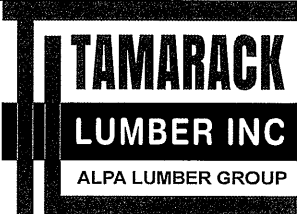


Products				
PlotID	Length	Product	Plies	Net Qty
J1	16-00-00	9 1/2" NI-40x	1	11
J1DJ	16-00-00	9 1/2" NI-40x	2	4
J2	14-00-00	9 1/2" NI-40x	1	19
J2DJ	14-00-00	9 1/2" NI-40x	2	12
J3	12-00-00	9 1/2" NI-40x	1	10
J4	10-00-00	9 1/2" NI-40x	1	12
J5	8-00-00	9 1/2" NI-40x	1	2
J6	6-00-00	9 1/2" NI-40x	1	3
J7	4-00-00	9 1/2" NI-40x	1	7
B1	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B7	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B3	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B5	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B2	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B4	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B6	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1

Connector Summary		
Qty	Manuf	Product
8	H1	IUS2.56/9.5
8	H1	IUS2.56/9.5
6	H1	IUS2.56/9.5
2	H3	HUS1.81/10
2	H3	HUS1.81/10

DATE: 2021-06-04

1st FLOOR



FROM PLAN DATED: AUG 2020

BUILDER: ROYALPINE HOMES

SITE: CENTREFIELD

MODEL: 38-10

ELEVATION: A

LOT:

CITY: RICHMOND HILL

SALESMAN: WILL GARCIA

DESIGNER: LBV

REVISION: AJ

NOTES:

REFER TO THE **NORDIC INSTALLATION** GUIDE FOR PROPER STORAGE AND INSTALLATION.

**SQUASH BLOCKS** OF 2x4, 2x6, 2x8 #2 S.P.F REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. **MULTIPLE SQUASH BLOCKS** REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. **CANTILEVERED JOISTS** INCLUDING **CANT' OVER BRICK** REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR **HOLES** INCLUDING **DUCT CHASE** AND **FIELD CUT OPENINGS** SEE FIGURE 7, TABLES 1 & 2. **CERAMIC TILE** APPLICATION AS PER O.B.C 9.30.6.

LOADING:

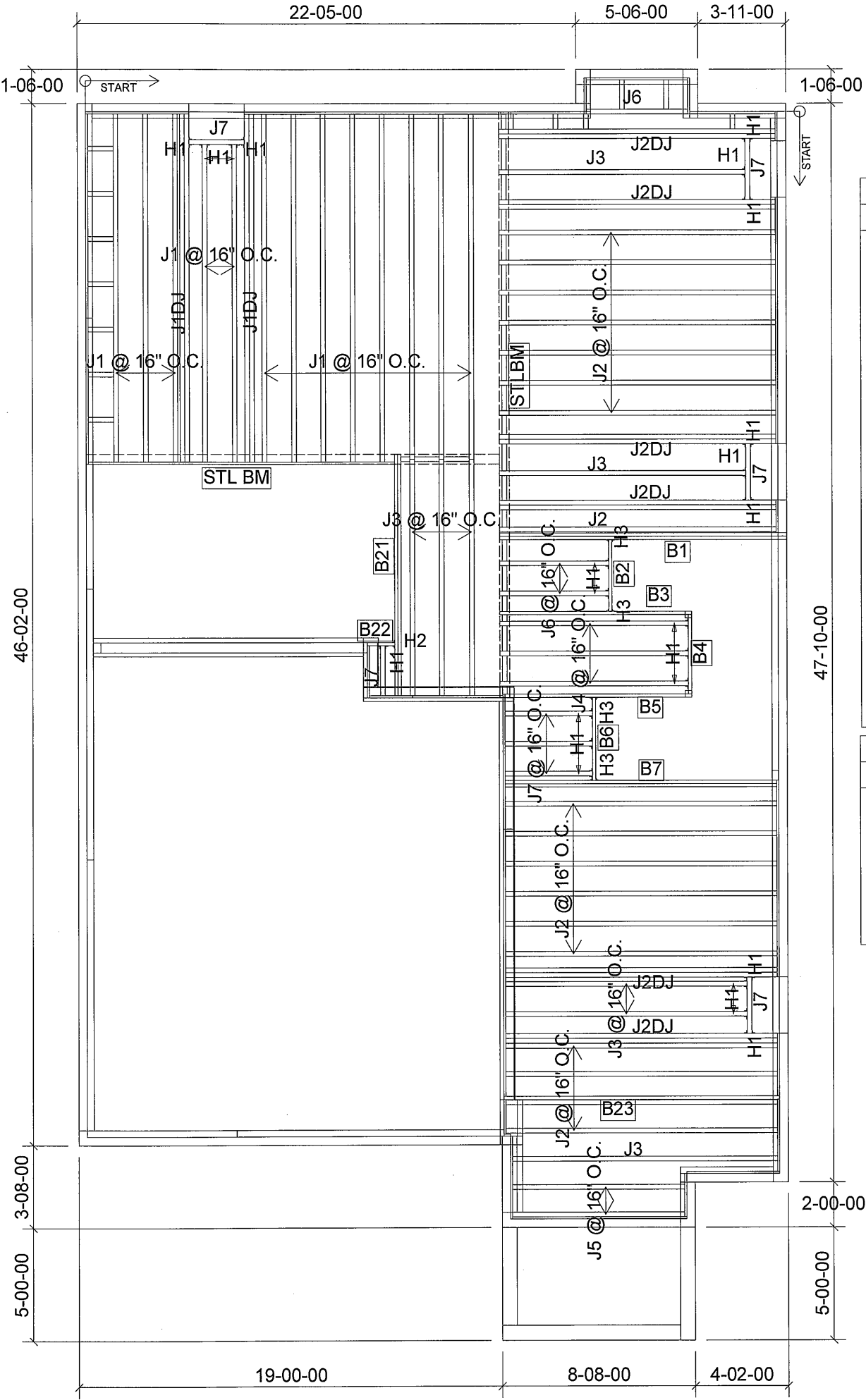
DESIGN LOADS: L/480.000

LIVE LOAD: 40.0 lb/ft<sup>2</sup>

DEAD LOAD: 15.0 lb/ft<sup>2</sup>

TILE LOAD: 20.0 lb/ft<sup>2</sup>

SUBFLOOR: 3/4" GLUED AND NAILED

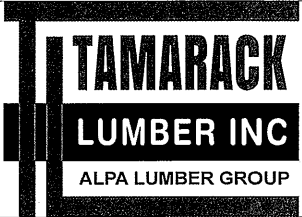


Products				
PlotID	Length	Product	Plies	Net Qty
J1	16-00-00	9 1/2" NI-40x	1	13
J1DJ	16-00-00	9 1/2" NI-40x	2	4
J2	14-00-00	9 1/2" NI-40x	1	18
J2DJ	14-00-00	9 1/2" NI-40x	2	12
J3	12-00-00	9 1/2" NI-40x	1	8
J4	10-00-00	9 1/2" NI-40x	1	3
J5	8-00-00	9 1/2" NI-40x	1	2
J6	6-00-00	9 1/2" NI-40x	1	3
J7	4-00-00	9 1/2" NI-40x	1	8
B23	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B1	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B7	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B21	12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B3	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B5	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B2	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B4	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B6	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B22	2-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1

Connector Summary		
Qty	Manuf	Product
9	H1	IUS2.56/9.5
8	H1	IUS2.56/9.5
6	H1	IUS2.56/9.5
1	H2	HUS1.81/10
2	H3	HUS1.81/10
2	H3	HUS1.81/10

DATE: 2021-06-04

1st FLOOR SUNKEN  
MUDROOM



FROM PLAN DATED: AUG 2020

BUILDER: ROYALPINE HOMES

SITE: CENTREFIELD

MODEL: 38-10

ELEVATION: A

LOT:

CITY: RICHMOND HILL

SALESMAN: WILL GARCIA

DESIGNER: LBV

REVISION: AJ

NOTES:

REFER TO THE **NORDIC INSTALLATION** GUIDE FOR PROPER STORAGE AND INSTALLATION.

**SQUASH BLOCKS** OF 2x4, 2x6, 2x8 #2 S.P.F REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. **MULTIPLE SQUASH BLOCKS** REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. **CANTILEVERED JOISTS** INCLUDING **CANT' OVER BRICK** REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR **HOLES** INCLUDING **DUCT CHASE** AND **FIELD CUT OPENINGS** SEE FIGURE 7, TABLES 1 & 2. **CERAMIC TILE** APPLICATION AS PER O.B.C 9.30.6.

LOADING:

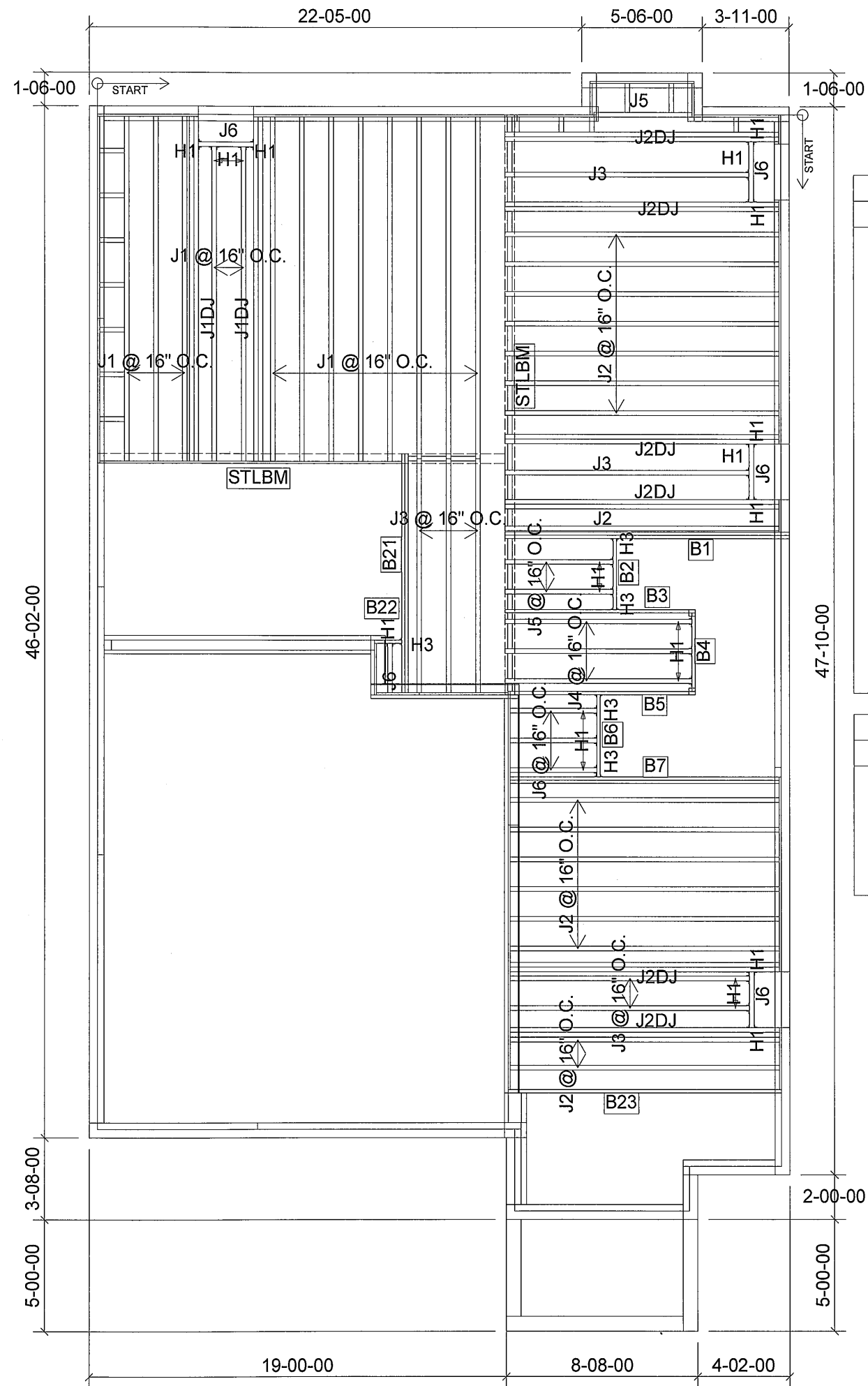
DESIGN LOADS: L/480.000

LIVE LOAD: 40.0 lb/ft<sup>2</sup>

DEAD LOAD: 15.0 lb/ft<sup>2</sup>

TILE LOAD: 20.0 lb/ft<sup>2</sup>

SUBFLOOR: 3/4" GLUED AND NAILED

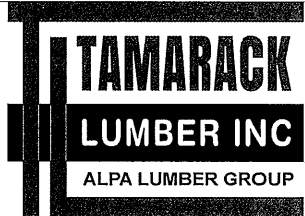


Products				
PlotID	Length	Product	Plies	Net Qty
J1	16-00-00	9 1/2" NI-40x	1	13
J1DJ	16-00-00	9 1/2" NI-40x	2	4
J2	14-00-00	9 1/2" NI-40x	1	16
J2DJ	14-00-00	9 1/2" NI-40x	2	12
J3	12-00-00	9 1/2" NI-40x	1	7
J4	10-00-00	9 1/2" NI-40x	1	3
J5	6-00-00	9 1/2" NI-40x	1	3
J6	4-00-00	9 1/2" NI-40x	1	8
B23	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B1	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B7	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B21	12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B3	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B5	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B2	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B4	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B6	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B22	2-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1

Connector Summary		
Qty	Manuf	Product
9	H1	IUS2.56/9.5
8	H1	IUS2.56/9.5
6	H1	IUS2.56/9.5
2	H3	HUS1.81/10
3	H3	HUS1.81/10

DATE: 2021-06-04

1st FLOOR SUNKEN



FROM PLAN DATED: AUG 2020

BUILDER: ROYALPINE HOMES

SITE: CENTREFIELD

MODEL: 38-10

ELEVATION: A

LOT:

CITY: RICHMOND HILL

SALESMAN: WILL GARCIA

DESIGNER: LBV

REVISION: AJ

NOTES:

REFER TO THE **NORDIC INSTALLATION** GUIDE FOR PROPER STORAGE AND INSTALLATION.

**SQUASH BLOCKS** OF 2x4, 2x6, 2x8 #2 S.P.F REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. **MULTIPLE SQUASH BLOCKS** REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. **CANTILEVERED JOISTS** INCLUDING **CANT' OVER BRICK** REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR **HOLES** INCLUDING **DUCT CHASE** AND **FIELD CUT OPENINGS** SEE FIGURE 7, TABLES 1 & 2. **CERAMIC TILE** APPLICATION AS PER O.B.C 9.30.6.

LOADING:

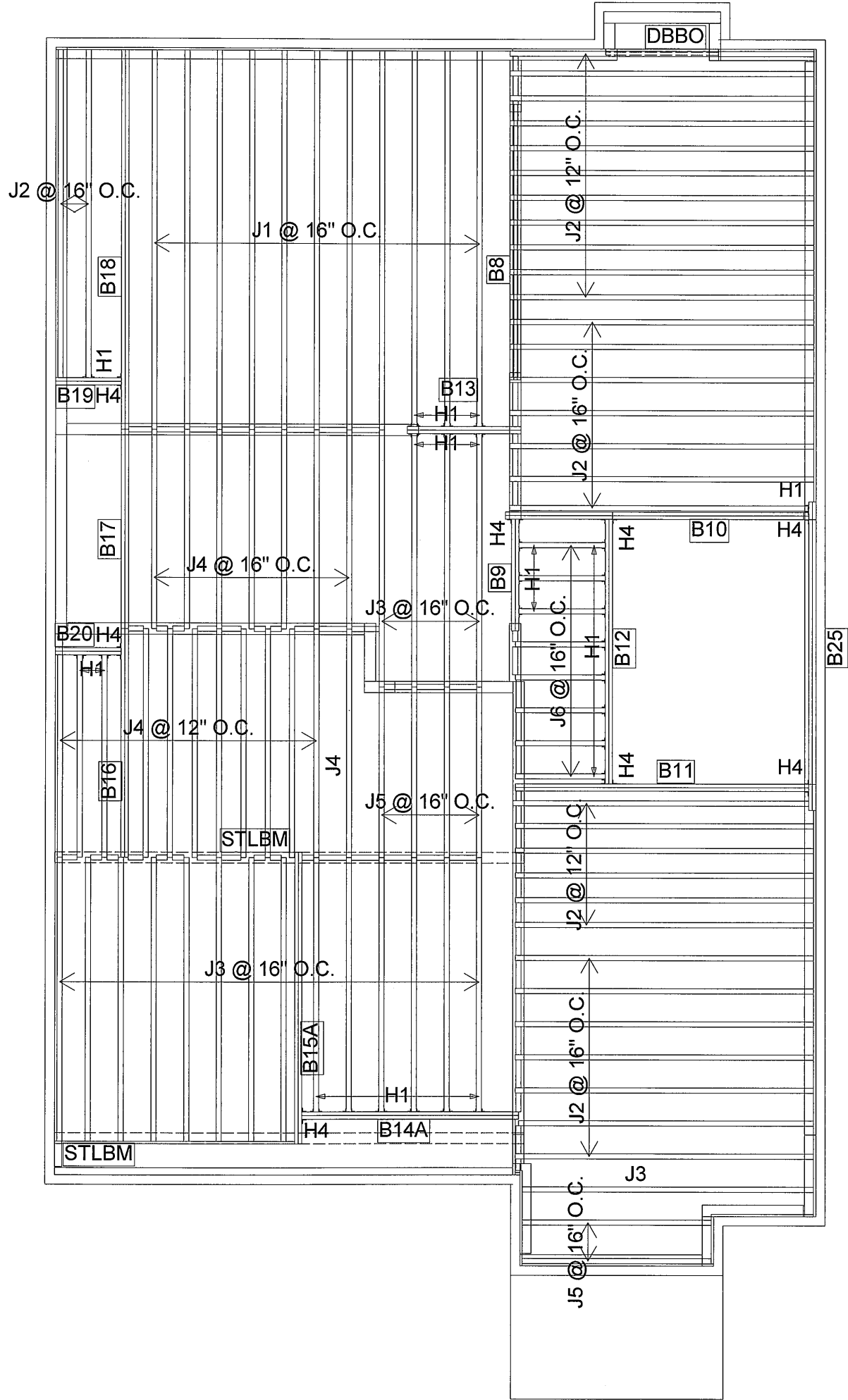
DESIGN LOADS: L/480.000

LIVE LOAD: 40.0 lb/ft<sup>2</sup>

DEAD LOAD: 15.0 lb/ft<sup>2</sup>

TILE LOAD: 20.0 lb/ft<sup>2</sup>

SUBFLOOR: 3/4" GLUED AND NAILED

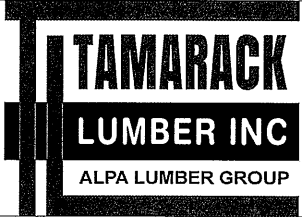


Products				
PlotID	Length	Product	Plies	Net Qty
J1	16-00-00	9 1/2" NI-40x	1	11
J2	14-00-00	9 1/2" NI-40x	1	33
J3	12-00-00	9 1/2" NI-40x	1	19
J4	10-00-00	9 1/2" NI-40x	1	19
J5	8-00-00	9 1/2" NI-40x	1	6
J6	4-00-00	9 1/2" NI-40x	1	8
B18	16-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B10	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B25	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B11	12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B12	12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B15A	12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B8	12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	3	3
B14A	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B16	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B17	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B13	6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B9	6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B19	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B20	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2

Connector Summary		
Qty	Manuf	Product
27	H1	IUS2.56/9.5
8	H4	HGUS410

DATE: 2021-06-04

2ND FLOOR



FROM PLAN DATED: AUG 2020

BUILDER: ROYALPINE HOMES

SITE: CENTREFIELD

MODEL: 38-10

ELEVATION: A

LOT:

CITY: RICHMOND HILL

SALESMAN: WILL GARCIA

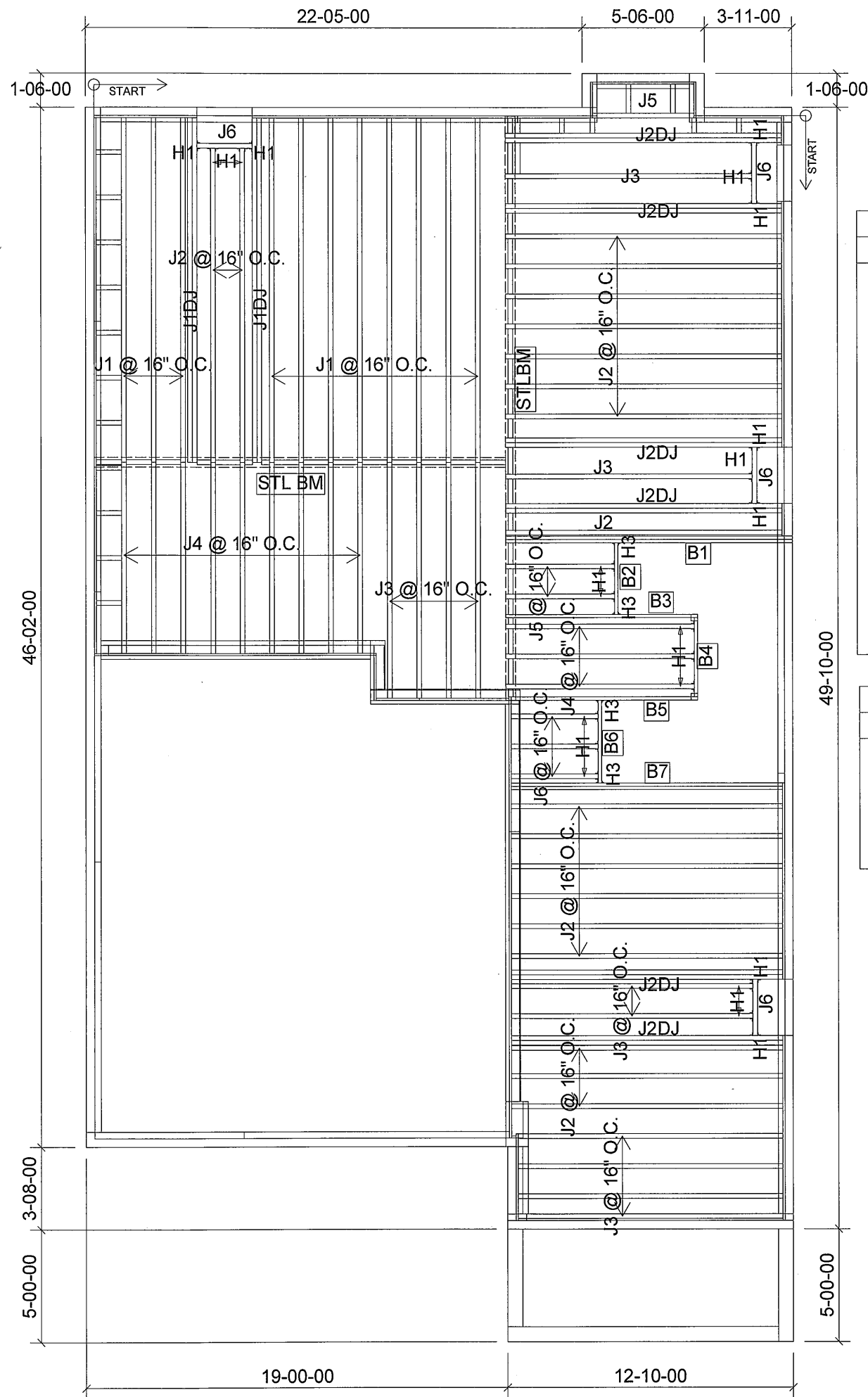
DESIGNER: LBV

REVISION:

**NOTES:**  
REFER TO THE **NORDIC INSTALLATION** GUIDE FOR PROPER STORAGE AND INSTALLATION.  
**SQUASH BLOCKS** OF 2x4, 2x6, 2x8 #2 S.P.F REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. **MULTIPLE SQUASH BLOCKS** REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. **CANTILEVERED JOISTS** INCLUDING **CANT' OVER BRICK** REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR **HOLES** INCLUDING **DUCT CHASE** AND **FIELD CUT OPENINGS** SEE FIGURE 7, TABLES 1 & 2. **CERAMIC TILE** APPLICATION AS PER O.B.C 9.30.6.

**LOADING:**  
DESIGN LOADS: L/480.000  
LIVE LOAD: 40.0 lb/ft²  
DEAD LOAD: 15.0 lb/ft²  
TILE LOAD: 20.0 lb/ft²

SUBFLOOR: 5/8" GLUED AND NAILED

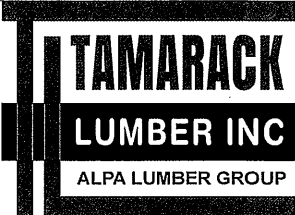


Products				
PlotID	Length	Product	Plies	Net Qty
J1	16-00-00	9 1/2" NI-40x	1	11
J1DJ	16-00-00	9 1/2" NI-40x	2	4
J2	14-00-00	9 1/2" NI-40x	1	19
J2DJ	14-00-00	9 1/2" NI-40x	2	12
J3	12-00-00	9 1/2" NI-40x	1	12
J4	10-00-00	9 1/2" NI-40x	1	12
J5	6-00-00	9 1/2" NI-40x	1	3
J6	4-00-00	9 1/2" NI-40x	1	7
B1	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B7	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B3	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B5	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B2	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B4	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B6	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1

Connector Summary		
Qty	Manuf	Product
8	H1	IUS2.56/9.5
8	H1	IUS2.56/9.5
6	H1	IUS2.56/9.5
2	H3	HUS1.81/10
2	H3	HUS1.81/10

DATE: 2021-06-04

1st FLOOR



FROM PLAN DATED: AUG 2020

BUILDER: ROYALPINE HOMES

SITE: CENTREFIELD

MODEL: 38-10

ELEVATION: B

LOT:

CITY: RICHMOND HILL

SALESMAN: WILL GARCIA

DESIGNER: LBV

REVISION: AJ

NOTES:

REFER TO THE **NORDIC INSTALLATION** GUIDE FOR PROPER STORAGE AND INSTALLATION.

**SQUASH BLOCKS** OF 2x4, 2x6, 2x8 #2 S.P.F REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. **MULTIPLE SQUASH BLOCKS** REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. **CANTILEVERED JOISTS** INCLUDING **CANT' OVER BRICK** REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR **HOLES** INCLUDING **DUCT CHASE** AND **FIELD CUT OPENINGS** SEE FIGURE 7, TABLES 1 & 2. **CERAMIC TILE** APPLICATION AS PER O.B.C 9.30.6.

LOADING:

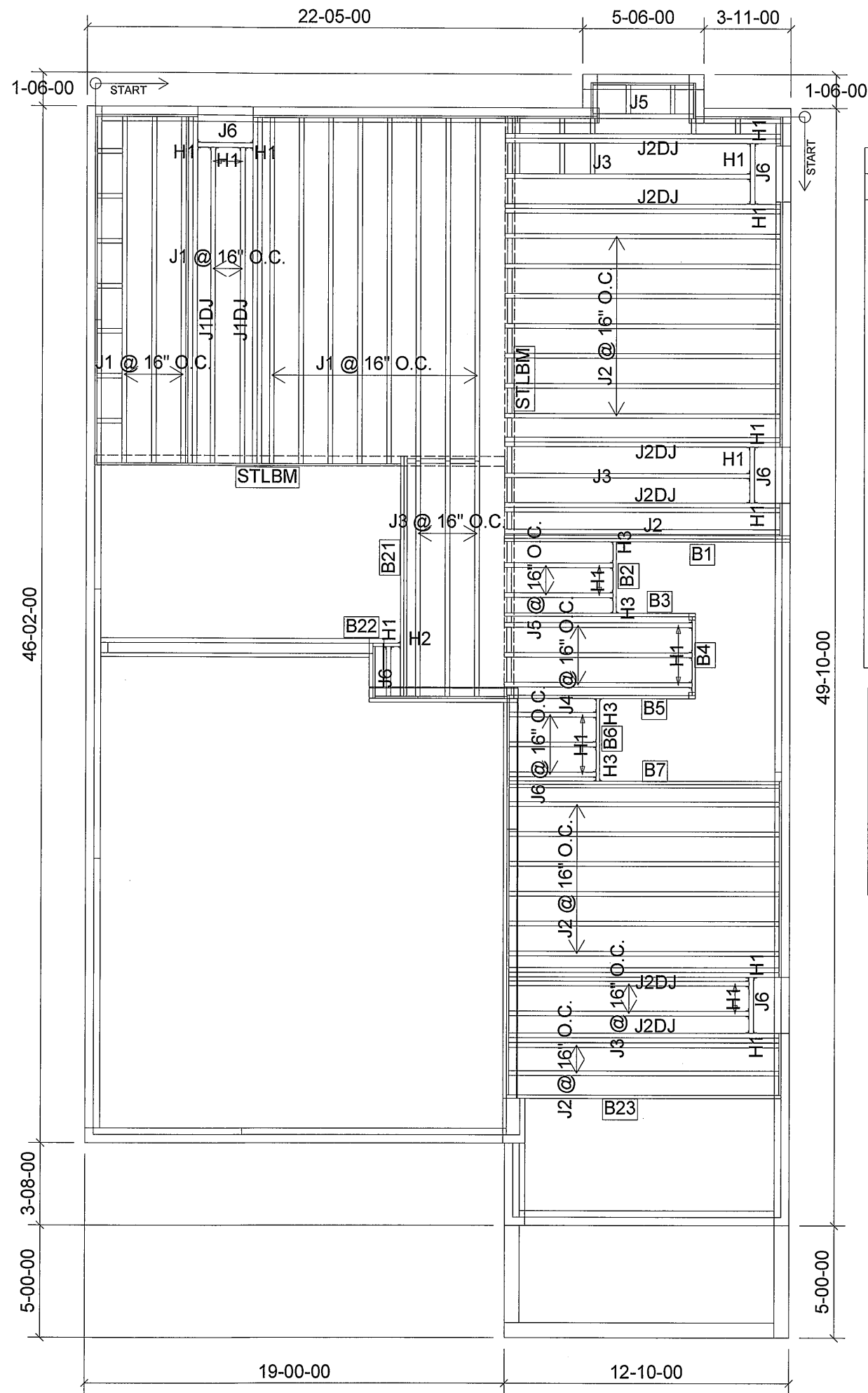
DESIGN LOADS: L/480.000

LIVE LOAD: 40.0 lb/ft<sup>2</sup>

DEAD LOAD: 15.0 lb/ft<sup>2</sup>

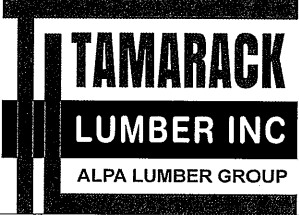
TILE LOAD: 20.0 lb/ft<sup>2</sup>

SUBFLOOR: 3/4" GLUED AND NAILED



Products				
PlotID	Length	Product	Plies	Net Qty
J1	16-00-00	9 1/2" NI-40x	1	13
J1DJ	16-00-00	9 1/2" NI-40x	2	4
J2	14-00-00	9 1/2" NI-40x	1	16
J2DJ	14-00-00	9 1/2" NI-40x	2	12
J3	12-00-00	9 1/2" NI-40x	1	7
J4	10-00-00	9 1/2" NI-40x	1	3
J5	6-00-00	9 1/2" NI-40x	1	3
J6	4-00-00	9 1/2" NI-40x	1	8
B23	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B1	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B7	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B21	12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B3	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B5	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B2	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B4	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B6	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B22	2-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1

Connector Summary		
Qty	Manuf	Product
9	H1	IUS2.56/9.5
8	H1	IUS2.56/9.5
6	H1	IUS2.56/9.5
1	H2	HUS1.81/10
2	H3	HUS1.81/10
2	H3	HUS1.81/10



FROM PLAN DATED: AUG 2020

BUILDER: ROYALPINE HOMES

SITE: CENTREFIELD

MODEL: 38-10

ELEVATION: B

LOT:

CITY: RICHMOND HILL

SALESMAN: WILL GARCIA

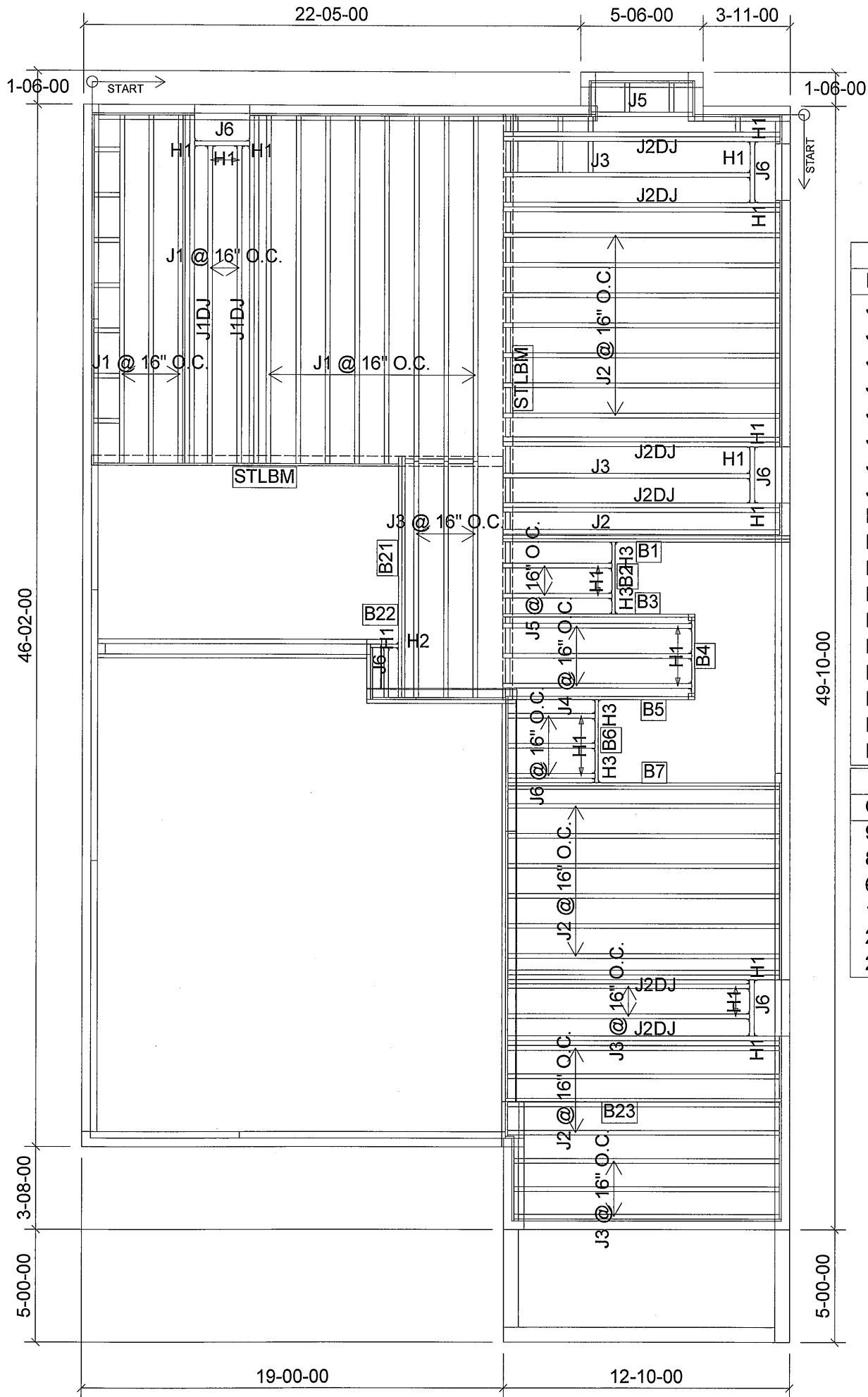
DESIGNER: LBV

REVISION: AJ

**NOTES:**  
REFER TO THE **NORDIC INSTALLATION** GUIDE FOR PROPER STORAGE AND INSTALLATION.  
**SQUASH BLOCKS** OF 2x4, 2x6, 2x8 #2 S.P.F REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. **MULTIPLE SQUASH BLOCKS** REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. **CANTILEVERED JOISTS** INCLUDING **CANT' OVER BRICK** REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR **HOLES** INCLUDING **DUCT CHASE** AND **FIELD CUT OPENINGS** SEE FIGURE 7, TABLES 1 & 2. **CERAMIC TILE** APPLICATION AS PER O.B.C 9.30.6.

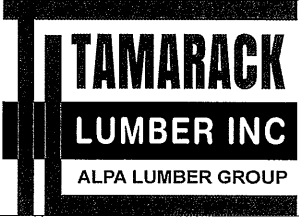
**LOADING:**  
DESIGN LOADS: L/480.000  
LIVE LOAD: 40.0 lb/ft²  
DEAD LOAD: 15.0 lb/ft²  
TILE LOAD: 20.0 lb/ft²  
**SUBFLOOR:** 3/4" GLUED AND NAILED

DATE: 2021-06-04
1st FLOOR
SUNKEN OPTIONS



Products				
PlotID	Length	Product	Plies	Net Qty
J1	16-00-00	9 1/2" NI-40x	1	13
J1DJ	16-00-00	9 1/2" NI-40x	2	4
J2	14-00-00	9 1/2" NI-40x	1	18
J2DJ	14-00-00	9 1/2" NI-40x	2	12
J3	12-00-00	9 1/2" NI-40x	1	10
J4	10-00-00	9 1/2" NI-40x	1	3
J5	6-00-00	9 1/2" NI-40x	1	3
J6	4-00-00	9 1/2" NI-40x	1	8
B23	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B1	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B7	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B21	12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B3	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B5	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B2	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B4	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B6	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B22	2-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1

Connector Summary		
Qty	Manuf	Product
9	H1	IUS2.56/9.5
8	H1	IUS2.56/9.5
6	H1	IUS2.56/9.5
1	H2	HUS1.81/10
2	H3	HUS1.81/10
2	H3	HUS1.81/10



FROM PLAN DATED: AUG 2020

BUILDER: ROYALPINE HOMES

SITE: CENTREFIELD

MODEL: 38-10

ELEVATION: B

LOT:

CITY: RICHMOND HILL

SALESMAN: WILL GARCIA

DESIGNER: LBV

REVISION: AJ

**NOTES:**  
REFER TO THE **NORDIC INSTALLATION** GUIDE FOR PROPER STORAGE AND INSTALLATION.  
**SQUASH BLOCKS** OF 2x4, 2x6, 2x8 #2 S.P.F REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. **MULTIPLE SQUASH BLOCKS** REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. **CANTILEVERED JOISTS** INCLUDING **CANT' OVER BRICK** REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR **HOLES** INCLUDING **DUCT CHASE** AND **FIELD CUT OPENINGS** SEE FIGURE 7, TABLES 1 & 2. **CERAMIC TILE** APPLICATION AS PER O.B.C 9.30.6.

**LOADING:**

DESIGN LOADS: L/480.000

LIVE LOAD: 40.0 lb/ft<sup>2</sup>

DEAD LOAD: 15.0 lb/ft<sup>2</sup>

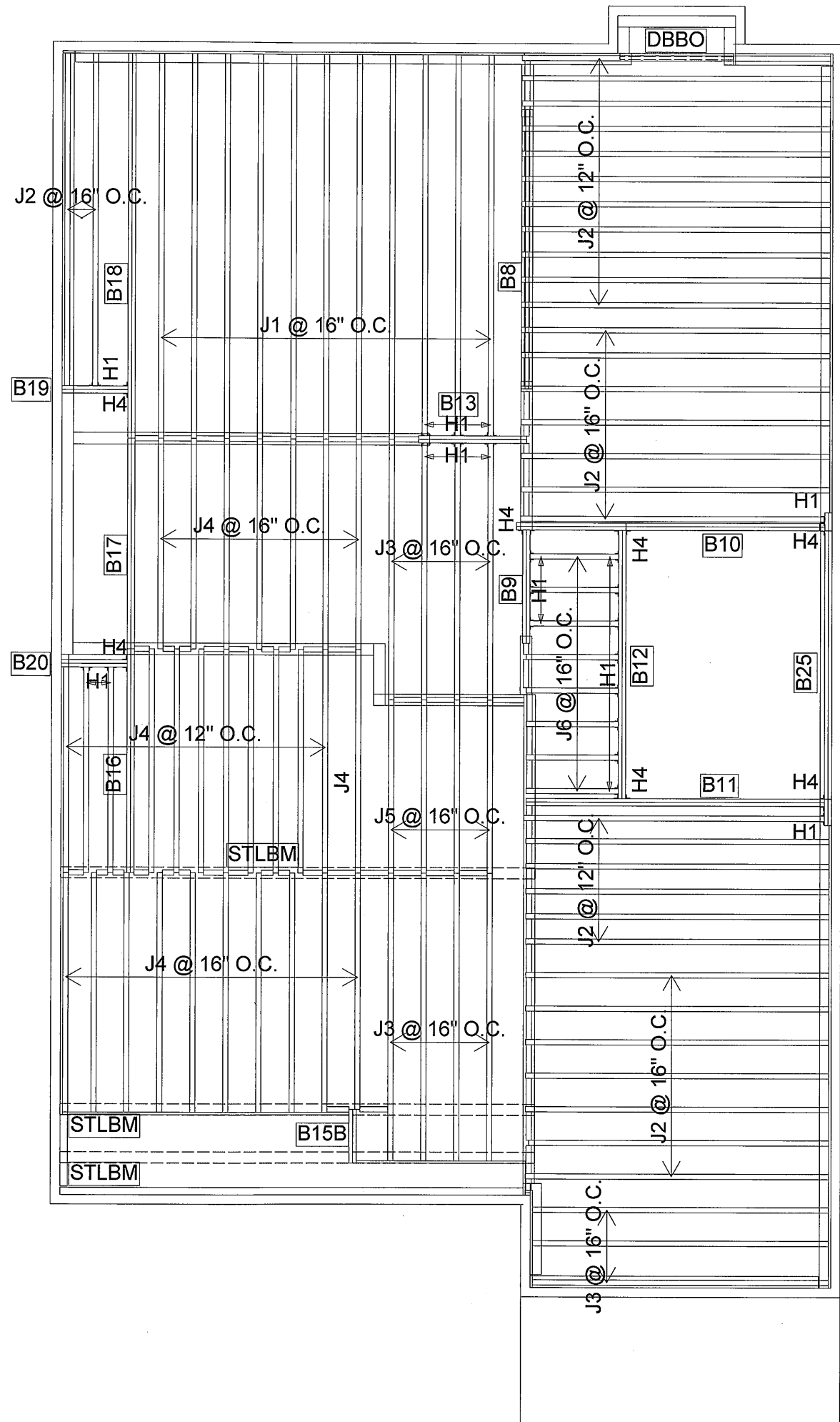
TILE LOAD: 20.0 lb/ft<sup>2</sup>

**SUBFLOOR:** 3/4" GLUED AND NAILED

DATE: 2021-06-04

1st FLOOR

SUNKEN  
MUDROOM

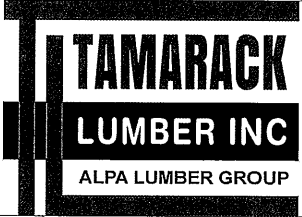


Products				
PlotID	Length	Product	Plies	Net Qty
J1	16-00-00	9 1/2" NI-40x	1	11
J2	14-00-00	9 1/2" NI-40x	1	33
J3	12-00-00	9 1/2" NI-40x	1	11
J4	10-00-00	9 1/2" NI-40x	1	29
J5	8-00-00	9 1/2" NI-40x	1	4
J6	4-00-00	9 1/2" NI-40x	1	8
B18	16-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B10	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B25	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B11	12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B12	12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B8	12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	3	3
B16	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B17	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B13	6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B9	6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B15B	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B19	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B20	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2

Connector Summary		
Qty	Manuf	Product
22	H1	IUS2.56/9.5
7	H4	HGUS410

DATE: 2021-06-04

2ND FLOOR



FROM PLAN DATED: AUG 2020

BUILDER: ROYALPINE HOMES

SITE: CENTREFIELD

MODEL: 38-10

ELEVATION: B

LOT:

CITY: RICHMOND HILL

SALESMAN: WILL GARCIA

DESIGNER: LBV

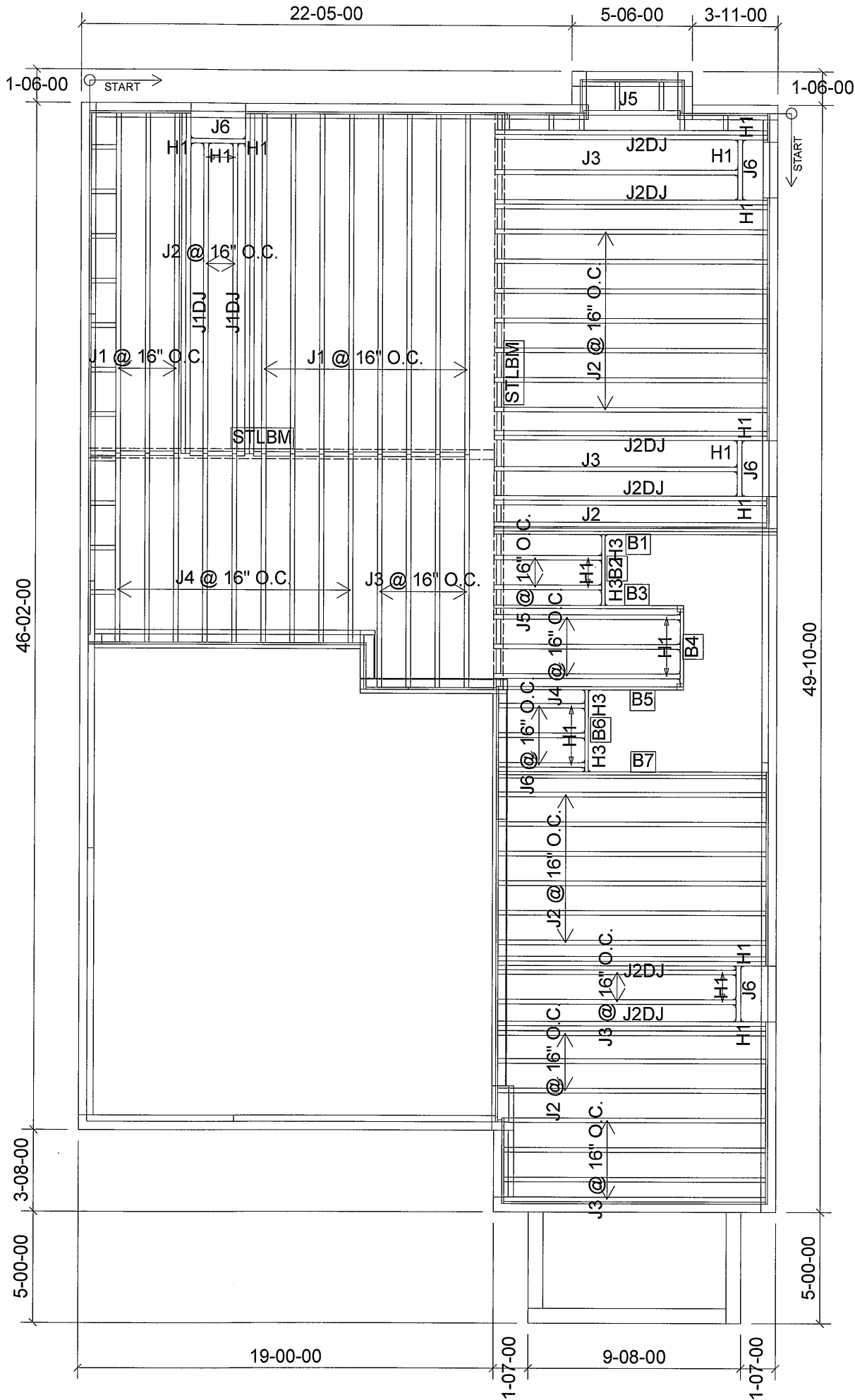
REVISION:

**NOTES:**  
REFER TO THE **NORDIC INSTALLATION** GUIDE FOR PROPER STORAGE AND INSTALLATION.  
**SQUASH BLOCKS** OF 2x4, 2x6, 2x8 #2 S.P.F REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. **MULTIPLE SQUASH BLOCKS** REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. **CANTILEVERED JOISTS** INCLUDING **CANT' OVER BRICK** REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR **HOLES** INCLUDING **DUCT CHASE** AND **FIELD CUT OPENINGS** SEE FIGURE 7, TABLES 1 & 2. **CERAMIC TILE** APPLICATION AS PER O.B.C 9.30.6.

**LOADING:**  
DESIGN LOADS: L/480.000  
LIVE LOAD: 40.0 lb/ft²  
DEAD LOAD: 15.0 lb/ft²  
TILE LOAD: 20.0 lb/ft²

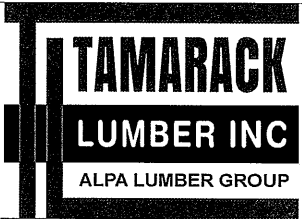
SUBFLOOR: 5/8" GLUED AND NAILED





Products				
PlotID	Length	Product	Plies	Net Qty
J1	16-00-00	9 1/2" NI-40x	1	11
J1DJ	16-00-00	9 1/2" NI-40x	2	4
J2	14-00-00	9 1/2" NI-40x	1	19
J2DJ	14-00-00	9 1/2" NI-40x	2	12
J3	12-00-00	9 1/2" NI-40x	1	12
J4	10-00-00	9 1/2" NI-40x	1	12
J5	6-00-00	9 1/2" NI-40x	1	3
J6	4-00-00	9 1/2" NI-40x	1	7
B1	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B7	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B3	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B5	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B2	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B4	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B6	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1

Connector Summary		
Qty	Manuf	Product
8	H1	IUS2.56/9.5
8	H1	IUS2.56/9.5
6	H1	IUS2.56/9.5
2	H3	HUS1.81/10
2	H3	HUS1.81/10



FROM PLAN DATED: AUG 2020

BUILDER: ROYALPINE HOMES

SITE: CENTREFIELD

MODEL: 38-10

ELEVATION: C

LOT:

CITY: RICHMOND HILL

SALESMAN: WILL GARCIA

DESIGNER: LBV

REVISION: AJ

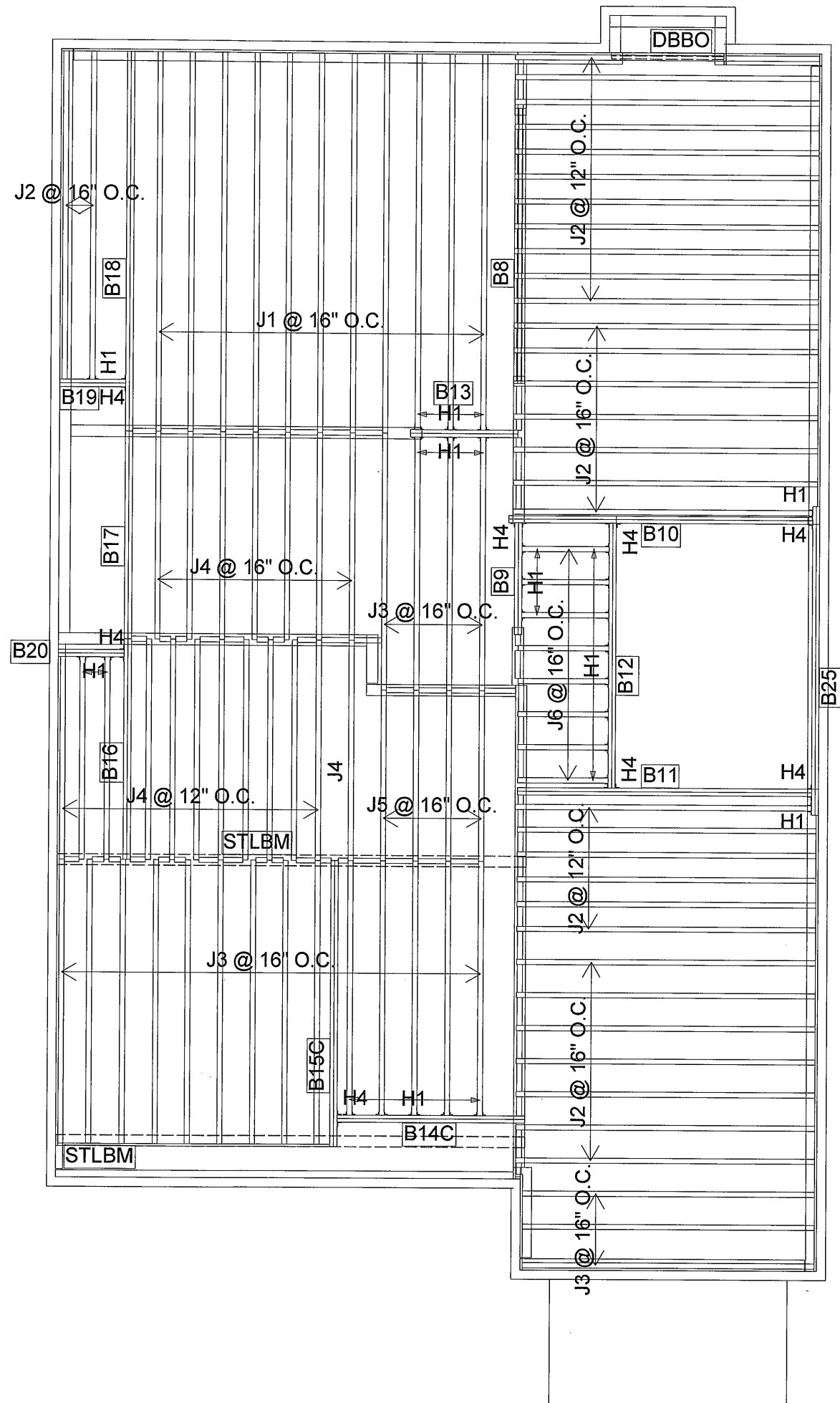
**NOTES:**  
REFER TO THE **NORDIC INSTALLATION** GUIDE FOR PROPER STORAGE AND INSTALLATION.  
**SQUASH BLOCKS** OF 2x4, 2x6, 2x8 #2 S.P.F REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. **MULTIPLE SQUASH BLOCKS** REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. **CANTILEVERED JOISTS** INCLUDING **CANT' OVER BRICK** REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR **HOLES** INCLUDING **DUCT CHASE** AND **FIELD CUT OPENINGS** SEE FIGURE 7, TABLES 1 & 2. **CERAMIC TILE** APPLICATION AS PER O.B.C 9.30.6.

**LOADING:**  
DESIGN LOADS: L/480.000  
LIVE LOAD: 40.0 lb/ft<sup>2</sup>  
DEAD LOAD: 15.0 lb/ft<sup>2</sup>  
TILE LOAD: 20.0 lb/ft<sup>2</sup>

DATE: 2021-06-04

1st FLOOR

SUBFLOOR: 3/4" GLUED AND NAILED

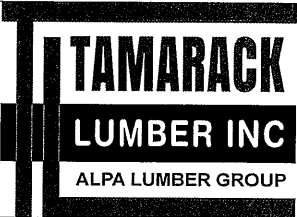


Products				
PlotID	Length	Product	Plies	Net Qty
J1	16-00-00	9 1/2" NI-40x	1	11
J2	14-00-00	9 1/2" NI-40x	1	33
J3	12-00-00	9 1/2" NI-40x	1	21
J4	10-00-00	9 1/2" NI-40x	1	19
J5	8-00-00	9 1/2" NI-40x	1	4
J6	4-00-00	9 1/2" NI-40x	1	8
B18	16-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B10	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B25	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B11	12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B12	12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B15C	12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B8	12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	3	3
B16	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B17	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B14C	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B13	6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B9	6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B19	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B20	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2

Connector Summary		
Qty	Manuf	Product
27	H1	IUS2.56/9.5
8	H4	HGUS410

DATE: 2021-06-04

2ND FLOOR



FROM PLAN DATED: AUG 2020

BUILDER: ROYALPINE HOMES

SITE: CENTREFIELD

MODEL: 38-10

ELEVATION: C

LOT:

CITY: RICHMOND HILL

SALESMAN: WILL GARCIA

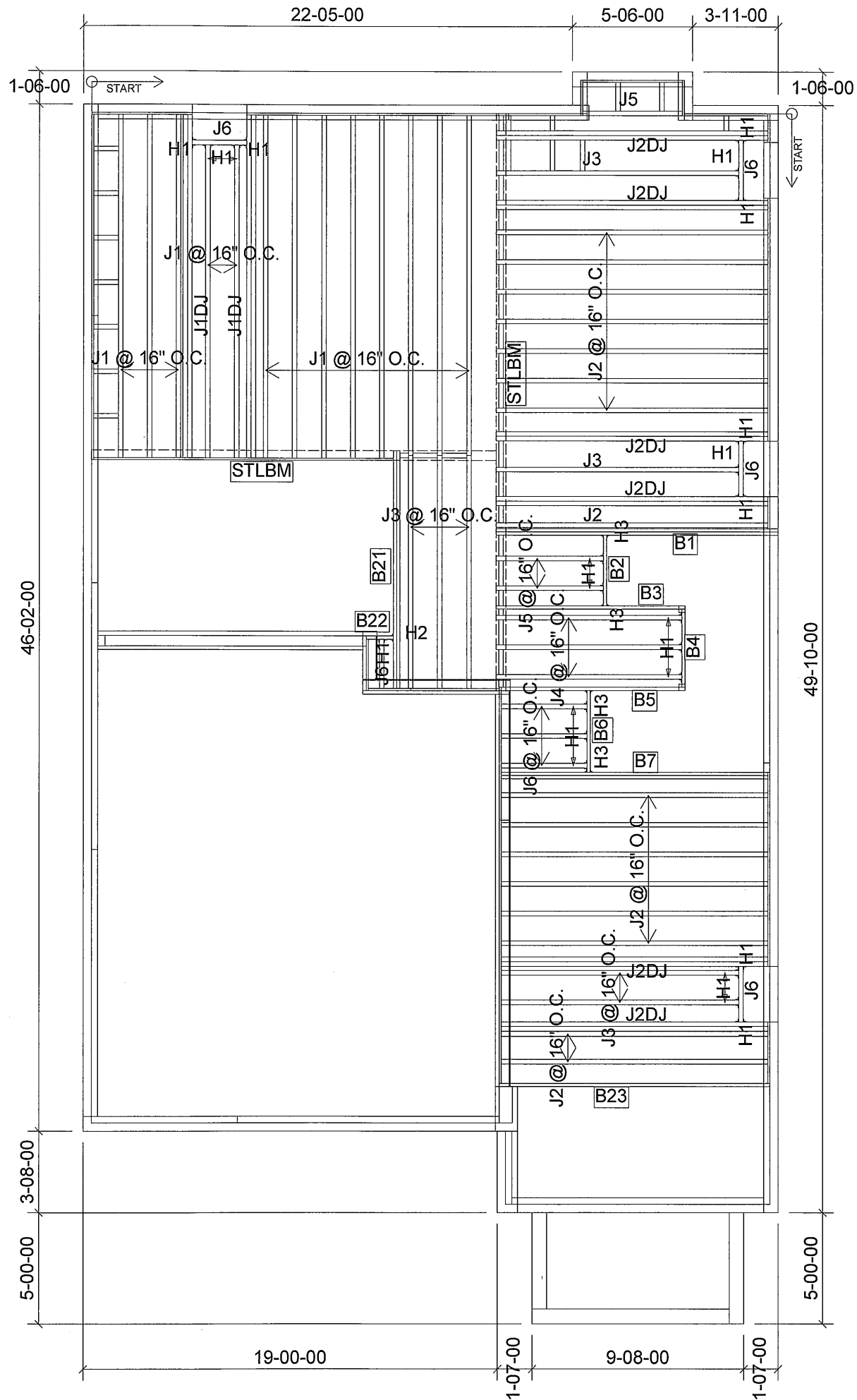
DESIGNER: LBV

REVISION:

**NOTES:**  
REFER TO THE **NORDIC INSTALLATION** GUIDE FOR PROPER STORAGE AND INSTALLATION.  
**SQUASH BLOCKS** OF 2x4, 2x6, 2x8 #2 S.P.F REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. **MULTIPLE SQUASH BLOCKS** REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. **CANTILEVERED JOISTS** INCLUDING **CANT' OVER BRICK** REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR **HOLES** INCLUDING **DUCT CHASE** AND **FIELD CUT OPENINGS** SEE FIGURE 7, TABLES 1 & 2. **CERAMIC TILE** APPLICATION AS PER O.B.C 9.30.6.

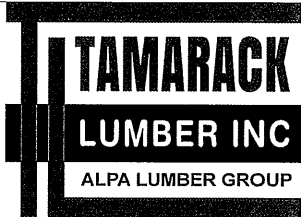
**LOADING:**  
DESIGN LOADS: L/480.000  
LIVE LOAD: 40.0 lb/ft²  
DEAD LOAD: 15.0 lb/ft²  
TILE LOAD: 20.0 lb/ft²

SUBFLOOR: 5/8" GLUED AND NAILED



Products				
PlotID	Length	Product	Plies	Net Qty
J1	16-00-00	9 1/2" NI-40x	1	13
J1DJ	16-00-00	9 1/2" NI-40x	2	4
J2	14-00-00	9 1/2" NI-40x	1	16
J2DJ	14-00-00	9 1/2" NI-40x	2	12
J3	12-00-00	9 1/2" NI-40x	1	7
J4	10-00-00	9 1/2" NI-40x	1	3
J5	6-00-00	9 1/2" NI-40x	1	3
J6	4-00-00	9 1/2" NI-40x	1	8
B23	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B1	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B7	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B21	12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B3	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B5	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B2	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B4	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B6	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B22	2-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1

Connector Summary		
Qty	Manuf	Product
9	H1	IUS2.56/9.5
8	H1	IUS2.56/9.5
6	H1	IUS2.56/9.5
1	H2	HUS1.81/10
2	H3	HUS1.81/10
2	H3	HUS1.81/10



FROM PLAN DATED: AUG 2020

BUILDER: ROYALPINE HOMES

SITE: CENTREFIELD

MODEL: 38-10

ELEVATION: C

LOT:

CITY: RICHMOND HILL

SALESMAN: WILL GARCIA

DESIGNER: LBV

REVISION: AJ

NOTES:

REFER TO THE **NORDIC INSTALLATION** GUIDE FOR PROPER STORAGE AND INSTALLATION.

**SQUASH BLOCKS** OF 2x4, 2x6, 2x8 #2 S.P.F REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. **MULTIPLE SQUASH BLOCKS** REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. **CANTILEVERED JOISTS** INCLUDING **CANT' OVER BRICK** REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR **HOLES** INCLUDING **DUCT CHASE** AND **FIELD CUT OPENINGS** SEE FIGURE 7, TABLES 1 & 2. **CERAMIC TILE** APPLICATION AS PER O.B.C 9.30.6.

LOADING:

DESIGN LOADS: L/480.000

LIVE LOAD: 40.0 lb/ft<sup>2</sup>

DEAD LOAD: 15.0 lb/ft<sup>2</sup>

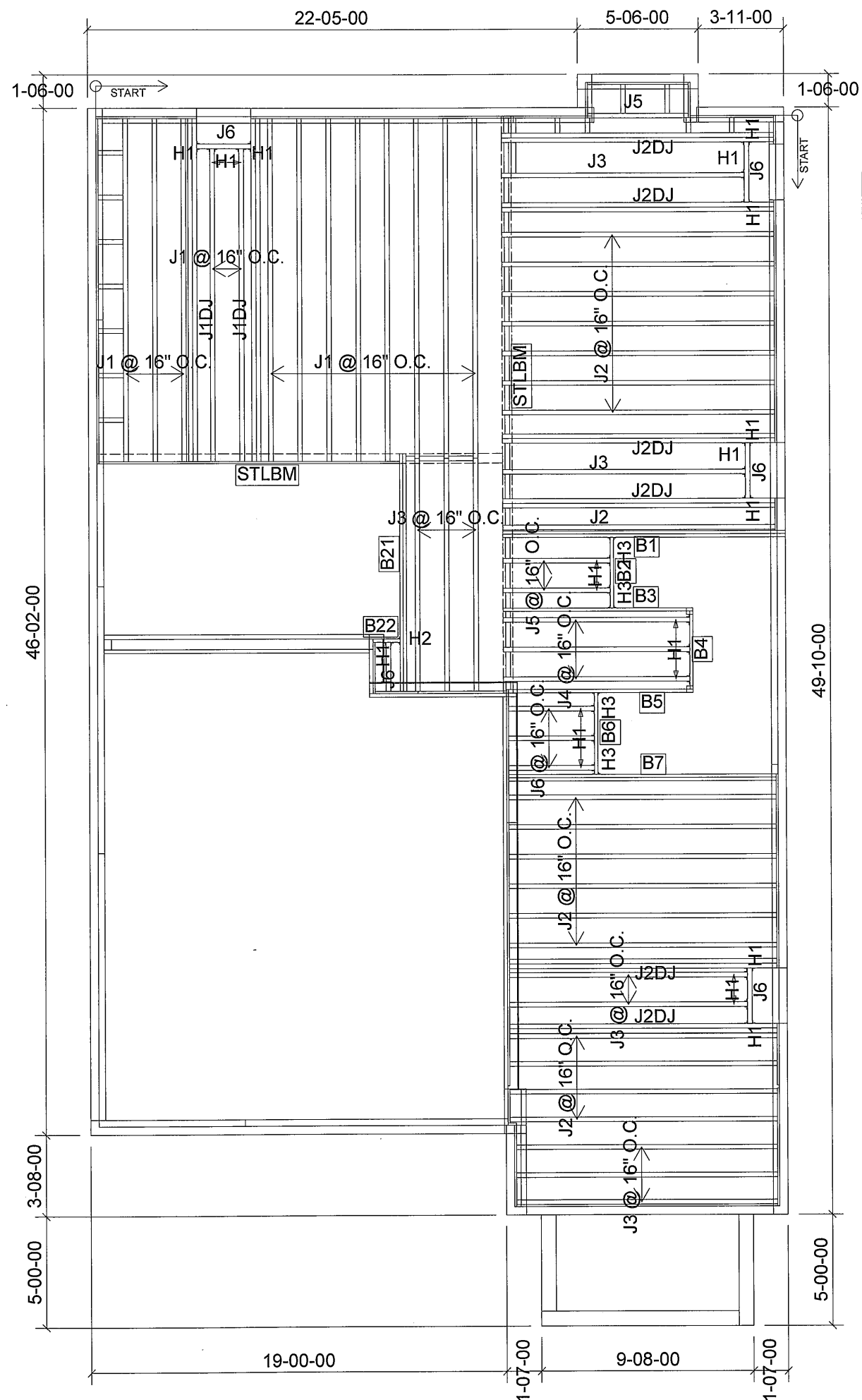
TILE LOAD: 20.0 lb/ft<sup>2</sup>

SUBFLOOR: 3/4" GLUED AND NAILED

DATE: 2021-06-04

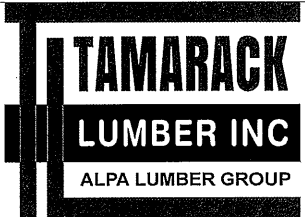
1st FLOOR

SUNKEN  
OPTIONS



Products				
PlotID	Length	Product	Plies	Net Qty
J1	16-00-00	9 1/2" NI-40x	1	13
J1DJ	16-00-00	9 1/2" NI-40x	2	4
J2	14-00-00	9 1/2" NI-40x	1	18
J2DJ	14-00-00	9 1/2" NI-40x	2	12
J3	12-00-00	9 1/2" NI-40x	1	10
J4	10-00-00	9 1/2" NI-40x	1	3
J5	6-00-00	9 1/2" NI-40x	1	3
J6	4-00-00	9 1/2" NI-40x	1	8
B1	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B7	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B21	12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B3	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B5	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B2	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B4	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B6	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B22	2-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1

Connector Summary		
Qty	Manuf	Product
9	H1	IUS2.56/9.5
8	H1	IUS2.56/9.5
6	H1	IUS2.56/9.5
1	H2	HUS1.81/10
2	H3	HUS1.81/10
2	H3	HUS1.81/10



**FROM PLAN DATED: AUG 2020**

**BUILDER:** ROYALPINE HOMES

**SITE: CENTREFIELD**

**MODEL: 38-10**

**ELEVATION: C**

**LOT:**

**CITY:** RICHMOND HILL

**SALESMAN: WILL GARCIA**

DESIGNER: LBV

**REVISION:** AJ

**NOTES:**

REFER TO THE NORDIC INSTALLATION  
GUIDE FOR PROPER STORAGE AND  
INSTALLATION.

**SQUASH BLOCKS** OF 2x4, 2x6, 2x8 #2 S.P.F. REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. **MULTIPLE SQUASH BLOCKS** REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. **CANTILEVERED JOISTS** INCLUDING **CANT' OVER BRICK** REQ. JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR **HOLES** INCLUDING **DUCT CHASE** AND **FIELD CUT OPENINGS** SEE FIGURE 7, TABLES 1 & 2. **CERAMIC TILE** APPLICATION AS PER O.B.C 9.30.6.

**LOADING:**

DESIGN LOADS: L/480.000

LIVE LOAD: 40.0 lb/ft<sup>2</sup>

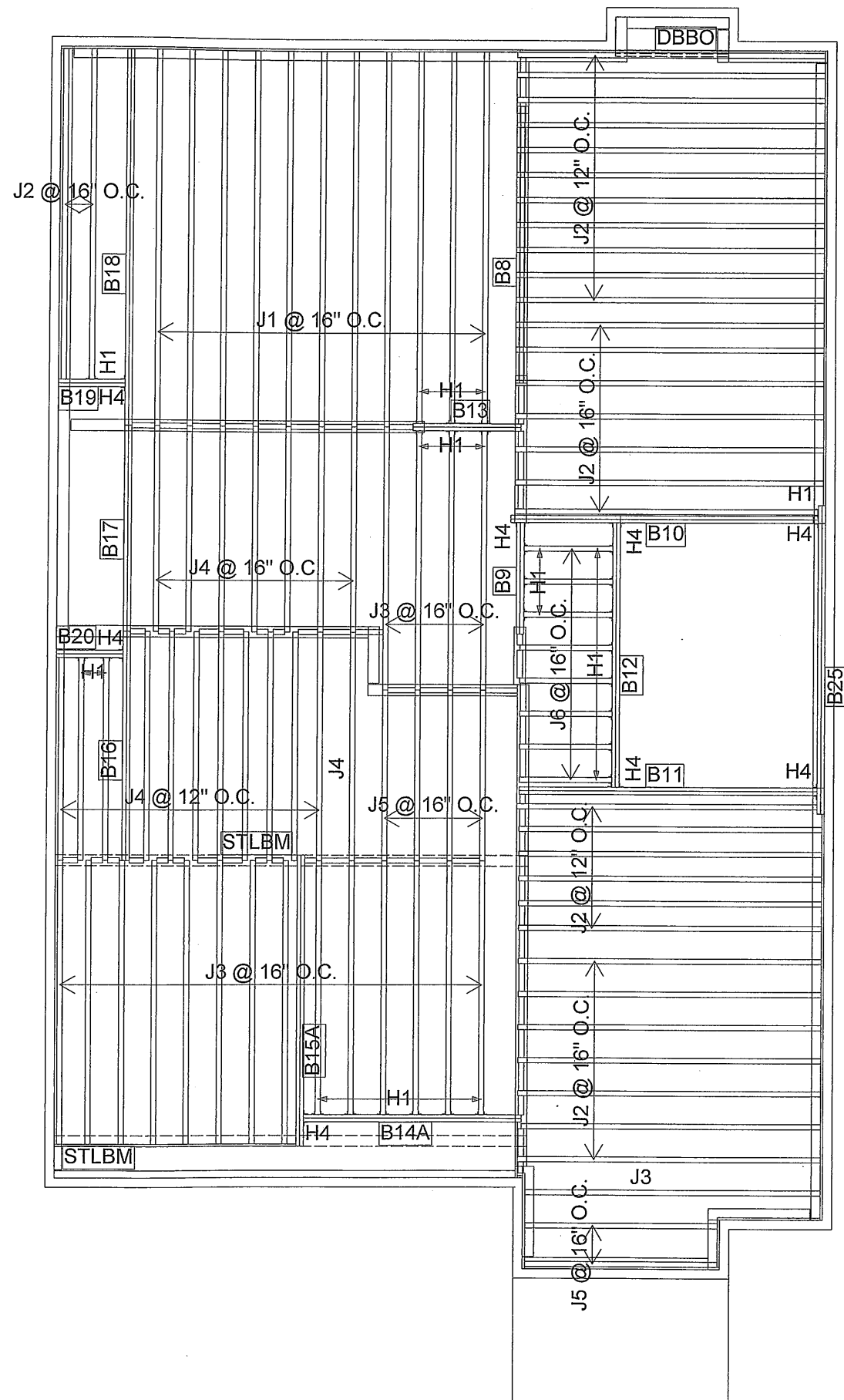
DEAD LOAD: 15.0 lb/ft<sup>2</sup>TILF LOAD: 20.0 lb/ft<sup>2</sup>

**SUBFLOOR: 3/4" GLUED AND NAILED**

**DATE:** 2021-06-04

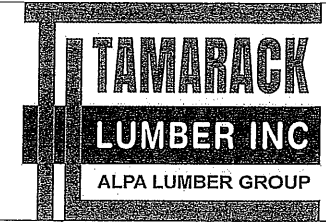
## 1st FLOOR

# SUNKEN MUDROOM



Products				
PlotID	Length	Product	Plies	Net Qty
J1	16-00-00	9 1/2" NI-40x	1	11
J2	14-00-00	9 1/2" NI-40x	1	33
J3	12-00-00	9 1/2" NI-40x	1	19
J4	10-00-00	9 1/2" NI-40x	1	19
J5	8-00-00	9 1/2" NI-40x	1	6
J6	4-00-00	9 1/2" NI-40x	1	8
B18	16-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B10	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B25	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B11	12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B12	12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B15A	12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B8	12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	3	3
B14A	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B16	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B17	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B13	6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B9	6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B19	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B20	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2

Connector Summary		
Qty	Manuf	Product
27	H1	IUS2.56/9.5
8	H4	HGUS410



FROM PLAN DATED: AUG 2020  
BUILDER: ROYALPINE HOMES  
SITE: CENTREFIELD  
MODEL: 38-10  
ELEVATION: A  
LOT:  
CITY: RICHMOND HILL  
SALESMAN: WILL GARCIA  
DESIGNER: LBV  
REVISION:

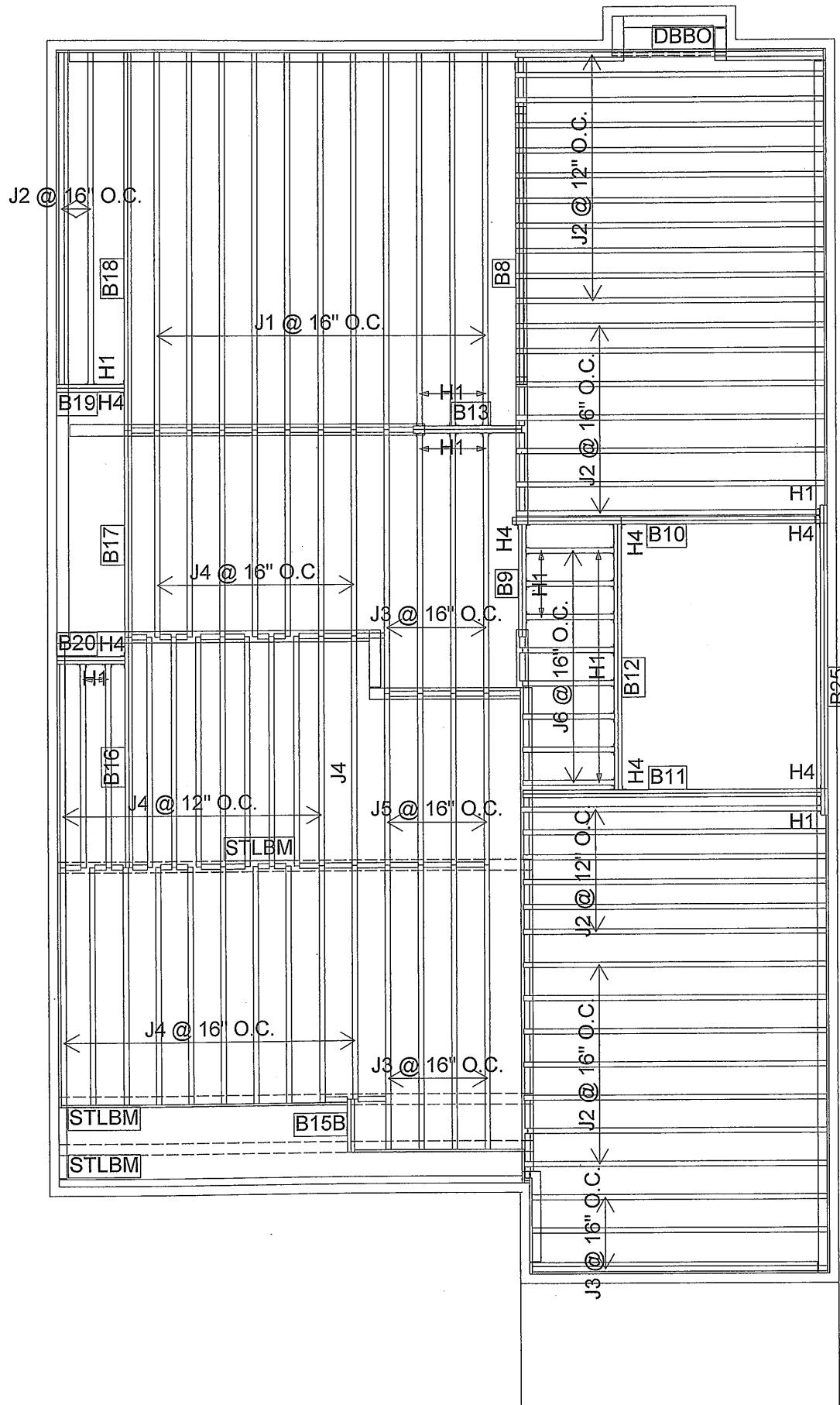
NOTES:  
REFER TO THE NORDIC INSTALLATION GUIDE FOR PROPER STORAGE AND INSTALLATION.  
SQUASH BLOCKS OF 2x4, 2x6, 2x8 #2 S.P.I REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. CANTILEVERED JOISTS INCLUDING CANT' OVER BRICK RE I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR HOLES INCLUDING DUCT CHASE AND FIELD CUT OPENINGS SEE FIGURE 7, TABLES 1 & 2. CERAMIC TI APPLICATION AS PER O.B.C 9.30.6.

LOADING:  
DESIGN LOADS: L/480.000  
LIVE LOAD: 40.0 lb/ft²  
DEAD LOAD: 15.0 lb/ft²  
TILE LOAD: 20.0 lb/ft²

SUBFLOOR: 5/8" GLUED AND NAILED

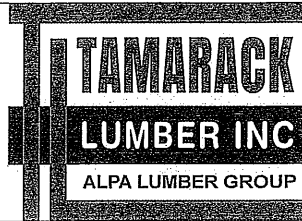
DATE: 2020-10-20

2ND FLOOR



Products				
PlotID	Length	Product	Plies	Net Qty
J1	16-00-00	9 1/2" NI-40x	1	11
J2	14-00-00	9 1/2" NI-40x	1	33
J3	12-00-00	9 1/2" NI-40x	1	11
J4	10-00-00	9 1/2" NI-40x	1	29
J5	8-00-00	9 1/2" NI-40x	1	4
J6	4-00-00	9 1/2" NI-40x	1	8
B18	16-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B10	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B25	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B11	12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B12	12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B8	12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	3	3
B16	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B17	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B13	6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B9	6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B15B	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B19	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B20	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2

Connector Summary		
Qty	Manuf	Product
22	H1	IUS2.56/9.5
7	H4	HGUS410



FROM PLAN DATED: AUG 2020

BUILDER: ROYALPINE HOMES

SITE: CENTREFIELD

MODEL: 38-10

ELEVATION: B

LOT:

CITY: RICHMOND HILL

SALESMAN: WILL GARCIA

DESIGNER: LBV

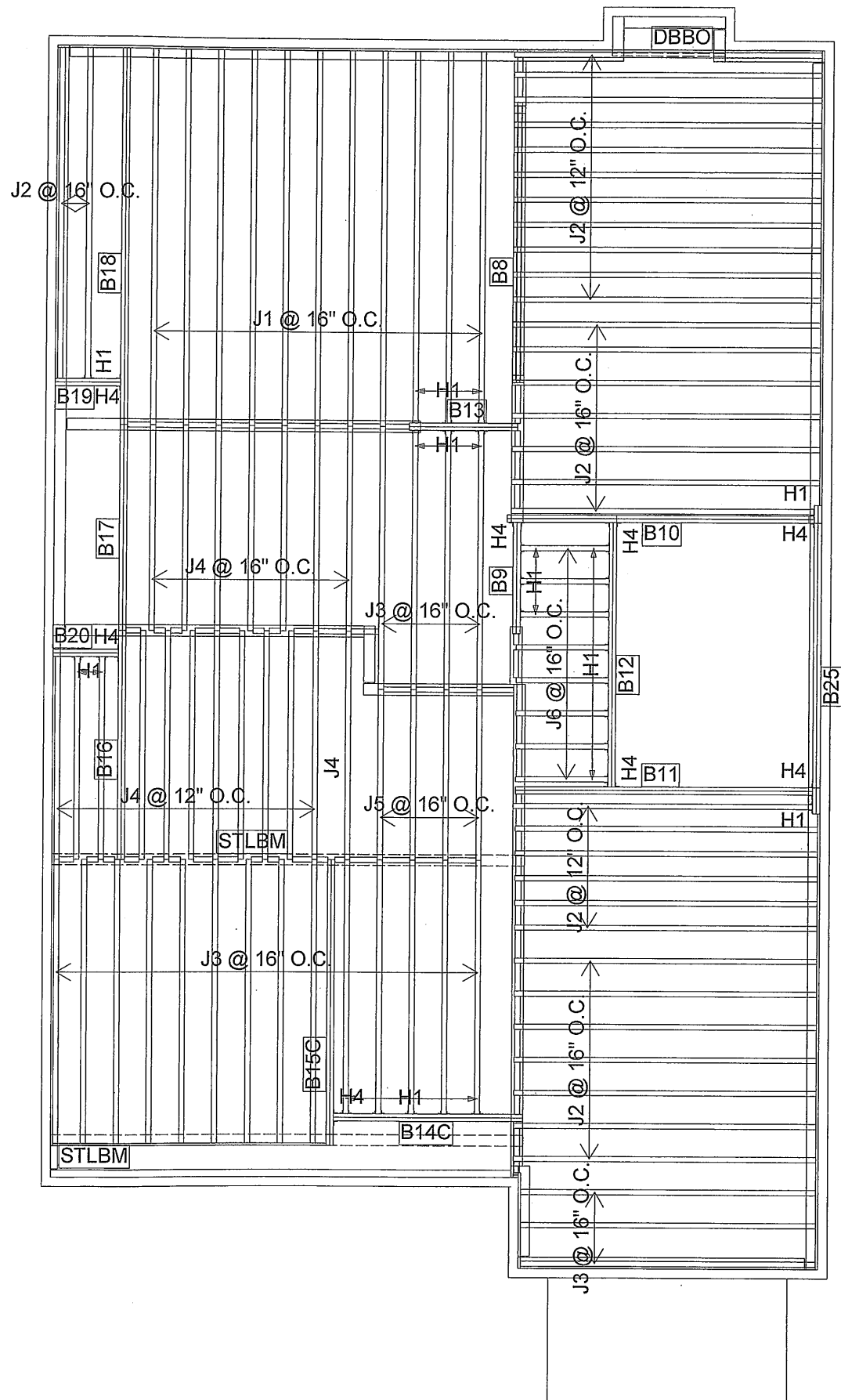
REVISION:

**NOTES:**  
REFER TO THE **NORDIC INSTALLATION** GUIDE FOR PROPER STORAGE AND INSTALLATION.  
**SQUASH BLOCKS** OF 2x4, 2x6, 2x8 #2 S.P.I REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. **MULTIPLE SQUASH BLOCKS** REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. **CANTILEVERED JOISTS** INCLUDING **CANT' OVER BRICK** RE I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR **HOLES** INCLUDING **DUCT CHASE** AND **FIELD CUT OPENINGS** SEE FIGURE 7, TABLES 1 & 2. **CERAMIC TI** APPLICATION AS PER O.B.C 9.30.6.

**LOADING:**  
DESIGN LOADS: L/480.000  
LIVE LOAD: 40.0 lb/ft<sup>2</sup>  
DEAD LOAD: 15.0 lb/ft<sup>2</sup>  
TILE LOAD: 20.0 lb/ft<sup>2</sup>  
**SUBFLOOR:** 5/8" GLUED AND NAILED

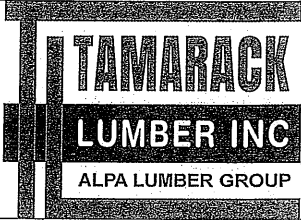
DATE: 2020-10-20

2ND FLOOR



Products				
PlotID	Length	Product	Plies	Net Qty
J1	16-00-00	9 1/2" NI-40x	1	11
J2	14-00-00	9 1/2" NI-40x	1	33
J3	12-00-00	9 1/2" NI-40x	1	21
J4	10-00-00	9 1/2" NI-40x	1	19
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B14C	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B13	6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
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FROM PLAN DATED: AUG 2020

BUILDER: ROYALPINE HOMES

SITE: CENTREFIELD

MODEL: 38-10

ELEVATION: C

LOT:

CITY: RICHMOND HILL

SALESMAN: WILL GARCIA

DESIGNER: LBV

REVISION:

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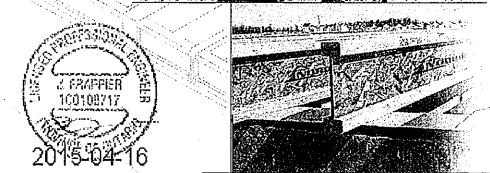
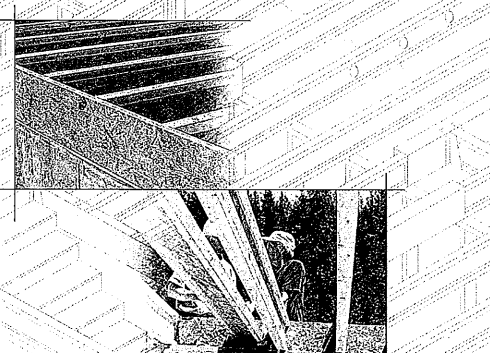
DATE: 2020-10-20

2ND FLOOR



## INSTALLATION GUIDE

FOR RESIDENTIAL FLOORS



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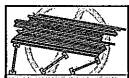


N-C301 / November 2014

### SAFETY AND CONSTRUCTION PRECAUTIONS



Do not walk on I-joists until fully fastened and braced, or serious injuries can result.



Never stack building materials over unsheathed I-joists. Once sheathed, do not over-stress I-joist with concentrated loads from building materials.

#### WARNING

I-joists are not stable until completely installed, and will not carry any load until fully braced and sheathed.

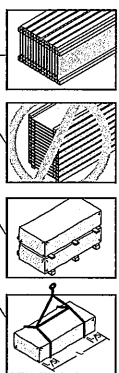
#### Avoid Accidents by Following these Important Guidelines:

- Brace and nail each I-joist as it is installed, using hangers, blocking panels, rim board, and/or cross-bridging at joist ends. When I-joists are applied continuous over interior supports and a load-bearing wall is planned at that location, blocking will be required at the interior support.
- When the building is completed, the floor sheathing will provide lateral support for the top flanges of the I-joists. Until this sheathing is applied, temporary bracing, often called struts, or temporary sheathing must be applied to prevent I-joist rollover or buckling.
  - Temporary bracing or struts must be 1x4 inch minimum, at least 8 feet long and spaced no more than 8 feet on centre, and must be secured with a minimum of two 2-1/2" nails fastened to the top surface of each I-joist. Nail the bracing to a lateral restraint at the end of each bay. Lap ends of adjoining bracing over at least two I-joists.
  - Or, sheathing (temporary or permanent) can be nailed to the top flange of the first 4 feet of I-joists at the end of the bay.
- For cantilevered I-joists, brace top and bottom flanges, and brace ends with closure panels, rim board, or cross-bridging.
- Install and fully nail permanent sheathing to each I-joist before placing loads on the floor system. Then, stack building materials over beams or walls only.
- Never install a damaged I-joist.

Improper storage or installation, failure to follow applicable building codes, failure to follow span ratings for Nordic I-joists, failure to follow allowable hole sizes and locations, or failure to use web stiffeners when required can result in serious accidents. Follow these installation guidelines carefully.

### STORAGE AND HANDLING GUIDELINES

- Bundle wrap can be slippery when wet. Avoid walking on wrapped bundles.
- Store, stack, and handle I-joists vertically and level only.
- Always stack and handle I-joists in the upright position only.
- Do not store I-joists in direct contact with the ground and/or flatwise.
- Protect I-joists from weather, and use spacers to separate bundles.
- Bundled units should be kept intact until time of installation.
- When handling I-joists with a crane on the job site, take a few simple precautions to prevent damage to the I-joists and injury to your work crew.
  - Pick I-joists in bundles as shipped by the supplier.
  - Orient the bundles so that the webs of the I-joists are vertical.
  - Pick the bundles at the 5th points, using a spreader bar if necessary.
- Do not handle I-joists in a horizontal orientation.
- NEVER USE OR TRY TO REPAIR A DAMAGED I-JOIST.



### MAXIMUM FLOOR SPANS

- Maximum clear spans applicable to simple-span or multiple-span residential floor construction with a design live load of 40 psf and dead load of 15 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration and a live load deflection limit of L/480. For multiple-span applications, the end spans shall be 40% or more of the adjacent span.
- Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 5/8 inch for a joist spacing of 19.2 inches or less, or 3/4 inch for joist spacing of 24 inches. Adhesive shall meet the requirements given in CGOS-71.26 Standard. No concrete topping or bridging element was assumed. Increased spans may be achieved with the use of gypsum and/or a row of blocking at mid-span.
- Minimum bearing length shall be 1-3/4 inches for the end bearings, and 3-1/2 inches for the intermediate bearings.
- Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.
- This span chart is based on uniform loads. For applications with other than uniform loads, an engineering analysis may be required based on the use of the design properties.
- Tables are based on Limit States Design per CAN/CSA O86-09 Standard, and NBC 2010.
- SI units conversion: 1 inch = 25.4 mm, 1 foot = 0.305 m

### MAXIMUM FLOOR SPANS FOR NORDIC I-JOISTS

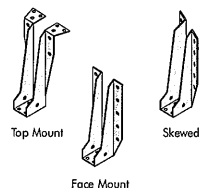
SIMPLE AND MULTIPLE SPANS

Joist Depth	Joist Series	Simple spans				Multiple spans			
		On centre spacing				On centre spacing			
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
9-1/2"	Ni-20	15'-1"	14'-2"	13'-9"	13'-5"	16'-3"	15'-4"	14'-10"	14'-7"
	Ni-40x	16'-1"	15'-2"	14'-8"	14'-9"	17'-5"	16'-5"	15'-10"	15'-5"
	Ni-60	16'-3"	15'-4"	14'-10"	14'-11"	17'-7"	16'-7"	16'-0"	16'-1"
	Ni-70	17'-1"	16'-1"	15'-6"	15'-7"	18'-7"	17'-4"	16'-9"	16'-10"
	Ni-80	17'-3"	16'-3"	15'-8"	15'-9"	18'-10"	17'-6"	16'-11"	17'-0"
11-7/8"	Ni-20	16'-11"	16'-0"	15'-5"	15'-6"	18'-4"	17'-3"	16'-8"	16'-7"
	Ni-40x	18'-1"	17'-0"	16'-5"	16'-6"	20'-0"	18'-6"	17'-9"	17'-7"
	Ni-60	18'-4"	17'-3"	16'-7"	16'-9"	20'-3"	18'-9"	18'-0"	18'-1"
	Ni-70	19'-6"	18'-0"	17'-4"	17'-5"	21'-6"	19'-11"	19'-0"	19'-1"
	Ni-80	19'-9"	18'-3"	17'-8"	17'-7"	21'-9"	20'-2"	19'-3"	19'-4"
14"	Ni-90	20'-2"	18'-7"	17'-10"	17'-11"	22'-3"	20'-7"	19'-8"	19'-9"
	Ni-90x	20'-4"	18'-9"	17'-11"	18'-0"	22'-5"	20'-9"	19'-10"	19'-11"
	Ni-40x	20'-1"	18'-7"	17'-10"	17'-11"	22'-2"	20'-6"	19'-8"	19'-4"
	Ni-60	20'-5"	18'-11"	18'-1"	18'-2"	22'-7"	20'-11"	20'-0"	20'-1"
	Ni-70	21'-7"	20'-0"	19'-1"	19'-2"	23'-10"	22'-1"	21'-1"	21'-2"
16"	Ni-80	21'-11"	20'-3"	19'-4"	19'-5"	24'-3"	22'-5"	21'-5"	21'-6"
	Ni-90	22'-5"	20'-8"	19'-9"	19'-10"	24'-9"	22'-10"	21'-10"	21'-10"
	Ni-90x	22'-7"	20'-11"	19'-11"	20'-0"	25'-0"	23'-1"	22'-2"	22'-2"
	Ni-60	22'-5"	20'-8"	19'-9"	19'-10"	24'-7"	22'-9"	21'-9"	21'-10"
	Ni-70	23'-6"	21'-9"	20'-9"	20'-10"	26'-0"	24'-0"	22'-11"	23'-0"

CCMC EVALUATION REPORT 13032-R

### I-JOIST HANGERS

- Hangers shown illustrate the three most commonly used metal hangers to support I-joists.
- All nailing must meet the hanger manufacturer's recommendations.
- Hangers should be selected based on the joist depth, flange width and load capacity based on the maximum spans.
- Web stiffeners are required when the sides of the hangers do not laterally brace the top flange of the I-joist.



### WEB STIFFENERS

#### RECOMMENDATIONS:

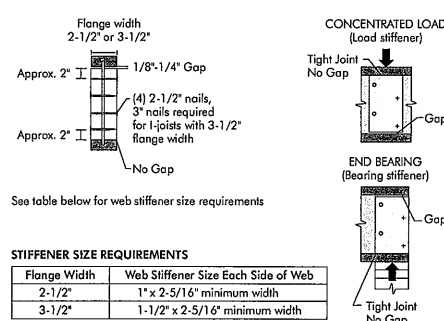
A bearing stiffener is required in all engineered applications with factored reactions greater than shown in the I-joist properties table found of the I-joist Construction Guide (C101). The gap between the stiffener and the flange is at the top.

A bearing stiffener is required when the I-joist is supported in a hanger and the sides of the hanger do not extend up to, and support, the top flange. The gap between the stiffener and flange is at the top.

A load stiffener is required at locations where a factored concentrated load greater than 2,370 lbs is applied to the top flange between supports, or in the case of a cantilever, anywhere between the cantilever tip and the support. These values are for standard term load duration, and may be adjusted for other load durations as permitted by the code. The gap between the stiffener and the flange is at the bottom.

SI units conversion: 1 inch = 25.4 mm

FIGURE 2  
WEB STIFFENER INSTALLATION DETAILS



### NORDIC I-JOIST SERIES

Series	Depth	Flange Width	Material Thickness	Minimum Depth
Ni-20	9-1/2"	15'-1"	1/2"	5-1/2"
Ni-40x	11-7/8"	16'-1"	3/4"	7-1/4"
Ni-60	14"	18'-1"	1"	7-1/4"
Ni-70	16"	21'-11"	1-1/2"	7-1/4"
Ni-80	18"	23'-6"	1-1/2"	7-1/4"
Ni-90	21"	26'-0"	1-1/2"	7-1/4"
Ni-90x	22"	27'-3"	1-1/2"	7-1/4"

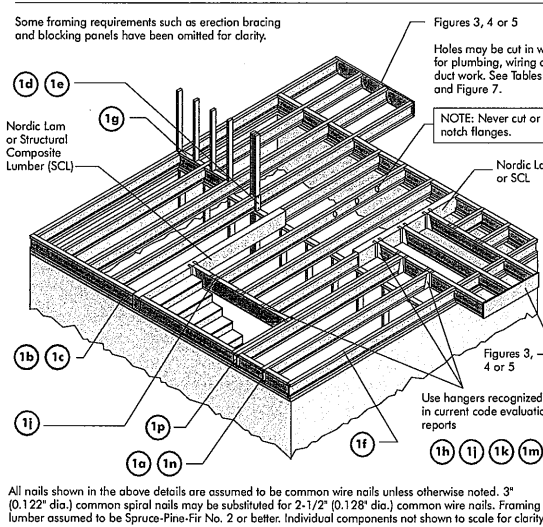
Chantiers Chibougamau Ltd. harvests its own trees, which enables Nordic products to adhere to strict quality control procedures throughout the manufacturing process. Every phase of the operation, from the raw log to the finished product, reflects our commitment to quality.

Nordic Engineered Wood I-joists use only finger-jointed lumber in their flanges, ensuring consistent quality, superior strength, and longer span carrying capacity.

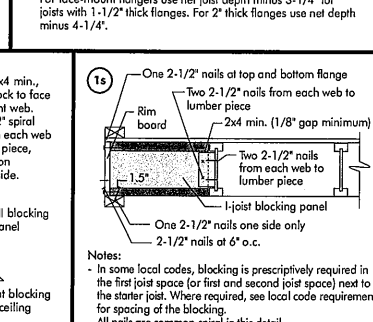
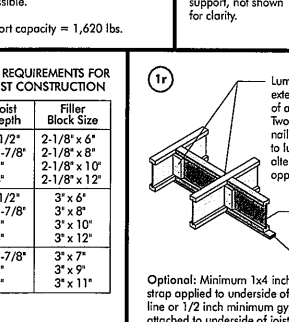
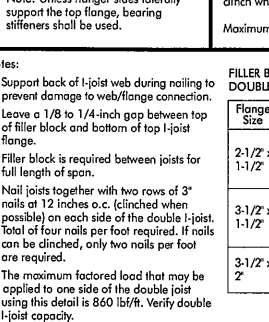
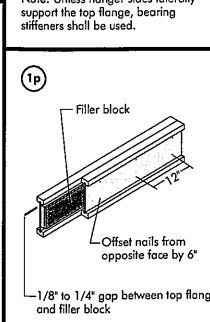
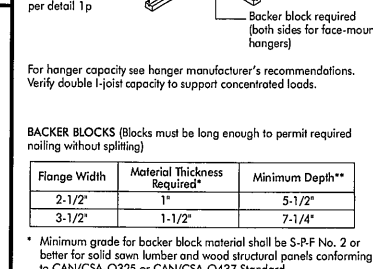
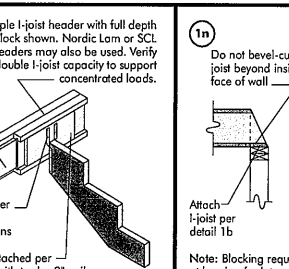
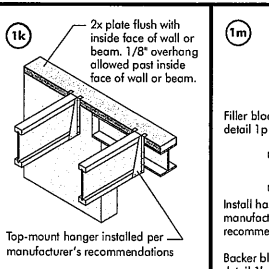
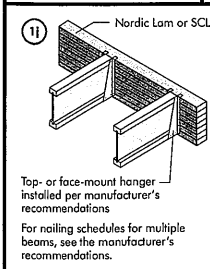
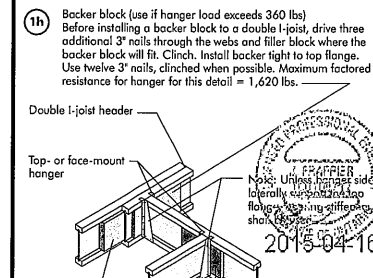
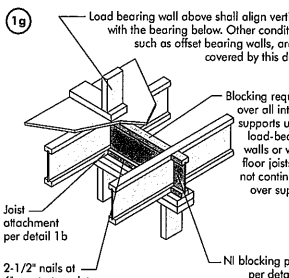
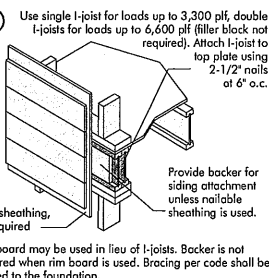
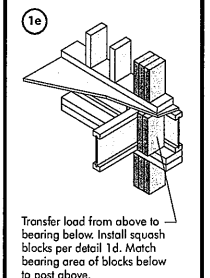
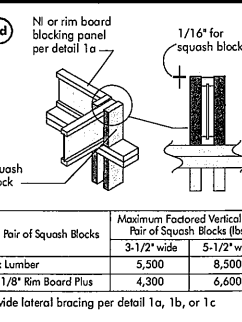
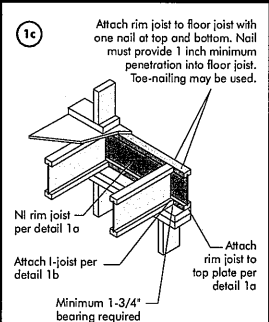
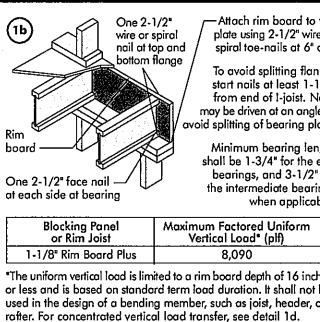
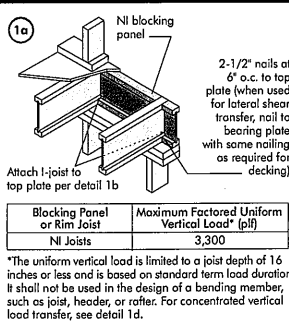
### INSTALLING NORDIC I-JOISTS

- Before laying out floor system components, verify that I-joist flange widths match hanger widths. If not, supplier.
- Except for cutting to length, I-joist flanges should **never** be cut, drilled, or notched.
- Install I-joists so that top and bottom flanges are within 1/2 inch of true vertical alignment.
- I-joists must be anchored securely to supports before floor sheathing is attached, and supports for multiple spans must be level.
- Minimum bearing lengths: 1-3/4 inches for end bearings and 3-1/2 inches for intermediate bearings.
- When using hangers, seat I-joists firmly in hanger bottoms to minimize settlement.
- Leave a 1/16-inch gap between the I-joist end and a header.
- Concentrated loads greater than those that can normally be expected in residential construction should only be applied to the top surface of the top flange. Normal concentrated loads include track lighting fixtures, audio equipment and security cameras. Never suspend unusual or heavy loads from the I-joist's bottom flange. Whenever possible, suspend all concentrated loads from the top of the I-joist. Or, attach the load to blocking that has been securely fastened to the I-joist webs.
- Never install I-joists where they will be permanently exposed to weather, or where they will remain in direct contact with concrete or masonry.
- Restrain ends of floor joists to prevent rollover. Use rim board, rim joists or I-joist blocking panels.
- For I-joists installed over and beneath bearing walls, use full depth blocking panels, rim board, or squash blocks (cripple members) to transfer gravity loads through the floor system to the wall or foundation below.
- Due to shrinkage, common framing lumber set on edge may **never** be used as blocking or rim boards. I-joist blocking panels or other engineered wood products – such as rim board – must be cut to fit between the I-joists, and an I-joist-compatible depth selected.
- Provide permanent lateral support of the bottom flange of all I-joists at interior supports of multiple-span joists. Similarly, support the bottom flange of all cantilevered I-joists at the end support next to the cantilever extension. In the completed structure, the gypsum wallboard ceiling provides this lateral support. Until the final finished ceiling is applied, temporary bracing or struts must be used.
- If square-edge panels are used, edges must be supported between I-joists with 2x4 blocking. Glue panels to blocking to minimize squeaks. Blocking is not required under structural finish flooring, such as wood strip flooring, or if a separate underlayment layer is installed.
- Nail spacing: Space nails installed to the flange's top face in accordance with the applicable building code requirements or approved building plans.

FIGURE 1  
TYPICAL NORDIC I-JOIST FLOOR FRAMING AND CONSTRUCTION DETAILS



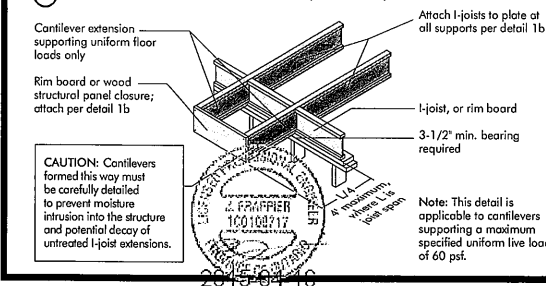
All nails shown in the above details are assumed to be common wire nails unless otherwise noted. 3" (10.122" dia.) common spiral nails may be substituted for 2-1/2" (6.35" dia.) common wire nails. Framing lumber assumed to be Spruce-Pine-Fir No. 2 or better. Individual components not shown to scale for clarity.



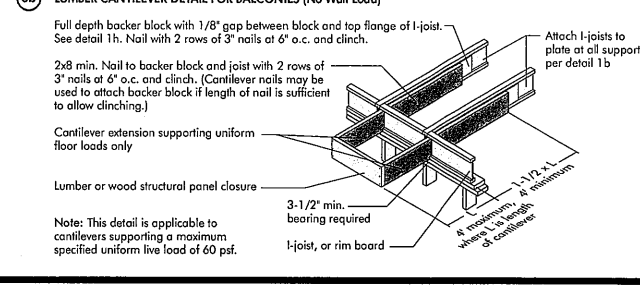


# CANTILEVER DETAILS FOR BALCONIES (NO WALL LOAD)

## 3a I-JOIST CANTILEVER DETAIL FOR BALCONIES (No Wall Load)

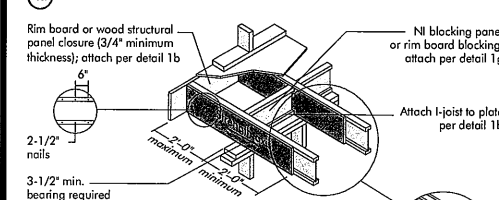


## 3b LUMBER CANTILEVER DETAIL FOR BALCONIES (No Wall Load)

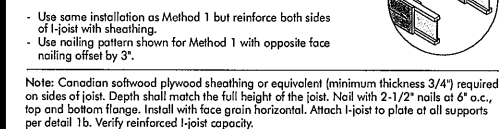


# CANTILEVER DETAILS FOR VERTICAL BUILDING OFFSET (CONCENTRATED WALL LOAD)

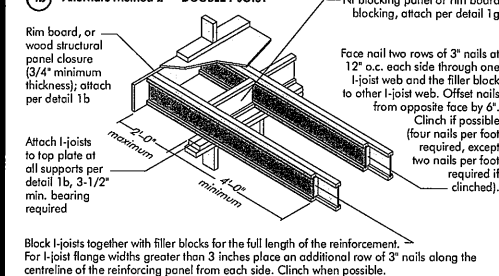
## 3a Method 1 — SHEATHING REINFORCEMENT ONE SIDE



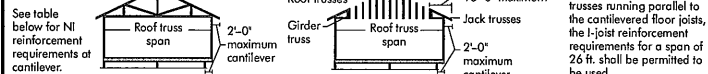
## Method 2 — SHEATHING REINFORCEMENT TWO SIDES



## 3b Alternate Method 2 — DOUBLE I-JOIST



## FIGURE 4 (continued)



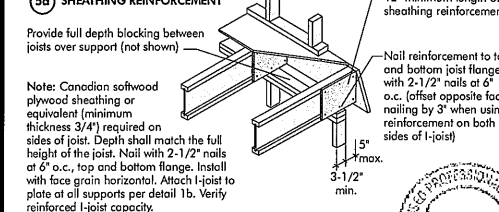
## CANTILEVER REINFORCEMENT METHODS ALLOWED

JOIST DEPTH (in.)	ROOF TRUSS SPAN (ft)	ROOF LOADING (UNFACTORED)											
		LL = 30 psf, DL = 15 psf				LL = 40 psf, DL = 15 psf				LL = 50 psf, DL = 15 psf			
		JOIST SPACING (in.)				JOIST SPACING (in.)				JOIST SPACING (in.)			
9-1/2"	26	N	N	1	2	N	1	2	X	N	2	X	X
	28	N	N	1	2	N	1	2	X	N	2	X	X
	30	N	N	1	2	N	1	2	X	N	2	X	X
	32	N	N	1	2	N	1	2	X	N	2	X	X
11-7/8"	26	N	N	1	2	N	1	2	X	N	2	X	X
	28	N	N	1	2	N	1	2	X	N	2	X	X
	30	N	N	1	2	N	1	2	X	N	2	X	X
	32	N	N	1	2	N	1	2	X	N	2	X	X
14"	26	N	N	1	2	N	1	2	X	N	2	X	X
	28	N	N	1	2	N	1	2	X	N	2	X	X
	30	N	N	1	2	N	1	2	X	N	2	X	X
	32	N	N	1	2	N	1	2	X	N	2	X	X
16"	26	N	N	1	2	N	1	2	X	N	2	X	X
	28	N	N	1	2	N	1	2	X	N	2	X	X
	30	N	N	1	2	N	1	2	X	N	2	X	X
	32	N	N	1	2	N	1	2	X	N	2	X	X

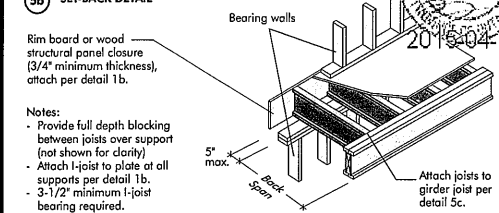
1. N = No reinforcement required.
2. = NI reinforced with 3/4" wood structural panel on one side only.
3. = NI reinforced with 3/4" wood structural panel on both sides, or double I-joist.
4. For larger openings, or multiple 3'-0" wide openings spaced less than 6'-0" o.c., additional joists beneath the opening's cripple studs may be required.
5. Table applies to joists 12" to 24" o.c. that meet the floor span requirements for a design live load of 40 psf and dead load of 15 psf, and a live load deflection limit of L/480. Use 12" o.c. requirements for lesser spacing.
6. Maximum design load shall be 15 psf dead load, 55 psf floor total load, and 80 psf wall load. Wall load is based on 3'-0" maximum width window or door openings.
7. For conventional roof construction using a ridge beam, the Roof Truss Span column above is equivalent to the distance between the supporting wall and the ridge beam. When the roof is framed using a ridge beam, the Roof Truss Span is equivalent to the distance between the supporting walls as if a truss is used.
8. Cantilevered joists supporting girder trusses or roof beams may require additional reinforcing.

# BRICK CANTILEVER DETAILS FOR VERTICAL BUILDING OFFSET (CONCENTRATED WALL LOAD)

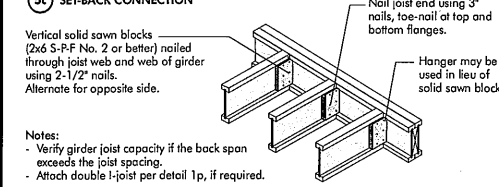
## 3a SHEATHING REINFORCEMENT



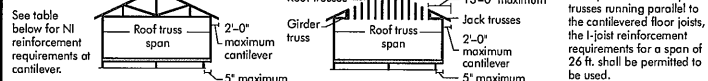
## 3b SET-BACK DETAIL



## 3c SET-BACK CONNECTION



## FIGURE 5 (continued)



## BRICK CANTILEVER REINFORCEMENT METHODS ALLOWED

JOIST DEPTH (in.)	ROOF TRUSS SPAN (ft)	ROOF LOADING (UNFACTORED)											
		LL = 30 psf, DL = 15 psf				LL = 40 psf, DL = 15 psf				LL = 50 psf, DL = 15 psf			
		JOIST SPACING (in.)				JOIST SPACING (in.)				JOIST SPACING (in.)			
9-1/2"	26	N	1	X	X	N	1	X	X	N	2	X	X
	28	N	1	X	X	N	1	X	X	N	2	X	X
	30	N	1	X	X	N	1	X	X	N	2	X	X
	32	N	1	X	X	N	1	X	X	N	2	X	X
11-7/8"	26	N	1	X	X	N	1	X	X	N	2	X	X
	28	N	1	X	X	N	1	X	X	N	2	X	X
	30	N	1	X	X	N	1	X	X	N	2	X	X
	32	N	1	X	X	N	1	X	X	N	2	X	X
14"	26	N	1	X	X	N	1	X	X	N	2	X	X
	28	N	1	X	X	N	1	X	X	N	2	X	X
	30	N	1	X	X	N	1	X	X	N	2	X	X
	32	N	1	X	X	N	1	X	X	N	2	X	X
16"	26	N	1	X	X	N	1	X	X	N	2	X	X
	28	N	1	X	X	N	1	X	X	N	2	X	X
	30	N	1	X	X	N	1	X	X	N	2	X	X
	32	N	1	X	X	N	1	X	X	N	2	X	X

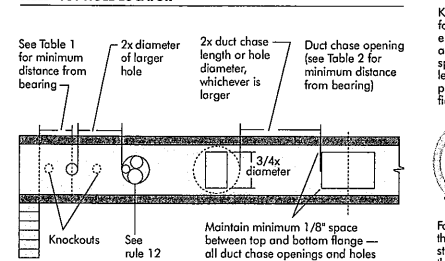
1. N = No reinforcement required.
2. = NI reinforced with 3/4" wood structural panel on one side only.
3. = NI reinforced with 3/4" wood structural panel on both sides, or double I-joist.
4. For larger openings, or multiple 3'-0" wide openings spaced less than 6'-0" o.c., additional joists beneath the opening's cripple studs may be required.
5. Table applies to joists 12" to 24" o.c. that meet the floor span requirements for a design live load of 40 psf and dead load of 15 psf, and a live load deflection limit of L/480. Use 12" o.c. requirements for lesser spacing.
6. Maximum design load shall be 15 psf dead load, 55 psf floor total load, and 80 psf wall load. Wall load is based on 3'-0" maximum width window or door openings.
7. For conventional roof construction using a ridge beam, the Roof Truss Span column above is equivalent to the distance between the supporting wall and the ridge beam. When the roof is framed using a ridge beam, the Roof Truss Span is equivalent to the distance between the supporting walls as if a truss is used.
8. Cantilevered joists supporting girder trusses or roof beams may require additional reinforcing.

# WEB HOLES

## RULES FOR CUTTING HOLES AND DUCT CHASE OPENINGS:

1. The distance between the inside edge of the support and the centreline of any hole or duct chase opening shall be in compliance with the requirements of Table 1 or 2, respectively.
2. I-joist top and bottom flanges must NEVER be cut, notched, or otherwise modified.
3. Whenever possible, field-cut holes should be centred on the middle of the web.
4. The maximum size hole or the maximum depth of a duct chase opening that can be cut into an I-joist web shall equal the clear distance between the flanges of the I-joist minus 1/4 inch. A minimum of 1/8 inch should always be maintained between the top or bottom of the hole or opening and the adjacent I-joist flange.
5. The sides of square holes or longest sides of rectangular holes should not exceed 3/4 of the diameter of the maximum round hole permitted at that location.
6. Where more than one hole is necessary, the distance between adjacent hole edges shall exceed twice the diameter of the largest round hole or twice the size of the largest square hole (or twice the length of the longest side of the longest rectangular hole or duct chase opening) and each hole and duct chase opening shall be sized and located in compliance with the requirements of Tables 1 and 2, respectively.
7. A knockout is NOT considered a hole, may be utilized anywhere it occurs, and may be ignored for purposes of calculating minimum distances between holes and/or duct chase openings.
8. Holes measuring 1-1/2 inches or smaller shall be permitted anywhere in a cantilevered section of a joist. Holes of greater size may be permitted subject to verification.
9. A 1-1/2 inch hole or smaller can be placed anywhere in the web provided that it meets the requirements of rule number 6 above.
10. All holes and duct chase openings shall be cut in a workman-like manner in accordance with the restrictions listed above and as illustrated in Figure 7.
11. Limit three maximum size holes per span, of which one may be a duct chase opening.
12. A group of round holes at approximately the same location shall be permitted if they meet the requirements for a single round hole circumscribed around them.

## FIGURE 7 FIELD-CUT HOLE LOCATOR



A knockout is NOT considered a hole, may be utilized wherever it occurs and may be ignored for purposes of calculating minimum distances between holes.

TABLE 1  
LOCATION OF CIRCULAR HOLES IN JOIST WEBS  
Simple or Multiple Span for Dead Loads up to 15 psf and Live Loads up to 40 psf

Joist Depth	Joist Series	Minimum distance from inside face of any support to centre of hole (ft-in.)											
		2	3	4	5	6	7	8	9	10	11	12	13
9-1/2"	N-20	0-7"	1-6"	2-10"	4-3"	5-8"	6-0"	---	---	---	---	---	13-6"
	N-40x	0-7"	1-6"	2-10"	4-3"	5-8"	6-0"	---	---	---	---	---	14-9"
	N-60	1-3"	2-4"	4-0"	5-4"	7-0"	7-3"	---	---	---	---	---	14-11"
	N-70	2-0"	3-4"	4-0"	6-3"	8-0"	8-4"	---	---	---	---	---	15-7"
11-7/8"	N-20	0-7"	1-6"	2-10"	4-3"	5-8"	6-0"	---	---	---	---	---	13-6"
	N-40x	0-7"	1-6"	2-10"	4-3"	5-8"	6-0"	---	---	---	---	---	14-9"
	N-60	1-3"	2-4"	4-0"	5-4"	7-0"	7-3"	---	---	---	---	---	14-11"
	N-70	2-0"	3-4"	4-0"	6-3"	8-0"	8-4"	---	---	---	---	---	15-7"
14"	N-20	0-7"	1-6"	2-10"	4-3"	5-8"	6-0"	---	---	---	---	---	13-6"
	N-40x	0-7"	1-6"	2-10"	4-3"	5-8"	6-0"	---	---	---	---	---	14-9"
	N-60	1-3"	2-4"	4-0"	5-4"	7-0"	7-3"	---	---	---	---	---	14-11"
	N-70	2-0"	3-4"	4-0"	6-3"	8-0"	8-4"	---	---	---	---	---	15-7"
16"	N-20	0-7"	1-6"	2-10"	4-3"	5-8"	6-0"	---	---	---	---	---	13-6"
	N-40x	0-7"	1-6"	2-10"	4-3"	5-8"	6-0"	---	---	---	---	---	14-9"
	N-60	1-3"	2-4"	4-0"	5-4"	7-0"	7-3"	---	---	---	---	---	14-11"
	N-70	2-0"	3-4"	4-0"	6-3"	8-0"	8-4"	---	---	---	---	---	15-7"

1. Above table may be used for I-joist spacing of 24 inches on centre or less.
2. Hole location distance is measured from inside face of supports to centre of hole.
3. Distances in this chart are based on uniformly loaded joists.

## OPTIONAL:

The above table is based on the I-joists used at their maximum span. If the I-joists are placed at less than their full maximum span (see Maximum Span Table), the minimum distance from the centreline of the hole to the face of any support (D) as given above may be reduced as follows:

$$D_{\text{reduced}} = \frac{D_{\text{actual}} \times D}{D_{\text{max}}}$$

Where:  
 $D_{\text{reduced}}$  = Distance from the inside face of any support to centre of hole, reduced for less-than-maximum span applications.  
 $D_{\text{actual}}$  = The actual measured span distance between the inside faces of supports (ft).  
 $D$  = Span Adjustment Factor given in this table.  
 $D_{\text{max}}$  = The minimum distance from the inside face of any support to centre of hole from this table.  
 If  $D_{\text{actual}}$  is greater than 1, use 1 in the above calculation for  $D_{\text{actual}}$ .

TABLE 2  
DUCT CHASE OPENING SIZES AND LOCATIONS — Simple Span Only

		Minimum distance from inside face of any support to centre of opening (ft.-in.)											
Joist Depth	Joist Series	Duct chase length (in.)											
		8	10	12	14	16	18	20	22	24	26	28	30
9-1/2"	N-20	4-1"	4-5"	4-10"	5-4"	5-8"	6-1"	6-6"	7-1"	7-5"	7-9"	8-3"	8-7"
	N-40	5-2"	5-2"	6-0"	6-5"	6-10"	6-10"	6-10"	6-10"	6-10"	6-10"	6-10"	6-10"
	N-60	5-1"	5-1"	5-10"	6-3"	6-7"	6-7"	6-7"	6-7"	6-7"	6-7"	6-7"	6-7"
	N-70	5-9"	5-9"	6-6"	8-0"	8-10"	8-10"	8-10"	8-10"	8-10"	8-10"	8-10"	8-10"
	N-80	5-3"	5-8"	6-6"	7-1"	7-5"	7-9"	8-3"	8-7"	9-1"	9-5"	9-9"	9-4"
11-7/8"	N-20	4-1"	4-5"	4-10"	5-4"	5-8"	6-1"	6-6"	7-1"	7-5"	7-9"	8-3"	8-7"
	N-40	5-2"	5-2"	6-0"	6-5"	6-10"	6-10"	6-10"	6-10"	6-10"	6-10"	6-10"	6-10"
	N-60	5-1"	5-1"	5-10"	6-3"	6-7"	6-7"	6-7"	6-7"	6-7"	6-7"	6-7"	6-7"
	N-70	7-1"	7-1"	7-9"	8-3"	8-7"	9-1"	9-5"	9-9"	10-3"	10-7"	11-1"	10-6"
	N-80	7-3"	7-2"	7-9"	8-3"	8-7"	9-1"	9-5"	9-9"	10-3"	10-7"	11-1"	10-6"
14"	N-20	4-1"	4-5"	4-10"	5-4"	5-8"	6-1"	6-6"	7-1"	7-5"	7-9"	8-3"	8-7"
	N-40	5-2"	5-2"	6-0"	6-5"	6-10"	6-10"	6-10"	6-10"	6-10"	6-10"	6-10"	6-10"
	N-60	5-1"	5-1"	5-10"	6-3"	6-7"	6-7"	6-7"	6-7"	6-7"	6-7"	6-7"	6-7"
	N-70	8-1"	8-1"	8-9"	9-3"	9-7"	10-1"	10-5"	10-9"	11-3"	11-7"	12-1"	11-6"
	N-80	8-3"	8-2"	8-9"	9-3"	9-7"	10-1"	10-5"	10-9"	11-3"	11-7"	12-1"	11-6"
16"	N-20	4-1"	4-5"	4-10"	5-4"	5-8"	6-1"	6-6"	7-1"	7-5"	7-9"	8-3"	8-7"
	N-40	5-2"	5-2"	6-0"	6-5"	6-10"	6-10"	6-10"	6-10"	6-10"	6-10"	6-10"	6-10"
	N-60	5-1"	5-1"	5-10"	6-3"	6-7"	6-7"	6-7"	6-7"	6-7"	6-7"	6-7"	6-7"
	N-70	10-3"	9-8"	10-6"	10-6"	10-11"	11-5"	11-9"	12-3"	12-7"	13-1"	13-5"	12-11"
	N-80	10-2"	10-8"	11-2"	11-2"	12-1"	12-6"	13-2"	13-7"	14-1"	14-5"	14-9"	14-0"



Refer to the Installation Guide for Residential Floors for additional information.  
CCMC EVALUATION REPORT 13032-R

## WEB HOLE SPECIFICATIONS

RULES FOR CUTTING HOLES AND DUCT CHASE OPENINGS:

- The distance between the inside edge of the support and the centreline of any hole or duct chase opening shall be in compliance with the requirements of Table 1 or 2, respectively.
- I-joist top and bottom flanges must NEVER be cut, notched, or otherwise modified.
- Whenever possible, field-cut holes should be centred on the middle of the web.
- The maximum size hole or the maximum depth of a duct chase opening that can be cut into an I-joist web shall equal the clear distance between the flanges of the I-joist minus 1/4 inch. A minimum of 1/8 inch should always be maintained between the top or bottom of the hole or opening and the adjacent I-joist flange.

- The sides of square holes or longest sides of rectangular holes should not exceed 3/4 of the diameter of the maximum round hole permitted at that location.
- Where more than one hole is necessary, the distance between adjacent hole edges shall exceed twice the diameter of the largest round hole or twice the size of the largest square hole (or twice the length of the longest side of the longest rectangular hole or duct chase opening) and each hole and duct chase opening shall be sized and located in compliance with the requirements of Tables 1 and 2, respectively.
- A knockout is **not** considered a hole, may be utilized anywhere it occurs, and may be ignored for purposes of calculating minimum distances between holes and/or duct chase openings.
- Holes measuring 1-1/2 inches or smaller are permitted anywhere in a cantilevered section of a joist. Holes of greater size may be permitted subject to verification.

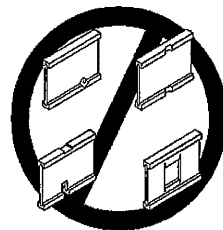
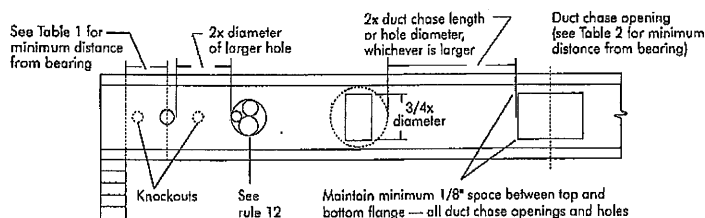
- A 1-1/2 inch hole or smaller can be placed anywhere in the web provided that it meets the requirements of rule number 6 above.
- All holes and duct chase openings shall be cut in a workman-like manner in accordance with the restrictions listed above and as illustrated in Figure 7.
- Limit three maximum size holes per span, of which one may be a duct chase opening.
- A group of round holes at approximately the same location shall be permitted if they meet the requirements for a single round hole circumscribed around them.

TABLE 1  
**LOCATION OF CIRCULAR HOLES IN JOIST WEBS**  
Simple or Multiple Span for Dead Loads up to 15 psf and Live Loads up to 40 psf

Joist Depth	Joist Series	Minimum Distance from Inside Face of Any Support to Centre of Hole (ft - in.)											
		Round Hole Diameter (in.)											
		2	3	4	5	6	6-1/4	7	8	8-5/8	9	10	10-3/4
9-1/2"	NI-20	0-7"	1-6"	2-10"	4-3"	5-8"	6-0"	---	---	---	---	---	---
	NI-40x	0-7"	1-6"	3-0"	4-4"	6-0"	6-4"	---	---	---	---	---	---
	NI-60	1-3"	2-6"	4-0"	5-4"	7-0"	7-5"	---	---	---	---	---	---
	NI-70	2-0"	3-4"	4-9"	6-3"	8-0"	8-5"	---	---	---	---	---	---
	NI-80	2-3"	3-6"	5-0"	6-6"	8-2"	8-8"	---	---	---	---	---	---
11-7/8"	NI-20	0-7"	0-8"	1-0"	2-4"	3-8"	4-0"	5-0"	6-6"	7-0"	---	---	---
	NI-40x	0-7"	0-8"	1-3"	2-8"	4-0"	4-4"	5-5"	7-0"	7-0"	---	---	---
	NI-60	0-7"	1-8"	3-0"	4-3"	5-9"	6-0"	7-3"	8-10"	10-0"	---	---	---
	NI-70	1-3"	2-6"	4-0"	5-4"	6-9"	7-2"	8-4"	10-0"	11-2"	---	---	---
	NI-80	1-6"	2-10"	4-2"	5-6"	7-0"	7-5"	8-6"	10-3"	11-4"	---	---	---
14"	NI-20	0-7"	0-8"	1-5"	3-2"	4-10"	5-4"	6-9"	8-9"	10-2"	---	---	---
	NI-40x	0-7"	0-8"	0-9"	2-5"	4-4"	4-9"	6-3"	---	---	---	---	---
	NI-60	0-7"	0-8"	0-8"	1-0"	2-4"	2-9"	3-9"	5-2"	6-0"	6-6"	8-3"	10-2"
	NI-70	0-7"	0-8"	1-8"	3-0"	4-3"	4-8"	5-8"	7-2"	8-0"	8-8"	10-4"	11-9"
	NI-80	0-7"	0-8"	1-10"	3-0"	4-5"	5-10"	6-2"	7-3"	8-9"	9-0"	10-4"	12-0"
16"	NI-20	0-7"	0-8"	0-10"	2-5"	4-0"	4-5"	5-9"	7-5"	8-0"	9-4"	11-4"	12-11"
	NI-40x	0-7"	0-8"	0-8"	2-0"	3-9"	4-2"	5-5"	7-3"	8-5"	9-2"	---	---
	NI-60	0-7"	0-8"	0-8"	1-6"	2-10"	3-2"	4-2"	5-6"	6-4"	7-0"	8-5"	9-8"
	NI-70	0-7"	1-0"	2-3"	3-6"	4-10"	5-3"	6-3"	7-8"	8-6"	9-2"	10-8"	12-0"
	NI-80	0-7"	1-3"	2-6"	3-10"	5-3"	5-6"	6-6"	8-0"	9-0"	9-5"	11-0"	12-3"

- Above table may be used for I-joist spacing of 24 inches on centre or less.
- Hole location distance is measured from inside face of supports to centre of hole.
- Distances in this chart are based on uniformly loaded joists.
- The above table is based on the I-joists being used at their maximum spans. The minimum distance as given above may be reduced for shorter spans; contact your local distributor.

FIGURE 7  
**FIELD-CUT HOLE LOCATOR**



Knockouts are precored holes provided for the contractor's convenience to install electrical or small plumbing lines. They are 1-1/2 inches in diameter, and are spaced 15 inches on centre along the length of the I-joist. Where possible, it is preferable to use knockouts instead of field-cut holes.

Never drill, cut or notch the flange, or over-cut the web.

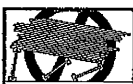
Holes in webs should be cut with a sharp saw.

For rectangular holes, avoid over-cutting the corners, as this can cause unnecessary stress concentrations. Slightly rounding the corners is recommended. Starting the rectangular hole by drilling a 1-inch diameter hole in each of the four corners and then making the cuts between the holes is another good method to minimize damage to the I-joist.

## SAFETY AND CONSTRUCTION PRECAUTIONS



Do not walk on I-joists until fully fastened and braced, or serious injuries can result.



Never stack building materials over unshelved I-joists. Once shelved, do not over-stress I-joists with concentrated loads from building materials.

WARNING: I-joists are not stable until completely installed, and will not carry any load until fully braced and sheathed.

AVOID ACCIDENTS BY FOLLOWING THESE IMPORTANT GUIDELINES:

- Brace and nail each I-joist as it is installed, using hangers, blocking panels, rim board, and/or cross-bridging at joist ends. When I-joists are applied continuous over interior supports and a load-bearing wall is planned at that location, blocking will be required at the interior support.
- When the building is completed, the floor sheathing will provide lateral support for the top flanges of the I-joists. Until this sheathing is applied, temporary bracing, often called struts, or temporary sheathing must be applied to prevent I-joist rollover or buckling.
  - Temporary bracing or struts must be 1x4 inch minimum, at least 8 feet long and spaced no more than 8 feet on centre, and must be secured with a minimum of two 2-1/2" nails fastened to the top surface of each I-joist. Nail the bracing to a lateral restraint at the end of each bay. Lap ends of adjoining bracing over at least two I-joists.
  - Or, sheathing (temporary or permanent) can be nailed to the top flange of the first 4 feet of I-joists at the end of the bay.
- For cantilevered I-joists, brace top and bottom flanges, and brace ends with closure panels, rim board, or cross-bridging.
- Install and fully nail permanent sheathing to each I-joist before placing loads on the floor system. Then, stack building materials over beams or walls only.
- Never install a damaged I-joist.

Improper storage or installation, failure to follow applicable building codes, failure to follow span ratings for Nordic I-joists, failure to follow allowable hole sizes and locations, or failure to use web stiffeners when required can result in serious accidents. Follow these installation guidelines carefully.



## PRODUCT WARRANTY

Chantiers Chibougamau guarantees that, in accordance with our specifications, Nordic products are free from manufacturing defects in material and workmanship.

Furthermore, Chantiers Chibougamau warrants that our products, when utilized in accordance with our handling and installation instructions, will meet or exceed our specifications for the lifetime of the structure.

**1a** NI blocking panel

Blocking Panel or Rim Joist	Maximum Factored Uniform Vertical Load* (plf)
NI Joists	3,300

\*The uniform vertical load is limited to a joist depth of 16 inches or less and is based on standard term load duration. It shall not be used in the design of a bending member, such as joist, header, or rafter. For concentrated vertical load transfer, see detail 1d.

Attach I-joist to top plate per detail 1b

2-1/2" nails at 6" o.c. to top plate (when used for lateral shear transfer, nail to bearing plate with some nailing as required for decking)

**1b** Rim board

Blocking Panel or Rim Joist	Maximum Factored Uniform Vertical Load* (plf)
1-1/8" Rim Board Plus	8,090

\*The uniform vertical load is limited to a rim board depth of 16 inches or less and is based on standard term load duration. It shall not be used in the design of a bending member, such as joist, header, or rafter. For concentrated vertical load transfer, see detail 1d.

One 2-1/2" wire or spiral nail at top and bottom flange

Attach rim board to top plate using 2-1/2" wire or spiral toe-nails at 6" o.c.

To avoid splitting flange, start nails at least 1-1/2" from end of I-joist. Nails may be driven at an angle to avoid splitting of bearing plate.

Minimum bearing length shall be 1-3/4" for the end bearings, and 3-1/2" for the intermediate bearings when applicable.

**1d** NI or rim board blocking panel per detail 1a

1-1/8" for squash blocks

Pair of Squash Blocks	Maximum Factored Vertical Load per Pair of Squash Blocks (lbs)
2x Lumber	5,500
1-1/8" Rim Board Plus	6,600

Provide lateral bracing per detail 1a or 1b

**1e** Transfer load from above to bearing below. Install squash blocks per detail 1d. Match bearing area of blocks below to post above.

**1g** Joist attachment per detail 1b

Load bearing wall above shall align vertically with the bearing below. Other conditions, such as offset bearing walls, are not covered by this detail.

Blocking required over all interior supports under load-bearing walls or when floor joists are not continuous over support

NI blocking panel per detail 1a

**1h** Backer block (use if hanger load exceeds 360 lbs). Before installing a backer block to a double I-joist, drive three additional 3" nails through the webs and filler block where the backer block will fit. Clinch. Install backer tight to top flange. Use twelve 3" nails, clinched when possible. Maximum factored resistance for hanger for this detail = 1,620 lbs.

Flange Width	Material Thickness Required*	Minimum Depth**
2-1/2"	1"	5-1/2"
3-1/2"	1-1/2"	7-1/4"

\* Minimum grade for backer block material shall be S-P-F No. 2 or better for solid sawn lumber and wood structural panels conforming to CAN/CSA-C325 or CAN/CSA-O437 Standard.

\*\* For face-mount hangers use net joist depth minus 3-1/4" for joists with 1-1/2" thick flanges. For 2" thick flanges use net depth minus 4-1/4".

**1i** Nordic Lam or Structural Composite Lumber (SCL)

For nailing schedules for multiple beams, see the manufacturer's recommendations.

Top- or face-mount hanger installed per manufacturer's recommendations

NOTE: Unless hanger sides laterally support the top flange, bearing stiffeners shall be used.

BACKER BLOCKS (Blocks must be long enough to permit required nailing without splitting)

**1k** 2x plate flush with inside face of wall or beam. 1/8" overhang allowed past inside face of wall or beam.

NOTE: Unless hanger sides laterally support the top flange, bearing stiffeners shall be used.

Top-mount hanger installed per manufacturer's recommendations

**1m** Multiple I-joist header with full depth filler block shown. Nordic Lam or SCL headers may also be used. Verify double I-joist capacity to support concentrated loads.

Backer block attached per detail 1h. Nail with twelve 3" nails, clinch when possible.

Install hanger per manufacturer's recommendations

Maximum support capacity = 1,620 lbs.

**1n** Do not bevel-cut joist beyond inside face of wall

Attach I-joist per detail 1b

NOTE: Blocking required at bearing for lateral support, not shown for clarity.

**1r** Lumber 2x4 min., extend block to face of adjacent web. Two 2-1/2" spiral nails from each web to lumber piece, alternate on opposite side.

NI blocking panel

OPTIONAL: Minimum 1x4 inch strap applied to underside of joist at blocking line or 1/2 inch minimum gypsum ceiling attached to underside of joists.

**1p** FILLER BLOCK REQUIREMENTS FOR DOUBLE I-JOIST CONSTRUCTION

Filler block

Offset nails from opposite face by 6"

1/8" to 1/4" gap between top flange and filler block

NOTES:

- Support back of I-joist web during nailing to prevent damage to web/flange connection.
- Leave a 1/8 to 1/4-inch gap between top of filler block and bottom of top I-joist flange.
- Filler block is required between joists for full length of span.
- Nail joists together with two rows of 3" nails at 12 inches o.c. (clinched when possible) on each side of the double I-joist. Total of four nails per foot required. If nails can be clinched, only two nails per foot are required.
- The maximum factored load that may be applied to one side of the double joist using this detail is 860 lb/ft. Verify double I-joist capacity.

**1s** One 2-1/2" nail at top and bottom flange

2x4 min. (1/8" gap minimum)

Two 2-1/2" nails from each web to lumber piece

One 2-1/2" nail one side only

Flange Size	Net Depth	Filler Block Size
2-1/2" x 1-1/2"	9-1/2"	2-1/8" x 6"
	11-7/8"	2-1/8" x 8"
	14"	2-1/8" x 10"
	16"	2-1/8" x 12"
3-1/2" x 1-1/2"	9-1/2"	3" x 6"
	11-7/8"	3" x 8"
	14"	3" x 10"
	16"	3" x 12"
3-1/2" x 2"	11-7/8"	3" x 7"
	14"	3" x 9"
	16"	3" x 11"

NOTES:

- In some local codes, blocking is prescriptively required in the first joist space (or first and second joist space) next to the starter joist. Where required, see local code requirements for spacing of the blocking.
- All nails are common spiral in this detail.

All nails shown in the above details are assumed to be common wire nails unless otherwise noted. 3" (0.122" dia.) common spiral nails may be substituted for 2-1/2" (0.128" dia.) common wire nails. Framing lumber assumed to be Spruce-Pine-Fir No. 2 or better. Individual components not shown to scale for clarity.

## WEB STIFFENERS

RECOMMENDATIONS:

- A **bearing stiffener** is required in all engineered applications with factored reactions greater than shown in the I-joist properties table found of the I-joist Construction Guide (C101). The gap between the stiffener and the flange is at the top.
- A **bearing stiffener** is required when the I-joist is supported in a hanger and the sides of the hanger do not extend up to, and support, the top flange. The gap between the stiffener and flange is at the top.
- A **load stiffener** is required at locations where a factored concentrated load greater than 2,370 lbs is applied to the top flange between supports, or in the case of a cantilever, anywhere between the cantilever tip and the support. These values are for standard term load duration, and may be adjusted for other load durations as permitted by the code. The gap between the stiffener and the flange is at the bottom.

FIGURE 2  
**WEB STIFFENER INSTALLATION DETAILS**

Flange width 2-1/2" or 3-1/2"

Approx. 2" 1/8"-1/4" Gap

(4) 2-1/2" nails, 3" nails required for I-joists with 3-1/2" flange width

No Gap

CONCENTRATED LOAD (Load stiffener)

Tight Joint No Gap

END BEARING (Bearing stiffener)

Gap

Tight Joint No Gap

See the adjacent table for web stiffener size requirements

Flange Width	Web Stiffener Size Each Side of Web
2-1/2"	1" x 2-5/16" minimum width
3-1/2"	1-1/2" x 2-5/16" minimum width

## CANTILEVER DETAILS FOR VERTICAL BUILDING OFFSET

**4a** Method 1 — SHEATHING REINFORCEMENT ONE SIDE

Rim board or wood structural panel closure (3/4" minimum thickness); attach per detail 1b

NI blocking panel or rim board blocking, attach per detail 1g

Attach I-joist to plate per detail 1b

2-1/2" min. bearing required

Method 2 — SHEATHING REINFORCEMENT TWO SIDES

Use same installation as Method 1 but reinforce both sides of I-joist with sheathing.

Use nailing pattern shown for Method 1 with opposite face nailing offset by 3".

NOTE: Canadian softwood plywood sheathing or equivalent (minimum thickness 3/4") required on sides of joist. Depth shall match the full height of the joist. Nail with 2-1/2" nails at 6" o.c., top and bottom flange. Install with face grain horizontal. Attach I-joist to plate of all supports per detail 1b. Verify reinforced I-joist capacity.

## RIM BOARD INSTALLATION DETAILS

**8a** ATTACHMENT DETAILS WHERE RIM BOARDS ABUT

Rim Board Joint Between Floor Joists

(1) 2-1/2" nail top and bottom (typical)

2-1/2" nails at 6" o.c. (typical)

Rim board joint

Rim Board Joint at Corner

2-1/2" toe-nails at 6" o.c. (typical)

1-1/2"

1-1/2"

**8b** TOE-NAIL CONNECTION AT RIM BOARD

Rim board

Top or sole plate

30°

2/3

# Triple 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP

## 2ND FLR FRAMING\Dropped Beams\B8(i2378) (Dropped Beam)

**PASSED**

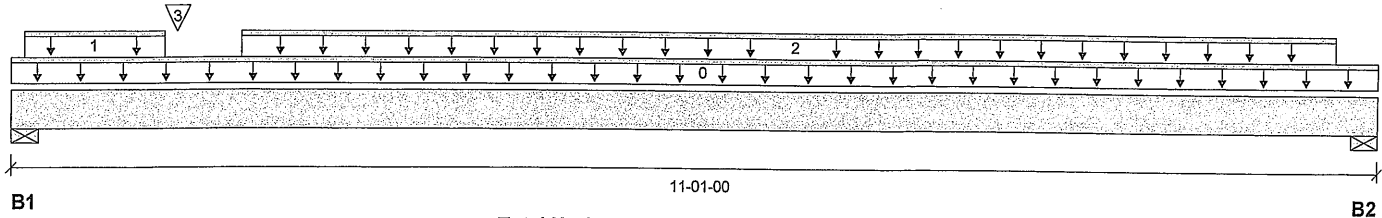
BC CALC® Member Report  
Build 7493

Dry | 1 span | No cant.

October 8, 2020 17:23:39

Job name:  
Address:  
City, Province, Postal Code: RICHMOND HILL  
Customer:  
Code reports: CCMC 12472-R

File name: 38-10.mmdl  
Description: 2ND FLR FRAMING\Dropped Beams\B8(i2378)  
Specifier:  
Designer: LBV  
Company:



Total Horizontal Product Length = 11-01-00

### Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B1, 3-1/2"	1322 / 0	746 / 0		
B2, 3-1/2"	1421 / 0	796 / 0		

### Load Summary

Tag	Description	Load Type	Ref.	Start	End	Loc.	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Tributary
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	11-01-00	Top		14			00-00-00
1	Bk2(i2880)	Unf. Lin. (lb/ft)	L	00-01-04	01-02-12	Top	29	15			n/a
2	Smoothed Load	Unf. Lin. (lb/ft)	L	01-10-00	10-08-12	Top	247	124			n/a
3	J2(i2357)	Conc. Pt. (lbs)	L	01-04-00	01-04-00	Top	278	139			n/a

### Controls Summary

	Factored Demand	Factored Resistance	Demand/Resistance	Case	Location
Pos. Moment	8334 ft-lbs	36222 ft-lbs	23.0%	1	05-04-00
End Shear	2870 lbs	17356 lbs	16.5%	1	10-00-00
Total Load Deflection	L/799 (0.159")	n/a	30.0%	4	05-05-04
Live Load Deflection	L/999 (0.102")	n/a	n/a	5	05-05-04
Max Defl.	0.159"	n/a	n/a	4	05-05-04
Span / Depth	13.4				

### Bearing Supports

	Dim. (LxW)	Demand	Demand/Resistance Support	Demand/Resistance Member	Material
B1	Wall/Plate 3-1/2" x 5-1/4"	2916 lbs	11.9%	13.0%	Spruce-Pine-Fir
B2	Wall/Plate 3-1/2" x 5-1/4"	3126 lbs	12.8%	13.9%	Spruce-Pine-Fir

### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume unbraced length of Top: 01-01-08, Bottom: 01-01-08.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor : Normal Part code : Part 9

CONFORMS TO OBC 2012

AMENDED 2020



OWG NO. YAW 14514-20  
STRUCTURAL  
COM. ONENT ONLY



**Triple 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP**  
**2ND FLR FRAMING\Dropped Beams\B8(i2378) (Dropped Beam)**

**PASSED**

BC CALC® Member Report  
 Build 7493

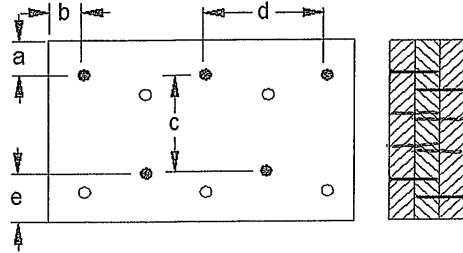
Dry | 1 span | No cant.

October 8, 2020 17:23:39

Job name:  
 Address:  
 City, Province, Postal Code: RICHMOND HILL  
 Customer:  
 Code reports: CCMC 12472-R

File name: 38-10.mmdl  
 Description: 2ND FLR FRAMING\Dropped Beams\B8(i2378)  
 Specifier:  
 Designer: LBV  
 Company:

**Connection Diagram: Full Length of Member**



*4-pows*

a minimum = 1"  
 b minimum = 3"  
 c = 6-1/2"  
 d = 12"  
 e minimum = 2"

Nailing applies to both sides of the member  
 Connectors are: 1 Nails

**3-1/2" ARDOX SPIRAL**



DWG NO. YAM 14514-20  
 STRUCTURAL  
 COMPONENT ONLY

**Disclosure**

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

BC CALC®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,

# Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 2ND FLR FRAMING\Flush Beams\B10(i3312) (Flush Beam)

**PASSED**

BC CALC® Member Report

Dry | 2 spans | L cant.

October 20, 2020 11:58:13

Build 7493

Job name:

File name: 38-10.mmdl

Address:

Description: 2ND FLR FRAMING\Flush Beams\B10(i3312)

City, Province, Postal Code: RICHMOND HILL

Specifier:

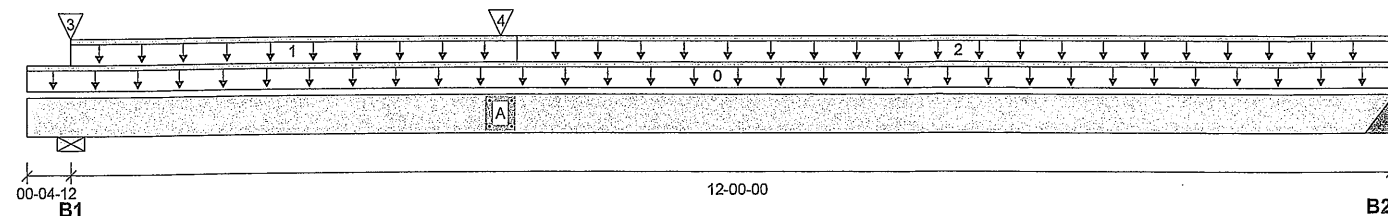
Customer:

Designer: LBV

Code reports:

CCMC 12472-R

Company:



Total Horizontal Product Length = 12-04-12

## Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B1, 5-1/2"	1068 / 0	649 / 0		
B2, 4"	434 / 0	293 / 0		

## Load Summary

Tag	Description	Load Type	Ref.	Start	End	Loc.	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Tributary
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	12-04-12	Top		10			00-00-00
1	FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-04-12	04-04-12	Top	30	15			n/a
2	FC2 Floor Material	Unf. Lin. (lb/ft)	L	04-04-12	12-04-12	Top	9	4			n/a
3	B9(i3438)	Conc. Pt. (lbs)	L	00-04-12	00-04-12	Top	201	121			n/a
4	B12(i3441)	Conc. Pt. (lbs)	L	04-03-00	04-03-00	Top	1109	605			n/a

## Controls Summary

	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	6974 ft-lbs	23220 ft-lbs	30.0%	1	04-03-00
End Shear	981 lbs	11571 lbs	8.5%	1	11-03-04
Cont. Shear	1878 lbs	11571 lbs	16.2%	1	01-05-00
Total Load Deflection	L/717 (0.196")	n/a	33.5%	4	05-09-08
Live Load Deflection	L/999 (0.122")	n/a	n/a	5	05-09-08
Total Neg. Defl.	2xL/1998 (-0.023")	n/a	n/a	4	00-00-00
Max Defl.	0.196"	n/a	n/a	4	05-09-08
Span / Depth	14.8				

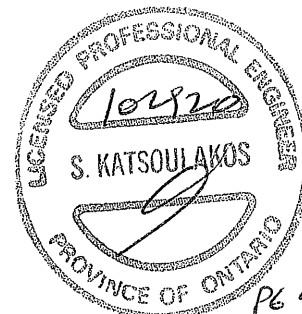
## Bearing Supports

	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Wall/Plate 5-1/2" x 3-1/2"	2413 lbs	20.4%	10.3%	Spruce-Pine-Fir
B2	Hanger 4" x 3-1/2"	1016 lbs	n/a	6.0%	HGUS410

## Cautions

Header for the hanger HGUS410 is a Double 1-3/4" x 9-1/2" LVL Beam.

Hanger model HGUS410 and seat length were input by the user. Hanger has not been analyzed for adequate capacity.


DWG NO. YAM/4515 -20  
STRUCTURAL  
COMPONENT ONLY

BC CALC® Member Report  
 Build 7493

Dry | 2 spans | L cant.

October 20, 2020 11:58:13

Job name:  
 Address:  
 City, Province, Postal Code: RICHMOND HILL  
 Customer:  
 Code reports: CCMC 12472-R

File name: 38-10.mmdl  
 Description: 2ND FLR FRAMING\Flush Beams\B10(i3312)  
 Specifier:  
 Designer: LBV  
 Company:

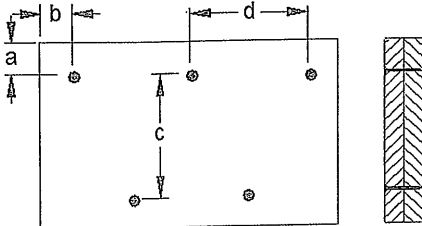
## Notes

Design meets Code minimum (L/240) Total load deflection criteria.  
 Design meets Code minimum (L/360) Live load deflection criteria.  
 Calculations assume unbraced length of Top: 00-00-00, Bottom: 00-00-00.  
 Hanger Manufacturer: Unassigned  
 Resistance Factor phi has been applied to all presented results per CSA O86.  
 BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.  
 Design based on Dry Service Condition.  
 Importance Factor : Normal Part code : Part 9  
 Cantilevers require sheathed bottom flanges, blocking at cantilever support and closure at ends.

CONFORMS TO OBC 2012

AMENDED 2020

## Connection Diagram: Full Length of Member



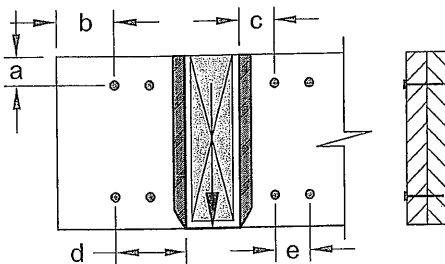
a minimum = 2"  
 b minimum = 3"  
 c = 5-1/2"  
 d = 20 1/2"

Calculated Side Load = 226.4 lb/ft

Connectors are: 1 Nails  
 3-1/2" ARDOX SPIRAL

## Connection Diagrams: Concentrated Side Loads

Connection Tag: A Applies to load tag(s): 3



a minimum = 2"  
 b minimum = 4"  
 c minimum = 4"  
 d maximum = 12"  
 e minimum = 4"  
 Connectors are: 16d Nails

3-1/2" ARDOX SPIRAL



HWB NO. TAM 14515-20  
 STRUCTURAL  
 COMPONENT ONLY

## Disclosure

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BC CALC®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,

BC CALC® Member Report

Build 7493

Job name:

File name: 38-10.mmdl

Address:

Description: 2ND FLR FRAMING\Flush Beams\B11(i3383)

City, Province, Postal Code: RICHMOND HILL

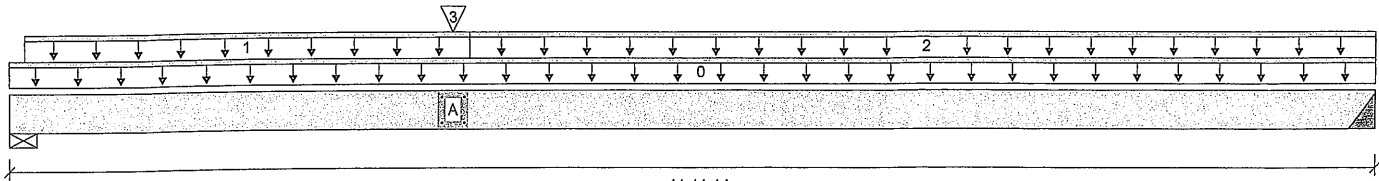
Specifier:

Customer:

Designer: LBV

Code reports: CCMC 12472-R

Company:



B1

Total Horizontal Product Length = 11-11-14

B2

### Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B1, 4-3/8"	508 / 0	347 / 0		
B2, 4"	276 / 0	211 / 0		

### Load Summary

Tag	Description	Load Type	Ref.	Start	End	Loc.	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Tributary
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	11-11-14	Top		10			00-00-00
1	FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-01-10	03-11-14	Top	21	10			n/a
2	FC2 Floor Material	Unf. Lin. (lb/ft)	L	03-11-14	11-11-14	Top	16	8			n/a
3	B12(i3441)	Conc. Pt. (lbs)	L	03-10-02	03-10-02	Top	578	340			n/a

### Controls Summary

	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	3840 ft-lbs	23220 ft-lbs	16.5%	1	03-10-02
End Shear	1136 lbs	11571 lbs	9.8%	1	01-01-14
Total Load Deflection	L/999 (0.107")	n/a	n/a	4	05-07-04
Live Load Deflection	L/999 (0.064")	n/a	n/a	5	05-07-04
Max Defl.	0.107"	n/a	n/a	4	05-07-04
Span / Depth	14.4				

Bearing Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Wall/Plate 4-3/8" x 3-1/2"	1196 lbs	12.7%	6.4%	Spruce-Pine-Fir
B2	Hanger 4" x 3-1/2"	678 lbs	n/a	4.0%	HGUS410

### Cautions

Header for the hanger HGUS410 is a Double 1-3/4" x 9-1/2" LVL Beam.

Hanger model HGUS410 and seat length were input by the user. Hanger has not been analyzed for adequate capacity.

### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor : Normal Part code : Part 9

CONFORMS TO OBC 2012

AMENDED 2020



OWN NO. TAW14516-20  
STRUCTURAL  
COMPONENT ONLY

P6 '2



BC CALC® Member Report  
 Build 7493

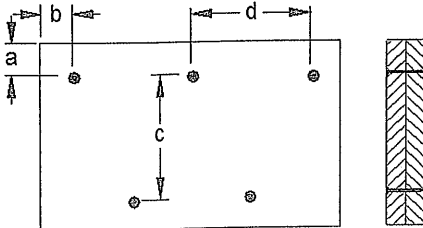
Dry | 1 span | No cant.

October 20, 2020 11:58:13

Job name:  
 Address:  
 City, Province, Postal Code: RICHMOND HILL  
 Customer:  
 Code reports: CCMC 12472-R

File name: 38-10.mmdl  
 Description: 2ND FLR FRAMING\Flush Beams\B11(i3383)  
 Specifier:  
 Designer: LBV  
 Company:

### Connection Diagram: Full Length of Member

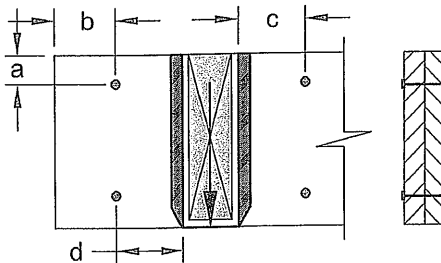


a minimum = 2"  
 b minimum = 3"  
 c = 5-1/2"  
 d = 2' 6"

Connectors are: 1 Nails  
**3-1/2" ARDOX SPIRAL**

### Connection Diagrams: Concentrated Side Loads

Connection Tag: A Applies to load tag(s): 2



a minimum = 2"  
 b minimum = 4"  
 c minimum = 4"  
 d maximum = 12"  
 Connectors are: 16d Nails

**3-1/2" ARDOX SPIRAL**



HWG NO. TAM 1451620  
 STRUCTURAL  
 COMPONENT ONLY

### Disclosure

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**Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP**  
**2ND FLR FRAMING\Flush Beams\B25(i3425) (Flush Beam)**

**PASSED**

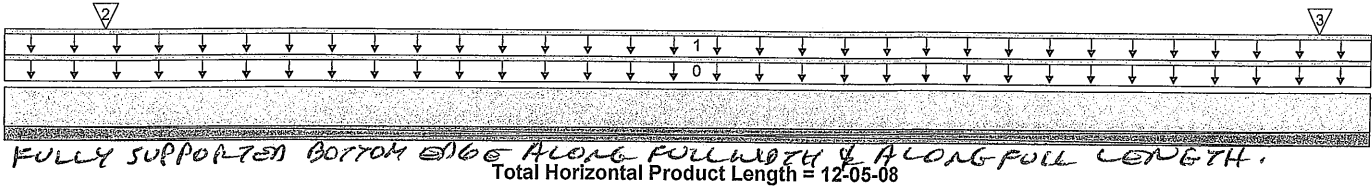
BC CALC® Member Report  
 Build 0

Dry | 1 span | No cant.

October 20, 2020 11:58:13

Job name:  
 Address:  
 City, Province, Postal Code: RICHMOND HILL  
 Customer:  
 Code reports: CCMC 12472-R

File name: 38-10.mmdl  
 Description: 2ND FLR FRAMING\Flush Beams\B25(i3425)  
 Specifier:  
 Designer: LBV  
 Company:



**Load Summary**

Tag	Description	Load Type	Ref.	Start	End	Loc.	Live	Dead	Snow	Wind	Tributary
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	12-05-08	Top	1.00	0.65	1.00	1.15	
1	E22(i613)	Unf. Lin. (lb/ft)	L	00-00-00	12-05-08	Top		81			00-00-00
2	B11(i3383)	Conc. Pt. (lbs)	L	00-10-14	00-10-14	Top	282	214			n/a
3	-	Conc. Pt. (lbs)	L	11-11-11	11-11-11	Top	640	395			n/a

**Controls Summary**

	Factored Demand	Factored Resistance	Demand/Resistance	Case	Location
Dist. Load	113.26 lb/ft	37469.25 lb/ft	0.3%		
Conc. Load	1454 lbs	16813 lbs	8.6%		

CONFORMS TO OBC 2012

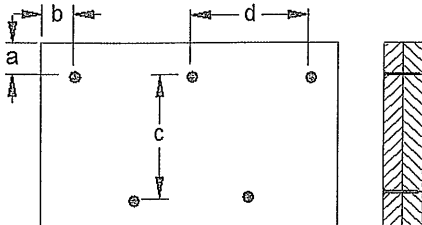
AMENDED 2020

**Cautions**

Concentrated side load(s) 3 are closer than 18" from end of member. Please consult a technical representative or Professional of Record.

*OK*

**Connection Diagram: Full Length of Member**



a minimum = 2"      c = 5-1/2"  
 b minimum = 3"      d = 12"

Calculated Side Load = 345.3 lb/ft  
 Connectors are: 16d Nails

**3-1/2" ARDOX SPIRAL**



SWK NO. TAM 14517-20  
 STRUCTURAL  
 COMPONENT ONLY

**Disclosure**

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# Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP

## 2ND FLR FRAMING\Flush Beams\B12(i2973) (Flush Beam)

**PASSED**

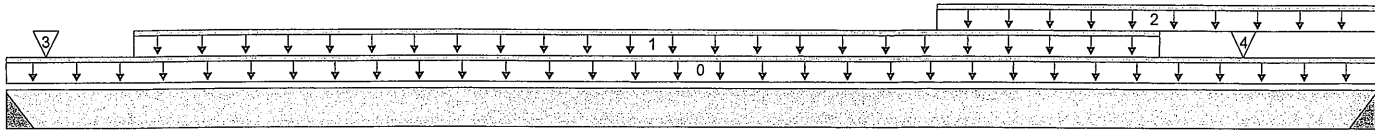
BC CALC® Member Report  
Build 7493

Dry | 1 span | No cant.

October 8, 2020 17:23:39

Job name:  
Address:  
City, Province, Postal Code: RICHMOND HILL  
Customer:  
Code reports: CCMC 12472-R

File name: 38-10.mmdl  
Description: 2ND FLR FRAMING\Flush Beams\B12(i2973)  
Specifier:  
Designer: LBV  
Company:



B1 10-08-04 B2  
Total Horizontal Product Length = 10-08-04

### Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B1, 4"	568 / 0	335 / 0		
B2, 4"	1120 / 0	610 / 0		

### Load Summary

Tag	Description	Load Type	Ref.	Start	End	Loc.	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Tributary
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	10-08-04	Top		10			00-00-00
1	Smoothed Load	Unf. Lin. (lb/ft)	L	00-11-10	08-11-10	Top	82	41			n/a
2	STAIR	Unf. Lin. (lb/ft)	L	07-02-04	10-08-04	Top	240	120			n/a
3	J6(i3006)	Conc. Pt. (lbs)	L	00-03-10	00-03-10	Top	87	44			n/a
4	J6(i2964)	Conc. Pt. (lbs)	L	09-07-10	09-07-10	Top	105	52			n/a

### Controls Summary

	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	3933 ft-lbs	23220 ft-lbs	16.9%	1	06-11-10
End Shear	1834 lbs	11571 lbs	15.8%	1	09-06-12
Total Load Deflection	L/999 (0.102")	n/a	n/a	4	05-07-10
Live Load Deflection	L/999 (0.065")	n/a	n/a	5	05-07-10
Max Defl.	0.102"	n/a	n/a	4	05-07-10
Span / Depth	12.8				

Bearing Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1 Hanger	4" x 3-1/2"	1270 lbs	n/a	7.4%	HGUS410
B2 Hanger	4" x 3-1/2"	2442 lbs	n/a	14.3%	HGUS410

### Cautions

Header for the hanger HGUS410 is a Double 1-3/4" x 9-1/2" LVL Beam.  
Hanger model HGUS410 and seat length were input by the user. Hanger has not been analyzed for adequate capacity.

### Notes

Design meets Code minimum (L/240) Total load deflection criteria.  
Design meets Code minimum (L/360) Live load deflection criteria.  
Calculations assume member is fully braced.  
Hanger Manufacturer: Unassigned  
Resistance Factor phi has been applied to all presented results per CSA O86.  
BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.  
Design based on Dry Service Condition.  
Importance Factor : Normal Part code : Part 9

CONFORMS TO OBC 2012

AMENDED 2020



OWB NO. TAM 14518-20  
STRUCTURAL  
COMPONENT ONLY



**Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP**  
**2ND FLR FRAMING\Flush Beams\B12(i2973) (Flush Beam)**

**PASSED**

BC CALC® Member Report  
Build 7493

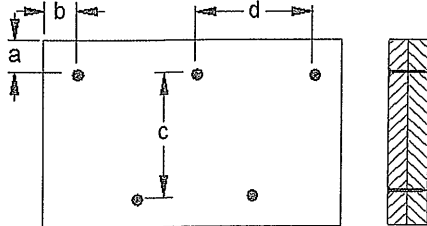
Dry | 1 span | No cant.

October 8, 2020 17:23:39

Job name:  
Address:  
City, Province, Postal Code: RICHMOND HILL  
Customer:  
Code reports: CCMC 12472-R

File name: 38-10.mmdl  
Description: 2ND FLR FRAMING\Flush Beams\B12(i2973)  
Specifier:  
Designer: LBV  
Company:

**Connection Diagram: Full Length of Member**



a minimum = 2"  
b minimum = 3"  
c = 5-1/2"  
d = 3-1/2"

Calculated Side Load = 235.3 lb/ft

Connectors are: 3-1/2" ARDOX SPIRAL Nails



UWB NO. TAM/4518-20  
STRUCTURAL  
COMPONENT ONLY

**Disclosure**

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BC CALC® Member Report

Build 7493

Job name:

Address:

City, Province, Postal Code: RICHMOND HILL

Customer:

Code reports: CCMC 12472-R

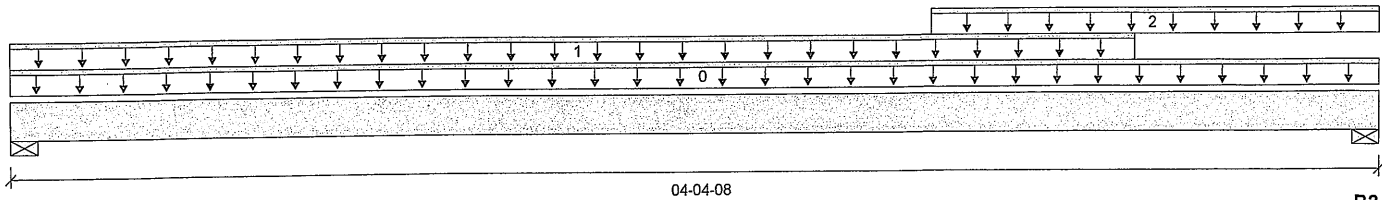
File name: 38-10.mmdl

Description: 2ND FLR FRAMING\Flush Beams\B13(i2367)

Specifier:

Designer: LBV

Company:



B1

B2

Total Horizontal Product Length = 04-04-08

### Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B1, 5-1/2"	1388 / 0	716 / 0		
B2, 2-3/4"	723 / 0	382 / 0		

### Load Summary

Tag	Description	Load Type	Ref.	Start	End	Loc.	Live	Dead	Snow	Wind	Tributary
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	04-04-08	Top	1.00	0.65	1.00	1.15	00-00-00
1	Smoothed Load	Unf. Lin. (lb/ft)	L	00-00-00	03-07-00	Top	568	284			n/a
2	FC2 Floor Material	Unf. Lin. (lb/ft)	L	02-11-00	04-04-08	Top	53	27			n/a

### Controls Summary

	Factored Demand	Factored Resistance	Demand/Resistance	Case	Location
Pos. Moment	1887 ft-lbs	23220 ft-lbs	8.1%	1	02-11-00
End Shear	1518 lbs	11571 lbs	13.1%	1	01-03-00
Total Load Deflection	L/999 (0.007")	n/a	n/a	4	02-03-08
Live Load Deflection	L/999 (0.005")	n/a	n/a	5	02-03-08
Max Defl.	0.007"	n/a	n/a	4	02-03-08
Span / Depth	4.8				

### Bearing Supports

	Dim. (LxW)	Demand	Demand/Resistance Support	Demand/Resistance Member	Material
B1	Wall/Plate 5-1/2" x 3-1/2"	2978 lbs	25.1%	12.7%	Spruce-Pine-Fir
B2	Wall/Plate 2-3/4" x 3-1/2"	1562 lbs	26.4%	13.3%	Spruce-Pine-Fir

### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor : Normal Part code : Part 9

CONFORMS TO OBC 2012

AMENDED 2020



OWN NO. 14519-20  
STRUCTURAL  
COMMENT ONLY

Build 7493

Job name:

File name: 38-10.mmdl

Address:

Description: 2ND FLR FRAMING\Flush Beams\B13(i2367)

City, Province, Postal Code: RICHMOND HILL

Specifier:

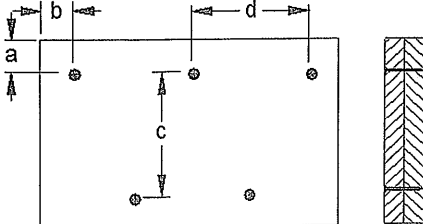
Customer:

Designer: LBV

Code reports: CCMC 12472-R

Company:

## Connection Diagram: Full Length of Member



a minimum = 2"

c = 5-1/2"

b minimum = 3"

d = 20" 6'

Calculated Side Load = 427.1 lb/ft

Connectors are: 16d Nails

3-1/2" ARDOX SPIRAL



BWG NO. TAM 14519-20  
STRUCTURAL  
COMPONENT ONLY

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# Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 2ND FLR FRAMING\Flush Beams\B16(i2629) (Flush Beam)

**PASSED**

BC CALC® Member Report

Dry | 1 span | No cant.

October 8, 2020 17:23:39

Build 7493

Job name:

File name: 38-10.mmdl

Address:

Description: 2ND FLR FRAMING\Flush Beams\B16(i2629)

City, Province, Postal Code: RICHMOND HILL

Specifier:

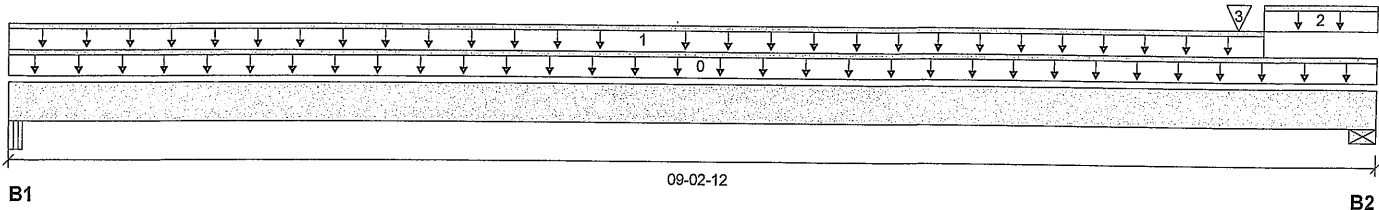
Customer:

Designer: LBV

Code reports:

CCMC 12472-R

Company:



Total Horizontal Product Length = 09-02-12

## Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B1, 2-5/8"	167 / 0	154 / 0	24 / 0	
B2, 2-3/4"	279 / 0	622 / 0	498 / 0	

## Load Summary

Tag	Description	Load Type	Ref.	Start	End	Loc.	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Tributary
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	09-02-12	Top		10			00-00-00
1	FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	08-05-08	Top	33	17			n/a
2	E32(i606)	Unf. Lin. (lb/ft)	L	08-05-08	09-02-12	Top		319	475		n/a
3	-	Conc. Pt. (lbs)	L	08-03-06	08-03-06	Top	164	300	155		n/a

## Controls Summary

	Factored Demand	Factored Resistance	Demand/Resistance	Case	Location
Pos. Moment	1239 ft-lbs	23220 ft-lbs	5.3%	1	05-07-07
End Shear	1467 lbs	11571 lbs	12.7%	13	08-02-08
Total Load Deflection	L/999 (0.027")	n/a	n/a	35	04-10-00
Live Load Deflection	L/999 (0.015")	n/a	n/a	51	04-10-00
Max Defl.	0.027"	n/a	n/a	35	04-10-00
Span / Depth	11.3				

## Bearing Supports

	Dim. (LxW)	Demand	Demand/Resistance Support	Demand/Resistance Member	Material
B1	Beam 2-5/8" x 3-1/2"	466 lbs	11.9%	4.2%	Unspecified
B2	Wall/Plate 2-3/4" x 3-1/2"	1804 lbs	30.5%	15.4%	Spruce-Pine-Fir

## Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

CONFORMS TO OBC 2012

AMENDED 2020

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Unbalanced snow loads determined from building geometry were used in selected product's verification.

Design based on Dry Service Condition.

Importance Factor : Normal Part code : Part 9


DWG NO. TAM 1452020  
STRUCTURAL  
COMPONENT ONLY

BC CALC® Member Report  
Build 7493

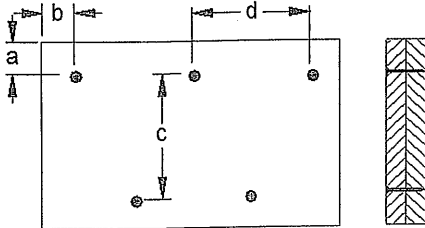
Dry | 1 span | No cant.

October 8, 2020 17:23:39

Job name:  
Address:  
City, Province, Postal Code: RICHMOND HILL  
Customer:  
Code reports: CCMC 12472-R

File name: 38-10.mmdl  
Description: 2ND FLR FRAMING\Flush Beams\B16(i2629)  
Specifier:  
Designer: LBV  
Company:

### Connection Diagram: Full Length of Member

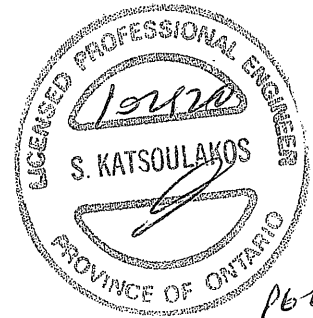


a minimum = 2"  
b minimum = 3"  
c = 5-1/2"  
d = 3-1/2"

Calculated Side Load = 269.0 lb/ft

Connectors are: 3-1/2" ARDOX SPIRAL Nails

3-1/2" ARDOX SPIRAL



BWG NO. TAM/4520-20  
STRUCTURAL  
COMPONENT ONLY

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BC CALC® Member Report

Dry | 1 span | No cant.

October 8, 2020 17:23:39

Build 7493

Job name:

File name: 38-10.mmdl

Address:

Description: 2ND FLR FRAMING\Flush Beams\B17(i2453)

City, Province, Postal Code: RICHMOND HILL

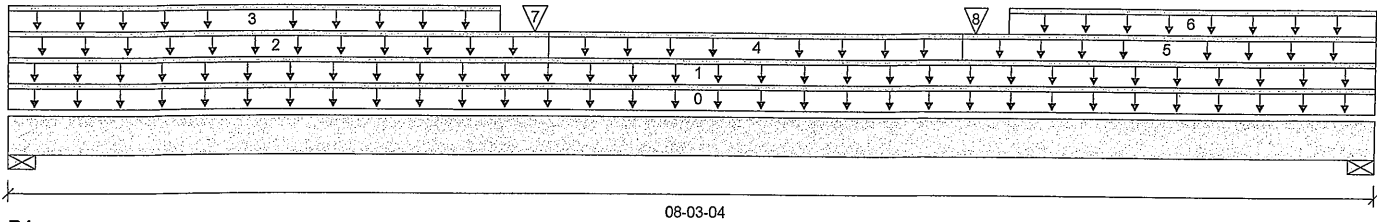
Specifier:

Customer:

Designer: LBV

Code reports: CCMC 12472-R

Company:



B1

08-03-04

B2

Total Horizontal Product Length = 08-03-04

**Reaction Summary (Down / Uplift) (lbs)**

Bearing	Live	Dead	Snow	Wind
B1, 2-3/4"	110 / 0	1391 / 0	1968 / 0	
B2, 2-3/4"	110 / 0	1383 / 0	1961 / 0	

**Load Summary**

Tag	Description	Load Type	Ref.	Start	End	Loc.	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Tributary
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	08-03-04	Top		10			00-00-00
1	FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	08-03-04	Top	27	13			n/a
2	E32(i606)	Unf. Lin. (lb/ft)	L	00-00-00	03-02-12	Top		81			n/a
3	E32(i606)	Unf. Lin. (lb/ft)	L	00-00-00	02-11-04	Top		238	475		n/a
4	E37(i1268)	Unf. Lin. (lb/ft)	L	03-02-12	05-08-12	Top		41			n/a
5	E36(i1267)	Unf. Lin. (lb/ft)	L	05-08-12	08-03-04	Top		81			n/a
6	E36(i1267)	Unf. Lin. (lb/ft)	L	06-00-04	08-03-04	Top		238	475		n/a
7	E32(i606)	Conc. Pt. (lbs)	L	03-01-12	03-01-12	Top		395	743		n/a
8	E36(i1267)	Conc. Pt. (lbs)	L	05-09-12	05-09-12	Top		385	721		n/a

**Controls Summary**

	Factored Demand	Factored Resistance	Demand/Resistance	Case	Location
Pos. Moment	8571 ft-lbs	23220 ft-lbs	36.9%	13	03-01-12
End Shear	3611 lbs	11571 lbs	31.2%	13	01-00-04
Total Load Deflection	L/669 (0.142")	n/a	35.9%	35	04-01-01
Live Load Deflection	L/999 (0.085")	n/a	n/a	51	04-01-01
Max Defl.	0.142"	n/a	n/a	35	04-01-01
Span / Depth	10.0				

**Bearing Supports**

	Dim. (LxW)	Demand	Demand/Resistance Support	Demand/Resistance Member	Material
B1	Wall/Plate 2-3/4" x 3-1/2"	4802 lbs	81.1%	40.9%	Spruce-Pine-Fir
B2	Wall/Plate 2-3/4" x 3-1/2"	4781 lbs	80.7%	40.7%	Spruce-Pine-Fir

**Notes**

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Unbalanced snow loads determined from building geometry were used in selected product's verification.

Design based on Dry Service Condition.

Importance Factor : Normal Part code : Part 9

CONFORMS TO OBC 2012

AMENDED 2020


BWC NO. TAM 14521-20  
STRUCTURAL  
COMPONENT ONLY





# Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP

PASSED

## 2ND FLR FRAMING\Flush Beams\B17(i2453) (Flush Beam)

Dry | 1 span | No cant.

October 8, 2020 17:23:39

BC CALC® Member Report

Build 7493

Job name:

File name: 38-10.mmdl

Address:

Description: 2ND FLR FRAMING\Flush Beams\B17(i2453)

City, Province, Postal Code: RICHMOND HILL

Specifier:

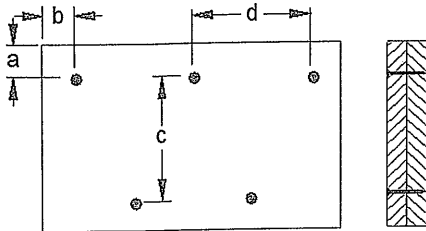
Customer:

Designer: LBV

Code reports: CCMC 12472-R

Company:

### Connection Diagram: Full Length of Member



a minimum = 2"  
b minimum = 3"

c = 5-1/2"  
d = 3"

Connectors are: 1 Nails  
3-1/2" ARDOX SPIRAL



DWG NO. TAM 14521-20  
STRUCTURAL  
COMPONENT ONLY

### Disclosure

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Build 7493

Job name:

File name: 38-10.mmdl

Address:

Description: 2ND FLR FRAMING\Flush Beams\B18(i2382)

City, Province, Postal Code: RICHMOND HILL

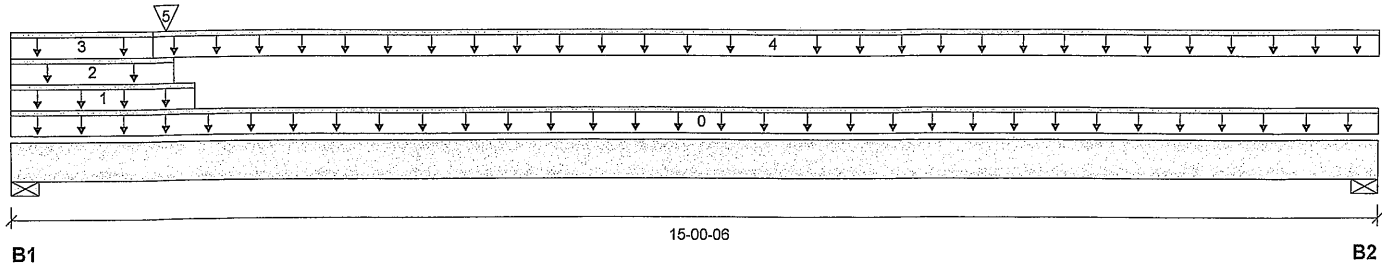
Specifier:

Customer:

Designer: LBV

Code reports: CCMC 12472-R

Company:



Total Horizontal Product Length = 15-00-06

### Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B1, 2-3/4"	490 / 0	976 / 0	838 / 0	
B2, 4-3/8"	418 / 0	325 / 0	47 / 0	

### Load Summary

Tag	Description	Load Type	Ref.	Start	End	Loc.	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Tributary
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	15-00-06	Top		10			00-00-00
1	E36(i1267)	Unf. Lin. (lb/ft)	L	00-00-00	02-00-00	Top		81			n/a
2	E36(i1267)	Unf. Lin. (lb/ft)	L	00-00-00	01-09-04	Top		238	475		n/a
3	FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	01-06-08	Top	27	13			n/a
4	FC2 Floor Material	Unf. Lin. (lb/ft)	L	01-06-08	15-00-06	Top	53	27			n/a
5	B19(i2346)	Conc. Pt. (lbs)	L	01-08-04	01-08-04	Top	148	190	36		n/a

### Controls Summary

	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	4331 ft-lbs	23220 ft-lbs	18.7%	1	06-05-07
End Shear	1832 lbs	11571 lbs	15.8%	1	01-00-04
Total Load Deflection	L/687 (0.254")	n/a	34.9%	35	07-02-06
Live Load Deflection	L/1181 (0.148")	n/a	30.5%	51	07-02-06
Max Defl.	0.254"	n/a	n/a	35	07-02-06
Span / Depth	18.4				

Bearing Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Wall/Plate 2-3/4" x 3-1/2"	2967 lbs	50.1%	25.3%	Spruce-Pine-Fir
B2	Wall/Plate 4-3/8" x 3-1/2"	1080 lbs	11.5%	5.8%	Spruce-Pine-Fir

### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

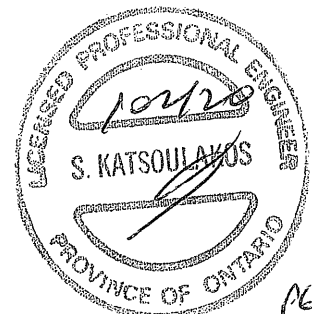
Unbalanced snow loads determined from building geometry were used in selected product's verification.

Design based on Dry Service Condition.

Importance Factor : Normal Part code : Part 9

CONFORMS TO OBC 2012

AMENDED 2020



BWR NO. TAM 1452220  
STRUCTURAL  
COMPONENT ONLY

BC CALC® Member Report  
 Build 7493

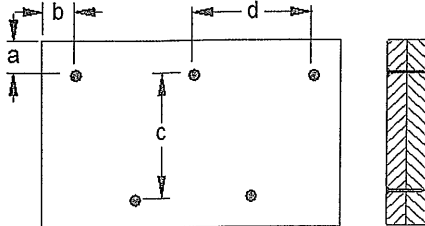
Dry | 1 span | No cant.

October 8, 2020 17:23:39

Job name:  
 Address:  
 City, Province, Postal Code: RICHMOND HILL  
 Customer:  
 Code reports: CCMC 12472-R

File name: 38-10.mmdl  
 Description: 2ND FLR FRAMING\Flush Beams\B18(i2382)  
 Specifier:  
 Designer: LBV  
 Company:

### Connection Diagram: Full Length of Member



a minimum = 2"  
 b minimum = 3"  
 c = 5-1/2"  
 d = 8"

Connectors are: 1/2" ARDOX SPIRAL Nails  
 3-1/2" ARDOX SPIRAL



HWB NO. TAM 1452420  
 STRUCTURAL  
 COMPONENT ONLY

### Disclosure

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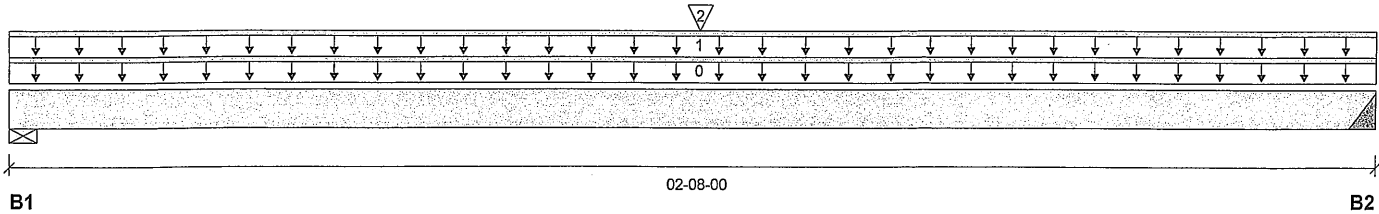
BC CALC® Member Report  
 Build 7493

Dry | 1 span | No cant.

October 8, 2020 17:23:39

Job name:  
 Address:  
 City, Province, Postal Code: RICHMOND HILL  
 Customer:  
 Code reports: CCMC 12472-R

File name: 38-10.mmdl  
 Description: 2ND FLR FRAMING\Flush Beams\B19(i2346)  
 Specifier:  
 Designer: LBV  
 Company:



### Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B1, 5-1/2"	187 / 0	239 / 0	45 / 0	
B2, 4"	166 / 0	215 / 0	41 / 0	

### Load Summary

Tag	Description	Load Type	Ref.	Start	End	Loc.	Live	Dead	Snow	Wind	Tributary
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	02-08-00	Top	1.00	0.65	1.00	1.15	00-00-00
1	E33(i609)	Unf. Lin. (lb/ft)	L	00-00-00	02-08-00	Top		95	32		n/a
2	J2(i2349)	Conc. Pt. (lbs)	L	01-04-00	01-04-00	Top	353	176			n/a

### Controls Summary

	Factored Demand	Factored Resistance	Demand/Resistance	Case	Location
Pos. Moment	454 ft-lbs	23220 ft-lbs	2.0%	1	01-04-00
End Shear	422 lbs	11571 lbs	3.6%	1	01-03-00
Total Load Deflection	L/999 (0")	n/a	n/a	35	01-04-10
Live Load Deflection	L/999 (0")	n/a	n/a	51	01-04-08
Max Defl.	0"	n/a	n/a	35	01-04-10
Span / Depth	2.5				

### Bearing Supports

	Dim. (LxW)	Demand	Demand/Resistance Support	Demand/Resistance Member	Material
B1	Wall/Plate 5-1/2" x 3-1/2"	625 lbs	5.3%	2.7%	Spruce-Pine-Fir
B2	Hanger 4" x 3-1/2"	558 lbs	n/a	3.3%	HGUS410

### Cautions

Header for the hanger HGUS410 is a Double 1-3/4" x 9-1/2" LVL Beam.  
 Hanger model HGUS410 and seat length were input by the user. Hanger has not been analyzed for adequate capacity.

### Notes

Design meets Code minimum (L/240) Total load deflection criteria.  
 Design meets Code minimum (L/360) Live load deflection criteria.  
 Calculations assume member is fully braced.  
 Hanger Manufacturer: Unassigned  
 Resistance Factor phi has been applied to all presented results per CSA O86.  
 BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.  
 Unbalanced snow loads determined from building geometry were used in selected product's verification.  
 Design based on Dry Service Condition.  
 Importance Factor : Normal Part code : Part 9

CONFORMS TO OBC 2012

AMENDED 2020



BWB NO. TAM 1452320  
 STRUCTURAL  
 COMPONENT ONLY



# Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP

PASSED

## 2ND FLR FRAMING\Flush Beams\B19(i2346) (Flush Beam)

BC CALC® Member Report  
Build 7493

Dry | 1 span | No cant.

October 8, 2020 17:23:39

Job name:

File name: 38-10.mmdl

Address:

Description: 2ND FLR FRAMING\Flush Beams\B19(i2346)

City, Province, Postal Code: RICHMOND HILL

Specifier:

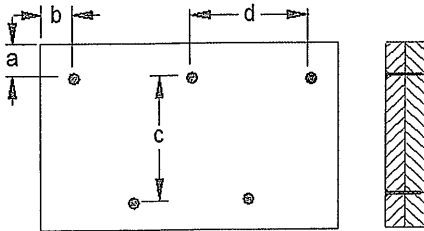
Customer:

Designer: LBV

Code reports: CCMC 12472-R

Company:

### Connection Diagram: Full Length of Member



a minimum = 2"  
b minimum = 3"

c = 5-1/2"  
d = 8"

Calculated Side Load = 374.8 lb/ft

Connectors are: 16d Nails

3-1/2" ARDOX SPIRAL



UWB NO. TAM/4523-20  
STRUCTURAL  
COMPONENT ONLY

### Disclosure

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Boise Cascade

**Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP****2ND FLR FRAMING\Flush Beams\B20(i2614) (Flush Beam)****PASSED**BC CALC® Member Report  
Build 7493

Dry | 1 span | No cant.

October 8, 2020 17:23:39

Job name:

File name: 38-10.mmdl

Address:

Description: 2ND FLR FRAMING\Flush Beams\B20(i2614)

City, Province, Postal Code: RICHMOND HILL

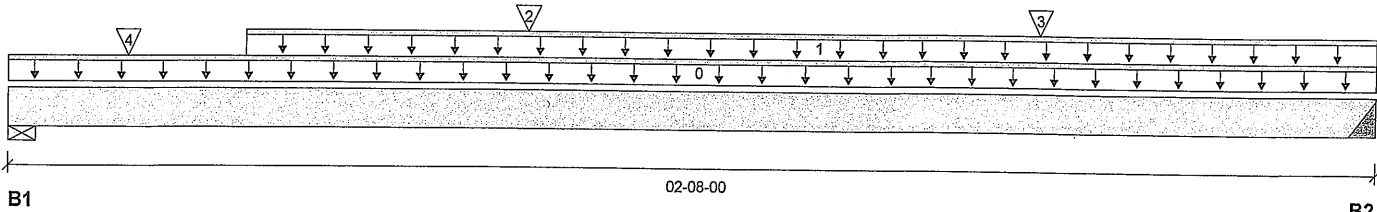
Specifier:

Customer:

Designer: LBV

Code reports: CCMC 12472-R

Company:



Total Horizontal Product Length = 02-08-00

**Reaction Summary (Down / Uplift) (lbs)**

Bearing	Live	Dead	Snow	Wind
B1, 3-1/2"	128 / 0	189 / 0	43 / 0	
B2, 4"	183 / 0	232 / 0	43 / 0	

**Load Summary**

Tag	Description	Load Type	Ref.	Start	End	Loc.	Live	Dead	Snow	Wind	Tributary
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	02-08-00	Top	1.00	0.65	1.00	1.15	
1	E31(i608)	Unf. Lin. (lb/ft)	L	00-05-08	02-08-00	Top		95	32		00-00-00
2	J4(i2541)	Conc. Pt. (lbs)	L	01-00-00	01-00-00	Top	155	78			n/a
3	J4(i2586)	Conc. Pt. (lbs)	L	02-00-00	02-00-00	Top	156	78			n/a
4	E30(i614)	Conc. Pt. (lbs)	L	00-02-12	00-02-12	Top		30	15		n/a

**Controls Summary**

	Factored Demand	Factored Resistance	Demand/Resistance	Case	Location
Pos. Moment	295 ft-lbs	23220 ft-lbs	1.3%	1	01-00-00
End Shear	371 lbs	11571 lbs	3.2%	1	01-01-00
Total Load Deflection	L/999 (0")	n/a	n/a	35	01-03-12
Live Load Deflection	L/999 (0")	n/a	n/a	51	01-03-12
Max Defl.	0"	n/a	n/a	35	01-03-12
Span / Depth	2.7				

**Bearing Supports**

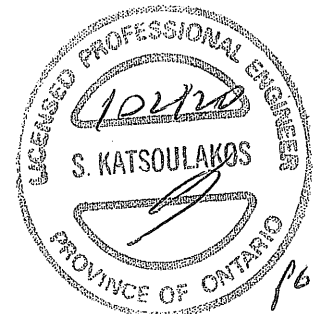
	Dim. (LxW)	Demand	Demand/Resistance Support	Demand/Resistance Member	Material
B1	Wall/Plate 3-1/2" x 3-1/2"	472 lbs	6.3%	3.2%	Spruce-Pine-Fir
B2	Hanger 4" x 3-1/2"	607 lbs	n/a	3.6%	HGUS410

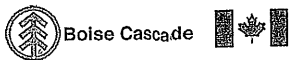
**Cautions**

Header for the hanger HGUS410 is a Double 1-3/4" x 9-1/2" LVL Beam.

Hanger model HGUS410 and seat length were input by the user. Hanger has not been analyzed for adequate capacity.

OK

DWG NO. TAW14524-20  
STRUCTURAL  
COMPONENT ONLY



**Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP**  
**2ND FLR FRAMING\Flush Beams\B20(i2614) (Flush Beam)**

**PASSED**

BC CALC® Member Report  
Build 7493

Job name:  
Address:  
City, Province, Postal Code: RICHMOND HILL  
Customer:  
Code reports: CCMC 12472-R

Dry | 1 span | No cant.

October 8, 2020 17:23:39

File name: 38-10.mmdl  
Description: 2ND FLR FRAMING\Flush Beams\B20(i2614)  
Specifier:  
Designer: LBV  
Company:

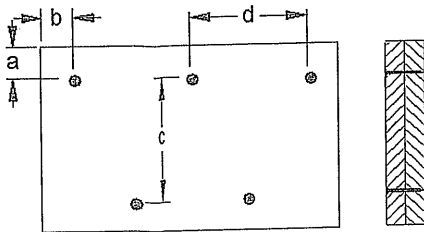
### Notes

Design meets Code minimum (L/240) Total load deflection criteria.  
Design meets Code minimum (L/360) Live load deflection criteria.  
Calculations assume member is fully braced.  
Hanger Manufacturer: Unassigned  
Resistance Factor phi has been applied to all presented results per CSA O86.  
BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.  
Unbalanced snow loads determined from building geometry were used in selected product's verification.  
Design based on Dry Service Condition.  
Importance Factor : Normal Part code : Part 9

CONFORMS TO OBC 2012

AMENDED 2020

### Connection Diagram: Full Length of Member

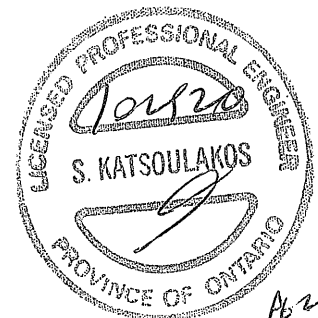


a minimum = 2"  
b minimum = 3"  
c = 5-1/2"  
d = 8"

Calculated Side Load = 165.8 lb/ft

Connectors are: 1 Nails

**3-1/2" ARDOX SPIRAL**



104524-20  
STRUCTURAL  
COMPONENT ONLY

### Disclosure

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ALLJOIST®, BC RIM BOARD™, BCI®,  
BOISE GLULAM™, BC FloorValue®,  
VERSA-LAM®, VERSA-RIM PLUS®,

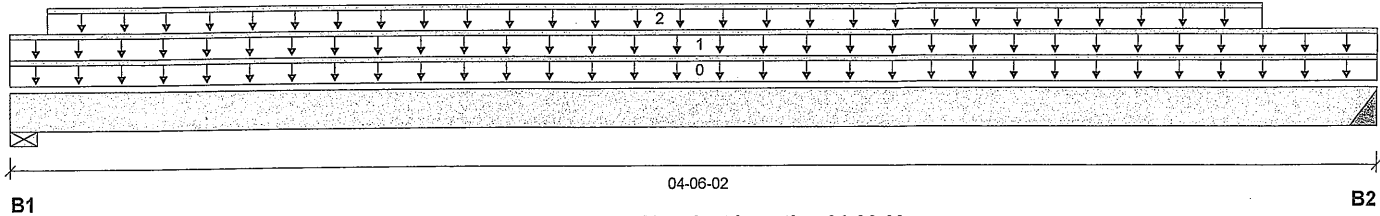
BC CALC® Member Report  
 Build 7493

Dry | 1 span | No cant.

October 8, 2020 17:23:39

Job name:  
 Address:  
 City, Province, Postal Code: RICHMOND HILL  
 Customer:  
 Code reports: CCMC 12472-R

File name: 38-10.mmdl  
 Description: 2ND FLR FRAMING\Flush Beams\B9(i3009)  
 Specifier:  
 Designer: LBV  
 Company:



Total Horizontal Product Length = 04-06-02

**Reaction Summary (Down / Uplift) (lbs)**

Bearing	Live	Dead	Snow	Wind
B1, 3-1/2"	227 / 0	134 / 0		
B2, 4"	208 / 0	126 / 0		

**Load Summary**

Tag	Description	Load Type	Ref.	Start	End	Loc.	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Tributary
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	04-06-02	Top		10			00-00-00
1	FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	04-06-02	Top	29	15			n/a
2	Smoothed Load	Unf. Lin. (lb/ft)	L	00-01-08	04-01-08	Top	76	38			n/a

**Controls Summary**

	Factored Demand	Factored Resistance	Demand/Resistance	Case	Location
Pos. Moment	508 ft-lbs	23220 ft-lbs	2.2%	1	02-01-08
End Shear	367 lbs	11571 lbs	3.2%	1	03-04-10
Total Load Deflection	L/999 (0.002")	n/a	n/a	4	02-02-15
Live Load Deflection	L/999 (0.001")	n/a	n/a	5	02-02-15
Max Defl.	0.002"	n/a	n/a	4	02-02-15
Span / Depth	5.1				

Bearing Supports	Dim. (LxW)	Demand	Demand/Resistance Support	Demand/Resistance Member	Material
B1	Wall/Plate 3-1/2" x 3-1/2"	509 lbs	6.8%	3.4%	Spruce-Pine-Fir
B2	Hanger 4" x 3-1/2"	470 lbs	n/a	2.8%	HGUS410

**Cautions**

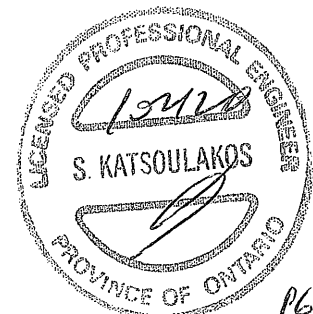
Header for the hanger HGUS410 is a Double 1-3/4" x 9-1/2" LVL Beam.  
 Hanger model HGUS410 and seat length were input by the user. Hanger has not been analyzed for adequate capacity.

**Notes**

Design meets Code minimum (L/240) Total load deflection criteria.  
 Design meets Code minimum (L/360) Live load deflection criteria.  
 Calculations assume unbraced length of Top: 00-00-00, Bottom: 00-00-00.  
 Hanger Manufacturer: Unassigned  
 Resistance Factor phi has been applied to all presented results per CSA O86.  
 BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.  
 Design based on Dry Service Condition.  
 Importance Factor : Normal Part code : Part 9

CONFORMS TO OBC 2012

AMENDED 2020



OWN NO. TAM 14525-20  
 STRUCTURAL  
 COMPONENT ONLY





**Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP**  
**2ND FLR FRAMING\Flush Beams\B9(i3009) (Flush Beam)**

PASSED

BC CALC® Member Report  
Build 7493

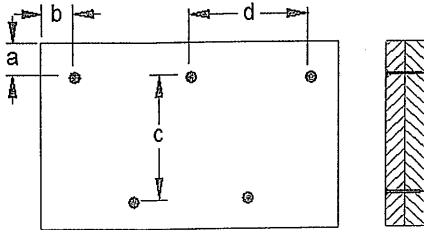
Dry | 1 span | No cant.

October 8, 2020 17:23:39

Job name:  
Address:  
City, Province, Postal Code: RICHMOND HILL  
Customer:  
Code reports: CCMC 12472-R

File name: 38-10.mmdl  
Description: 2ND FLR FRAMING\Flush Beams\B9(i3009)  
Specifier:  
Designer: LBV  
Company:

**Connection Diagram: Full Length of Member**



a minimum = 2"      c = 5-1/2"  
b minimum = 3"      d = 8"

Calculated Side Load = 109.1 lb/ft

Connectors are: 3-1/2" ARDOX SPIRAL Nails



DWG NO. TAM 14325-20  
STRUCTURAL  
COMPONENT ONLY

**Disclosure**

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BC CALC®, BC FRAMER®, AJST™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®

BC CALC® Member Report

Build 7493

Job name:

File name: 38-10.mmdl

Address:

Description: 1ST FLR FRAMING\Flush Beams\B1(i2975)

City, Province, Postal Code: RICHMOND HILL

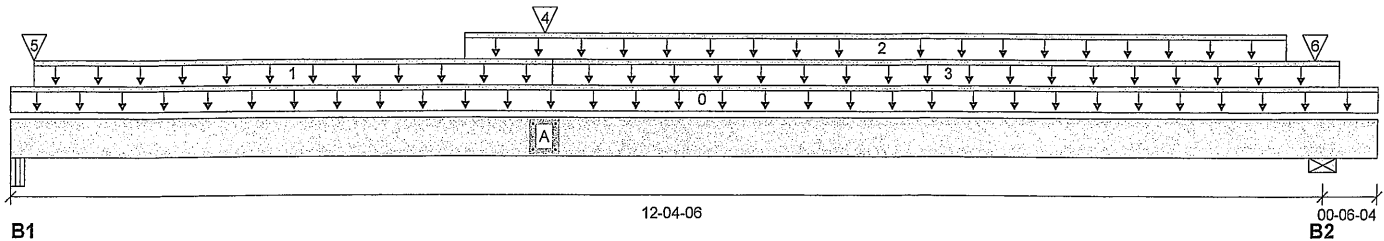
Specifier:

Customer:

Designer: LBV

Code reports: CCMC 12472-R

Company:



Total Horizontal Product Length = 12-10-10

### Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B1, 5-1/4"	842 / 0	731 / 0		
B2, 3-1/2"	455 / 0	877 / 0		

### Load Summary

Tag	Description	Load Type	Ref.	Start	End	Loc.	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Tributary
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	12-10-10	Top		10			00-00-00
1	FC1 Floor Material	Unf. Lin. (lb/ft)	L	00-02-10	05-00-08	Top	24	12			n/a
2	5(i2665)	Unf. Lin. (lb/ft)	L	04-02-10	12-00-02	Top		65			n/a
3	FC1 Floor Material	Unf. Lin. (lb/ft)	L	05-00-08	12-06-02	Top	6	3			n/a
4	B2(i2168)	Conc. Pt. (lbs)	L	04-11-10	04-11-10	Top	495	255			n/a
5	2(i562)	Conc. Pt. (lbs)	L	00-02-10	00-02-10	Top	430	280			n/a
6	E9(i432)	Conc. Pt. (lbs)	L	12-03-06	12-03-06	Top	213	365			n/a

### Controls Summary

	Factored Demand	Factored Resistance	Demand/Resistance	Case	Location
Pos. Moment	4685 ft-lbs	23220 ft-lbs	20.2%	2	04-11-10
Neg. Moment	-2 ft-lbs	-15093 ft-lbs	n/a	0	12-04-06
End Shear	1116 lbs	11571 lbs	9.6%	1	01-02-12
Cont. Shear	693 lbs	7521 lbs	9.2%	0	11-05-02
Total Load Deflection	L/912 (0.158")	n/a	26.3%	9	06-01-11
Live Load Deflection	L/999 (0.068")	n/a	n/a	12	05-11-05
Total Neg. Defl.	2xL/1998 (-0.021")	n/a	n/a	9	12-10-10
Max Defl.	0.158"	n/a	n/a	9	06-01-11
Span / Depth	15.1				

### Bearing Supports

	Dim. (LxW)	Demand	Demand/Resistance Support	Demand/Resistance Member	Material
B1	Beam 5-1/4" x 3-1/2"	2177 lbs	27.7%	9.7%	Unspecified
B2	Wall/Plate 3-1/2" x 3-1/2"	1228 lbs	25.1%	12.6%	Spruce-Pine-Fir

### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume unbraced length of Top: 00-04-08, Bottom: 00-04-08.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor : Normal Part code : Part 9

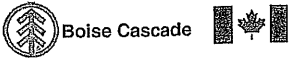
Cantilevers require sheathed bottom flanges, blocking at cantilever support and closure at ends.

CONFORMS TO OBC 2012

AMENDED 2020



DWG NO. TAM 1452620  
STRUCTURAL  
COMPONENT ONLY



Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP  
1ST FLR FRAMING\Flush Beams\B1(i2975) (Flush Beam)

PASSED

BC CALC® Member Report  
Build 7493

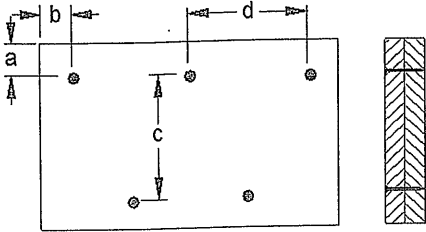
Dry | 2 spans | R cant.

October 8, 2020 17:23:39

Job name:  
Address:  
City, Province, Postal Code: RICHMOND HILL  
Customer:  
Code reports: CCMC 12472-R

File name: 38-10.mmdl  
Description: 1ST FLR FRAMING\Flush Beams\B1(i2975)  
Specifier:  
Designer: LBV  
Company:

Connection Diagram: Full Length of Member

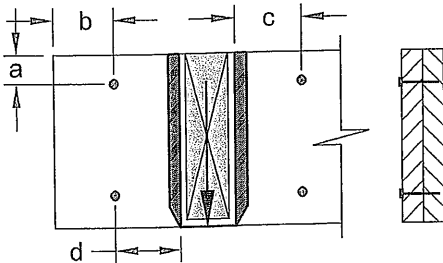


a minimum = 2"  
b minimum = 3"  
c = 5-1/2"  
d = 6"

Connectors are: 3-1/2" ARDOX SPIRAL Nails

Connection Diagrams: Concentrated Side Loads

Connection Tag: A Applies to load tag(s): 4



a minimum = 2"  
b minimum = 4"  
c minimum = 4"  
d maximum = 12"  
Connectors are: 3-1/2" ARDOX SPIRAL Nails

3-1/2" ARDOX SPIRAL



1624  
DWG NO. TAM/4526/20  
STRUCTURAL  
COMPONENT ONLY

Disclosure

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BC CALC®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,

BC CALC® Member Report

Build 7493

Job name:

File name: 38-10.mmdl

Address:

Description: 1ST FLR FRAMING\Flush Beams\B2(i2168)

City, Province, Postal Code: RICHMOND HILL

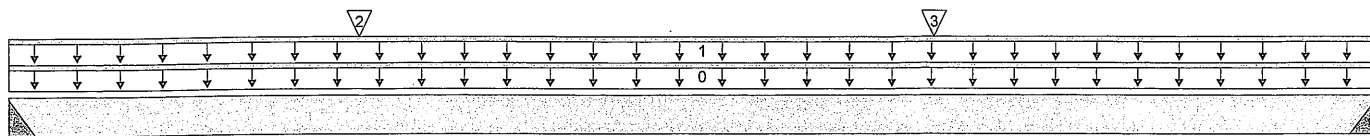
Specifier:

Customer:

Designer: LBV

Code reports: CCMC 12472-R

Company:



B1

03-02-04

B2

Total Horizontal Product Length = 03-02-04

### Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B1, 3"	507 / 0	261 / 0		
B2, 3"	494 / 0	255 / 0		

### Load Summary

Tag	Description	Load Type	Ref.	Start	End	Loc.	Live	Dead	Snow	Wind	Tributary
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	03-02-04	Top	1.00	0.65	1.00	1.15	00-00-00
1	STAIR	Unf. Lin. (lb/ft)	L	00-00-00	03-02-04	Top	240	120			n/a
2	J6(i2167)	Conc. Pt. (lbs)	L	00-09-10	00-09-10	Top	110	55			n/a
3	J6(i2188)	Conc. Pt. (lbs)	L	02-01-10	02-01-10	Top	126	63			n/a

### Controls Summary

	Factored Demand	Factored Resistance	Demand/Resistance	Case	Location
Pos. Moment	698 ft-lbs	11610 ft-lbs	6.0%	1	01-07-14
End Shear	521 lbs	5785 lbs	9.0%	1	02-01-12
Total Load Deflection	L/999 (0.003")	n/a	n/a	4	01-07-01
Live Load Deflection	L/999 (0.002")	n/a	n/a	5	01-07-01
Max Defl.	0.003"	n/a	n/a	4	01-07-01
Span / Depth	3.6				

### Bearing Supports

	Dim. (LxW)	Demand	Demand/Resistance Support	Demand/Resistance Member	Material
B1	Hanger 3" x 1-3/4"	1087 lbs	n/a	17.0%	HUS1.81/10
B2	Hanger 3" x 1-3/4"	1059 lbs	n/a	16.5%	HUS1.81/10

### Cautions

Header for the hanger HUS1.81/10 is a Single 1-3/4" x 9-1/2" LVL Beam.

Hanger model HUS1.81/10 and seat length were input by the user. Hanger has not been analyzed for adequate capacity.

Header for the hanger HUS1.81/10 is a Double 1-3/4" x 9-1/2" LVL Beam.

### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA O86.

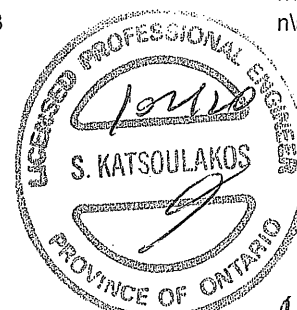
BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor : Normal Part code : Part 9

CONFORMS TO CBC 2012

AMENDED 2020



DWG NO. TAM 14527-20  
STRUCTURAL  
COMPONENT ONLY

### Disclosure

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BC CALC® Member Report

Build 7493

Job name:

File name: 38-10.mmdl

Address:

Description: 1ST FLR FRAMING\Flush Beams\B3(i2171)

City, Province, Postal Code: RICHMOND HILL

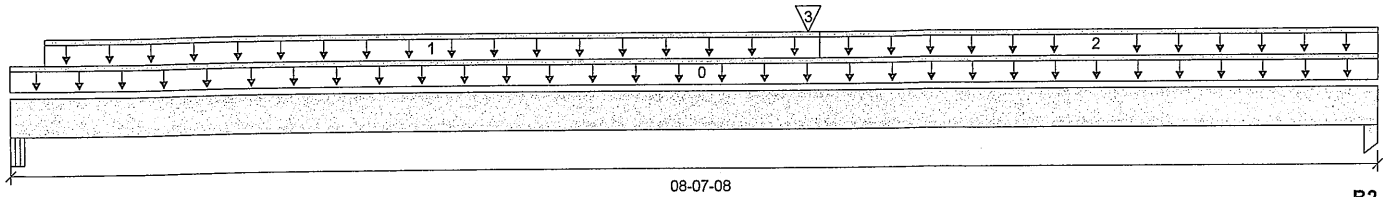
Specifier:

Customer:

Designer: LBV

Code reports: CCMC 12472-R

Company:



Total Horizontal Product Length = 08-07-08

### Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B1, 5-1/4"	316 / 0	183 / 0		
B2, 3-1/2"	357 / 0	203 / 0		

### Load Summary

Tag	Description	Load Type	Ref.	Start	End	Loc.	Live	Dead	Snow	Wind	Tributary
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	08-07-08	Top	1.00	0.65	1.00	1.15	00-00-00
1	FC1 Floor Material	Unf. Lin. (lb/ft)	L	00-02-10	05-00-08	Top	27	13			n/a
2	FC1 Floor Material	Unf. Lin. (lb/ft)	L	05-00-08	08-07-08	Top	11	5			n/a
3	B2(i2168)	Conc. Pt. (lbs)	L	04-11-10	04-11-10	Top	506	261			n/a

### Controls Summary

	Factored Demand	Factored Resistance	Demand/Resistance	Case	Location
Pos. Moment	2514 ft-lbs	11610 ft-lbs	21.7%	1	04-11-10
End Shear	758 lbs	5785 lbs	13.1%	1	07-06-08
Total Load Deflection	L/999 (0.069")	n/a	n/a	4	04-06-00
Live Load Deflection	L/999 (0.044")	n/a	n/a	5	04-06-00
Max Defl.	0.069"	n/a	n/a	4	04-06-00
Span / Depth	10.1				

### Bearing Supports

	Dim. (LxW)	Demand	Demand/Resistance Support	Demand/Resistance Member	Material
B1	Beam 5-1/4" x 1-3/4"	702 lbs	17.9%	6.3%	Unspecified
B2	Column 3-1/2" x 1-3/4"	789 lbs	19.8%	10.6%	Unspecified

### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor : Normal Part code : Part 9

CONFORMS TO OBC 2012

AMENDED 2020



### Disclosure

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BC CALC® Member Report

Build 7493

Job name:

Address:

City, Province, Postal Code: RICHMOND HILL

Customer:

Code reports: CCMC 12472-R

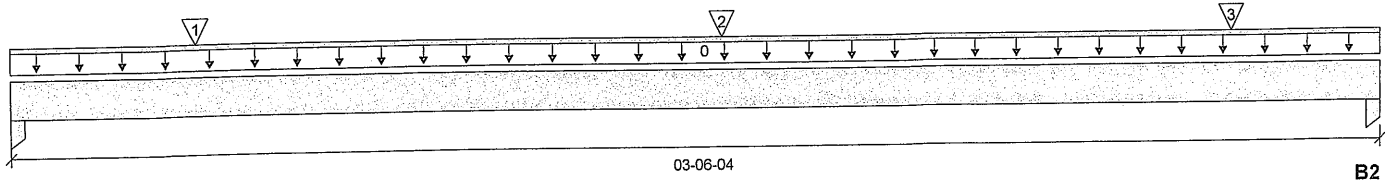
File name: 38-10.mmdl

Description: 1ST FLR FRAMING\Flush Beams\B4(i2988)

Specifier:

Designer: LBV

Company:



Total Horizontal Product Length = 03-06-04

### Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B1, 1-3/4"	269 / 0	142 / 0		
B2, 1-3/4"	277 / 0	147 / 0		

### Load Summary

Tag	Description	Load Type	Ref.	Start	End	Loc.	Live	Dead	Snow	Wind	Tributary
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	03-06-04	Top	1.00	0.65	1.00	1.15	00-00-00
1	J4(i2996)	Conc. Pt. (lbs)	L	00-05-10	00-05-10	Top	163	81			n/a
2	J4(i3001)	Conc. Pt. (lbs)	L	01-09-10	01-09-10	Top	227	113			n/a
3	J4(i2197)	Conc. Pt. (lbs)	L	03-01-10	03-01-10	Top	156	78			n/a

### Controls Summary

	Factored Demand	Factored Resistance	Demand/Resistance	Case	Location
Pos. Moment	528 ft-lbs	11610 ft-lbs	4.6%	1	01-09-10
End Shear	371 lbs	5785 lbs	6.4%	1	00-11-04
Total Load Deflection	L/999 (0.003")	n/a	n/a	4	01-09-00
Live Load Deflection	L/999 (0.002")	n/a	n/a	5	01-09-00
Max Defl.	0.003"	n/a	n/a	4	01-09-00
Span / Depth	4.2				

### Bearing Supports

	Dim. (LxW)	Demand	Demand/Resistance Support	Demand/Resistance Member	Material
B1	Column 1-3/4" x 1-3/4"	582 lbs	29.2%	15.6%	Unspecified
B2	Column 1-3/4" x 1-3/4"	599 lbs	30.1%	16.0%	Unspecified

### Notes

Design meets Code minimum (L/240) Total load deflection criteria.  
 Design meets Code minimum (L/360) Live load deflection criteria.  
 Calculations assume unbraced length of Top: 00-00-00, Bottom: 00-00-00.  
 Resistance Factor phi has been applied to all presented results per CSA O86.  
 BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.  
 Design based on Dry Service Condition.  
 Importance Factor : Normal Part code : Part 9

CONFORMS TO OBC 2012

AMENDED 2020



ENG NO. 192426  
 STRUCTURAL  
 COMPONENT ONLY

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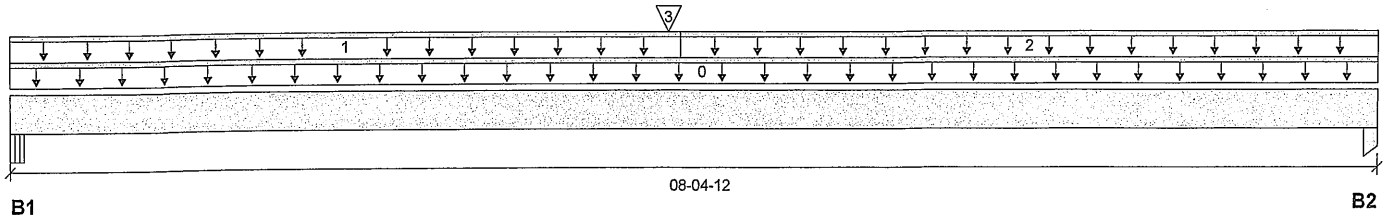
BC CALC® Member Report  
 Build 7493

Dry | 1 span | No cant.

October 8, 2020 17:23:39

Job name:  
 Address:  
 City, Province, Postal Code: RICHMOND HILL  
 Customer:  
 Code reports: CCMC 12472-R

File name: 38-10.mmdl  
 Description: 1ST FLR FRAMING\Flush Beams\B5(i2990)  
 Specifier:  
 Designer: LBV  
 Company:



Total Horizontal Product Length = 08-04-12

**Reaction Summary (Down / Uplift) (lbs)**

Bearing	Live	Dead	Snow	Wind
B1, 4-7/8"	370 / 0	211 / 0		
B2, 3-1/2"	324 / 0	186 / 0		

**Load Summary**

Tag	Description	Load Type	Ref.	Start	End	Loc.	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Tributary
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	08-04-12	Top		5			00-00-00
1	FC1 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	04-00-12	Top	15	7			n/a
2	FC1 Floor Material	Unf. Lin. (lb/ft)	L	04-00-12	08-04-12	Top	12	6			n/a
3	B6(i3021)	Conc. Pt. (lbs)	L	03-11-14	03-11-14	Top	580	300			n/a

**Controls Summary**

	Factored Demand	Factored Resistance	Demand/Resistance	Case	Location
Pos. Moment	2690 ft-lbs	11610 ft-lbs	23.2%	1	03-11-14
End Shear	774 lbs	5785 lbs	13.4%	1	01-02-06
Total Load Deflection	L/999 (0.069")	n/a	n/a	4	04-02-00
Live Load Deflection	L/999 (0.044")	n/a	n/a	5	04-02-00
Max Defl.	0.069"	n/a	n/a	4	04-02-00
Span / Depth	9.9				

**Bearing Supports**

	Dim. (LxW)	Demand	Demand/Resistance Support	Demand/Resistance Member	Material
B1	Beam 4-7/8" x 1-3/4"	819 lbs	22.5%	7.9%	Unspecified
B2	Column 3-1/2" x 1-3/4"	718 lbs	18.1%	9.6%	Unspecified

**Notes**

Design meets Code minimum (L/240) Total load deflection criteria.  
 Design meets Code minimum (L/360) Live load deflection criteria.  
 Calculations assume member is fully braced.  
 Resistance Factor phi has been applied to all presented results per CSA O86.  
 BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.  
 Design based on Dry Service Condition.  
 Importance Factor : Normal Part code : Part 9

CONFORMS TO OBC 2012

AMENDED 2020



DWG NO. TAM 14530-20  
**STRUCTURAL**  
 COMPONENT ONLY

**Disclosure**

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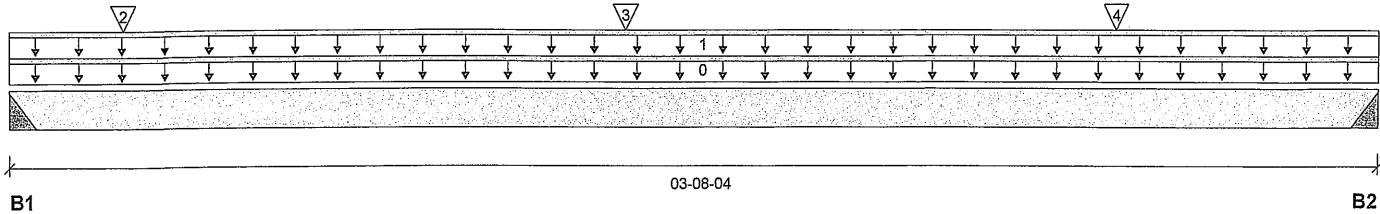
BC CALC® Member Report  
 Build 7493

Dry | 1 span | No cant.

October 8, 2020 17:23:39

Job name:  
 Address:  
 City, Province, Postal Code: RICHMOND HILL  
 Customer:  
 Code reports: CCMC 12472-R

File name: 38-10.mmdl  
 Description: 1ST FLR FRAMING\Flush Beams\B6(i3021)  
 Specifier:  
 Designer: LBV  
 Company:



### Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B1, 3"	587 / 0	303 / 0		
B2, 3"	578 / 0	298 / 0		

### Load Summary

Tag	Description	Load Type	Ref.	Start	End	Loc.	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Tributary
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	03-08-04	Top		5			00-00-00
1	STAIR	Unf. Lin. (lb/ft)	L	00-00-00	03-08-04	Top	240	120			n/a
2	J7(i2969)	Conc. Pt. (lbs)	L	00-03-10	00-03-10	Top	69	35			n/a
3	J7(i2963)	Conc. Pt. (lbs)	L	01-07-10	01-07-10	Top	104	52			n/a
4	J7(i2961)	Conc. Pt. (lbs)	L	02-11-10	02-11-10	Top	107	54			n/a

### Controls Summary

	Factored Demand	Factored Resistance	Demand/Resistance	Case	Location
Pos. Moment	932 ft-lbs	11610 ft-lbs	8.0%	1	01-08-12
End Shear	609 lbs	5785 lbs	10.5%	1	02-07-12
Total Load Deflection	L/999 (0.005")	n/a	n/a	4	01-10-04
Live Load Deflection	L/999 (0.003")	n/a	n/a	5	01-10-04
Max Defl.	0.005"	n/a	n/a	4	01-10-04
Span / Depth	4.2				

Bearing Supports	Dim. (LxW)	Demand	Demand/Resistance Support	Demand/Resistance Member	Material
B1 Hanger	3" x 1-3/4"	1260 lbs	n/a	19.7%	HUS1.81/10
B2 Hanger	3" x 1-3/4"	1239 lbs	n/a	19.4%	HUS1.81/10

### Cautions

Header for the hanger HUS1.81/10 is a Double 1-3/4" x 9-1/2" LVL Beam.  
 Hanger model HUS1.81/10 and seat length were input by the user. Hanger has not been analyzed for adequate capacity.  
 Header for the hanger HUS1.81/10 is a Single 1-3/4" x 9-1/2" LVL Beam.

### Notes

Design meets Code minimum (L/240) Total load deflection criteria.  
 Design meets Code minimum (L/360) Live load deflection criteria.  
 Calculations assume member is fully braced.  
 Hanger Manufacturer: Unassigned  
 Resistance Factor phi has been applied to all presented results per CSA O86.  
 BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.  
 Design based on Dry Service Condition.  
 Importance Factor : Normal Part code : Part 9



OWB NO. TAM 14531-20  
 STRUCTURAL  
 COMPONENT ONLY

### Disclosure

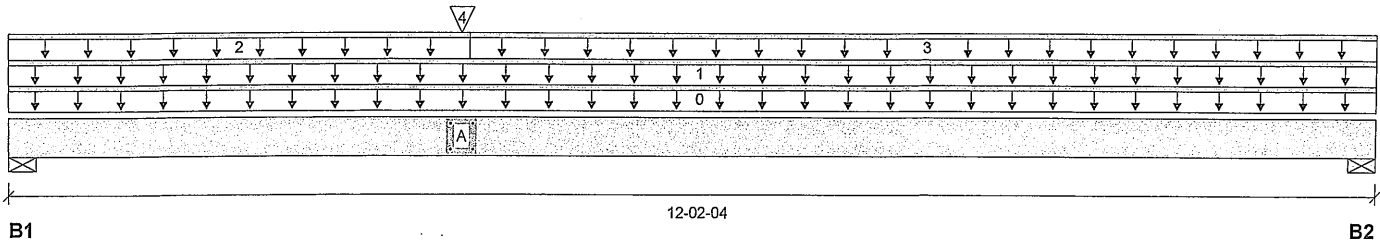
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CONFORMS TO OBC 2012

AMENDED 2020





Total Horizontal Product Length = 12-02-04

### Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B1, 4-3/8"	558 / 0	345 / 0		
B2, 2-3/8"	327 / 0	224 / 0		

### Load Summary

Tag	Description	Load Type	Ref.	Start	End	Loc.	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Tributary
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	12-02-04	Top		10			00-00-00
1	FC1 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	12-02-04	Top	18	9			n/a
2	FC1 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	04-00-12	Top	9	4			n/a
3	FC1 Floor Material	Unf. Lin. (lb/ft)	L	04-00-12	12-02-04	Top	6	3			n/a
4	B6(i3021)	Conc. Pt. (lbs)	L	03-11-14	03-11-14	Top	585	302			n/a

### Controls Summary

	Factored Demand	Factored Resistance	Demand/Resistance	Case	Location
Pos. Moment	4130 ft-lbs	23220 ft-lbs	17.8%	1	03-11-14
End Shear	1188 lbs	11571 lbs	10.3%	1	01-01-14
Total Load Deflection	L/999 (0.125")	n/a	n/a	4	05-10-03
Live Load Deflection	L/999 (0.077")	n/a	n/a	5	05-10-03
Max Defl.	0.125"	n/a	n/a	4	05-10-03
Span / Depth	14.8				

### Bearing Supports

	Dim. (LxW)	Demand	Demand/Resistance Support	Demand/Resistance Member	Material
B1	Wall/Plate 4-3/8" x 3-1/2"	1267 lbs	13.5%	6.8%	Spruce-Pine-Fir
B2	Wall/Plate 2-3/8" x 3-1/2"	771 lbs	15.1%	7.6%	Spruce-Pine-Fir

### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

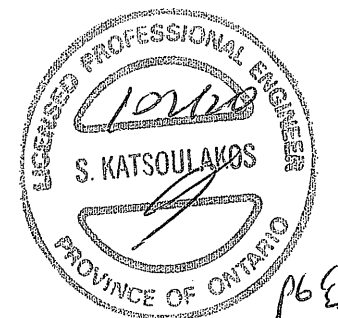
BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor : Normal Part code : Part 9

CONFORMS TO OBC 2012

AMENDED 2020



DWG NO. TAM/453220  
STRUCTURAL  
COMPONENT ONLY

BC CALC® Member Report

Build 7493

Job name:

Address:

City, Province, Postal Code: RICHMOND HILL

Customer:

Code reports: CCMC 12472-R

File name: 38-10.mmdl

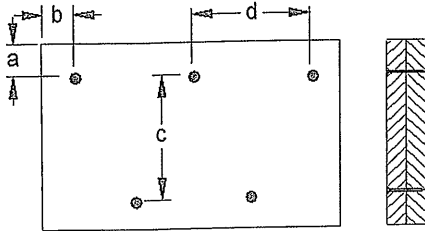
Description: 1ST FLR FRAMING\Flush Beams\B7(i2981)

Specifier:

Designer: LBV

Company:

## Connection Diagram: Full Length of Member



a minimum = 2"  
b minimum = 3"

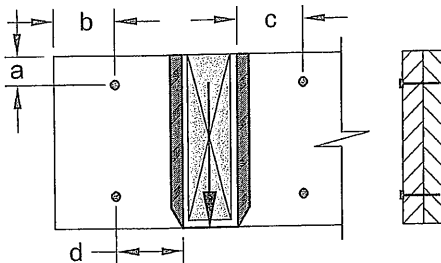
c = 5-1/2"  
d = 8"

Connectors are: 3-1/2" ARDOX SPIRAL Nails

## Connection Diagrams: Concentrated Side Loads

Connection Tag: A

Applies to load tag(s): 3



a minimum = 2"  
b minimum = 4"  
c minimum = 4"  
d maximum = 12"

Connectors are: 16d Nails

3-1/2" ARDOX SPIRAL



DWG NO. TAM 14532-20  
STRUCTURAL  
COMPONENT ONLY

## Disclosure

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BC CALC® Member Report

Build 7493

Job name:

File name: 38-10 SUNKEN.mmdl

Address:

Description: 1ST FLR FRAMING\Flush Beams\B21(i3238)

City, Province, Postal Code: RICHMOND HILL

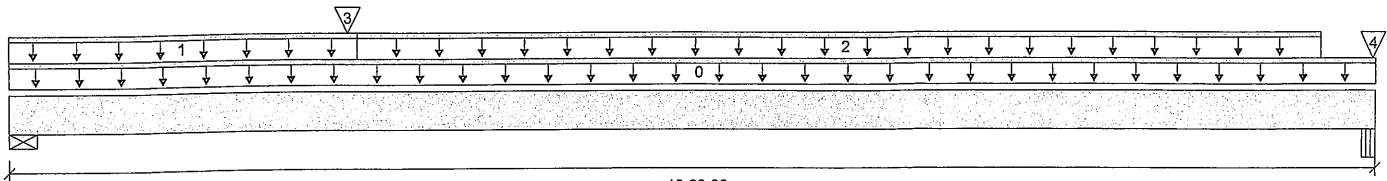
Specifier:

Customer:

Designer: LBV

Code reports: CCMC 12472-R

Company:



B1

Total Horizontal Product Length = 10-08-08

B2

### Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B1, 4-3/8"	121 / 0	109 / 0		
B2, 5-1/4"	345 / 0	265 / 0		

### Load Summary

Tag	Description	Load Type	Ref.	Start	End	Loc.	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Tributary
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	10-08-08	Top		10			00-00-00
1	FC1 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	02-08-06	Top	27	13			n/a
2	FC1 Floor Material	Unf. Lin. (lb/ft)	L	02-08-06	10-03-04	Top	16	8			n/a
3	B22(i3261)	Conc. Pt. (lbs)	L	02-07-08	02-07-08	Top	14	4			n/a
4	4(i590)	Conc. Pt. (lbs)	L	10-08-04	10-08-04	Top	260	171			n/a

### Controls Summary

	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	640 ft-lbs	23220 ft-lbs	2.8%	1	05-00-06
End Shear	239 lbs	11571 lbs	2.1%	1	01-01-14
Total Load Deflection	L/999 (0.017")	n/a	n/a	4	05-02-14
Live Load Deflection	L/999 (0.009")	n/a	n/a	5	05-02-14
Max Defl.	0.017"	n/a	n/a	4	05-02-14
Span / Depth	12.7				

			Demand/ Resistance Support	Demand/ Resistance Member	Material	
Bearing Supports	Dim. (LxW)	Demand				
B1	Wall/Plate	4-3/8" x 3-1/2"	318 lbs	3.4%	1.7%	Spruce-Pine-Fir
B2	Beam	5-1/4" x 3-1/2"	848 lbs	10.8%	3.8%	Unspecified

### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

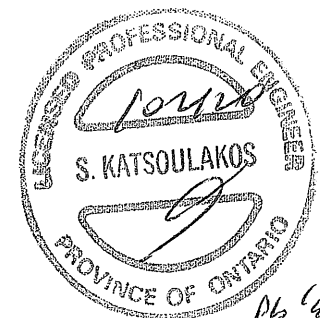
BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor : Normal Part code : Part 9

CONFORMS TO OBC 2012

AMENDED 2020



OWN NO. TAM 14533-20  
STRUCTURAL  
COM. ONENT ONLY

BC CALC® Member Report

Dry | 1 span | No cant.

October 8, 2020 17:36:33

Build 7493

Job name:

File name: 38-10 SUNKEN.mmdl

Address:

Description: 1ST FLR FRAMING\Flush Beams\B21(i3238)

City, Province, Postal Code: RICHMOND HILL

Specifier:

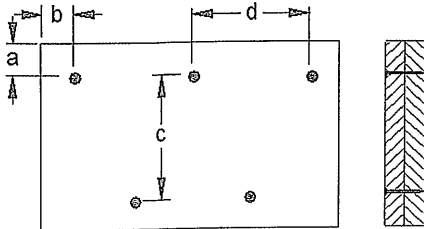
Customer:

Designer: LBV

Code reports: CCMC 12472-R

Company:

## Connection Diagram: Full Length of Member



a minimum = 2"

c = 5-1/2"

b minimum = 3"

d = 8"

Calculated Side Load = 13.0 lb/ft

Connectors are: 3-1/2" ARDOX SPIRAL Nails

3-1/2" ARDOX SPIRAL



DWG NO. TAM 14533-20  
STRUCTURAL  
COMPONENT ONLY

## Disclosure

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BC CALC® Member Report

Dry | 1 span | No cant.

October 8, 2020 17:36:33

Build 7493

Job name:

File name: 38-10 SUNKEN.mmdl

Address:

Description: 1ST FLR FRAMING\Flush Beams\B22(i3261)

City, Province, Postal Code: RICHMOND HILL

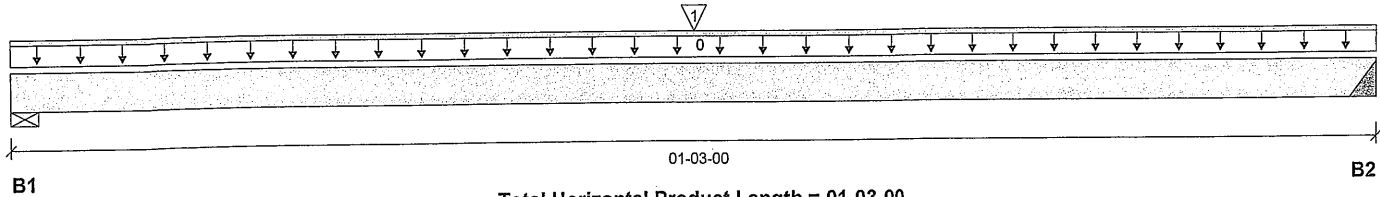
Specifier:

Customer:

Designer: LBV

Code reports: CCMC 12472-R

Company:



### Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B1, 5-1/2"	20 / 0	26 / 0		
B2, 2"	15 / 0	9 / 0		

### Load Summary

Tag	Description	Load Type	Ref.	Start	End	Loc.	Live	Dead	Snow	Wind	Tributary
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	01-03-00	Top	1.00	0.65	1.00	1.15	00-00-00
1	-	Conc. Pt. (lbs)	L	00-07-07	00-07-07	Top	35	29			n/a

### Controls Summary

	Factored Demand	Factored Resistance	Demand/Resistance	Case	Location
Pos. Moment	14 ft-lbs	11610 ft-lbs	0.1%	1	00-08-08
End Shear	6 lbs	5785 lbs	0.1%	1	00-03-08
Span / Depth	0.9				

### Bearing Supports

	Dim. (LxW)	Demand	Demand/Resistance Support	Demand/Resistance Member	Material
B1	Wall/Plate 5-1/2" x 1-3/4"	63 lbs	1.1%	0.5%	Spruce-Pine-Fir
B2	Hanger 2" x 1-3/4"	34 lbs	n/a	0.8%	Hanger

### Cautions

Hanger model Hanger was not found. Hanger has not been analyzed for adequate capacity.

### Notes

Calculations assume member is fully braced.

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor : Normal Part code : Part 9

CONFORMS TO OBC 2012

AMENDED 2020

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DWG NO. TAM 14534-20  
 STRUCTURAL  
 COMPONENT ONLY

BC CALC® Member Report

Dry | 1 span | No cant.

October 8, 2020 17:36:33

Build 7493

Job name:

File name: 38-10 SUNKEN.mmdl

Address:

Description: 1ST FLR FRAMING\Flush Beams\B23(i3350)

City, Province, Postal Code: RICHMOND HILL

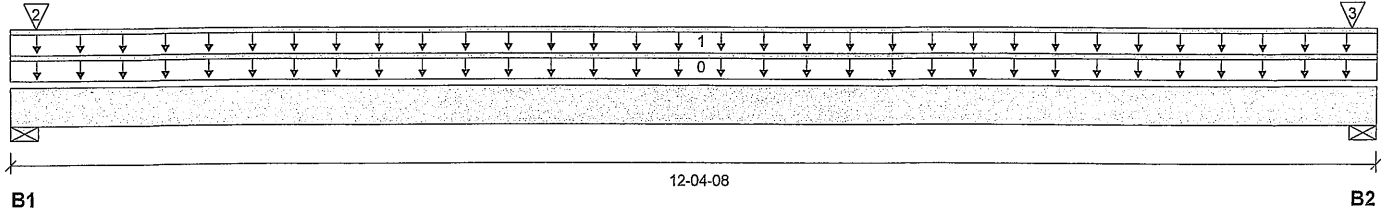
Specifier:

Customer:

Designer: LBV

Code reports: CCMC 12472-R

Company:



Total Horizontal Product Length = 12-04-08

**Reaction Summary (Down / Uplift) (lbs)**

Bearing	Live	Dead	Snow	Wind
B1, 5-1/2"	364 / 0	405 / 0	152 / 0	
B2, 3-1/2"	173 / 0	140 / 0		

**Load Summary**

Tag	Description	Load Type	Ref.	Start	End	Loc.	Live	Dead	Snow	Wind	Tributary
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	12-04-08	Top	5				00-00-00
1	FC1 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	12-04-08	Top	22	11			n/a
2	E43(i3386)	Conc. Pt. (lbs)	L	00-02-12	00-02-12	Top	223	304	152		n/a
3	E11(i445)	Conc. Pt. (lbs)	L	12-01-12	12-01-12	Top	36	42			n/a

**Controls Summary**

	Factored Demand	Factored Resistance	Demand/Resistance	Case	Location
Pos. Moment	929 ft-lbs	11610 ft-lbs	8.0%	1	06-03-04
End Shear	270 lbs	5785 lbs	4.7%	1	01-03-00
Total Load Deflection	L/999 (0.066")	n/a	n/a	35	06-03-04
Live Load Deflection	L/999 (0.039")	n/a	n/a	51	06-03-04
Max Defl.	0.066"	n/a	n/a	35	06-03-04
Span / Depth	14.8				

**Bearing Supports**

	Dim. (LxW)	Demand	Demand/Resistance Support	Demand/Resistance Member	Material
B1	Wall/Plate 5-1/2" x 1-3/4"	1204 lbs	20.3%	10.3%	Spruce-Pine-Fir
B2	Wall/Plate 3-1/2" x 1-3/4"	435 lbs	11.5%	5.8%	Spruce-Pine-Fir

**Notes**

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

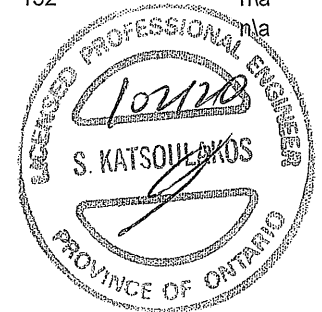
Unbalanced snow loads determined from building geometry were used in selected product's verification.

Design based on Dry Service Condition.

Importance Factor : Normal Part code : Part 9

CONFORMS TO OBC 2012

AMENDED 2020


BWG NO. TAM 14535-20  
**STRUCTURAL**  
**COM-ONENT ONLY**
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BC CALC® Member Report

Dry | 1 span | No cant.

October 8, 2020 17:12:23

Build 7493

Job name:

File name: 38-10.mmdl

Address:

Description: 2ND FLR FRAMING\Flush Beams\B14A(i2435)

City, Province, Postal Code: RICHMOND HILL

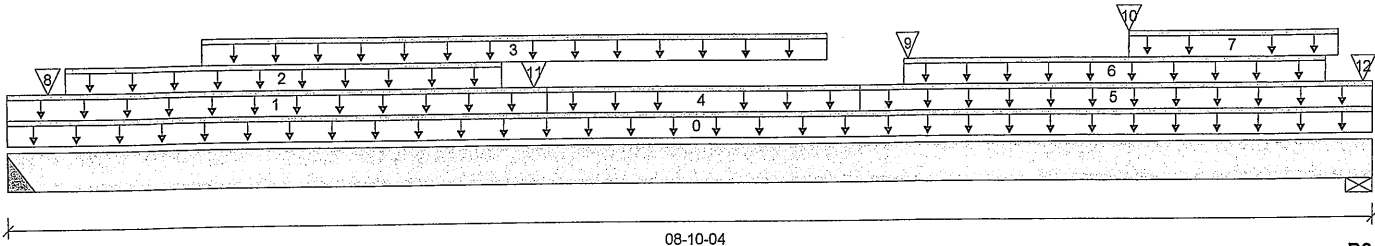
Specifier:

Customer:

Designer: LBV

Code reports: CCMC 12472-R

Company:



B1

Total Horizontal Product Length = 08-10-04

B2

### Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B1, 4"	913 / 0	1325 / 0	1053 / 0	
B2, 2-3/4"	772 / 0	1268 / 0	1112 / 0	

### Load Summary

Tag	Description	Load Type	Ref.	Start	End	Loc.	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Tributary
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	08-10-04	Top		10			00-00-00
1	E40(i1271)	Unf. Lin. (lb/ft)	L	00-00-00	03-05-08	Top		81			n/a
2	E40(i1271)	Unf. Lin. (lb/ft)	L	00-04-08	03-02-00	Top		56	129		n/a
3	Smoothed Load	Unf. Lin. (lb/ft)	L	01-03-00	05-03-00	Top	214	107			n/a
4	E41(i1272)	Unf. Lin. (lb/ft)	L	03-05-08	05-05-08	Top		41			n/a
5	E27(i618)	Unf. Lin. (lb/ft)	L	05-05-08	08-10-04	Top		81			n/a
6	E27(i618)	Unf. Lin. (lb/ft)	L	05-09-00	08-06-08	Top		56	129		n/a
7	FC2 Floor Material	Unf. Lin. (lb/ft)	L	07-03-00	08-07-08	Top	14				n/a
8	-	Conc. Pt. (lbs)	L	00-03-02	00-03-02	Top	221	363	523		n/a
9	-	Conc. Pt. (lbs)	L	05-09-05	05-09-05	Top	285	233	163		n/a
10	J3(i2625)	Conc. Pt. (lbs)	L	07-03-00	07-03-00	Top	305	152			n/a
11	E40(i1271)	Conc. Pt. (lbs)	L	03-04-08	03-04-08	Top		93	170		n/a
12	E27(i618)	Conc. Pt. (lbs)	L	08-09-08	08-09-08	Top		279	589		n/a

### Controls Summary

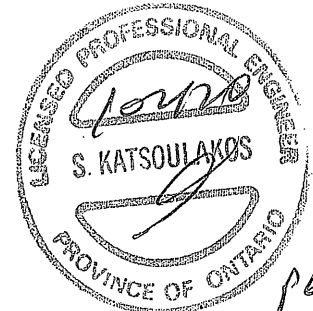
	Factored Demand	Factored Resistance	Demand/Resistance	Case	Location
Pos. Moment	6606 ft-lbs	23220 ft-lbs	28.4%	1	04-07-00
End Shear	2791 lbs	11571 lbs	24.1%	1	01-01-08
Total Load Deflection	L/770 (0.131")	n/a	31.2%	35	04-05-05
Live Load Deflection	L/999 (0.076")	n/a	n/a	51	04-05-05
Max Defl.	0.131"	n/a	n/a	35	04-05-05
Span / Depth	10.6				

### Bearing Supports

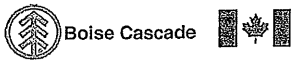
	Dim. (LxW)	Demand	Demand/Resistance Support	Demand/Resistance Member	Material
B1	Hanger 4" x 3-1/2"	4148 lbs	n/a	24.3%	HGUS410
B2	Wall/Plate 2-3/4" x 3-1/2"	4025 lbs	68.0%	34.3%	Spruce-Pine-Fir

### Cautions

Header for the hanger HGUS410 is a Double 1-3/4" x 9-1/2" LVL Beam.  
Hanger model HGUS410 and seat length were input by the user. Hanger has not been analyzed for adequate capacity.



HWB NO. TAM 1453620  
STRUCTURAL  
COM. ONENT ONLY



**Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP**  
**2ND FLR FRAMING\Flush Beams\B14A(i2435) (Flush Beam)**

**PASSED**

BC CALC® Member Report

Dry | 1 span | No cant.

October 8, 2020 17:12:23

Build 7493

Job name:

File name: 38-10.mmdl

Address:

Description: 2ND FLR FRAMING\Flush Beams\B14A(i2435)

City, Province, Postal Code: RICHMOND HILL

Specifier:

Customer:

Designer: LBV

Code reports: CCMC 12472-R

Company:

### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Unbalanced snow loads determined from building geometry were used in selected product's verification.

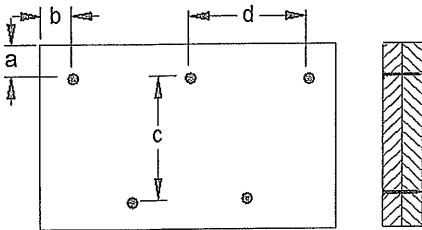
Design based on Dry Service Condition.

Importance Factor : Normal Part code : Part 9

CONFORMS TO OBC 2012

AMENDED 2020

### Connection Diagram: Full Length of Member



a minimum = 2"

c = 5-1/2"

b minimum = 3"

d = 10 3/8"

Calculated Side Load = 626.9 lb/ft

Connectors are: 16d Nails

**3-1/2" ARDOX SPIRAL**



HWG NO. TAM 14536-20  
STRUCTURAL  
COMPONENT ONLY

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BC CALC® Member Report

Build 7493

Job name:

File name: 38-10.mmdl

Address:

Description: 2ND FLR FRAMING\Flush Beams\B15A(i2587)

City, Province, Postal Code: RICHMOND HILL

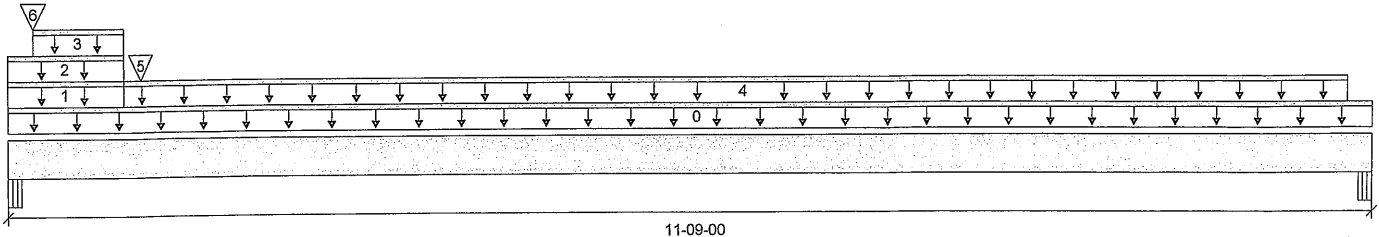
Specifier:

Customer:

Designer: LBV

Code reports: CCMC 12472-R

Company:



B1

Total Horizontal Product Length = 11-09-00

B2

### Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B1, 5-1/4"	969 / 0	1528 / 0	1147 / 0	
B2, 5-1/4"	212 / 0	228 / 0	77 / 0	

### Load Summary

Tag	Description	Load Type	Ref.	Start	End	Loc.	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Tributary
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	11-09-00	Top		10			00-00-00
1	E28(i617)	Unf. Lin. (lb/ft)	L	00-00-00	00-11-14	Top		81			n/a
2	FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	00-11-14	Top	15				n/a
3	E28(i617)	Unf. Lin. (lb/ft)	L	00-02-10	00-11-14	Top		75	163		n/a
4	FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-11-14	11-06-06	Top	27	13			n/a
5	-	Conc. Pt. (lbs)	L	01-01-10	01-01-10	Top	885	1344	1069		n/a
6	E28(i617)	Conc. Pt. (lbs)	L	00-02-10	00-02-10	Top			30		n/a

### Controls Summary

	Factored Demand	Factored Resistance	Demand/Resistance	Case	Location
Pos. Moment	3249 ft-lbs	23220 ft-lbs	14.0%	13	01-02-10
End Shear	3914 lbs	11571 lbs	33.8%	13	01-02-12
Total Load Deflection	L/999 (0.101")	n/a	n/a	35	05-03-07
Live Load Deflection	L/999 (0.058")	n/a	n/a	51	05-03-07
Max Defl.	0.101"	n/a	n/a	35	05-03-07
Span / Depth	13.9				

Bearing Supports	Dim. (LxW)	Demand	Demand/Resistance Support	Demand/Resistance Member	Material
B1 Beam	5-1/4" x 3-1/2"	4600 lbs	58.6%	20.5%	Unspecified
B2 Beam	5-1/4" x 3-1/2"	680 lbs	8.7%	3.0%	Unspecified

### Cautions

Concentrated side load(s) 14,15,16,17,18 are closer than 18" from end of member. Please consult a technical representative or Professional of Record.



DWG NO. TAM/4537-20  
STRUCTURAL  
COMPONENT ONLY



**Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP**  
**2ND FLR FRAMING\Flush Beams\B15A(i2587) (Flush Beam)**

**PASSED**

BC CALC® Member Report  
Build 7493

Dry | 1 span | No cant.

October 8, 2020 17:12:23

Job name:  
Address:  
City, Province, Postal Code: RICHMOND HILL  
Customer:  
Code reports: CCMC 12472-R

File name: 38-10.mmdl  
Description: 2ND FLR FRAMING\Flush Beams\B15A(i2587)  
Specifier:  
Designer: LBV  
Company:

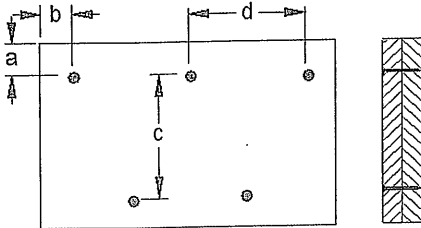
### Notes

Design meets Code minimum (L/240) Total load deflection criteria.  
Design meets Code minimum (L/360) Live load deflection criteria.  
Calculations assume unbraced length of Top: 00-00-00, Bottom: 00-00-00.  
Resistance Factor phi has been applied to all presented results per CSA O86.  
BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.  
Unbalanced snow loads determined from building geometry were used in selected product's verification.  
Design based on Dry Service Condition.  
Importance Factor : Normal Part code : Part 9

CONFORMS TO OBC 2012

AMENDED 2020

### Connection Diagram: Full Length of Member



a minimum = 2"  
b minimum = 3"

c = 5-1/2"  
d = 2-1/2"

Connectors are: 1. Nails  
3-1/2" ARDOX SPIRAL



14537-20  
STRUCTURAL  
COMPONENT ONLY

### Disclosure

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BC CALC®, BC FRAMER®, AJS™,  
ALLJOIST®, BC RIM BOARD™, BCI®,  
BOISE GLULAM™, BC FloorValue®,  
VERSA-LAM®, VERSA-RIM PLUS®,

BC CALC® Member Report

Dry | 1 span | No cant.

October 9, 2020 08:05:16

Build 7493

Job name:

File name: 38-10 EL B.mmdl

Address:

Description: 2ND FLR FRAMING\Flush Beams\B15B(i3483)

City, Province, Postal Code: RICHMOND HILL

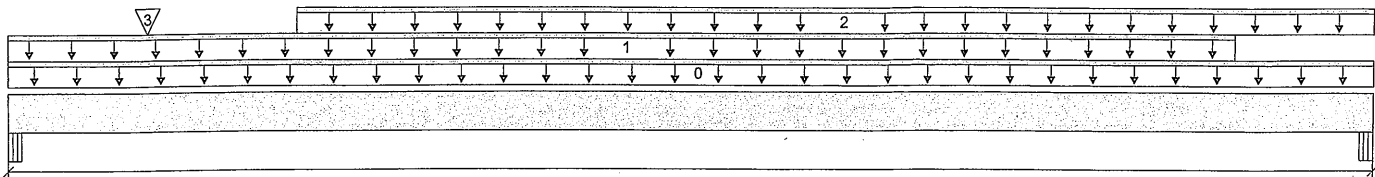
Specifier:

Customer:

Designer: LBV

Code reports: CCMC 12472-R

Company:



B1

02-01-10

B2

Total Horizontal Product Length = 02-01-10

### Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B1, 5-1/4"	39 / 0	199 / 0	187 / 0	
B2, 2-3/4"	25 / 0	172 / 0	157 / 0	

### Load Summary

Tag	Description	Load Type	Ref.	Start	End	Loc.	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Tributary
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	02-01-10	Top		10			00-00-00
1	FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	01-11-00	Top	33	17			n/a
2	E28(i617)	Unf. Lin. (lb/ft)	L	00-05-06	02-01-10	Top		156	163		n/a
3	E40(i1271)	Conc. Pt. (lbs)	L	00-02-10	00-02-10	Top		55	69		n/a

### Controls Summary

	Factored Demand	Factored Resistance	Demand/Resistance	Case	Location
Pos. Moment	160 ft-lbs	23220 ft-lbs	0.7%	13	01-02-01
End Shear	317 lbs	11571 lbs	2.7%	23	01-02-12
Total Load Deflection	L/999 (0")	n/a	n/a	35	01-02-01
Live Load Deflection	L/999 (0")	n/a	n/a	51	01-02-01
Max Defl.	0"	n/a	n/a	35	01-02-01
Span / Depth	2.0				

### Bearing Supports

	Dim. (LxW)	Demand	Demand/Resistance Support	Demand/Resistance Member	Material
B1 Beam	5-1/4" x 3-1/2"	569 lbs	7.2%	2.5%	Unspecified
B2 Beam	2-3/4" x 3-1/2"	475 lbs	11.5%	4.0%	Unspecified

### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume unbraced length of Top: 00-00-00, Bottom: 00-00-00.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Unbalanced snow loads determined from building geometry were used in selected product's verification.

Design based on Dry Service Condition.

Importance Factor : Normal Part code : Part 9

CONFORMS TO OBC 2012

AMENDED 2020


DWG NO. TAM/4538-20  
STRUCTURAL  
COMPONENT ONLY

BC CALC® Member Report

Dry | 1 span | No cant.

October 9, 2020 08:05:16

Build 7493

Job name:

File name: 38-10 EL B.mmdl

Address:

Description: 2ND FLR FRAMING\Flush Beams\B15B(i3483)

City, Province, Postal Code: RICHMOND HILL

Specifier:

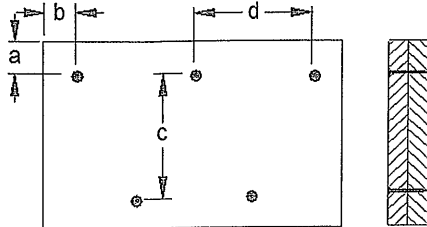
Customer:

Designer: LBV

Code reports: CCMC 12472-R

Company:

### Connection Diagram: Full Length of Member



a minimum = 2"  
b minimum = 3"

c = 5-1/2"  
d = 6"

Connectors are: 1 Nails  
3-1/2" ARDOX SPIRAL



ENG NO. TAM 14538-20  
STRUCTURAL  
COM. ONENT ONLY

### Disclosure

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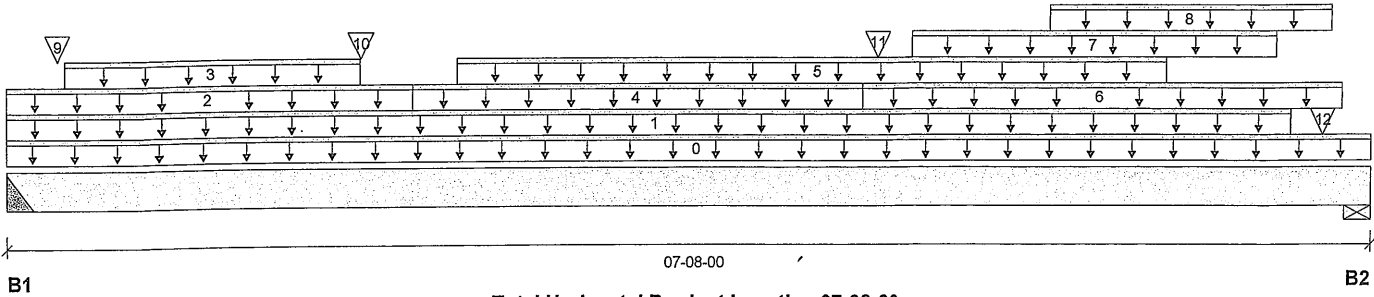
BC CALC® Member Report  
Build 7493

Dry | 1 span | No cant.

October 9, 2020 08:50:30

Job name:  
Address:  
City, Province, Postal Code: RICHMOND HILL  
Customer:  
Code reports: CCMC 12472-R

File name: 38-10 EL C.mmdl  
Description: 2ND FLR FRAMING\Flush Beams\B14C(i4493)  
Specifier:  
Designer: LBV  
Company:



### Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B1, 4"	771 / 0	983 / 0	611 / 0	
B2, 5-1/2"	648 / 0	1058 / 0	817 / 0	

### Load Summary

Tag	Description	Load Type	Ref.	Start	End	Loc.	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Tributary
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	07-08-00	Top		10			00-00-00
1	FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	07-02-08	Top	6	3			n/a
2	E46(i3782)	Unf. Lin. (lb/ft)	L	00-00-00	02-03-00	Top		81			n/a
3	E46(i3782)	Unf. Lin. (lb/ft)	L	00-03-15	01-11-08	Top		48	110		n/a
4	E45(i3785)	Unf. Lin. (lb/ft)	L	02-03-00	04-09-00	Top		41			n/a
5	Smoothed Load	Unf. Lin. (lb/ft)	L	02-06-00	06-06-00	Top	207	104			n/a
6	E44(i3780)	Unf. Lin. (lb/ft)	L	04-09-00	07-06-00	Top		81			n/a
7	E44(i3780)	Unf. Lin. (lb/ft)	L	05-00-08	07-01-08	Top		48	110		n/a
8	FC2 Floor Material	Unf. Lin. (lb/ft)	L	05-10-00	07-05-04	Top	41	20			n/a
9	-	Conc. Pt. (lbs)	L	00-03-08	00-03-08	Top	206	222	232		n/a
10	-	Conc. Pt. (lbs)	L	01-11-09	01-11-09	Top	277	237	172		n/a
11	E44(i3780)	Conc. Pt. (lbs)	L	04-10-00	04-10-00	Top		96	167		n/a
12	-	Conc. Pt. (lbs)	L	07-04-11	07-04-11	Top		258	449		n/a

### Controls Summary

	Factored Demand	Factored Resistance	Demand/Resistance	Case	Location
Pos. Moment	4202 ft-lbs	23220 ft-lbs	18.1%	1	04-02-00
End Shear	2126 lbs	11571 lbs	18.4%	1	06-05-00
Total Load Deflection	L/999 (0.058")	n/a	n/a	35	03-09-00
Live Load Deflection	L/999 (0.034")	n/a	n/a	51	03-09-00
Max Defl.	0.058"	n/a	n/a	35	03-09-00
Span / Depth	8.8				

Bearing Supports	Dim. (LxW)	Demand	Demand/Resistance Support	Demand/Resistance Member	Material
B1 Hanger	4" x 3-1/2"	2997 lbs	n/a	17.5%	HGUS410
B2 Wall/Plate	5-1/2" x 3-1/2"	3196 lbs	27.0%	13.6%	Spruce-Pine-Fir

### Cautions

Header for the hanger HGUS410 is a Double 1-3/4" x 9-1/2" LVL Beam.  
Hanger model HGUS410 and seat length were input by the user. Hanger has not been analyzed for adequate capacity.



000 NO. TAM 14539-00  
STRUCTURAL  
COMPONENT ONLY

BC CALC® Member Report

Dry | 1 span | No cant.

October 9, 2020 08:50:30

Build 7493

Job name:

File name: 38-10 EL C.mmdl

Address:

Description: 2ND FLR FRAMING\Flush Beams\B14C(i4493)

City, Province, Postal Code: RICHMOND HILL

Specifier:

Customer:

Designer: LBV

Code reports: CCMC 12472-R

Company:

## Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

CONFORMS TO OBC 2012

Hanger Manufacturer: Unassigned

AMENDED 2020

Resistance Factor phi has been applied to all presented results per CSA O86.

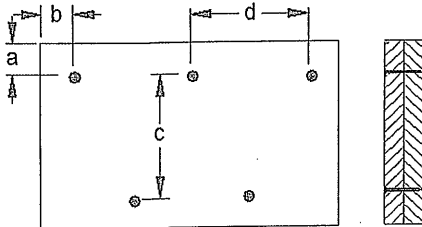
BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Unbalanced snow loads determined from building geometry were used in selected product's verification.

Design based on Dry Service Condition.

Importance Factor : Normal Part code : Part 9

## Connection Diagram: Full Length of Member



a minimum = 2"

c = 5-1/2"

b minimum = 3"

d = 8"

Calculated Side Load = 589.3 lb/ft

Connectors are: 16d Nails

3-1/2" ARDOX SPIRAL



DWG NO. TAM 14539-20  
 STRUCTURAL  
 COMPONENT ONLY

## Disclosure

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Build 7493

Job name:

File name: 38-10 EL C.mmdl

Address:

Description: 2ND FLR FRAMING\Flush Beams\B15C(i4448)

City, Province, Postal Code: RICHMOND HILL

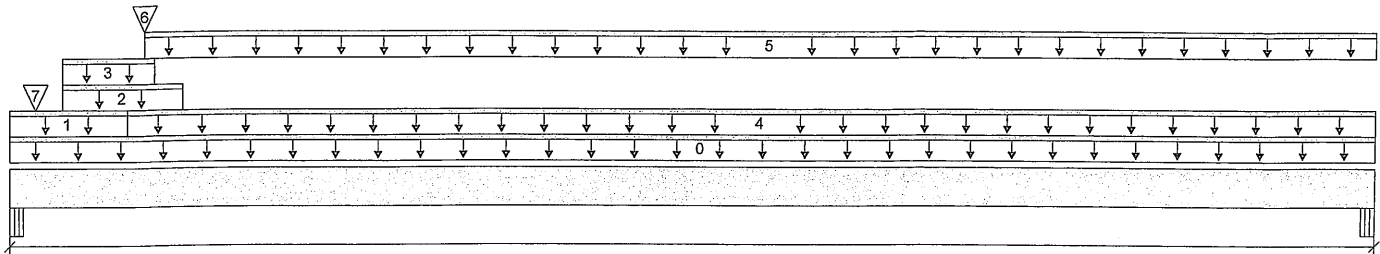
Specifier:

Customer:

Designer: LBV

Code reports: CCMC 12472-R

Company:



B1

Total Horizontal Product Length = 11-06-06

B2

**Reaction Summary (Down / Uplift) (lbs)**

Bearing	Live	Dead	Snow	Wind
B1, 5-1/4"	834 / 0	1203 / 0	754 / 0	
B2, 2-5/8"	202 / 0	202 / 0	47 / 0	

**Load Summary**

Tag	Description	Load Type	Ref.	Start	End	Loc.	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Tributary
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	11-06-06	Top		10			00-00-00
1	FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	00-11-13	Top	17				n/a
2	E47(i3779)	Unf. Lin. (lb/ft)	L	00-05-06	01-05-06	Top		81			n/a
3	E47(i3779)	Unf. Lin. (lb/ft)	L	00-05-06	01-02-10	Top		75	163		n/a
4	FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-11-13	11-06-06	Top	14	7			n/a
5	FC2 Floor Material	Unf. Lin. (lb/ft)	L	01-01-10	11-06-06	Top	13	6			n/a
6	B14C(i4493)	Conc. Pt. (lbs)	L	01-01-10	01-01-10	Top	740	950	600		n/a
7	E48(i3784)	Conc. Pt. (lbs)	L	00-02-10	00-02-10	Top		57	75		n/a

**Controls Summary**

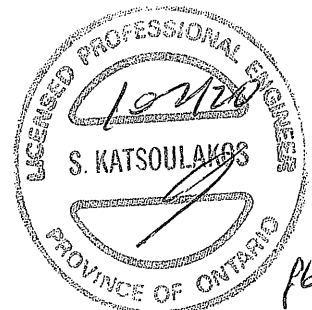
	Factored Demand	Factored Resistance	Demand/Resistance	Case	Location
Pos. Moment	2544 ft-lbs	23220 ft-lbs	11.0%	1	02-08-11
End Shear	2989 lbs	11571 lbs	25.8%	1	01-02-12
Total Load Deflection	L/999 (0.082")	n/a	n/a	35	05-05-00
Live Load Deflection	L/999 (0.046")	n/a	n/a	51	05-05-00
Max Defl.	0.082"	n/a	n/a	35	05-05-00
Span / Depth	13.9				

**Bearing Supports**

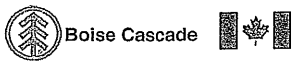
	Dim. (LxW)	Demand	Demand/Resistance Support	Demand/Resistance Member	Material
B1 Beam	5-1/4" x 3-1/2"	3508 lbs	35.8%	15.7%	Unspecified
B2 Beam	2-5/8" x 3-1/2"	602 lbs	12.3%	5.4%	Unspecified

**Cautions**

Concentrated side load(s) 17 are closer than 18" from end of member. Please consult a technical representative or Professional of Record.



DWG NO. TAM 14540-20  
STRUCTURAL  
COMPONENT ONLY



**Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP**  
**2ND FLR FRAMING\Flush Beams\B15C(i4448) (Flush Beam)**

**PASSED**

BC CALC® Member Report  
Build 7493

Dry | 1 span | No cant.

October 9, 2020 08:50:30

Job name:  
Address:  
City, Province, Postal Code: RICHMOND HILL  
Customer:  
Code reports: CCMC 12472-R

File name: 38-10 EL C.mmdl  
Description: 2ND FLR FRAMING\Flush Beams\B15C(i4448)  
Specifier:  
Designer: LBV  
Company:

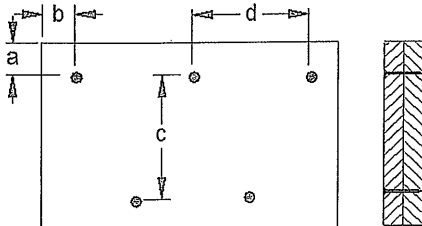
### Notes

Design meets Code minimum (L/240) Total load deflection criteria.  
Design meets Code minimum (L/360) Live load deflection criteria.  
Calculations assume unbraced length of Top: 00-00-00, Bottom: 00-00-00.  
Resistance Factor phi has been applied to all presented results per CSA O86.  
BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.  
Unbalanced snow loads determined from building geometry were used in selected product's verification.  
Design based on Dry Service Condition.  
Importance Factor : Normal Part code : Part 9

**CONFORMS TO OBC 2012**

**AMENDED 2020**

### Connection Diagram: Full Length of Member



a minimum = 2"  
b minimum = 3"  
c = 5-1/2"  
d = 8"

Calculated Side Load = 450.0 lb/ft

Connectors are: 1 Nails  
3-1/2" ARDOX SPIRAL



DWG NO. TAM14540-20  
STRUCTURAL  
COMPONENT ONLY

### Disclosure

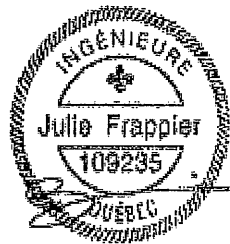
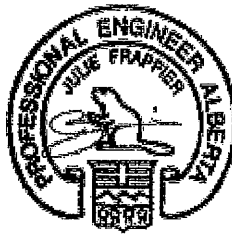
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ALLJOIST®, BC RIM BOARD™, BCI®,  
BOISE GLULAM™, BC FloorValue®,  
VERSA-LAM®, VERSA-RIM PLUS®,



## Maximum Floor Spans

Live Load = 40 psf, Dead Load = 15 psf  
Simple Spans, L/480 Deflection Limit  
5/8" OSB G&N Sheathing

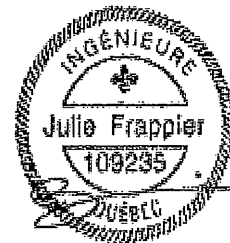
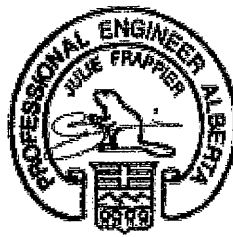


Depth	Series	Bare				1/2" Gypsum Ceiling			
		On Centre Spacing				On Centre Spacing			
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
9-1/2"	NI-20	15'-1"	14'-2"	13'-9"	N/A	15'-7"	14'-8"	14'-2"	N/A
	NI-40x	16'-1"	15'-2"	14'-8"	N/A	16'-7"	15'-7"	15'-1"	N/A
	NI-60	16'-3"	15'-4"	14'-10"	N/A	16'-8"	15'-9"	15'-3"	N/A
	NI-70	17'-1"	16'-1"	15'-6"	N/A	17'-5"	16'-5"	15'-10"	N/A
	NI-80	17'-3"	16'-3"	15'-8"	N/A	17'-8"	16'-7"	16'-0"	N/A
11-7/8"	NI-20	16'-11"	16'-0"	15'-5"	N/A	17'-6"	16'-6"	16'-0"	N/A
	NI-40x	18'-1"	17'-0"	16'-5"	N/A	18'-9"	17'-6"	16'-11"	N/A
	NI-60	18'-4"	17'-3"	16'-7"	N/A	19'-0"	17'-8"	17'-1"	N/A
	NI-70	19'-6"	18'-0"	17'-4"	N/A	20'-1"	18'-7"	17'-9"	N/A
	NI-80	19'-9"	18'-3"	17'-6"	N/A	20'-4"	18'-10"	17'-11"	N/A
14"	NI-90x	20'-4"	18'-9"	17'-11"	N/A	20'-10"	19'-3"	18'-5"	N/A
	NI-40x	20'-1"	18'-7"	17'-10"	N/A	20'-10"	19'-4"	18'-6"	N/A
	NI-60	20'-5"	18'-11"	18'-1"	N/A	21'-2"	19'-7"	18'-9"	N/A
	NI-70	21'-7"	20'-0"	19'-1"	N/A	22'-3"	20'-7"	19'-8"	N/A
	NI-80	21'-11"	20'-3"	19'-4"	N/A	22'-7"	20'-11"	20'-0"	N/A
16"	NI-90x	22'-7"	20'-11"	19'-11"	N/A	23'-3"	21'-6"	20'-6"	N/A
	NI-60	22'-3"	20'-8"	19'-9"	N/A	23'-1"	21'-5"	20'-6"	N/A
	NI-70	23'-6"	21'-9"	20'-9"	N/A	24'-3"	22'-5"	21'-5"	N/A
	NI-80	23'-11"	22'-1"	21'-1"	N/A	24'-8"	22'-10"	21'-9"	N/A
	NI-90x	24'-8"	22'-9"	21'-9"	N/A	25'-4"	23'-5"	22'-4"	N/A

Depth	Series	Mid-Span Blocking				Mid-Span Blocking and 1/2" Gypsum Ceiling			
		On Centre Spacing				On Centre Spacing			
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
9-1/2"	NI-20	16'-8"	15'-3"	14'-5"	N/A	16'-8"	15'-3"	14'-5"	N/A
	NI-40x	17'-11"	16'-11"	16'-1"	N/A	18'-5"	17'-1"	16'-1"	N/A
	NI-60	18'-2"	17'-1"	16'-4"	N/A	18'-7"	17'-4"	16'-4"	N/A
	NI-70	19'-2"	17'-10"	17'-2"	N/A	19'-7"	18'-3"	17'-7"	N/A
	NI-80	19'-5"	18'-0"	17'-4"	N/A	19'-10"	18'-5"	17'-8"	N/A
11-7/8"	NI-20	19'-6"	18'-1"	17'-3"	N/A	19'-11"	18'-3"	17'-3"	N/A
	NI-40x	21'-0"	19'-6"	18'-8"	N/A	21'-7"	20'-2"	19'-2"	N/A
	NI-60	21'-4"	19'-9"	18'-11"	N/A	21'-11"	20'-4"	19'-6"	N/A
	NI-70	22'-6"	20'-10"	19'-11"	N/A	23'-0"	21'-5"	20'-5"	N/A
	NI-80	22'-9"	21'-1"	20'-1"	N/A	23'-3"	21'-7"	20'-8"	N/A
14"	NI-90x	23'-4"	21'-8"	20'-8"	N/A	23'-10"	22'-2"	21'-2"	N/A
	NI-40x	23'-7"	21'-11"	20'-11"	N/A	24'-3"	22'-7"	21'-7"	N/A
	NI-60	24'-0"	22'-3"	21'-3"	N/A	24'-8"	22'-11"	21'-11"	N/A
	NI-70	25'-3"	23'-4"	22'-3"	N/A	25'-10"	24'-0"	22'-11"	N/A
	NI-80	25'-7"	23'-8"	22'-7"	N/A	26'-2"	24'-4"	23'-2"	N/A
16"	NI-90x	26'-4"	24'-4"	23'-3"	N/A	26'-10"	24'-11"	23'-9"	N/A
	NI-60	26'-5"	24'-6"	23'-4"	N/A	27'-2"	25'-3"	24'-2"	N/A
	NI-70	27'-9"	25'-8"	24'-6"	N/A	28'-5"	26'-5"	25'-2"	N/A
	NI-80	28'-2"	26'-1"	24'-10"	N/A	28'-10"	26'-9"	25'-6"	N/A
	NI-90x	29'-0"	26'-10"	25'-7"	N/A	29'-7"	27'-5"	26'-2"	N/A

- Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 15 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/480 and a total load deflection limit of L/240.
- Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 5/8 inch for a joist spacing of 19.2 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.
- Minimum bearing length shall be 1-3/4 inches for the end bearings.
- Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.
- This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA O86-09, NBC 2010, and OBC 2012.
- Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.



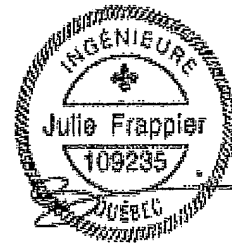
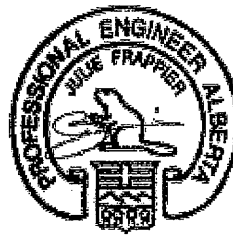
## Maximum Floor Spans

Live Load = 40 psf, Dead Load = 15 psf  
Simple Spans, L/480 Deflection Limit  
3/4" OSB G&N Sheathing

Depth	Series	Bare				1/2" Gypsum Ceiling			
		On Centre Spacing				On Centre Spacing			
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
9-1/2"	NI-20	15'-10"	15'-0"	14'-5"	13'-5"	16'-4"	15'-5"	14'-6"	13'-5"
	NI-40x	17'-0"	16'-0"	15'-5"	14'-9"	17'-5"	16'-5"	15'-10"	15'-2"
	NI-60	17'-2"	16'-2"	15'-7"	14'-11"	17'-6"	16'-7"	15'-11"	15'-3"
	NI-70	18'-0"	16'-11"	16'-3"	15'-7"	18'-5"	17'-3"	16'-7"	15'-11"
	NI-80	18'-3"	17'-1"	16'-5"	15'-9"	18'-8"	17'-5"	16'-9"	16'-1"
11-7/8"	NI-20	17'-10"	16'-10"	16'-2"	15'-6"	18'-6"	17'-4"	16'-9"	16'-1"
	NI-40x	19'-4"	17'-11"	17'-3"	16'-6"	19'-11"	18'-6"	17'-9"	17'-0"
	NI-60	19'-7"	18'-2"	17'-5"	16'-9"	20'-2"	18'-9"	17'-11"	17'-2"
	NI-70	20'-9"	19'-2"	18'-3"	17'-5"	21'-4"	19'-9"	18'-10"	17'-10"
	NI-80	21'-1"	19'-5"	18'-6"	17'-7"	21'-7"	20'-0"	19'-0"	18'-0"
	NI-90x	21'-8"	20'-0"	19'-1"	18'-0"	22'-2"	20'-6"	19'-6"	18'-6"
14"	NI-40x	21'-5"	19'-10"	18'-11"	17'-11"	22'-1"	20'-6"	19'-7"	18'-7"
	NI-60	21'-10"	20'-2"	19'-3"	18'-2"	22'-5"	20'-10"	19'-11"	18'-10"
	NI-70	23'-0"	21'-3"	20'-3"	19'-2"	23'-8"	21'-11"	20'-10"	19'-9"
	NI-80	23'-5"	21'-7"	20'-7"	19'-5"	24'-0"	22'-3"	21'-2"	20'-0"
	NI-90x	24'-1"	22'-3"	21'-2"	20'-0"	24'-8"	22'-10"	21'-9"	20'-7"
16"	NI-60	23'-9"	22'-0"	20'-11"	19'-10"	24'-6"	22'-9"	21'-8"	20'-6"
	NI-70	25'-1"	23'-2"	22'-0"	20'-10"	25'-9"	23'-10"	22'-9"	21'-6"
	NI-80	25'-6"	23'-6"	22'-4"	21'-2"	26'-1"	24'-2"	23'-1"	21'-10"
	NI-90x	26'-4"	24'-3"	23'-1"	21'-10"	26'-11"	24'-11"	23'-8"	22'-5"

Depth	Series	Mid-Span Blocking				Mid-Span Blocking and 1/2" Gypsum Ceiling			
		On Centre Spacing				On Centre Spacing			
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
9-1/2"	NI-20	16'-10"	15'-5"	14'-6"	13'-5"	16'-10"	15'-5"	14'-6"	13'-5"
	NI-40x	18'-8"	17'-2"	16'-3"	15'-2"	18'-10"	17'-2"	16'-3"	15'-2"
	NI-60	18'-11"	17'-6"	16'-6"	15'-5"	19'-2"	17'-6"	16'-6"	15'-5"
	NI-70	20'-0"	18'-7"	17'-9"	16'-7"	20'-5"	18'-11"	17'-10"	16'-7"
	NI-80	20'-3"	18'-10"	17'-11"	16'-10"	20'-8"	19'-3"	18'-2"	16'-10"
11-7/8"	NI-20	20'-1"	18'-5"	17'-5"	16'-2"	20'-1"	18'-5"	17'-5"	16'-2"
	NI-40x	21'-10"	20'-4"	19'-4"	17'-8"	22'-5"	20'-6"	19'-4"	17'-8"
	NI-60	22'-1"	20'-7"	19'-7"	18'-4"	22'-8"	20'-10"	19'-8"	18'-4"
	NI-70	23'-4"	21'-8"	20'-8"	19'-7"	23'-10"	22'-3"	21'-2"	19'-9"
	NI-80	23'-7"	21'-11"	20'-11"	19'-9"	24'-1"	22'-6"	21'-5"	20'-0"
	NI-90x	24'-3"	22'-6"	21'-6"	20'-4"	24'-8"	23'-0"	22'-0"	20'-9"
14"	NI-40x	24'-5"	22'-9"	21'-8"	19'-5"	25'-1"	23'-2"	21'-9"	19'-5"
	NI-60	24'-10"	23'-1"	22'-0"	20'-10"	25'-6"	23'-8"	22'-4"	20'-10"
	NI-70	26'-1"	24'-3"	23'-2"	21'-10"	26'-8"	24'-11"	23'-9"	22'-4"
	NI-80	26'-6"	24'-7"	23'-5"	22'-2"	27'-1"	25'-3"	24'-1"	22'-9"
	NI-90x	27'-3"	25'-4"	24'-1"	22'-9"	27'-9"	25'-11"	24'-8"	23'-4"
16"	NI-60	27'-3"	25'-5"	24'-2"	22'-10"	28'-0"	26'-2"	24'-9"	23'-1"
	NI-70	28'-8"	26'-8"	25'-4"	23'-11"	29'-3"	27'-4"	26'-1"	24'-8"
	NI-80	29'-1"	27'-0"	25'-9"	24'-4"	29'-8"	27'-9"	26'-5"	25'-0"
	NI-90x	29'-11"	27'-10"	26'-6"	25'-0"	30'-6"	28'-5"	27'-2"	25'-8"

- Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 15 psf. The ultimate limit states are based on the factored loads of  $1.50L + 1.25D$ . The serviceability limit states include the consideration for floor vibration, a live load deflection limit of  $L/480$  and a total load deflection limit of  $L/240$ .
- Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 3/4 inch for a joist spacing of 24 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.
- Minimum bearing length shall be 1-3/4 inches for the end bearings.
- Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.
- This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA O86-09, NBC 2010, and OBC 2012.
- Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.

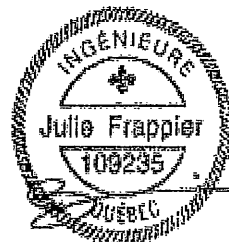
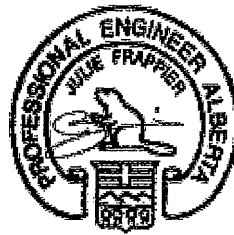


## Maximum Floor Spans

Live Load = 40 psf, Dead Load = 30 psf  
Simple Spans, L/480 Deflection Limit  
5/8" OSB G&N Sheathing

Depth	Series	Bare				1/2" Gypsum Ceiling			
		On Centre Spacing				On Centre Spacing			
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
9-1/2"	NI-20	15'-1"	14'-1"	13'-3"	N/A	15'-7"	14'-1"	13'-3"	N/A
	NI-40x	16'-1"	15'-2"	14'-8"	N/A	16'-7"	15'-7"	15'-1"	N/A
	NI-60	16'-3"	15'-4"	14'-10"	N/A	16'-8"	15'-9"	15'-3"	N/A
	NI-70	17'-1"	16'-1"	15'-6"	N/A	17'-5"	16'-5"	15'-10"	N/A
	NI-80	17'-3"	16'-3"	15'-8"	N/A	17'-8"	16'-7"	16'-0"	N/A
11-7/8"	NI-20	16'-11"	16'-0"	15'-5"	N/A	17'-6"	16'-6"	16'-0"	N/A
	NI-40x	18'-1"	17'-0"	16'-5"	N/A	18'-9"	17'-6"	16'-11"	N/A
	NI-60	18'-4"	17'-3"	16'-7"	N/A	19'-0"	17'-8"	17'-1"	N/A
	NI-70	19'-6"	18'-0"	17'-4"	N/A	20'-1"	18'-7"	17'-9"	N/A
	NI-80	19'-9"	18'-3"	17'-6"	N/A	20'-4"	18'-10"	17'-11"	N/A
14"	NI-90x	20'-4"	18'-9"	17'-11"	N/A	20'-10"	19'-3"	18'-5"	N/A
	NI-40x	20'-1"	18'-7"	17'-10"	N/A	20'-10"	19'-4"	18'-6"	N/A
	NI-60	20'-5"	18'-11"	18'-1"	N/A	21'-2"	19'-7"	18'-9"	N/A
	NI-70	21'-7"	20'-0"	19'-1"	N/A	22'-3"	20'-7"	19'-8"	N/A
	NI-80	21'-11"	20'-3"	19'-4"	N/A	22'-7"	20'-11"	20'-0"	N/A
16"	NI-90x	22'-7"	20'-11"	19'-11"	N/A	23'-3"	21'-6"	20'-6"	N/A
	NI-60	22'-3"	20'-8"	19'-9"	N/A	23'-1"	21'-5"	20'-6"	N/A
	NI-70	23'-6"	21'-9"	20'-9"	N/A	24'-3"	22'-5"	21'-5"	N/A
	NI-80	23'-11"	22'-1"	21'-1"	N/A	24'-8"	22'-10"	21'-9"	N/A
	NI-90x	24'-8"	22'-9"	21'-9"	N/A	25'-4"	23'-5"	22'-4"	N/A
Depth	Series	Mid-Span Blocking				Mid-Span Blocking and 1/2" Gypsum Ceiling			
		On Centre Spacing				On Centre Spacing			
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
9-1/2"	NI-20	15'-7"	14'-1"	13'-3"	N/A	15'-7"	14'-1"	13'-3"	N/A
	NI-40x	17'-9"	16'-1"	15'-1"	N/A	17'-9"	16'-1"	15'-1"	N/A
	NI-60	18'-1"	16'-4"	15'-4"	N/A	18'-1"	16'-4"	15'-4"	N/A
	NI-70	19'-2"	17'-10"	16'-9"	N/A	19'-7"	17'-10"	16'-9"	N/A
	NI-80	19'-5"	18'-0"	17'-1"	N/A	19'-10"	18'-3"	17'-1"	N/A
11-7/8"	NI-20	18'-9"	17'-0"	16'-0"	N/A	18'-9"	17'-0"	16'-0"	N/A
	NI-40x	21'-0"	19'-3"	17'-9"	N/A	21'-3"	19'-3"	17'-9"	N/A
	NI-60	21'-4"	19'-8"	18'-5"	N/A	21'-8"	19'-8"	18'-5"	N/A
	NI-70	22'-6"	20'-10"	19'-11"	N/A	23'-0"	21'-4"	20'-0"	N/A
	NI-80	22'-9"	21'-1"	20'-1"	N/A	23'-3"	21'-7"	20'-5"	N/A
14"	NI-90x	23'-4"	21'-8"	20'-8"	N/A	23'-10"	22'-2"	21'-2"	N/A
	NI-40x	23'-7"	21'-5"	19'-6"	N/A	24'-1"	21'-5"	19'-6"	N/A
	NI-60	24'-0"	22'-3"	21'-0"	N/A	24'-8"	22'-5"	21'-0"	N/A
	NI-70	25'-3"	23'-4"	22'-3"	N/A	25'-10"	24'-0"	22'-9"	N/A
	NI-80	25'-7"	23'-8"	22'-7"	N/A	26'-2"	24'-4"	23'-2"	N/A
16"	NI-90x	26'-4"	24'-4"	23'-3"	N/A	26'-10"	24'-11"	23'-9"	N/A
	NI-60	26'-5"	24'-6"	23'-4"	N/A	27'-2"	24'-10"	23'-4"	N/A
	NI-70	27'-9"	25'-8"	24'-6"	N/A	28'-5"	26'-5"	25'-2"	N/A
	NI-80	28'-2"	26'-1"	24'-10"	N/A	28'-10"	26'-9"	25'-6"	N/A
	NI-90x	29'-0"	26'-10"	25'-7"	N/A	29'-7"	27'-5"	26'-2"	N/A

- Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 30 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/480 and a total load deflection limit of L/240.
- Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 5/8 inch for a joist spacing of 19.2 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.
- Minimum bearing length shall be 1-3/4 inches for the end bearings.
- Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.
- This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA O86-09, NBC 2010, and OBC 2012.
- Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.



## Maximum Floor Spans

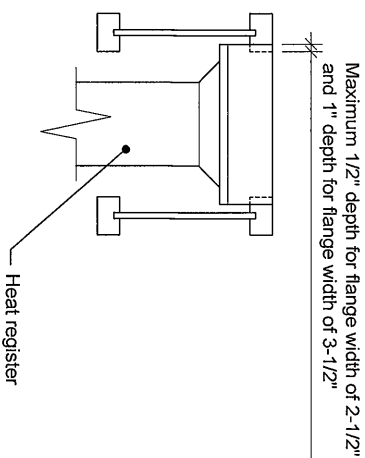
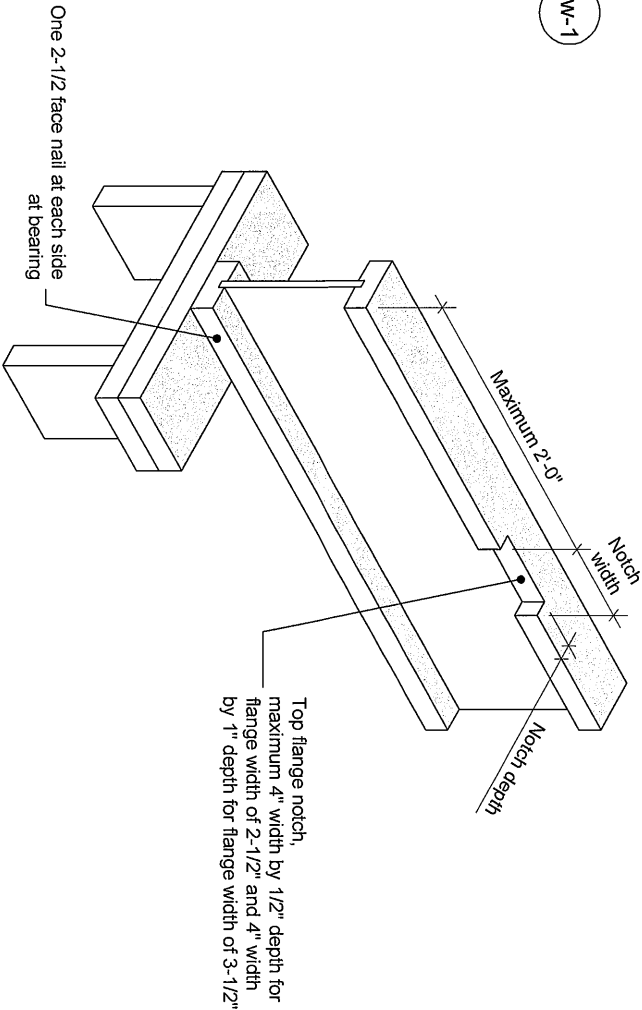
Live Load = 40 psf, Dead Load = 30 psf  
Simple Spans, L/480 Deflection Limit  
3/4" OSB G&N Sheathing

Depth	Series	Bare				1/2" Gypsum Ceiling			
		On Centre Spacing				On Centre Spacing			
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
9-1/2"	NI-20	15'-7"	14'-2"	13'-4"	12'-4"	15'-7"	14'-2"	13'-4"	12'-4"
	NI-40x	17'-0"	16'-0"	15'-1"	13'-11"	17'-5"	16'-1"	15'-1"	13'-11"
	NI-60	17'-2"	16'-2"	15'-5"	14'-3"	17'-6"	16'-5"	15'-5"	14'-3"
	NI-70	18'-0"	16'-11"	16'-3"	15'-6"	18'-5"	17'-3"	16'-7"	15'-6"
	NI-80	18'-3"	17'-1"	16'-5"	15'-9"	18'-8"	17'-5"	16'-9"	15'-10"
11-7/8"	NI-20	17'-10"	16'-10"	16'-0"	14'-10"	18'-6"	17'-1"	16'-0"	14'-10"
	NI-40x	19'-4"	17'-11"	17'-3"	15'-10"	19'-11"	18'-6"	17'-9"	15'-10"
	NI-60	19'-7"	18'-2"	17'-5"	16'-9"	20'-2"	18'-9"	17'-11"	17'-1"
	NI-70	20'-9"	19'-2"	18'-3"	17'-5"	21'-4"	19'-9"	18'-10"	17'-10"
	NI-80	21'-1"	19'-5"	18'-6"	17'-7"	21'-7"	20'-0"	19'-0"	18'-0"
14"	NI-90x	21'-8"	20'-0"	19'-1"	18'-0"	22'-2"	20'-6"	19'-6"	18'-6"
	NI-40x	21'-5"	19'-10"	18'-11"	17'-5"	22'-1"	20'-6"	19'-6"	17'-5"
	NI-60	21'-10"	20'-2"	19'-3"	18'-2"	22'-5"	20'-10"	19'-11"	18'-10"
	NI-70	23'-0"	21'-3"	20'-3"	19'-2"	23'-8"	21'-11"	20'-10"	19'-9"
	NI-80	23'-5"	21'-7"	20'-7"	19'-5"	24'-0"	22'-3"	21'-2"	20'-0"
16"	NI-90x	24'-1"	22'-3"	21'-2"	20'-0"	24'-8"	22'-10"	21'-9"	20'-7"
	NI-60	23'-9"	22'-0"	20'-11"	19'-10"	24'-6"	22'-9"	21'-8"	20'-6"
	NI-70	25'-1"	23'-2"	22'-0"	20'-10"	25'-9"	23'-10"	22'-9"	21'-6"
	NI-80	25'-6"	23'-6"	22'-4"	21'-2"	26'-1"	24'-2"	23'-1"	21'-10"
	NI-90x	26'-4"	24'-3"	23'-1"	21'-10"	26'-11"	24'-11"	23'-8"	22'-5"

Depth	Series	Mid-Span Blocking				Mid-Span Blocking and 1/2" Gypsum Ceiling			
		On Centre Spacing				On Centre Spacing			
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
9-1/2"	NI-20	15'-7"	14'-2"	13'-4"	12'-4"	15'-7"	14'-2"	13'-4"	12'-4"
	NI-40x	17'-9"	16'-1"	15'-1"	13'-11"	17'-9"	16'-1"	15'-1"	13'-11"
	NI-60	18'-1"	16'-5"	15'-5"	14'-3"	18'-1"	16'-5"	15'-5"	14'-3"
	NI-70	19'-10"	17'-11"	16'-9"	15'-6"	19'-10"	17'-11"	16'-9"	15'-6"
	NI-80	20'-2"	18'-3"	17'-1"	15'-10"	20'-2"	18'-3"	17'-1"	15'-10"
11-7/8"	NI-20	18'-10"	17'-1"	16'-0"	14'-10"	18'-10"	17'-1"	16'-0"	14'-10"
	NI-40x	21'-3"	19'-3"	17'-9"	15'-10"	21'-3"	19'-3"	17'-9"	15'-10"
	NI-60	21'-9"	19'-8"	18'-5"	17'-1"	21'-9"	19'-8"	18'-5"	17'-1"
	NI-70	23'-4"	21'-5"	20'-1"	18'-6"	23'-8"	21'-5"	20'-1"	18'-6"
	NI-80	23'-7"	21'-10"	20'-5"	18'-11"	24'-1"	21'-10"	20'-5"	18'-11"
14"	NI-90x	24'-3"	22'-6"	21'-3"	19'-7"	24'-8"	22'-7"	21'-3"	19'-7"
	NI-40x	24'-2"	21'-5"	19'-6"	17'-5"	24'-2"	21'-5"	19'-6"	17'-5"
	NI-60	24'-9"	22'-5"	21'-0"	19'-6"	24'-9"	22'-5"	21'-0"	19'-6"
	NI-70	26'-1"	24'-3"	22'-9"	21'-0"	26'-8"	24'-3"	22'-9"	21'-0"
	NI-80	26'-6"	24'-7"	23'-3"	21'-6"	27'-1"	24'-10"	23'-3"	21'-6"
16"	NI-90x	27'-3"	25'-4"	24'-1"	22'-4"	27'-9"	25'-10"	24'-3"	22'-4"
	NI-60	27'-3"	24'-11"	23'-5"	21'-7"	27'-6"	24'-11"	23'-5"	21'-7"
	NI-70	28'-8"	26'-8"	25'-3"	23'-4"	29'-3"	26'-11"	25'-3"	23'-4"
	NI-80	29'-1"	27'-0"	25'-9"	23'-10"	29'-8"	27'-6"	25'-10"	23'-10"
	NI-90x	29'-11"	27'-10"	26'-6"	24'-10"	30'-6"	28'-5"	26'-11"	24'-10"

- Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 30 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/480 and a total load deflection limit of L/240.
- Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 3/4 inch for a joist spacing of 24 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.
- Minimum bearing length shall be 1-3/4 inches for the end bearings.
- Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.
- This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA O86-09, NBC 2010, and OBC 2012.
- Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.

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- Notes:
1. Blocking required at bearing for lateral support, not shown for clarity.
  2. The maximum dimensions for a notch on the side of the top flange are 4-inch width by 1/2-inch depth for flange width of 2-1/2 inches, and 4-inch width by 1-inch depth for flange width of 3-1/2 inches.
  3. This detail applies to simple-span joists and multiple-span joists where the notch is located at the end half-span.
  4. For other applications, contact Nordic Structures.

This document supersedes all previous versions. If the document has been in effect for more than one year, consult nordic.ca or contact Nordic Structures.  
All nails shown in the details are assumed to be common nails unless otherwise noted. Nails shall have a diameter not less than 0.128 inch for 2-1/2-inch nails, or 0.144 inch for 3-inch nails. Individual components not shown to scale for clarity.

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TITLE  
Notch in I-joist for Heat Register

CATEGORY  
I-joist - Typical Floor Framing and Construction Details

DOCUMENT  
-

DATE  
2018-04-10

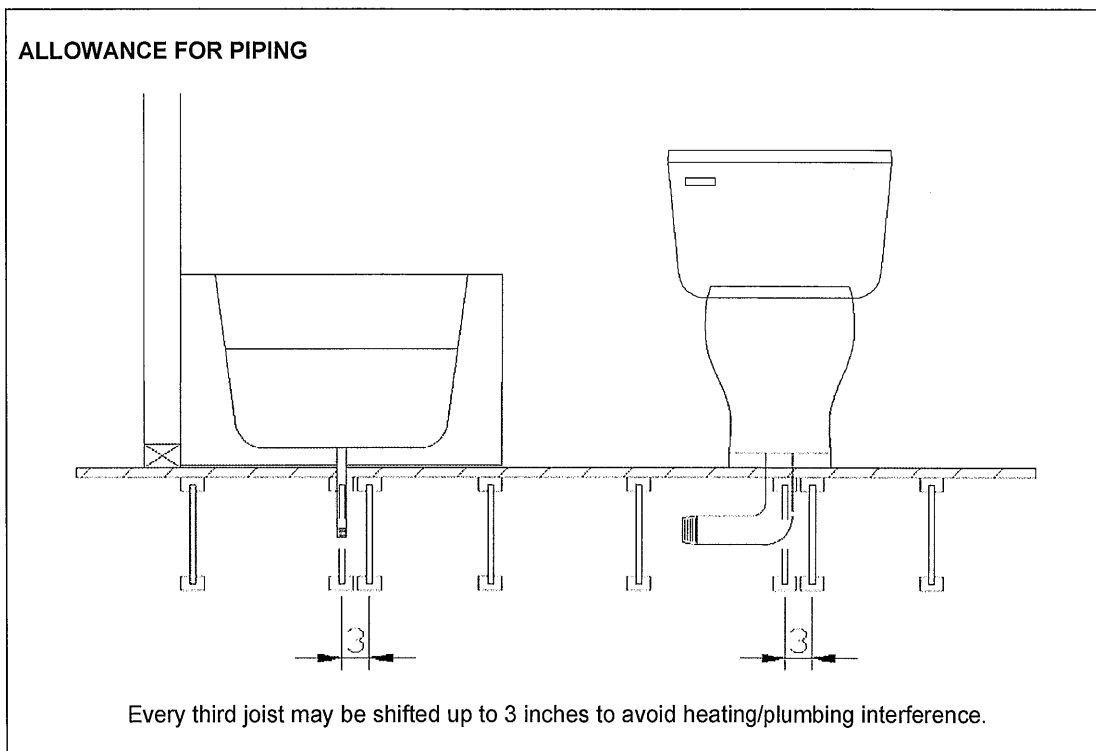
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1w-1

## Allowance for Piping (Installation Notes)

The floor layouts have usually not been checked for heating and/or plumbing interference. On-site adjustment of joists of up to 3 inches is permitted to avoid interferences. When moving a joist, the subfloor thickness shall be checked with code requirements when the joist spacing exceeds 19.2 inches. Except for cutting to length, I-joist flanges should never be cut, drilled, or notched.

Installation of Nordic I-joists shall be as per *Nordic Joist Installation Guide for Residential Floors*. Refer to Tables 1 and 2 for maximum web hole and duct chase openings, respectively. These tables are based on the I-joists being used at their maximum spans. The minimum distance given may be reduced for shorter spans; contact your distributor for additional information.

The detail below shows the 3-inch allowance for piping. Every third joist may be shifted up to 3 inches to avoid heating/plumbing interference. For other applications, please contact your distributor.



Revised April 12, 2012