1)	10.00			

TOTAL WEIGHT = 12 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
BOT CH.	LL = 6.0	PSF
	DL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 46.8	PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2010, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.16/1.00 (C-F:1), BC=0.13/1.00 (D-E:1), WB=0.00/1.00 (E-F:1), SS=0.17/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

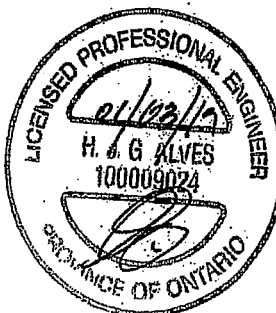
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 788	1987 1658

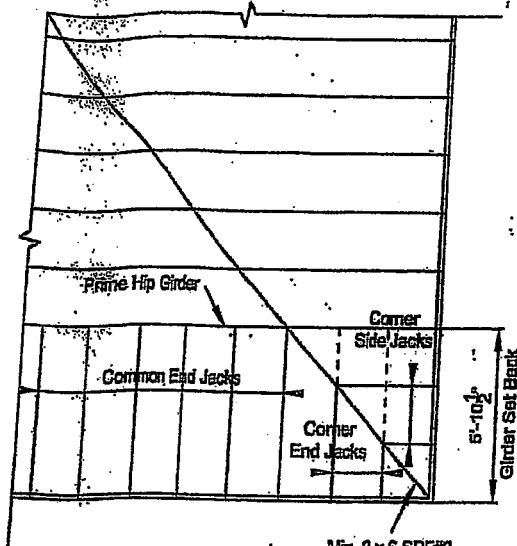
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.20 (B) (INPUT = 0.80)
JSI METAL= 0.04 (B) (INPUT = 1.00)



DRWG NO. TAM 11900056
STRUCTURAL
COMPONENT ONLY



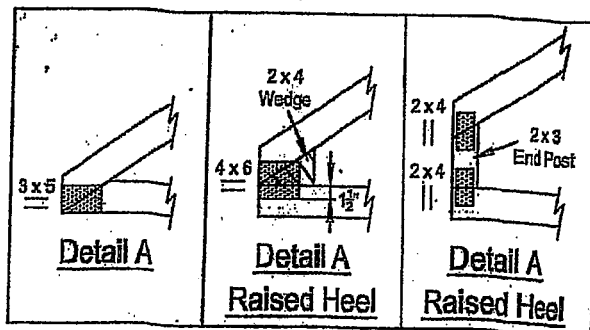
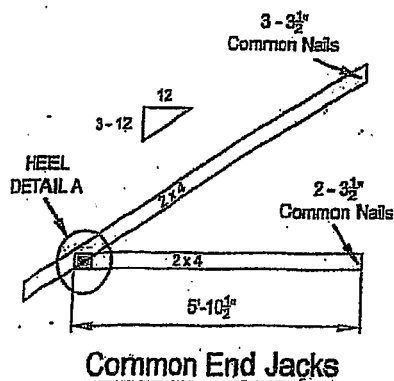
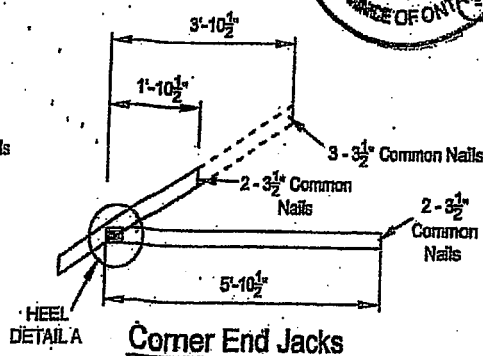
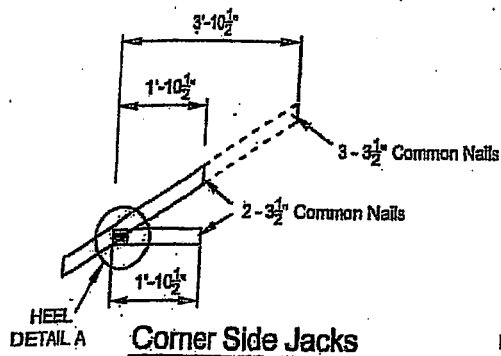
45° Hip End

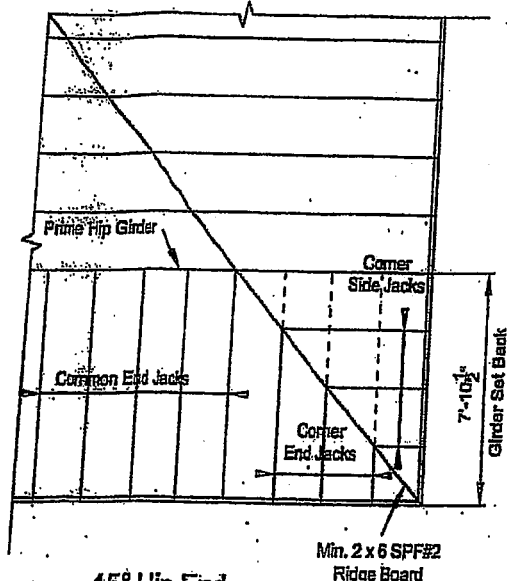
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.
TOTAL LOAD : 50.5 P.S.F.





45° Hip End

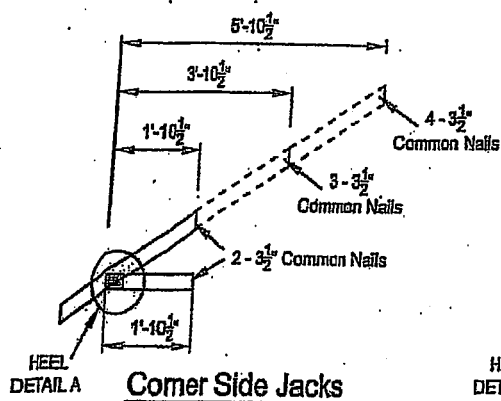
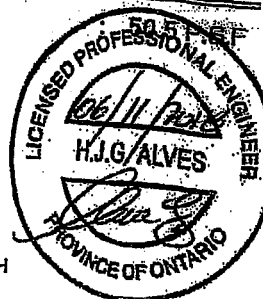
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

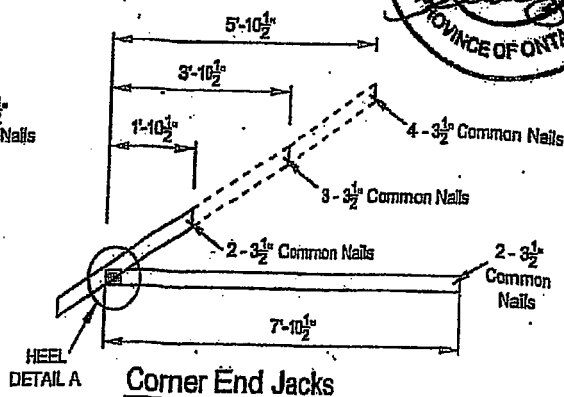
DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

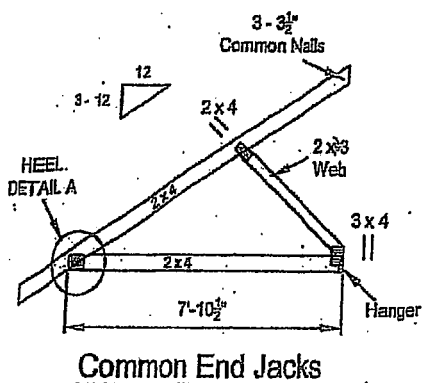
TOTAL LOAD



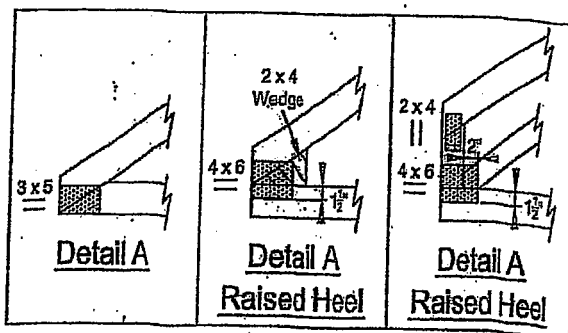
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800217

LUS - Double Shear Joist Hangers



All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

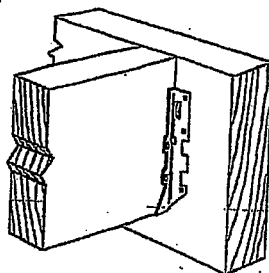
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

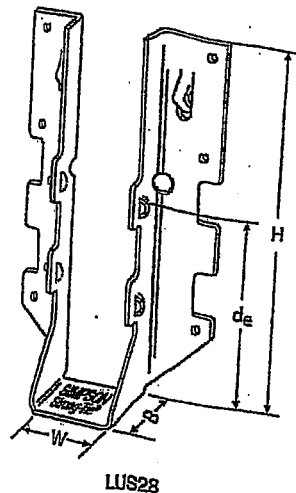
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified



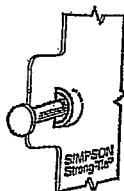
Typical LUS Installation



LUS28

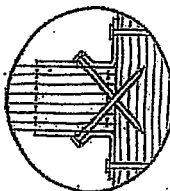
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.F.R.-L		S.P.F.	
		W	H	B	d ₁ ¹	Face	Joist	Uplift (K ₁ =1.15)	Normal (K ₂ =1.00)	Uplift (K ₁ =1.15)	Normal (K ₂ =1.00)
LUS24	18	1 1/2	3 1/2	1 1/2	1 1/2	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3 1/2	3 1/2	2	1 1/2	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1 1/2	4 1/4	1 1/4	3 3/4	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3 1/2	4 1/4	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4 1/2	4 1/4	2	3 1/4	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 1/2	6 1/2	1 1/4	3 3/4	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3 1/2	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4 1/2	6 1/4	2	3 1/4	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 1/2	7 1/2	1 1/4	3 3/4	(6) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3 1/2	9	2	6	(6) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4 1/2	8 1/4	2	5 1/4	(6) 16d	(6) 16d	2580	3345	2320	2375

1. d₁ is the distance from the seat of the hanger to the highest joist nail.

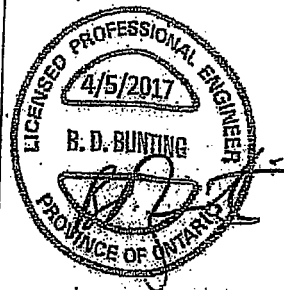


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Top View.



HMT
STATS
DESIGN

Information on the design of these hangers is available in the Simpson Strong-Tie catalog. For more information, contact your local distributor or Simpson Strong-Tie Company. Simpson Strong-Tie Company is a registered provider of continuing education for architects, engineers, and contractors.

(800) 999-5099
strongtie.com

HUS/LJS - Double Shear Joist Hangers



All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

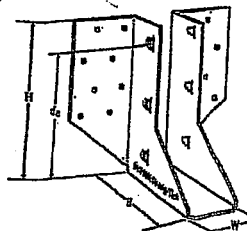
- Factored resistances are in accordance with CSA 086-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

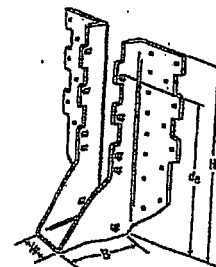
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

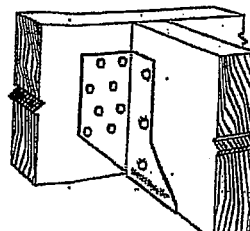
- See current catalogue for options



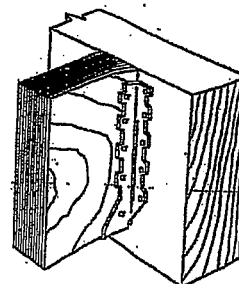
LJS26DS



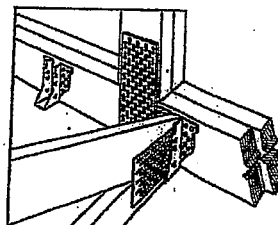
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



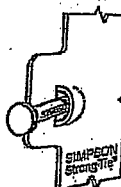
Typical HUS
Installation



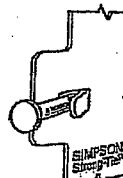
Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

Model No.	Ga.	Dimensions (in.)					Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e	Face	Joist		D.F.T-1		S.F.F	
									Uplift (K _u =1.15)	Normal (K _u =1.00)	Uplift (K _u =1.15)	Normal (K _u =1.00)
LJS26DS	18	1 1/16	5	3 1/2	4 1/4	(16) 16d	(8) 16d		2055	4265	1490	4115
HUS26	16	1 5/8	5 1/2	3	3 3/4	(14) 16d	(8) 16d		2705	4940	2065	3875
HUS28	16	1 5/8	7 3/16	3	6 3/16	(22) 16d	(8) 16d		3605	5365	2875	4345
HUS210	16	1 5/8	9 1/2	3	7 3/16	(30) 16d	(10) 16d		4505	5785	4010	4740
HUS181/10	16	1 1/4	9	3	8	(30) 16d	(10) 16d		4505	6450	4010	5200

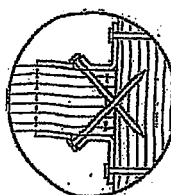
1. d_e is the distance from the seat of the hanger to the highest joist nail.



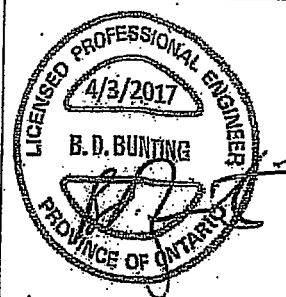
Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).
U.S. Patent
5,603,580



Double
Shear
Nailing
Side View.
Do not
bend tab
back.



Double
Shear
Nailing
Top View.



IMF
DESIGN

For more information, visit us online at www.simpsonstrongtie.com
or call 1-800-999-5099
SIMPSON Strong-Tie

(800) 999-5099
strongtie.com

HGUS - Double Shear Joist Hangers



All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

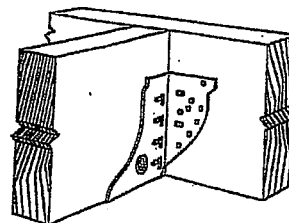
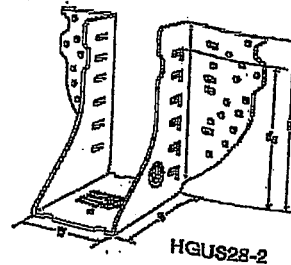
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%.
- No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

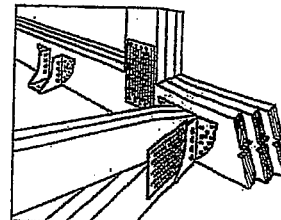
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

- See current catalogue for options



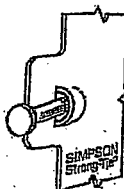
Typical HGUS Installation



Typical HGUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

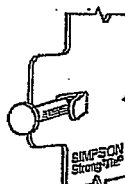
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _g ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _u =1.15)	Normal (K _p =1.00)	Uplift (K _u =1.15)	Normal (K _p =1.00)
HGUS26	12	1 1/2"	5 1/2"	5	4 3/8"	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3 3/4"	5 7/8"	4	4 1/4"	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4 1/4"	5 1/2"	4	4 1/4"	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6 3/8"	5 7/8"	4	4 1/4"	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1 1/2"	7 1/4"	5	6 1/8"	(36) 16d	(12) 16d	3310	7675	3100	6500
HGUS28-2	12	3 3/4"	7 1/8"	4	6 1/4"	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4 1/4"	7 1/4"	4	6 1/4"	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6 3/8"	7 1/8"	4	6 1/4"	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210-2	12	3 3/4"	9 3/8"	4	8 1/4"	(45) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-3	12	4 1/4"	9 1/4"	4	8 1/4"	(45) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6 3/8"	9 3/8"	4	8 1/4"	(45) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6 3/8"	10 3/4"	4	10 1/4"	(58) 16d	(20) 16d	7840	14985	5425	10845
HGUS214-4	12	6 3/8"	12 3/4"	4	11 1/4"	(66) 16d	(22) 16d	10130	16400	7195	11645

1. d_g is the distance from the seat of the hanger to the highest joist nail.

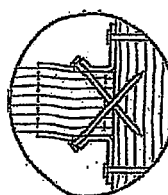


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

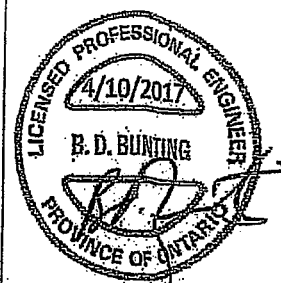
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



DESIGN

(800) 999-5099
strongtie.com

Symbols

PLATE LOCATION AND ORIENTATION



Central plate on joint unless x-y offset is shown. Indicate dimensions in ft-in-kleantits or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 X 2 orientation, locate plates 0-1/4" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

*Plate location details available in Mittek software or upon request.

PLATE SIZE

4 X 4

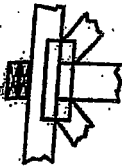
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use L, I or Eliminator bracing if indicated.

BEARING



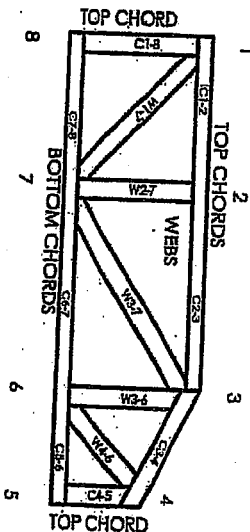
Indicates location where bearings (supports) occur. Icons vary but number where bearings occur.

Indicate the location of bearings for light metal trusses and specifications for light metal trusses and specifications for light metal trusses and specifications for light metal trusses.

DSB-89: Design Standard for Bracing, Building Component Safety Information, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-kleantits or mm (drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.
CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:
11996-L, 10819-L, 13270-L, 12691-R

© 2007 Mittek® All Rights Reserved

Mittek
POWER TO PERFORM™

Mittek Engineering Reference Sheet Mittek-7429C rev. 10-08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral bracing themselves may require bracing, or alternative L, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and warps at joint locations are regulated by TFC.
7. Design assumes trusses will be suitably protected from the environment in accord with TFC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with the relevant preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of the fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or pulins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10' ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (text, drawings, and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TFC Quality Criteria.

H - Seismic and Hurricane Ties

SIMPSON
Strong-Tie

The H connector series provides wind and seismic ties for trusses and rafters.

Material: 18 gauge **Finish:** G90 galvanized

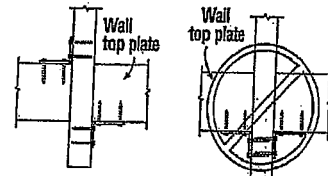
Design: • Factored resistances are in accordance with CSA O86-14
• Factored resistances have been increased 15%. No further increase is permitted.

Installation: • Use all specified fasteners

- Nails: 8d = 0.131" dia. x 2½" long common wire, 8d x 1½" = 0.131" x 1½" long, 10d x 1½" = 0.146" x 1½" long
- H1 can be installed with flanges facing outwards
- Hurricane ties do not replace solid blocking

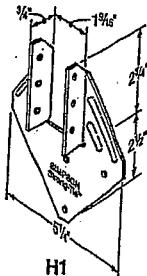
Factored resistances for more than one direction for a single connection cannot be added together. A factored load which can be divided into components in the directions given must be evaluated as follows: Factored Shear/Resisting Shear + Factored Tension/Resisting Tension ≤ 1.0.

Hurricane Tie Installations to Achieve Twice the Load (Top View)

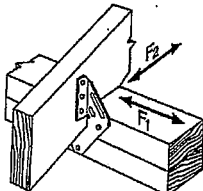


Install diagonally across from each other for minimum 2x truss.

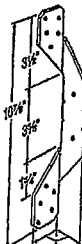
Nailing into both sides of a single ply 2x truss may cause the wood to split.



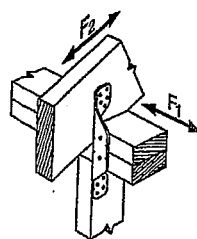
H1



H1 Installation



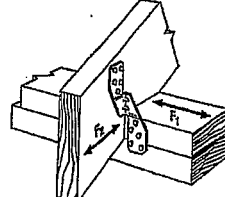
H2A



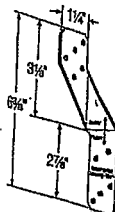
H2A Installation



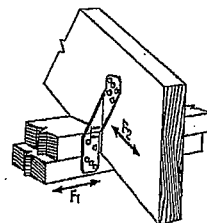
H2.5A



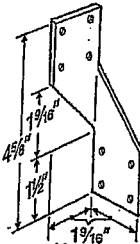
H2.5A Installation



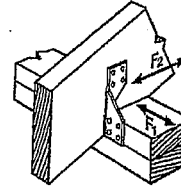
H2.5T



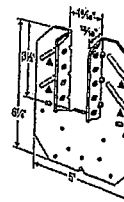
H2.5T Installation
(Nails into both top plates)



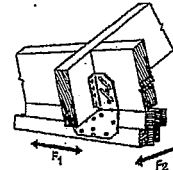
H3



H3 Installation



H10A



H10A Installation

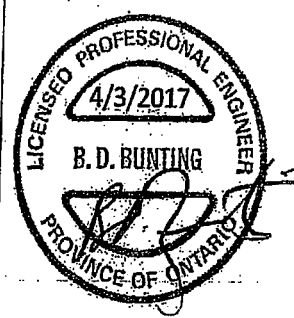
Model No.	Ga.	Fasteners			Factored Resistance (lb.)					
					D.Fir-L			S-P-F		
		To Rafter	To Plates	To Studs	Uplift	Normal		Uplift	Normal	
						F ₁	F ₂		F ₁	F ₂
H1	18	(6) 8d x 1½"	(4) 8d	—	740	685	300	680	485	215
H2A	18	(5) 8d x 1½"	(2) 8d x 1½"	(5) 8d x 1½"	830	220	75	590	155	55
H2.5A	18	(5) 8d	(5) 8d	—	805	160	160	755	160	160
H2.5T	18	(5) 8d	(5) 8d	—	835	175	240	740	160	210
H3	18	(4) 8d	(4) 8d	—	740	180	285	615	125	190
H10A	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1735	795	410	1505	565	290

1. Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.

2. Factored resistances are for one anchor. A minimum rafter thickness of 2½" must be used when framing anchors are installed on each side of the joist and on the same side of the plate.

3. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.

4. Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a Continuous Load Path, connections must be on same side of the wall.



(800) 999-5099
strongtie.com

CP Crush Plates – Bearing Enhancers

SIMPSON
Strong-Tie

The CP transfers load from the truss or girder to plates for bearing limited conditions. Replaces nail-on scabs or in some cases, an additional ply when needed for bearing.

Material: See table

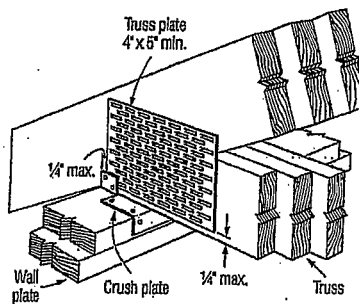
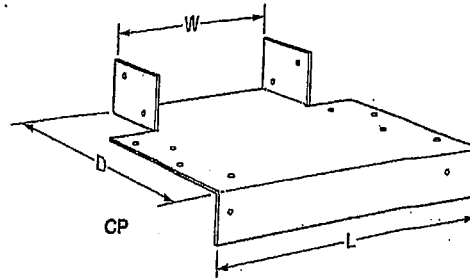
Finish: G90 galvanized

Design:

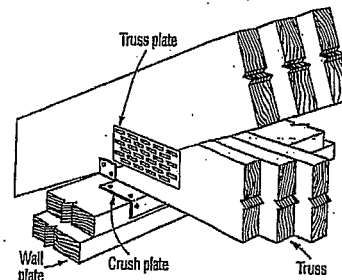
- Factored resistances are in accordance with CSA O86-14 assuming Q_u/A_p and $Q_u/A'_p = 812$ psi for D.Fir-L and 616 psi for S-P-F. See clauses 6.5.4 and 7.5.9 TPIC 2014 when compression loads are applied to both sides of truss chord members at bearing locations.

Installation:

- Use all specified fasteners.
- Nails: 10d = 0.148" dia. x 3" long common wire, 10d x 1½" = 0.148" dia. x 1½" long.
- For Case 1, truss plates must be located a maximum of ¼" from the underside of the truss chord and a maximum of ¼" from the edge of the wall plates in accordance with the reinforcing requirements of 7.5.9 TPIC 2014.



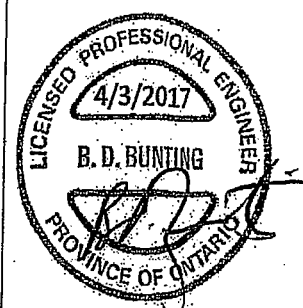
Case 1



Case 2

Model No.	Ga.	Wall Plate	Dimensions (in.)			Fasteners		Factored Resistance (lb.)				
			W	D	L	Wall Plate	Truss	D.Fir-L		S-P-F		
								Uplift	Bearing	Uplift	Bearing	
												(K _u =1.15)
CASE 1 (Truss Plate Reinforcement)												
CP1-4	20	2x4	1½	3½	4½	(6) 10d	(4) 10d x 1½"	225	5965	225	4515	
CP2-4	16		3¼	3½	5½	(6) 10d	(4) 10d x 1½"	225	11390	225	9030	
CP3-4	16		4¾	3½	7½	(6) 10d	(4) 10d x 1½"	225	17895	225	13545	
CP4-4	12		6½	3½	9½	(6) 10d	(4) 10d x 1½"	225	23860	225	18065	
CP1-6	20	2x6	1½	5½	4½	(10) 10d	(4) 10d x 1½"	225	9370	225	7095	
CP2-6	16		3¼	5½	5½	(10) 10d	(4) 10d x 1½"	225	18740	225	14190	
CP3-6	16		4¾	5½	7½	(10) 10d	(4) 10d x 1½"	225	28110	225	21285	
CP4-6	12		6½	5½	9½	(10) 10d	(4) 10d x 1½"	225	37495	225	28390	
CASE 2 (No Reinforcement)												
CP1-4	20	2x4	1½	3½	4½	(6) 10d	(4) 10d x 1½"	225	4685	225	3550	
CP2-4	16		3¼	3½	5½	(6) 10d	(4) 10d x 1½"	225	9370	225	7100	
CP3-4	16		4¾	3½	7½	(6) 10d	(4) 10d x 1½"	225	14055	225	10650	
CP4-4	12		6½	3½	9½	(6) 10d	(4) 10d x 1½"	225	18750	225	14195	
CP1-6	20	2x6	1½	5½	4½	(10) 10d	(4) 10d x 1½"	225	7365	225	5575	
CP2-6	16		3¼	5½	5½	(10) 10d	(4) 10d x 1½"	225	14730	225	11150	
CP3-6	16		4¾	5½	7½	(10) 10d	(4) 10d x 1½"	225	22095	225	16725	
CP4-6	12		6½	5½	9½	(10) 10d	(4) 10d x 1½"	225	29460	225	22305	

1. Factored bearing resistances assume wall plate and truss are the same species. For a mixed species system use S-P-F values.


LIMIT
STATES
DESIGN

This technical bulletin is effective until June 30, 2019, and reflects information available on March 1, 2017. This information is updated periodically and should be re-checked on and after June 30, 2017. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

© 2017 Simpson Strong-Tie Company Inc.

TSPEC/CP17-3/17 exp. 6/19

(800) 999-5099
 strongtie.com

TS/LTS/MTS/HTS Twist Straps

SIMPSON
Strong-Tie

Twist straps provide a tension connection between two wood members. They resist uplift at the heel of a truss economically. The 3" bend section eliminates interference at the transition points between wood members. TS twist straps come with an equal number of left and right hand units in each carton.

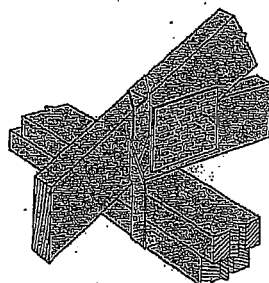
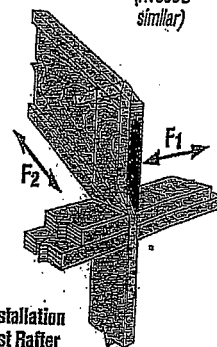
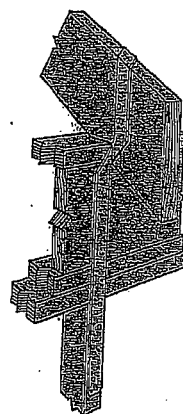
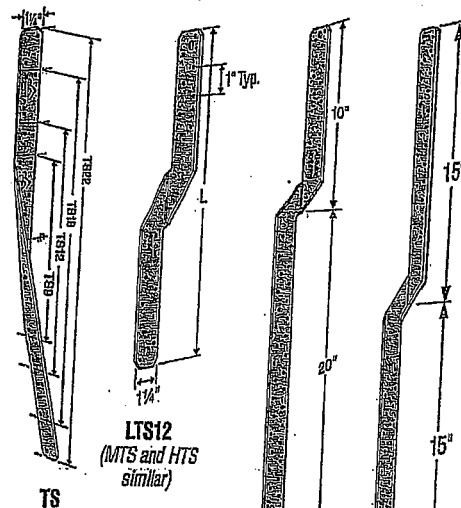
MATERIAL: LTS-18 gauge; MTS-16 gauge; HTS-14 gauge; TS-16 gauge
FINISH: Galvanized. Some products available in stainless steel and ZMAX®; see Corrosion Information, pages 14-17.

INSTALLATION: Use all specified fasteners. See General Notes.

- TS should be installed in pairs to reduce eccentricity.
- When LTS/MTS is installed as Truss-to-Top Plate tie, install 3-10d $\times$ 1½" nails to the w/s of the plate and 3-10d $\times$ 1½" nails into the edge of the double top plate.

These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.

These products are approved for installation with the Strong-Drive® SD Connector screw. See page 24 for more information.



Model No.	L (in)	Fasteners (Total)	Factored Resistance (k _n = 1.15)	
			lbs	lbs
TS9	9	8-16d	1125	1040
TS12	11	10-16d	1410	1300
TS18	17	14-16d	1970	1820
TS22	21	18-16d	2125	2125
MTS12	12	12-10d $\times$ 1½"	1015	720
MTS16	16	12-10d $\times$ 1½"	1015	720
MTS18	18	12-10d $\times$ 1½"	1015	720
MTS20	20	12-10d $\times$ 1½"	1015	720
MTS24	24	12-10d $\times$ 1½"	1015	720
MTS30	30	14-10d $\times$ 1½"	1570	1180
HTS16	16	16-10d $\times$ 1½"	2050	1455
HTS20	20	24-10d $\times$ 1½"	2050	1455
HTS24	24	24-10d $\times$ 1½"	2050	1455
HTS28	28	24-10d $\times$ 1½"	2050	1455

1. LTS12 thru LTS20, MTS16 through MTS30, HTS24 through HTS30C (except HTS30) have additional nail holes.
2. Install half of the fasteners on each end of strap to achieve maximum factored resistance.
3. Factored resistances have been increased 15% for earthquake or wind loading; no further increase allowed; reduce where other loads govern.
4. All straps except the MTS30 and HTS30 have the twist in the center of the strap.
5. Ties for rafter do not have to be installed.



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8- Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1800218

Feb 09, 2018

Schedule 1: Designer Information

Use one form for each individual who reviews and takes responsibility for design activities with respect to the project.

A. Project Information		Application number:	
Building number, street name		Unit no. UNITS 1 - 4	Lot/con. BLOCK 8
Municipality CITY OF BRAMPTON	Postal code	Plan number/ other description	
B. Individual who reviews and takes responsibility for design activities			
Name SAM KATSOULAKOS		Firm MICRO CITY ENGINEERING SERVICES INC.	
Street address R.R #1, PO BOX 61		Unit no.	Lot/con.
Municipality GLENCOE	Postal code N0L 1M0	Province ONTARIO	E-mail
Telephone number (519) 287-2242 Business	Fax number (519) 287-5750	Cell number	
C. Design activities undertaken by individual identified in Section B. [Building Code Table 3.5.2.1. of Division C]			
☐ House ☐ HVAC – House ☒ Building Structural ☐ Small Buildings ☐ Building Services ☐ Plumbing – House ☐ Large Buildings ☐ Detection, Lighting and Power ☐ Plumbing – All Buildings ☐ Complex Buildings ☐ Fire Protection ☐ On-site Sewage Systems			
Description of designer's work ROYAL PINE HOMES – FORESTSIDE PH6 – BLOCK 8 – UNITS 1 - 4 (SCHEDULE IS ISSUED AS LOT SPECIFIC) REVIEW PRE-ENGINEERED ROOF SYSTEM COMPONENT DRAWINGS AND LAYOUT PLACEMENT PLAN SUPPLIED BY TAMARACK ROOF TRUSSES INC. (SEE DWG #TAM1151-19 DATED 1-18-19). SUPPORTING STRUCTURE TO BE REVIEWED AND VERIFIED BY QUALIFIED BUILDING DESIGNER.			
D. Declaration of Designer			
I, <u>SAM KATSOULAKOS</u> declare that (choose one as appropriate): (print name)			
☒ I review and take responsibility for the design work on behalf of a firm registered under subsection 3.2.4. of Division C, of the Building Code. I am qualified, and the firm is registered, in the appropriate classes/categories.			
Individual BCIN: <u>26064</u>			
Firm BCIN: <u>29991</u>			
☐ I review and take responsibility for the design and am qualified in the appropriate category as an "other designer" under subsection 3.2.5. of Division C, of the Building Code.			
Individual BCIN: _____			
Basis for exemption from registration: _____			
☐ The design work is exempt from the registration and qualification requirements of the Building Code.			
Basis for exemption from registration and qualification: _____			
I certify that:			
1. The information contained in this schedule is true to the best of my knowledge.			
2. I have submitted this application with the knowledge and consent of the firm.			
Date		<u>1-18-19</u> Signature of Designer	

NOTE:

- For the purposes of this form, "individual" means the "person" referred to in Clause 3.2.4.7(1) d) of Division C, Article 3.2.5.1. of Division C, and all other persons who are exempt from qualification under Subsections 3.2.4. and 3.2.5. of Division C.
- Schedule 1 is not required to be completed by a holder of a license, temporary license, or a certificate of authorization, issued by the Ontario Association of Architects. Schedule 1 is also not required to be completed by a holder of a license to practise, a limited license to practise, or a certificate of authorization, issued by the Association of Professional Engineers of Ontario.

DWG #TAM 1151-19S
 DWG #TAM 1152-19S