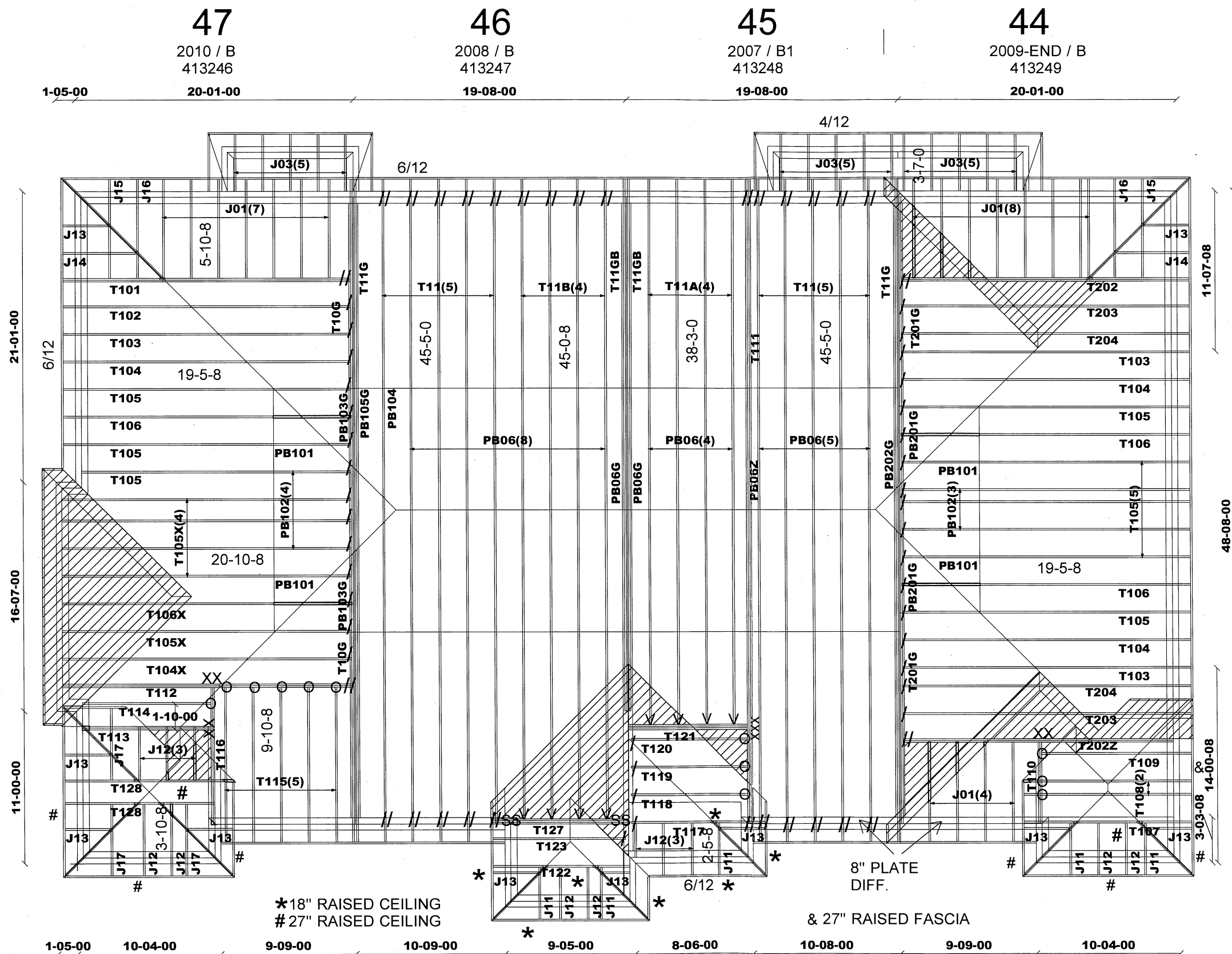




ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

DESIGN CONFORMS WITH OBC 2012
OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 31.4 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 25.6 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

 **DENOTES:**
CONVENTIONAL
FRAMING

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)
HGUS26-3 - (XXX)
H2.5T - (I)
HTS16 - (S)

All conventional framing to conform with Part 9 of O.B.C. 2012. Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.



Job Track: **51012**
Plan Log: **203571**
Layout ID: **413226**

Builder / Location:
ROYAL PINE HOMES / RICHMOND HILL
Project: **CENTREFIELD**
Date: 2021-05-28 Sales: **Mario DiCano** Designer: AC

Model / Elevation:
BLOCK 52 / UNITS 44 - 47

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

Mitek ver 8.4.2.286

DELIVERY SHIPLIST



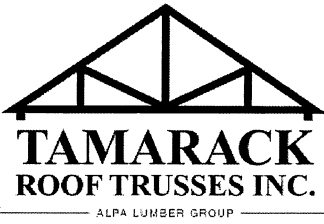
Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: CENTREFIELD
 Location: RICHMOND HILL
 Model: BLOCK 52
 Lot #:
 Elevation: B / UNIT47BLK290

Job Track: 51012
 PlanLog: 203571
 Layout ID: 413246
 Ref #
 Page: 1 of 3
 Date: 05-28-2021
 Designer: Andrew Conway
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	T10G GABLE	6 /12	22-08-08	8-01-04	2 x 4	1-05-00	1-02-00 8-01-04	243.75 153.67		
	1 2-ply	T101 Half Hip Girder	6 /12	19-05-08	4-01-04	2 x 4 2 x 6	1-03-08	1-02-00 4-01-04	182.72 116.00		
	1	T102 Half Hip	6 /12	19-05-08	5-01-04	2 x 4	1-03-08	1-02-00 5-01-04	79.56 50.67		
	1	T103 Half Hip	6 /12	19-05-08	6-01-04	2 x 4	1-03-08	1-02-00 6-01-04	86.12 54.83		
	1	T104 Half Hip	6 /12	19-05-08	7-01-04	2 x 4	1-03-08	1-02-00 7-01-04	86.52 54.00		
	1	T104X Half Hip	6 /12	19-05-08	7-01-04	2 x 4	1-03-08	1-02-00 7-01-04	88.21 54.67		
	3	T105 Half Hip	6 /12	19-05-08	8-01-04	2 x 4	1-03-08	1-02-00 8-01-04	258.51 163.00		
	5	T105X Piggyback Base	6 /12	19-05-08	8-01-04	2 x 4	1-03-08	1-02-00 8-01-04	439.32 275.00		
	1	T106 Half Hip	6 /12	19-05-08	9-01-04	2 x 4	1-03-08	1-02-00 9-01-04	90.51 57.00		
	1	T106X Half Hip	6 /12	19-05-08	9-01-04	2 x 4	1-03-08	1-02-00 9-01-04	92.67 58.17		
	1 2-ply	T112 Half Hip Girder	6 /12	20-10-08	6-01-04	2 x 4 2 x 6		5-08 6-01-04	215.22 127.67		
	1 2-ply	T113 Flat	0 /12	9-01-08	2-03-00	2 x 4		1-06-00 1-06-00	66.54 44.00		
	1	T114 Half Hip	6 /12	9-01-08	5-06-00	2 x 4		1-02-00 5-06-00	41.45 26.50		
	5	T115 Monopitch	6 /12	9-10-08	6-01-04	2 x 4	1-03-08	1-02-00 6-01-04	217.09 137.50		

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: CENTREFIELD
 Location: RICHMOND HILL
 Model: BLOCK 52
 Lot #:
 Elevation: B / UNIT47BLK290

Job Track: 51012
 PlanLog: 203571
 Layout ID: 413246
 Ref #
 Page: 2 of 3
 Date: 05-28-2021
 Designer: Andrew Conway
 Sales Rep: Mario DiCano

Roof Trusses

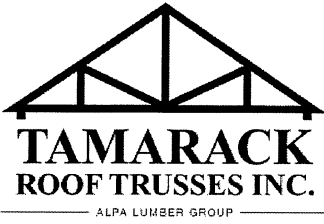
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T116 Flat Girder	0 /12	9-10-08	2-03-00	2 x 4 2 x 6		1-06-00 1-06-00	85.82 54.67		
	2	T128 Hip Girder	6 /12	9-06-00	3-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	80.58 51.67		
	2	PB101 Piggyback	6 /12	5-07-00	2-00-00	2 x 4		2-00-00	32.19 22.67		
	4	PB102 Piggyback	6 /12	5-07-00	2-09-08	2 x 4		2-09-08	58.98 40.00		
	2	PB103G GABLE	6 /12	8-10-00	2-09-08	2 x 4		2-09-08	53.06 37.33		
	7	J01 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	117.56 74.67		
	5	J03 Jack-Open	4 /12	3-07-00	1-11-03	2 x 4	1-03-08	3-15 1-03-04	50.28 33.33		
	5	J12 Jack-Open	6 /12	2-05-08	2-04-12	2 x 4	1-03-08	1-02-00 2-04-12	42.71 30.00		
	4	J13 Jack-Open	6 /12	1-10-15	2-01-08	2 x 4	1-03-08 1-01	1-02-00 2-01-08	29.29 18.67		
	1	J14 Jack-Open	6 /12	2-00-00	3-01-08	2 x 4	1-03-08 1-10-15	1-02-00 2-02-00	9.87 6.00		
	1	J15 Jack-Open	6 /12	1-10-15	2-01-08	2 x 4	1-03-08 3-11-09	1-02-00 2-01-08	11.74 7.33		
	1	J16 Jack-Open	6 /12	3-10-15	3-01-08	2 x 4	1-03-08 1-11-09	1-02-00 3-01-08	14.29 8.67		
	3	J17 Jack-Open	6 /12	1-10-15	2-01-08	2 x 4	1-03-08 1-11-09	1-02-00 2-01-08	28.38 18.00		

TOTAL # TRUSS= 67

TOTAL BFT OF ALL TRUSSES= 1775.69

BFT. TOTAL WEIGHT OF ALL TRSSES 2802.98 LBS

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: CENTREFIELD
 Location: RICHMOND HILL
 Model: BLOCK 52
 Lot #:
 Elevation: B / UNIT47BLK290

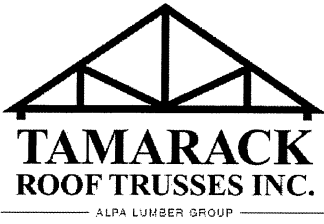
Job Track: 51012
 PlanLog: 203571
 Layout ID: 413246
 Ref #
 Page: 3 of 3
 Date: 05-28-2021
 Designer: Andrew Conway
 Sales Rep: Mario DiCano

HARDWARE

QTY	TYPE	MODEL	LENGTH
6	Hardware	LUS24	
2	Hardware	HGUS26-2	
18	Hardware	H2.5T	

TOTAL NUMBER OF ITEMS= **26**

DELIVERY SHIPLIST

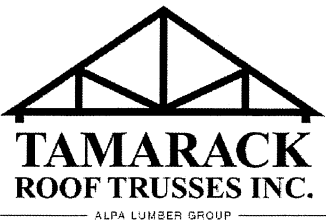


Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: CENTREFIELD
 Location: RICHMOND HILL
 Model: BLOCK 52
 Lot #:
 Elevation: B / UNIT46BLK290

Job Track: 51012
 PlanLog: 203571
 Layout ID: 413247
 Ref #
 Page: 1 of 2
 Date: 05-28-2021
 Designer: Andrew Conway
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	5	T11 Piggyback Base	6 /12	45-05-00	8-01-04	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	1496.73 900.00		
	4	T11B Piggyback Base	6 /12	45-00-08	8-01-04	2 x 6	1-03-08	1-02-00 1-02-00	1167.69 704.00		
	1	T11G GABLE	6 /12	45-05-00	8-01-04	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	272.06 171.67		
	1	T11GB GABLE	6 /12	37-03-00	8-01-04	2 x 6	1-03-08	1-02-00 5-03-00	236.81 149.83		
	1	T122 Hip Girder	6 /12	8-07-00	2-04-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	35.14 23.67		
	1	T123 Common	6 /12	8-03-08	3-03-12	2 x 4	1-03-08	1-02-00 1-03-12	32.19 20.83		
	1 3-ply	T127 Common Girder	6 /12	8-05-08	4-09-12	2 x 4 2 x 8	1-03-08	2-08-00 2-08-12	153.2 102.00		
	8	PB06 Piggyback	6 /12	17-08-00	4-05-00	2 x 4			410.9 256.00		
	1	PB06G GABLE	6 /12	17-08-00	4-05-00	2 x 4			50.76 31.00		
	1	PB104 Piggyback	6 /12	17-08-00	3-11-04	2 x 4			57.06 37.00		
	1	PB105G Piggyback	6 /12	8-10-00	2-08-00	2 x 4		2-09-08	26.21 18.67		
	2	J11 Jack-Open	6 /12	1-10-15	2-01-08	2 x 4	1-03-08 6-09	1-02-00 2-01-08	15.69 10.67		
	2	J12 Jack-Open	6 /12	2-05-08	2-04-12	2 x 4	1-03-08	1-02-00 2-04-12	17.09 12.00		
	2	J13 Jack-Open	6 /12	1-10-15	2-01-08	2 x 4	1-03-08 1-01	1-02-00 2-01-08	14.65 9.33		

 TAMARACK ROOF TRUSSES INC. ALPHA LUMBER GROUP	DELIVERY SHIPLIST	
	Lumber Yard: TAMARACK LUMBER Builder: ROYAL PINE HOMES Project: CENTREFIELD Location: RICHMOND HILL Model: BLOCK 52 Lot #: Elevation: B / UNIT46BLK290	Job Track: 51012 PlanLog: 203571 Layout ID: 413247 Ref # Page: 2 of 2 Date: 05-28-2021 Designer: Andrew Conway Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
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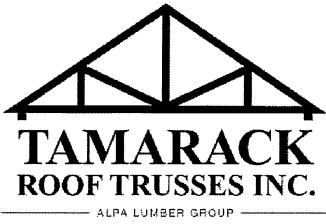
TOTAL # TRUSS= 33 TOTAL BFT OF ALL TRUSSES= 2446.67 BFT. TOTAL WEIGHT OF ALL TRSSES 3986.17 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
4	Hardware	LJS26DS	
29	Hardware	H2.5T	
4		HTS16	

TOTAL NUMBER OF ITEMS= 37

DELIVERY SHIPLIST

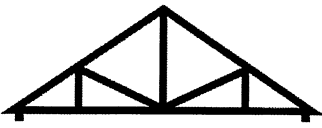


Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: CENTREFIELD
 Location: RICHMOND HILL
 Model: BLOCK 52
 Lot #:
 Elevation: B1 / UNIT45BLK290

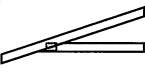
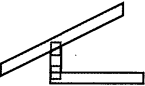
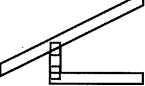
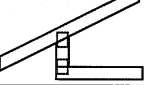
Job Track: 51012
 PlanLog: 203571
 Layout ID: 413248
 Ref #
 Page: 1 of 2
 Date: 05-28-2021
 Designer: Andrew Conway
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	5	T11 Piggyback Base	6 /12	45-05-00	8-01-04	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	1496.73 900.00		
	4	T11A Piggyback Base	6 /12	38-03-00	8-01-04	2 x 6	1-03-08	1-02-00 4-09-00	1018.89 624.00		
	1	T11G GABLE	6 /12	45-05-00	8-01-04	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	272.06 171.67		
	1	T11GB GABLE	6 /12	37-03-00	8-01-04	2 x 6	1-03-08	1-02-00 5-03-00	236.81 149.83		
	1 3-ply	T111 Piggyback Base Girder	6 /12	45-05-00	8-01-04	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	919.48 558.00		
	1	T117 Half Hip Girder	6 /12	8-04-08	2-04-12	2 x 4	1-03-08	1-02-00 2-04-12	34.08 22.83		
	1	T118 Half Hip	6 /12	8-04-08	4-10-12	2 x 4		2-08-00 4-10-12	39.12 26.00		
	1	T119 Half Hip	6 /12	8-04-08	5-10-12	2 x 4		2-08-00 5-10-12	45.3 29.67		
	1	T120 Monopitch	6 /12	8-04-08	6-10-04	2 x 4		2-08-00 6-10-04	41.6 27.33		
	1 3-ply	T121 Monopitch Girder	6 /12	8-06-08	6-11-04	2 x 4 2 x 6		2-08-00 6-11-04	141.44 94.00		
	9	PB06 Piggyback	6 /12	17-08-00	4-05-00	2 x 4			462.27 288.00		
	1	PB06G GABLE	6 /12	17-08-00	4-05-00	2 x 4			50.76 31.00		
	1 3-ply	PB06Z Piggyback	6 /12	17-08-00	4-05-00	2 x 4			154.09 96.00		
	1	PB202G GABLE	6 /12	17-08-00	3-07-04	2 x 4			50.36 31.83		

 TAMARACK ROOF TRUSSES INC. ALPA LUMBER GROUP	DELIVERY SHIPLIST				Lumber Yard: TAMARACK LUMBER Builder: ROYAL PINE HOMES Project: CENTREFIELD Location: RICHMOND HILL Model: BLOCK 52 Lot #: Elevation: B1 / UNIT45BLK290		Job Track: 51012 PlanLog: 203571 Layout ID: 413248 Ref # Page: 2 of 2 Date: 05-28-2021 Designer: Andrew Conway Sales Rep: Mario DiCano	
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Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	5	J03 Jack-Open	4 /12	3-07-00	1-11-03	2 x 4	1-03-08	3-15 1-03-04	50.28 33.33		
	1	J11 Jack-Open	6 /12	1-10-15	2-01-08	2 x 4	1-03-08 6-09	1-02-00 2-01-08	7.85 5.33		
	3	J12 Jack-Open	6 /12	2-05-08	2-04-12	2 x 4	1-03-08	1-02-00 2-04-12	25.63 18.00		
	1	J13 Jack-Open	6 /12	1-10-15	2-01-08	2 x 4	1-03-08 1-01	1-02-00 2-01-08	7.32 4.67		

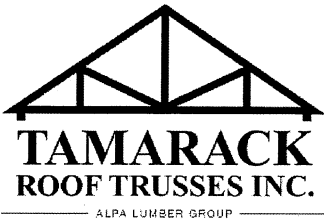
TOTAL # TRUSS= 45 TOTAL BFT OF ALL TRUSSES= 3111.49 BFT. TOTAL WEIGHT OF ALL TRSSES 5054.04 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
3	Hardware	LUS24	
4	Hardware	LJS26DS	
1	Hardware	HGUS26-3	
28	Hardware	H2.5T	

TOTAL NUMBER OF ITEMS= 36

DELIVERY SHIPLIST

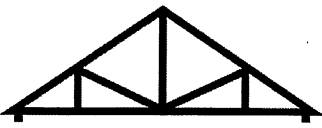


Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: CENTREFIELD
 Location: RICHMOND HILL
 Model: BLOCK 52
 Lot #:
 Elevation: B / UNIT44BLK290

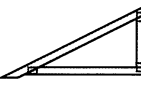
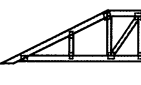
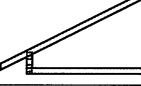
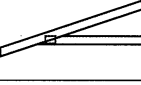
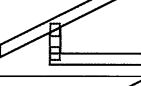
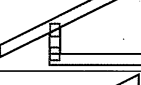
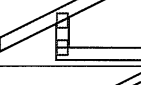
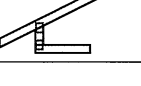
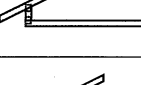
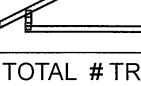
Job Track: 51012
 PlanLog: 203571
 Layout ID: 413249
 Ref #
 Page: 1 of 2
 Date: 05-28-2021
 Designer: Andrew Conway
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	T103 Half Hip	6 /12	19-05-08	6-01-04	2 x 4	1-03-08	1-02-00 6-01-04	172.24 109.67		
	2	T104 Half Hip	6 /12	19-05-08	7-01-04	2 x 4	1-03-08	1-02-00 7-01-04	173.04 108.00		
	7	T105 Half Hip	6 /12	19-05-08	8-01-04	2 x 4	1-03-08	1-02-00 8-01-04	603.2 380.33		
	2	T106 Half Hip	6 /12	19-05-08	9-01-04	2 x 4	1-03-08	1-02-00 9-01-04	181.02 114.00		
	1	T107 Hip Girder	6 /12	9-06-00	2-04-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	37.9 25.50		
	2	T108 Hip	6 /12	9-06-00	4-10-12	2 x 4	1-03-08 1-03-08	2-08-00 2-08-00	99.25 67.00		
	1	T109 Hip	6 /12	9-06-00	3-10-12	2 x 4	1-03-08 1-03-08	2-08-00 2-08-00	45.71 31.67		
	1 2-ply	T110 Monopitch Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	58.39 37.67		
	2	T201G GABLE	6 /12	22-08-08	8-01-04	2 x 4		6-00 8-01-04	226.67 143.00		
	1 2-ply	T202 Hip Girder	6 /12	19-05-08	4-01-04	2 x 4 2 x 6	1-03-08	1-02-00 1-00-12	182.93 116.00		
	1 2-ply	T202Z Hip Girder	6 /12	19-05-08	4-01-04	2 x 4 2 x 6	1-03-08	1-02-00 1-00-12	182.93 116.00		
	2	T203 Hip	6 /12	19-05-08	5-01-04	2 x 4	1-03-08	1-02-00 1-00-12	159.05 102.67		
	2	T204 Common	6 /12	19-05-08	5-11-12	2 x 4	1-03-08	1-02-00 1-00-12	150.1 96.67		
	2	PB101 Piggyback	6 /12	5-07-00	2-00-00	2 x 4		2-00-00	32.19 22.67		

 TAMARACK ROOF TRUSSES INC. — ALPA LUMBER GROUP —	DELIVERY SHIPLIST				Lumber Yard: TAMARACK LUMBER Builder: ROYAL PINE HOMES Project: CENTREFIELD Location: RICHMOND HILL Model: BLOCK 52 Lot #: Elevation: B / UNIT44BLK290		Job Track: 51012 PlanLog: 203571 Layout ID: 413249 Ref # Page: 2 of 2 Date: 05-28-2021 Designer: Andrew Conway Sales Rep: Mario DiCano	
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Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	3	PB102 Piggyback	6 / 12	5-07-00	2-09-08	2 x 4		2-09-08	44.24 30.00		
	2	PB201G Piggyback	6 / 12	7-06-00	2-09-08	2 x 4		2-09-08	47.69 33.33		
	12	J01 Jack-Open	6 / 12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	201.53 128.00		
	5	J03 Jack-Open	4 / 12	3-07-00	1-11-03	2 x 4	1-03-08	3-15 1-03-04	50.28 33.33		
	2	J11 Jack-Open	6 / 12	1-10-15	2-01-08	2 x 4	1-03-08 6-09	1-02-00 2-01-08	15.69 10.67		
	2	J12 Jack-Open	6 / 12	2-05-08	2-04-12	2 x 4	1-03-08	1-02-00 2-04-12	17.09 12.00		
	3	J13 Jack-Open	6 / 12	1-10-15	2-01-08	2 x 4	1-03-08 1-01	1-02-00 2-01-08	21.97 14.00		
	1	J14 Jack-Open	6 / 12	2-00-00	3-01-08	2 x 4	1-03-08 1-10-15	1-02-00 2-02-00	9.87 6.00		
	1	J15 Jack-Open	6 / 12	1-10-15	2-01-08	2 x 4	1-03-08 3-11-09	1-02-00 2-01-08	11.74 7.33		
	1	J16 Jack-Open	6 / 12	3-10-15	3-01-08	2 x 4	1-03-08 1-11-09	1-02-00 3-01-08	14.29 8.67		

TOTAL # TRUSS= 63 TOTAL BFT OF ALL TRUSSES= 1754.18 BFT. TOTAL WEIGHT OF ALL TRSSES 2739.01 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
3	Hardware	LUS24	
1	Hardware	HGUS26-2	
21	Hardware	H2.5T	

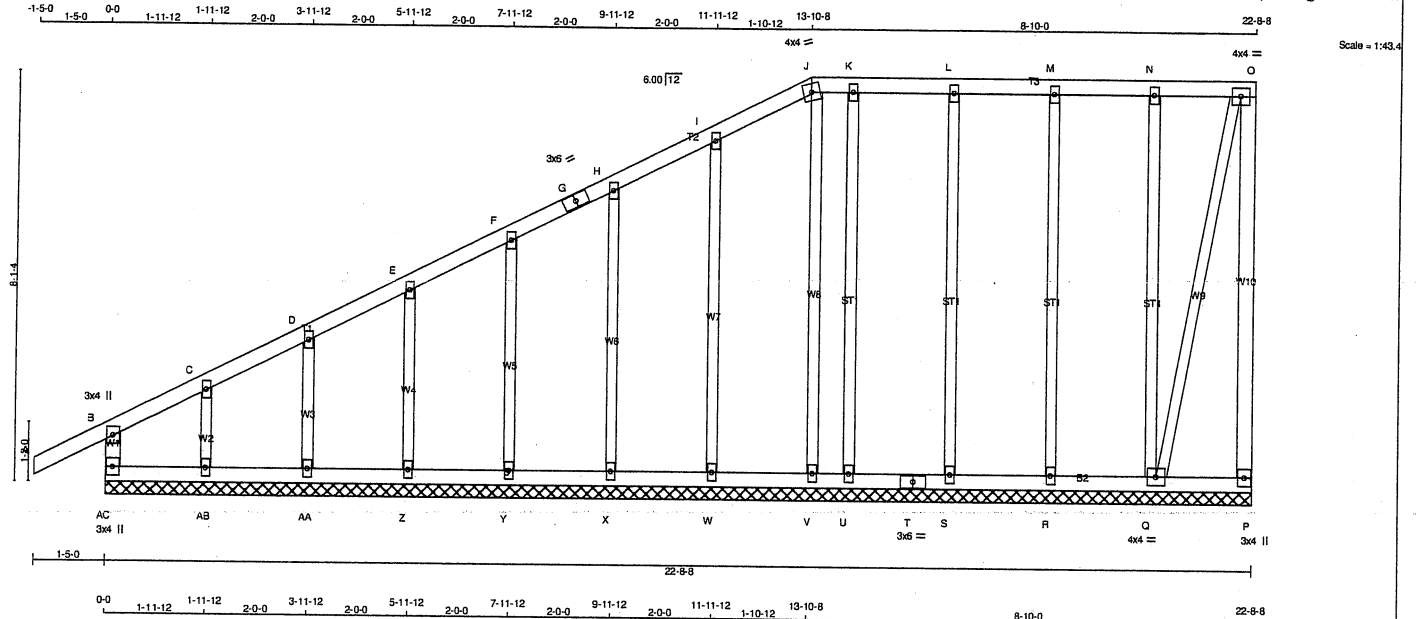
TOTAL NUMBER OF ITEMS= 25

JOB NAME 413226	TRUSS NAME T10G	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 122 = 244 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
AC - B	2x4	DRY	No.2
A - G	2x4	DRY	No.2
G - J	2x4	DRY	No.2
J - O	2x4	DRY	No.2
P - O	2x4	DRY	No.2
AC - T	2x4	DRY	No.2
T - P	2x4	DRY	No.2

ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

GABLE STUDS SPACED AT 2'-0" OC.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (CSI (LC))	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	LC1 MAX (CSI (LC))
FR-TO		FROM TO		FR-TO			
AC-B	-271 / 0	0.0	0.03 (1)	Q-N	-200 / 0	7.81	0.25 (1)
A-B	0 / 30	-91.8	-91.8 0.14 (1)	R-M	-182 / 0	10.00	0.23 (1)
B-C	-21 / 0	-91.8	-91.8 0.14 (1)	S-L	-190 / 0	6.25	0.24 (1)
C-D	0 / 10	-91.8	-91.8 0.05 (1)	U-K	-149 / 0	10.00	0.19 (1)
D-E	0 / 9	-91.8	-91.8 0.05 (1)	V-J	-95 / 0	10.00	0.12 (1)
E-F	0 / 14	-91.8	-91.8 0.04 (1)	AB-C	-118 / 0	10.00	0.02 (1)
F-G	0 / 16	-91.8	-91.8 0.04 (1)	AA-D	-196 / 0	10.00	0.04 (1)
G-H	0 / 16	-91.8	-91.8 0.04 (1)	Z-E	-179 / 0	10.00	0.05 (1)
H-I	0 / 19	-91.8	-91.8 0.05 (1)	Y-F	-183 / 0	10.00	0.07 (1)
I-J	0 / 13	-91.8	-91.8 0.05 (1)	X-H	-180 / 0	10.00	0.11 (1)
J-K	0 / 20	-91.8	-91.8 0.03 (1)	W-I	-200 / 0	10.00	0.18 (1)
K-L	0 / 20	-91.8	-91.8 0.05 (1)	Q-O	-79 / 0	10.00	0.11 (1)
L-M	0 / 20	-91.8	-91.8 0.05 (1)				
M-N	0 / 20	-91.8	-91.8 0.05 (1)				
N-O	0 / 20	-91.8	-91.8 0.05 (1)				
P-O	0 / 0	0.0	0.00 (1)				
AC-AB	0 / 0	-18.5	-18.5 0.02 (4)				
AB-AA	-4 / 0	-18.5	-18.5 0.02 (4)				
AA-Z	-9 / 0	-18.5	-18.5 0.01 (4)				
Z-Y	-12 / 0	-18.5	-18.5 0.01 (4)				
Y-X	-14 / 0	-18.5	-18.5 0.01 (4)				
X-W	-16 / 0	-18.5	-18.5 0.02 (4)				
W-V	-18 / 0	-18.5	-18.5 0.02 (4)				
V-U	-20 / 0	-18.5	-18.5 0.01 (4)				
U-T	-20 / 0	-18.5	-18.5 0.02 (4)				
T-S	-20 / 0	-18.5	-18.5 0.02 (4)				
S-R	-20 / 0	-18.5	-18.5 0.02 (4)				
R-Q	-20 / 0	-18.5	-18.5 0.02 (4)				
Q-P	0 / 0	-18.5	-18.5 0.02 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6	PSF
DL	= 6.0	PSF
BOT CH. LL	= 0.0	PSF
DL	= 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.
(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (A-B:1), BC=0.02/1.00 (Q-R:4), WB=0.25/1.00 (N-Q:1), SS=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.59 (J) (INPUT = 0.90)
JSI METAL= 0.08 (I) (INPUT = 1.00)

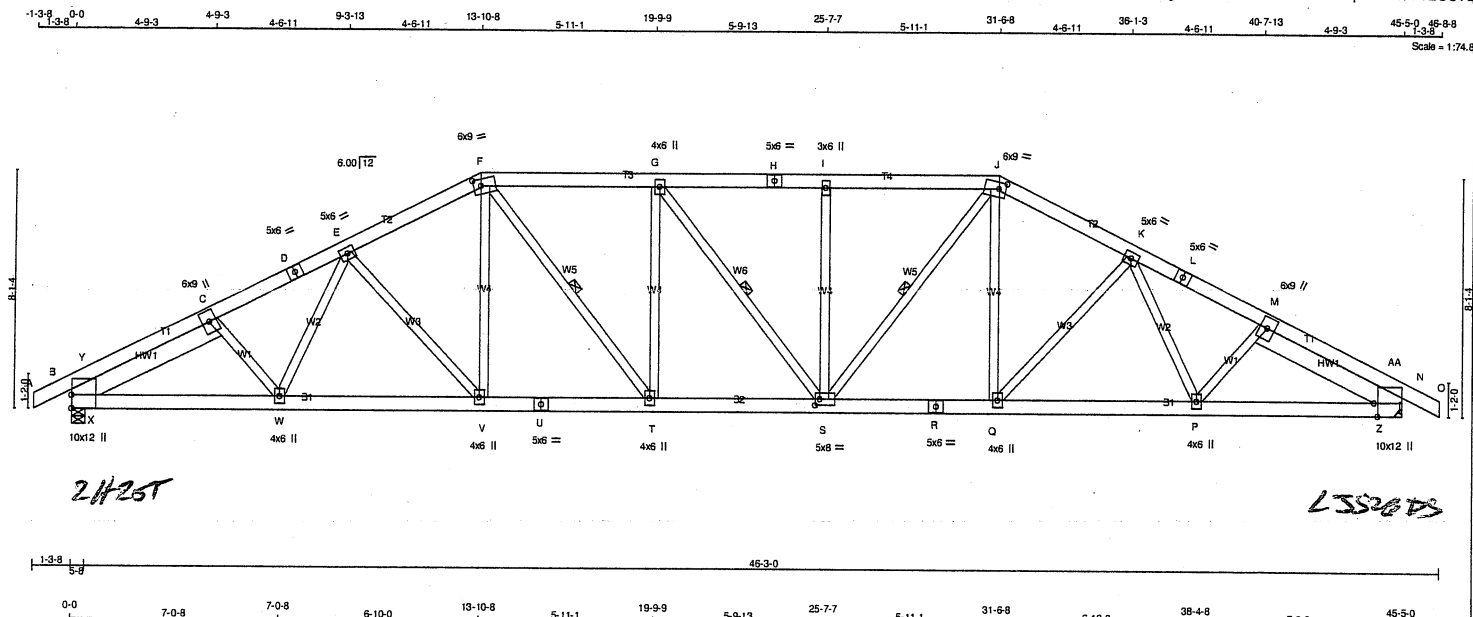


Structural component only
DWG# T-2115849

JOB NAME 413226	TRUSS NAME T11	QUANTITY 10	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x6	DRY	No.2
D - F	2x6	DRY	No.2
F - H	2x6	DRY	No.2
H - J	2x6	DRY	No.2
J - L	2x6	DRY	No.2
L - O	2x6	DRY	No.2
B - U	2x6	DRY	No.2
U - R	2x6	DRY	No.2
R - N	2x6	DRY	No.2
REINFORCING MEMBERS			
HW1	2x8	DRY	No.2
HW2	2x8	DRY	No.2
ALL WEBS 2x4 DRY SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMBMW1+I	MT20	10.0	12.0	5.50
C	TMWW+t	MT20	6.0	9.0	
D, H, L					
D	TS-t	MT20	5.0	6.0	
E	TMWW-t	MT20	5.0	6.0	
F	TTWW-m	MT20	6.0	9.0	2.75 3.00
G	TMWW-t	MT20	4.0	6.0	
I	TMWW-w	MT20	3.0	6.0	
J	TTWW-m	MT20	6.0	9.0	2.75 3.00
K	TMWW-t	MT20	5.0	6.0	
M	TMWW-t	MT20	6.0	9.0	
N	TMBMW1+I	MT20	10.0	12.0	Edge 1.50
P, Q, T, V, W					
P	BMWW-t	MT20	4.0	6.0	
R	BS-t	MT20	5.0	6.0	
S	BMWW-t	MT20	5.0	8.0	2.50 2.00
U	BS-t	MT20	5.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED			MAXIMUM FACTORED		INPUT	REQRD
JT	GROSS REACTION		GROSS REACTION		BRG	BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX
B	3675	0	3709	195	-1082	5-8
N	3675	0	3709	0	-1082	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT N. MINIMUM BEARING LENGTH AT JOINT N = 3-8.

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 1082 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT N FOR 1082 LBS. FACTORED UPLIFT

PROVIDE FOR 195 LBS. FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	2713	1634 / 0	477 / 0	0 / 0	0 / -1073	625 / 0	0 / 0
N	2713	1634 / 0	477 / 0	0 / 0	59 / -1073	625 / 0	0 / 0

HORIZONTAL REACTIONS		WIND		DEAD		SOIL	
B	0 / 0	0 / 0	0 / 0	139 / -139	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 3.53 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-T, G-S, J-S.

LOADING

TOTAL LOAD CASES: (18)

CHORDS				WEBS			
MEMB.	MAX. FACTORED	FACTORED	MAX. FACTORED	MEMB.	MAX. FACTORED	FACTORED	MAX. FACTORED
	FORCE (LBS)	VERT. LOAD LC1 (PLF)	CS1 (LC)		FORCE (LBS)	CS1 (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	-0 / 1	-115.2	-115.2	0.09 (2)	10.00	C-W	-30 / 380
B-Y	-4270 / 1161	-115.2	-115.2	0.18 (2)	4.08	W-E	-108 / 274
Y-C	-2819 / 902	-115.2	-115.2	0.15 (2)	4.88	E-V	-825 / 384
C-D	-5532 / 1713	-115.2	-115.2	0.33 (2)	3.53	V-F	-234 / 885
D-E	-5532 / 1713	-115.2	-115.2	0.33 (2)	3.53	F-T	-482 / 1515
E-F	-4993 / 1604	-115.2	-115.2	0.29 (2)	3.73	T-G	-977 / 432
F-G	-5190 / 1688	-115.2	-115.2	0.41 (1)	3.53	G-S	-326 / 310
G-H	-5185 / 1686	-115.2	-115.2	0.41 (1)	3.53	S-I	-721 / 361
H-I	-5185 / 1686	-115.2	-115.2	0.41 (1)	3.53	S-J	-477 / 1506
I-J	-5185 / 1685	-115.2	-115.2	0.41 (1)	3.55	J-Q	-237 / 890
J-K	-4994 / 1605	-115.2	-115.2	0.29 (3)	3.73	Q-K	-824 / 384
K-L	-5532 / 1713	-115.2	-115.2	0.33 (3)	3.53	K-P	-120 / 274
L-M	-5532 / 1713	-115.2	-115.2	0.33 (3)	3.53	P-M	-24 / 380
M-AA	-2820 / 904	-115.2	-115.2	0.15 (3)	4.88	X-Y	-385 / 1912
AA-N	-4270 / 1164	-115.2	-115.2	0.18 (3)	4.08	X-C	-2971 / 828
N-O	0 / 1	-115.2	-115.2	0.09 (3)	10.00	M-Z	-2970 / 827
						Z-AA	-387 / 1912
B-X	-866 / 2518	-39.5	-39.5	0.30 (2)	6.25		
X-W	-1526 / 4875	-39.5	-39.5	0.68 (1)	6.25		
W-V	-1438 / 4893	-39.5	-39.5	0.67 (1)	6.25		
V-U	-1189 / 4457	-39.5	-39.5	0.62 (1)	6.25		
U-T	-1189 / 4457	-39.5	-39.5	0.62 (1)	6.25		
T-S	-1376 / 5190	-39.5	-39.5	0.68 (1)	6.25		
S-R	-1028 / 4458	-39.5	-39.5	0.62 (1)	6.25		
R-Q	-1028 / 4458	-39.5	-39.5	0.62 (1)	6.25		
Q-P	-1244 / 4893	-39.5	-39.5	0.67 (1)	6.25		
P-Z	-1333 / 4874	-39.5	-39.5	0.68 (1)	6.25		
Z-N	-673 / 2518	-39.5	-39.5	0.30 (3)	6.25		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	33.4	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	57.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 4 OF CBC 2018, ABC 2019
- PART 4 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- SLOPE REDUCTION FACTOR NOT USED
- PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED.

(80 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 33.4 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.51")
CALCULATED VERT. DEFL.(LL)= L/999 (0.23")
ALLOWABLE DEFL.(TL)= L/180 (3.03")
CALCULATED VERT. DEFL.(TL)= L/999 (0.32")

CSI: TC=0.41/1.00 (F-G-1), BC=0.68/1.00 (P-Z-1), WB=0.78/1.00 (G-T-3), SS=0.24/1.00 (F-G-2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Degrees

JSI GRIP= 0.88 (F) (INPUT = 0.90)
JSI METAL= 0.86 (C) (INPUT = 1.00)



Structural component only
DWG# T-2115850

JOB NAME 413226	TRUSS NAME T11	QUANTITY 10	PLY 1	JOB DESC. ROYAL PINE HOMES TRUSS DESC.	DRWG NO.
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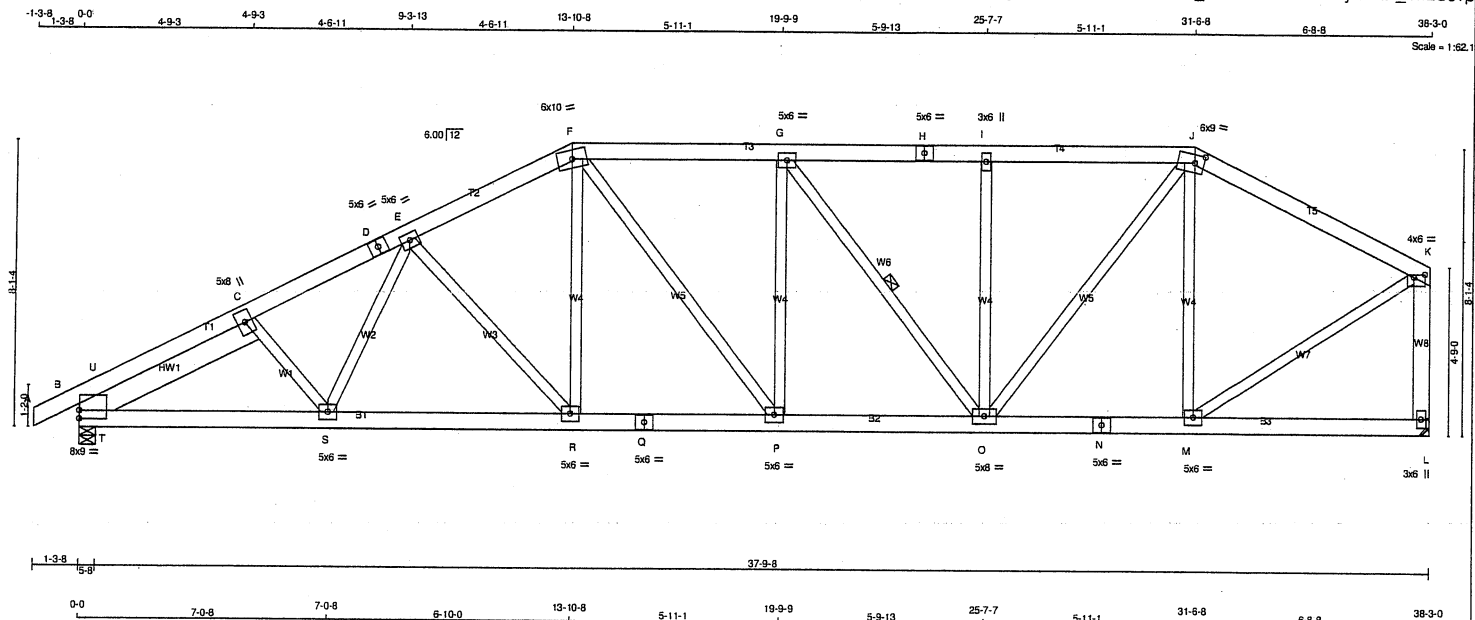
TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.2) PSF AT (31-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.



JOB NAME 413226	TRUSS NAME T11A	QUANTITY 4	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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TOTAL WEIGHT = 4 X 255 = 1019 lb
[M/F]

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x6 DRY	No.2	SPF
D - F	2x6 DRY	No.2	SPF
F - H	2x6 DRY	No.2	SPF
H - J	2x6 DRY	No.2	SPF
J - K	2x6 DRY	No.2	SPF
L - K	2x6 DRY	No.2	SPF
B - Q	2x6 DRY	No.2	SPF
Q - N	2x6 DRY	No.2	SPF
N - L	2x6 DRY	No.2	SPF

REINFORCING MEMBERS			
HW1	2x8 DRY	No.2	SPF

ALL WEBS EXCEPT			
2x4 DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN Y	X
B TMBMW1-t	MT20	8.0	9.0	2.75
C TMBWW-t	MT20	5.0	8.0	
D TS-t	MT20	5.0	6.0	
E TMBWW-t	MT20	5.0	6.0	
F TTBWW-m	MT20	6.0	10.0	
G TMBWW-t	MT20	5.0	6.0	
H TS-t	MT20	5.0	6.0	
I TMBW-w	MT20	3.0	6.0	
J TTBWW-m	MT20	6.0	9.0	2.75 3.25
K TMBWW-p	MT20	4.0	6.0	1.00 3.75
L BMV1+p	MT20	3.0	6.0	
M, P, R, S				
M BMWW-t	MT20	5.0	6.0	
N BS-t	MT20	5.0	6.0	
O BMWWW-t	MT20	5.0	8.0	
Q BS-t	MT20	5.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM, FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	UPLIFT
B	2237	0	2237	0
L	2109	0	2109	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT L. MINIMUM BEARING LENGTH AT JOINT L = 3-8.

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS						
JT COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
B	1579	1050 / 0	0 / 0	0 / 0	529 / 0	0 / 0	
L	1491	979 / 0	0 / 0	0 / 0	513 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.65 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 0	-91.8 -91.8	0.05 (1)	10.00	C-S	-15 / 43	0.01 (4)
B-U	-2536 / 0	-91.8 -91.8	0.08 (1)	5.16	S-E	0 / 153	0.03 (4)
U-C	-1682 / 0	-91.8 -91.8	0.07 (1)	6.03	E-R	-551 / 0	0.36 (1)
C-D	-3194 / 0	-91.8 -91.8	0.14 (1)	4.65	R-F	0 / 539	0.09 (1)
D-E	-3194 / 0	-91.8 -91.8	0.14 (1)	4.65	F-P	0 / 369	0.06 (1)
E-F	-2750 / 0	-91.8 -91.8	0.13 (1)	4.94	P-G	-183 / 31	0.15 (1)
F-G	-2678 / 0	-91.8 -91.8	0.20 (1)	4.91	G-O	-506 / 0	0.22 (1)
G-H	-2371 / 0	-91.8 -91.8	0.18 (1)	5.18	O-I	-578 / 0	0.46 (1)
H-I	-2371 / 0	-91.8 -91.8	0.18 (1)	5.18	I-J	0 / 1354	0.22 (1)
I-J	-2371 / 0	-91.8 -91.8	0.20 (1)	5.16	M-J	-825 / 0	0.66 (1)
J-K	-1742 / 0	-91.8 -91.8	0.28 (1)	5.69	M-K	0 / 1834	0.29 (1)
L-K	-2058 / 0	0.0 0.0	0.44 (1)	7.06	T-U	0 / 1069	0.00 (1)
				T-C	-1720 / 0	0.29 (1)	
B-T	0 / 1503	-18.5 -18.5	0.19 (1)	10.00			
T-S	0 / 2874	-18.5 -18.5	0.40 (1)	10.00			
S-R	0 / 2805	-18.5 -18.5	0.37 (1)	10.00			
R-Q	0 / 2452	-18.5 -18.5	0.33 (1)	10.00			
Q-P	0 / 2452	-18.5 -18.5	0.33 (1)	10.00			
P-O	0 / 2678	-18.5 -18.5	0.35 (1)	10.00			
O-N	0 / 1541	-18.5 -18.5	0.24 (1)	10.00			
N-M	0 / 1541	-18.5 -18.5	0.24 (1)	10.00			
M-L	0 / 0	-18.5 -18.5	0.10 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
DL = 6.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.4 PSF	
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.27")
CALCULATED VERT. DEFL.(LL) = L/999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.27")
CALCULATED VERT. DEFL.(TL) = L/999 (0.18")

CSI: TC=0.44/1.00 (K-L:1), BC=0.40/1.00 (S-T:1), WB=0.66/1.00 (J-M:1), SSI=0.19/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

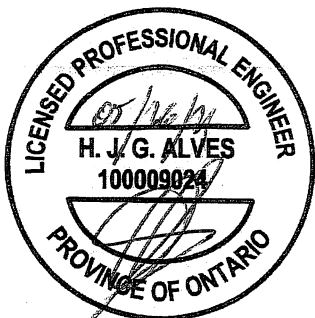
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.84 (J) (INPUT = 0.90)
JSI METAL = 0.59 (C) (INPUT = 1.00)

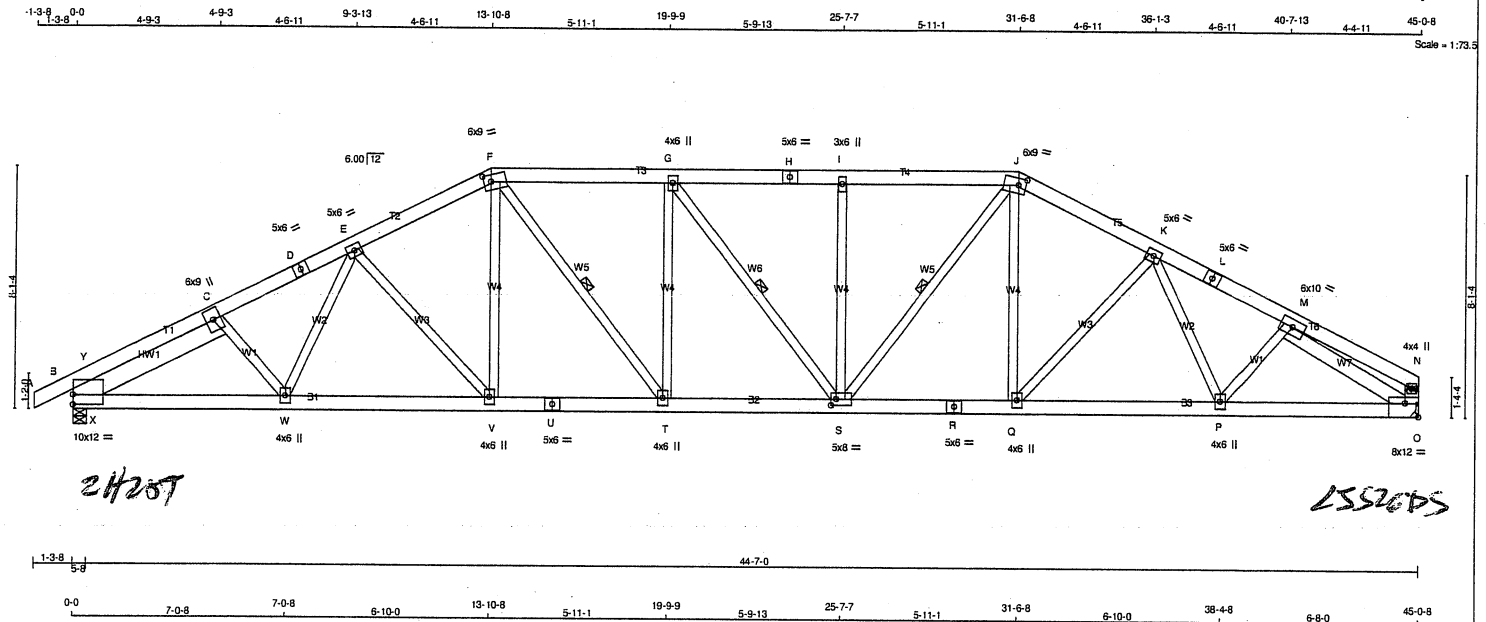


Structural component only
DWG# T-2115851

JOB NAME 413226	TRUSS NAME T11B	QUANTITY 4	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x6	DRY	No.2
D - F	2x6	DRY	No.2
F - H	2x6	DRY	No.2
H - J	2x6	DRY	No.2
J - L	2x6	DRY	No.2
L - N	2x6	DRY	No.2
O - N	2x6	DRY	No.2
B - U	2x6	DRY	No.2
U - R	2x6	DRY	No.2
R - O	2x6	DRY	No.2
REINFORCING MEMBERS			
HW1	2x8	DRY	No.2
ALL WEBS EXCEPT			
M - O	2x6	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B	TMBMW1-t	MT20	10.0	12.0 4.00
C	TMWW-t	MT20	6.0	9.0
D, H, L				
D	TS-t	MT20	5.0	6.0
E	TMWW-t	MT20	5.0	6.0
F	TTWW-m	MT20	6.0	9.0 2.75 3.00
G	TMWW-t	MT20	4.0	6.0
I	TMWW-t	MT20	3.0	6.0
J	TTWW-m	MT20	6.0	9.0 2.75 3.00
K	TMWW-t	MT20	5.0	6.0
M	TMWW-t	MT20	6.0	10.0
N	TMV-t	MT20	4.0	4.0
O	BMVW1-t	MT20	8.0	12.0 5.50 Edge
P, Q, T, V, W				
P	BMVW-t	MT20	4.0	6.0
R	BS-t	MT20	5.0	6.0
S	BMVWW-t	MT20	5.0	8.0 2.50 2.00
U	BS-t	MT20	5.0	6.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	DOWN	GROSS REACTION	HORIZ	BRG	IN-SX	BRG	IN-SX
B	3646	0	3690	196	-1065	5-8	5-8		
O	3485	0	3487	0	-1004	MECHANICAL			

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT O. MINIMUM BEARING LENGTH AT JOINT O = 3-8.

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 1065 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT O FOR 1004 LBS. FACTORED UPLIFT

PROVIDE FOR 196 LBS. FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS

JT	COMBINED	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	PERM.LIVE	WIND	DEAD	SOIL
B	2692	1628 / 0	473 / 0	0 / 0	0 / -1059	620 / 0	0 / 0
O	2582	1506 / 0	473 / 0	0 / 0	65 / -1007	604 / 0	0 / 0

HORIZONTAL REACTIONS

B	0 / 0	0 / 0	0 / 0	140 / -120	0 / 0	0 / 0
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BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 3.55 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-T, G-S, J-S.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (18)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 1	-115.2 -115.2	0.09 (2)	C-W	-29 / 374	0.06 (3)	
B-Y	-4246 / 1142	-115.2 -115.2	0.18 (2)	W-E	-104 / 278	0.04 (5)	
Y-C	-2804 / 889	-115.2 -115.2	0.15 (2)	E-V	-827 / 382	0.54 (2)	
C-D	-5478 / 1685	-115.2 -115.2	0.33 (2)	V-F	-232 / 887	0.18 (13)	
D-E	-5478 / 1685	-115.2 -115.2	0.33 (2)	F-T	-468 / 1479	0.24 (3)	
E-F	-4934 / 1576	-115.2 -115.2	0.29 (2)	T-G	-948 / 420	0.78 (3)	
F-G	-5115 / 1656	-115.2 -115.2	0.41 (1)	G-S	-350 / 274	0.16 (2)	
G-H	-5088 / 1642	-115.2 -115.2	0.41 (1)	S-I	-722 / 358	0.58 (2)	
H-I	-5088 / 1642	-115.2 -115.2	0.41 (1)	I-J	-480 / 1532	0.25 (2)	
I-J	-5088 / 1641	-115.2 -115.2	0.41 (1)	J-K	-210 / 803	0.17 (14)	
J-K	-4862 / 1552	-115.2 -115.2	0.29 (3)	K-P	-707 / 342	0.47 (3)	
K-L	-5236 / 1603	-115.2 -115.2	0.32 (3)	P-M	-220 / 176	0.08 (9)	
L-M	-5236 / 1603	-115.2 -115.2	0.32 (3)	M-O	-22 / 556	0.09 (2)	
M-N	-16 / 85	-115.2 -115.2	0.16 (3)	O-X	-5457 / 1535	1.00 (1)	
O-N	-253 / 139	0.0 0.0	0.02 (14)	X-C	-377 / 1894	0.00 (1)	
B-X	-856 / 2504	-39.5 -39.5	0.30 (2)				
X-W	-1505 / 4830	-39.5 -39.5	0.67 (1)				
W-V	-1416 / 4843	-39.5 -39.5	0.66 (1)				
V-U	-1169 / 4404	-39.5 -39.5	0.61 (1)				
U-T	-1169 / 4404	-39.5 -39.5	0.61 (1)				
T-S	-1349 / 5115	-39.5 -39.5	0.67 (1)				
S-R	-993 / 4338	-39.5 -39.5	0.61 (1)				
R-Q	-993 / 4338	-39.5 -39.5	0.61 (1)				
Q-P	-1197 / 4699	-39.5 -39.5	0.65 (1)				
P-O	-1238 / 4460	-39.5 -39.5	0.64 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 33.4	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 57.3	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF BCBC 2018, ABC 2019
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- SLOPE REDUCTION FACTOR NOT USED
- PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED.

(80 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 33.4 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.50")
CALCULATED VERT. DEFL.(LL) = L/999 (0.23")
ALLOWABLE DEFL.(TL) = L/180 (3.00")
CALCULATED VERT. DEFL.(TL) = L/999 (0.32")

CSI: TC=0.41/1.00 (F-G:1), BC=0.67/1.00 (W-X:1), WB=1.00/1.00 (M-O:1), SSI=0.24/1.00 (F-G:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (J) (INPUT = 0.90)
JSI METAL = 0.85 (C) (INPUT = 1.00)



Structural component only
DWG# T-2115852

CONTINUED ON PAGE 2

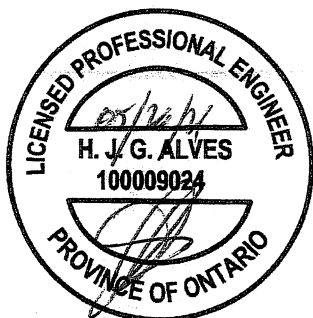
JOB NAME 413226	TRUSS NAME T11B	QUANTITY 4	PLY 1	JOB DESC. ROYAL PINE HOMES TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed May 26 17:26:09 2021 Page 2
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TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.2) PSF AT (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_{pe} , BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.

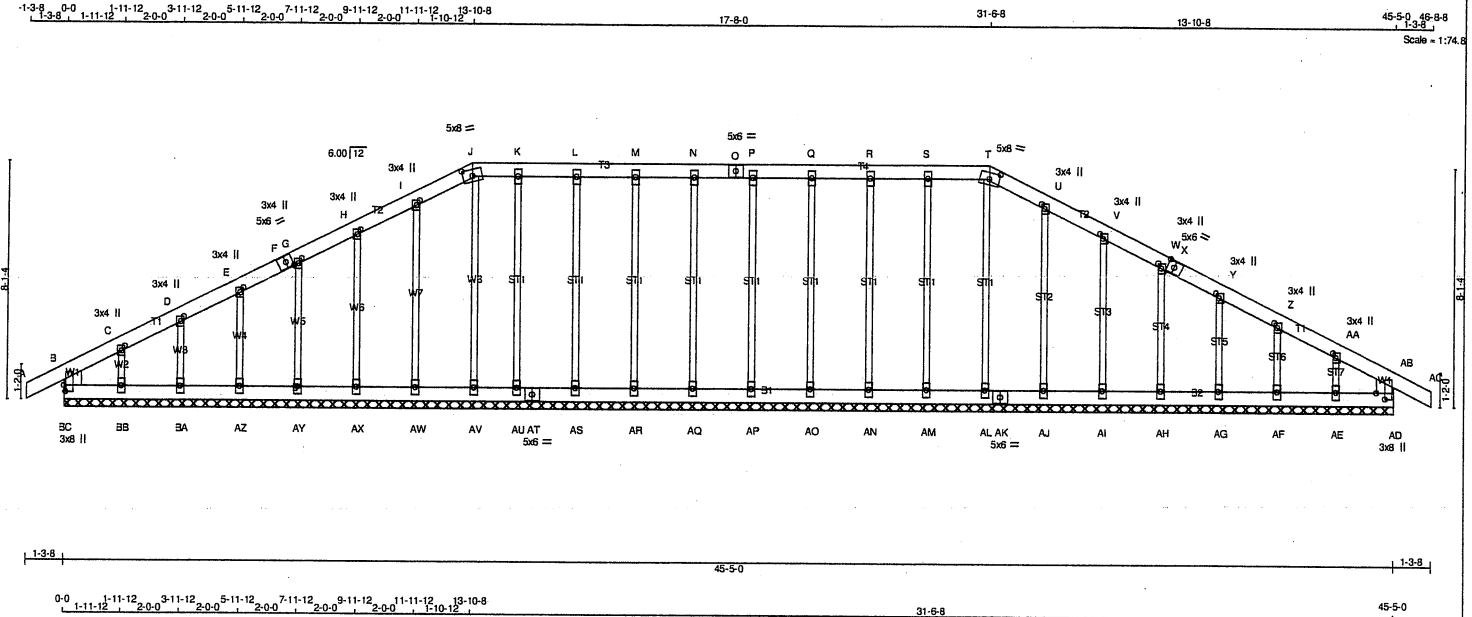


Structural component only
DWG# T-2115852 *W*

JOB NAME 413226	TRUSS NAME T11G	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

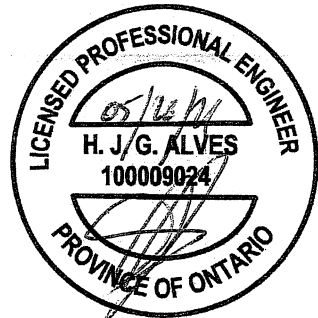
Version 8.420 S Jan 21 2021 MITek Industries, Inc. Wed May 26 14:46:40 2021 Page 1
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LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				TOTAL WEIGHT = 2 X 272 = 544 lb			
N. L. G. A. RULES				BEARINGS							
CHORDS	SIZE	LUMBER	DESCR.	THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.				DESIGN CRITERIA			
BC - B	2x8	DRY	No.2	THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.				SPECIFIED LOADS:			
A - F	2x6	DRY	No.2	BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)				TOP CH. LL = 25.6 PSF			
F - J	2x6	DRY	No.2	BRACING				DL = 6.0 PSF			
J - O	2x6	DRY	No.2	TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.				BOT CH. LL = 0.0 PSF			
O - T	2x6	DRY	No.2	MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.				DL = 7.4 PSF			
T - X	2x6	DRY	No.2	ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.				TOTAL LOAD = 39.0 PSF			
X - AC	2x6	DRY	No.2	LOADING				SPACING = 24.0 IN./C			
AD - AB	2x8	DRY	No.2	TOTAL LOAD CASES: (4)				LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM			
BC - AT	2x6	DRY	No.2					THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015			
AT - AK	2x6	DRY	No.2					THIS DESIGN COMPLIES WITH:			
AK - AD	2x6	DRY	No.2					- PART 9 OF BCBC 2018, ABC 2019			
ALL WEBS 2x3	DRY	No.2	SPF					- PART 9 OF OBC 2012 (2019 AMENDMENT)			
ALL GABLE WEBS 2x3	DRY	No.2	SPF					- CSA 086-14			
DRY: SEASONED LUMBER.								- TPIC 2014			
GABLE STUDS SPACED AT 2-0-0 OC.								DESIGN ASSUMPTIONS			

PLATES (table is in inches)				CHORDS				WEBS			
JT TYPE	PLATES	W	LEN Y X	MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	FACTORED LC1 MAX CSI (LO)	MEMB.	FORCE (LBS)	MAX. FACTORED MAX CSI (LC)	
B, AB, AD, BC				FR-TO				FR-TO			
B				BC-B	-260 / 0	0.0	0.02 (1)	7.81	AL-T	-186 / 0	0.21 (1)
C, D, E, G, H, I, U, V, W, Y, Z, AA				A-B	0 / 29	-91.8	-91.8 0.06 (1)	10.00	AM-S	-190 / 0	0.21 (1)
C TMW+w	MT20	3.0	4.0 2.00 1.25	B-C	-50 / 0	-91.8	-91.8 0.05 (1)	6.25	AN-R	-183 / 0	0.21 (1)
F TS-t	MT20	5.0	6.0 2.50 2.75	C-D	-24 / 0	-91.8	-91.8 0.02 (1)	6.25	AO-Q	-183 / 0	0.21 (1)
J TTW-m	MT20	5.0	8.0 2.75 4.00	D-E	-19 / 0	-91.8	-91.8 0.02 (1)	6.25	AP-P	-183 / 0	0.21 (1)
K, L, M, N, P, Q, R, S				E-F	-13 / 0	-91.8	-91.8 0.02 (1)	6.25	AQ-N	-183 / 0	0.21 (1)
K TMW+w	MT20	3.0	6.0	F-G	-13 / 0	-91.8	-91.8 0.02 (1)	6.25	AR-M	-184 / 0	0.21 (1)
O TS-t	MT20	5.0	6.0	G-H	-9 / 0	-91.8	-91.8 0.02 (1)	10.00	AS-L	-184 / 0	0.21 (1)
T TTW-m	MT20	5.0	8.0 2.75 4.00	H-I	-5 / 0	-91.8	-91.8 0.02 (1)	10.00	AU-K	-163 / 0	0.18 (1)
X TS-t	MT20	5.0	6.0 2.50 2.75	I-J	-3 / 0	-91.8	-91.8 0.02 (1)	10.00	AJ-U	-183 / 0	0.14 (1)
AD TMBMV1+p	MT20	3.0	8.0 2.50 3.75	J-K	0 / 0	-91.8	-91.8 0.02 (1)	10.00	AI-V	-182 / 0	0.09 (1)
AE, AF, AG, AH, AI, AJ, AL, AM, AN, AO, AP, AQ, AR, AS, AU, AV, AW, AX, AY, AZ, BA, BB				K-L	0 / 0	-91.8	-91.8 0.02 (1)	10.00	AH-W	-182 / 0	0.06 (1)
AE BMW1+w	MT20	3.0	6.0	L-M	0 / 0	-91.8	-91.8 0.02 (1)	10.00	AG-Y	-180 / 0	0.04 (1)
AK BS-t	MT20	5.0	6.0	M-N	0 / 0	-91.8	-91.8 0.02 (1)	10.00	AF-Z	-188 / 0	0.03 (1)
AT BS-t	MT20	5.0	6.0	N-O	0 / 0	-91.8	-91.8 0.02 (1)	10.00	AE-AA	-137 / 0	0.02 (1)
BC TMBMV1+p	MT20	3.0	8.0 2.50 0.50	O-P	0 / 0	-91.8	-91.8 0.02 (1)	10.00	AV-J	-159 / 0	0.18 (1)
				P-Q	0 / 0	-91.8	-91.8 0.02 (1)	10.00	BB-C	-137 / 0	0.02 (1)
				Q-R	0 / 0	-91.8	-91.8 0.02 (1)	10.00	BA-D	-188 / 0	0.03 (1)
				R-S	0 / 0	-91.8	-91.8 0.02 (1)	10.00	AZ-E	-180 / 0	0.04 (1)
				S-T	0 / 0	-91.8	-91.8 0.02 (1)	10.00	AY-G	-181 / 0	0.06 (1)
				T-U	-2 / 0	-91.8	-91.8 0.02 (1)	10.00	AX-H	-183 / 0	0.10 (1)
				U-V	-5 / 0	-91.8	-91.8 0.02 (1)	10.00	AW-I	-184 / 0	0.14 (1)
				V-W	-9 / 0	-91.8	-91.8 0.02 (1)	10.00			
				W-X	-14 / 0	-91.8	-91.8 0.02 (1)	6.25			
				X-Y	-14 / 0	-91.8	-91.8 0.02 (1)	6.25			
				Y-Z	-20 / 0	-91.8	-91.8 0.02 (1)	6.25			
				Z-AA	-24 / 0	-91.8	-91.8 0.02 (1)	6.25			
				AA-AB	-51 / 0	-91.8	-91.8 0.05 (1)	6.25			
				AB-AC	0 / 29	-91.8	-91.8 0.06 (1)	10.00			
				AD-AB	-260 / 0	0.0	0.02 (1)	7.81			

PLATE PLACEMENT TOL = 0.250 inches				PLATE ROTATION TOL = 5.0 Deg.			
JSI GRIP = 0.25 (J) (INPUT = 0.90)				JSI METAL = 0.09 (AD) (INPUT = 1.00)			
COMPANION LIVE LOAD FACTOR = 1.00				AUTOSOLVE HEELS OFF			
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.				NAIL VALUES			
PLATE GRIP(DRY) SHEAR SECTION (PS) (PLJ) (PLI)				MAX MIN MAX MIN MAX MIN			
MT20 650 371 1747 788 1987 1873				PLATE PLACEMENT TOL = 0.250 inches			
PLATE ROTATION TOL = 5.0 Deg.				JSI GRIP = 0.25 (J) (INPUT = 0.90)			
JSI METAL = 0.09 (AD) (INPUT = 1.00)				JSI METAL = 0.09 (AD) (INPUT = 1.00)			



Structural component only
DWG# T-2115853

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
413226	T11G	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MITek Industries, Inc. Wed May 26 14:46:40 2021 Page 2
ID:U6yi?rbeFFwxf UFm koDybSsJ-JsrEHUeCmZKuwYfK72bn04eJG2k8re803VehvszCcVi

LOADING

TOTAL LOAD CASES: (4)

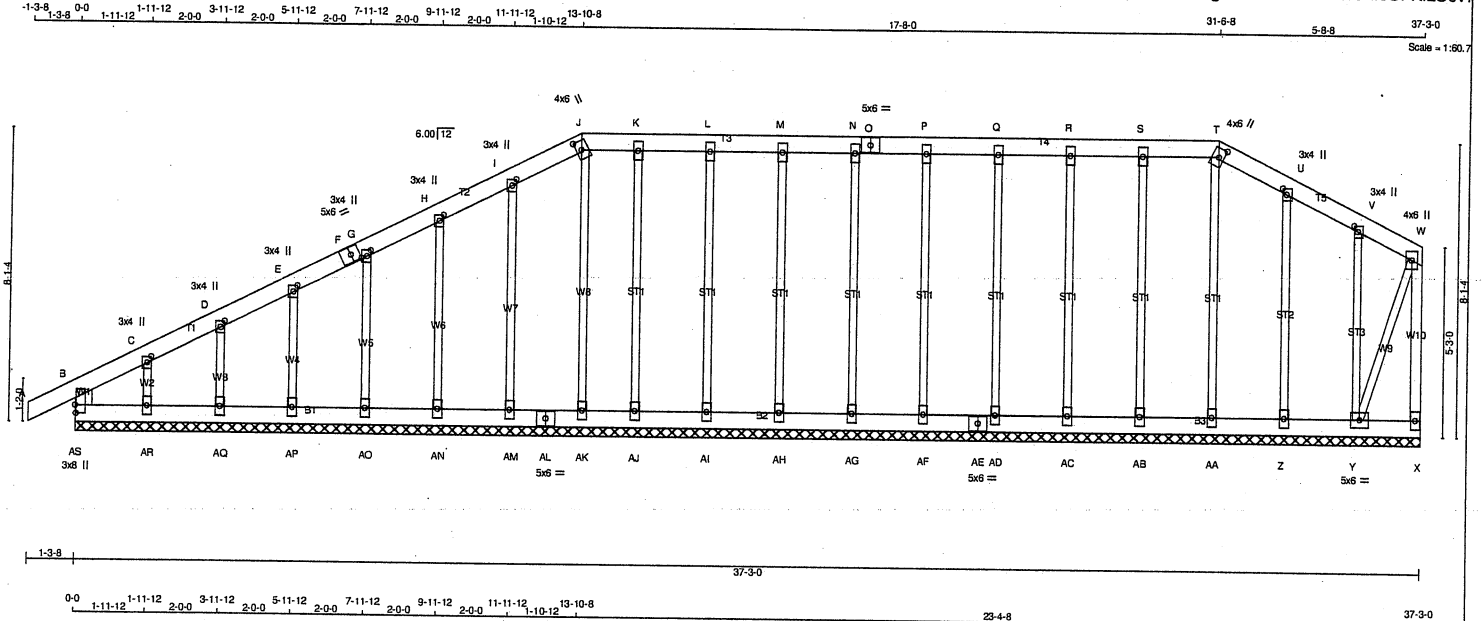
C H O R D S				W E B S			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			FR-TO		
AI-AH	0 / 8	-18.5 -18.5	0.01 (4)	10.00			
AH-AG	0 / 12	-18.5 -18.5	0.01 (4)	10.00			
AG-AF	0 / 17	-18.5 -18.5	0.01 (4)	10.00			
AF-AE	0 / 25	-18.5 -18.5	0.01 (1)	10.00			
AE-AD	0 / 33	-18.5 -18.5	0.03 (1)	10.00			



Structural component only
DWG# T-2115853 *W*

JOB NAME 413226	TRUSS NAME T11GB	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed May 26 14:46:41 2021 Page 1
ID:hEKUVWZApjnyW9leD6V_jMya4z_n3PcVqfQW5SIY6EXgm60YHBU?R4ia56AI9OFrIzCcVj



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
AS- B	2x6	DRY	No.2 SPF
A- F	2x6	DRY	No.2 SPF
F- J	2x6	DRY	No.2 SPF
J- O	2x6	DRY	No.2 SPF
O- T	2x6	DRY	No.2 SPF
T- W	2x6	DRY	No.2 SPF
X- W	2x4	DRY	No.2 SPF
AS- AL	2x6	DRY	No.2 SPF
AL- AE	2x6	DRY	No.2 SPF
AE- X	2x6	DRY	No.2 SPF
ALL WEBS	2x3	DRY	No.2 SPF
ALL GABLE WEBS	2x3	DRY	No.2 SPF
DRY: SEASONED LUMBER.			

GABLE STUDS SPACED AT 2'-0" OC.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER			
BEARINGS			
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.			
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.			
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)			
BRACING			
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.			
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.			
LOADING			
TOTAL LOAD CASES: (4)			

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
AS-B	-255 / 0	AA-T	-170 / 0
A-B	0 / 29	AB-S	-206 / 0
B-C	-18 / 0	AC-R	-183 / 0
C-D	0 / 14	AD-Q	-183 / 0
D-E	0 / 16	AE-P	-184 / 0
E-F	0 / 23	AG-N	-183 / 0
F-G	0 / 23	AH-M	-184 / 0
G-H	0 / 27	AI-L	-186 / 0
H-I	0 / 31	AJ-K	-178 / 0
I-J	0 / 27	Z-U	-206 / 0
J-K	0 / 33	Y-V	-173 / 0
K-L	0 / 33	AK-J	-146 / 0
L-M	0 / 33	AR-C	-122 / 0
M-N	0 / 33	AQ-D	-193 / 0
N-O	0 / 33	AP-E	-179 / 0
O-P	0 / 33	AO-G	-181 / 0
P-Q	0 / 33	AN-H	-182 / 0
Q-R	0 / 33	AM-I	-198 / 0
R-S	0 / 33	Y-W	-68 / 0
S-T	0 / 33		
T-U	0 / 26		
U-V	0 / 34		
V-W	0 / 32		
X-W	0 / 9		

AS-AR	0 / 0	-18.5	-18.5	0.01 (4)	10.00
AR-AQ	-8 / 0	-18.5	-18.5	0.01 (4)	10.00
AQ-AP	-15 / 0	-18.5	-18.5	0.01 (4)	6.25
AP-AO	-20 / 0	-18.5	-18.5	0.01 (4)	6.25
AO-AN	-24 / 0	-18.5	-18.5	0.01 (4)	6.25
AN-AM	-28 / 0	-18.5	-18.5	0.01 (4)	6.25
AM-AL	-31 / 0	-18.5	-18.5	0.01 (4)	6.25
AL-AK	-31 / 0	-18.5	-18.5	0.01 (4)	6.25
AK-AJ	-33 / 0	-18.5	-18.5	0.01 (4)	6.25
AJ-AI	-33 / 0	-18.5	-18.5	0.01 (4)	6.25
AI-AH	-33 / 0	-18.5	-18.5	0.01 (4)	6.25
AH-AG	-33 / 0	-18.5	-18.5	0.01 (4)	6.25
AG-AF	-33 / 0	-18.5	-18.5	0.01 (4)	6.25
AF-AE	-33 / 0	-18.5	-18.5	0.01 (4)	6.25
AE-AD	-33 / 0	-18.5	-18.5	0.01 (4)	6.25
AD-AC	-33 / 0	-18.5	-18.5	0.01 (4)	6.25
AC-AB	-33 / 0	-18.5	-18.5	0.01 (4)	6.25
AB-AA	-33 / 0	-18.5	-18.5	0.01 (4)	6.25
AA-Z	-30 / 0	-18.5	-18.5	0.01 (4)	6.25
Z-Y	-27 / 0	-18.5	-18.5	0.01 (4)	6.25
Y-X	0 / 0	-18.5	-18.5	0.01 (4)	10.00

TOTAL WEIGHT = 2 X 237 = 474 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.
(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.06/1.00 (A-B:1), BC=0.01/1.00 (AQ-AR:4), WB=0.23/1.00 (S-AB:1), SS=0.07/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

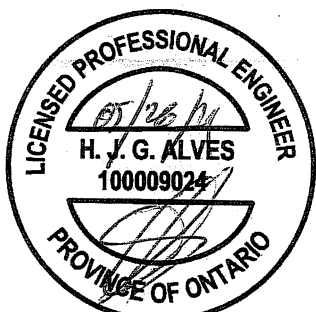
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

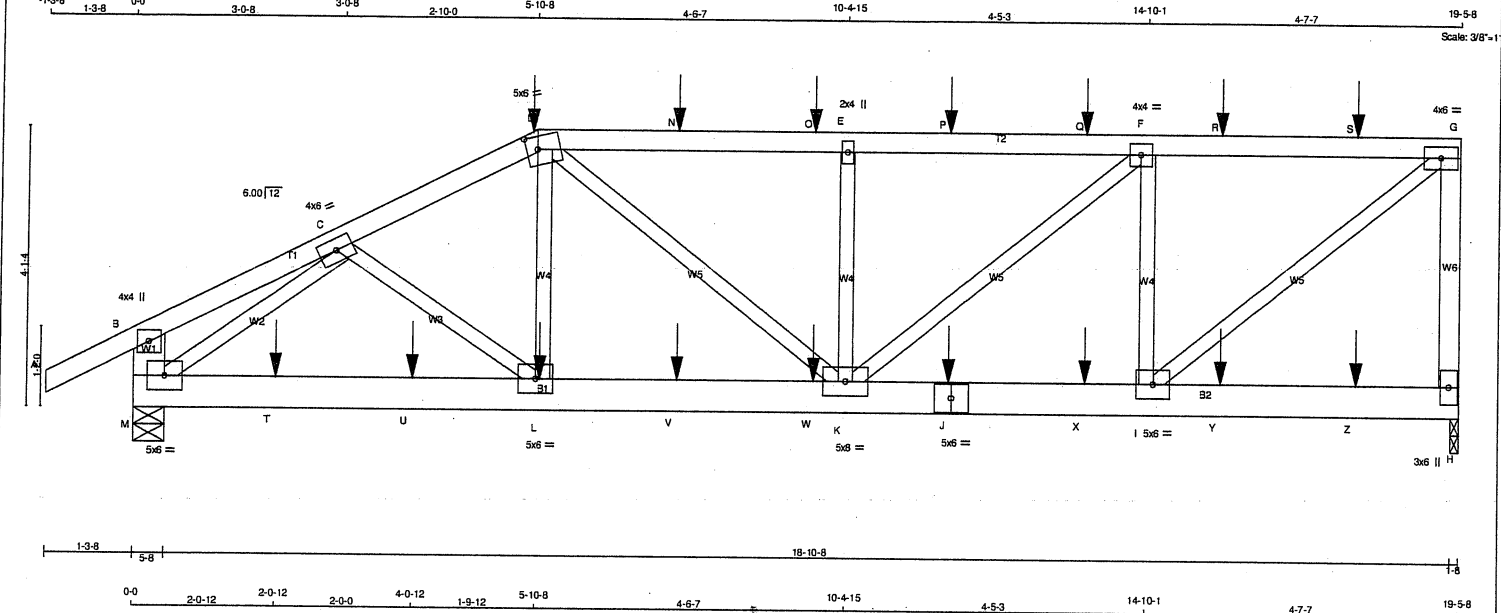
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.43 (T) (INPUT = 0.90)
JSI METAL= 0.07 (AS) (INPUT = 1.00)



Structural component only
DWG# T-2115854



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF **2** TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - D 1	12	SIDE(61.0)
D - G 1	12	SIDE(61.0)
G - H 1	12	TOP
M - B 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
M - J 2	12	SIDE(183.1)
J - H 2	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
H	1774	0	1774	0	1-8	1-8
M	1855	0	1855	0	5-8	5-8

UNFACTORED REACTIONS							
	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1255	820 / 0	0 / 0	0 / 0	0 / 0	435 / 0	0 / 0
M	1311	867 / 0	0 / 0	0 / 0	0 / 0	444 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.42 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

FR-TO	MAX. FORCE (LBS)	FACTORED		MAX. UNBRACED LENGTH	MEMB.	MAX. FORCE (LBS)	MAX. FACTORED CSI (LC)
		VERT. LOAD	LC1 MAX				
A - B	0 / 28	-91.8	-91.8	0.07 (1)	10.00	C - L	0 / 214
B - C	0 / 9	-91.8	-91.8	0.05 (1)	10.00	L - D	0 / 151
C - D	-2468 / 0	-91.8	-91.8	0.07 (1)	5.69	M - C	-2546 / 0
D - N	-2488 / 0	-91.8	-91.8	0.23 (1)	5.46	I - G	0 / 2332
N - O	-2488 / 0	-91.8	-91.8	0.23 (1)	5.46	D - K	0 / 375
O - E	-2488 / 0	-91.8	-91.8	0.23 (1)	5.46	I - F	-1302 / 0
E - P	-2488 / 0	-91.8	-91.8	0.26 (1)	5.42	K - E	-702 / 0
P - Q	-2488 / 0	-91.8	-91.8	0.26 (1)	5.42	K - F	0 / 880
Q - F	-2488 / 0	-91.8	-91.8	0.26 (1)	5.42		
F - R	-1814 / 0	-91.8	-91.8	0.25 (1)	6.13		
R - S	-1814 / 0	-91.8	-91.8	0.25 (1)	6.13		
S - G	-1814 / 0	-91.8	-91.8	0.25 (1)	6.13		
H - G	-1712 / 0	0.0	0.0	0.21 (1)	7.81		
M - B	-239 / 0	0.0	0.0	0.01 (1)	7.81		
M - T	0 / 2025	-18.5	-18.5	0.17 (1)	10.00		
T - U	0 / 2025	-18.5	-18.5	0.17 (1)	10.00		
U - L	0 / 2025	-18.5	-18.5	0.17 (1)	10.00		
L - V	0 / 2198	-18.5	-18.5	0.17 (1)	10.00		
V - W	0 / 2198	-18.5	-18.5	0.17 (1)	10.00		
W - K	0 / 2198	-18.5	-18.5	0.17 (1)	10.00		
K - J	0 / 1814	-18.5	-18.5	0.15 (1)	10.00		
J - X	0 / 1814	-18.5	-18.5	0.15 (1)	10.00		
X - I	0 / 1814	-18.5	-18.5	0.15 (1)	10.00		
I - Y	0 / 0	-18.5	-18.5	0.04 (4)	10.00		
Y - Z	0 / 0	-18.5	-18.5	0.04 (4)	10.00		
Z - H	0 / 0	-18.5	-18.5	0.04 (4)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	5-10-8	-317	-317	---	BACK	VERT	TOTAL	---	C1
J	11-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
L	5-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
N	7-11-4	-76	-76	---	BACK	VERT	TOTAL	---	C1
O	9-11-4	-76	-76	---	BACK	VERT	TOTAL	---	C1
P	11-11-4	-76	-76	---	BACK	VERT	TOTAL	---	C1
Q	13-11-4	-76	-76	---	BACK	VERT	TOTAL	---	C1
R	15-11-4	-76	-76	---	BACK	VERT	TOTAL	---	C1
S	17-11-4	-76	-76	---	BACK	VERT	TOTAL	---	C1
T	2-0-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
U	4-0-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
V	7-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
W	9-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.65")

CALCULATED VERT. DEFL.(LL) = L/999 (0.04")

ALLOWABLE DEFL.(TL) = L/360 (0.65")

CALCULATED VERT. DEFL.(TL) = L/999 (0.07")

CSI: TC=0.26/1.00 (E-F:1), BC=0.17/1.00 (L-M:1), WB=0.30/1.00 (C-M:1), SSI=0.18/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.69 (G) (INPUT = 0.90)

JSI METAL = 0.30 (C) (INPUT = 1.00)



Structural component only
DWG# T-2115855

JOB NAME 413226	TRUSS NAME T101	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	6.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	5.0	8.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMVW1-t	MT20	5.0	6.0		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
X	13-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
Y	15-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
Z	17-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

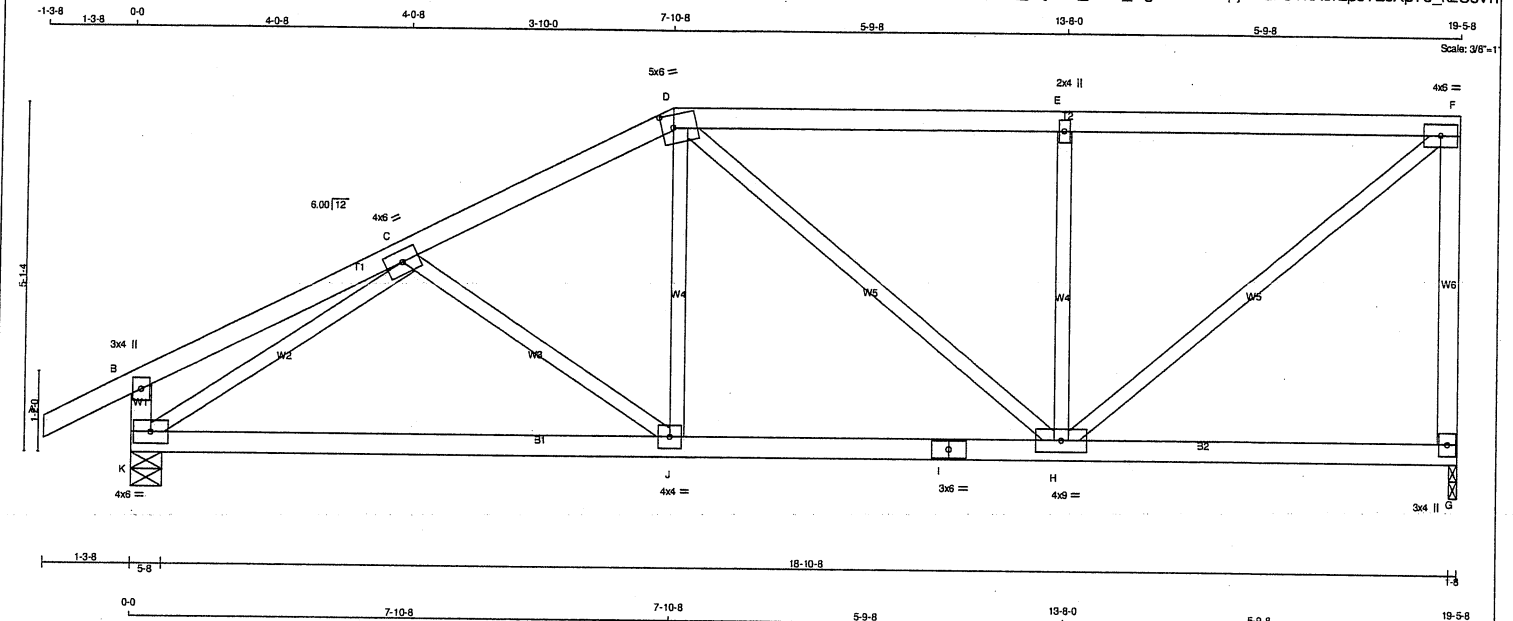


Structural component only
DWG# T-2115855 *W*

JOB NAME 413226	TRUSS NAME T102	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed May 26 14:46:42 2021 Page 1
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LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER		
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
K - B	2x4	DRY	No.2	SPF	
K - I	2x4	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0	
C	TMVW-t	MT20	4.0	6.0	
D	TTWW-m	MT20	5.0	6.0	2.25 2.00
E	TMVW-t	MT20	2.0	4.0	
F	TMVW-t	MT20	4.0	6.0	
G	BMV1+p	MT20	3.0	4.0	
H	BMVW-t	MT20	4.0	9.0	
I	BS-t	MT20	3.0	6.0	
J	BMVW-t	MT20	4.0	4.0	
K	BMVW-t	MT20	4.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	IN-SX	BRG	IN-SX
G	1073	0	1073	0	0	1-8	1-8		
K	1197	0	1197	0	0	5-8	5-8		

UNFACTORED REACTIONS		1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
G	759	498 / 0	0 / 0	0 / 0	0 / 0	261 / 0	0 / 0		
K	844	567 / 0	0 / 0	0 / 0	0 / 0	277 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.39 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING		TOTAL LOAD CASES: (4)							
CHORDS		MEMB.		WEBS					
FR-TO	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	MAX. UNBRACED LENGTH	FR-TO	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	FR-TO	MAX. FACTORED FORCE (LBS)
A-B	0 / 28	-91.8	-91.8 0.12 (1)	10.00	C-J	-182 / 2	0.07 (1)		
B-C	0 / 18	-91.8	-91.8 0.21 (1)	10.00	J-D	0 / 253	0.06 (4)		
C-D	-1207 / 0	-91.8	-91.8 0.24 (1)	5.54	D-H	-94 / 0	0.10 (1)		
D-E	-996 / 0	-91.8	-91.8 0.55 (1)	5.39	H-E	-658 / 0	0.25 (1)		
E-F	-996 / 0	-91.8	-91.8 0.56 (1)	5.39	H-F	0 / 1291	0.29 (1)		
G-F	-1026 / 0	0.0	0.0 0.45 (1)	7.74	K-C	-1491 / 0	0.57 (1)		
K-B	-266 / 0	0.0	0.0 0.03 (1)	7.81					
K-J	0 / 1212	-18.5	-18.5 0.34 (4)	10.00					
J-I	0 / 1068	-18.5	-18.5 0.34 (4)	10.00					
I-H	0 / 1068	-18.5	-18.5 0.34 (4)	10.00					
H-G	0 / 0	-18.5	-18.5 0.13 (4)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/999 (0.15")

CSI: TC=0.58/1.00 (E-F:1), BC=0.34/1.00 (H-J:4), WB=0.57/1.00 (C-K:1), SSI=0.26/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

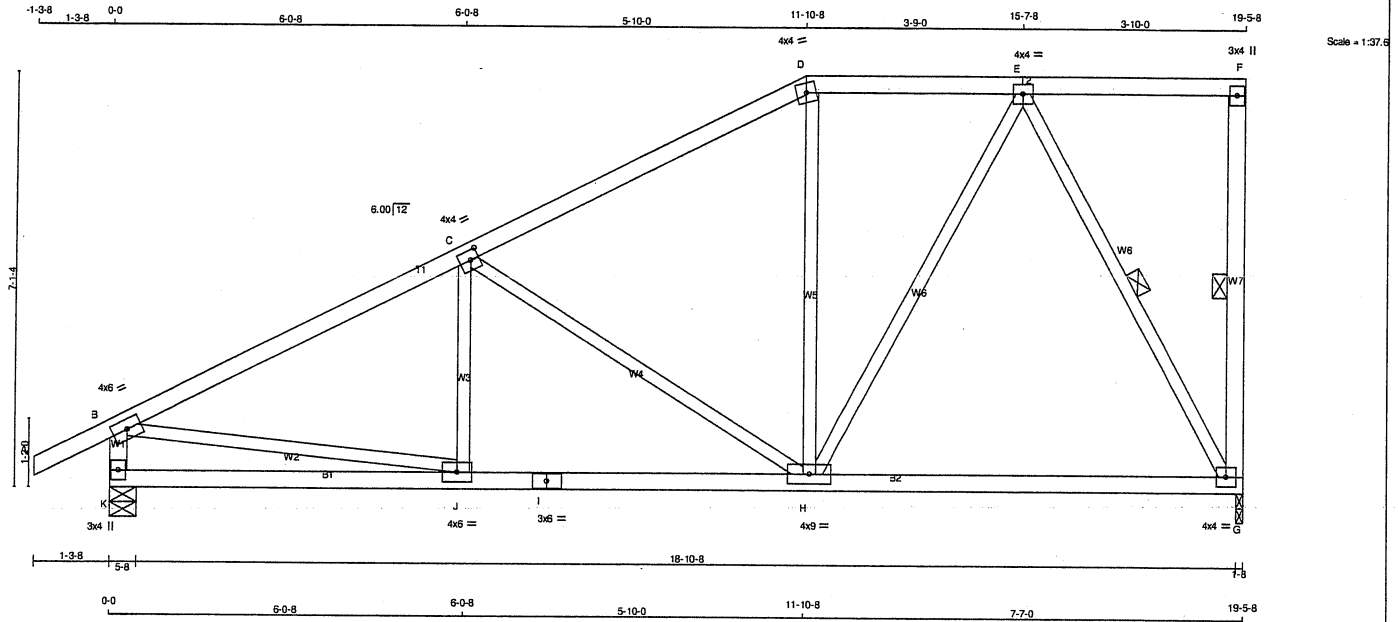
JSI GRIP= 0.78 (F) (INPUT = 0.90)
JSI METAL= 0.32 (C) (INPUT = 1.00)



Structural component only
DWG# T-2115857

JOB NAME 413226	TRUSS NAME T104	QUANTITY 3	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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TOTAL WEIGHT = 3 X 87 = 260 lb [M/F]

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY No.2	SPF
D - F	2x4	DRY No.2	SPF
G - H	2x4	DRY No.2	SPF
K - B	2x4	DRY No.2	SPF
K - I	2x4	DRY No.2	SPF
I - G	2x4	DRY No.2	SPF
ALL WEBS	2x3	DRY No.2	SPF
EXCEPT			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-t	MT20	4.0	6.0	
C	TMVW-t	MT20	4.0	4.0	2.00 1.75
D	TTW-m	MT20	4.0	4.0	
E	TMVW-t	MT20	4.0	4.0	
F	TMV-p	MT20	3.0	4.0	
G	BMVW-t	MT20	4.0	4.0	
H	BMVWW-t	MT20	4.0	9.0	
I	BS-t	MT20	3.0	6.0	
J	BMVW-t	MT20	4.0	6.0	
K	BMV1-p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1073	0	1073	0	1-8	1-8	
K	1197	0	1197	0	5-8	5-8	

UNFACTORED REACTIONS		MAX./MIN. COMPONENT REACTIONS					
JT	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
G	759	498 / 0	0 / 0	0 / 0	0 / 0	261 / 0	0 / 0
K	844	567 / 0	0 / 0	0 / 0	0 / 0	277 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.99 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.
1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, E-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

CHORDS				WEBS			
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-91.8 -91.8	0.12 (1)	J-C	-70 / 66	0.02 (4)	
B-C	-1384 / 0	-91.8 -91.8	0.45 (1)	C-H	-629 / 0	0.61 (1)	
C-D	-855 / 0	-91.8 -91.8	0.42 (1)	H-D	0 / 69	0.02 (4)	
D-E	-738 / 0	-91.8 -91.8	0.17 (1)	H-E	0 / 512	0.12 (1)	
E-F	0 / 0	-91.8 -91.8	0.22 (1)	E-G	-1012 / 0	0.41 (1)	
G-F	-134 / 0	0.0 0.0	0.03 (1)	B-J	0 / 1276	0.29 (1)	
K-B	-1148 / 0	0.0 0.0	0.12 (1)				
K-J	0 / 0	-18.5 -18.5	0.14 (4)				
J-I	0 / 1264	-18.5 -18.5	0.33 (1)				
I-H	0 / 1264	-18.5 -18.5	0.33 (1)				
H-G	0 / 496	-18.5 -18.5	0.26 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CSI: TC=0.45/1.00 (B-C:1), BC=0.33/1.00 (H-J:1), WB=0.61/1.00 (C-H:1), SS=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.74 (J) (INPUT = 0.90)
JSI METAL= 0.37 (I) (INPUT = 1.00)

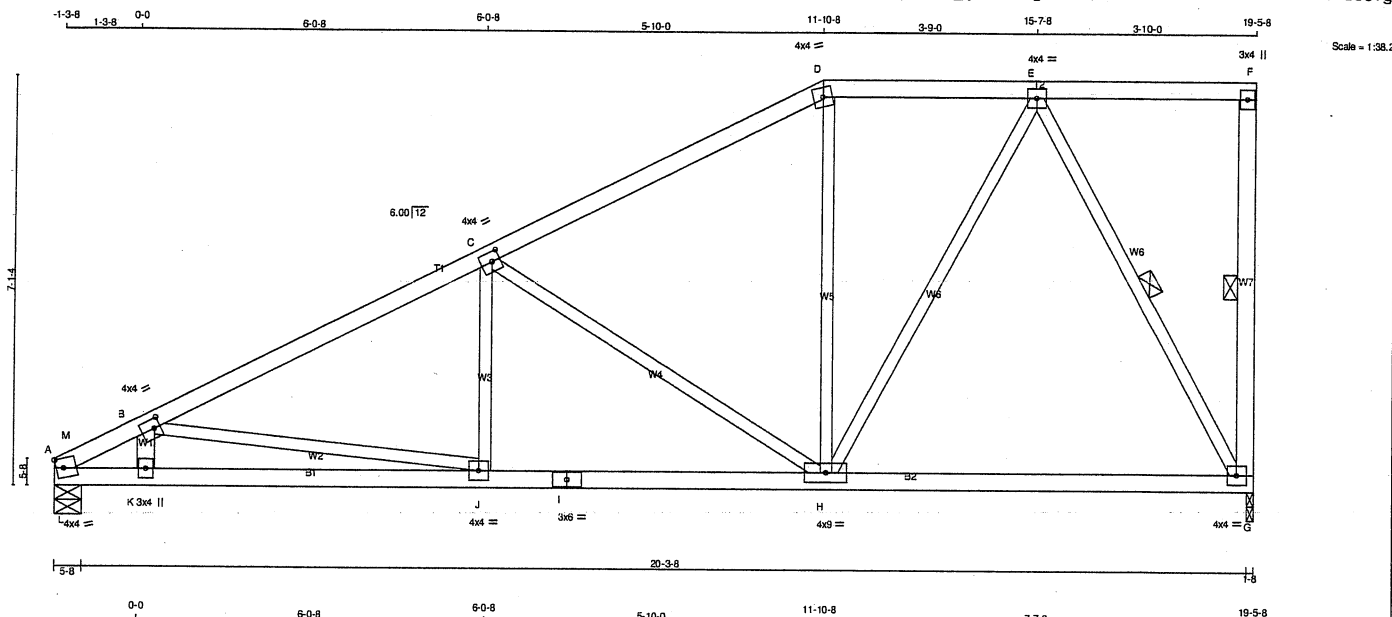


Structural component only
DWG# T-2115858

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
413226	T104X	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER					
N. L. G. A. RULES					
CHORDS		SIZE	LUMBER		DESCR
A - D	2x4	DRY	No.2		SPF
D - F	2x4	DRY	No.2		SPF
G - F	2x4	DRY	No.2		SPF
K - B	2x4	DRY	No.2		SPF
A - I	2x4	DRY	No.2		SPF
I - G	2x4	DRY	No.2		SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-m	MT20	4.0	4.0		Edge
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TMVW-t	MT20	4.0	4.0	2.00	1.75
D	TMVW-t	MT20	4.0	4.0		
E	TMVW-t	MT20	4.0	4.0		
F	TMVW-t	MT20	4.0	4.0		
G	BMVW-t	MT20	4.0	4.0		
H	BMVW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMVW-t	MT20	4.0	4.0		
K	BMVW-t	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
A	1151	0	1151	0	5-8	5-8
G	1151	0	1151	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS		DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE		
A	814	534 / 0	0 / 0	0 / 0	280 / 0	0 / 0
G	814	534 / 0	0 / 0	0 / 0	280 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.24 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, E-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. VERT. LOAD LC1	MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. VERT. LOAD LC1	MAX. CSI (LC)
FR-TO		FROM TO				FR-TO			
A-M	-2504 / 0	-91.8	-91.8	0.16 (1)	4.24	J-C	0 / 169	0.05 (4)	
M-B	-2072 / 0	-91.8	-91.8	0.17 (1)	4.57	C-H	-846 / 0	0.82 (1)	
B-C	-1693 / 0	-91.8	-91.8	0.44 (1)	4.64	H-D	0 / 84	0.03 (4)	
C-D	-947 / 0	-91.8	-91.8	0.40 (1)	5.84	H-E	0 / 601	0.14 (1)	
D-E	-823 / 0	-91.8	-91.8	0.17 (1)	6.25	E-G	-1101 / 0	0.44 (1)	
E-F	0 / 0	-91.8	-91.8	0.22 (1)	10.00	B-J	-335 / 0	0.21 (1)	
G-F	-134 / 0	0.0	0.0	0.03 (1)	6.25	L-M	0 / 721	0.00 (1)	
K-B	-100 / 62	0.0	0.0	0.02 (4)	7.81				
A-L	0 / 1862	-18.5	-18.5	0.48 (1)	10.00				
L-K	0 / 1862	-18.5	-18.5	0.48 (1)	10.00				
K-J	0 / 1862	-18.5	-18.5	0.42 (1)	10.00				
J-I	0 / 1531	-18.5	-18.5	0.37 (1)	10.00				
I-H	0 / 1531	-18.5	-18.5	0.37 (1)	10.00				
H-G	0 / 540	-18.5	-18.5	0.27 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL) = L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/999 (0.14")

CSI: TC=0.44/1.00 (B-C:1), BC=0.48/1.00 (A-L:1), WB=0.82/1.00 (C-H:1), SS=0.44/1.00 (A-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (A) (INPUT = 0.90)
JSI METAL= 0.70 (A) (INPUT = 1.00)

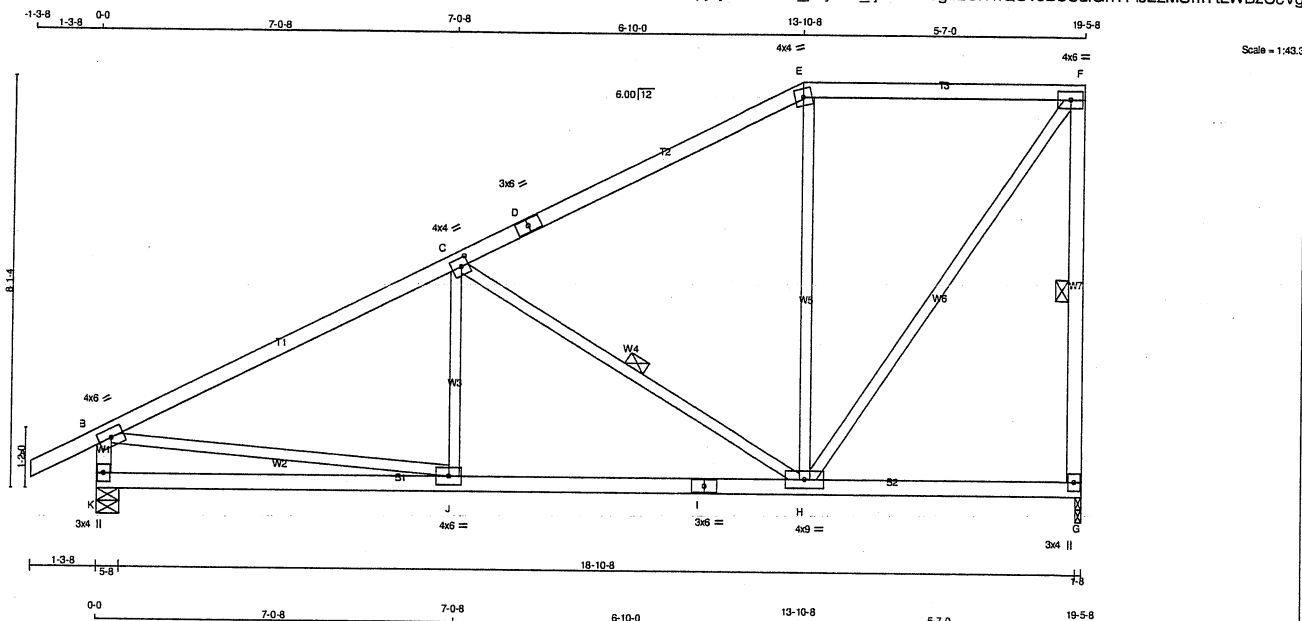


Structural component only
DWG# T-2115859

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
413226	T105	10	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
E - F	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
K - B	2x4	DRY	No.2	SPF	
K - I	2x4	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-t	MT20	4.0	6.0		
C TMVW-t	MT20	4.0	4.0	2.00	1.75
D TS-t	MT20	3.0	6.0		
E TTW-m	MT20	4.0	4.0		
F TMVW-t	MT20	4.0	6.0		
G BMV1+p	MT20	3.0	4.0		
H BMVWW-t	MT20	4.0	9.0		
I BS-t	MT20	3.0	6.0		
J BMVW-t	MT20	4.0	6.0		
K BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED		MAXIMUM FACTORED		INPUT	REQRD
	GROSS REACTION	GROSS REACTION	DOWN	HORIZ		
JT	VERT	HORIZ	DOWN	HORIZ	UPLIFT	IN-SX
G	1073	0	1073	0	0	1-8
K	1197	0	1197	0	0	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN.	COMPONENT REACTIONS					
G	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	
K	759	498 / 0	0 / 0	0 / 0	0 / 0	261 / 0	0 / 0	
	844	567 / 0	0 / 0	0 / 0	0 / 0	277 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.69 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, C-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-91.8 -91.8	0.12 (1)	10.00	J-C	-8 / 113	0.04 (4)
B-C	-1355 / 0	-91.8 -91.8	0.64 (1)	4.69	C-H	-821 / 0	0.38 (1)
C-D	-649 / 0	-91.8 -91.8	0.58 (1)	6.25	H-E	-216 / 6	0.27 (1)
D-E	-649 / 0	-91.8 -91.8	0.58 (1)	6.25	H-F	0 / 951	0.21 (1)
E-F	-546 / 0	-91.8 -91.8	0.37 (1)	6.25	B-J	0 / 1252	0.28 (1)
G-F	-1034 / 0	0.0 0.0	0.31 (1)	6.18			
K-B	-1144 / 0	0.0 0.0	0.12 (1)	7.43			
K-J	0 / 0	-18.5 -18.5	0.22 (4)	10.00			
J-I	0 / 1243	-18.5 -18.5	0.32 (4)	10.00			
I-H	0 / 1243	-18.5 -18.5	0.32 (4)	10.00			
H-G	0 / 0	-18.5 -18.5	0.15 (4)	10.00			

TOTAL WEIGHT = 10 X 86 = 862 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.65")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.04")
ALLOWABLE DEFL.(TL) = $L/360$ (0.65")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.10")

CSI: TC=0.64/1.00 (B-C:1), BC=0.32/1.00 (H-J:4), WB=0.38/1.00 (C-H:1), SS=0.28/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

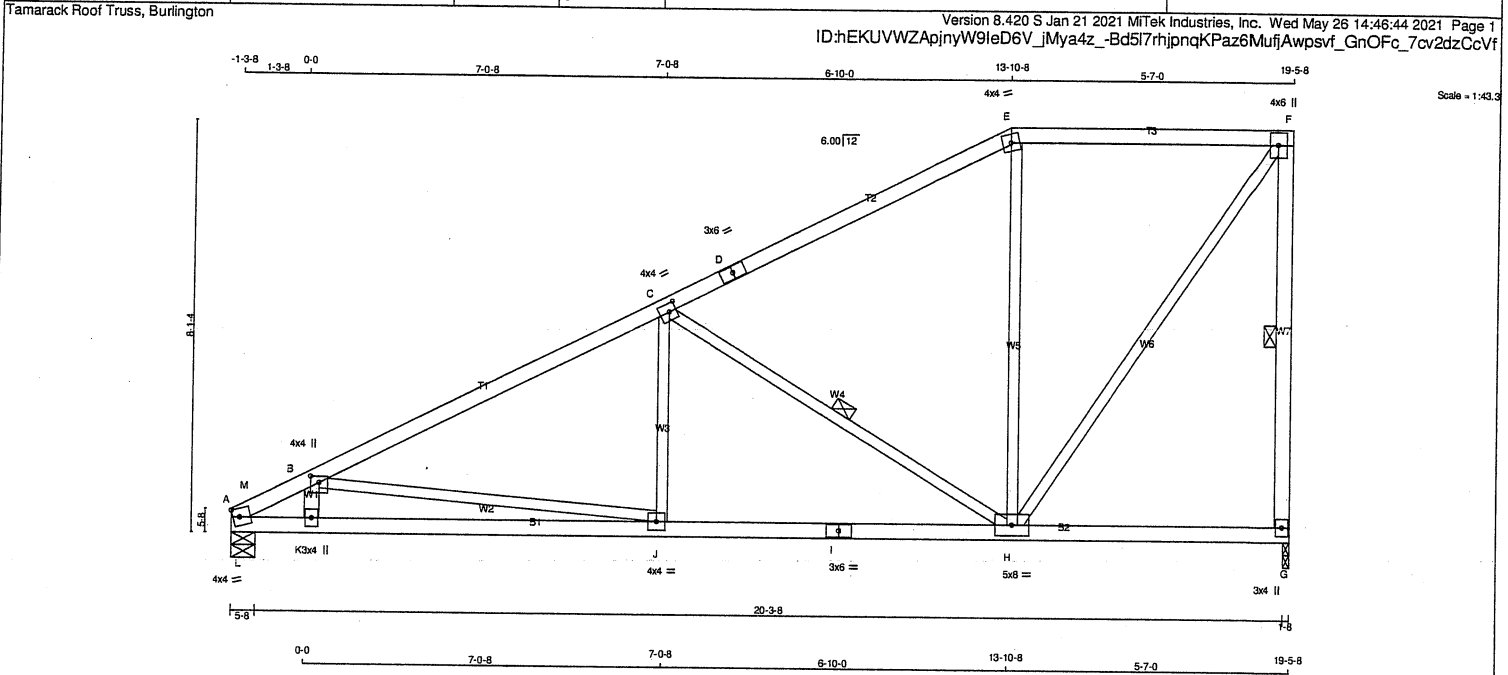
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (H) (INPUT = 0.90)
JSI METAL= 0.41 (I) (INPUT = 1.00)



Structural component only
DWG# T-2115860

JOB NAME 413226	TRUSS NAME T105X	QUANTITY 5	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
TRUSS DESC.					



LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER										TOTAL WEIGHT = 5 X 88 = 439 lb			
N. L. G. A. RULES				BEARINGS										[M][F]			
CHORDS		SIZE	LUMBER	DESCR.	FACTORED		MAXIMUM FACTORED		INPUT		REQRD		DESIGN CRITERIA				
A - D		2x4	DRY	No.2	SPF	GROSS REACTION		GROSS REACTION		BRG		BRG		SPECIFIED LOADS:			
D - E		2x4	DRY	No.2	SPF	JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	TOP CH. LL = 25.6 PSF			
E - F		2x4	DRY	No.2	SPF	G	1151	0	1151	0	0	1-8	1-8	DL = 6.0 PSF			
G - F		2x4	DRY	No.2	SPF	A	1151	0	1151	0	0	5-8	5-8	BOT CH. LL = 0.0 PSF			
K - B		2x4	DRY	No.2	SPF											DL = 7.4 PSF	
A - I		2x4	DRY	No.2	SPF											TOTAL LOAD = 39.0 PSF	
I - G		2x4	DRY	No.2	SPF												
ALL WEBS		2x3	DRY	No.2	SPF	UNFACTORED REACTIONS										SPACING = 24.0 IN. C/C	
EXCEPT						1ST LCASE MAX./MIN. COMPONENT REACTIONS											
						JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	LOADING IN FLAT SECTION BASED ON A SLOPE			
						G	814	534 / 0	0 / 0	0 / 0	0 / 0	280 / 0	0 / 0	OF 2.00/12 MINIMUM			
						A	814	534 / 0	0 / 0	0 / 0	0 / 0	280 / 0	0 / 0				
DRY: SEASONED LUMBER.																	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-m	MT20	4.0	4.0		Edge
B	TMVW+p	MT20	4.0	4.0	1.50	2.00
C	TMVW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
E	TTW-m	MT20	4.0	4.0		
F	TMVW+p	MT20	4.0	4.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMVW-t	MT20	5.0	8.0		
I	BS-t	MT20	3.0	6.0		
J	BMVW-t	MT20	4.0	4.0		
K	BMV-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.13 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, C-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-M	-2641 / 0	-91.8 -91.8	0.17 (1)	J-C	0 / 214	0.06 (4)	
M-B	-2142 / 0	-91.8 -91.8	0.25 (1)	C-H	-1010 / 15	0.46 (1)	
B-C	-1610 / 0	-91.8 -91.8	0.60 (1)	H-E	-196 / 15	0.25 (1)	
C-D	-706 / 0	-91.8 -91.8	0.55 (1)	H-F	0 / 1045	0.24 (1)	
D-E	-706 / 0	-91.8 -91.8	0.55 (1)	B-J	-496 / 0	0.46 (1)	
E-F	-600 / 0	-91.8 -91.8	0.37 (1)	L-M	0 / 845	0.00 (1)	
G-F	-1111 / 0	0.0 0.0	0.33 (1)				
K-B	-112 / 75	0.0 0.0	0.02 (4)				
A-L	0 / 1950	-18.5 -18.5	0.53 (1)				
L-K	0 / 1950	-18.5 -18.5	0.53 (1)				
K-J	0 / 1950	-18.5 -18.5	0.45 (1)				
J-I	0 / 1457	-18.5 -18.5	0.37 (1)				
I-H	0 / 1457	-18.5 -18.5	0.37 (1)				
H-G	0 / 0	-18.5 -18.5	0.16 (4)				

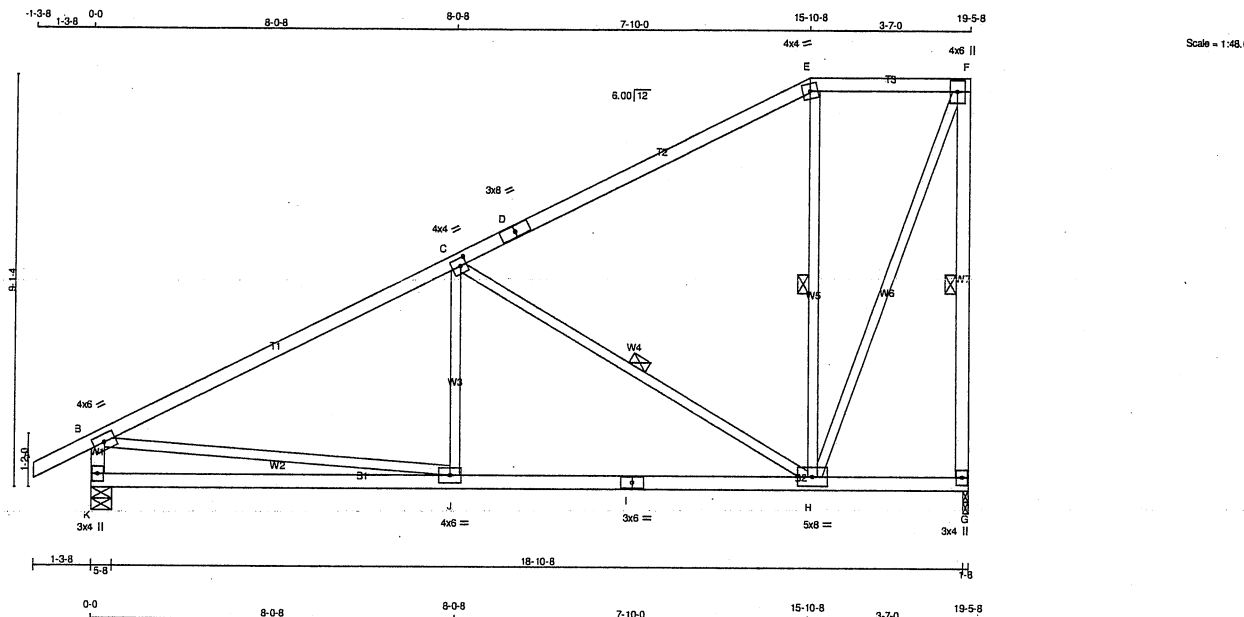


Structural component only
DWG# T-2115861

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
413226	T106	3	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 3 X 91 = 272 lb
[M][F]

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
G - H	2x4	DRY	No.2
K - B	2x4	DRY	No.2
K - I	2x4	DRY	No.2
I - G	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			SPF
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMVW-t	MT20	4.0	6.0
C	TMWW-t	MT20	4.0	4.0 2.00 1.75
D	TS-t	MT20	3.0	8.0
E	TTW-m	MT20	4.0	4.0
F	TMVW-p	MT20	4.0	6.0
G	BMV1-p	MT20	3.0	4.0
H	BMWW-t	MT20	5.0	8.0
I	BS-t	MT20	3.0	6.0
J	BMWW-t	MT20	4.0	6.0
K	BMV1-p	MT20	3.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
G	1073	0	1073	0	1-8
K	1197	0	1197	0	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
G	759	498 / 0	0 / 0	0 / 0	0 / 0	261 / 0	0 / 0
K	844	567 / 0	0 / 0	0 / 0	0 / 0	277 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.15 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, C-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX UNBRACED LENGTH FR-TO	MAX LC1 (LC)
A-B	0 / 28	-91.8	-91.8 0.12 (1)	J-C	0 / 149	10.00	0.05 (4)
B-C	-1311 / 0	-91.8	-91.8 0.86 (1)	C-H	-1000 / 0	4.15	0.61 (1)
C-D	-439 / 0	-91.8	-91.8 0.77 (1)	H-E	-255 / 0	6.25	0.14 (1)
D-E	-439 / 0	-91.8	-91.8 0.77 (1)	H-F	0 / 961	6.25	0.22 (1)
E-F	-353 / 0	-91.8	-91.8 0.15 (1)	B-J	0 / 1214	6.13	0.27 (1)
G-F	-1057 / 0	0.0	0.0 0.41 (1)			7.45	
K-B	-1138 / 0	0.0	0.0 0.11 (1)				
K-J	0 / 0	-18.5	-18.5 0.30 (4)			10.00	
J-I	0 / 1208	-18.5	-18.5 0.40 (4)			10.00	
I-H	0 / 1208	-18.5	-18.5 0.40 (4)			10.00	
H-G	0 / 0	-18.5	-18.5 0.15 (4)			10.00	

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/999 (0.14")

CSI: TC=0.86/1.00 (B-C:1), BC=0.40/1.00 (H-J:4), WB=0.61/1.00 (C-H:1), SS=0.32/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

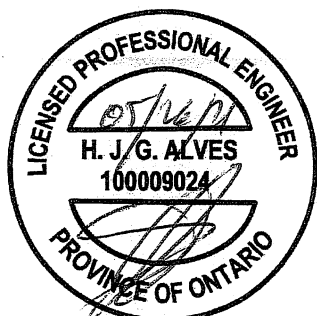
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)	
MAX	MIN	MAX	MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (H) (INPUT = 0.90)
JSI METAL= 0.49 (I) (INPUT = 1.00)

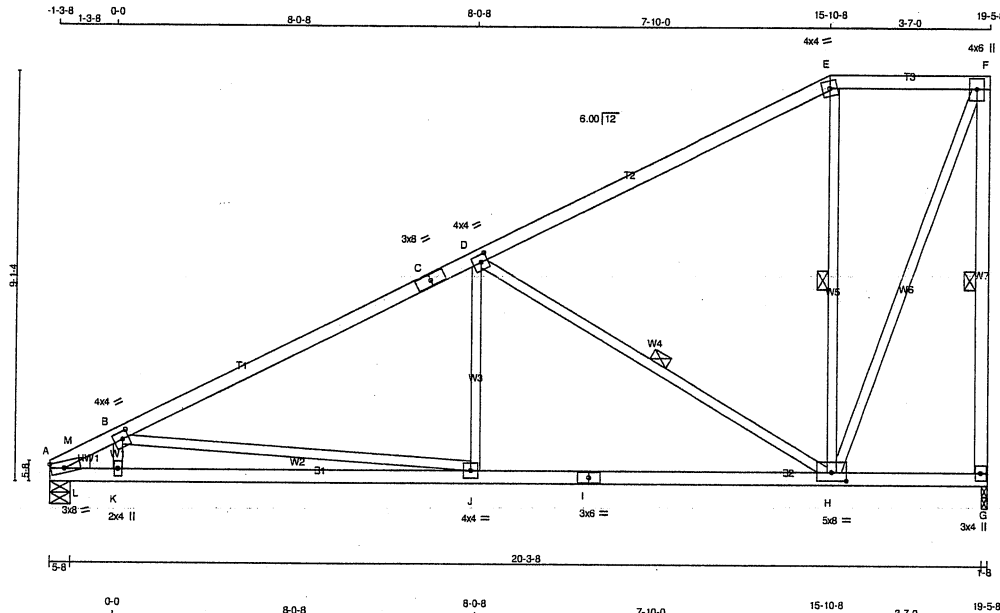


Structural component only
DWG# T-2115862

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
413226	T106X	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 93 lb [M/F]

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
G - F	2x4	DRY	No.2
A - I	2x4	DRY	No.2
I - G	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
A	TBMH1-m	MT20	3.0	8.0 1.75 Edge
B	TMW-w-t	MT20	4.0	4.0 2.00 1.75
C	TS-t	MT20	3.0	8.0
D	TMW-w-t	MT20	4.0	4.0 2.00 1.75
E	TTW-m	MT20	4.0	4.0
F	TMVW-p	MT20	4.0	6.0
G	BMV1-p	MT20	3.0	4.0
H	BMW-w-t	MT20	5.0	8.0 2.25 4.00
I	BS-t	MT20	3.0	6.0
J	BMW-w-t	MT20	4.0	4.0
K	BMW-w	MT20	2.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD		HEEL	
JT	VERT	GROSS REACTION	HORZ	DOWN	HORZ	UP	IN-SX	BRG	IN-SX	WEDGE	
G	1151	0	1151	0	0	1-8	1-8				
A	1151	0	1151	0	0	5-8	5-8			2x4 L	

UNFACTORED REACTIONS		1ST LCASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL		
G	814	534 / 0	0 / 0	0 / 0	0 / 0	280 / 0	0 / 0		
A	814	534 / 0	0 / 0	0 / 0	0 / 0	280 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, A

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.24 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX UNBRACED LENGTH FR-TO	MAX LC1 CSI (LC)
FR-TO		FROM TO		FR-TO			
A-M	-2452 / 0	-91.8 -91.8	0.15 (1)	J-D	0 / 254	4.28	0.07 (4)
M-B	-2188 / 0	-91.8 -91.8	0.34 (1)	D-H	-1163 / 0	4.27	0.71 (1)
B-C	-1519 / 0	-91.8 -91.8	0.77 (1)	H-E	-250 / 0	4.24	0.13 (1)
C-D	-1519 / 0	-91.8 -91.8	0.77 (1)	H-F	0 / 1044	4.24	0.23 (1)
D-E	-468 / 0	-91.8 -91.8	0.72 (1)	K-B	-171 / 94	6.25	0.03 (4)
E-F	-383 / 0	-91.8 -91.8	0.15 (1)	B-J	-674 / 0	6.25	0.93 (1)
G-F	-1134 / 0	0.0 0.0	0.44 (1)	L-M	0 / 486	5.97	0.00 (1)
A-L	0 / 2047	-18.5 -18.5	0.55 (1)				
L-K	0 / 2047	-18.5 -18.5	0.55 (1)				
K-J	0 / 2047	-18.5 -18.5	0.51 (1)				
J-I	0 / 1377	-18.5 -18.5	0.40 (1)				
I-H	0 / 1377	-18.5 -18.5	0.40 (1)				
H-G	0 / 0	-18.5 -18.5	0.16 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD = 39.0 PSF				

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/999 (0.21")

CSI: TC=0.77/1.00 (B-D:1), BC=0.55/1.00 (A-L:1), WB=0.93/1.00 (B-J:1), SSI=0.31/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE GRIP(DRY)	SHEAR	SECTION	(PSI)	(PLI)	
MT20	650	371	1747	788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

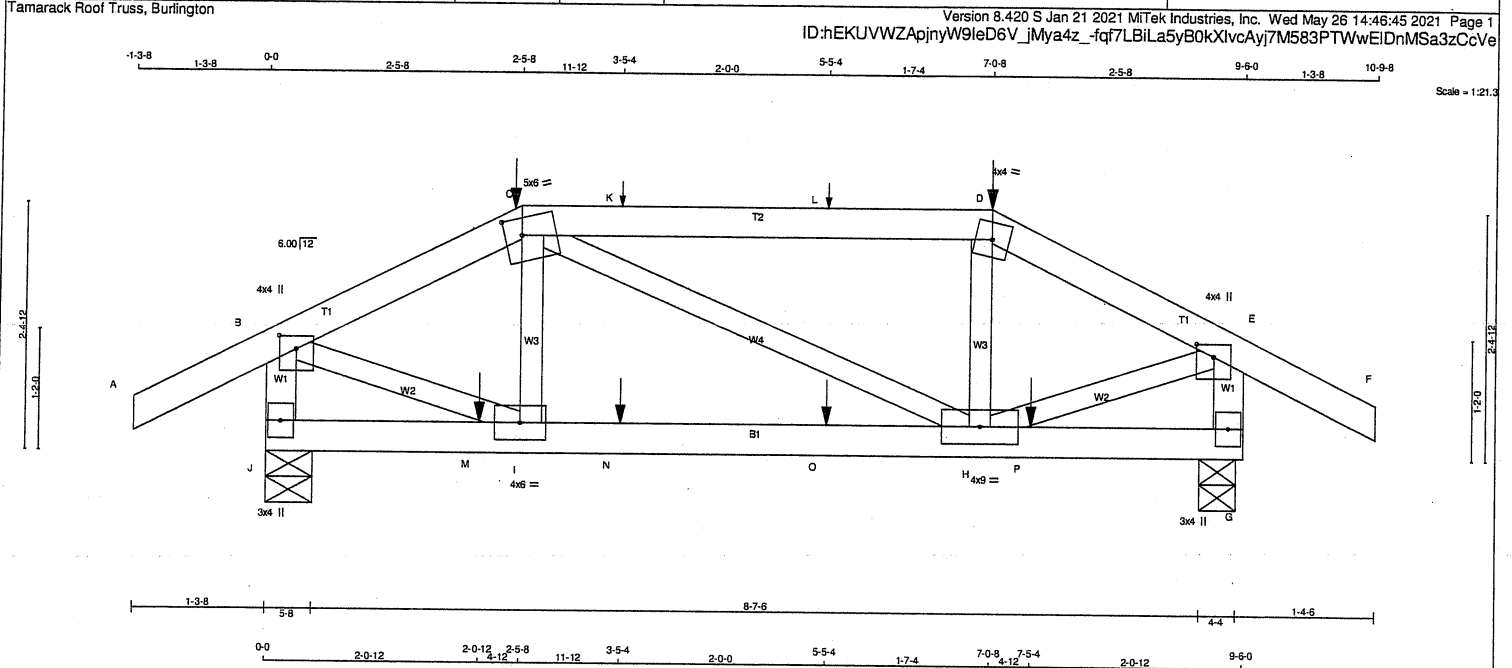
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.44 (I) (INPUT = 1.00)



Structural component only
DWG# T-2115863

JOB NAME 413226	TRUSS NAME T107	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Wed May 26 14:46:45 2021 Page 1 ID:hEKUVWZApinyW9leD6V_jMya4z_-fqf7LBiLa5yB0kXlvcAy7M583PTWwEIdnMSa3zCcVe			



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. TOTAL WEIGHT = 38 lb

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
J - B	2x4	DRY	No.2
G - E	2x4	DRY	No.2
J - G	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.50	2.00
C	TTWW-m	MT20	5.0	6.0	2.00	2.00
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	4.0	1.50	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	9.0		
I	BMWW-t	MT20	4.0	6.0		
J	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS								
FACTORED			MAXIMUM FACTORED			INPUT	REQRD	
GROSS REACTION			GROSS REACTION			BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
J	762	0	762	0	0	5-8	5-8	
G	762	0	762	0	0	4-4	4-4	

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN	COMPONENT REACTIONS
J	535	373 / 0	0 / 0
G	535	373 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX		MAX		WEBS		FACTORED		MAX	
MEMB.	FORCE (LBS)	VERT.	PLF	LC1	MAX	UNBRAC	MAX	MEMB.	FORCE (LBS)	MAX	CS1 (LC)	MEMB.	FORCE (LBS)
FR-TO								FR-TO				FR-TO	
A-B	0 / 28	-91.8	-91.8	0.13 (1)	10.00	I-C	-126 / 22	0.02 (1)				I-C	-126 / 22
B-C	-678 / 0	-91.8	-91.8	0.11 (1)	6.25	C-H	0 / 0	0.00 (1)				C-H	0 / 0
C-K	-598 / 0	-91.8	-91.8	0.37 (1)	6.25	H-D	-127 / 21	0.02 (1)				H-D	-127 / 21
K-L	-598 / 0	-91.8	-91.8	0.37 (1)	6.25	B-I	0 / 638	0.16 (1)				B-I	0 / 638
L-D	-598 / 0	-91.8	-91.8	0.37 (1)	6.25	H-E	0 / 638	0.16 (1)				H-E	0 / 638
D-E	-677 / 0	-91.8	-91.8	0.11 (1)	6.25								
E-F	0 / 28	-91.8	-91.8	0.13 (1)	10.00								
J-B	-745 / 0	0.0	0.0	0.08 (1)	7.81								
J-G	-744 / 0	0.0	0.0	0.08 (1)	7.81								
J-M	0 / 0	-18.5	-18.5	0.06 (4)	10.00								
M-I	0 / 0	-18.5	-18.5	0.06 (4)	10.00								
I-N	0 / 599	-18.5	-18.5	0.15 (1)	10.00								
N-O	0 / 599	-18.5	-18.5	0.15 (1)	10.00								
O-H	0 / 599	-18.5	-18.5	0.15 (1)	10.00								
H-P	0 / 0	-18.5	-18.5	0.06 (4)	10.00								
P-G	0 / 0	-18.5	-18.5	0.06 (4)	10.00								

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-5-8	-74	-74	---	FRONT	VERT	TOTAL	---	C1
D	7-0-8	-74	-74	---	FRONT	VERT	TOTAL	---	C1
K	3-5-4	1	1	---	FRONT	VERT	TOTAL	---	C1
L	5-5-4	1	1	---	FRONT	VERT	TOTAL	---	C1
M	2-0-12	-3	-3	---	FRONT	VERT	TOTAL	---	C1
N	3-5-4	-1	-1	---	FRONT	VERT	TOTAL	---	C1
O	5-5-4	-1	-1	---	FRONT	VERT	TOTAL	---	C1
P	7-5-4	-3	-3	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.32")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.32")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.37/1.00 (C-D:1), BC=0.15/1.00 (H-I:1), WB=0.16/1.00 (B-I:1), SS=0.18/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR	SECTION
(PSI)	(PLI)
MAX MIN MAX MIN MAX MIN	
MT20 650 371 1747 788 1987 1873	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.65 (B) (INPUT = 0.90)

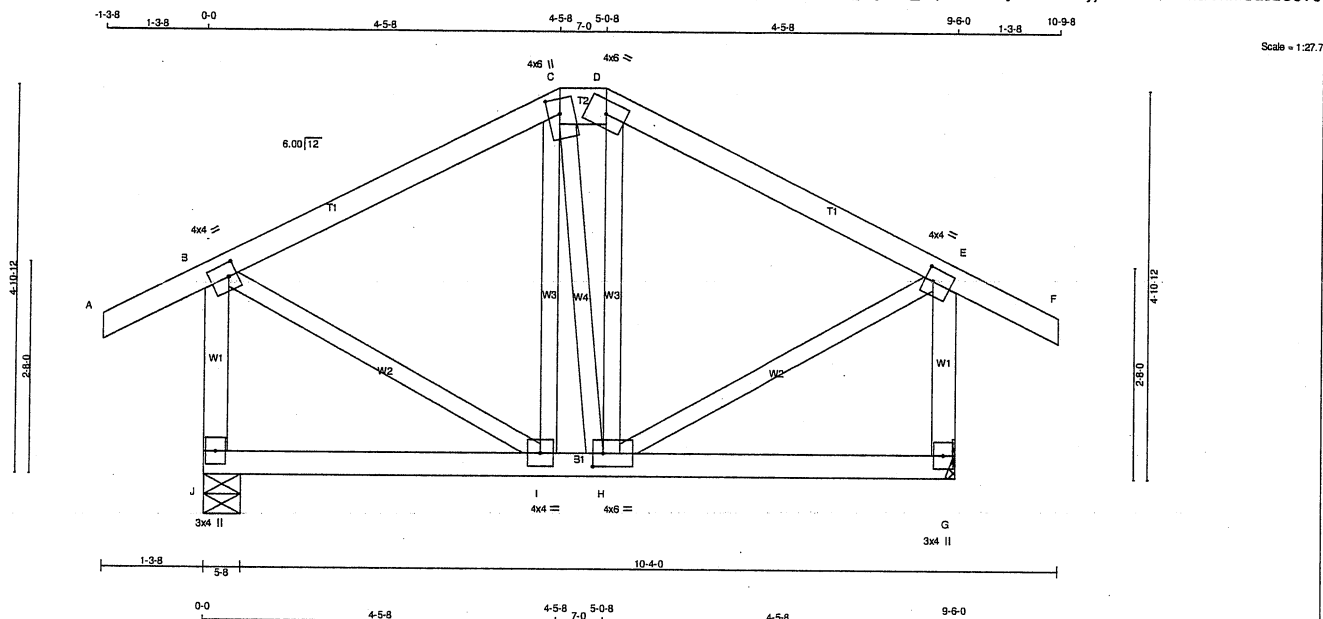
JSI METAL= 0.20 (B) (INPUT = 1.00)



Structural component only
DWG# T-2115864

JOB NAME 413226	TRUSS NAME T108	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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TOTAL WEIGHT = 2 X 50 = 99 lb
[M][F]

LUMBER	N L G A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - D	2x6	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
J - B	2x4	DRY	No.2	SPF	
G - E	2x4	DRY	No.2	SPF	
J - G	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TTWW+m	MT20	4.0	6.0	2.25	1.75
D	TTW-h	MT20	4.0	6.0		
E	TMVW-t	MT20	4.0	4.0	2.00	1.25
G	BMV1+p	MT20	3.0	4.0		
H	BMVWW-t	MT20	4.0	6.0	2.00	1.50
I	BMVWW-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQRD
JT	VERT	HORZ	DOWN	HORZ
J	648	0	648	0
G	648	0	648	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
J	456	312 / 0	0 / 0	0 / 0	0 / 0	0 / 0	144 / 0	0 / 0
G	456	312 / 0	0 / 0	0 / 0	0 / 0	0 / 0	144 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	LC2 MAX	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO						FR-TO			
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	I-C	-88 / 7	0.03 (1)	
B-C	-309 / 0	-91.8	-91.8	0.23 (1)	6.25	H-D	-96 / 7	0.03 (1)	
C-D	-278 / 0	-91.8	-91.8	0.00 (1)	6.25	B-I	0 / 315	0.07 (1)	
D-E	-307 / 0	-91.8	-91.8	0.23 (1)	6.25	H-E	0 / 314	0.07 (1)	
E-F	0 / 28	-91.8	-91.8	0.12 (1)	10.00	C-H	-7 / 0	0.00 (1)	
J-B	-615 / 0	0.0	0.0	0.09 (1)	7.81				
G-E	-614 / 0	0.0	0.0	0.09 (1)	7.81				
J-I	0 / 0	-18.5	-18.5	0.08 (4)	10.00				
I-H	0 / 279	-18.5	-18.5	0.10 (4)	10.00				
H-G	0 / 0	-18.5	-18.5	0.08 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.32")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.32")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.23/1.00 (B-C:1), BC=0.10/1.00 (H-I:4), WB=0.07/1.00 (B-I:1), SSI=0.14/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.45 (B) (INPUT = 0.90)

JSI METAL = 0.16 (B) (INPUT = 1.00)

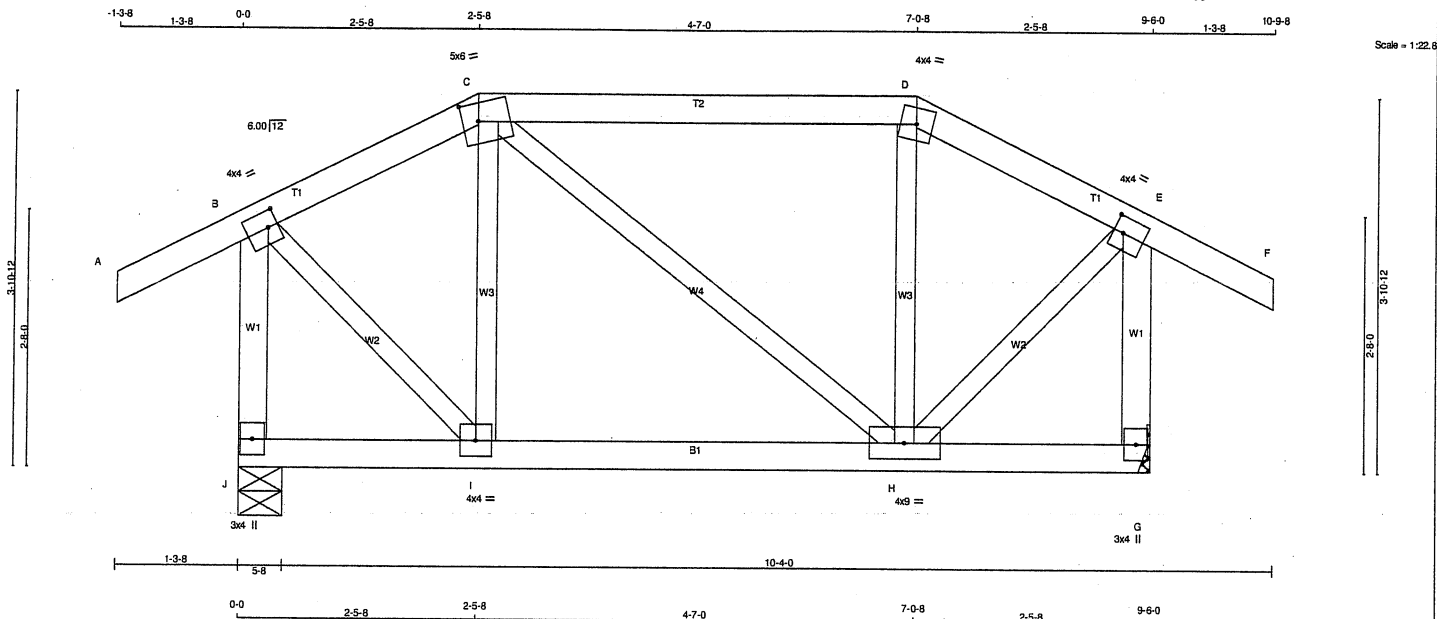


Structural component only
DWG# T-2115865

JOB NAME 413226	TRUSS NAME T109	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 46 lb
[M][F]

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4 DRY	No.2	SPF
C - D	2x4 DRY	No.2	SPF
D - F	2x4 DRY	No.2	SPF
J - B	2x4 DRY	No.2	SPF
G - E	2x4 DRY	No.2	SPF
J - G	2x4 DRY	No.2	SPF
ALL WEBS EXCEPT	2x3 DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00 1.25
C	TTWW-m	MT20	5.0	5.0	2.25 2.00
D	TTW-m	MT20	4.0	4.0	
E	TMVW-t	MT20	4.0	4.0	2.00 1.25
G	BMV1+p	MT20	3.0	4.0	
H	BMVWW-t	MT20	4.0	9.0	
I	BMVW-t	MT20	4.0	4.0	
J	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	IN-SX	BRG	IN-SX
J	648	0	648	0	0	5-8	5-8		
G	648	0	648	0	0	MECHANICAL			

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	456	312 / 0	0 / 0	0 / 0	0 / 0	144 / 0	0 / 0
G	456	312 / 0	0 / 0	0 / 0	0 / 0	144 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX. FACTORED		WEBS		FACTORED		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX	UNBRAC	MEMB.	FORCE (LBS)	MAX	UNBRAC	MEMB.	FORCE (LBS)
FR-TO		FROM	TO	LENGTH	FR-TO						
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	I-C	-183 / 0	0.04 (1)			
B-C	-311 / 0	-91.8	-91.8	0.07 (1)	6.25	C-H	0 / 0	0.00 (1)			
C-D	-272 / 0	-91.8	-91.8	0.25 (1)	6.25	H-D	-183 / 0	0.04 (1)			
D-E	-311 / 0	-91.8	-91.8	0.07 (1)	6.25	B-I	0 / 378	0.09 (1)			
E-F	0 / 28	-91.8	-91.8	0.12 (1)	10.00	H-E	0 / 377	0.08 (1)			
J-B	-633 / 0	0.0	0.0	0.09 (1)	7.81						
G-E	-632 / 0	0.0	0.0	0.09 (1)	7.81						
J-I	0 / 0	-18.5	-18.5	0.06 (4)	10.00						
I-H	0 / 273	-18.5	-18.5	0.08 (4)	10.00						
H-G	0 / 0	-18.5	-18.5	0.06 (4)	10.00						

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCG 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.32")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.32")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.25/1.00 (C-D:1), BC=0.08/1.00 (H-I:4), WB=0.09/1.00 (B-I:1), SSI=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

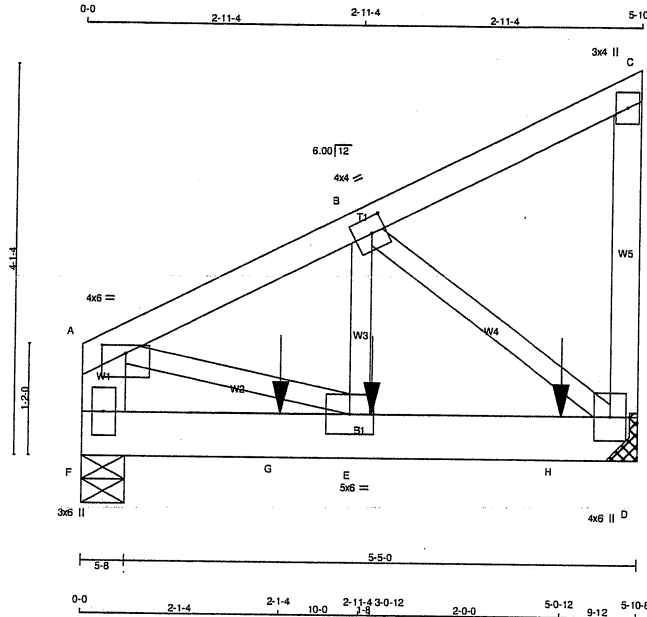
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.52 (B) (INPUT = 0.90)
JSI METAL= 0.14 (B) (INPUT = 1.00)



Structural component only
DWG# T-2115866

JOB NAME 413226	TRUSS NAME T110	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.		Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed May 26 14:46:46 2021 Page 1 ID:hEKUVVWZApjnyW9leD6V_jMya4z_-70DVYXjzLP42et6UTJiBFLuLpSksFNjvSR507WzCcVd	



LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x6	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122\"X3\") SPIRAL NAILS		
A-C 1	12	TOP
C-D 1	12	TOP
F-A 2	12	TOP
BOTTOM CHORDS : (0.122\"X3\") SPIRAL NAILS		
F-D 2	12	SIDE(183.1)
WEBS : (0.122\"X3\") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00	3.00
B	TMVW-t	MT20	4.0	4.0	2.00	1.75
C	TMV-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG	
	VERT	HORZ	DOWN	HORZ		IN-SX	IN-SX
D	1408	0	1408	0	0	MECHANICAL	
F	1130	0	1130	0	5-8	MECHANICAL	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM.LIVE	WIND			
D	989	688 / 0	0 / 0	0 / 0	0 / 0	301 / 0	0 / 0
F	794	550 / 0	0 / 0	0 / 0	0 / 0	244 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)		FACTORED VERT. LOAD LC1 MAX (PLF)		MAX. CSI (LC)	WEBS MAX. FACTORED FORCE (LBS)		MAX. CSI (LC)
	FR-TO	FROM	TO	TO		FR-TO	FROM	
A-B	-1250 / 0	-91.8	-91.8	0.06 (1)	6.25	E-B	0 / 1106	0.14 (1)
B-C	-11 / 0	-91.8	-91.8	0.05 (1)	6.25	B-D	-1419 / 0	0.17 (1)
D-C	-110 / 0	0.0	0.0	0.01 (1)	7.81	A-E	0 / 1166	0.14 (1)
F-A	-971 / 0	0.0	0.0	0.03 (1)	7.81			
F-G	0 / 0	-18.5	-18.5	0.11 (1)	10.00			
G-E	0 / 0	-18.5	-18.5	0.11 (1)	10.00			
E-H	0 / 1128	-18.5	-18.5	0.20 (1)	10.00			
H-D	0 / 1128	-18.5	-18.5	0.20 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	3-0-12	-441	-441	---	BACK	VERT	TOTAL	---	C1
G	2-1-4	-441	-441	---	BACK	VERT	TOTAL	---	C1
H	5-0-12	-443	-443	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

- C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 29 = 58 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	25.6	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL(LL)= L/360 (0.20")
CALCULATED VERT. DEFL(LL) = L/999 (0.01")
ALLOWABLE DEFL(TL)= L/360 (0.20")
CALCULATED VERT. DEFL(TL) = L/999 (0.01")

CSI: TC=0.06/1.00 (A-B:1), BC=0.20/1.00 (D-E:1),
WB=0.17/1.00 (B-D:1), SSI=0.17/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.72 (B) (INPUT = 0.90)

JSI METAL= 0.25 (D) (INPUT = 1.00)



Structural component only
DWG# T-2115867

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
413226	T110	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
D	BMVW1+p	MT20	4.0	6.0		
E	BMWW-t	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	6.0		

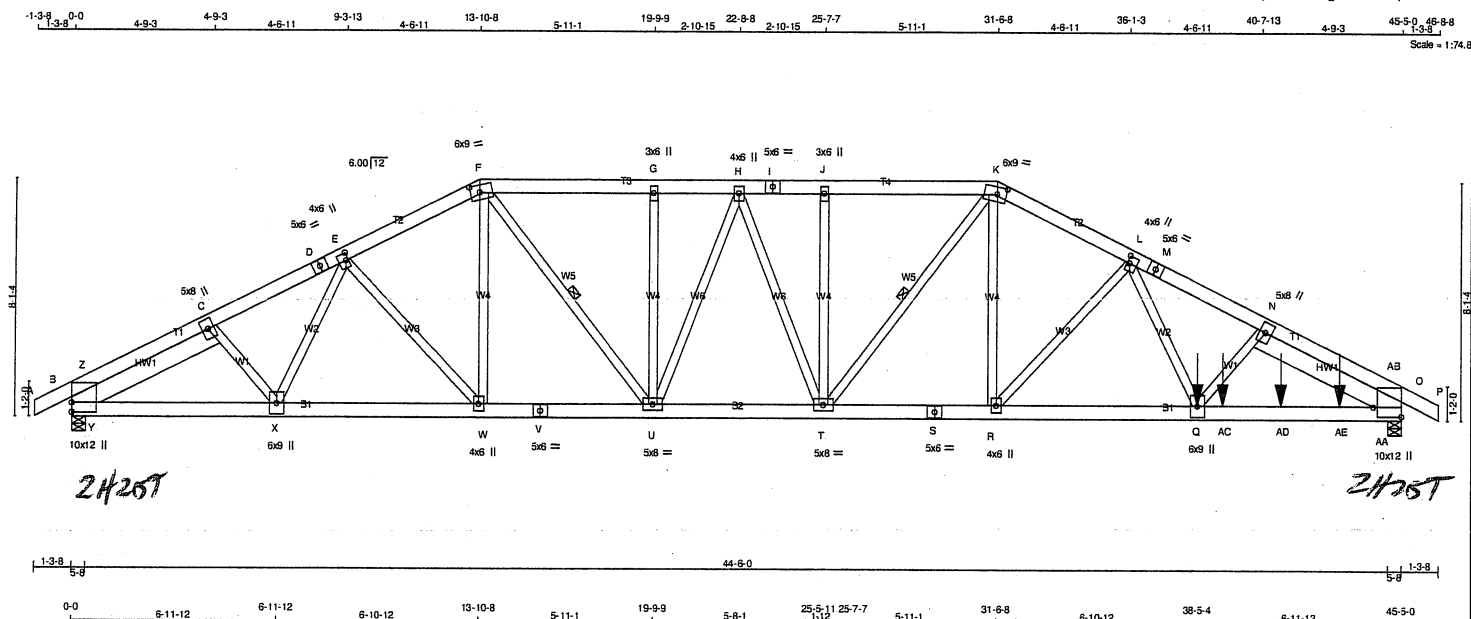


Structural component only
DWG# T-2115867 *in*

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
413226	T111	1	3	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed May 26 17:24:30 2021 Page 1
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LUMBER	SIZE	DESCR.
N. L. G. A. RULES		
CHORDS		
A - D	2x6 DRY No.2	SPF
D - F	2x6 DRY No.2	SPF
F - I	2x6 DRY No.2	SPF
I - K	2x6 DRY No.2	SPF
K - M	2x6 DRY No.2	SPF
M - P	2x6 DRY No.2	SPF
B - V	2x6 DRY No.2	SPF
V - S	2x6 DRY No.2	SPF
S - O	2x6 DRY No.2	SPF
REINFORCING MEMBERS		
HW1	2x8 DRY No.2	SPF
HW2	2x8 DRY No.2	SPF
ALL WEBS	2x4 DRY No.2	SPF
DRY: SEASONED LUMBER.		

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - D	2 12	TOP
D - F	2 12	TOP
F - I	2 12	TOP
I - K	2 12	TOP
K - M	2 12	TOP
M - P	2 12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
B - V	3 4	TOP
V - S	2 12	TOP
S - O	3 4	SIDE(1409.5)
WEBS : (0.122"x3") SPIRAL NAILS		
2x4	1 6	
2x8	2 6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED	MAXIMUM FACTORED	INPUT	REQRD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	DOWN	UPLIFT	IN-SX
B 4557 0	4592 193	-1072 5-8	5-8
O 9082 0	9117 0	-1072 5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 1072 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT O FOR 1072 LBS. FACTORED UPLIFT

PROVIDE FOR 193 LBS. FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS
JT COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
B 3336 2050 / 0	477 / 0 0 / 0 0 / -1066 832 / 0 0 / 0
O 6528 4190 / 0	477 / 0 0 / 0 59 / -1066 1884 / 0 0 / 0

HORIZONTAL REACTIONS

B	0 / 0	0 / 0	0 / 0	138 / -138	0 / 0	0 / 0
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BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, O

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 3.85 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-U, K-T.

LOADING

TOTAL LOAD CASES: (18)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1 MAX (PLF)	MEMB.	FORCE (LBS)
FR-TO		FROM TO	FR-TO	
A-B	0 / 1	-115.2 -115.2 0.03 (2)	C-X	-32 / 588 0.03 (3)
B-Z	-5249 / 1142	-115.2 -115.2 0.06 (2)	X-E	-287 / 202 0.04 (10)
Z-C	-3508 / 897	-115.2 -115.2 0.05 (2)	E-W	-720 / 382 0.16 (2)
C-D	-7168 / 1698	-115.2 -115.2 0.09 (13)	W-F	-233 / 818 0.06 (13)
D-E	-7168 / 1698	-115.2 -115.2 0.09 (13)	F-U	-471 / 2601 0.14 (3)
E-F	-6792 / 1590	-115.2 -115.2 0.08 (13)	U-G	-657 / 331 0.18 (3)
F-G	-7462 / 1669	-115.2 -115.2 0.12 (1)	G-J	-647 / 330 0.17 (2)
G-H	-7462 / 1669	-115.2 -115.2 0.10 (1)	H-K	-471 / 399 0.07 (13)
H-I	-8139 / 1669	-115.2 -115.2 0.10 (1)	K-L	-233 / 3540 0.19 (3)
I-J	-8139 / 1669	-115.2 -115.2 0.10 (1)	L-Q	-4449 / 382 0.98 (3)
J-K	-8139 / 1669	-115.2 -115.2 0.13 (1)	Q-R	-39 / 5650 0.30 (3)
K-L	-8999 / 1590	-115.2 -115.2 0.10 (3)	R-N	0 / 1288 0.07 (2)
L-M	-14614 / 1699	-115.2 -115.2 0.21 (3)	N-A	-1284 / 104 0.41 (3)
M-N	-14614 / 1699	-115.2 -115.2 0.21 (3)	A-H	-46 / 1160 0.06 (3)
N-AB	-7727 / 898	-115.2 -115.2 0.10 (2)	Y-Z	-366 / 2351 0.00 (1)
AB-O	-11756 / 1144	-115.2 -115.2 0.13 (3)	Y-C	-3902 / 817 0.22 (1)
O-P	0 / 1	-115.2 -115.2 0.03 (3)	AA-AA	-7070 / 816 0.39 (1)
			AA-AB	-368 / 5727 0.00 (1)
B-Y	-859 / 3121	-39.5 -39.5 0.14 (2)		
Y-X	-1510 / 6220	-39.5 -39.5 0.29 (1)		
X-W	-1426 / 6431	-39.5 -39.5 0.29 (1)		
W-V	-1178 / 6063	-39.5 -39.5 0.27 (1)		
V-U	-1178 / 6063	-39.5 -39.5 0.27 (1)		
U-T	-1343 / 7822	-39.5 -39.5 0.33 (1)		
T-S	-1017 / 8090	-39.5 -39.5 0.34 (1)		
S-R	-1017 / 8090	-39.5 -39.5 0.34 (1)		
R-Q	-1233 / 10870	-39.5 -39.5 0.50 (1)		
Q-AC	-1318 / 12488	-39.5 -39.5 0.71 (1)		
AC-AD	-1318 / 12488	-39.5 -39.5 0.71 (1)		
AD-AE	-1318 / 12488	-39.5 -39.5 0.71 (1)		
AE-AA	-1318 / 12488	-39.5 -39.5 0.71 (1)		
AA-O	-688 / 6864	-39.5 -39.5 0.38 (1)		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 33.4 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.4 PSF
TOTAL LOAD = 57.3 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF CBC 2018, ABC 2019
- PART 4 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR USED
- PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED.

(80 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 33.4 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (1.51")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.13")
ALLOWABLE DEFL.(TL) = $L/180$ (3.03")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.19")

CSI: TC=0.21/1.00 (L-N3), BC=0.71/1.00 (Q-AA:1), WB=0.98/1.00 (L-R3), SSI=0.16/1.00 (Q-AA:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (E) (INPUT = 0.90)
JSI METAL= 0.83 (N) (INPUT = 1.00)



Structural component only
DWG# T-2115868

CONTINUED ON PAGE 2

JOB NAME 413226	TRUSS NAME T111	QUANTITY 1	PLY 3	JOB DESC. ROYAL PINE HOMES TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMW1+I	MT20	10.0	12.0	4.00	
C	TMWW+t	MT20	5.0	8.0		
D, I, M						
D	TS-t	MT20	5.0	6.0		
E	TMWW+t	MT20	4.0	6.0	3.00	1.00
F	TTWW-m	MT20	6.0	9.0	3.00	3.75
G	TMW+w	MT20	3.0	6.0		
H	TMWW+t	MT20	4.0	6.0		
J	TMW+w	MT20	3.0	6.0		
K	TTWW-m	MT20	6.0	9.0	3.00	3.75
L	TMWW+t	MT20	4.0	6.0	3.00	1.00
N	TMWW+t	MT20	5.0	8.0		
O	TMBMW1+I	MT20	10.0	12.0	4.00	Edge
Q	BMWW+t	MT20	6.0	9.0		
R	BMWW+t	MT20	4.0	6.0		
S	BS-t	MT20	5.0	6.0		
T	BMWWW-t	MT20	5.0	8.0		
U	BMWWW-t	MT20	5.0	8.0		
V	BS-t	MT20	5.0	6.0		
W	BMWW+t	MT20	4.0	6.0		
X	BMWW+t	MT20	6.0	9.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
Q	38-5-4	-3502	-3502	---	FRONT	VERT	TOTAL	---	C1
AC	39-3-12	-312	-312	---	FRONT	VERT	TOTAL	---	C1
AD	41-3-12	-312	-312	---	FRONT	VERT	TOTAL	---	C1
AE	43-3-12	-312	-312	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING

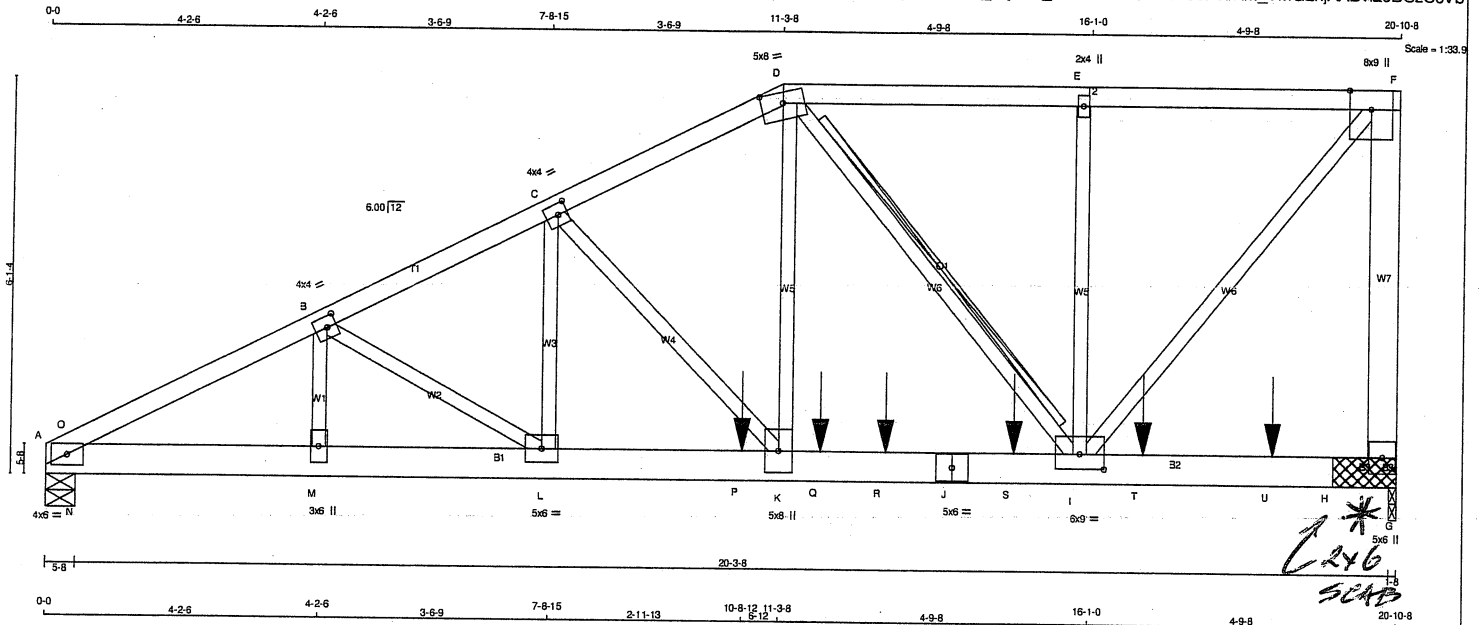
AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.2} PSF AT {30-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpG, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.



Structural component only
DWG# T-2115868 *212*

JOB NAME 413226	TRUSS NAME T112	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Wed May 26 14:46:48 2021 Page 1 ID:hEKUVWZApjnyW9leD6V_jMya4z_4PKFzDkDt0KitBGtbfKfKm_VMGLnjAABvia6BoZCcVb			



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
G - F	2x6	DRY	No.2
A - J	2x6	DRY	No.2
J - J	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - D 1	12	TOP
D - F 1	12	TOP
F - G 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
A - J 2	12	SIDE(0.0)
J - G 2	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMB1-H	MT20	4.0	6.0	
B	TMWW-t	MT20	4.0	4.0	2.00 1.75
C	TMWW-t	MT20	4.0	4.0	2.00 1.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
A	2914	0	2914	0	0	5-8	5-8	
G	4158	0	4158	0	0	1-8	1-8 & BLOCK	

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS						
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
A	2055	1380 / 0	0 / 0	0 / 0	0 / 0	675 / 0	0 / 0	
G	2931	1978 / 0	0 / 0	0 / 0	0 / 0	953 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, G

2x6 DRY SPF No.2 BEARING BLOCK 12" LONG AT JT. G ATTACHED TO FRONT AND BACK SIDE WITH 3 ROWS OF (0.122"x3") SPIRAL NAILS SPACED 3" C.C. 12 NAILS TOTAL.

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.88 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT D-I

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		MAX. FACTORED		WEBS		MAX. FACTORED	
	MAX.	FORCE (LBS)	VERT.	LC1 MAX (PLF)	MAX. (LC)	UNBRAC LENGTH	MEMB.	MAX.	FORCE (LBS)	MAX. (LC)
FR-TO							FR-TO			
A-O	-5897 / 0		-91.8	-91.8	0.18 (1)	3.88	M-B	-160 / 4		0.01 (1)
O-B	-5684 / 0		-91.8	-91.8	0.20 (1)	3.94	B-L	-146 / 0		0.02 (1)
B-C	-5529 / 0		-91.8	-91.8	0.18 (1)	4.02	L-C	0 / 531		0.07 (1)
C-D	-4910 / 0		-91.8	-91.8	0.17 (1)	4.24	C-K	-844 / 0		0.20 (1)
D-E	-3126 / 0		-91.8	-91.8	0.22 (1)	5.02	K-D	0 / 3503		0.43 (1)
E-F	-3127 / 0		-91.8	-91.8	0.23 (1)	5.02	D-I	-2080 / 0		0.53 (1)
G-F	-3822 / 0		0.0	0.0	0.82 (1)	7.23	I-E	-531 / 0		0.15 (1)
							I-F	0 / 4801		0.59 (1)
							N-O	0 / 257		0.00 (1)
A-N	0 / 5076		-18.5	-18.5	0.35 (1)	10.00				
N-M	0 / 5076		-18.5	-18.5	0.44 (1)	10.00				
M-L	0 / 5076		-18.5	-18.5	0.44 (1)	10.00				
L-P	0 / 4951		-18.5	-18.5	0.56 (1)	10.00				
P-K	0 / 4951		-18.5	-18.5	0.56 (1)	10.00				
K-Q	0 / 4441		-18.5	-18.5	0.53 (1)	10.00				
Q-R	0 / 4441		-18.5	-18.5	0.53 (1)	10.00				
R-J	0 / 4441		-18.5	-18.5	0.53 (1)	10.00				
J-S	0 / 4441		-18.5	-18.5	0.53 (1)	10.00				
S-I	0 / 4441		-18.5	-18.5	0.53 (1)	10.00				
I-T	0 / 0		-18.5	-18.5	0.24 (1)	10.00				
T-U	0 / 0		-18.5	-18.5	0.24 (1)	10.00				
U-H	0 / 0		-18.5	-18.5	0.24 (1)	10.00				
H-G	0 / 0		-18.5	-18.5	0.24 (1)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
P	10-8-12	-1507	-1507	---	BACK	VERT	---	C1
Q	11-11-4	-370	-370	---	BACK	VERT	---	C1
R	12-11-4	-370	-370	---	BACK	VERT	---	C1
S	14-11-4	-370	-370	---	BACK	VERT	---	C1
T	16-11-4	-370	-370	---	BACK	VERT	---	C1
U	18-11-4	-370	-370	---	BACK	VERT	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/999 (0.09")
ALLOWABLE DEFL.(TL) = L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/999 (0.17")

CSI: TC=0.82/1.00 (F-G-1), BC=0.56/1.00 (K-L-1), WB=0.59/1.00 (F-I-1), SS=0.64/1.00 (K-L-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MT20	650	371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (D) (INPUT = 0.90)

JSI METAL = 0.63 (D) (INPUT = 1.00)



Structural component only
DWG# T-2115869

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
413226	T112	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
D	TTWW-m	MT20	5.0	8.0	2.00	4.00
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	8.0	9.0	Edge	
G	BMV1+p	MT20	5.0	6.0		
I	BMVWW-t	MT20	6.0	9.0	2.75	4.50
J	BS-t	MT20	5.0	6.0		
K	BMVW+t	MT20	5.0	8.0		
L	BMVW-t	MT20	5.0	6.0		
M	BMV+w	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

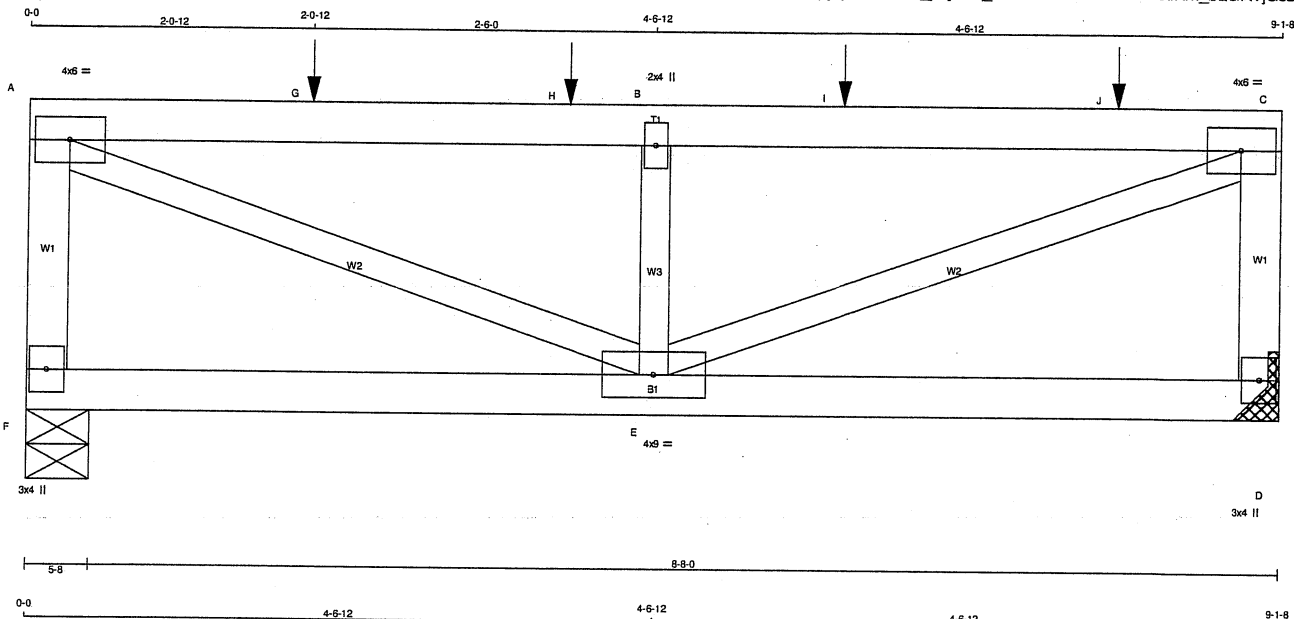


Structural component only
DWG# T-2115869

JOB NAME 413226	TRUSS NAME T113	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 33 = 67 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
F - A	2x4	DRY	No.2
A - C	2x4	DRY	No.2
D - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
F-A	12	TOP
A-C	12	TOP
C-D	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F-D	12	SIDE(21.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	SNOW	DOWN	BRG	BRG
F	1126	0	1126	0	5-8
D	1293	0	1293	0	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	812	444 / 0	0 / 0	0 / 0	0 / 0	0 / 0	369 / 0	0 / 0
D	929	529 / 0	0 / 0	0 / 0	0 / 0	0 / 0	400 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.79 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX.	MEMB.	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. (PLF)	CSI (LO)	UNBRAC	LENGTH	FR-TO	MEMB.	FORCE (LBS)	MAX (LC)
F-A	-1018 / 0	0.0	0.0	0.06 (1)	7.81	A-E	0 / 1988	0.25 (1)	
A-G	-1827 / 0	-91.8	-91.8	0.43 (1)	5.79	E-B	-1274 / 0	0.11 (1)	
G-H	-1827 / 0	-91.8	-91.8	0.43 (1)	5.79	E-C	0 / 1988	0.25 (1)	
H-B	-1827 / 0	-91.8	-91.8	0.43 (1)	5.79				
B-I	-1827 / 0	-91.8	-91.8	0.43 (1)	5.79				
I-J	-1827 / 0	-91.8	-91.8	0.43 (1)	5.79				
J-C	-1827 / 0	-91.8	-91.8	0.43 (1)	5.79				
D-C	-1185 / 0	0.0	0.0	0.08 (1)	7.81				
F-E	0 / 0	-56.0	-56.0	0.16 (4)	10.00				
E-D	0 / 0	-56.0	-56.0	0.16 (4)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	2-0-12	-131	-131	---	TOP	VERT	TOTAL	---	C1
H	3-11-4	-208	-208	---	TOP	VERT	TOTAL	---	C1
I	5-11-4	-208	-208	---	TOP	VERT	TOTAL	---	C1
J	7-11-4	-208	-208	---	TOP	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.30")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.30")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.43/1.00 (A-B:1), BC=0.16/1.00 (E-F:4), WB=0.25/1.00 (A-E:1), SSI=0.29/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

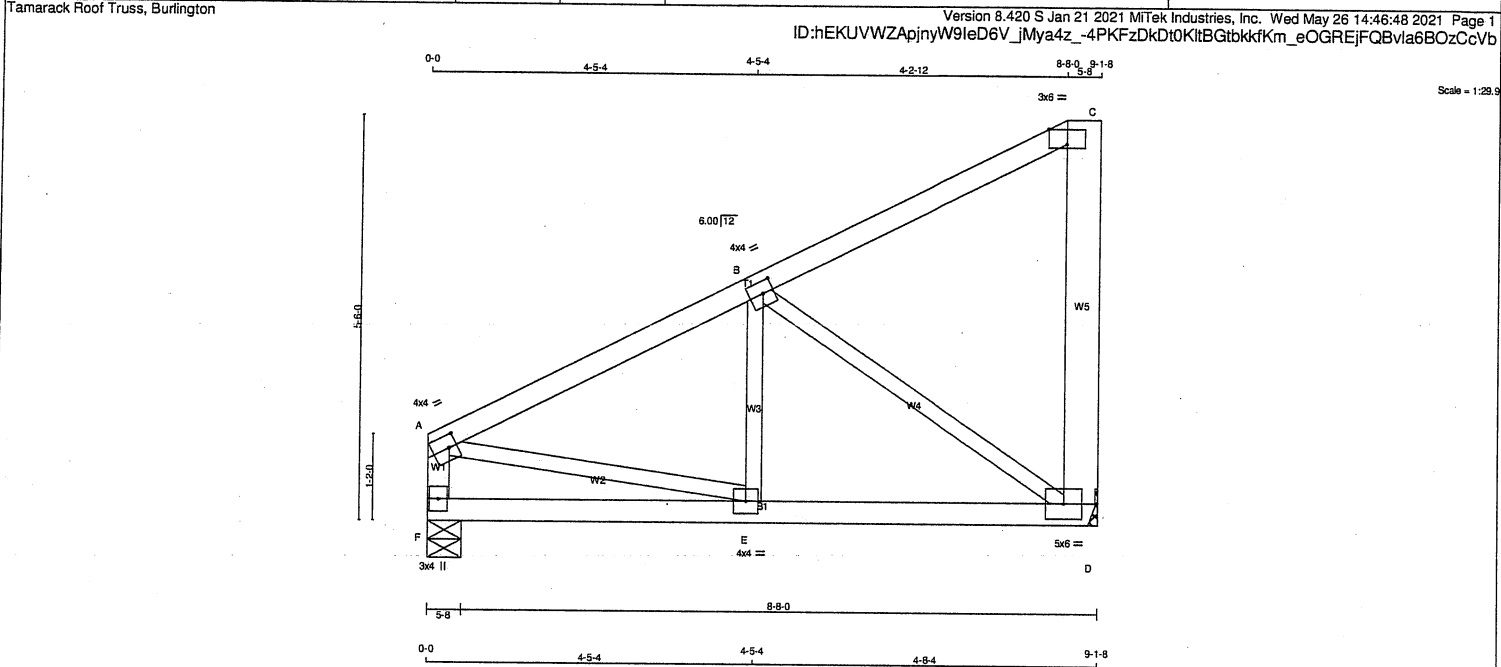
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (C) (INPUT = 0.90)
JSI METAL= 0.22 (E) (INPUT = 1.00)



Structural component only
DWG# T-2115870

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
413226	T114	1	1	TRUSS DESC.		



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
D - C	2x6	DRY	No.2
F - A	2x4	DRY	No.2
F - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0	2.00	1.25
B	TMVW-t	MT20	4.0	4.0	2.00	1.75
C	TVM-p	MT20	3.0	6.0	Edge	3.00
D	BMVW1-t	MT20	5.0	6.0		
E	BMVW-t	MT20	4.0	4.0		
F	BMV1-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
D	503	0	503	0	0
F	503	0	503	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	356	233 / 0	0 / 0	0 / 0	0 / 0	122 / 0	0 / 0
F	356	233 / 0	0 / 0	0 / 0	0 / 0	122 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	MEMB.	FORCE (LBS)	MAX. UNBRAC LENGTH	MAX. FACTORED (LC)
FR-TO		FROM	TO	FR-TO			
A-B	-491 / 0	-91.8	-91.8 0.25 (1)	E-B	0 / 86	6.25	0.03 (4)
B-C	-22 / 0	-91.8	-91.8 0.24 (1)	B-D	-551 / 0	6.25	0.26 (1)
D-C	-166 / 0	0.0	0.0 0.07 (1)	A-E	0 / 469	7.81	0.11 (1)
F-A	-471 / 0	0.0	0.0 0.05 (1)				
F-E	0 / 0	-18.5	-18.5 0.11 (4)				
E-D	0 / 460	-18.5	-18.5 0.14 (4)				

TOTAL WEIGHT = 41 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.30")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.30")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.25/1.00 (A-B:1), BC=0.14/1.00 (D-E:4), WB=0.26/1.00 (B-D:1), SSI=0.18/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.79 (A) (INPUT = 0.90)
JSI METAL = 0.20 (A) (INPUT = 1.00)



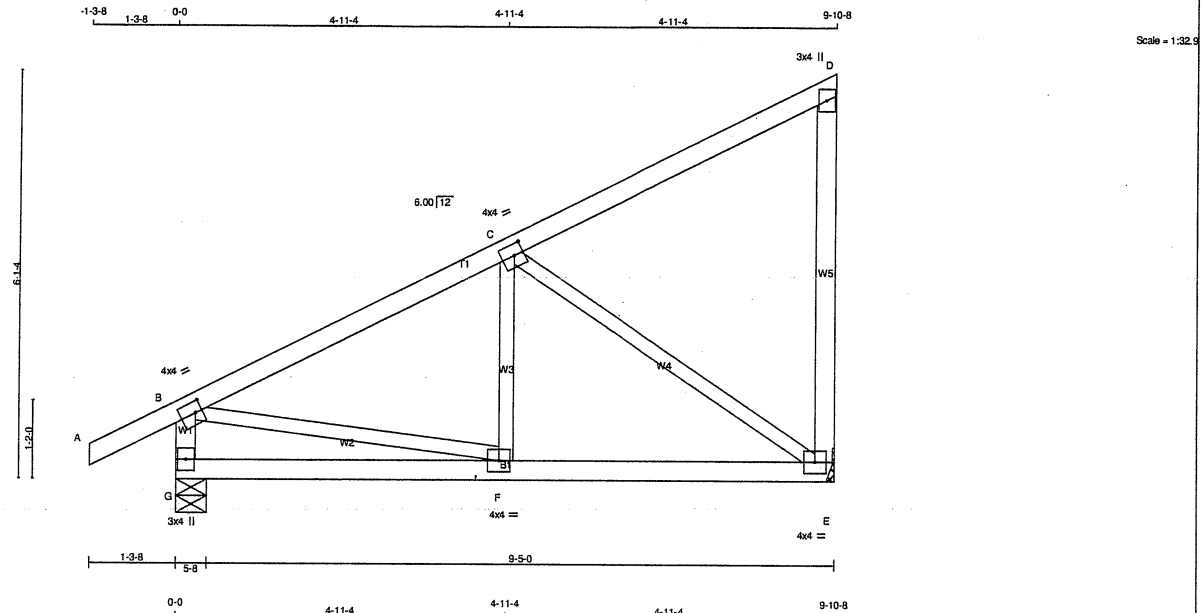
Structural component only
DWG# T-2115871

JOB NAME 413226	TRUSS NAME T115	QUANTITY 5	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

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LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
E - D	2x4	DRY	No.2
G - B	2x4	DRY	No.2
G - E	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2

EXCEPT

SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TMVW-t	MT20	4.0	4.0	2.00	1.75
D	TMV+p	MT20	3.0	4.0		
E	BMVW1-t	MT20	4.0	4.0		
F	BMVW-t	MT20	4.0	4.0		
G	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	544	0	544	0	0	MECHANICAL	5-8
G	669	0	669	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	385	253 / 0	0 / 0	0 / 0	0 / 0	132 / 0	0 / 0
G	471	322 / 0	0 / 0	0 / 0	0 / 0	149 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LO)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED (LBS)	MAX. FACTORED (L.C)
FR-TO		FROM	TO	FR-TO			
A-B	0 / 28	-91.8	-91.8 0.12 (1)	10.00	F-C	0 / 97	0.03 (4)
B-C	-534 / 0	-91.8	-91.8 0.29 (1)	6.25	C-E	-602 / 0	0.35 (1)
C-D	-25 / 0	-91.8	-91.8 0.28 (1)	6.25	B-F	0 / 507	0.11 (1)
E-D	-172 / 0	0.0	0.0 0.12 (1)	7.81			
G-B	-633 / 0	0.0	0.0 0.06 (1)	7.81			
G-F	0 / 0	-18.5	-18.5 0.12 (4)	10.00			
F-E	0 / 499	-18.5	-18.5 0.17 (4)	10.00			

TOTAL WEIGHT = 5 X 43 = 217 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.33")

CALCULATED VERT. DEFL.(LL) = L/999 (0.01")

ALLOWABLE DEFL.(TL)= L/360 (0.33")

CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.29/1.00 (B-C:1) , BC=0.17/1.00 (E-F:4) , WB=0.35/1.00 (C-E:1) , SSI=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (B) (INPUT = 0.90)

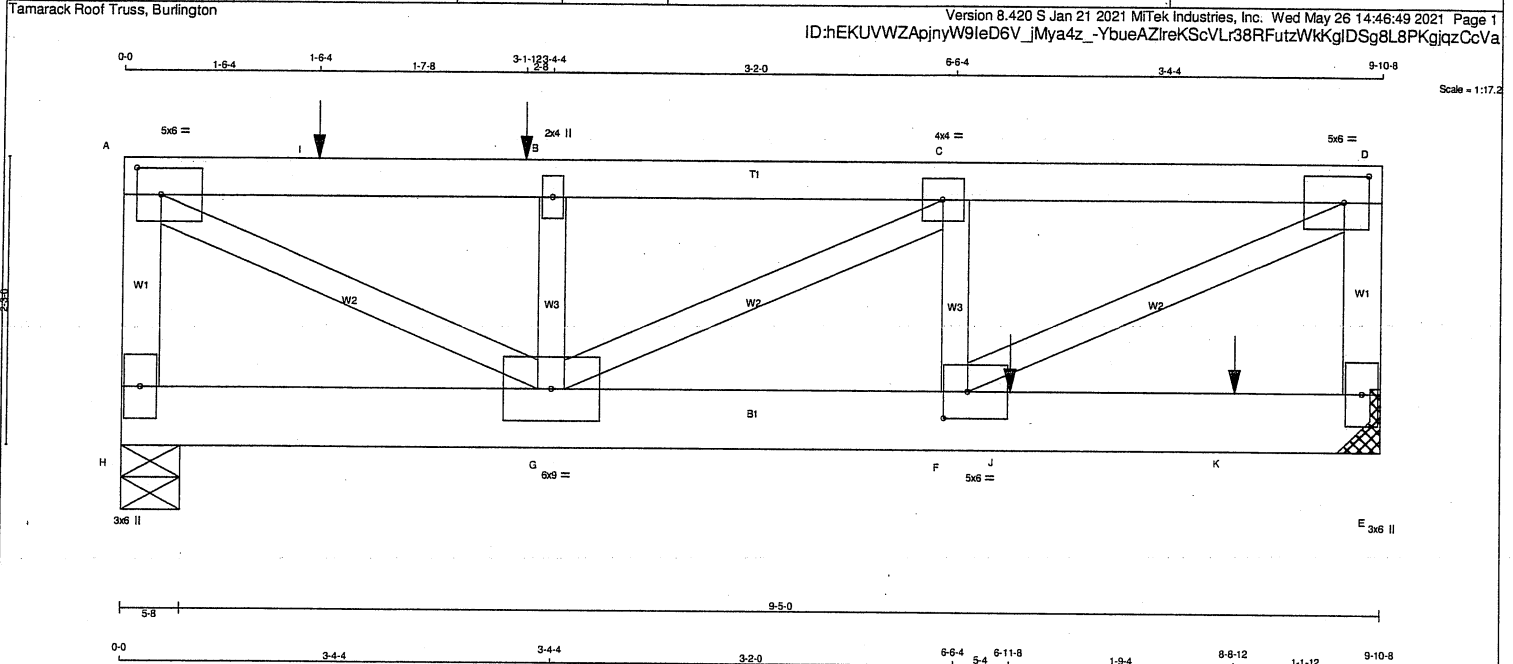
JSI METAL= 0.24 (B) (INPUT = 1.00)



Structural component only

DWG# T-2115872

JOB NAME 413226	TRUSS NAME T116	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Wed May 26 14:46:49 2021 Page 1			
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LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
H - A	2x4	DRY	No.2
A - D	2x4	DRY	No.2
E - D	2x4	DRY	No.2
H - E	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
H - A	12	TOP
A - D	12	TOP
D - E	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
H - E	12	SIDE(21.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0	2.50	2.25
B	TMW+w	MT20	2.0	4.0		
C	TMWV-t	MT20	4.0	4.0		
D	TMVW-t	MT20	5.0	6.0	2.50	2.25
E	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ
H	2344	0	0	5-8
E	2252	0	2252	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. SNOW	MAX/MIN. LIVE	MAX/MIN. PERM. LIVE	MAX/MIN. WIND	MAX/MIN. DEAD	MAX/MIN. SOIL
H	1669	1032 / 0	0 / 0	0 / 0	0 / 0	636 / 0	0 / 0
E	1596	1027 / 0	0 / 0	0 / 0	0 / 0	569 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.53 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 MAX. (LC)	MEMB.	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 MAX. (LC)
FR-TO		FROM	TO	FR-TO		FROM	TO
H - A	-2173 / 0	0.0	0.0	0.13 (1)	7.55	F - D	0 / 3388
A - I	-2963 / 0	-91.8	-91.8	0.55 (1)	4.53	A - G	0 / 3395
I - B	-2963 / 0	-91.8	-91.8	0.55 (1)	4.53	F - C	-200 / 0
B - C	-2963 / 0	-91.8	-91.8	0.26 (1)	5.05	G - B	-1694 / 0
C - D	-2958 / 0	-91.8	-91.8	0.13 (1)	5.23	G - C	0 / 41
E - D	-1811 / 0	0.0	0.0	0.11 (1)	7.81		
H - G	0 / 0	-56.0	-56.0	0.10 (1)	10.00		
G - F	0 / 2958	-56.0	-56.0	0.29 (1)	10.00		
F - J	0 / 0	-56.0	-56.0	0.18 (1)	10.00		
J - K	0 / 0	-18.5	-18.5	0.18 (1)	10.00		
K - E	0 / 0	-18.5	-18.5	0.18 (1)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
B	3-1-12	-584	-584	---	TOP	VERT	TOTAL	---	C1
I	1-6-4	-584	-584	---	TOP	VERT	TOTAL	---	C1
J	6-11-8	-777	-777	---	FRONT	VERT	TOTAL	---	C1
K	8-8-12	-341	-341	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

- C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.33")
CALCULATED VERT. DEFL. (LL) = L/999 (0.03")
ALLOWABLE DEFL. (TL) = L/360 (0.33")
CALCULATED VERT. DEFL. (TL) = L/999 (0.06")

CSI: TC=0.55/1.00 (A-B:1), BC=0.29/1.00 (F-G:1), WB=0.42/1.00 (A-G:1), SS=0.41/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00

COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

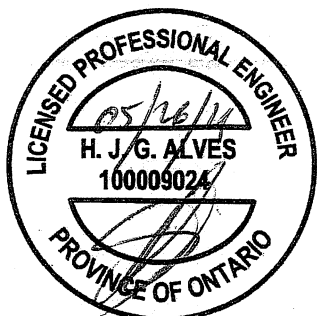
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.89 (A) (INPUT = 0.90)
JSI METAL = 0.34 (A) (INPUT = 1.00)



Structural component only
DWG# T-2115873

JOB NAME 413226	TRUSS NAME T116	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
F	BMW-t	MT20	5.0	6.0	2.50	2.25
G	BMW-t	MT20	6.0	9.0		
H	BMV1+p	MT20	3.0	6.0		



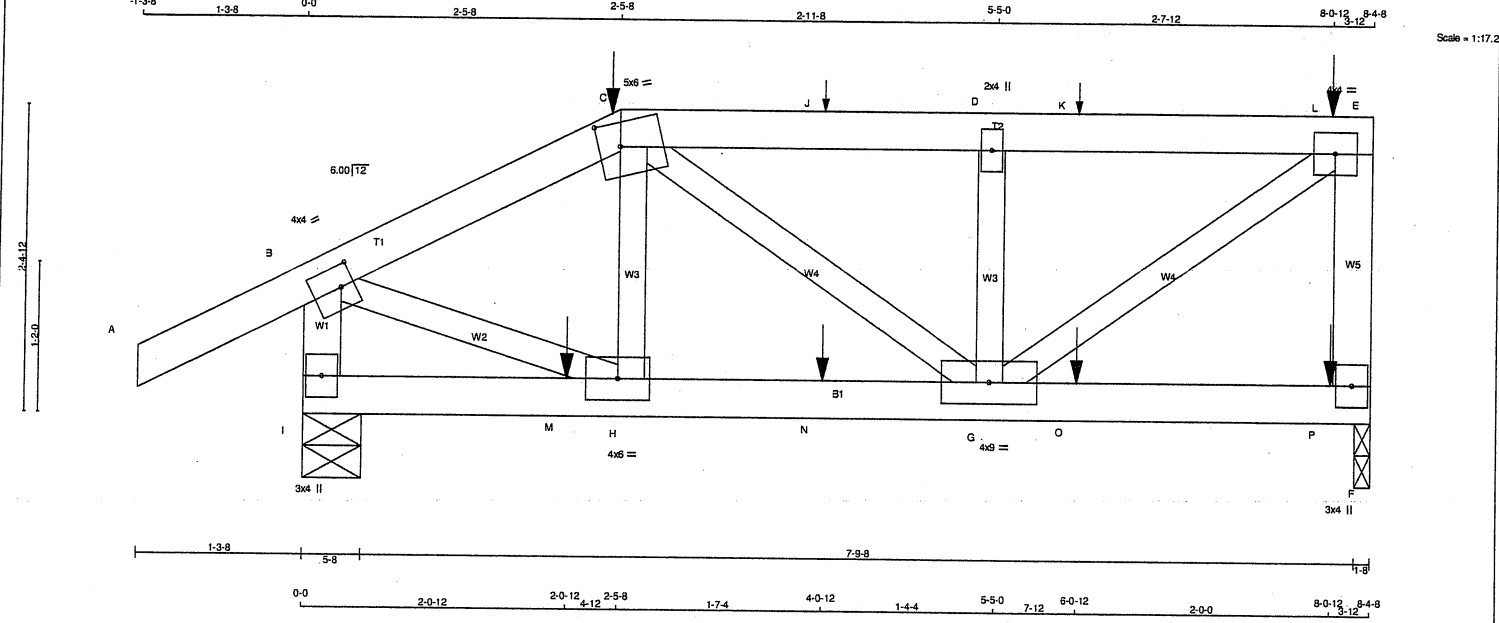
Structural component only
DWG# T-2115873

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
413226	T117	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER

N. L. G. A. RULES

CHORDS SIZE LUMBER DESCR.

A - C 2x4 DRY No.2 SPF

C - E 2x4 DRY No.2 SPF

F - H 2x4 DRY No.2 SPF

I - J 2x4 DRY No.2 SPF

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE PLATES W LEN Y X

B TMVW-t MT20 4.0 4.0 2.00 1.25

C TTWW-m MT20 5.0 6.0 2.25 2.00

D TMW-w MT20 2.0 4.0

E TMVW-t MT20 4.0 4.0

F BMV1+p MT20 3.0 4.0

G BMWVW-t MT20 4.0 9.0

H BMWV-t MT20 4.0 6.0

I BMV1+p MT20 3.0 4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED MAXIMUM FACTORED INPUT REQD

GROSS REACTION GROSS REACTION BRG BRG

JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX

F 527 0 527 0 0 1-8 1-8

I 668 0 668 0 0 5-8 5-8

UNFACTORED REACTIONS

1ST LCASE MAX/MIN. COMPONENT REACTIONS

JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL

F 373 245 / 0 0 / 0 0 / 0 128 / 0 0 / 0

I 469 327 / 0 0 / 0 0 / 0 142 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. MAX. FACTORED FORCE (LBS) FACTORED VERT. LOAD LC1 MAX (PLF) CSI (LC) MAX. UNBRAC LENGTH FR-TO

A-B 0 / 28 -91.8 -91.8 0.13 (1) 10.00

B-C -546 / 0 -91.8 -91.8 0.11 (1) 6.25

C-J -518 / 0 -91.8 -91.8 0.14 (1) 6.25

J-D -518 / 0 -91.8 -91.8 0.14 (1) 6.25

D-K -518 / 0 -91.8 -91.8 0.14 (1) 6.25

K-L -518 / 0 -91.8 -91.8 0.14 (1) 6.25

L-E -518 / 0 -91.8 -91.8 0.14 (1) 6.25

F-E -494 / 0 0.0 0.0 0.07 (1) 7.81

I-B -646 / 0 0.0 0.0 0.07 (1) 7.81

I-M 0 / 0 -18.5 -18.5 0.03 (4) 10.00

M-H 0 / 0 -18.5 -18.5 0.03 (4) 10.00

H-N 0 / 483 -18.5 -18.5 0.11 (1) 10.00

N-G 0 / 483 -18.5 -18.5 0.11 (1) 10.00

G-O 0 / 0 -18.5 -18.5 0.04 (4) 10.00

O-P 0 / 0 -18.5 -18.5 0.04 (4) 10.00

P-F 0 / 0 -18.5 -18.5 0.04 (4) 10.00

WEBS

MEMB. MAX. FACTORED FORCE (LBS) MAX. UNBRAC LENGTH FR-TO

H-C -106 / 11 0.02 (1) 0.02 (1)

C-G 0 / 44 0.01 (1) 0.01 (1)

G-D -332 / 0 0.06 (1) 0.06 (1)

D-E 0 / 633 0.16 (1) 0.16 (1)

B-H 0 / 515 0.13 (1) 0.13 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT LOC. LC1 MAX- MAX+ FACE DIR. TYPE HEEL CONN.

C 2-5-8 -74 -74 --- FRONT VERT TOTAL --- C1

J 4-0-12 1 1 --- FRONT VERT TOTAL --- C1

K 6-0-12 1 1 --- FRONT VERT TOTAL --- C1

L 8-0-12 -17 -17 --- FRONT VERT TOTAL --- C1

M 2-0-12 -3 -3 --- FRONT VERT TOTAL --- C1

N 4-0-12 -1 -1 --- FRONT VERT TOTAL --- C1

O 6-0-12 -1 -1 --- FRONT VERT TOTAL --- C1

P 8-0-12 -6 -6 --- FRONT VERT TOTAL --- C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019

- PART 9 OF OBC 2012 (2019 AMENDMENT)

- CSA 086-14

- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.28")

CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")

ALLOWABLE DEFL.(TL)= L/360 (0.28")

CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.14/1.00 (D-E:1) , BC=0.11/1.00 (G-H:1) , WB=0.16/1.00 (E-G:1) , SS=0.15/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00

COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

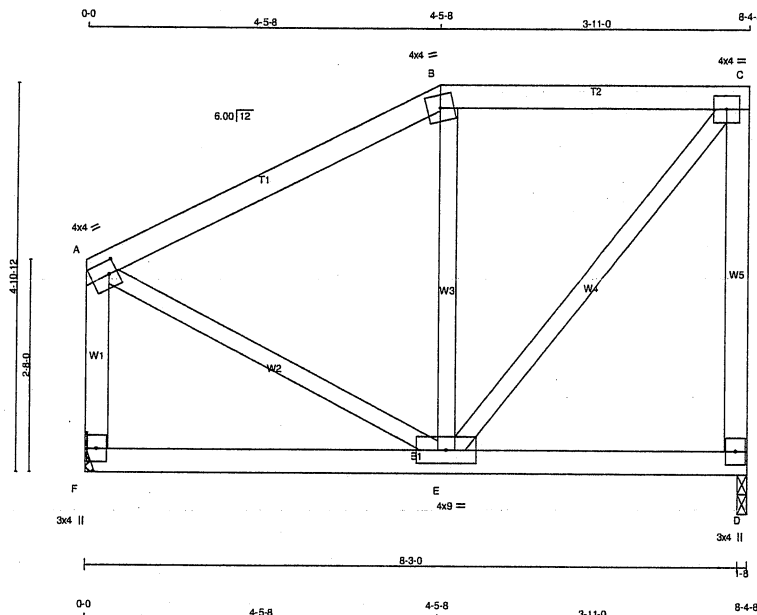
JSI GRIP= 0.79 (B) (INPUT = 0.90)

JSI METAL= 0.22 (B) (INPUT = 1.00)



Structural component only
DWG# T-2115874

JOB NAME 413226	TRUSS NAME T118	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.420 8 Jan 21 MiTek Industries, Inc. Wed May 26 14:46:50 2021 Page 1 ID:hEKUVWZApjnyW9leD6V_jMya4z_0nS0OvmTPdaT7VQFi9m7QB3z448VBCSUN33DGHzCcVZ			



TOTAL WEIGHT = 39 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - B	2x4 DRY	No.2	SPF
B - C	2x4 DRY	No.2	SPF
D - C	2x4 DRY	No.2	SPF
F - A	2x4 DRY	No.2	SPF
F - D	2x4 DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0	2.00	1.25
B	TTW-m	MT20	4.0	4.0		
C	TMVW-t	MT20	4.0	4.0		
D	BMV1+p	MT20	3.0	4.0		
E	BMVWW-t	MT20	4.0	9.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
D	462	0	462	0	0
F	462	0	462	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
D	326	214 / 0	0 / 0	0 / 0	0 / 0	112 / 0	0 / 0	0 / 0
F	326	214 / 0	0 / 0	0 / 0	0 / 0	112 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX	MEMB.	FORCE (LBS)	MAX	LC1
FR-TO		FROM	TO	FR-TO			
A-B	-245 / 0	-91.8	-91.8 0.23 (1)	E-B	-275 / 0	0.10	(1)
B-C	-212 / 0	-91.8	-91.8 0.18 (1)	E-C	0 / 333	0.07	(1)
D-C	-436 / 0	0.0	0.0 0.17 (1)	A-E	0 / 246	0.06	(1)
F-A	-427 / 0	0.0	0.0 0.06 (1)				
F-E	0 / 0	-18.5	-18.5 0.09 (4)				
E-D	0 / 0	-18.5	-18.5 0.09 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.28")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.28")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.23/1.00 (A-B:1), BC=0.09/1.00 (E-F:4),
WB=0.10/1.00 (B-E:1), SS=0.14/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

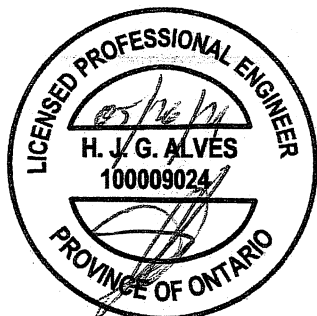
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

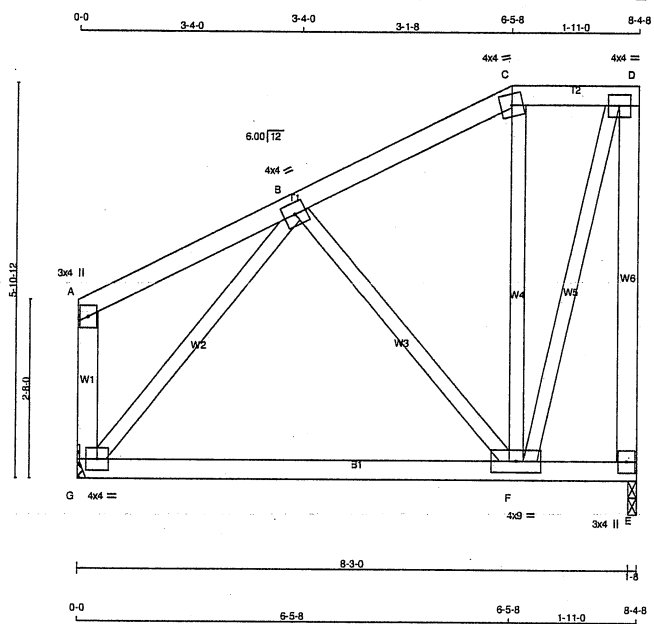
JSI GRIP= 0.36 (A) (INPUT = 0.90)
JSI METAL= 0.11 (A) (INPUT = 1.00)



Structural component only
DWG# T-2115875

JOB NAME 413226	TRUSS NAME T119	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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TOTAL WEIGHT = 45 lb
[M/F]

<u>LUMBER</u>					
N. L. G. A. RULES					
CHORDS		SIZE	LUMBER		DESCR
A - C	2x4	DRY	No.2		SPF
C - D	2x4	DRY	No.2		SPF
E - D	2x4	DRY	No.2		SPF
G - A	2x4	DRY	No.2		SPF
G - E	2x4	DRY	No.2		SPF

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0			
B	TMW-t	MT20	4.0	4.0			
C	TTW-m	MT20	4.0	4.0			
D	TMV-t	MT20	4.0	4.0			
E	BMV1+p	MT20	3.0	4.0			
F	BMW-t	MT20	4.0	9.0			
G	BMV1-t	MT20	4.0	4.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
E	462	0	462	0	0	1-8	1-8		
G	462	0	462	0	0	MECHANICAL			

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	326	214 / 0	0 / 0	0 / 0	0 / 0	0 / 0	112 / 0	0 / 0
G	326	214 / 0	0 / 0	0 / 0	0 / 0	0 / 0	112 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO	CSI (LC)	FR-TO			
A-B	0 / 16	-91.8 -91.8	0.16 (1)	10.00	B-F	-175 / 0	0.08 (1)
B-C	-162 / 0	-91.8 -91.8	0.12 (1)	6.25	F-C	-135 / 0	0.07 (1)
C-D	-128 / 0	-91.8 -91.8	0.04 (1)	6.25	F-D	0 / 418	0.09 (1)
E-D	-483 / 0	0.0 0.0	0.32 (1)	7.81	G-B	-385 / 0	0.16 (1)
G-A	-113 / 0	0.0 0.0	0.02 (1)	7.81			
G-F	0 / 243	-18.5 -18.5	0.19 (4)	10.00			
F-E	0 / 0	-18.5 -18.5	0.18 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.28")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.01")
ALLOWABLE DEFL. (TL) = $L/360$ (0.28")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.05")

CSI: TC=0.32/1.00 (D-E:1), BC=0.19/1.00 (F-G:4),
WB=0.16/1.00 (B-G:1), SSI=0.13/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

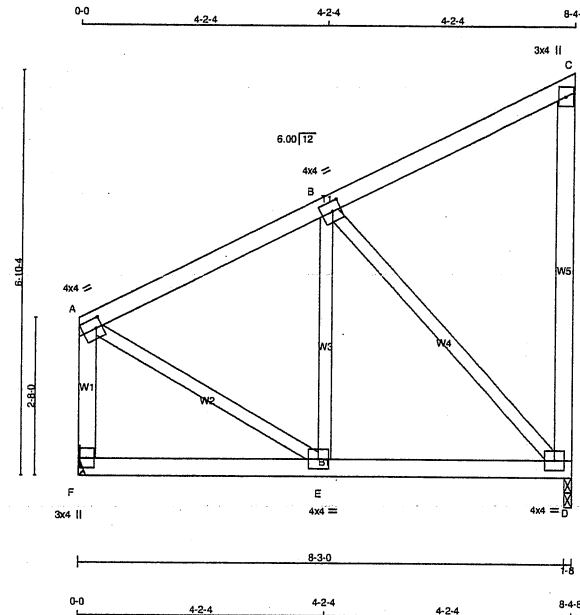
JSI GRIP= 0.55 (D) (INPUT = 0.90)
JSI METAL= 0.12 (D) (INPUT = 1.00)



Structural component only
DWG# T-2115876

JOB NAME 413226	TRUSS NAME T120	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1/32"

TOTAL WEIGHT = 42 lb

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0	2.00	1.25
B	TMVW-t	MT20	4.0	4.0	2.00	1.75
C	TMV+p	MT20	3.0	4.0		
D	BMVW1-t	MT20	4.0	4.0		
E	BMVW-t	MT20	4.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
D	462	0	462	0	0
F	462	0	462	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
D	326	214 / 0	0 / 0	0 / 0	0 / 0	0 / 0	112 / 0	0 / 0
F	326	214 / 0	0 / 0	0 / 0	0 / 0	0 / 0	112 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	-280 / 0	E-B	-58 / 56
B-C	-21 / 0	B-D	-393 / 0
D-C	-145 / 0	A-E	0 / 309
F-A	-431 / 0		
F-E	0 / 0		
E-D	0 / 269		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.28")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.28")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.21/1.00 (A-B:1), BC=0.11/1.00 (D-E:4), WB=0.24/1.00 (B-D:1), SSI=0.17/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

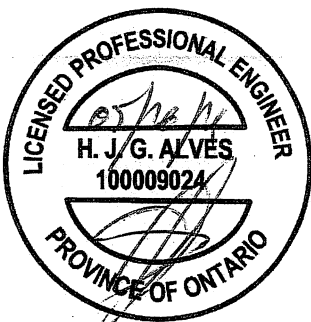
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.44 (A) (INPUT = 0.90)
JSI METAL= 0.12 (A) (INPUT = 1.00)



Structural component only
DWG# T-2115877

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
413226	T121	1	3	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
E	BMWw-t	MT20	5.0	8.0	4.25	2.50
F	BMV1+p	MT20	3.0	6.0		

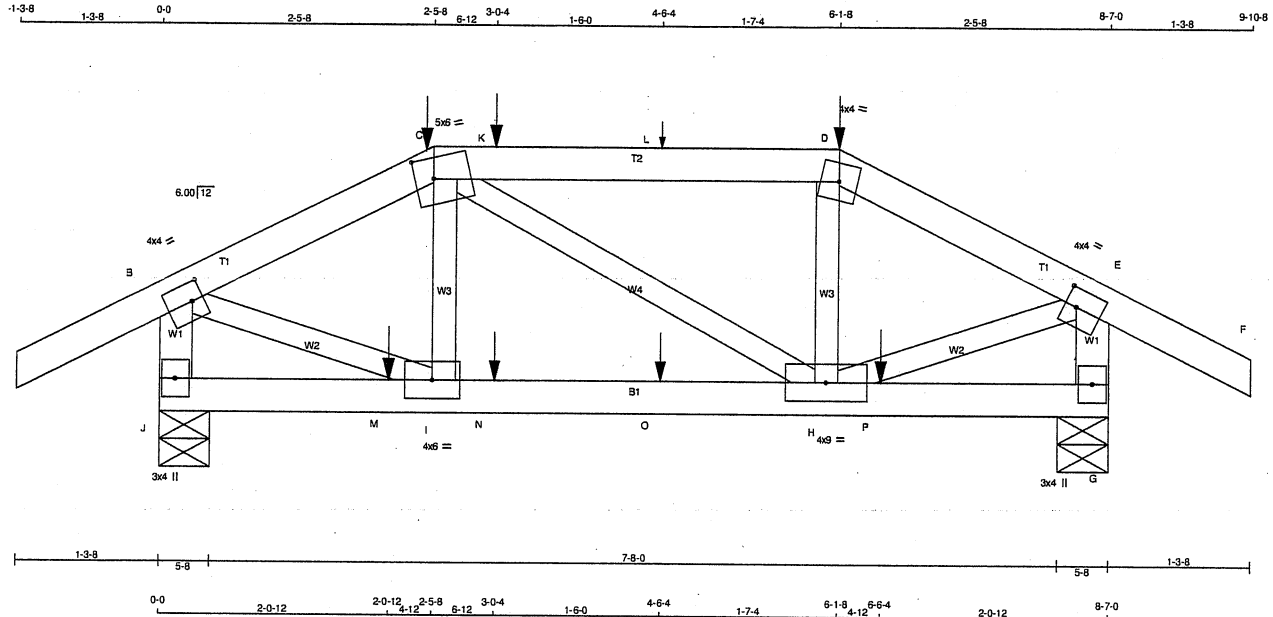


Structural component only
DWG# T-2115878

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
413226	T122	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF		
C - D	2x4	DRY	No.2	SPF		
D - F	2x4	DRY	No.2	SPF		
J - B	2x4	DRY	No.2	SPF		
G - E	2x4	DRY	No.2	SPF		
J - G	2x4	DRY	No.2	SPF		
ALL WEBS	2x3	DRY	No.2	SPF		
EXCEPT						

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TTW-m	MT20	5.0	6.0	2.25	2.00
D	TTW-m	MT20	4.0	4.0		
E	TMVW-t	MT20	4.0	4.0	2.00	1.25
G	BMV1+p	MT20	3.0	4.0		
H	BMVWW-t	MT20	4.0	9.0		
I	BMVWW-t	MT20	4.0	6.0		
J	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
J	720	0	720	0
G	716	0	716	0

UNFACTORED REACTIONS

JT	1ST CASE	MAX./MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE	
J	505	354 / 0	0 / 0	0 / 0	151 / 0
G	502	352 / 0	0 / 0	0 / 0	150 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX.	MEMB.	WEBS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX.
		FORCE	(LBS)	(PLF)	(LBS)	(LBS)	(LBS)			FORCE	(LBS)	(LBS)	(LBS)	(LBS)	(LBS)
FR-TO								FR-TO							
A-B	0 / 28							I-C	-119 / 16	0.02 (1)					
B-C	-617 / 0							C-H	-6 / 0	0.00 (1)					
C-K	-540 / 0							H-D	-116 / 17	0.02 (1)					
K-L	-540 / 0							B-I	0 / 582	0.14 (1)					
L-D	-540 / 0							H-E	0 / 576	0.14 (1)					
D-E	-611 / 0														
E-F	0 / 28														
J-B	-700 / 0														
G-E	-695 / 0														
J-M	0 / 0														
M-I	0 / 0														
I-N	0 / 546														
N-O	0 / 546														
O-H	0 / 546														
H-P	0 / 0														
P-G	0 / 0														

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-5-8	-74	-74		BACK	VERT	TOTAL		C1
D	6-1-8	-74	-74		BACK	VERT	TOTAL		C1
K	3-0-4	-9	-9		BACK	VERT	TOTAL		C1
L	4-6-4	1	1		BACK	VERT	TOTAL		C1
M	2-0-12	-3	-3		BACK	VERT	TOTAL		C1
N	3-0-4	-1	-1		BACK	VERT	TOTAL		C1
O	4-6-4	-1	-1		BACK	VERT	TOTAL		C1
P	6-6-4	-3	-3		BACK	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.29")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.01")
ALLOWABLE DEFL.(TL) = $L/360$ (0.29")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.02")

CSI: TC=0.24/1.00 (C-D:1), BC=0.12/1.00 (H-I:1), WB=0.14/1.00 (B-I:1), SSI=0.15/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

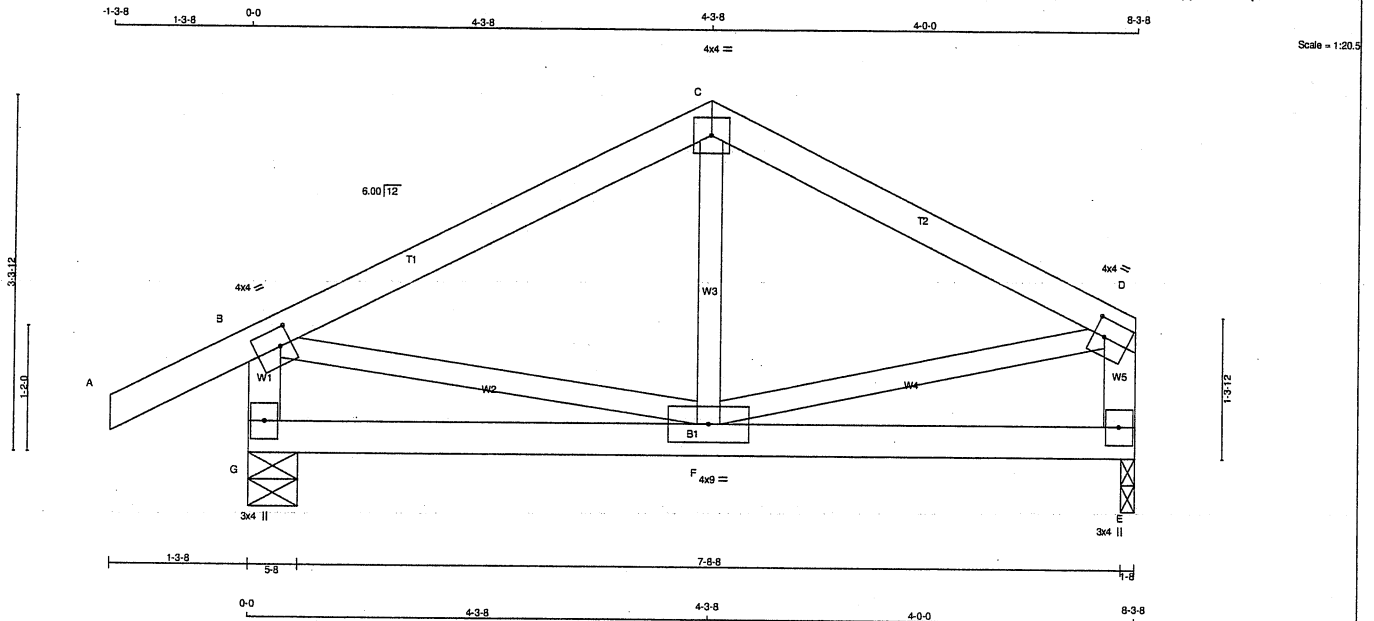
JSI GRIP= 0.90 (B) (INPUT = 0.90)

JSI METAL= 0.25 (B) (INPUT = 1.00)



Structural component only
DWG# T-2115879

JOB NAME 413226	TRUSS NAME T123	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.420 5 Jan 21 2021 MiTek Industries, Inc. Wed May 26 14:46:52 2021 Page 1			
		ID:hEKUVWZApjnyW9leD6V_jMya4z_yAampankxqBmpaeqobVc8Jqtp2f6GngNYKK9zCcVX			



TOTAL WEIGHT = 32 lb [M/F]

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
G - B	2x4	DRY	No.2
E - D	2x4	DRY	No.2
G - E	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-t	MT20	4.0	4.0	2.00 1.25
C	TTW-p	MT20	4.0	4.0	
D	TMVW-t	MT20	4.0	4.0	2.00 1.25
E	BMV1+p	MT20	3.0	4.0	
F	BMVWW-t	MT20	4.0	9.0	
G	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	BRG
G	582	0	582	0	0	5-8	5-8		
E	457	0	457	0	0	1-8	1-8		

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN.	COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
G	409	282 / 0	0 / 0	0 / 0	0 / 0	127 / 0	0 / 0		
E	323	212 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 28	-91.8	-91.8 0.12 (1)	10.00	F-C	-54 / 52	0.02 (4)
B-C	-364 / 0	-91.8	-91.8 0.22 (1)	6.25	B-F	0 / 332	0.07 (1)
C-D	-364 / 0	-91.8	-91.8 0.19 (1)	6.25	F-D	0 / 336	0.08 (1)
G-B	-550 / 0	0.0	0.0 0.06 (1)	7.81			
E-D	-428 / 0	0.0	0.0 0.04 (1)	7.81			
G-F	0 / 0	-18.5	-18.5 0.09 (4)	10.00			
F-E	0 / 0	-18.5	-18.5 0.09 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.28")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.28")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.22/1.00 (B-C:1), BC=0.09/1.00 (F-G:4),
WB=0.08/1.00 (D-F:1), SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.56 (B) (INPUT = 0.90)

JSI METAL= 0.17 (B) (INPUT = 1.00)

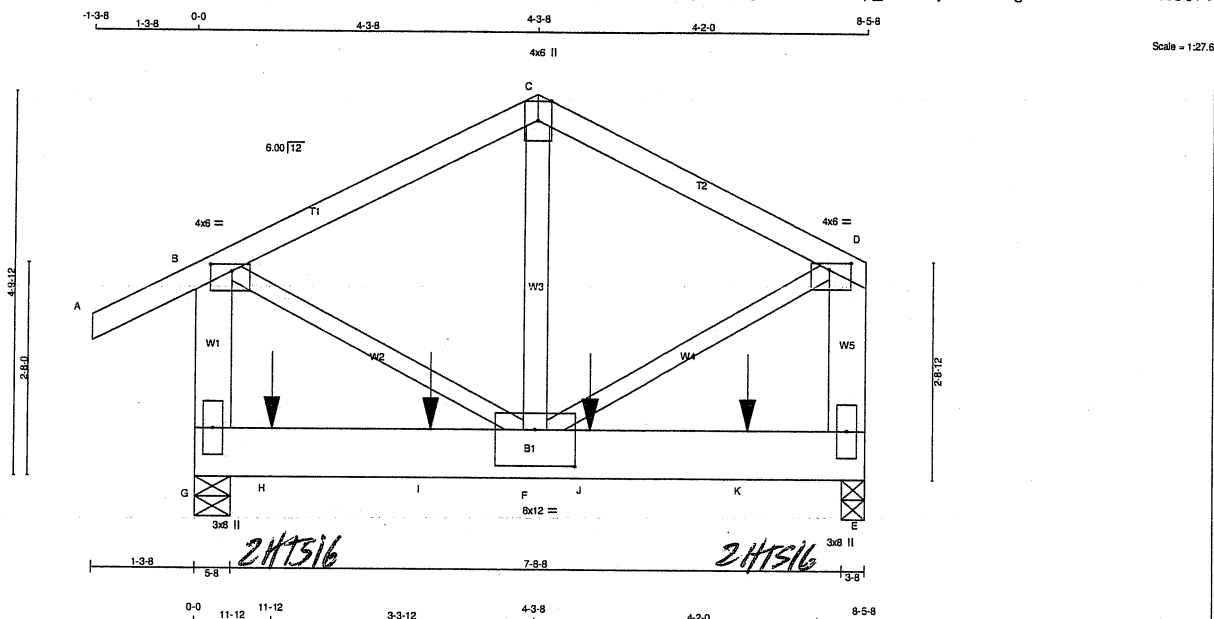


Structural component only
DWG# T-2115880

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
413226	T127	1	3	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 3 X 51 = 153 lb

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - D	2x4	DRY No.2	SPF
G - B	2x6	DRY No.2	SPF
E - D	2x6	DRY No.2	SPF
G - E	2x8	DRY No.2	SPF

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
F - C	2x4	DRY	No.2	SPF

DRY; SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C 1	12	TOP
C - D 1	12	TOP
G - B 2	12	TOP
E - D 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
G - E 2	4	SIDE (899.3)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	
2x4 1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORIZ	DOWN	HORIZ
G	8113	0	8113	138
E	7141	0	7141	0

PROVIDE ANCHORAGE AT BEARING JOINT G FOR 1408 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT E FOR 1236 LBS. FACTORED UPLIFT

PROVIDE FOR 138 LBS. FACTORED HORIZONTAL REACTION AT JOINT G

UNFACTORED REACTIONS

1ST LCASE	MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
G	5987	3564 / 0	1046 / 0	0 / 0	96 / -1668	1377 / 0
E	5277	3117 / 0	939 / 0	0 / 0	102 / -1470	1221 / 0

HORIZONTAL REACTIONS						
G	0 / 0	0 / 0	0 / 0	99 / -84	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, E

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 4.92 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (18)

CHORDS	MAX. FACTORED	FACTORED	W E B S	MAX. FACTORED
MEMB.	FORCE	VERT. LOAD LC1	MEMB.	FORCE
	(LBS)	(PLF)		(LBS)
FR-TO		FROM TO	FR-TO	
A-B	0 / 41	-115.2 -115.2 0.06 (2)	F-C	-725 / 4243
B-C	-5185 / 944	-115.2 -115.2 0.19 (2)	B-F	-884 / 5212
C-D	-5185 / 949	-115.2 -115.2 0.18 (3)	F-D	-904 / 5278
G-B	-5101 / 903	0.0 0.0 0.14 (1)		
E-D	-5078 / 898	0.0 0.0 0.14 (1)		
G-H	-106 / 103	-39.5 -39.5 0.51 (3)		
H-I	-106 / 103	-39.5 -39.5 0.51 (3)		
I-F	-106 / 103	-39.5 -39.5 0.51 (3)		
F-J	-16 / 33	-39.5 -39.5 0.51 (3)		
J-K	-16 / 33	-39.5 -39.5 0.51 (3)		
K-E	-16 / 33	-39.5 -39.5 0.51 (3)		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
H	11-12	-2547	-2547	277	BACK	VERT	TOTAL	---	C1
I	2-11-12	-2547	-2547	277	BACK	VERT	TOTAL	---	C1
J	4-11-12	-2547	-2547	277	BACK	VERT	TOTAL	---	C1
K	6-11-12	-2547	-2547	277	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.2} PSF AT (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, Cp, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (ROUGH TERRAIN) AT MINIMUM (1.000) MILES DISTANCE FROM OPEN TERRAIN, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 33.4 PSF
DL	= 6.0 PSF
BOT CH. LL	= 10.5 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 57.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 4 OF BCBC 2018, ABC 2019
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- SLOPE REDUCTION FACTOR USED
- PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED.

(80 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 33.4 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.28")
CALCULATED VERT. DEFL. (LL) = L/999 (0.03")
ALLOWABLE DEFL. (TL) = L/180 (0.56")
CALCULATED VERT. DEFL. (TL) = L/999 (0.04")

CSI: TC=0.19/1.00 (B-C-2), BC=0.51/1.00 (F-G-3),
WB=0.40/1.00 (D-F-1), SSI=0.89/1.00 (E-F-3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (B) (INPUT = 0.90)

JSI METAL= 0.36 (B) (INPUT = 1.00)



Structural component only
DWG# T-2115881

JOB NAME 413226	TRUSS NAME T127	QUANTITY 1	PLY 3	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	6.0	1.00	3.25
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMVW-p	MT20	4.0	6.0	1.00	3.25
E	BMV1+p	MT20	3.0	8.0		
F	BMWW-t	MT20	8.0	12.0	5.50	6.00
G	BMV1+p	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

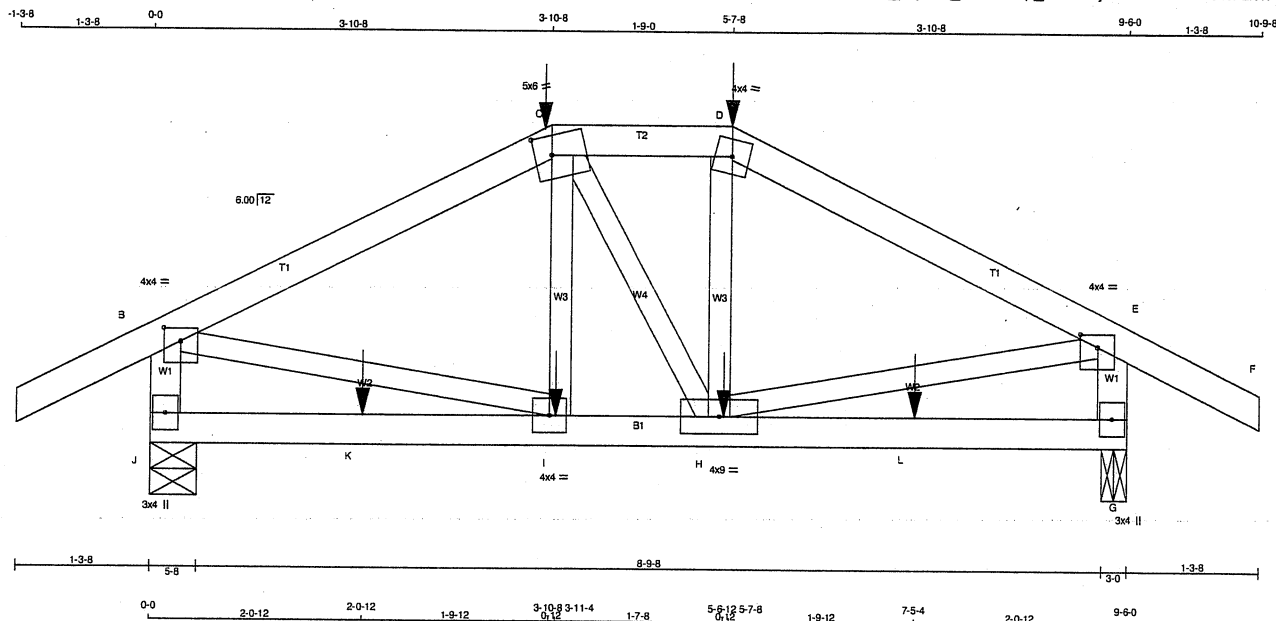


Structural component only
DWG# T-2115881 *ml*

JOB NAME 413226	TRUSS NAME T128	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

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LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
A - C	2x4 DRY	No.2	SPF
C - D	2x4 DRY	No.2	SPF
D - F	2x4 DRY	No.2	SPF
J - B	2x4 DRY	No.2	SPF
G - E	2x4 DRY	No.2	SPF
J - G	2x4 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-p	MT20	4.0	4.0	1.50 2.00
C	TTWV-m	MT20	5.0	6.0	2.25 2.00
D	TTW-m	MT20	4.0	4.0	
E	TMVW-p	MT20	4.0	4.0	1.50 2.00
G	BMV1+p	MT20	3.0	4.0	
H	BMVWW-t	MT20	4.0	9.0	
I	BMVWW-t	MT20	4.0	4.0	
J	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT		REQD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	IN-SX
J	921	0	921	0	0	5-8	5-8
G	920	0	920	0	0	3-0	3-0

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	647	447 / 0	0 / 0	0 / 0	0 / 0	200 / 0	0 / 0
G	647	447 / 0	0 / 0	0 / 0	0 / 0	200 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.07 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-91.8 -91.8	0.13 (1)	10.00	I-C	-100 / 28	0.02 (1)
B-C	-905 / 0	-91.8 -91.8	0.28 (1)	6.08	C-H	0 / 2	0.00 (4)
C-D	-805 / 0	-91.8 -91.8	0.06 (1)	6.25	H-D	-100 / 30	0.02 (1)
D-E	-907 / 0	-91.8 -91.8	0.28 (1)	6.07	B-I	0 / 827	0.20 (1)
E-F	0 / 28	-91.8 -91.8	0.13 (1)	10.00	H-E	0 / 829	0.21 (1)
J-B	-883 / 0	0.0 0.0	0.10 (1)	7.81			
G-E	-882 / 0	0.0 0.0	0.10 (1)	7.81			
J-K	0 / 0	-18.5 -18.5	0.09 (4)	10.00			
K-I	0 / 0	-18.5 -18.5	0.09 (4)	10.00			
I-H	0 / 805	-18.5 -18.5	0.17 (1)	10.00			
H-L	0 / 0	-18.5 -18.5	0.09 (4)	10.00			
L-G	0 / 0	-18.5 -18.5	0.09 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-10-8	-172	-172	---	BACK	VERT	TOTAL	---	C1
D	5-7-8	-172	-172	---	BACK	VERT	TOTAL	---	C1
H	5-6-12	-10	-10	---	BACK	VERT	TOTAL	---	C1
I	3-11-4	-10	-10	---	BACK	VERT	TOTAL	---	C1
K	2-0-12	-10	-10	---	BACK	VERT	TOTAL	---	C1
L	7-5-4	-10	-10	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

- C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	25.6	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.32")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.01")
ALLOWABLE DEFL.(TL) = $L/360$ (0.32")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.02")

CSI: TC=0.28/1.00 (D-E:1), BC=0.17/1.00 (H-I:1), WB=0.21/1.00 (E-H:1), SS=0.14/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

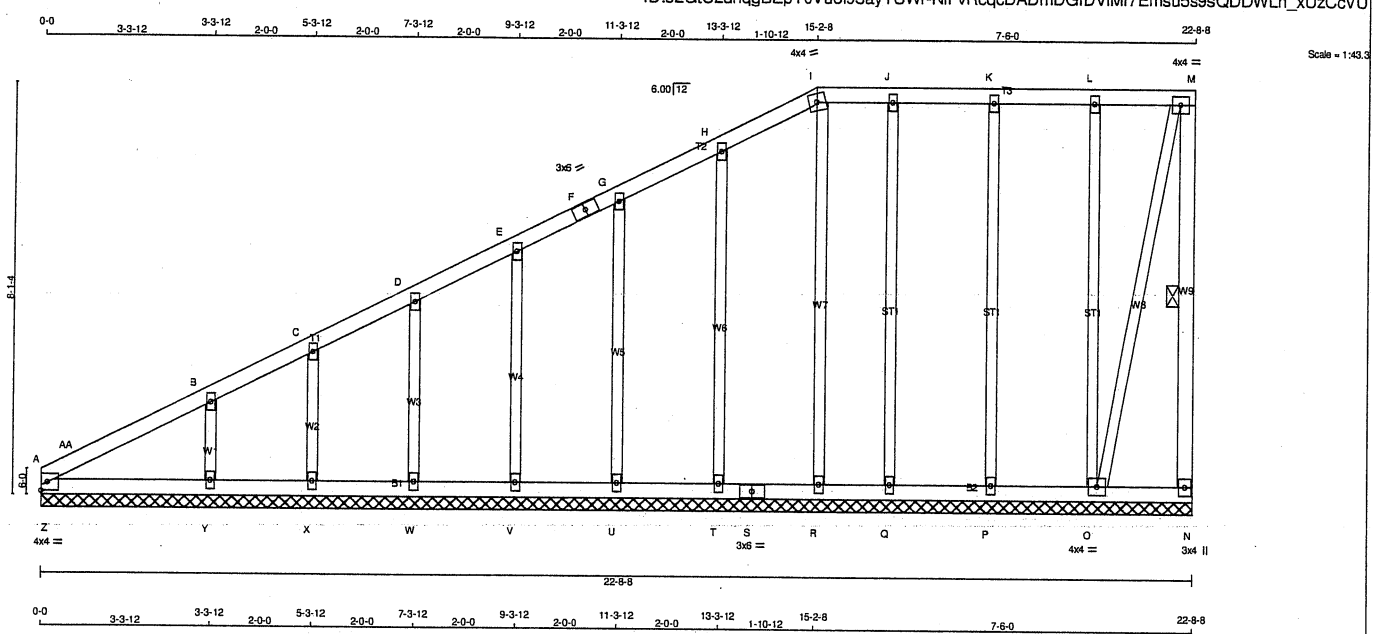
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (I) (INPUT = 0.90)

JSI METAL= 0.29 (E) (INPUT = 1.00)



Structural component only
DWG# T-2115882



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
I - M	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
A - S	2x4	DRY	No.2	SPF
S - N	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMB1-t	MT20	4.0	4.0		Edge
B, C, D, E, G, H, J, K, L						
B	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
I	TTW-m	MT20	4.0	4.0		
M	TMVW-t	MT20	4.0	4.0		
N	BMV1+p	MT20	3.0	4.0		
O	BMWV1-t	MT20	4.0	4.0		
P, Q, R, T, U, V, W, X, Y						
P	BMW1+w	MT20	2.0	4.0		
S	BS-t	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF M-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)		
FR-TO		FROM TO			FR-TO				
A-AA	-30 / 59	-91.8 -91.8	0.04 (1)	6.25	O-L	-201 / 0	0.25 (1)		
AA-B	-18 / 0	-91.8 -91.8	0.10 (1)	6.25	P-K	-185 / 0	0.23 (1)		
B-C	-27 / 0	-91.8 -91.8	0.10 (1)	6.25	Q-J	-176 / 0	0.22 (1)		
C-D	-8 / 0	-91.8 -91.8	0.04 (1)	10.00	R-I	-123 / 0	0.16 (1)		
D-E	-8 / 0	-91.8 -91.8	0.04 (1)	10.00	Y-B	-272 / 0	0.04 (1)		
E-F	-4 / 0	-91.8 -91.8	0.04 (1)	10.00	X-C	-149 / 0	0.03 (1)		
F-G	-4 / 0	-91.8 -91.8	0.04 (1)	10.00	W-D	-189 / 0	0.05 (1)		
G-H	-1 / 0	-91.8 -91.8	0.05 (1)	10.00	V-E	-181 / 0	0.07 (1)		
H-I	-8 / 0	-91.8 -91.8	0.05 (1)	10.00	U-G	-180 / 0	0.11 (1)		
I-J	0 / 2	-91.8 -91.8	0.04 (1)	10.00	T-H	-201 / 0	0.18 (1)		
J-K	0 / 2	-91.8 -91.8	0.04 (1)	10.00	O-M	-7 / 1	0.01 (1)		
K-L	0 / 2	-91.8 -91.8	0.05 (1)	10.00	Z-AA	-228 / 9	0.00 (1)		
L-M	0 / 2	-91.8 -91.8	0.05 (1)	10.00					
N-M	-69 / 0	0.0 0.0	0.02 (1)	6.25					
A-Z	0 / 23	-18.5 -18.5	0.06 (1)	10.00					
Z-Y	0 / 23	-18.5 -18.5	0.06 (1)	10.00					
Y-X	0 / 13	-18.5 -18.5	0.05 (1)	10.00					
X-W	0 / 10	-18.5 -18.5	0.02 (4)	10.00					
W-V	0 / 6	-18.5 -18.5	0.02 (4)	10.00					
V-U	0 / 4	-18.5 -18.5	0.01 (4)	10.00					
U-T	0 / 2	-18.5 -18.5	0.02 (4)	10.00					
T-S	0 / 0	-18.5 -18.5	0.02 (4)	10.00					
S-R	0 / 0	-18.5 -18.5	0.02 (4)	10.00					
R-Q	-2 / 0	-18.5 -18.5	0.01 (4)	10.00					
Q-P	-2 / 0	-18.5 -18.5	0.01 (4)	10.00					
P-O	-2 / 0	-18.5 -18.5	0.02 (4)	10.00					
O-N	0 / 0	-18.5 -18.5	0.02 (4)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.10/1.00 (B-AA:1) , BC=0.06/1.00 (Y-Z:1) , WB=0.25/1.00 (L-O:1) , SSI=0.18/1.00 (A-Z:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.59 (I) (INPUT = 0.90)

JSI METAL = 0.11 (B) (INPUT = 1.00)



JOB NAME 413226	TRUSS NAME T202	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

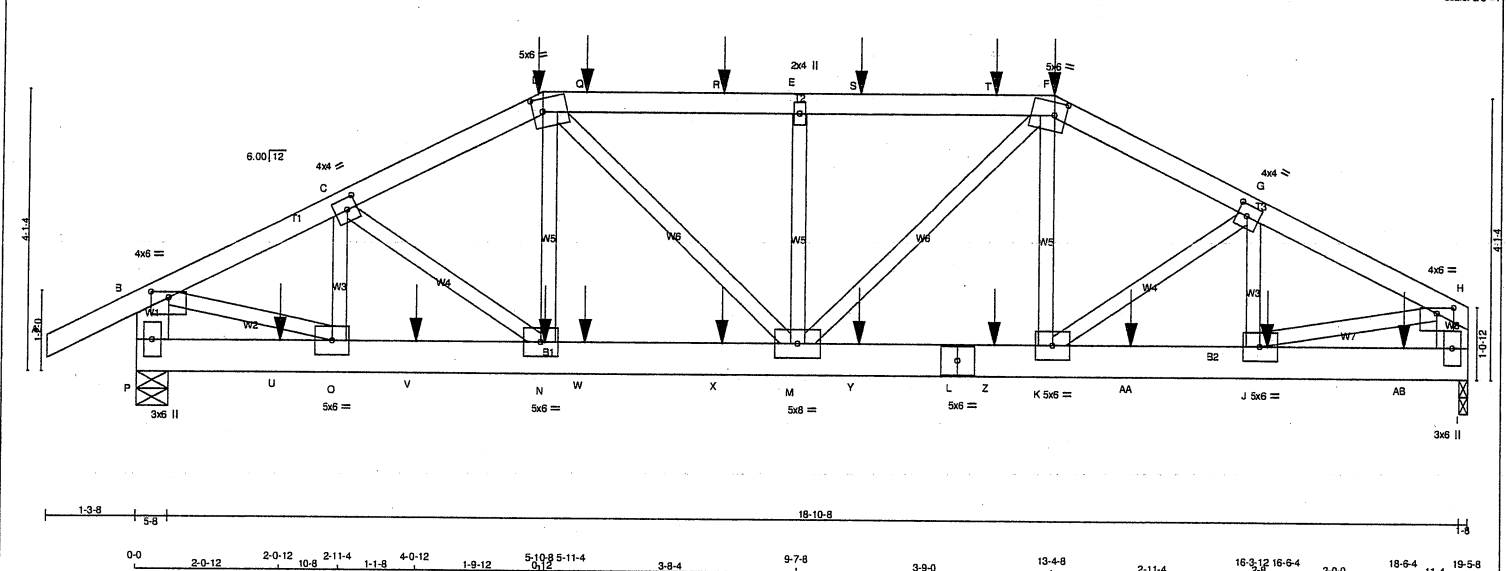
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LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
P - B	2x6	DRY	No.2
I - H	2x6	DRY	No.2
P - L	2x6	DRY	No.2
L - I	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF **2** TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-D	12	SIDE(61.0)
D-F	12	SIDE(61.0)
F-H	12	SIDE(61.0)
P-B	2	TOP
I-H	2	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
P-L	2	SIDE(183.1)
L-I	2	SIDE(183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQRD
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
P	2023	0	2023	0	5-8
I	1877	0	1877	0	5-8

UNFACTORED REACTIONS		1ST LCASE	MAX./MIN. COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1428	953 / 0	0 / 0	0 / 0	0 / 0	475 / 0	0 / 0
I	1328	868 / 0	0 / 0	0 / 0	0 / 0	460 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.21 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED	FACTORED	MAX.	UNBRAC	MEMB.	MAX. FACTORED	MAX.
MEMB.	FORCE (LBS)	VERT.	PLF	LC1	CS1 (LC)	FR-TO	FORCE (LBS)	CS1 (LC)
A-B	0 / 28	-91.8	-91.8	0.07 (1)	10.00	C-C	-582 / 0	0.05 (1)
B-C	-2504 / 0	-91.8	-91.8	0.07 (1)	5.65	C-N	0 / 261	0.03 (1)
C-D	-2763 / 0	-91.8	-91.8	0.08 (1)	5.44	N-D	0 / 127	0.02 (4)
D-Q	-2834 / 0	-91.8	-91.8	0.22 (1)	5.21	D-M	0 / 526	0.07 (1)
Q-R	-2834 / 0	-91.8	-91.8	0.22 (1)	5.21	M-E	-672 / 0	0.09 (1)
R-E	-2834 / 0	-91.8	-91.8	0.22 (1)	5.21	M-F	0 / 515	0.06 (1)
E-S	-2833 / 0	-91.8	-91.8	0.22 (1)	5.21	K-F	0 / 130	0.02 (4)
S-T	-2833 / 0	-91.8	-91.8	0.22 (1)	5.21	K-G	0 / 153	0.02 (1)
T-F	-2833 / 0	-91.8	-91.8	0.22 (1)	5.21	J-G	-485 / 0	0.04 (1)
F-G	-2771 / 0	-91.8	-91.8	0.08 (1)	5.43	B-O	0 / 2326	0.29 (1)
G-H	-2609 / 0	-91.8	-91.8	0.09 (1)	5.55	J-H	0 / 2395	0.30 (1)
P-B	-1954 / 0	0.0	0.0	0.07 (1)	7.81			
I-H	-1792 / 0	0.0	0.0	0.06 (1)	7.81			

CHORDS		MAX. FACTORED	FACTORED	MAX.	UNBRAC	MEMB.	MAX. FACTORED	MAX.
MEMB.	FORCE (LBS)	VERT.	PLF	LC1	CS1 (LC)	FR-TO	FORCE (LBS)	CS1 (LC)
P-U	0 / 0	-18.5	-18.5	0.04 (1)	10.00			
U-O	0 / 0	-18.5	-18.5	0.04 (1)	10.00			
O-V	0 / 2249	-18.5	-18.5	0.18 (1)	10.00			
V-N	0 / 2249	-18.5	-18.5	0.18 (1)	10.00			
N-W	0 / 2460	-18.5	-18.5	0.19 (1)	10.00			
W-X	0 / 2460	-18.5	-18.5	0.19 (1)	10.00			
X-M	0 / 2460	-18.5	-18.5	0.19 (1)	10.00			
M-Y	0 / 2468	-18.5	-18.5	0.19 (1)	10.00			
Y-L	0 / 2468	-18.5	-18.5	0.19 (1)	10.00			
L-Z	0 / 2468	-18.5	-18.5	0.19 (1)	10.00			
Z-K	0 / 2468	-18.5	-18.5	0.19 (1)	10.00			
K-AA	0 / 2342	-18.5	-18.5	0.19 (1)	10.00			
AA-J	0 / 2342	-18.5	-18.5	0.19 (1)	10.00			
J-AB	0 / 0	-18.5	-18.5	0.04 (1)	10.00			
AB-I	0 / 0	-18.5	-18.5	0.04 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)		JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	5-10-8	-317	-317	---	---	---	FRONT	VERT	TOTAL	---	C1
F	13-4-8	-60	-60	---	---	---	FRONT	VERT	DEAD	---	C1
F	13-4-8	-245	-245	---	---	---	FRONT	VERT	SNOW	---	C1
J	16-6-4	-21	-21	---	---	---	FRONT	VERT	TOTAL	---	C1
N	5-11-4	-21	-21	---	---	---	FRONT	VERT	TOTAL	---	C1
Q	6-6-4	-87	-87	---	---	---	FRONT	VERT	TOTAL	---	C1
R	8-6-4	-76	-76	---	---	---	FRONT	VERT	TOTAL	---	C1
S	10-6-4	-76	-76	---	---	---	FRONT	VERT	TOTAL	---	C1
T	12-6-4	-81	-81	---	---	---	FRONT	VERT	TOTAL	---	C1
U	2-0-12	-21	-21	---	---	---	FRONT	VERT	TOTAL	---	C1
V	4-0-12	-21	-21	---	---	---	FRONT	VERT	TOTAL	---	C1

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:				
TOP	CH.	LL	=	25.6 PSF
		DL	=	6.0 PSF
BOT	CH.	LL	=	0.0 PSF
		DL	=	7.4 PSF
TOTAL	LOAD	=		39.0 PSF

SPACING = 24.0 IN/C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL)= L/999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL)= L/999 (0.07")

CS1: TC=0.22/1.00 (D-E:1), BC=0.19/1.00 (J-K:1), WB=0.30/1.00 (H-J:1), SS1=0.15/1.00 (E-F:1)

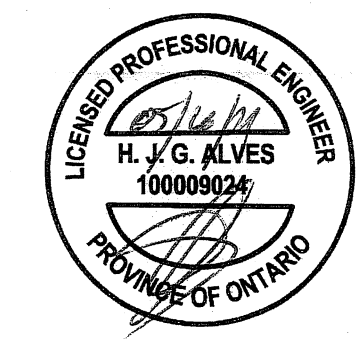
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873
PLATE PLACEMENT TOL = 0.250 inches
PLATE ROTATION TOL = 5.0 Deg.
JSI GRIP= 0.71 (H) (INPUT = 0.90)
JSI METAL= 0.28 (H) (INPUT = 1.00)



Structural component only
DWG# T-2115884 1/12

JOB NAME 413226	TRUSS NAME T202	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MITek Industries, Inc. Wed May 26 14:46:57 2021 Page 2
ID:o2GtC2dhggBEpY0Vu6l93ayYSWr-J8NfslrtInTUSaSbc7OmCfrAYuVYKJ3W fg50NzCcVS

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	6.0	1.00	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	6.0	2.25	1.75
E	TMW-w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TMVW-p	MT20	4.0	6.0	1.00	3.00
I	BMV1+p	MT20	3.0	6.0		
J, K, N, O						
J	BMWW-t	MT20	5.0	6.0		
L	BS-t	MT20	5.0	6.0		
M	BMWWW-t	MT20	5.0	8.0		
P	BMV1+p	MT20	3.0	6.0		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
W	6-6-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
X	8-6-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
Y	10-6-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
Z	12-6-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AA	14-6-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AB	18-6-4	-22	-22	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

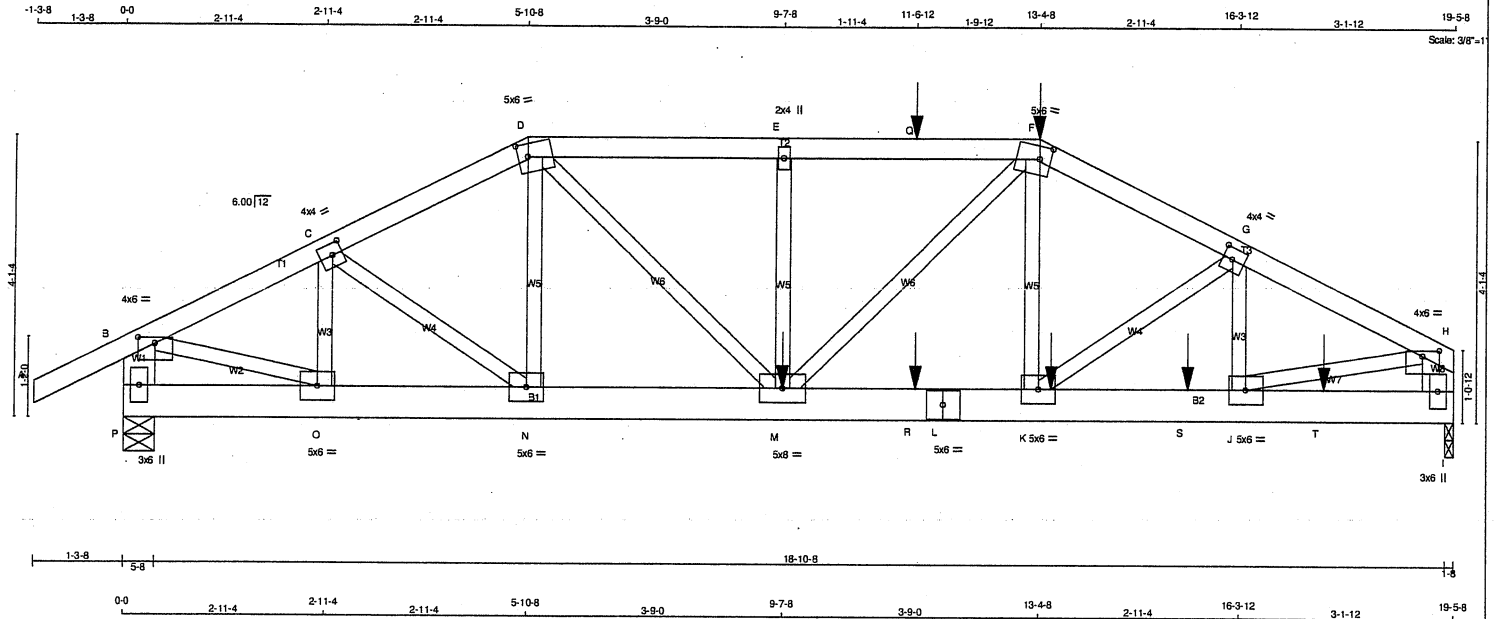


Structural component only
DWG# T-2115884 3/12

JOB NAME 413226	TRUSS NAME T202Z	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed May 26 14:46:58 2021 Page 1
ID:o2GtC2dhqgBEpY0Vu6l93ayYSWr-nKx13esVW5bL4k1oAqv?ktOLalpe3lAgCJ?eYpZCcVR



TOTAL WEIGHT = 2 X 91 = 183 lb [M]

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
P - B	2x6	DRY	No.2
I - H	2x6	DRY	No.2
P - L	2x6	DRY	No.2
L - I	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - D 1	12	TOP
D - F 1	12	SIDE(61.0)
F - H 1	12	SIDE(61.0)
P - B 2	12	TOP
I - H 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
P - L 2	12	SIDE(183.1)
L - I 2	12	SIDE(183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	
E - M 1	6	SIDE(179.5)

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	BRG	IN-SX
P	2146	0	2146	0	0	5-8	5-8		
I	2289	0	2289	0	0	1-8	1-8		

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS								
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL		
P	1510	1036 / 0	0 / 0	0 / 0	0 / 0	474 / 0	0 / 0		
I	1614	1085 / 0	0 / 0	0 / 0	0 / 0	529 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.64 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-91.8 -91.8 0.07 (1)	10.00	C-C	-694 / 0	0.06 (1)	
B-C	-2680 / 0	-91.8 -91.8 0.08 (1)	5.51	C-N	0 / 374	0.05 (1)	
C-D	-3040 / 0	-91.8 -91.8 0.08 (1)	5.24	N-D	-20 / 40	0.01 (4)	
D-E	-3845 / 0	-91.8 -91.8 0.17 (1)	4.68	D-M	0 / 1603	0.20 (1)	
E-Q	-3845 / 0	-91.8 -91.8 0.20 (1)	4.64	M-E	-462 / 0	0.06 (1)	
Q-F	-3845 / 0	-91.8 -91.8 0.20 (1)	4.64	M-F	0 / 949	0.12 (1)	
F-G	-3558 / 0	-91.8 -91.8 0.10 (1)	4.91	K-F	0 / 122	0.02 (4)	
G-H	-3249 / 0	-91.8 -91.8 0.10 (1)	5.08	K-G	0 / 317	0.04 (1)	
P-B	-2075 / 0	0.0 0.0 0.07 (1)	7.81	J-G	-646 / 0	0.06 (1)	
I-H	-2202 / 0	0.0 0.0 0.08 (1)	7.81	B-O	0 / 2488	0.31 (1)	
				J-H	0 / 2979	0.37 (1)	
P-O	0 / 0	-18.5 -18.5 0.05 (1)	10.00				
O-N	0 / 2406	-18.5 -18.5 0.20 (1)	10.00				
N-M	0 / 2706	-18.5 -18.5 0.24 (1)	10.00				
M-R	0 / 3171	-18.5 -18.5 0.27 (1)	10.00				
R-L	0 / 3171	-18.5 -18.5 0.27 (1)	10.00				
L-K	0 / 3171	-18.5 -18.5 0.27 (1)	10.00				
K-S	0 / 2914	-18.5 -18.5 0.24 (1)	10.00				
S-J	0 / 2914	-18.5 -18.5 0.24 (1)	10.00				
J-T	0 / 0	-18.5 -18.5 0.05 (1)	10.00				
T-I	0 / 0	-18.5 -18.5 0.05 (1)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	13-4-8	-80	-80		FRONT	VERT	DEAD		C1
F	13-4-8	-81	-81		BACK	VERT	TOTAL		C1
F	13-4-8	-245	-245		FRONT	VERT	SNOW		C1
K	13-6-12	-21	-21		BACK	VERT	TOTAL		C1
M	9-7-8	-974	-974		BACK	VERT	TOTAL		C1
Q	11-6-12	-76	-76		BACK	VERT	TOTAL		C1
R	11-6-12	-21	-21		BACK	VERT	TOTAL		C1
S	15-6-12	-21	-21		BACK	VERT	TOTAL		C1
T	17-6-12	-21	-21		BACK	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL)= L/999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL)= L/999 (0.09")

CSI: TC=0.20/1.00 (E-F:1), BC=0.27/1.00 (K-M:1),
WB=0.37/1.00 (H-J:1), SSI=0.11/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.35 (H) (INPUT = 1.00)



Structural component only
DWG# T-2115885

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
413226	T202Z	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.420 5 Jan 21 2021 MiTek Industries, Inc. Wed May 26 14:46:58 2021 Page 2

ID:o2GtC2dhggBEpY0Vu6l93ayYSWr-nKx13esVW5bL4k1oAqv?ktOLaIpe3IAgCJ?eYpzCcVR

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	6.0	1.00	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	6.0	2.25	1.75
E	TMW-w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TMVW-p	MT20	4.0	6.0	1.00	3.00
I	BMV1+p	MT20	3.0	6.0		
J, K, N, O						
J	BMWW-t	MT20	5.0	6.0		
L	BS-t	MT20	5.0	6.0		
M	BMWWW-t	MT20	5.0	8.0		
P	BMV1+p	MT20	3.0	6.0		

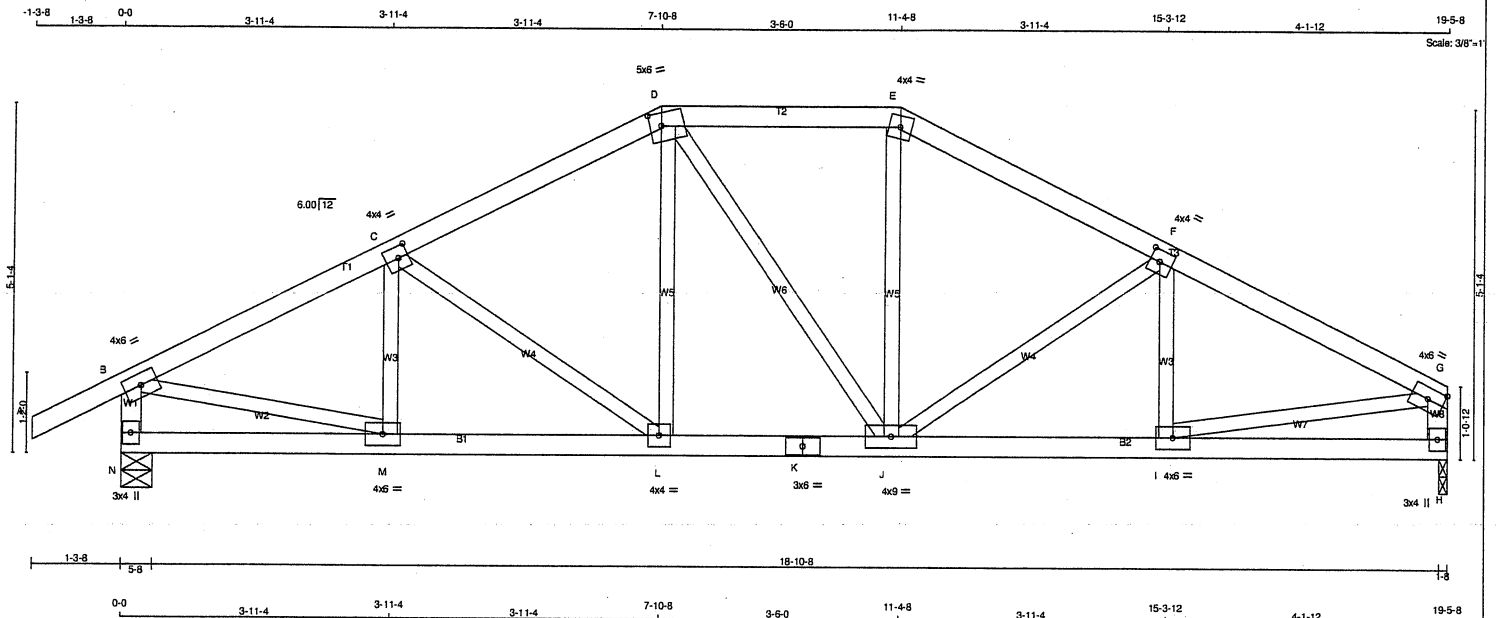


Structural component only
DWG# T-2115885 *ml*

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
413226	T203	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MTek Industries, Inc. Wed May 26 14:47:00 2021 Page 1
ID:o2GtC2dhqgBEpY0V6l93ayYSWr-ji3oUJl2ir2K1BAIFyTqIth16WYXgnygdUlcizCcVP



TOTAL WEIGHT = 2 X 80 = 159 lb
(M/F)

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
N - B	2x4	DRY	No.2
H - G	2x4	DRY	No.2
N - K	2x4	DRY	No.2
K - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMVW-t	MT20	4.0	6.0
C	TMVW-t	MT20	4.0	4.0 2.00 1.75
D	TTWW-m	MT20	5.0	6.0 2.25 2.00
E	TTW-m	MT20	4.0	4.0
F	TMVW-t	MT20	4.0	4.0 2.00 1.75
G	TMVW-t	MT20	4.0	6.0 Edge
H	BMV1+p	MT20	3.0	4.0
I	BMVW-t	MT20	4.0	6.0
J	BMVW-t	MT20	4.0	9.0
K	BS-t	MT20	3.0	6.0
L	BMVW-t	MT20	4.0	4.0
M	BMVW-t	MT20	4.0	6.0
N	BMV1+p	MT20	3.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
N	1197	0	1197	0	5-8
H	1073	0	1073	0	1-8

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	844	567 / 0	0 / 0	0 / 0	0 / 0	277 / 0	0 / 0
H	759	498 / 0	0 / 0	0 / 0	0 / 0	261 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	
FR-TO				FR-TO			
A-B	0 / 28	-91.8 -91.8 0.12 (1)	10.00	M-C	-197 / 0	0.04 (1)	
B-C	-1382 / 0	-91.8 -91.8 0.19 (1)	5.34	C-L	-251 / 0	0.10 (1)	
C-D	-1189 / 0	-91.8 -91.8 0.18 (1)	5.66	L-D	0 / 212	0.05 (1)	
D-E	-1060 / 0	-91.8 -91.8 0.16 (1)	5.94	D-J	0 / 15	0.00 (1)	
E-F	-1200 / 0	-91.8 -91.8 0.19 (1)	5.63	J-E	0 / 231	0.05 (1)	
F-G	-1437 / 0	-91.8 -91.8 0.20 (1)	5.25	J-F	-301 / 0	0.12 (1)	
N-B	-1164 / 0	0.0 0.0 0.12 (1)	7.39	I-F	-160 / 14	0.03 (1)	
H-G	-1037 / 0	0.0 0.0 0.10 (1)	7.71	B-M	0 / 1282	0.29 (1)	
				I-G	0 / 1323	0.30 (1)	
N-M	0 / 0	-18.5 -18.5 0.06 (4)	10.00				
M-L	0 / 1252	-18.5 -18.5 0.23 (1)	10.00				
L-K	0 / 1051	-18.5 -18.5 0.20 (1)	10.00				
K-J	0 / 1051	-18.5 -18.5 0.20 (1)	10.00				
J-I	0 / 1302	-18.5 -18.5 0.24 (1)	10.00				
I-H	0 / 0	-18.5 -18.5 0.06 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/999 (0.07")

CSI: TC=0.20/1.00 (F-G:1), BC=0.24/1.00 (I-J:1), WB=0.30/1.00 (G-I:1), SS=0.16/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (M) (INPUT = 0.90)
JSI METAL= 0.36 (G) (INPUT = 1.00)

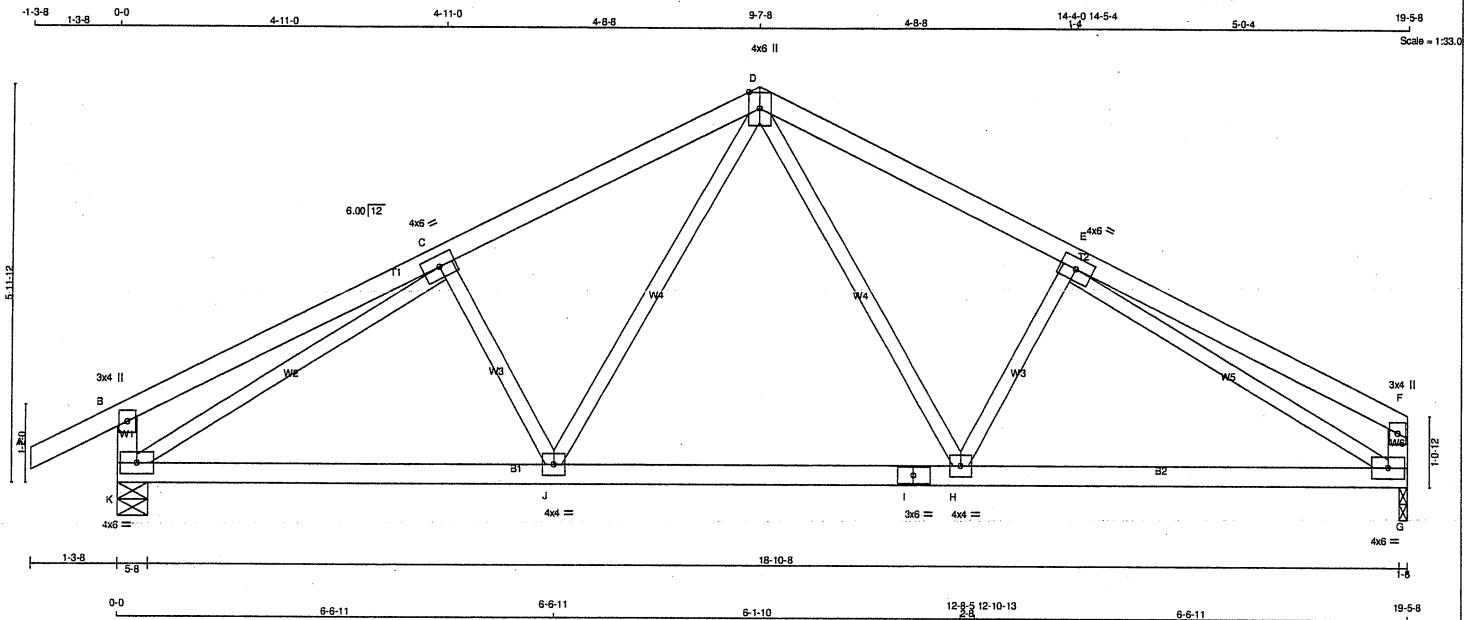


Structural component only
DWG# T-2115886

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
413226	T204	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 75 = 150 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
K - B	2x4	DRY	No.2
G - F	2x4	DRY	No.2
K - I	2x4	DRY	No.2
I - G	2x4	DRY	No.2

ALL WEBS EXCEPT			
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
C	TMWW-t	MT20	4.0	6.0	
D	TTWW+p	MT20	4.0	6.0	Edge
E	TMWW-t	MT20	4.0	6.0	
F	TMV+p	MT20	3.0	4.0	
G	BMVW1-t	MT20	4.0	6.0	
H	BMWW-t	MT20	4.0	4.0	
I	BS-t	MT20	3.0	6.0	
J	BMWW-t	MT20	4.0	4.0	
K	BMVW1-t	MT20	4.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	IN-SX
K	1197	0	1197	0	0	5-8	5-8		
G	1073	0	1073	0	0	1-8	1-8		

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
K	844	567 / 0	0 / 0	0 / 0	0 / 0	277 / 0	0 / 0
G	759	498 / 0	0 / 0	0 / 0	0 / 0	261 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX LC1 (LC)	UNBRAC LENGTH
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-91.8 -91.8	0.12 (1)	10.00	C-J	-286 / 0	0.07 (1)
B-C	0 / 23	-91.8 -91.8	0.34 (1)	10.00	J-D	0 / 431	0.10 (1)
C-D	-1276 / 0	-91.8 -91.8	0.28 (1)	5.39	D-H	0 / 472	0.11 (1)
D-E	-1300 / 0	-91.8 -91.8	0.29 (1)	5.33	H-E	-323 / 0	0.08 (1)
E-F	0 / 23	-91.8 -91.8	0.35 (1)	10.00	K-C	-1517 / 0	0.85 (1)
K-B	-295 / 0	0.0 0.0	0.03 (1)	7.81	E-G	-1542 / 0	0.93 (1)
G-F	-179 / 0	0.0 0.0	0.02 (1)	7.81			
K-J	0 / 1256	-18.5 -18.5	0.30 (1)	10.00			
J-I	0 / 916	-18.5 -18.5	0.25 (4)	10.00			
I-H	0 / 916	-18.5 -18.5	0.25 (4)	10.00			
H-G	0 / 1293	-18.5 -18.5	0.31 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
DL = 6.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.4	PSF	
TOTAL LOAD = 39.0	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.65")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.05")
ALLOWABLE DEFL.(TL) = $L/360$ (0.65")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.10")

CSI: TC=0.35/1.00 (E-F:1), BC=0.31/1.00 (G-H:1),
WB=0.93/1.00 (E-G:1), SS=0.20/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (C) (INPUT = 0.90)
JSI METAL= 0.35 (E) (INPUT = 1.00)

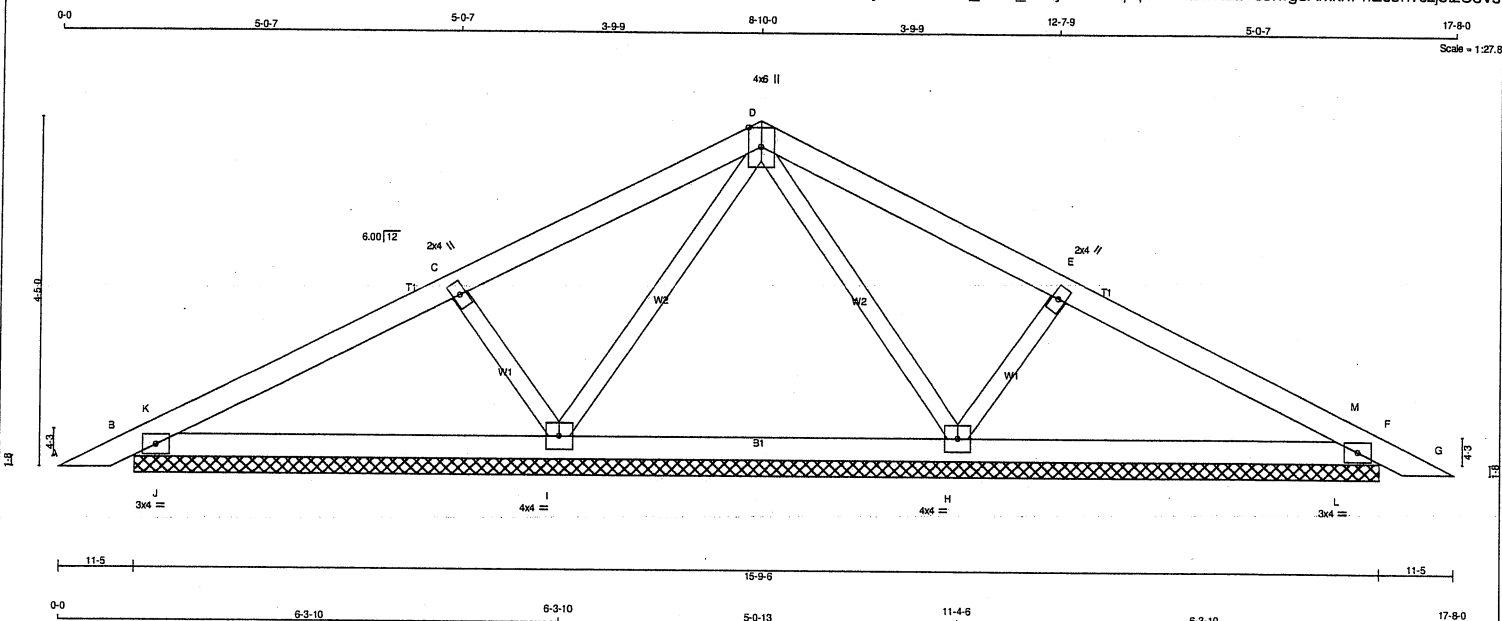


Structural component only
DWG# T-2115887

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
413226	PB06	17	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER						
N. L. G. A. RULES						
CHORDS		SIZE	LUMBER		DESCR.	
A - D	2x4	DRY	No.2			SPF
D - G	2x4	DRY	No.2			SPF
B - F	2x4	DRY	No.2			SPF
ALL WEBS		2x3	DRY	No.2		SPF
DRY: SEASONED LUMBER.						

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0			
C	TMW+w	MT20	2.0	4.0			
D	TTWW+p	MT20	4.0	6.0	Edge		
E	TMW+w	MT20	2.0	4.0			
F	TMB1-I	MT20	3.0	4.0			
H	BMWW1-I	MT20	4.0	4.0			
I	BMWW1-I	MT20	4.0	4.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
B	340	0	340	0	15-9-6	15-9-6
F	340	0	340	0	15-9-6	15-9-6
H	610	0	610	0	15-9-6	15-9-6
I	610	0	610	0	15-9-6	15-9-6

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	239	166 / 0	0 / 0	0 / 0	0 / 0	73 / 0	0 / 0
F	239	166 / 0	0 / 0	0 / 0	0 / 0	73 / 0	0 / 0
H	432	283 / 0	0 / 0	0 / 0	0 / 0	149 / 0	0 / 0
I	432	283 / 0	0 / 0	0 / 0	0 / 0	149 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, H, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		MAX. UNBRAC LENGTH	MEMB.	WEBS	
	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	MAX. FORCE (LBS)			MAX. FORCE (LBS)	MAX. CSI (LC)
FR-TO						FR-TO		
A-B	0 / 17	-91.8	-91.8	0.05 (1)	10.00	D-H	-197 / 0	0.07 (1)
B-K	-199 / 0	-91.8	-91.8	0.06 (4)	6.25	H-E	-396 / 0	0.07 (1)
K-C	-143 / 0	-91.8	-91.8	0.23 (1)	6.25	I-D	-197 / 0	0.07 (1)
C-D	0 / 54	-91.8	-91.8	0.24 (1)	10.00	C-I	-396 / 0	0.07 (1)
D-E	0 / 54	-91.8	-91.8	0.24 (1)	10.00	J-K	-69 / 58	0.00 (1)
E-M	-143 / 0	-91.8	-91.8	0.23 (1)	6.25	L-M	-69 / 58	0.00 (1)
M-F	-199 / 0	-91.8	-91.8	0.06 (4)	6.25			
F-G	0 / 17	-91.8	-91.8	0.05 (1)	10.00			
B-J	0 / 149	-18.5	-18.5	0.08 (1)	10.00			
J-I	0 / 149	-18.5	-18.5	0.12 (4)	10.00			
I-H	0 / 40	-18.5	-18.5	0.11 (4)	10.00			
H-L	0 / 149	-18.5	-18.5	0.12 (4)	10.00			
L-F	0 / 149	-18.5	-18.5	0.08 (1)	10.00			

TOTAL WEIGHT = 17 X 51 = 873 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.24/1.00 (C-D:1), BC=0.12/1.00 (I-J:4), WB=0.07/1.00 (D-I:1), SSI=0.15/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (F) (INPUT = 0.90)

JSI METAL= 0.08 (C) (INPUT = 1.00)

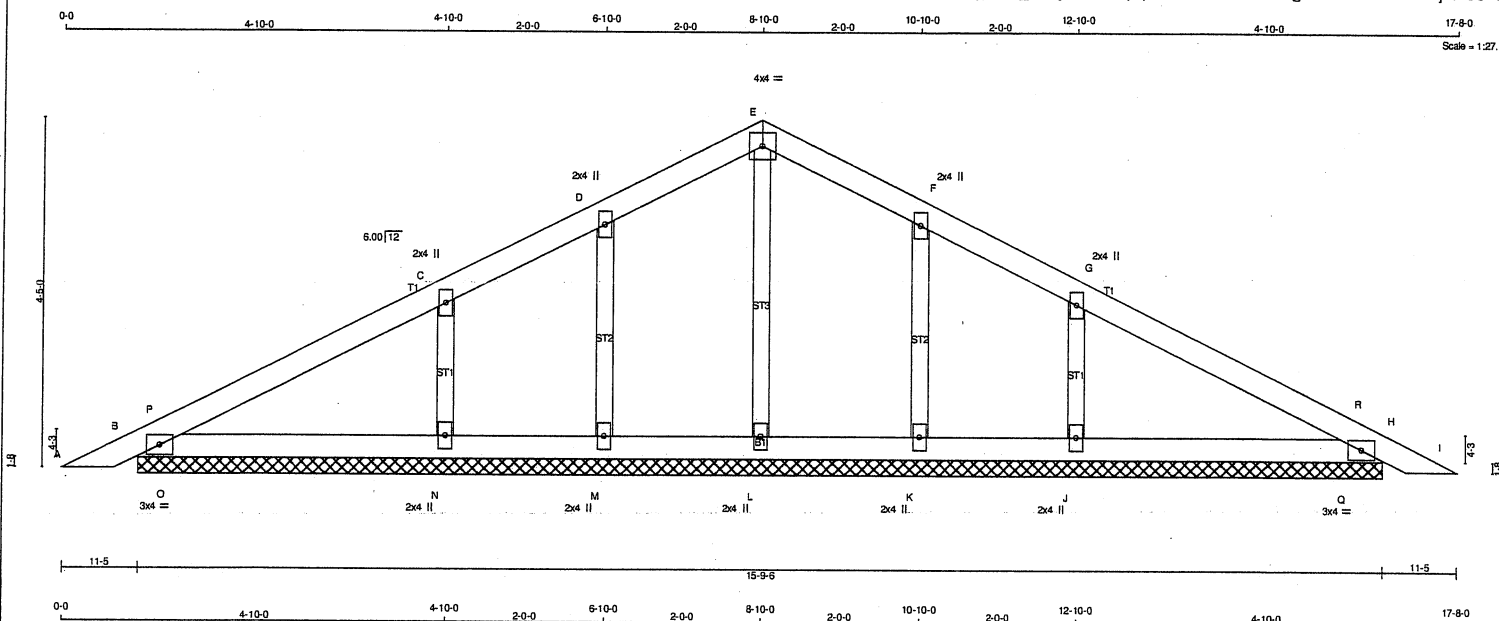


Structural component only
DWG# T-2115839

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
413226	PB06G	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 51 = 102 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - E	2x4	DRY	No.2
E - I	2x4	DRY	No.2
B - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B TMB1-I	MT20	3.0	4.0	
C, D, F, G				
C TMW+w	MT20	2.0	4.0	
E TTW-p	MT20	4.0	4.0	
H TMB1-I	MT20	3.0	4.0	
J, K, L, M, N				
J BMW1+w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	UNBRAC LENGTH	LC1 MAX (LC)
FR-TO		FROM TO		FR-TO			
A-B	0 / 17	-91.8 -91.8	0.05 (1)	L-E	-131 / 0	10.00	0.04 (1)
B-P	-44 / 0	-91.8 -91.8	0.02 (4)	M-D	-160 / 0	6.25	0.03 (1)
P-C	-44 / 0	-91.8 -91.8	0.13 (1)	N-C	-306 / 0	6.25	0.05 (1)
C-D	-60 / 0	-91.8 -91.8	0.13 (1)	K-F	-160 / 0	6.25	0.03 (1)
D-E	-45 / 0	-91.8 -91.8	0.05 (1)	J-G	-306 / 0	6.25	0.05 (1)
E-F	-45 / 0	-91.8 -91.8	0.05 (1)	O-P	-171 / 5	6.25	0.00 (1)
F-G	-60 / 0	-91.8 -91.8	0.13 (1)	Q-R	-171 / 5	6.25	0.00 (1)
G-R	-44 / 0	-91.8 -91.8	0.13 (1)				
R-H	-44 / 0	-91.8 -91.8	0.02 (4)				
H-I	0 / 17	-91.8 -91.8	0.05 (1)				
B-O	0 / 49	-18.5 -18.5	0.10 (1)				
O-N	0 / 49	-18.5 -18.5	0.10 (1)				
N-M	0 / 39	-18.5 -18.5	0.07 (1)				
M-L	0 / 36	-18.5 -18.5	0.02 (4)				
L-K	0 / 36	-18.5 -18.5	0.02 (4)				
K-J	0 / 39	-18.5 -18.5	0.07 (1)				
J-Q	0 / 49	-18.5 -18.5	0.10 (1)				
Q-H	0 / 49	-18.5 -18.5	0.10 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6	PSF
DL	= 6.0	PSF
BOT CH. LL	= 0.0	PSF
DL	= 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13/1.00 (C-P:1), BC=0.10/1.00 (B-O:1), WB=0.05/1.00 (C-N:1), SSI=0.14/1.00 (H-Q:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (H) (INPUT = 0.90)

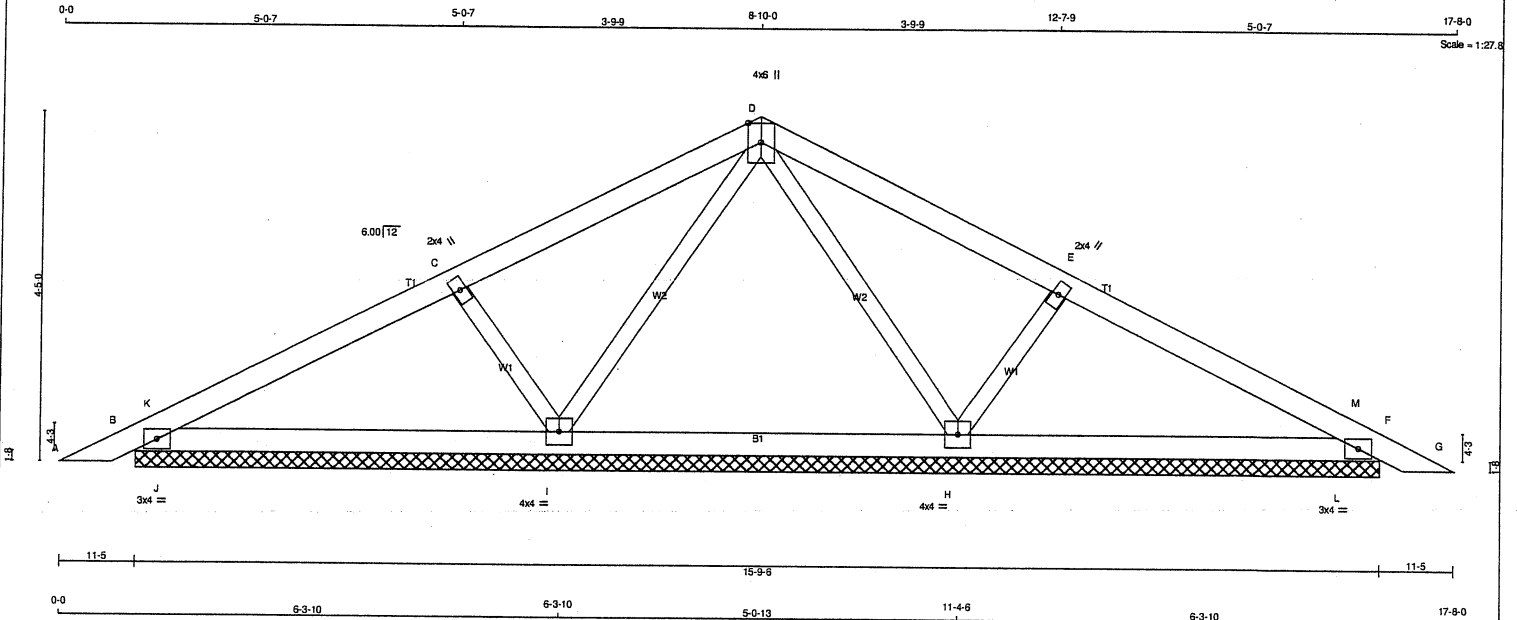
JSI METAL= 0.13 (G) (INPUT = 1.00)



Structural component only
DWG# T-2115840

JOB NAME 413226	TRUSS NAME PB06Z	QUANTITY 1	PLY 3	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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TOTAL WEIGHT = 3 X 51 = 154 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - G	2x4	DRY	No.2	SPF	
B - F	2x4	DRY	No.2	SPF	
ALL WEBS 2x3 DRY No.2 SPF					
DRY: SEASONED LUMBER.					

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - D 1 12	TOP	
D - G 1 12	TOP	
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
B - F 1 12	TOP	
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1 6		

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
B	340	0	340	0	0	15-9-6	15-9-6
F	340	0	340	0	0	15-9-6	15-9-6
H	610	0	610	0	0	15-9-6	15-9-6
I	610	0	610	0	0	15-9-6	15-9-6

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. SNOW	MAX/MIN. LIVE	MAX/MIN. PERM. LIVE	MAX/MIN. WIND	MAX/MIN. DEAD	MAX/MIN. SOIL
B	239	166 / 0	0 / 0	0 / 0	0 / 0	73 / 0	0 / 0
F	239	166 / 0	0 / 0	0 / 0	0 / 0	73 / 0	0 / 0
H	432	283 / 0	0 / 0	0 / 0	0 / 0	149 / 0	0 / 0
I	432	283 / 0	0 / 0	0 / 0	0 / 0	149 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, H, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	LC1 MAX (PLF)	MAX. UNBRACED LENGTH	MEMB.	FORCE (LBS)	MAX. UNBRACED LENGTH
FR-TO					FR-TO		
A-B	0 / 17	-91.8	-91.8 0.02 (1)	10.00	D-H	-197 / 0	0.02 (1)
B-K	-199 / 0	-91.8	-91.8 0.02 (4)	6.25	H-E	-396 / 0	0.02 (1)
K-C	-144 / 0	-91.8	-91.8 0.08 (1)	6.25	I-D	-197 / 0	0.02 (1)
C-D	0 / 54	-91.8	-91.8 0.08 (1)	10.00	C-I	-396 / 0	0.02 (1)
D-E	0 / 54	-91.8	-91.8 0.08 (1)	10.00	J-K	-68 / 57	0.00 (1)
E-M	-144 / 0	-91.8	-91.8 0.08 (1)	6.25	L-M	-68 / 57	0.00 (1)
M-F	-199 / 0	-91.8	-91.8 0.02 (4)	6.25			
F-G	0 / 17	-91.8	-91.8 0.02 (1)	10.00			
B-J	0 / 149	-18.5	-18.5 0.03 (1)	10.00			
J-I	0 / 149	-18.5	-18.5 0.04 (4)	10.00			
I-H	0 / 40	-18.5	-18.5 0.04 (4)	10.00			
H-L	0 / 149	-18.5	-18.5 0.04 (4)	10.00			
L-F	0 / 149	-18.5	-18.5 0.03 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.08/1.00 (C-D:1), BC=0.04/1.00 (I-J:4), WB=0.02/1.00 (D-I:1), SSI=0.05/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

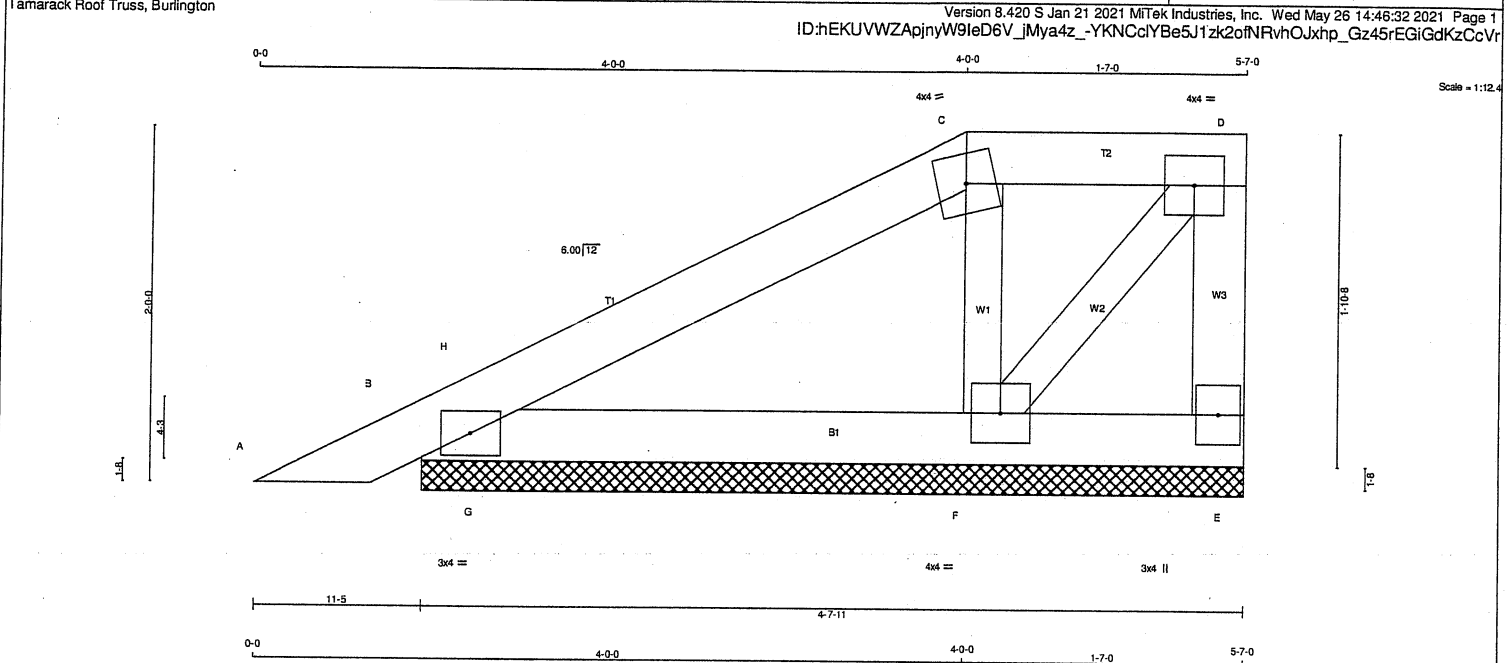
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.08 (F) (INPUT = 0.90)
JSI METAL = 0.03 (C) (INPUT = 1.00)



Structural component only
DWG# T-2115841

JOB NAME 413226	TRUSS NAME PB101	QUANTITY 4	PLY 1	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
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LUMBER					DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER										TOTAL WEIGHT = 4 X 16 = 64 lb			
N. L. G. A. RULES					BEARINGS										DESIGN CRITERIA			
CHORDS SIZE															SPECIFIED LOADS:			
A - C	2x4	DRY	No.2	DESCR.	SPF	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG	TOP CH. LL = 25.8 PSF								
C - D	2x4	DRY	No.2	SPF	JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	DL = 6.0 PSF					
E - D	2x4	DRY	No.2	SPF	E	76	0	76	0	0	4-7-11	4-7-11	LL = 0.0 PSF					
B - E	2x4	DRY	No.2	SPF	B	252	0	252	0	0	4-7-11	4-7-11	DL = 7.4 PSF					
ALL WEBS 2x3 DRY										F	264	0	264	0	0	4-7-11	4-7-11	TOTAL LOAD = 39.0 PSF
DRY: SEASONED LUMBER.																		

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-t	MT20	3.0	4.0	
C	TTW-m	MT20	4.0	4.0	
D	TMVW-t	MT20	4.0	4.0	
E	BMV1+p	MT20	3.0	4.0	
F	BMWV1-t	MT20	4.0	4.0	

UNFACTORED REACTIONS							
JT	1ST LCASE	MAX/MIN.	COMPONENT REACTIONS				
E	COMBINED	SNOW	LIVE	PERM.	LIVE	WIND	DEAD
B	53	39 / 0	0 / 0	0 / 0	0 / 0	0 / 0	14 / 0
F	177	126 / 0	0 / 0	0 / 0	0 / 0	0 / 0	50 / 0
F	188	118 / 0	0 / 0	0 / 0	0 / 0	0 / 0	70 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING				TOTAL LOAD CASES: (4)			
CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LOAD LC1 MAX (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (PLF)	MAX. LOAD LC1 MAX (LC)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 17	-91.8	-91.8 0.05 (1)	F-C	-176 / 0	0.03 (1)	0.03 (1)
B-H	-18 / 0	-91.8	-91.8 0.02 (1)	F-D	0 / 24	0.01 (1)	0.01 (1)
H-C	-33 / 0	-91.8	-91.8 0.10 (1)	G-H	-170 / 0	0.00 (1)	0.00 (1)
C-D	-16 / 0	-91.8	-91.8 0.04 (1)				
E-D	-90 / 0	0.0	0.0 0.01 (1)				
B-G	0 / 28	-18.5	-18.5 0.09 (1)				
G-F	0 / 28	-18.5	-18.5 0.09 (1)				
F-E	0 / 0	-18.5	-18.5 0.06 (1)				

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.10/1.00 (C-H:1), BC=0.09/1.00 (F-G:1), WB=0.03/1.00 (C-F:1), SSI=0.13/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

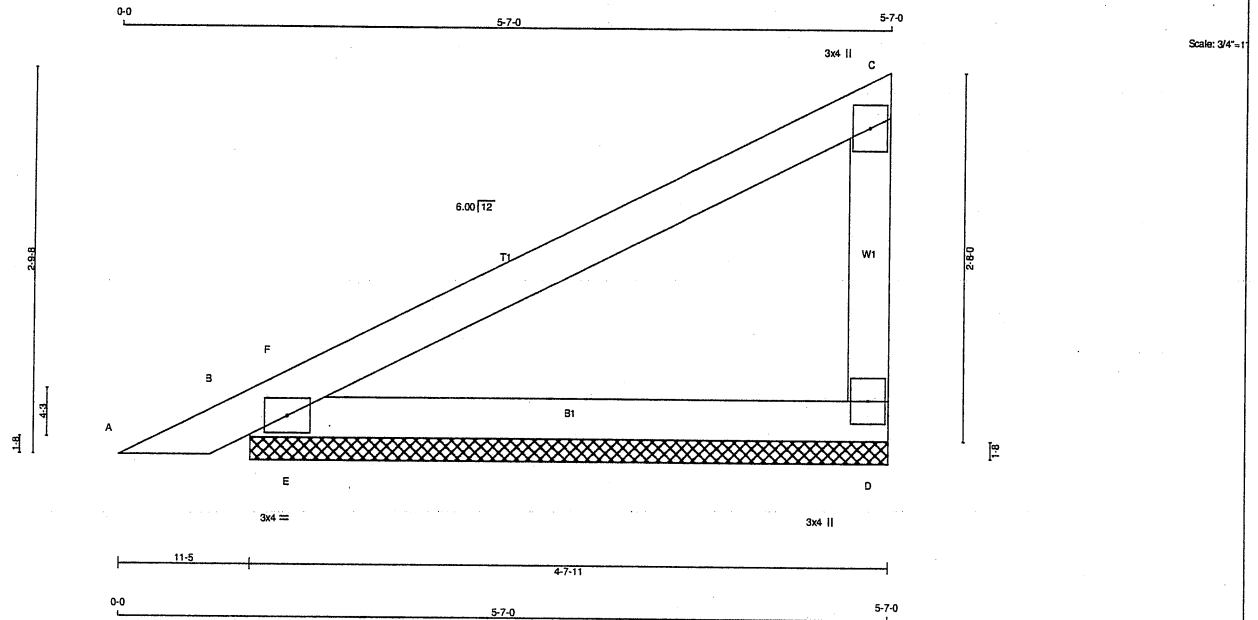
JSI GRIP= 0.20 (B) (INPUT = 0.90)
JSI METAL= 0.06 (C) (INPUT = 1.00)



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
413226	PB102	7	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 7 X 15 = 103 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2	
D - C	2x4	DRY	No.2	
B - D	2x4	DRY	No.2	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TMV+p	MT20	3.0	4.0		
D	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
D	256	0	256	0	4-7-11	4-7-11
B	336	0	336	0	4-7-11	4-7-11

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	181	119 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0
B	236	164 / 0	0 / 0	0 / 0	0 / 0	73 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LC1 CSI (LC)		
FR-TO		FROM TO			FR-TO				
A-B	0 / 17	-91.8	-91.8	0.05 (1)	10.00	E-F	-310 / 7	0.00 (1)	
B-F	-21 / 45	-91.8	-91.8	0.06 (1)	6.25				
F-C	-4 / 2	-91.8	-91.8	0.26 (1)	10.00				
D-C	-186 / 0	0.0	0.0	0.02 (1)	7.81				
B-E	0 / 0	-18.5	-18.5	0.18 (1)	10.00				
E-D	0 / 0	-18.5	-18.5	0.18 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.26/1.00 (C-F:1), BC=0.18/1.00 (D-E:1), WB=0.00/1.00 (E-F:1), SSI=0.25/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

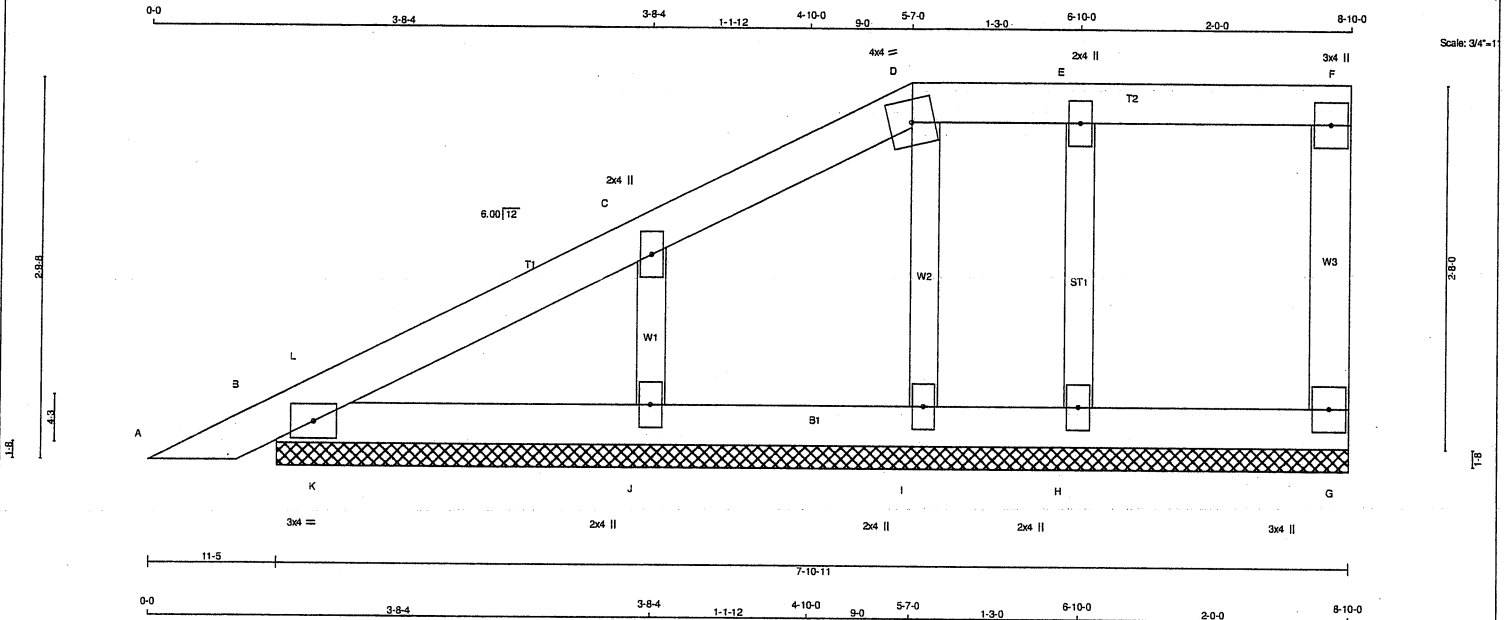
JSI GRIP= 0.26 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)



Structural component only
DWG# T-2115843

JOB NAME 413226	TRUSS NAME PB103G	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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TOTAL WEIGHT = 2 X 27 = 53 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
G - F	2x4	DRY	No.2
B - G	2x4	DRY	No.2

ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	
C	TMW+w	MT20	2.0	4.0	
D	TTW-m	MT20	4.0	4.0	
E	TMW+w	MT20	2.0	4.0	
F	TMV+p	MT20	3.0	4.0	
G	BMV1+p	MT20	3.0	4.0	
H, I, J					
H	BMW1+w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX. (CSI (LC))	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. (CSI (LC))
FR-TO		FROM	TO		FR-TO		
A-B	0 / 17	-91.8	-91.8 0.05 (1)	10.00	H-E	-191 / 0	0.03 (1)
B-L	-23 / 0	-91.8	-91.8 0.01 (4)	6.25	I-D	-92 / 0	0.02 (1)
L-C	-6 / 0	-91.8	-91.8 0.08 (1)	10.00	J-C	-243 / 0	0.03 (1)
C-D	-17 / 0	-91.8	-91.8 0.08 (1)	6.25	K-L	-85 / 3	0.00 (1)
D-E	0 / 0	-91.8	-91.8 0.04 (1)	10.00			
E-F	0 / 0	-91.8	-91.8 0.04 (1)	10.00			
G-F	-76 / 0	0.0	0.0 0.01 (1)	7.81			
B-K	0 / 16	-18.5	-18.5 0.05 (1)	10.00			
K-J	0 / 16	-18.5	-18.5 0.05 (1)	10.00			
J-I	0 / 4	-18.5	-18.5 0.03 (1)	10.00			
I-H	0 / 0	-18.5	-18.5 0.02 (4)	10.00			
H-G	0 / 0	-18.5	-18.5 0.02 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.6	PSF
	DL	= 6.0	PSF
BOT CH.	LL	= 0.0	PSF
	DL	= 7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.08/1.00 (C-D:1), BC=0.05/1.00 (B-K:1), WB=0.03/1.00 (C-J:1), SSI=0.09/1.00 (C-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (B) (INPUT = 0.90)

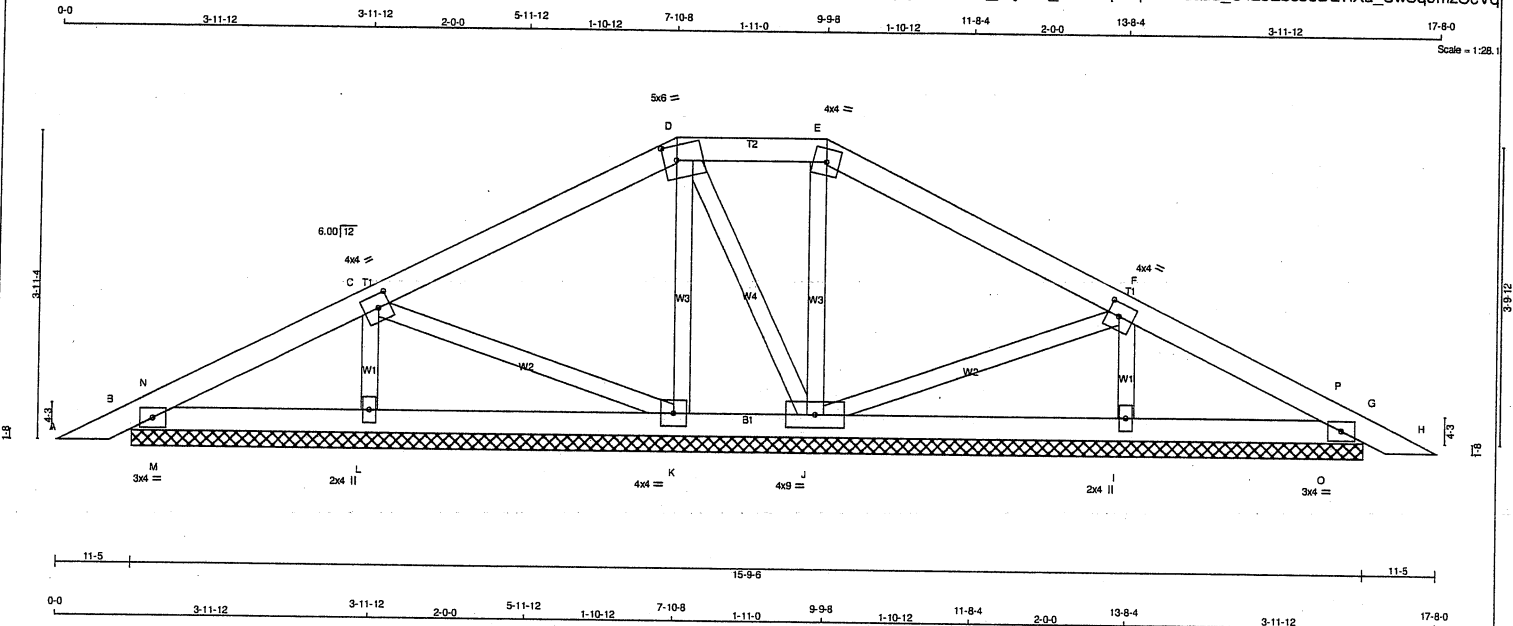
JSI METAL= 0.10 (C) (INPUT = 1.00)



Structural component only
DWG# T-2115844

JOB NAME 413226	TRUSS NAME PB104	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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TOTAL WEIGHT = 57 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
E - H	2x4	DRY	No.2	SPF	
B - G	2x4	DRY	No.2	SPF	
ALL WEBS 2x3 DRY No.2 SPF					
DRY: SEASONED LUMBER.					

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0			
C	TMWW-t	MT20	4.0	4.0	2.00	1.75	
D	TTWW-m	MT20	5.0	6.0	2.25	2.00	
E	TTWW-m	MT20	4.0	4.0			
F	TMWW-t	MT20	4.0	4.0	2.00	1.75	
G	TMB1-I	MT20	3.0	4.0			
I	BMW1-w	MT20	2.0	4.0			
J	BMWW1-t	MT20	4.0	9.0			
K	BMWW1-t	MT20	4.0	4.0			
L	BMW1-w	MT20	2.0	4.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT		REQD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
B	222	0	222	0	0	15-9-6	15-9-6
G	213	0	213	0	0	15-9-6	15-9-6
K	205	0	205	0	0	15-9-6	15-9-6
J	385	0	385	0	0	15-9-6	15-9-6
L	447	0	447	0	0	15-9-6	15-9-6
I	428	0	428	0	0	15-9-6	15-9-6

UNFACTORED REACTIONS							
	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SC
B	155	112 / 0	0 / 0	0 / 0	0 / 0	43 / 0	0 / 0
G	149	108 / 0	0 / 0	0 / 0	0 / 0	42 / 0	0 / 0
K	147	86 / 0	0 / 0	0 / 0	0 / 0	61 / 0	0 / 0
J	271	187 / 0	0 / 0	0 / 0	0 / 0	84 / 0	0 / 0
L	316	208 / 0	0 / 0	0 / 0	0 / 0	108 / 0	0 / 0
I	303	197 / 0	0 / 0	0 / 0	0 / 0	106 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, G, K, J, L, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	
FR-TO		FROM TO		FR-TO			
A-B	0 / 17	-91.8 -91.8 0.05 (1)	10.00	D-J	-86 / 0	0.02 (1)	
B-N	-80 / 0	-91.8 -91.8 0.01 (1)	6.25	C-K	-37 / 0	0.01 (1)	
N-C	-39 / 0	-91.8 -91.8 0.19 (1)	6.25	J-F	-59 / 0	0.02 (1)	
C-D	-42 / 0	-91.8 -91.8 0.19 (1)	6.25	K-D	-143 / 0	0.03 (1)	
D-E	0 / 21	-91.8 -91.8 0.06 (1)	10.00	J-E	-238 / 0	0.05 (1)	
E-F	0 / 0	-91.8 -91.8 0.19 (1)	10.00	L-C	-361 / 0	0.05 (1)	
F-P	-21 / 0	-91.8 -91.8 0.19 (1)	6.25	I-F	-343 / 0	0.05 (1)	
P-G	-63 / 0	-91.8 -91.8 0.02 (1)	6.25	M-N	-50 / 4	0.00 (1)	
G-H	0 / 17	-91.8 -91.8 0.05 (1)	10.00	O-P	-50 / 4	0.00 (1)	
B-M	0 / 57	-18.5 -18.5 0.04 (1)	10.00				
M-L	0 / 57	-18.5 -18.5 0.06 (4)	10.00				
L-K	0 / 57	-18.5 -18.5 0.06 (4)	10.00				
K-J	0 / 19	-18.5 -18.5 0.04 (4)	10.00				
J-I	0 / 42	-18.5 -18.5 0.06 (4)	10.00				
I-O	0 / 42	-18.5 -18.5 0.06 (4)	10.00				
O-G	0 / 42	-18.5 -18.5 0.04 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.19/1.00 (F-P:1), BC=0.06/1.00 (K-L:4), WB=0.05/1.00 (E-J:1), SS=0.15/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	MAX MIN	
MT20	650	371	1747	788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

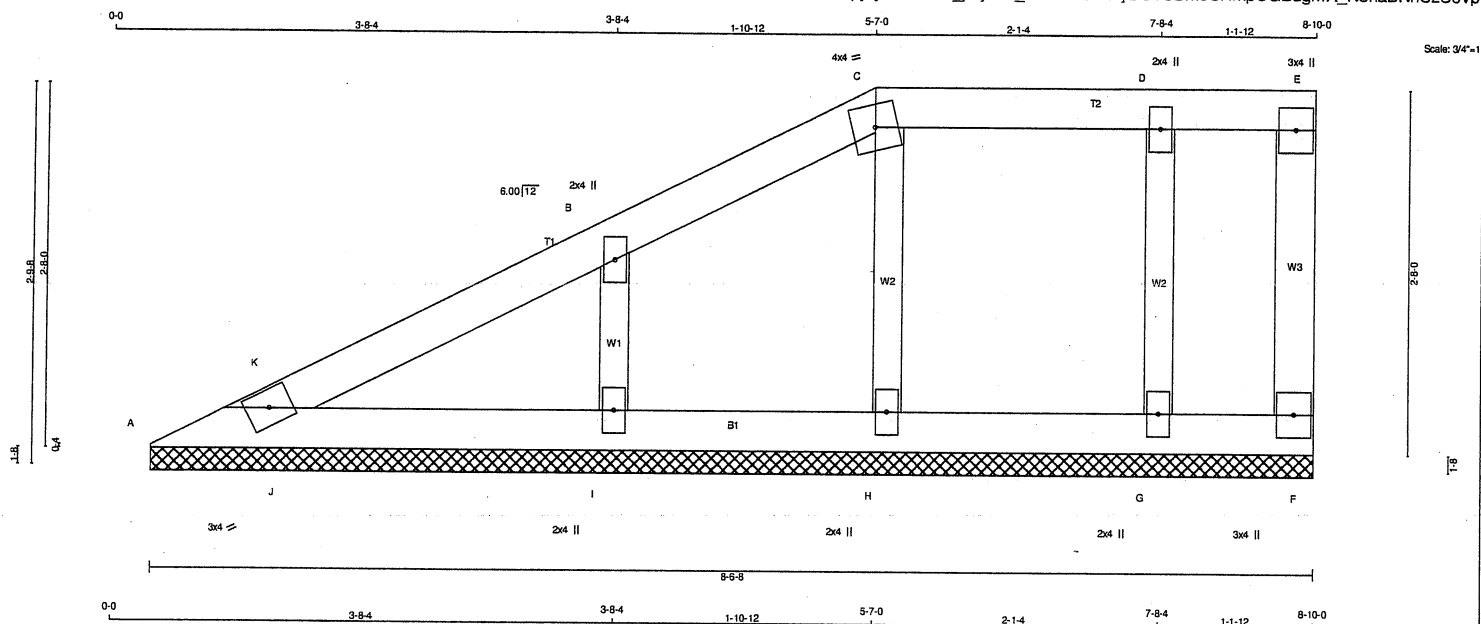
JSI GRIP= 0.30 (C) (INPUT = 0.90)
JSI METAL= 0.09 (E) (INPUT = 1.00)



Structural component only
DWG# T-2115845

JOB NAME 413226	TRUSS NAME PB105G	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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TOTAL WEIGHT = 26 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
F - E	2x4	DRY	No.2
A - F	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TBM1-h	MT20	3.0	4.0	
B	TMW+w	MT20	2.0	4.0	
C	TTW-m	MT20	4.0	4.0	
D	TMW+w	MT20	2.0	4.0	
E	TMV+p	MT20	3.0	4.0	
F	BMV1+p	MT20	3.0	4.0	
G, H, I					
G	BMV1+w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT		REQRD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	BRG	IN-SX	BRG
A	157	0	157	0	0	8-6-8	8-6-8	8-6-8	8-6-8
F	26	0	26	0	0	8-6-8	8-6-8	8-6-8	8-6-8
H	137	0	137	0	0	8-6-8	8-6-8	8-6-8	8-6-8
G	238	0	238	0	0	8-6-8	8-6-8	8-6-8	8-6-8
I	384	0	384	0	0	8-6-8	8-6-8	8-6-8	8-6-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	COMBINED	SOIL
A	111	73/0	0/0	0/0	0/0	38/0	0/0	0/0	0/0
F	19	11/0	0/0	0/0	0/0	7/0	0/0	0/0	0/0
H	97	60/0	0/0	0/0	0/0	37/0	0/0	0/0	0/0
G	167	113/0	0/0	0/0	0/0	54/0	0/0	0/0	0/0
I	271	179/0	0/0	0/0	0/0	92/0	0/0	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, F, H, G, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	FORCE (LBS)	MAX. LC1 (LC)	MAX. LC2 (LC)
FR-TO		FROM TO		FR-TO			
A-K	-31/0	-91.8 -91.8 0.05 (1)	6.25	H-C	-130/0	0.02 (1)	
K-B	0/3	-91.8 -91.8 0.10 (1)	10.00	G-D	-195/0	0.03 (1)	
B-C	-23/0	-91.8 -91.8 0.10 (1)	6.25	I-B	-274/0	0.04 (1)	
C-D	0/0	-91.8 -91.8 0.05 (1)	10.00	J-K	-75/3	0.00 (1)	
D-E	0/0	-91.8 -91.8 0.05 (1)	10.00				
F-E	-23/0	0.0 0.0 0.00 (1)	7.81				
A-J	0/10	-18.5 -18.5 0.09 (1)	10.00				
J-I	0/19	-18.5 -18.5 0.09 (1)	10.00				
I-H	0/6	-18.5 -18.5 0.06 (1)	10.00				
H-G	0/0	-18.5 -18.5 0.01 (4)	10.00				
G-F	0/0	-18.5 -18.5 0.01 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.10/1.00 (B-C:1), BC=0.09/1.00 (I-J:1), WB=0.04/1.00 (B-I:1), SSI=0.11/1.00 (B-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

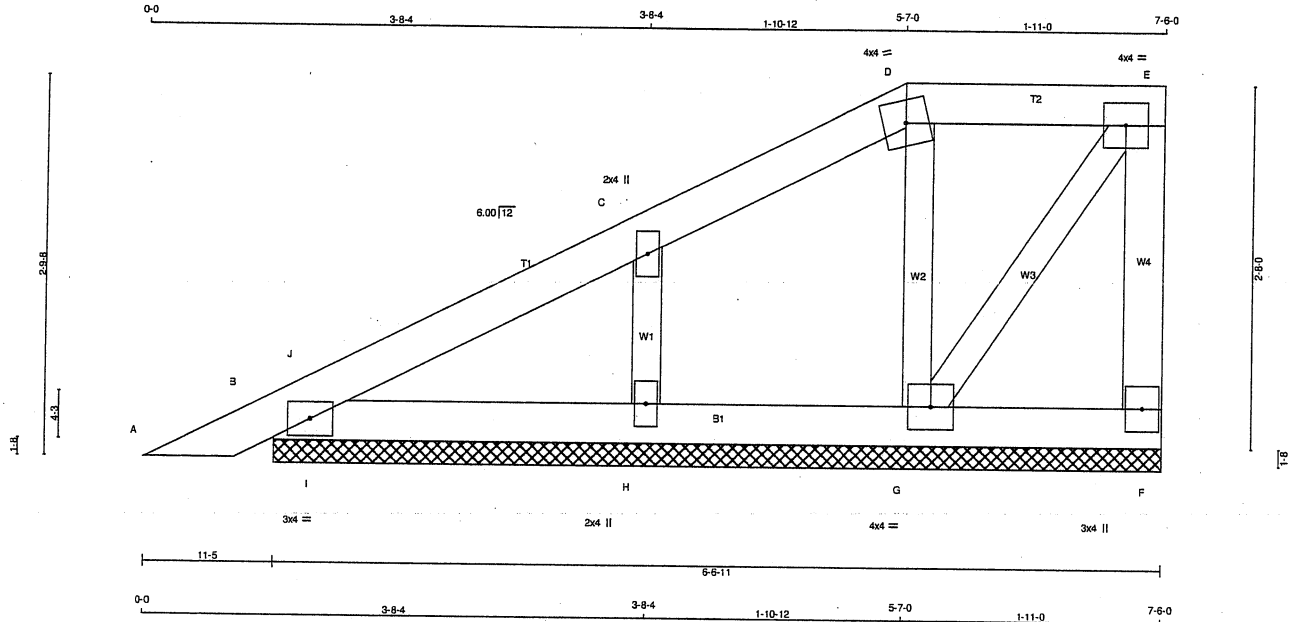
JSI GRIP= 0.16 (B) (INPUT = 0.90)
JSI METAL= 0.11 (B) (INPUT = 1.00)



Structural component only
DWG# T-2115846

JOB NAME 413226	TRUSS NAME PB201G	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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TOTAL WEIGHT = 2 X 24 = 48 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4 DRY	No.2	SPF
D - E	2x4 DRY	No.2	SPF
F - E	2x4 DRY	No.2	SPF
B - F	2x4 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	
C	TMW-w	MT20	2.0	4.0	
D	TTW-m	MT20	4.0	4.0	
E	TMVW-t	MT20	4.0	4.0	
F	BMV1-p	MT20	3.0	4.0	
G	BMVW1-t	MT20	4.0	4.0	
H	BMV1-w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT		REQRD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	IN-SX
F	102	0	102	0	0	6-6-11	6-6-11
B	209	0	209	0	0	6-6-11	6-6-11
G	175	0	175	0	0	6-6-11	6-6-11
H	318	0	318	0	0	6-6-11	6-6-11

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	72	49 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
B	146	105 / 0	0 / 0	0 / 0	0 / 0	41 / 0	0 / 0
G	124	79 / 0	0 / 0	0 / 0	0 / 0	45 / 0	0 / 0
H	225	149 / 0	0 / 0	0 / 0	0 / 0	76 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, G, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	MAX. LC1 (LC)
FR-TO		FROM	TO	FR-TO			
A-B	0 / 17	-91.8	-91.8 0.05 (1)	10.00	G-D	-147 / 0	0.02 (1)
B-J	-25 / 0	-91.8	-91.8 0.01 (4)	6.25	H-C	-244 / 0	0.04 (1)
J-C	-7 / 0	-91.8	-91.8 0.08 (1)	10.00	G-E	-2 / 0	0.00 (1)
C-D	-19 / 0	-91.8	-91.8 0.08 (1)	6.25	I-J	-84 / 4	0.00 (1)
D-E	0 / 1	-91.8	-91.8 0.06 (1)	10.00			
F-E	-86 / 0	0.0	0.0 0.01 (1)	7.81			
B-I	0 / 17	-18.5	-18.5 0.05 (1)	10.00			
I-H	0 / 17	-18.5	-18.5 0.05 (1)	10.00			
H-G	0 / 5	-18.5	-18.5 0.03 (1)	10.00			
G-F	0 / 0	-18.5	-18.5 0.01 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.6	PSF
	DL	= 6.0	PSF
BOT CH.	LL	= 0.0	PSF
	DL	= 7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.08/1.00 (C-D:1), BC=0.05/1.00 (B-I:1), WB=0.04/1.00 (C-H:1), SSI=0.09/1.00 (C-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

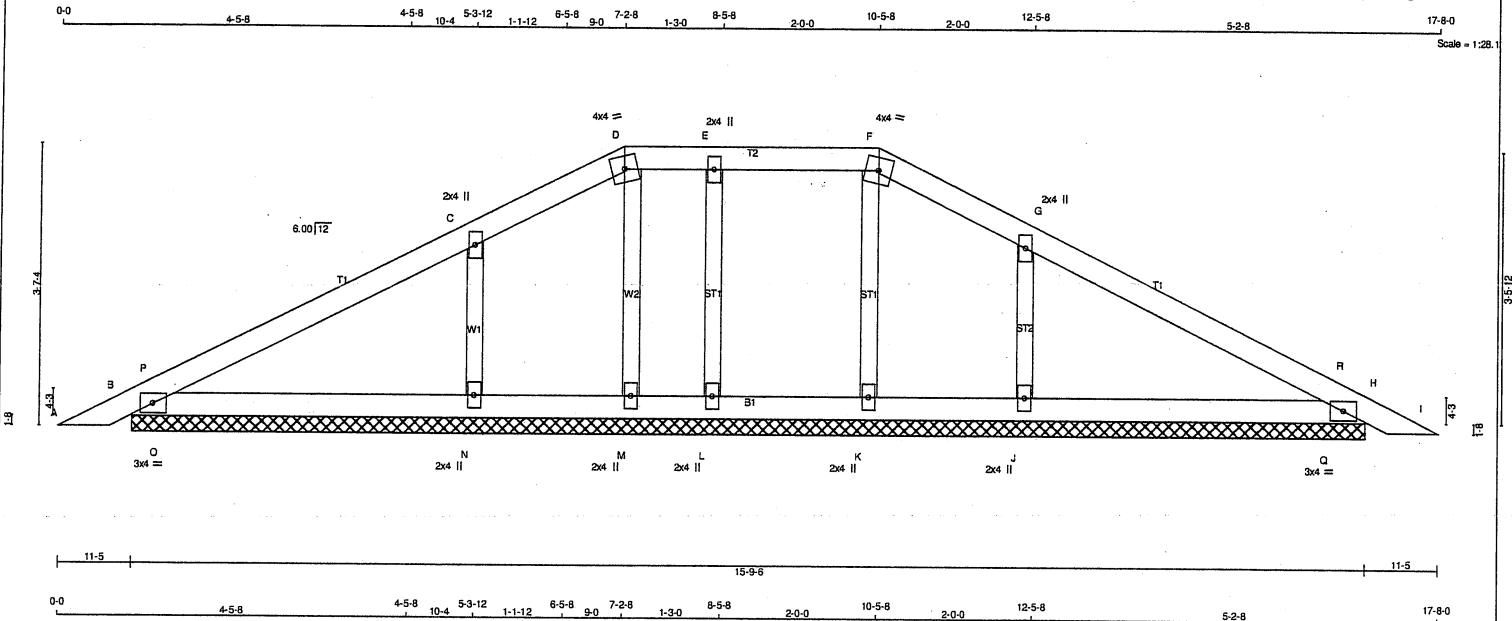
JSI GRIP= 0.15 (B) (INPUT = 0.90)
JSI METAL= 0.10 (C) (INPUT = 1.00)



Structural component only
DWG# T-2115847

JOB NAME 413226	TRUSS NAME PB202G	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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TOTAL WEIGHT = 50 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
B - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

GABLE STUDS SPACED AT 2'-0" O.C.

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B TMB1-I	MT20	3.0	4.0	
C, E, G				
C TMW+w	MT20	2.0	4.0	
D TTW-m	MT20	4.0	4.0	
F TTW-m	MT20	4.0	4.0	
H TMB1-I	MT20	3.0	4.0	
J, K, L, M, N				
J BMW1+w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

PROVIDE ANCHORAGE AT BEARING JOINT M FOR 150 LBS. FACTORED UPLIFT BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S).

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD LC1 (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	UNBRAC LENGTH
FR-TO		FROM TO		FR-TO			
A-B	0 / 17	-91.8 -91.8	0.05 (1)	10.00	K-F	-87 / 0	0.02 (1)
B-P	-31 / 0	-91.8 -91.8	0.03 (4)	6.25	L-E	-193 / 0	0.04 (1)
P-C	-39 / 0	-91.8 -91.8	0.17 (1)	6.25	J-G	-357 / 0	0.06 (1)
C-D	-69 / 0	-91.8 -91.8	0.17 (1)	6.25	M-D	-31 / 0	0.01 (1)
D-E	-34 / 0	-91.8 -91.8	0.04 (1)	6.25	N-C	-357 / 0	0.06 (1)
E-F	-35 / 0	-91.8 -91.8	0.05 (1)	6.25	O-P	-211 / 6	0.00 (1)
F-G	-71 / 0	-91.8 -91.8	0.17 (1)	6.25	Q-R	-210 / 7	0.00 (1)
G-R	-41 / 0	-91.8 -91.8	0.17 (1)	6.25			
R-H	-33 / 0	-91.8 -91.8	0.03 (4)	6.25			
H-I	0 / 17	-91.8 -91.8	0.05 (1)	10.00			
B-O	0 / 46	-18.5 -18.5	0.13 (1)	10.00			
O-N	0 / 46	-18.5 -18.5	0.13 (1)	10.00			
N-M	0 / 36	-18.5 -18.5	0.09 (1)	10.00			
M-L	0 / 35	-18.5 -18.5	0.03 (1)	10.00			
L-K	0 / 35	-18.5 -18.5	0.02 (1)	10.00			
K-J	0 / 37	-18.5 -18.5	0.09 (1)	10.00			
J-Q	0 / 48	-18.5 -18.5	0.13 (1)	10.00			
Q-H	0 / 48	-18.5 -18.5	0.13 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17/1.00 (G-R:1), BC=0.13/1.00 (N-O:1), WB=0.06/1.00 (G-J:1), SS=0.17/1.00 (B-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

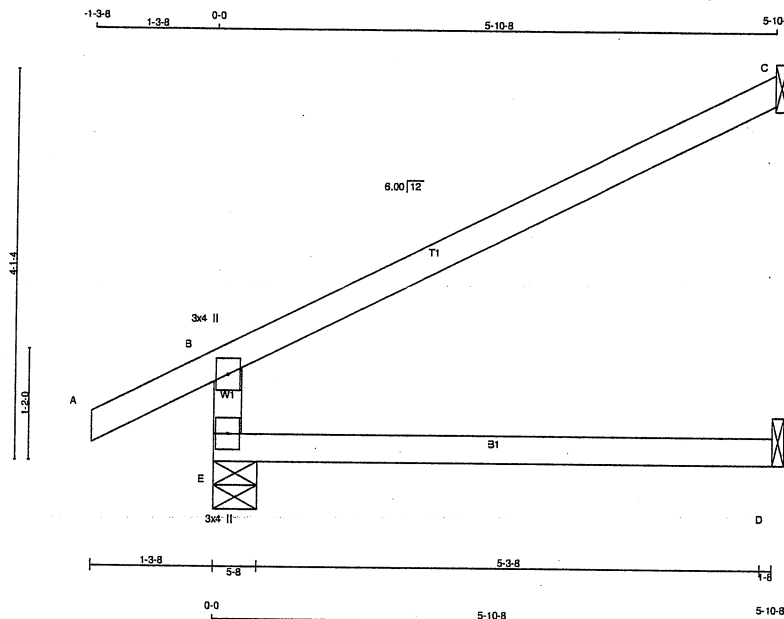
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.23 (H) (INPUT = 0.90)
JSI METAL = 0.15 (G) (INPUT = 1.00)



Structural component only
DWG# T-2115848

JOB NAME 413226	TRUSS NAME J01	QUANTITY 19	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.420 S Jan 21 2021 MITek Industries, Inc. Wed May 26 14:46:28 2021 Page 1			
		ID:U6yi?rbeFFwxf_UFm_koDybSsj-fY7imNVgatpbU6k1QXNzXY98rCcl1GVFJek3UYzCcVv			
		TRUSS DESC.			



LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	3.0	4.0
E	BMV1+p	MT20	3.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	525	0	525	0	5-8	5-8
C	202	0	202	0	1-8	1-8
D	45	0	50	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	WIND			
E	369	257 / 0	0 / 0	0 / 0	111 / 0	0 / 0	0 / 0
C	139	113 / 0	0 / 0	0 / 0	26 / 0	0 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS			WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC1)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO		FR-TO	
E-B	-461 / 0	0.0	0.0 0.13 (4)	7.81	
A-B	0 / 28	-91.8	-91.8 0.12 (1)	10.00	
B-C	-30 / 0	-91.8	-91.8 0.54 (1)	6.25	
E-D	0 / 0	-18.5	-18.5 0.13 (4)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.54/1.00 (B-C:1), BC=0.13/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (E) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)

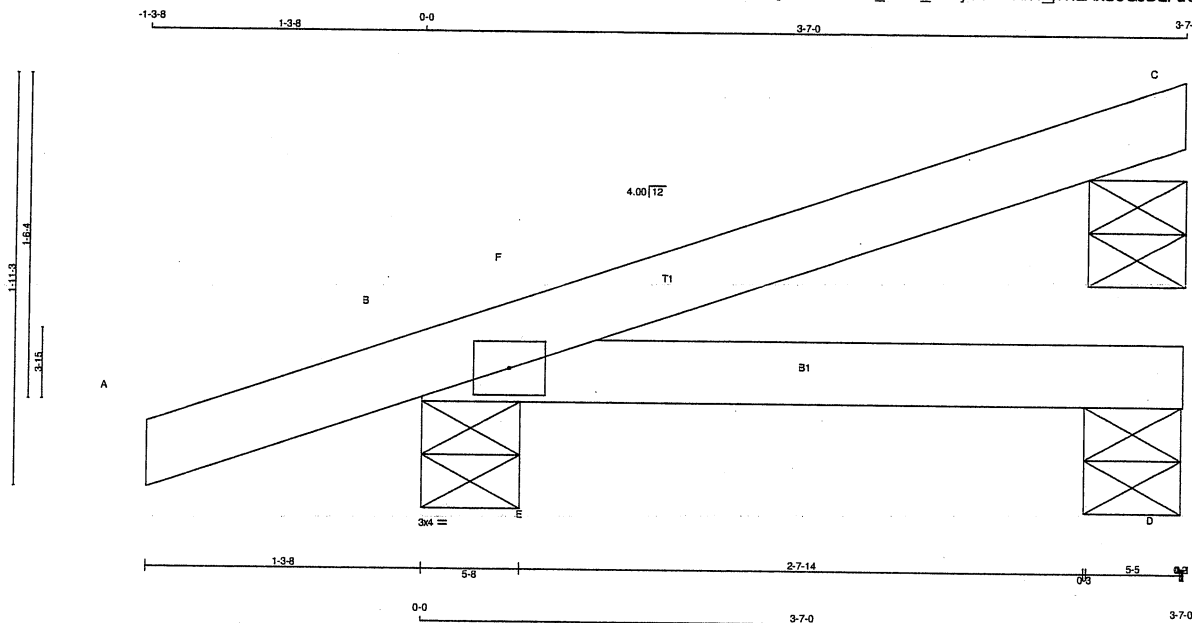


Structural component only
DWG# T-2115830

JOB NAME 413226	TRUSS NAME J03	QUANTITY 15	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1:10.3



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
B - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	DOWN	GROSS REACTION	BRG	BRG
C	141	0	141	0	5-8 (5-7)
B	320	0	320	0	5-8
D	57	0	57	0	5-8

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): C

UNFACTORED REACTIONS

JT	1ST CASE	MAX./MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
C	98	76 / 0	0 / 0	0 / 0	0 / 0	0 / 0	22 / 0	0 / 0
B	224	160 / 0	0 / 0	0 / 0	0 / 0	0 / 0	64 / 0	0 / 0
D	42	16 / 0	0 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX	MEMB.	FORCE (LBS)	MAX	LC1
FR-TO		FROM	TO	FR-TO			
A-B	0 / 18	-91.8	-91.8 0.13 (5)	10.00	E-F	-138 / 5	0.00 (1)
B-F	-10 / 0	-91.8	-91.8 0.04 (4)	6.25			
F-C	0 / 2	-91.8	-91.8 0.15 (1)	10.00			
B-E	0 / 0	-18.5	-18.5 0.12 (1)	10.00			
E-D	0 / 0	-18.5	-18.5 0.12 (1)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 15 X 10 = 151 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.6	PSF
	DL	= 6.0	PSF
BOT CH.	LL	= 0.0	PSF
	DL	= 7.4	PSF
TOTAL LOAD		= 39.0	PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.01")
ALLOWABLE DEFL. (TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.02")

CSI: TC=0.15/1.00 (C-F:1), BC=0.12/1.00 (B-E:1),
WB=0.00/1.00 (E-F:1), SS=0.11/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

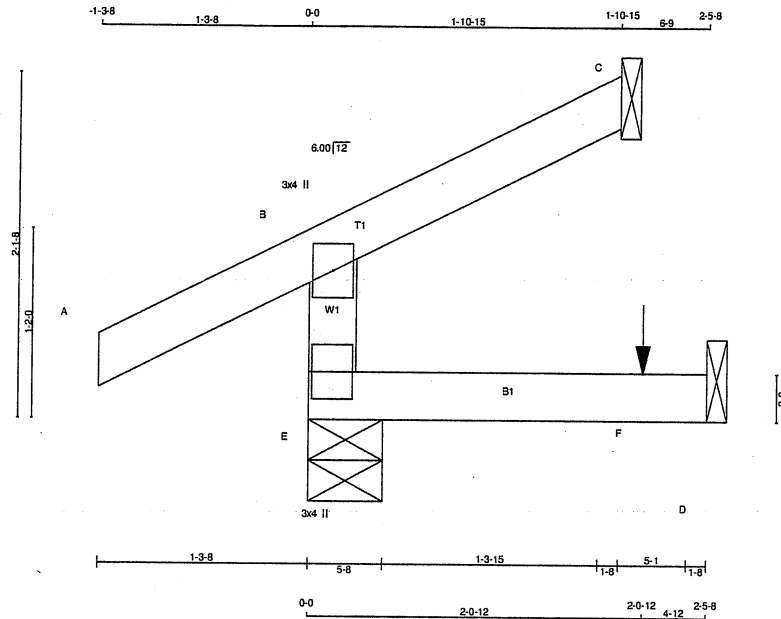
JSI GRIP = 0.20 (B) (INPUT = 0.90)
JSI METAL = 0.05 (B) (INPUT = 1.00)



Structural component only
DWG# T-2115831

JOB NAME 413226	TRUSS NAME J11	QUANTITY 5	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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Scale = 1:13.5

TOTAL WEIGHT = 5 X 8 = 39 lb

LUMBER				DESCR.
N. L. G. A. RULES	SIZE	LUMBER		
CHORDS				
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	3.0	4.0
E	BMV1+p	MT20	3.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	IN-SX	BRG	IN-SX
E	260	0	260	0	0	5-8	5-8	5-8	5-8
C	66	0	66	0	0	1-8	1-8	1-8	1-8
D	23	0	26	0	0	1-8	1-8	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	
E	182	130 / 0	0 / 0	0 / 0	0 / 0	52 / 0	0 / 0	
C	46	37 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0	
D	18	0 / 0	0 / 0	0 / 0	0 / 0	18 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS		FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRAC LENGTH FR-TO	MEMB.	FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO					
E-B	-234 / 0	0.0	0.0	0.02 (4)	7.81		
A-B	0 / 28	-91.8	-91.8	0.13 (5)	10.00		
B-C	-10 / 0	-91.8	-91.8	0.06 (1)	10.00		
E-F	0 / 0	-18.5	-18.5	0.03 (4)	10.00		
F-D	0 / 0	-18.5	-18.5	0.03 (4)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	2-0-12	-3	-3	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.13/1.00 (A-B-5), BC=0.03/1.00 (D-E-4),
WB=0.00/1.00 (n/a:0), SSI=0.09/1.00 (A-B-5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

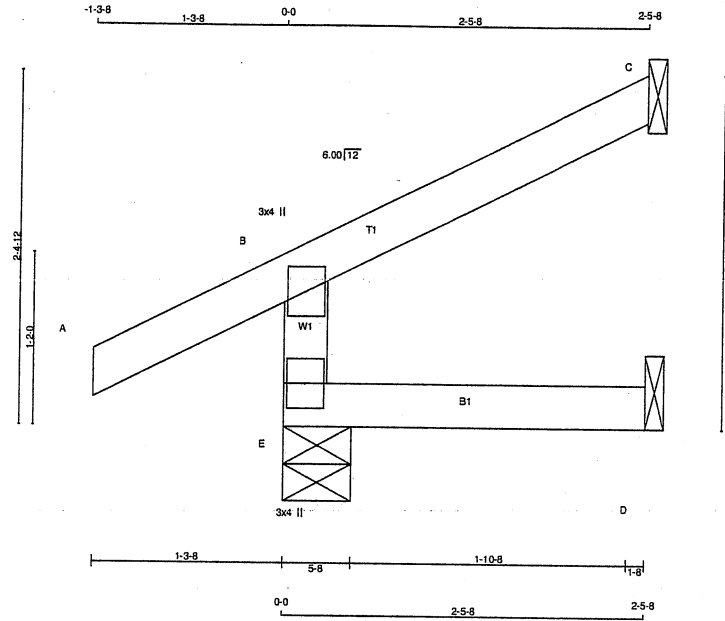
JSI GRIP= 0.09 (E) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)



Structural component only
DWG# T-2115832

JOB NAME 413226	TRUSS NAME J12	QUANTITY 12	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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Scale = 1:14.8

TOTAL WEIGHT = 12 X 9 = 103 lb

LUMBER				DESCR.
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					X
JT TYPE	PLATES	W	LEN	Y	
B TMV+p	MT20	3.0	4.0		
E BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	291	0	291	0	5-8	5-8
C	85	0	85	0	1-8	1-8
D	20	0	22	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	UNBRAC			
E	203	148 / 0	0 / 0	0 / 0	0 / 0	55 / 0	0 / 0
C	58	47 / 0	0 / 0	0 / 0	0 / 0	11 / 0	0 / 0
D	16	0 / 0	0 / 0	0 / 0	0 / 0	16 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	FORCE	FACTORED		MEMB.	FORCE	FACTORED	
	(LBS)	VERT. LOAD	LC1		(LBS)	MAX. UNBRAC	MAX. CS1 (LC)
FR-TO		FROM	TO	FR-TO		LENGTH	
E-B	-265 / 0	0.0	0.0	0.02 (4)	7.81		
A-B	0 / 28	-91.8	-91.8	0.12 (5)	10.00		
B-C	-12 / 0	-91.8	-91.8	0.09 (1)	6.25		
E-D	0 / 0	-18.5	-18.5	0.03 (4)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	25.6	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.00")

CSI: TC=0.12/1.00 (A-B-5), BC=0.03/1.00 (D-E-4), WB=0.00/1.00 (n/a), SSI=0.10/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PL)
MAX	MIN	MAX
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

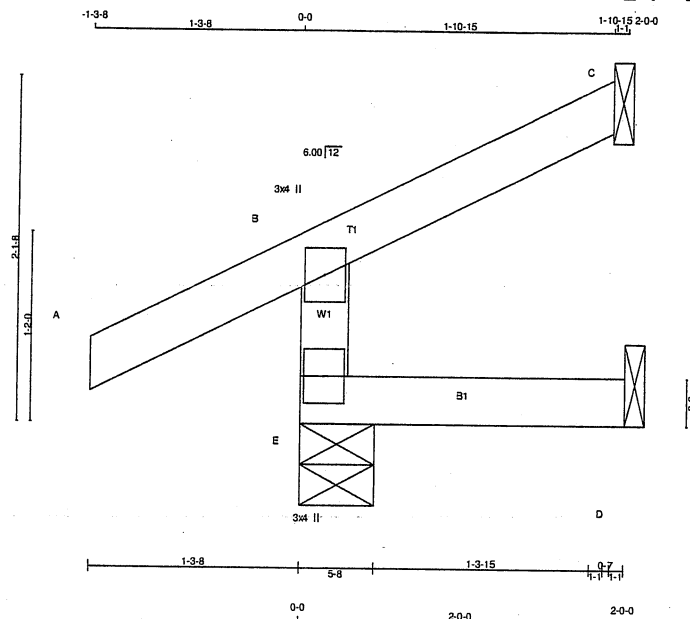
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.11 (E) (INPUT = 0.90)
JSI METAL = 0.07 (B) (INPUT = 1.00)



Structural component only
DWG# T-2115833

JOB NAME 413226	TRUSS NAME J13	QUANTITY 10	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.420 S Jan 21 2021 MITek Industries, Inc. Wed May 26 14:46:29 2021 Page 1 ID:hEKUVWZApjnyW9leD6V_jMya4z_-7lh4_jWILAxSSGjDzFuC3lhQ9c_lmjiOYIUc0?zCcVu			



TOTAL WEIGHT = 10 X 7 = 73 lb [M]

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B TMV+p	MT20	3.0	4.0	
E BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX		
E	254	0	254	0	5-8	5-8		
C	66	0	66	0	1-8	1-8		
D	16	0	18	0	1-8	1-8		

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
E	177	130 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	46	37 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0
D	13	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS		FACTORED		WEBS		MAX. FACTORED	
	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	LC1 MAX	MAX. MEMB. FORCE (LBS)	MAX. MEMB. FORCE (LBS)	MAX. FORCE (LBS)	MAX. FORCE (LBS)
FR-TO								
E-B	-234 / 0	0.0	0.0	0.01 (4)	7.81			
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00			
B-C	-10 / 0	-91.8	-91.8	0.06 (1)	10.00			
E-D	0 / 0	-18.5	-18.5	0.02 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.12/1.00 (A-B:1), BC=0.02/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
			788
			1987
			1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

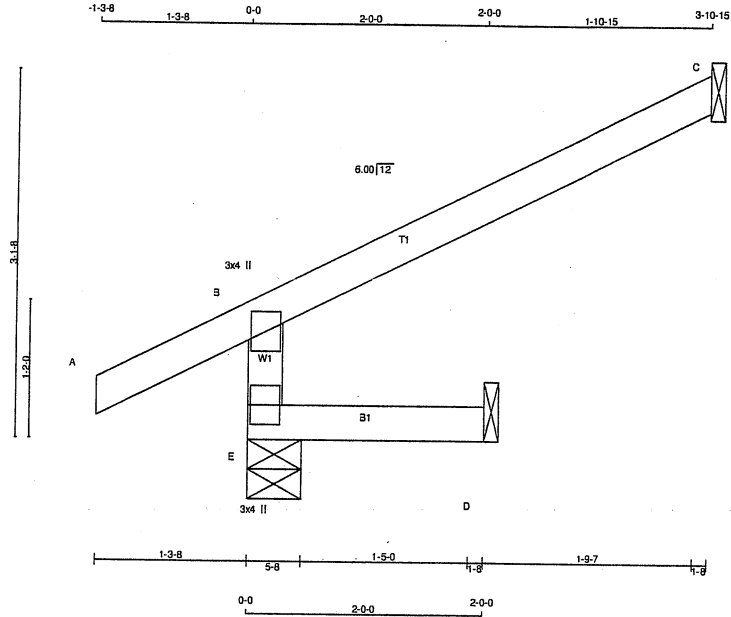
JSI GRIP= 0.09 (E) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)



Structural component only
DWG# T-2115834

JOB NAME 413226	TRUSS NAME J14	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Wed May 26 14:46:30 2021 Page 1
ID:hEKUJVVWZApjnyW9leD6V_jMya4z_-bxFSB3Wx6U3JjQuPXyPRczEZ20K_VA?YmyD9YRzCcVt



Scale = 1:18.6

LUMBER				DESCR.
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					X
JT	TYPE	PLATES	W	LEN	
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	369	0	369	0	5-8	5-8
C	135	0	135	0	1-8	1-8
D	16	0	18	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	WIND			
E	256	194 / 0	0 / 0	0 / 0	62 / 0	0 / 0	0 / 0
C	93	75 / 0	0 / 0	0 / 0	18 / 0	0 / 0	0 / 0
D	13	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS		FACTORED		WEBS		MEMB.	FACTORED	
	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX. CSI (LC)	UNBRAC LENGTH	FR-TO		MAX. FORCE (LBS)	MAX. CSI (LC)
FR-TO									
E-B	-349 / 0	0.0	0.0	0.01 (4)	7.81				
A-B	0 / 28	-91.8	-91.8	0.13 (5)	10.00				
B-C	-20 / 0	-91.8	-91.8	0.24 (1)	6.25				
E-D	0 / 0	-18.5	-18.5	0.02 (4)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 2 X 10 = 20 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.00")

CSI: TC=0.24/1.00 (B-C:1), BC=0.02/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

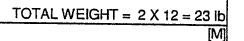
JSI GRIP = 0.14 (E) (INPUT = 0.90)

JSI METAL = 0.10 (B) (INPUT = 1.00)



Structural component only
DWG# T-2115835

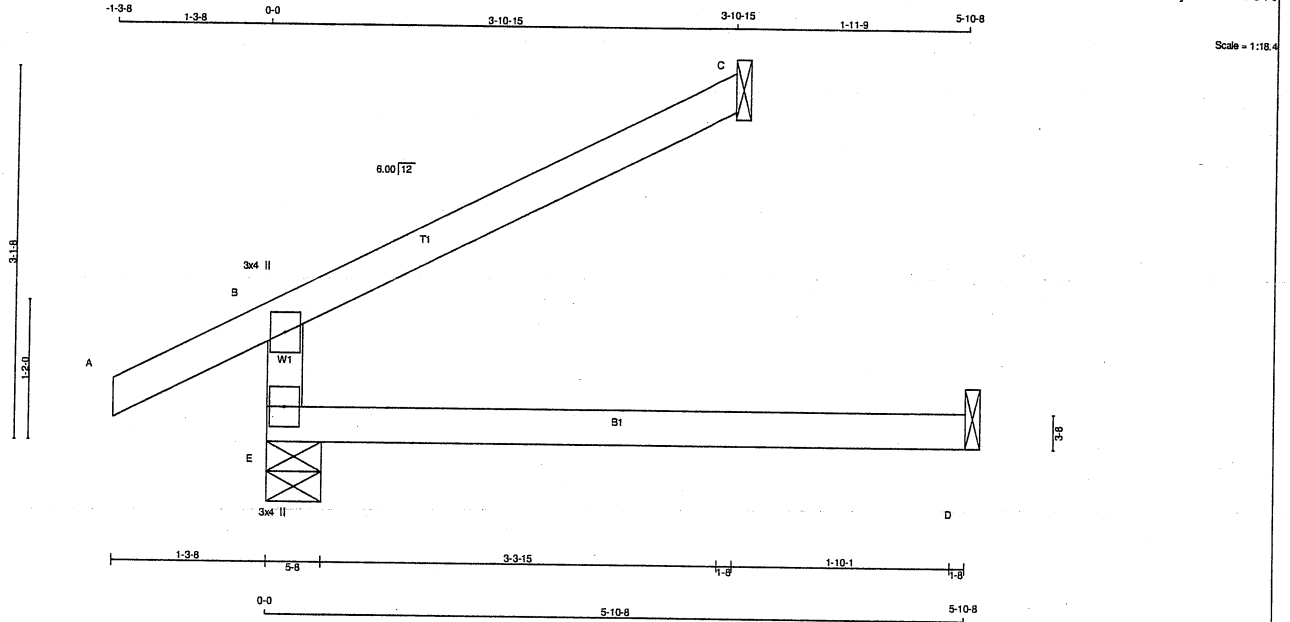
Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed May 26 14:46:30 2021 Page 1
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Structural component only
DWG# T-2115836

JOB NAME 413226	TRUSS NAME J16	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed May 26 14:46:30 2021 Page 1
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TOTAL WEIGHT = 2 X 14 = 29 lb [M]

LUMBER			
N.L.G.A. RULES	SIZE	LUMBER	DESCR.
E - B	2x4 DRY	No.2	SPF
A - C	2x4 DRY	No.2	SPF
E - D	2x4 DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B TMV+p	MT20	3.0	4.0	
E BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	412	0	412	0	0	5-8	5-8
C	135	0	135	0	0	1-8	1-8
D	45	0	50	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	291	194/0	0/0	0/0	0/0	97/0	0/0
C	93	75/0	0/0	0/0	0/0	18/0	0/0
D	36	0/0	0/0	0/0	0/0	36/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	LC1 MAX (PLF)	MAX. UNBRAC LENGTH (LC)	MEMB. FORCE (LBS)
FR-TO					
E-B	-349/0	0.0	0.0	0.13 (4)	7.81
A-B	0/28	-91.8	-91.8	0.12 (1)	10.00
B-C	-20/0	-91.8	-91.8	0.24 (1)	6.25
E-D	0/0	-18.5	-18.5	0.13 (4)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.03")

CSI: TC=0.24/1.00 (B-C:1), BC=0.13/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
		788	1987
			1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (E) (INPUT = 0.90)

JSI METAL= 0.10 (B) (INPUT = 1.00)

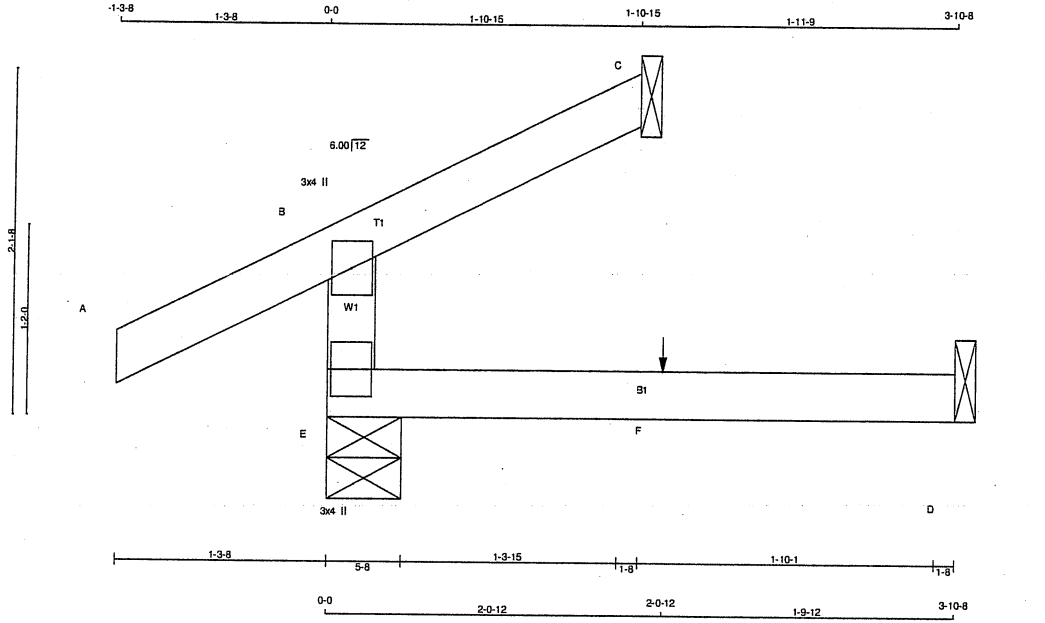


Structural component only
DWG# T-2115837

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
413226	J17	3	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 3 X 9 = 28 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	275	0	275	0	5-8	5-8
C	66	0	66	0	1-8	1-8
D	30	0	34	0	1-8	1-8

SEE MITK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	COMBINED	1ST LCASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
		SNOW	LIVE	PERM.LIVE	PERM.LIVE			
E	194	130/0	0/0	0/0	0/0	64/0	0/0	0/0
C	46	37/0	0/0	0/0	0/0	9/0	0/0	0/0
D	24	0/0	0/0	0/0	0/0	24/0	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS		FACTORED		WEBS		FACTORED	
	FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX	MEMB. FORCE (LBS)	MAX	FORCE (LBS)	MAX
FR-TO		FROM	TO		FR-TO			
E-B	-234/0	0.0	0.0	0.05 (4)	7.81			
A-B	0/28	-91.8	-91.8	0.14 (5)	10.00			
B-C	-10/0	-91.8	-91.8	0.06 (1)	10.00			
E-F	0/0	-18.5	-18.5	0.06 (4)	10.00			
F-D	0/0	-18.5	-18.5	0.06 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	2-0-12	1	1	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.14/1.00 (A-B-5), BC=0.06/1.00 (D-E-4),
WB=0.00/1.00 (n/a-0), SSI=0.10/1.00 (A-B-5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	650	371	1747
	788	1987	1873

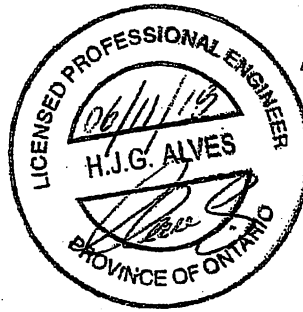
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.09 (E) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)



Structural component only
DWG# T-2115838



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering tem and General Safety notes.

T-1800218

Feb 09, 2018

TOE-NAIL CAPACITY DETAILS

LATERAL AND WITHDRAWAL RESISTANCE OF BEARING ANCHORAGE BY TOE-NAILS

			SPF	D. FIR	SPF	D. FIR
COMMON WIRE	3.00	0.144	122	139	30	42
	3.25	0.144	127	144	32	45
	3.50	0.160	152	173	38	52
COMMON SPIRAL	3.00	0.122	96	108	26	36
	3.25	0.122	97	108	28	40
	3.50	0.152	142	161	36	50
3.25" Gun nail	3.25	0.120	94	105	28	39

Note: If using truss with D. Fir lumber and SPF bearing plate, use tabulated SPF values in table.

Nail type:	Common wire	Common spiral	Common wire	Common spiral	Gun Nail
Diameter (in.)	0.160	0.152	0.144	0.122	0.120
Length (in.)	3.50	3.50	3.00	3.00	3.25
MINIMUM NUMBER OF TOE-NAILS					
2x4 SPF	2	2	3	3	3
2x6 SPF	4	4	4	5	5
2x4 D. FIR	2	2	2	2	2
2x6 D. FIR	3	3	3	4	4

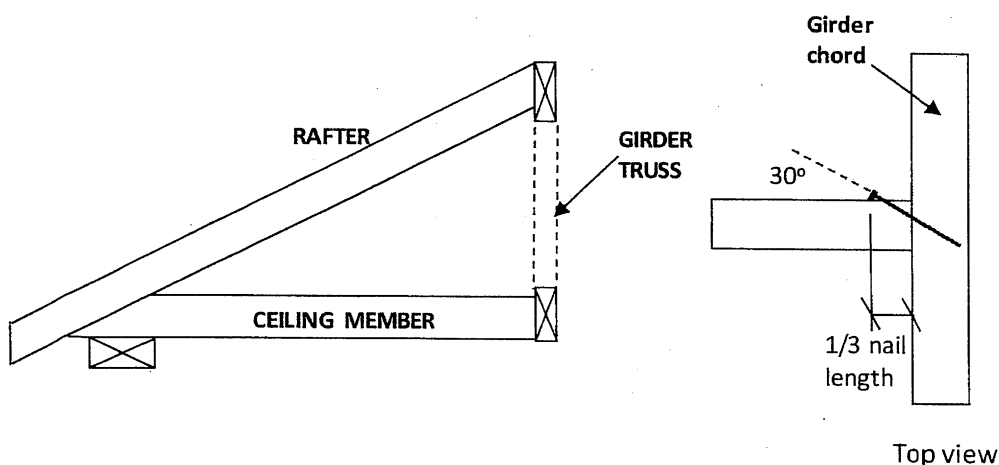


Figure 1: Toe-Nailing Rafter / Ceiling Member to Girder Truss

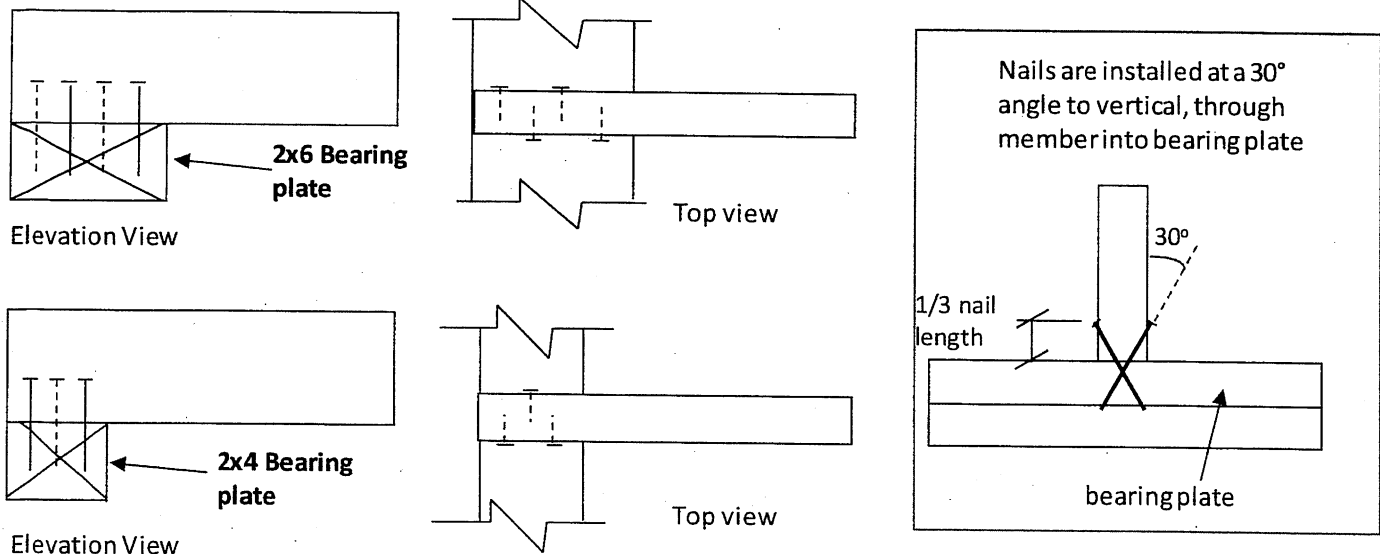
PEO
Certificate No. 10889485



December 21, 2020

TOE-NAIL CAPACITY DETAILS

Figure 2: Toe-Nail Anchorage to Bearing Plate for Uplift



NOTES:

1. Rafter and ceiling members may be connected to top and bottom chords of girder truss by toe-nailing the members into the girder chords (see fig. 1), provided the factored vertical reactions of the supported members do not exceed the lateral resistance of the toe-nails. Mechanical connectors (hangers) are required if factored vertical reactions exceed the toe-nail capacity, or if the connection must resist horizontal loads (loads perpendicular to the face of girder or rafter).
2. Trusses, rafters or ceiling members may be anchored to the bearing plate with toe-nails (see fig. 2), provided that the factored uplift reactions due to **wind or earthquake loads** do not exceed the **withdrawal resistance of the toe-nails**. Mechanical anchors (tie-downs) are required for reactions that exceed the toe-nail withdrawal capacity. Toe-nail anchorage to bearing plates is **NOT** permitted if uplift reactions are generated from gravity loads (snow, floor live, dead).
3. Tabulated toe-nail resistances on page 1 are for **one** toe-nail. Multiply unit values by the number of nails used in the connection. Maximum number of nails in a connection shall not exceed the tabulated limits shown on page 1 for a given lumber size /species.
4. Nail values are based on specific gravity of $G = 0.42$ (SPF) and $G = 0.49$ (D. Fir).
5. Toe-nails shall be driven at approximately $1/3$ the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member.
6. For wind / earthquake loads, tabulated lateral resistances may be multiplied by 1.15 (K_0 factor). No increases are permitted for tabulated withdrawal resistances.
7. Lumber must be dry ($< 19\%$ moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-19, Clause 12.9.

PEO
Certificate No. 10889485



LUL/LUS/LJS/HUS/HHUS/HGUS**SIMPSON**
Strong-Tie

Standard and Double-Shear Joist Hangers



This product is preferable to similar connectors because of
a) easier installation, b) higher capacities, c) lower installed
cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 217–218.

Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 18–20.

Installation:

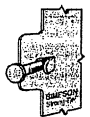
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

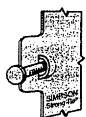
- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on pp. 105–107.



Double-Shear
Nailing
Top View

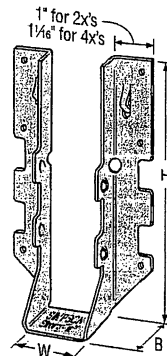
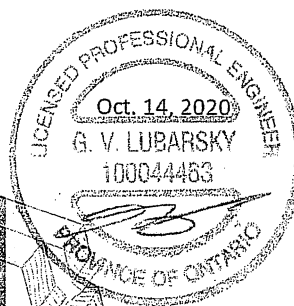
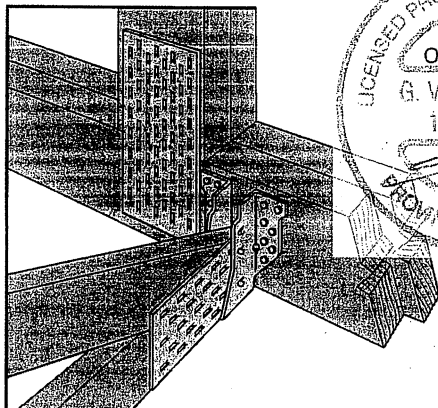


Double-Shear
Nailing
Side View;
Do not
bend tab

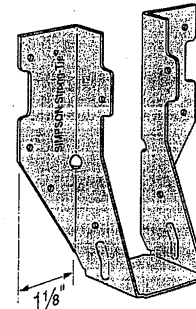


Dome Double-Shear
Nailing
Side View
(available on
some models)

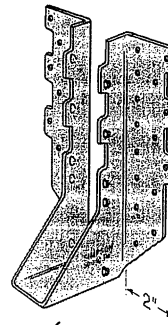
**Typical HUS26
Installation
with Reduced
Heel Height**
(Truss Designer
to provide
fastener quantity
for connecting
multiple members
together)



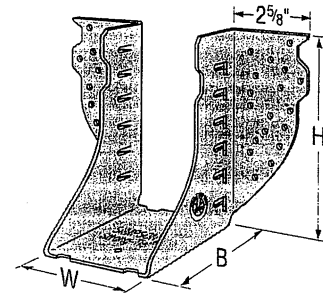
LUS28



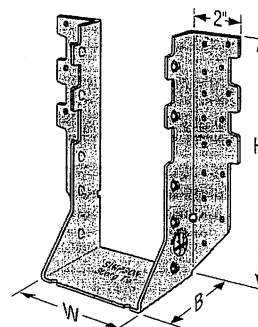
LU26L



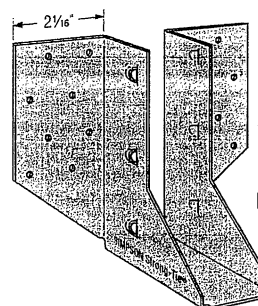
HUS210
(HUS26, HUS28,
and HHUS similar)



HGUS28-2



HHUS210-2



LJS26DS

LUS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

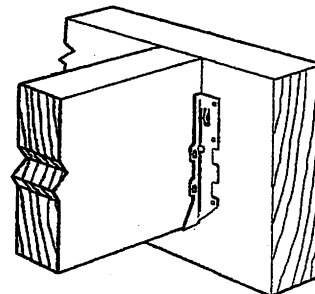
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

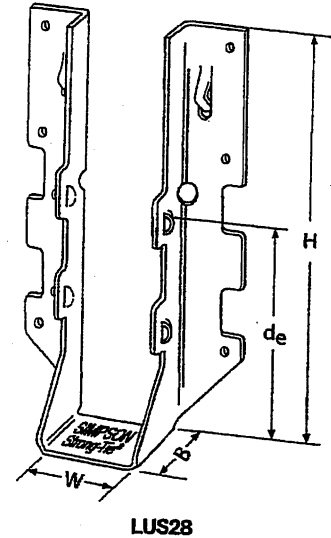
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire,
10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified

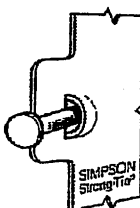


Typical LUS Installation



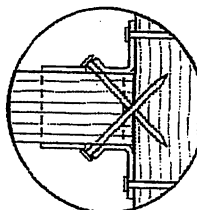
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d ₁	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K ₁ =1.15)	Normal (K ₂ =1.00)	Uplift (K ₁ =1.15)	Normal (K ₂ =1.00)
LUS24	18	1½	3½	1½	1 11/16	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3½	3½	2	1 11/16	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1½	4¾	1¾	3¾	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3½	4¾	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4¾	4¾	2	3¼	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1½	6½	1¾	3¾	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3½	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4¾	6¼	2	3¼	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1½	7 11/16	1¾	3¾	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3½	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4¾	8¾	2	5¼	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d₂ is the distance from the seat of the hanger to the highest joist nail.

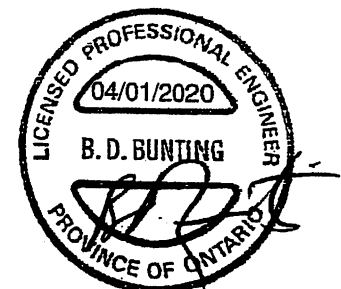


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPEC LUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

HUS/LJS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

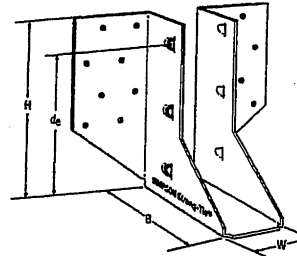
- Factored resistances are in accordance with CSA O86 -14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

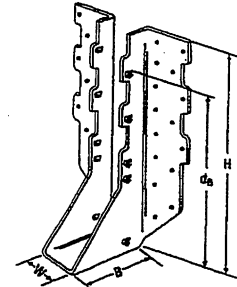
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

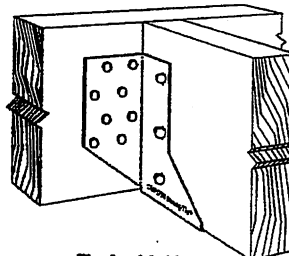
- See current catalogue for options



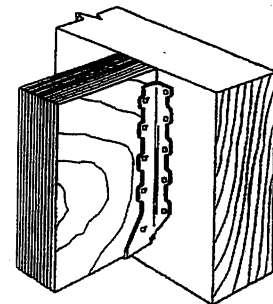
LJS26DS



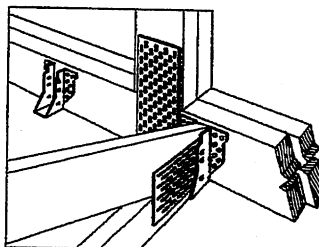
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



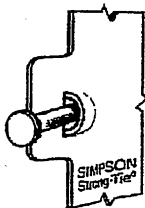
Typical HUS
Installation



Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

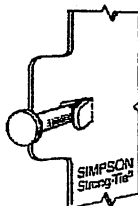
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d ₁	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K ₁ =1.15)	Normal (K ₂ =1.00)	Uplift (K ₁ =1.15)	Normal (K ₂ =1.00)
LJS26DS	18	1½	5	3½	4½	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1½	5½	3	3½	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1½	7½	3	6½	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1½	9½	3	7½	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1½	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

1. d₁ is the distance from the seat of the hanger to the highest joist nail.

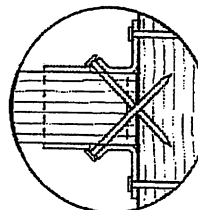


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

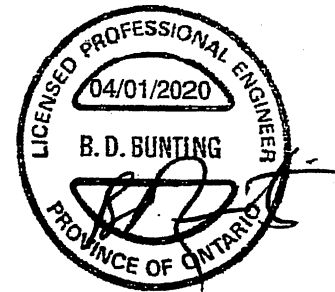
U.S. Patent
5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT
STATES
DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECHUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

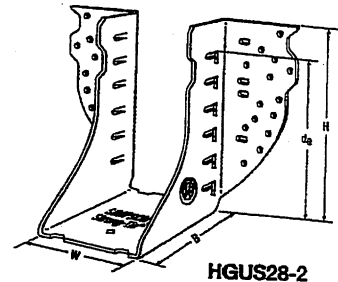
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%.
No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

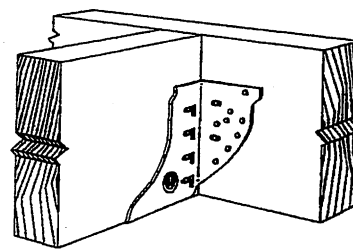
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

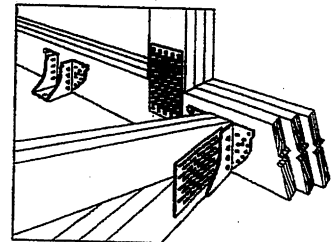
- See current catalogue for options



HGUS28-2



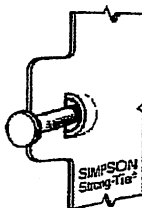
Typical HGUS
Installation



Typical HGUS Installation
(Truss Designer to
provide fastener quantity
for connecting multiple
members together)

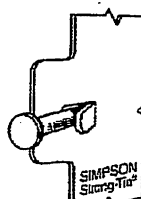
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d ₁ ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _u =1.15)	Normal (K _n =1.00)	Uplift (K _u =1.15)	Normal (K _n =1.00)
HGUS26	12	1½	5½	5	4½	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3½	5½	4	4½	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4½	5½	4	4½	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6½	5½	4	4½	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1½	7½	5	6½	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3½	7½	4	6½	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4½	7½	4	6½	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6½	7½	4	6½	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210	12	1½	9½	5	7½	(46) 16d	(16) 16d	3535	11070	2510	8090
HGUS210-2	12	3½	9½	4	8½	(46) 16d	(16) 16d	6840	14015	4855	10270
HGUS210-3	12	4½	9½	4	8½	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6½	9½	4	8½	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6½	10½	4	10½	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6½	12½	4	11½	(66) 16d	(22) 16d	10130	16400	7195	11645

1. d₁ is the distance from the seat of the hanger to the highest joist nail.

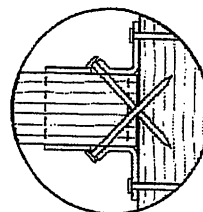


Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).

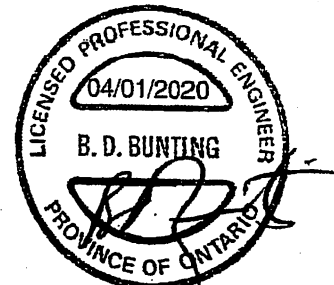
U.S. Patent
5,603,580



Double
Shear
Nailing
Side View.
Do not
bend tab
back.



Double
Shear
Nailing
Top View.



**LIMIT
STATES
DESIGN**

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

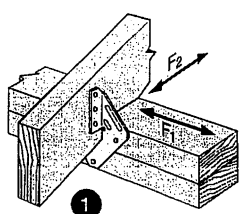
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T-SPECHGUS20 3/20 exp. 6/22

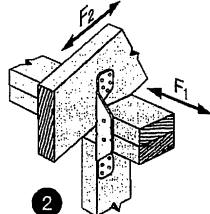
(800) 999-5099
strongtie.com

H/TSP

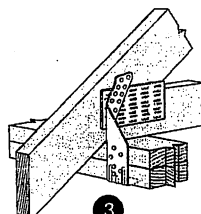
Seismic and Hurricane Ties (cont.)



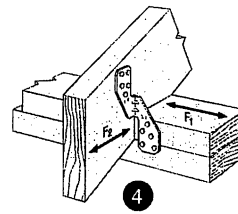
1 H1 Installation



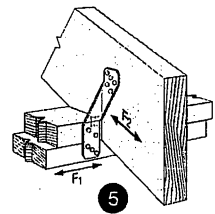
2 H2A Installation



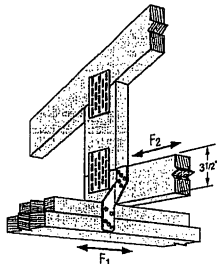
3 TSP Installation



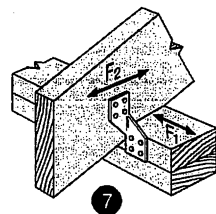
4 H2.5A Installation
(Nails into both top plates)



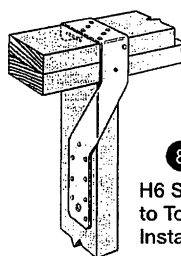
5 H2.5T Installation
(Nails into both top plates)



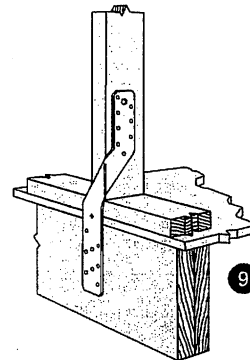
6 H2.5T Installation



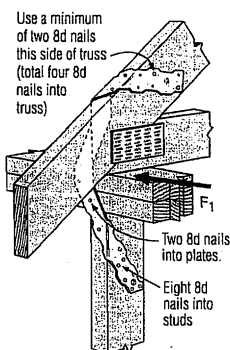
7 H3 Installation
(Nails into upper top plate)



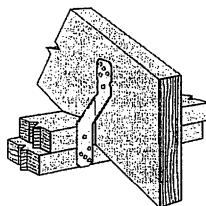
8 H6 Stud to Top Plate Installation



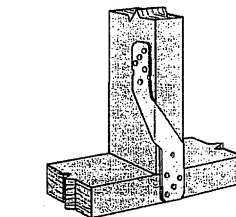
9 H6 Stud to Band Joist Installation



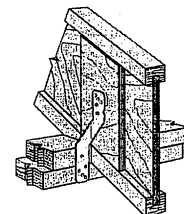
10 H7Z Installation



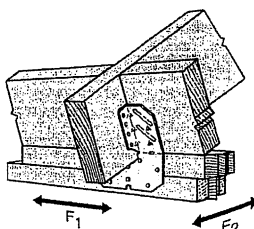
11 H8 Attaching Rafter to Double Top Plates



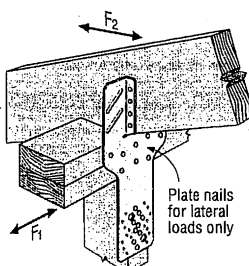
12 H8 attaching Stud to Sill
((4) 8d into plate, (5) 8d into stud)



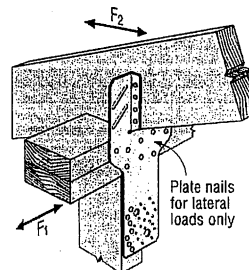
13 H8 attaching I-Joist to Double Top Plates



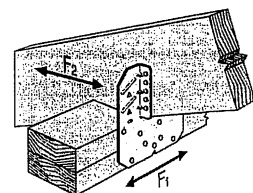
14 H10A Field-Bent Installation



15 H10S Installation

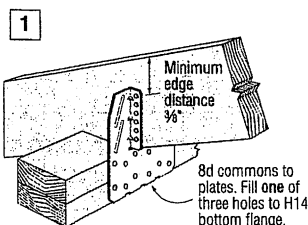


16 H10S Installation with Stud Offset

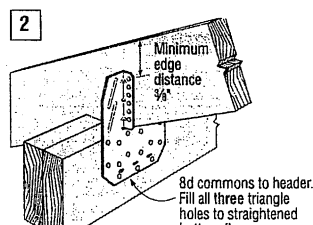


17 H10A Installation

H10A optional positive angle nailing connects shear blocking to rafter. Use 8d common nails. Slot allows maximum field-bending up to a pitch of 6/12, use 75% of the table uplift value; bend one time only.

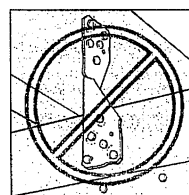


18 H14 Installation to Double Top Plates



19 H14 Installation to Double 2x Header

Avoid a Misinstallation



Do not make new holes or overdrive nails.

H/TSP

Seismic and Hurricane Ties

Simpson Strong-Tie® hurricane ties provide a positive connection between truss/rafter and the wall of the structure to resist wind and seismic forces. New additions to the line provide even more options.

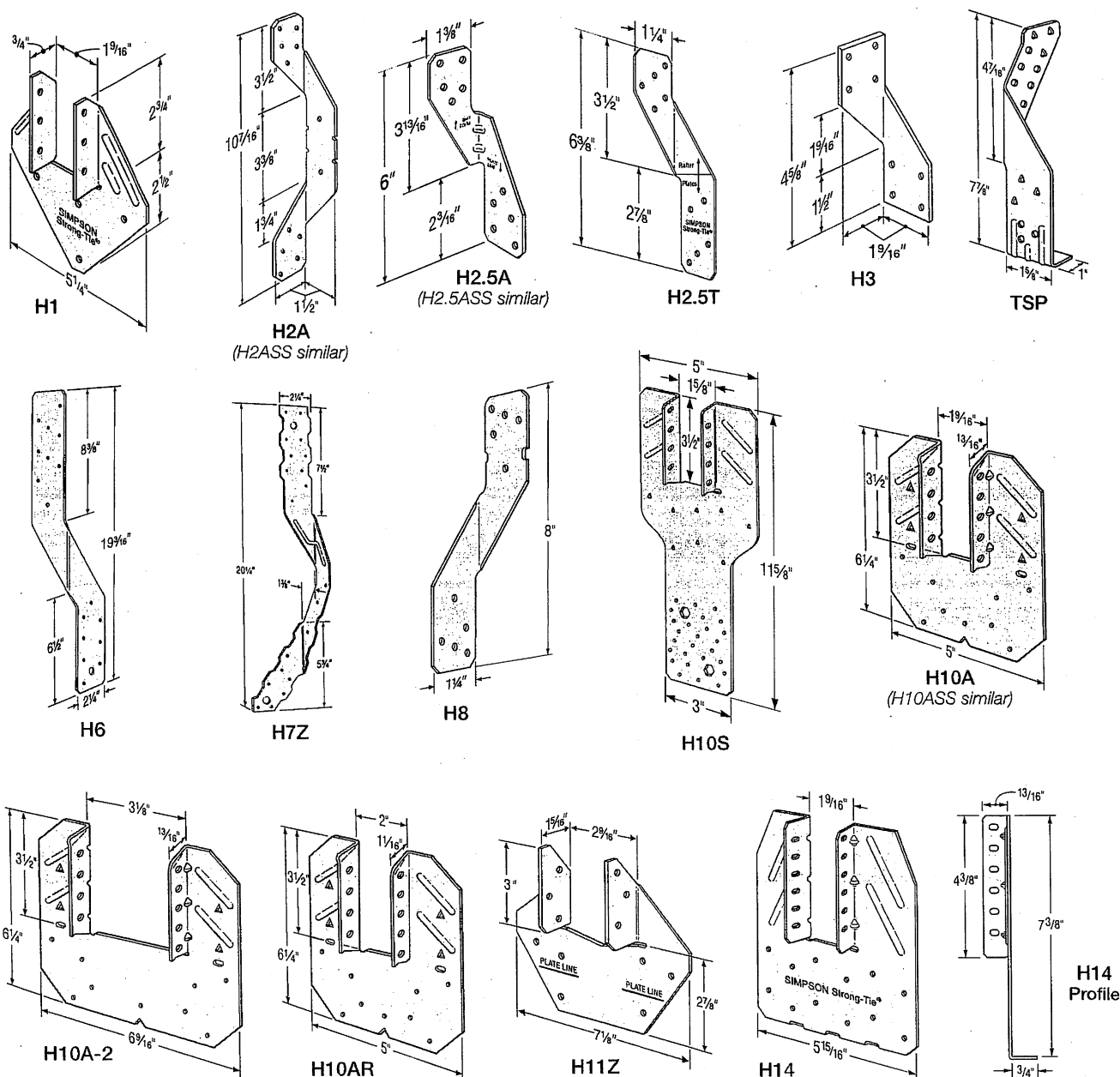
- H10AR — The heavy-duty design of the H10A available with a 2" wide throat to accommodate rough lumber
- H10A-2 — The H10A design with a 3" throat for double 2x members
- H2ASS, H2.5ASS and H10ASS — Popular ties now available in stainless steel

Material: See table

Finish: Galvanized. H7Z and H11Z — ZMAX® coating. Some models available in stainless steel or ZMAX; see Corrosion Information, pp. 20–24 or visit strongtie.com.

Installation:

- Use all specified fasteners; see General Notes.
- H1 can be installed with flanges facing inward (reverse of H1 installation drawing; number 1).
- H2.5T, H3 and H6 ties are shipped in equal quantities of right and left versions (right versions shown).
- Hurricane ties do not replace solid blocking.
- When installing ties on plated trusses (on the side opposite the truss plate) do not fasten through the truss plate from behind. This can force the truss plate off of the truss and compromise truss performance.
- H10A optional nailing to connect shear blocking, use 8d nails. Slots allow maximum field bending up to a pitch of 6:12, use H10A sloped loads for field bent installation.



H – Seismic and Hurricane Ties

SIMPSON
Strong-Tie

The H connector series provides wind and seismic ties for trusses and rafters.

Material: 18 gauge **Finish:** G90 galvanized

- Design:**
- Factored resistances are in accordance with CSA O86-14
 - Factored resistances have been increased 15%. No further increase is permitted.

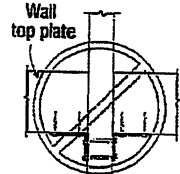
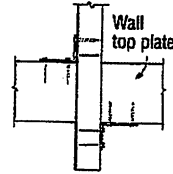
Installation:

- Use all specified fasteners

- Nails: 8d = 0.131" dia. x 2½" long common wire, 8d x 1½" = 0.131" x 1½" long, 10d x 1½" = 0.146" x 1½" long
- H1 can be installed with flanges facing outwards
- Hurricane ties do not replace solid blocking

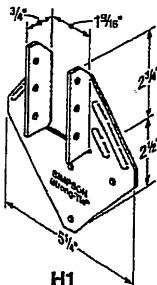
Factored resistances for more than one direction for a single connection cannot be added together. A factored load which can be divided into components in the directions given must be evaluated as follows: Factored Shear/Resisting Shear + Factored Tension/Resisting Tension ≤ 1.0.

Hurricane Tie Installations to Achieve Twice the Load (Top View)

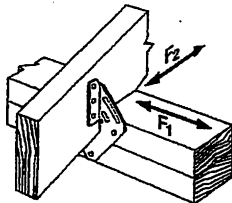


Install diagonally across from each other for minimum 2x truss.

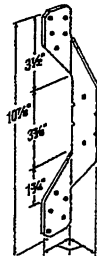
Nailing into both sides of a single ply 2x truss may cause the wood to split.



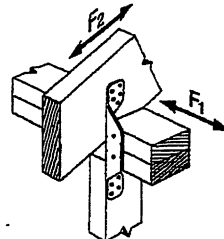
H1



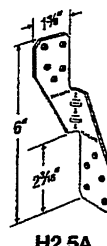
H1 Installation



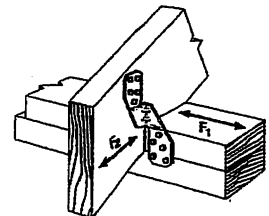
H2A



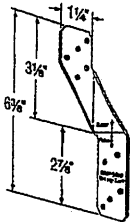
H2A Installation



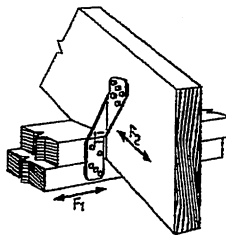
H2.5A



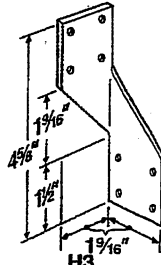
H2.5A Installation



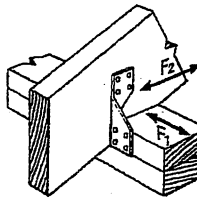
H2.5T



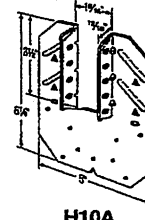
H2.5T Installation
(Nails into both top plates)



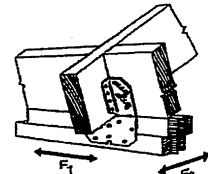
H3



H3 Installation



H10A

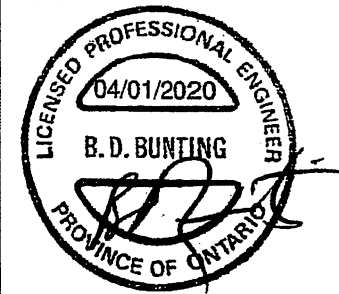


H10A Installation

Model No.	Ga.	Fasteners			Factored Resistance (lb.)					
					D.Fir-L			S-P-F		
		To Rafter	To Plates	To Studs	Uplift	Normal		Uplift	Normal	
						F ₁	F ₂		F ₁	F ₂
					(K _p =1.15)					(K _p =1.15)
H1	18	(6) 8d x 1½"	(4) 8d	—	740	685	300	680	485	215
H2A	18	(5) 8d x 1½"	(2) 8d x 1½"	(5) 8d x 1½"	830	220	75	590	155	55
H2.5A	18	(5) 8d	(5) 8d	—	805	160	160	755	160	160
H2.5T	18	(5) 8d	(5) 8d	—	835	175	240	740	160	210
H3	18	(4) 8d	(4) 8d	—	740	180	265	615	125	190
H10A	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1735	795	410	1505	565	290

1. Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.
2. Factored resistances are for one anchor. A minimum rafter thickness of 2½" must be used when framing anchors are installed on each side of the joist and on the same side of the plate.

3. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.
4. Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a Continuous Load Path, connections must be on same side of the wall.



LIMIT
STATES
DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECH20 3/20 exp. 6/22

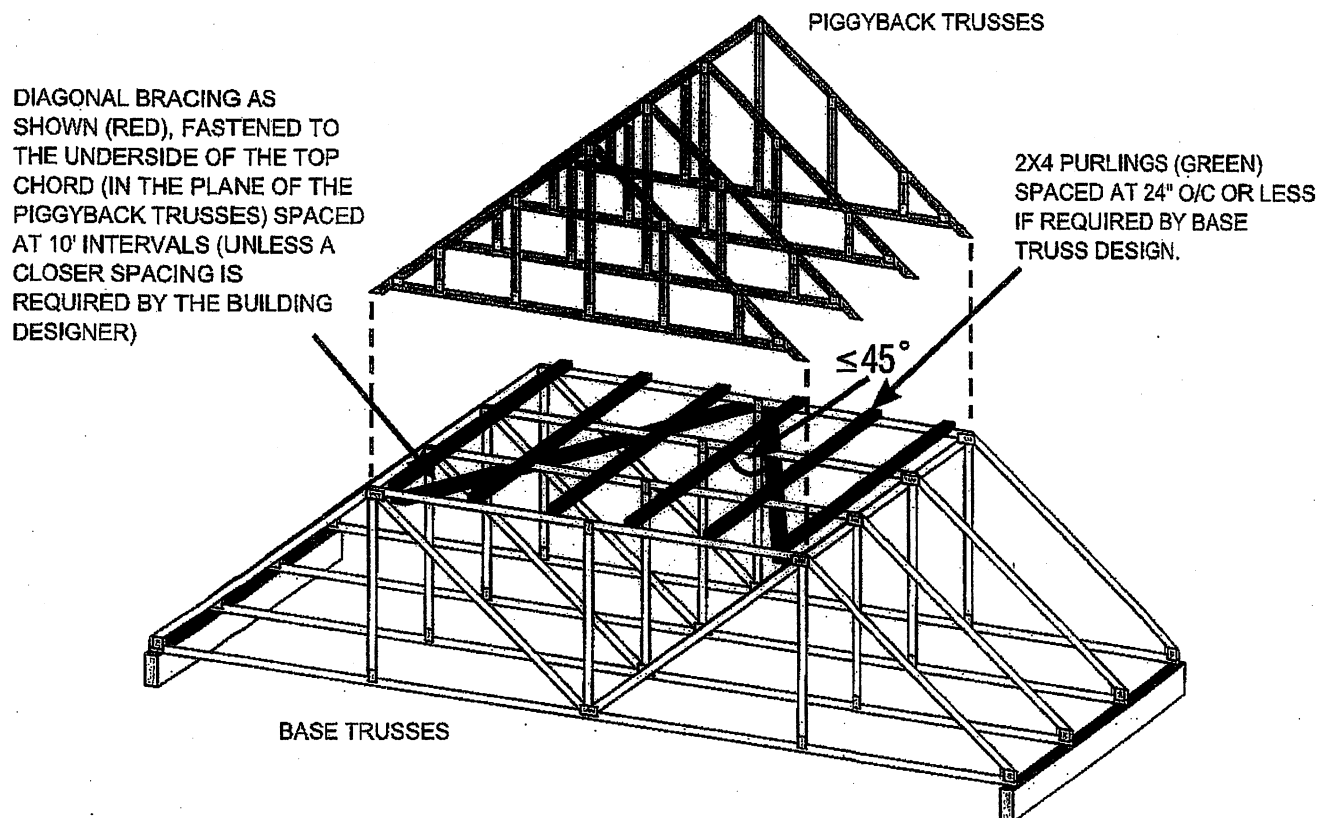
(800) 999-5099
strongtie.com

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

TS/LTS/MTS/HTS

Twist Straps

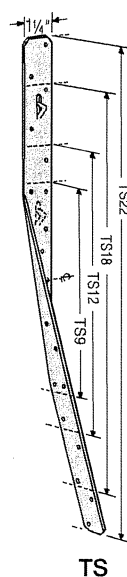
Twist straps provide a tension connection between two wood members. They resist uplift at the heel of a truss economically. The 3"-bend section eliminates interference at the transition points between wood members. TS twist straps come with an equal number of left and right hand units in each carton.

Material: LTS — 18 gauge; MTS/TS — 16 gauge; HTS — 14 gauge

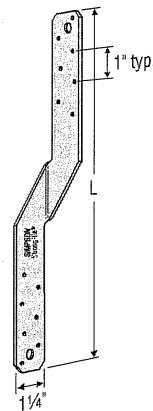
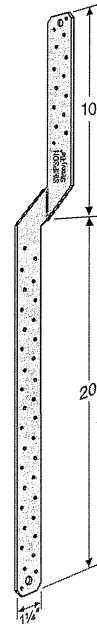
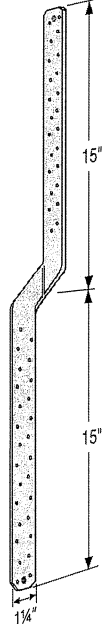
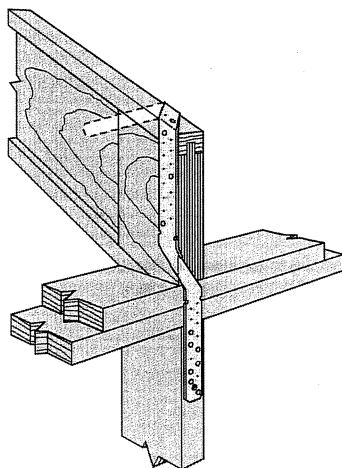
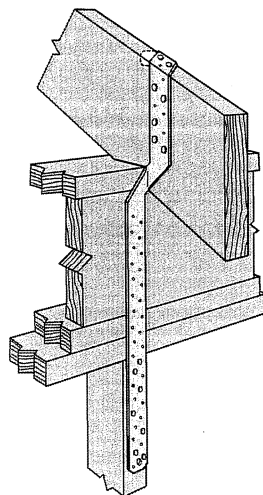
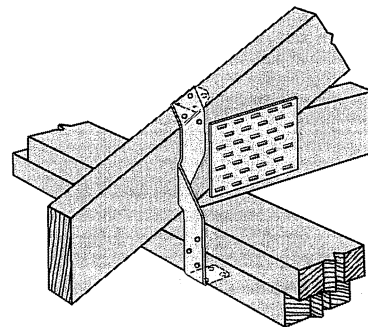
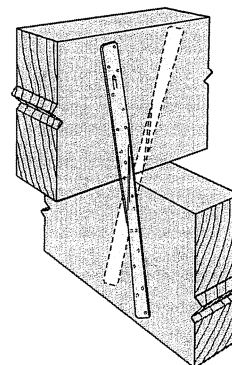
Finish: Galvanized. Some products available in stainless steel and ZMAX® coating; see Corrosion Information, pp. 20–24.

Installation:

- Use all specified fasteners; see General Notes
- TS should be installed in pairs to reduce eccentricity
- When LTS/MTS is installed as truss-to-top plate tie, install (3) 10d x 1½" nails to the underside of the plate and (3) 10d x 1½" nails into the edge of the double top plate
- LTS, MTS and HTS are available with the bend reversed. Specify "-REV" after model number, such as MTS16-REV



TS

LTS12
(MTS and HTS similar)MTS30
(HTS30 similar)HTS30C
(MTS30C similar)MTS30 Installation
with I-Joist RafterTypical MTS30
InstallationMTS Installation as a
Truss-to-Top Plate TieTypical TS
Installation

TS/LTS/MTS/HTS

Twist Straps (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

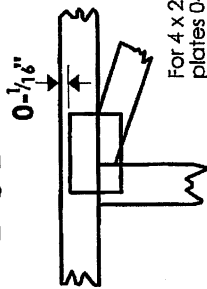
	Model No.	L (in.)	Fasteners (Total)	Factored Resistance ($K_D = 1.15$)	
				D.Fir-L	S-P-F
				lb.	lb.
				kN	kN
	TS9	9	(8) 16d	1125	1040
				5.00	4.63
	TS12	11½	(10) 16d	1410	1300
				6.27	5.78
	TS18	17¾	(14) 16d	1970	1820
				8.76	8.10
	TS22	21¾	(18) 16d	2125	2125
				9.45	9.45
SS	LTS12	12	(12) 10d x 1½"	1015	720
				4.52	3.20
	LTS16	16	(12) 10d x 1½"	1015	720
				4.52	3.20
SS	LTS18	18	(12) 10d x 1½"	1015	720
				4.52	3.20
	LTS20	20	(12) 10d x 1½"	1015	720
				4.52	3.20
	MTS12	12	(14) 10d x 1½"	1570	1180
				6.98	5.25
	MTS16	16	(14) 10d x 1½"	1570	1180
				6.98	5.25
	MTS18	18	(14) 10d x 1½"	1570	1180
				6.98	5.25
SS	MTS20	20	(14) 10d x 1½"	1570	1180
				6.98	5.25
	MTS30	30	(14) 10d x 1½"	1570	1180
				6.98	5.25
	MTS24C	24	(14) 10d x 1½"	1570	1180
				6.98	5.25
	MTS30C	30	(14) 10d x 1½"	1570	1180
				6.98	5.25
	HTS16	16	(16) 10d x 1½"	2050	1455
				9.12	6.47
	HTS20	20	(24) 10d x 1½"	2050	1455
				9.12	6.47
	HTS24	24	(24) 10d x 1½"	2050	1455
				9.12	6.47
	HTS28	28	(24) 10d x 1½"	2050	1455
				9.12	6.47
	HTS30	30	(24) 10d x 1½"	2050	1455
				9.12	6.47
	HTS30C	30	(24) 10d x 1½"	2050	1455
				9.12	6.47

1. LTS12 through LTS20, MTS16 through MTS30, HTS24 through HTS30C (except HTS30) have additional nail holes.
2. Install half of the fasteners on each end of strap to achieve maximum factored resistance.
3. Factored resistances have been increased 15% for earthquake or wind loading. No further increase allowed; reduce where other loads govern.
4. All straps except the MTS30 and HTS30 have the twist in the centre of the strap.
5. Twist straps do not have to be wrapped over the truss to achieve the load.
6. Optional nail holes are provided on some straps.
7. When used as a truss-to-top plate tie multiply the tabulated values by 0.95 for LTS and 0.74 for MTS. HTS cannot be used in this application.
8. Nails: 10d = 0.148" dia. x 3" long, 10d x 1½" = 0.148" dia. x 1½" long. See pp. 27–28 for other nail sizes and information.

Symbols

PLATE LOCATION AND ORIENTATION

Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/8" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.



* Plate location details available in Mitek software or upon request.

PLATE SIZE

4 X 4

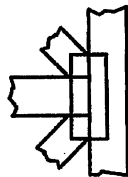
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



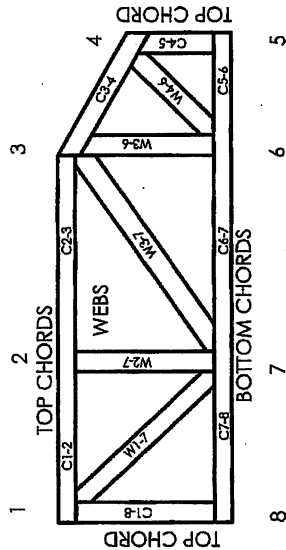
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

- TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
- DSB-89: Design Standard for Bracing
- BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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Mitek
POWER TO PERFORM.™

Mitek Engineering Reference Sheet: MII-7473C rev. 10-'08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and waste at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with the retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.