

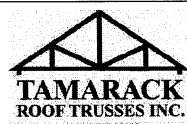
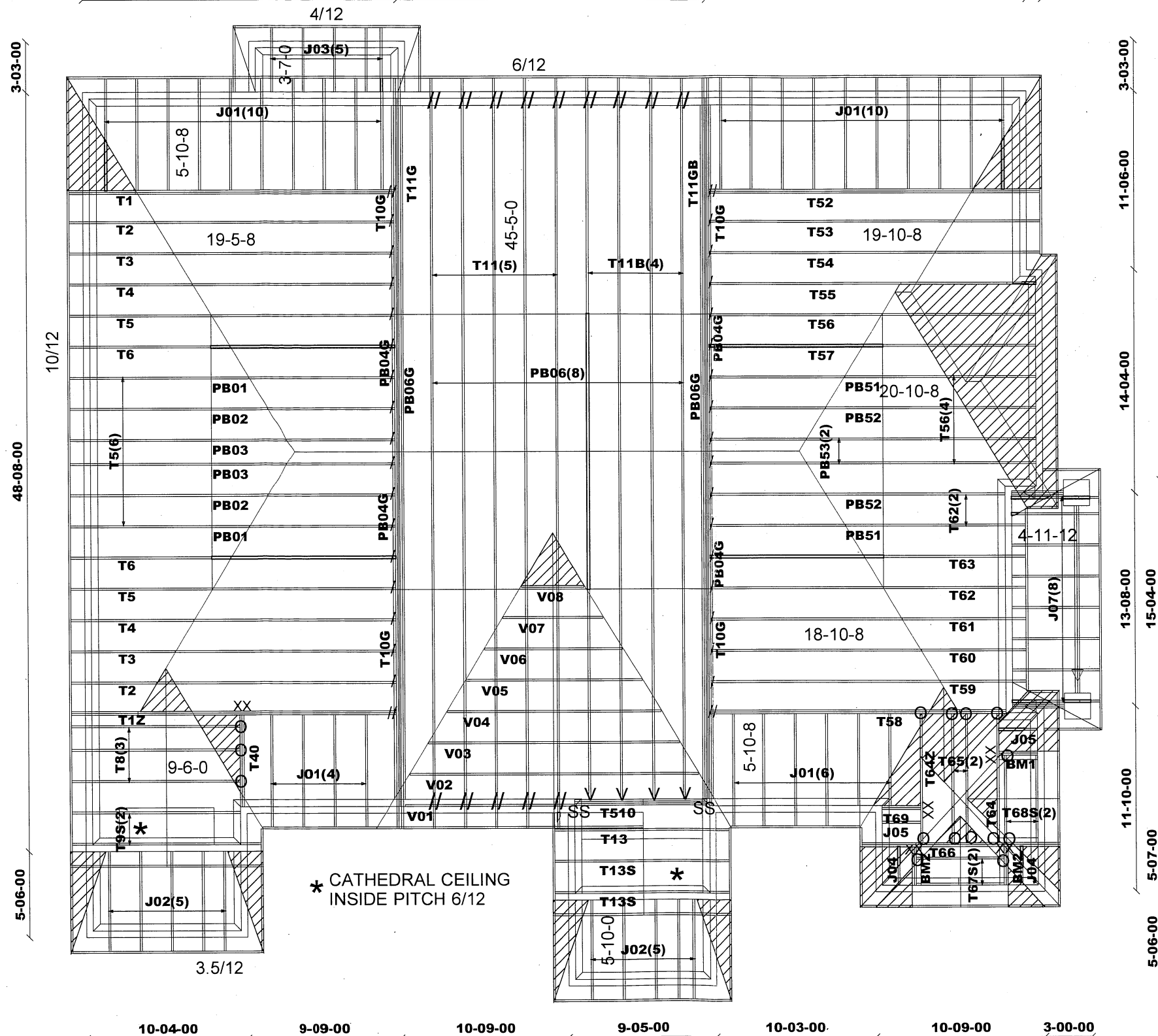
20-01-00

19-08-00

20-06-00

1-00-00

All conventional framing to conform with Part 9 of O.B.C. 2012. Roof rafters that cross over or meet trusses to be min. 2x4 SFP #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.



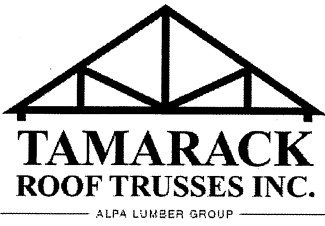
Layout ID: 412982

Designer: AC

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Mitek ver 8.4.2.286

DELIVERY SHIPLIST

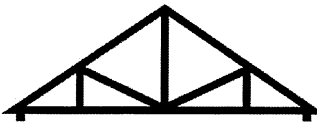


Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: CENTREFIELD
 Location: RICHMOND HILL
 Model: BLOCK 53
 Lot #:
 Elevation: A1 / UNIT43BLK289

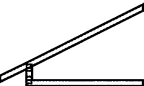
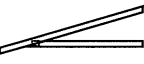
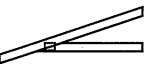
Job Track: 51012
 PlanLog: 203531
 Layout ID: 412995
 Ref #
 Page: 1 of 2
 Date: 05-31-2021
 Designer: Andrew Conway
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Half Hip Girder	10 / 12	19-05-08	4-01-04	2 x 4 2 x 6	1-03-08	1-07-11 4-01-04	190.33 121.67		
	1 2-ply	T1Z Half Hip Girder	10 / 12	19-05-08	4-01-04	2 x 4 2 x 6	1-03-08	1-07-11 4-01-04	190.33 121.67		
	2	T2 Half Hip	10 / 12	19-05-08	5-01-04	2 x 4	1-03-08	1-07-11 5-01-04	169.97 109.00		
	2	T3 Half Hip	10 / 12	19-05-08	6-01-04	2 x 4	1-03-08	1-07-11 6-01-04	182.13 117.33		
	2	T4 Half Hip	10 / 12	19-05-08	7-01-04	2 x 4	1-03-08	1-07-11 7-01-04	181.47 112.67		
	8	T5 Half Hip	10 / 12	19-05-08	8-01-04	2 x 4	1-03-08	1-07-11 8-01-04	766.42 485.33		
	2	T6 Half Hip	10 / 12	19-05-08	9-01-04	2 x 4	1-03-08	1-07-11 9-01-04	206.88 129.67		
	3	T8 Common	10 / 12	9-06-00	5-07-03	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	130.39 86.50		
	2	T9S Roof Special	10 / 12 6 / 12	9-06-00	5-07-03	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	95.47 67.33		
	2	T10G GABLE	6 / 12	22-08-08	8-01-04	2 x 4	1-05-00	1-02-00 8-01-04	243.75 153.67		
	1 2-ply	T40 Monopitch Girder	6 / 12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	58.39 37.67		
	2	PB01 Piggyback	10 / 12	11-08-07	2-00-00	2 x 4		2-00-00	75.37 51.33		
	2	PB02 Piggyback	10 / 12	11-08-07	3-00-00	2 x 4		3-00-00	81.28 54.67		
	2	PB03 Piggyback	10 / 12	11-08-07	4-00-00	2 x 4		4-00-00	83.07 54.00		

DELIVERY SHIPLIST				
 TAMARACK ROOF TRUSSES INC. ALPHA LUMBER GROUP	Lumber Yard:	TAMARACK LUMBER	Job Track:	51012
	Builder:	ROYAL PINE HOMES	PlanLog:	203531
	Project:	CENTREFIELD	Layout ID:	412995
	Location:	RICHMOND HILL	Ref #	
	Model:	BLOCK 53	Page:	2 of 2
	Lot #:		Date:	05-31-2021
	Elevation:	A1 / UNIT43BLK289	Designer:	Andrew Conway
		Sales Rep:	Mario DiCano	

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	PB04G GABLE	6 /12	8-10-00	4-05-00	2 x 4		4-05-00	56.15 34.33		
	14	J01 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	235.12 149.33		
	5	J02 Jack-Open	3.5 /12	5-10-00	2-04-09	2 x 4	1-03-08	3-14 1-09-11	76.06 46.67		
	5	J03 Jack-Open	4 /12	3-07-00	1-11-03	2 x 4	1-03-08	3-15 1-03-04	50.28 33.33		

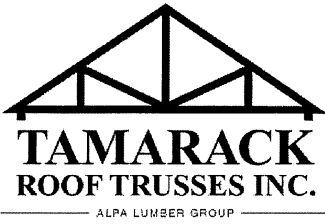
TOTAL # TRUSS= 61 TOTAL BFT OF ALL TRUSSES= 1966.17 BFT. TOTAL WEIGHT OF ALL TRSSES 3072.88 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
3	Hardware	LUS24	
1	Hardware	HGUS26-2	
20	Hardware	H2.5T	

TOTAL NUMBER OF ITEMS= 24

DELIVERY SHIPLIST

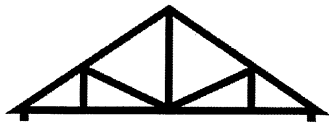


Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: CENTREFIELD
 Location: RICHMOND HILL
 Model: BLOCK 53
 Lot #:
 Elevation: A1 / UNIT42BLK289

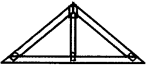
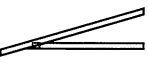
Job Track: 51012
 PlanLog: 203531
 Layout ID: 412996
 Ref #
 Page: 1 of 2
 Date: 05-31-2021
 Designer: Andrew Conway
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	5	T11 Piggyback Base	6 /12	45-05-00	8-01-04	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	1496.73 900.00		
	4	T11B Common	6 /12	45-00-00	8-01-04	2 x 6	1-03-08	1-02-00 1-04-08	1169.26 708.00		
	2	T11G GABLE	6 /12	45-05-00	8-01-04	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	544.11 339.33		
	1	T13 Common	10 /12	8-07-00	5-02-10	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	40.03 26.83		
	2	T13S Scissor	10 /12 6 /12	8-07-00	5-02-10	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	81.56 52.00		
	1 3-ply	T510 Common Girder	10 /12	8-05-00	5-02-10	2 x 4 2 x 8		1-07-11 1-07-11	143.61 98.00		
	8	PB06 Piggyback	6 /12	17-08-00	4-05-00	2 x 4			410.9 256.00		
	2	PB06G GABLE	6 /12	17-08-00	4-05-00	2 x 4			101.52 62.00		
	1	V01 Valley	10 /12	19-00-00	8-08-02	2 x 4		9-02 9-02	48.86 31.50		
	1	V02 Valley	10 /12	18-05-01	7-08-02	2 x 4			59.02 36.67		
	1	V03 Valley	10 /12	16-00-04	6-08-02	2 x 4			50.28 30.50		
	1	V04 Valley	10 /12	13-07-07	5-08-02	2 x 4			41.54 26.33		
	1	V05 Valley	10 /12	11-02-10	4-08-02	2 x 4			31.56 19.83		
	1	V06 Valley	10 /12	8-09-14	3-08-02	2 x 4			24.45 16.00		

DELIVERY SHIPLIST				
 TAMARACK ROOF TRUSSES INC. ALPA LUMBER GROUP	Lumber Yard:	TAMARACK LUMBER	Job Track:	51012
	Builder:	ROYAL PINE HOMES	PlanLog:	203531
	Project:	CENTREFIELD	Layout ID:	412996
	Location:	RICHMOND HILL	Ref #	
	Model:	BLOCK 53	Page:	2 of 2
	Lot #:		Date:	05-31-2021
	Elevation:	A1 / UNIT42BLK289	Designer:	Andrew Conway
			Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	V07 Valley	10 /12	6-05-01	2-08-02	2 x 4			17.33 11.50		
	1	V08 Valley	10 /12	4-00-04	1-08-02	2 x 4			9.4 6.67		
	5	J02 Jack-Open	3.5 /12	5-10-00	2-04-09	2 x 4	1-03-08	3-14 1-09-11	76.06 46.67		

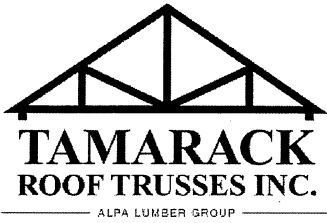
TOTAL # TRUSS= 40 TOTAL BFT OF ALL TRUSSES= 2667.83 BFT. TOTAL WEIGHT OF ALL TRSSES 4346.2 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
4		HTS16	
28	Hardware	H2.5T	
4	Hardware	LJS26DS	

TOTAL NUMBER OF ITEMS= 36

DELIVERY SHIPLIST



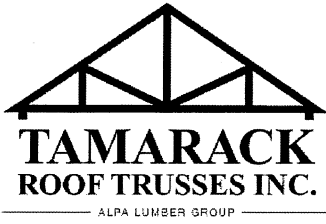
Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: CENTREFIELD
 Location: RICHMOND HILL
 Model: BLOCK 53
 Lot #:
 Elevation: A / UNIT41BLK289

Job Track: 51012
 PlanLog: 203531
 Layout ID: 412997
 Ref #
 Page: 1 of 3
 Date: 05-31-2021
 Designer: Andrew Conway
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	T10G GABLE	6 / 12	22-08-08	8-01-04	2 x 4	1-05-00	1-02-00 8-01-04	243.75 153.67		
	1 2-ply	T52 Half Hip Girder	10 / 12	19-10-08	4-01-04	2 x 4	1-03-08	1-07-11 4-01-04	170.05 108.33		
	1	T53 Half Hip	10 / 12	19-10-08	5-01-04	2 x 4	1-03-08	1-07-11 5-01-04	86.19 54.50		
	1	T54 Half Hip	10 / 12	19-10-08	6-01-04	2 x 4	1-03-08	1-07-11 6-01-04	92.23 58.67		
	1	T55 Roof Special	10 / 12	20-10-08	7-01-04	2 x 4		1-06-00 7-01-04	93.57 57.83		
	5	T56 Piggyback Base	10 / 12	20-10-08	8-01-04	2 x 4		1-06-00 8-01-04	503.75 318.33		
	1	T57 Roof Special	10 / 12	20-10-08	9-01-04	2 x 4		1-06-00 9-01-04	106.33 66.33		
	1 2-ply	T58 Half Hip Girder	10 / 12	18-10-08	4-01-04	2 x 4 2 x 6		1-07-11 4-01-04	182.37 113.00		
	1	T59 Half Hip	10 / 12	18-10-08	5-01-04	2 x 4	1-03-08	1-07-11 5-01-04	83.31 53.17		
	1	T60 Half Hip	10 / 12	18-10-08	6-01-04	2 x 4	1-03-08	1-07-11 6-01-04	81.77 51.33		
	1	T61 Half Hip	10 / 12	18-10-08	7-01-04	2 x 4	1-03-08	1-07-11 7-01-04	89.08 55.67		
	3	T62 Half Hip	10 / 12	18-10-08	8-01-04	2 x 4	1-03-08	1-07-11 8-01-04	282.59 180.00		
	1	T63 Half Hip	10 / 12	18-10-08	9-01-04	2 x 4	1-03-08	1-07-11 9-01-04	101.88 63.50		
	1	T64 Half Hip Girder	9.22 / 1 2	8-03-00	3-08-04	2 x 4 2 x 6		1-06-15 3-08-04	39 25.33		

DELIVERY SHIPLIST

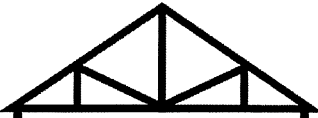


Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: CENTREFIELD
 Location: RICHMOND HILL
 Model: BLOCK 53
 Lot #:
 Elevation: A / UNIT41BLK289

Job Track: 51012
 PlanLog: 203531
 Layout ID: 412997
 Ref #
 Page: 2 of 3
 Date: 05-31-2021
 Designer: Andrew Conway
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T64Z Half Hip Girder	9.22 /1 2	8-03-00	3-08-04	2 x 4 2 x 6		1-06-15 3-08-04	39 25.33		
	2	T65 Hip	9.22 /1 2	8-03-00	5-04-04	2 x 4		1-06-15 3-07-09	90.08 61.33		
	1 2-ply	T66 Hip Girder	10 /12	9-11-00	3-05-04	2 x 4	1-03-08 1-05-00	1-07-11 1-07-11	96.4 63.00		
	2	T67S Scissor	10 /12 6 /12	5-07-00	4-11-04	2 x 4		2-07-05 2-07-05	57.87 40.67		
	2	T68S Scissor	10 /12 6 /12	5-07-00	5-04-11	2 x 4		3-00-12 3-00-12	61.62 40.67		
	1 2-ply	T69 Monopitch Girder	10 /12	2-05-08	3-08-04	2 x 4 2 x 6		1-07-11 3-08-04	29.05 21.00		
	2	PB04G GABLE	6 /12	8-10-00	4-05-00	2 x 4		4-05-00	56.15 34.33		
	2	PB51 Piggyback	10 /12	11-01-07	2-00-00	2 x 4		2-00-00	71.79 48.67		
	2	PB52 Piggyback	10 /12	11-01-07	3-00-00	2 x 4		3-00-00	77.82 52.00		
	2	PB53 Piggyback	10 /12	11-01-07	4-00-00	2 x 4		4-00-00	76.34 49.67		
	16	J01 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	268.71 170.67		
	2	J04 Jack-Open	9.22 /1 2	2-05-08	3-05-04	2 x 4	1-03-08	1-06-09 3-05-04	22.53 15.33		
	2	J05 Jack-Open	10 /12	2-05-08	3-08-04	2 x 4	1-03-08	1-07-11 3-08-04	23.25 16.67		
	8	J07 Jack-Open	4 /12	4-11-12	2-04-12	2 x 4	1-03-08	3-15 1-09-03	106.61 64.00		

 TAMARACK ROOF TRUSSES INC. ALPA LUMBER GROUP	DELIVERY SHIPLIST			
	Lumber Yard: TAMARACK LUMBER Builder: ROYAL PINE HOMES Project: CENTREFIELD Location: RICHMOND HILL Model: BLOCK 53 Lot #: Elevation: A / UNIT41BLK289	Job Track: 51012 PlanLog: 203531 Layout ID: 412997 Ref # Page: 3 of 3 Date: 05-31-2021 Designer: Andrew Conway Sales Rep: Mario DiCano		

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
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TOTAL # TRUSS= 70
TOTAL BFT OF ALL TRUSSES= 2063
BFT.
TOTAL WEIGHT OF ALL TRSSES 3233.09 LBS

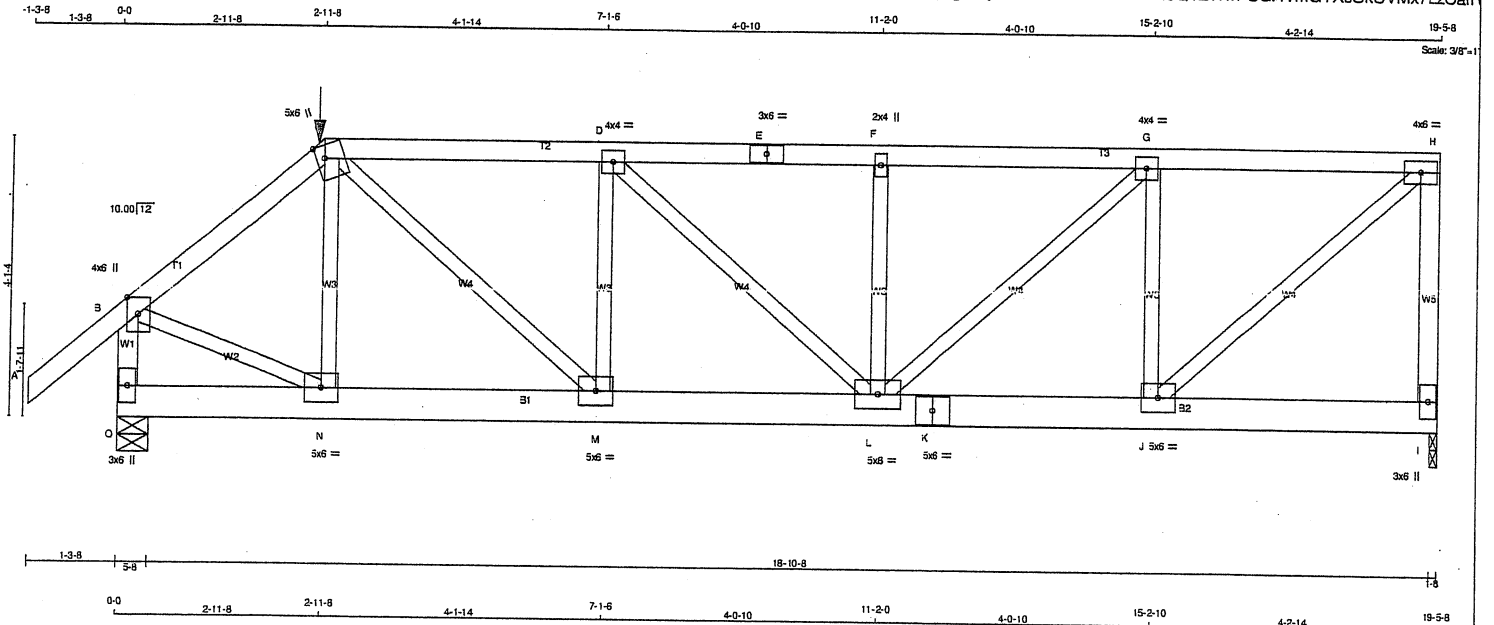
HARDWARE

QTY	TYPE	MODEL	LENGTH
12	Hardware	LUS24	
4	Hardware	HGUS26-2	
20	Hardware	H2.5T	

TOTAL NUMBER OF ITEMS= 36

JOB NAME 412982	TRUSS NAME T1	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.420 S Jan 21 2021 MITek Industries, Inc. Wed May 26 16:46:26 2021 Page 1
ID:wTM7iolSByniqocTeDgf3Pyb9F8-vK3rTdcRSH1I0Q4EWwFSGHvmGYXcOkUVMx7LzCaIf



TOTAL WEIGHT = 2 X 95 = 190 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
O - K	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
I - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C 1	12	SIDE(61.0)
C - E 1	12	SIDE(43.0)
E - H 1	12	SIDE(43.0)
O - B 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
O - K 2	12	SIDE(9.7)
K - I 2	12	SIDE(9.7)
WEBS : (0.122"x3") SPIRAL NAILS		
N - C 1	6	SIDE(8.7)
2x3 1	6	
2x4 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	DOWN	BRG	UPLIFT	IN-SX	IN-SX
I	2100	0	0	2100	0	0	1-8	1-8	
O	2123	0	0	2123	0	0	5-8	5-8	

UNFACTORED REACTIONS

1ST CASE	MAX./MIN.	COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1484	978 / 0	0 / 0	0 / 0	0 / 0	506 / 0	0 / 0
O	1500	991 / 0	0 / 0	0 / 0	0 / 0	509 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.26 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	FORCE (LBS)	MEMB.	FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 41	I-H	-2029 / 0
B-C	-1919 / 0	N-C	-427 / 0
C-D	-2600 / 0	B-N	0 / 1582
D-E	-2664 / 0	J-H	0 / 2612
E-F	-2664 / 0	C-M	0 / 1531
F-G	-2664 / 0	J-G	-1564 / 0
G-H	-1882 / 0	M-D	-880 / 0
O-B	-2084 / 0	L-G	0 / 1062
		D-L	0 / 87
		L-F	-656 / 0
O-N	0 / 0		
N-M	0 / 1457		
M-L	0 / 2600		
L-K	0 / 1882		
K-J	0 / 1882		
J-I	-79 / 0		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-11-8	-136	-136		FRONT	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.6	PSF
	DL	= 6.0	PSF
BOT CH.	LL	= 0.0	PSF
	DL	= 7.4	PSF
TOTAL LOAD		= 39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 2-11-8
RIGHT SETBACK = 0-0
END SETBACK = 5-10-8
END WALL WIDTH = 1-8
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADD'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.64")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.64")
CALCULATED VERT. DEFL.(TL) = L/999 (0.07")

CSI: TC=0.27/1.00 (F-G:1), BC=0.21/1.00 (L-M:1), WB=0.32/1.00 (H-J:1), SS=0.19/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

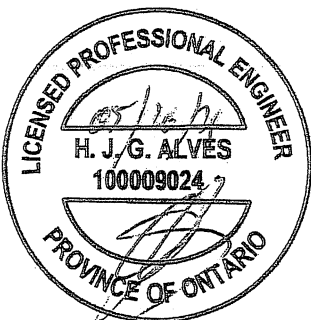
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.80 (H) (INPUT = 0.90)
JSI METAL=0.43 (B) (INPUT = 1.00)



Structural component only
DWG# T-2115901 1/2

CONTINUED ON PAGE 2

JOB NAME 412982	TRUSS NAME T1	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES TRUSS DESC.	DRWG NO.
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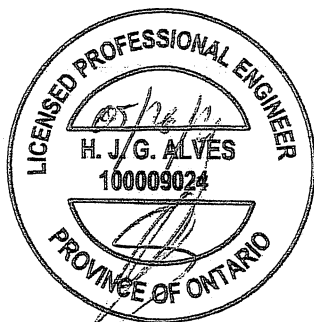
Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed May 26 16:46:26 2021 Page 2
ID:wTM7iolSBYniqocTeDgf3Pvb9F8-vK3rTdckRShH1I0Q4EWWFSGHVmGYXcOkUVMx7LzCaIR

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TMWW-t	MT20	4.0	4.0		
H	TMWW-t	MT20	4.0	6.0		
I	BMV1+w	MT20	3.0	6.0		
J, M, N						
J	BMWW-t	MT20	5.0	6.0		
K	BS-t	MT20	5.0	6.0		
L	BMWW-t	MT20	5.0	8.0		
O	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

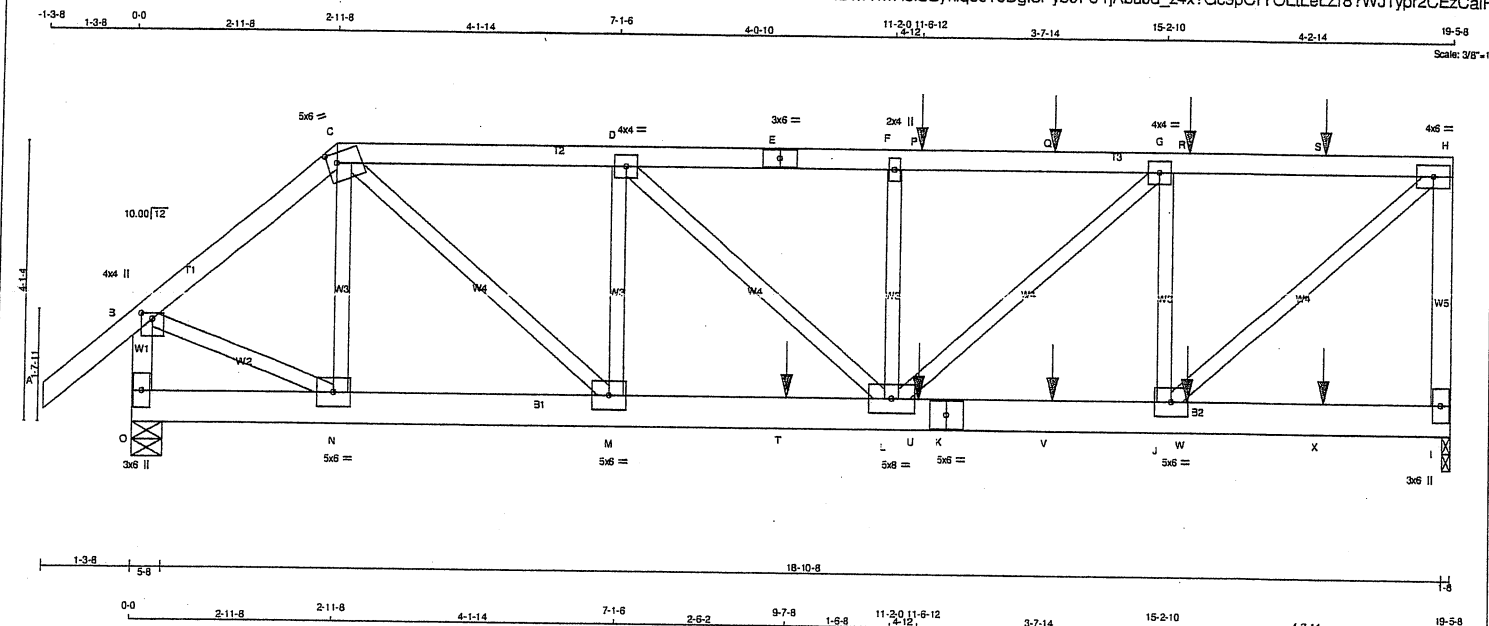


Structural component only
DWG# T-2115901 *216*

JOB NAME 412982	TRUSS NAME T1Z	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MITek Industries, Inc. Wed May 26 16:46:28 2021 Page 1
ID:wTM7ioISByniqocTeDg3Pyb9F8-rjAbuJd_z4x?Gc9pCFYOLtLeLZr8?WJ1ypr2CEzCaIP



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4 DRY No.2	SPF			
C - E	2x4 DRY No.2	SPF			
E - H	2x4 DRY No.2	SPF			
O - B	2x4 DRY No.2	SPF			
O - K	2x6 DRY No.2	SPF			
K - I	2x6 DRY No.2	SPF			

ALL WEBS	2x3 DRY No.2	SPF
EXCEPT		
I - H	2x4 DRY No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C 1	12	TOP
C - E 1	12	TOP
E - H 1	12	SIDE(61.0)
O - B 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
K - I 2	12	SIDE(0.0)
K - I 2	12	SIDE(183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	
2x4 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	HORZ	GROSS REACTION	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	IN-SX
I	2204	0	2204	0	0	0	1-8	1-8	
O	2044	0	2044	0	0	0	5-8	5-8	

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
I	1555	1040 / 0	0 / 0	0 / 0	0 / 0	515 / 0	0 / 0
O	1439	981 / 0	0 / 0	0 / 0	0 / 0	458 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.79 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 41	-91.8 -91.8 0.07 (1)	10.00	I-H	-2127 / 0	0.19 (1)	
B-C	-1832 / 0	-91.8 -91.8 0.09 (1)	6.25	N-C	-599 / 0	0.08 (1)	
C-D	-3033 / 0	-91.8 -91.8 0.15 (1)	5.17	B-N	0 / 1510	0.19 (1)	
D-E	-3433 / 0	-91.8 -91.8 0.15 (1)	4.91	J-H	0 / 2911	0.36 (1)	
E-F	-3433 / 0	-91.8 -91.8 0.15 (1)	4.91	C-M	0 / 2208	0.27 (1)	
F-P	-3433 / 0	-91.8 -91.8 0.25 (1)	4.79	J-G	-1931 / 0	0.24 (1)	
P-Q	-3433 / 0	-91.8 -91.8 0.25 (1)	4.79	M-D	-770 / 0	0.10 (1)	
Q-G	-3433 / 0	-91.8 -91.8 0.25 (1)	4.79	L-G	0 / 1807	0.22 (1)	
G-R	-2103 / 0	-91.8 -91.8 0.22 (1)	5.83	D-L	0 / 542	0.07 (1)	
R-S	-2103 / 0	-91.8 -91.8 0.22 (1)	5.83	L-F	-469 / 0	0.06 (1)	
S-H	-2103 / 0	-91.8 -91.8 0.22 (1)	5.83				
O-B	-2002 / 0	0.0 0.0 0.11 (1)	7.78				
O-N	0 / 0	-18.5 -18.5 0.02 (1)	10.00				
N-M	0 / 1386	-18.5 -18.5 0.21 (1)	10.00				
M-T	0 / 3034	-18.5 -18.5 0.58 (1)	10.00				
T-L	0 / 3034	-18.5 -18.5 0.58 (1)	10.00				
L-U	0 / 2103	-18.5 -18.5 0.27 (1)	10.00				
U-K	0 / 2103	-18.5 -18.5 0.27 (1)	10.00				
K-V	0 / 2103	-18.5 -18.5 0.27 (1)	10.00				
V-J	0 / 2103	-18.5 -18.5 0.27 (1)	10.00				
J-W	-83 / 0	-18.5 -18.5 0.04 (1)	6.25				
W-X	-83 / 0	-18.5 -18.5 0.04 (1)	6.25				
X-I	-83 / 0	-18.5 -18.5 0.04 (1)	6.25				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
P	11-6-12	-76	-76	FRONT	VERT	TOTAL	---	C1
Q	13-6-12	-76	-76	FRONT	VERT	TOTAL	---	C1
R	15-6-12	-76	-76	FRONT	VERT	TOTAL	---	C1
S	17-6-12	-76	-76	FRONT	VERT	TOTAL	---	C1
T	9-7-8	-1003	-1003	FRONT	VERT	TOTAL	---	C1
U	11-6-12	-21	-21	FRONT	VERT	TOTAL	---	C1
V	13-6-12	-21	-21	FRONT	VERT	TOTAL	---	C1
W	15-6-12	-21	-21	FRONT	VERT	TOTAL	---	C1
X	17-6-12	-21	-21	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.64")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL) = L/360 (0.64")
CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CSI: TC=0.25/1.00 (F-G:1), BC=0.58/1.00 (L-M:1),
WB=0.36/1.00 (H-J:1), SS=0.31/1.00 (L-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.90 (B) (INPUT = 0.90)
JSI METAL = 0.32 (H) (INPUT = 1.00)



Structural component only
DWG# T-2115902

CONTINUED ON PAGE 2

JOB NAME 412982	TRUSS NAME T1Z	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed May 26 16:46:28 2021 Page 2
ID:wTM7ioISBvniqocTeDg3Pyb9F8-riAbuJd z4x?Gc9pCfYOLtLeLZr8?WJ1ypr2CEzCaIP

PLATES (table is in inches)

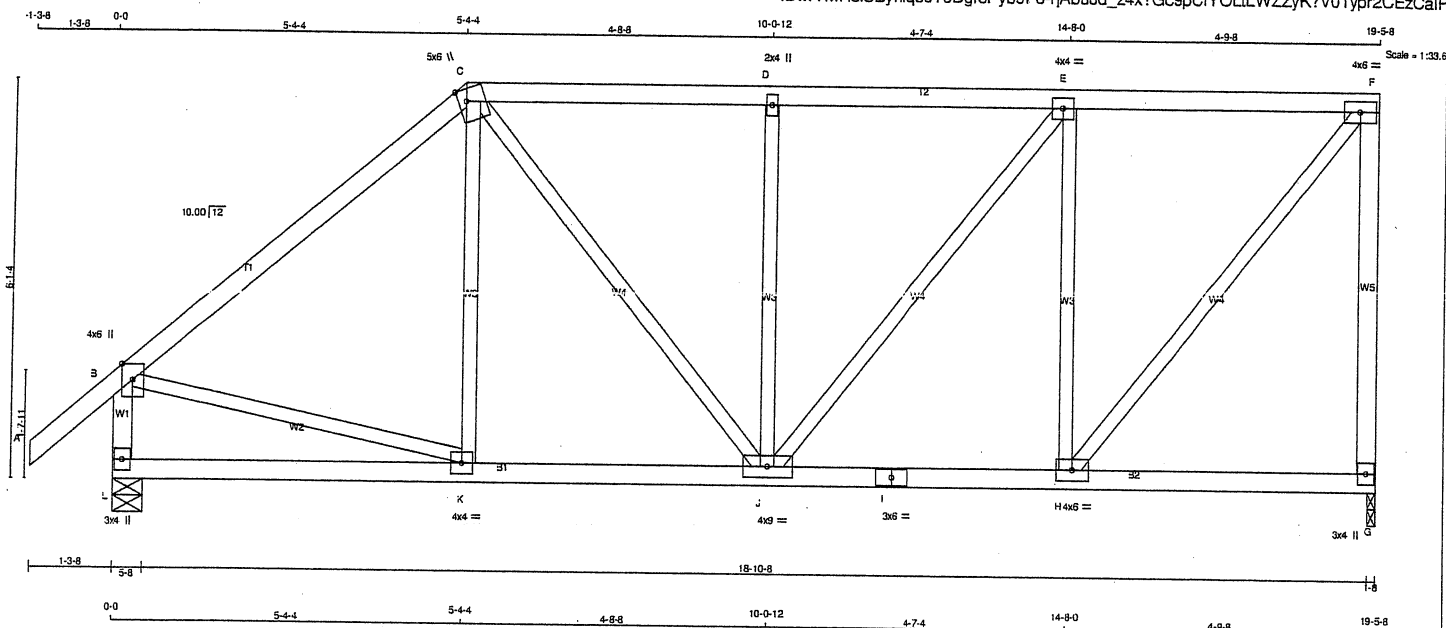
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTWW-m	MT20	5.0	6.0	1.75	1.75
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TMWW-t	MT20	4.0	4.0		
H	TMWW-t	MT20	4.0	6.0		
I	BMW1+w	MT20	3.0	6.0		
J, M, N						
J	BMWW-t	MT20	5.0	6.0		
K	BS-t	MT20	5.0	6.0		
L	BMWW-t	MT20	5.0	8.0		
O	BMV1+p	MT20	3.0	6.0		



Structural component only
DWG# T-2115902 *7/2*

EWING 7240000		
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JOB NAME 412982	TRUSS NAME T3	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					
Version 8.420 S Jan 21 2021 MITek Industries, Inc. Wed May 26 16:46:28 2021 Page 1 ID:wTM7IolSBYniqocTeDgf3Pyb9F8-rjAbuJd_z4x?Gc9pCfYOLILWZZyK?V01ypr2CEzCaIf					
TRUSS DESC.					



LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQ'D
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	UPLIFT
G	1073	0	1073	0
L	1200	0	1200	0

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	759	498 / 0	0 / 0	0 / 0	0 / 0	261 / 0	0 / 0
L	846	569 / 0	0 / 0	0 / 0	0 / 0	277 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.62 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 41	-91.8 -91.8 0.13 (1)	10.00	K-C	-84 / 62	0.04 (1)	
B-C	-952 / 0	-91.8 -91.8 0.51 (1)	5.62	B-K	0 / 751	0.17 (1)	
C-D	-931 / 0	-91.8 -91.8 0.28 (1)	6.04	H-F	0 / 1112	0.25 (1)	
D-E	-932 / 0	-91.8 -91.8 0.29 (1)	6.01	C-J	0 / 321	0.07 (1)	
E-F	-708 / 0	-91.8 -91.8 0.29 (1)	6.25	H-E	-758 / 0	0.44 (1)	
G-F	-1037 / 0	0.0 0.0 0.75 (1)	7.71	J-D	-460 / 0	0.27 (1)	
L-B	-1159 / 0	0.0 0.0 0.12 (1)	7.40	J-E	0 / 361	0.08 (1)	
L-K	0 / 0	-18.5 -18.5 0.12 (4)	10.00				
K-J	0 / 729	-18.5 -18.5 0.19 (1)	10.00				
J-I	0 / 708	-18.5 -18.5 0.17 (1)	10.00				
I-H	0 / 708	-18.5 -18.5 0.17 (1)	10.00				
H-G	0 / 0	-18.5 -18.5 0.10 (4)	10.00				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMW-w	MT20	4.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMVW-t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMVW-w	MT20	4.0	9.0		
K	BMVW-t	MT20	4.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

TOTAL WEIGHT = 2 X 91 = 182 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/999 (0.06")

CSI: TC=0.75/1.00 (F-G:1), BC=0.19/1.00 (J-K:1), WB=0.44/1.00 (E-H:1), SSI=0.20/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (H) (INPUT = 0.90)
JSI METAL= 0.50 (B) (INPUT = 1.00)



Structural component only
DWG# T-2115904

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 91 = 181 lb

DESCR.	SPF	SPF	SPF	SPF	SPF

SPF

BUILDING BEARINGS

REQRD
BRG
IN-SX
1-8
5-8

1ST LCASE MAX./MIN. COMPONENT REACTIONS

DEAD	SOIL
261 / 0	0 / 0
277 / 0	0 / 0

L. K

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.62 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CE

ALLY RESTRAINED.

ALLY RESTRAINED

ED IN

TOTAL LOAD CASES: (4)

W E B S

SPECIFIED LOADS:

BOT	CH.	LL	=	0.0	PSF
		DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF	

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OF

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

ALLOWABLE DEFL.(LL)= $L/360$ (0.65")

CSI: TC=0.69/1.00 (E-F:1), BC=0.24/1.00 (H-J:4)
WR=0.64/1.00 (E-H:1), SSI=0.29/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

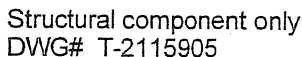
COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURED TO ORDER

NAIL VALUES
PLATE CRIFORM SHEAR SECTION

PLATE PLACEMENT TOL. = 0.250 inches

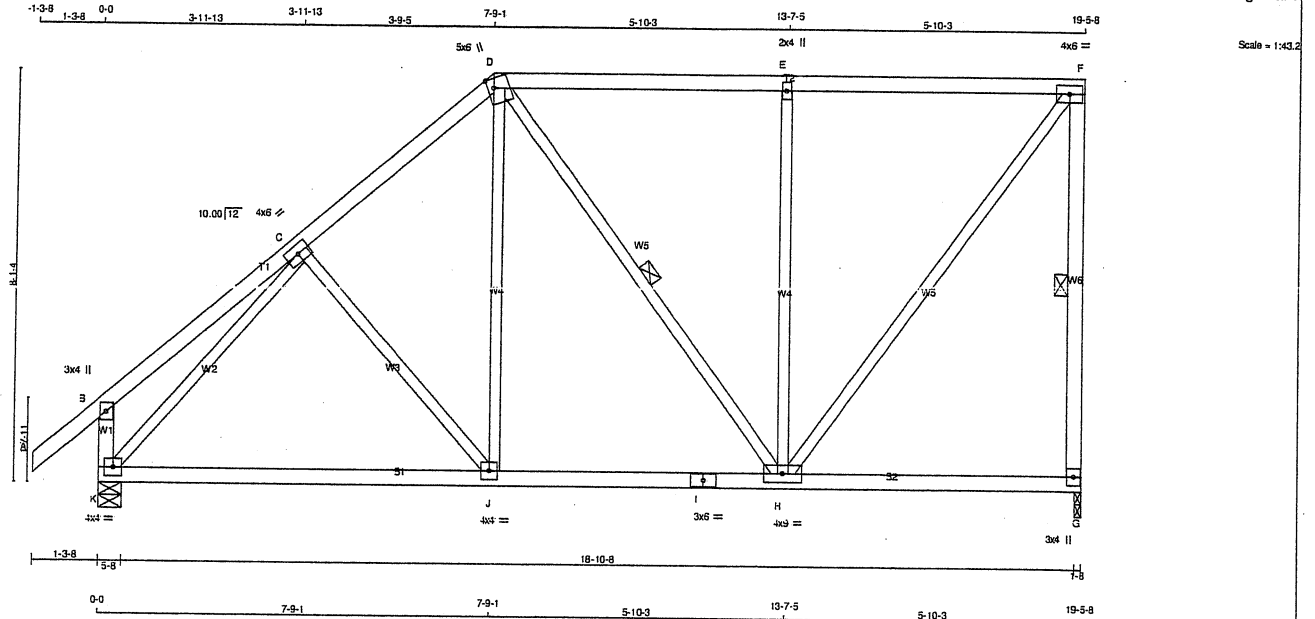
PLATE ROTATION TOL. = 5.0 Deg.



Structural component only
DWG# T-2115905

JOB NAME 412982	TRUSS NAME T5	QUANTITY 8	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:wTM7iolSBYniqocTeDgf3Pyb9F8-JvkzSeeckN3sulk?mN3dt5ukTzGtKr2AATbbkgzCalO



TOTAL WEIGHT = 8 X 96 = 768 lb [M]

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4 DRY	No.2	SPF		
D - F	2x4 DRY	No.2	SPF		
G - F	2x4 DRY	No.2	SPF		
K - B	2x4 DRY	No.2	SPF		
K - I	2x4 DRY	No.2	SPF		
I - G	2x4 DRY	No.2	SPF		

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	BRG
G	1073	0	1073	0	0	0	1-8	1-8	
K	1200	0	1200	0	0	0	5-8	5-8	

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
G	759	498 / 0	0 / 0	0 / 0	0 / 0	261 / 0	0 / 0
K	846	569 / 0	0 / 0	0 / 0	0 / 0	277 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.24 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 41	-91.8 -91.8	0.13 (1)	10.00	C-J	-144 / 0	0.09 (1)
B-C	0 / 26	-91.8 -91.8	0.22 (1)	10.00	J-D	0 / 255	0.06 (1)
C-D	-878 / 0	-91.8 -91.8	0.24 (1)	6.24	D-H	-62 / 0	0.04 (1)
D-E	-618 / 0	-91.8 -91.8	0.55 (1)	6.25	H-E	-665 / 0	0.84 (1)
E-F	-619 / 0	-91.8 -91.8	0.55 (1)	6.25	H-F	0 / 1027	0.23 (1)
G-F	-1026 / 0	0.0 0.0	0.30 (1)	6.20	K-C	-1153 / 0	0.68 (1)
K-B	-262 / 0	0.0 0.0	0.03 (1)	7.81			
K-J	0 / 748	-18.5 -18.5	0.29 (4)	10.00			
J-I	0 / 656	-18.5 -18.5	0.30 (4)	10.00			
I-H	0 / 656	-18.5 -18.5	0.30 (4)	10.00			
H-G	0 / 0	-18.5 -18.5	0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL)= L/999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL)= L/999 (0.12")

CSI: TC=0.55/1.00 (E-F:1), BC=0.30/1.00 (H-J:4), WB=0.84/1.00 (E-H:1), SSI=0.26/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

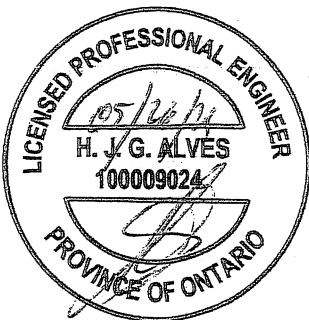
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

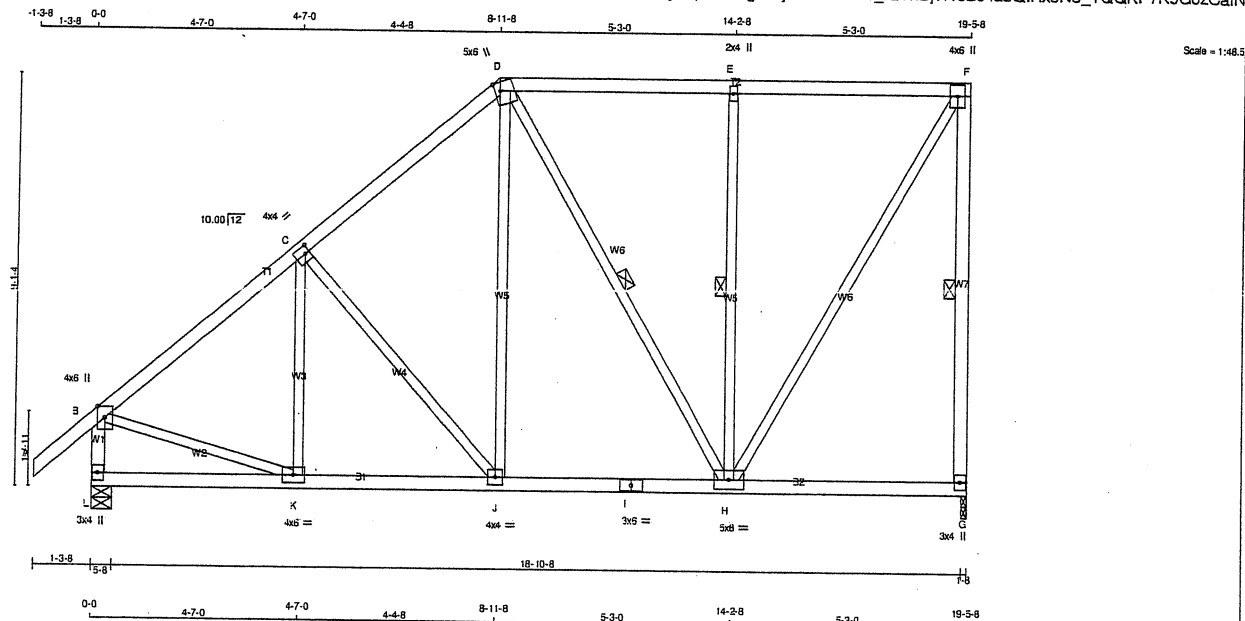
JSI GRIP= 0.88 (K) (INPUT = 0.90)
JSI METAL= 0.26 (C) (INPUT = 1.00)



Structural component only
DWG# T-2115906

JOB NAME 412982	TRUSS NAME T6	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:wTM7iolSBYniqocTeDgf3Pyb9F8-n5ILJ_fEVhBjWwJB4asQIRx3Ne_TQKPK7K9G6zCaiN



TOTAL WEIGHT = 2 X 103 = 207 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
L - B	2x4	DRY	No.2	SPF	
L - I	2x4	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
G	1073	0	1073	0
L	1200	0	1200	0

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	759	498 / 0	0 / 0	0 / 0	0 / 0	251 / 0	0 / 0
L	846	569 / 0	0 / 0	0 / 0	0 / 0	277 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.83 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 41	-91.8 -91.8	0.13 (1)	10.00	K-C	-132 / 29	0.06 (1)
B-C	-988 / 0	-91.8 -91.8	0.34 (1)	5.83	C-J	-293 / 0	0.25 (1)
C-D	-803 / 0	-91.8 -91.8	0.33 (1)	6.25	J-D	0 / 305	0.07 (1)
D-E	-509 / 0	-91.8 -91.8	0.43 (1)	6.25	D-H	-166 / 0	0.13 (1)
E-F	-509 / 0	-91.8 -91.8	0.43 (1)	6.25	H-E	-596 / 0	0.32 (1)
G-F	-1033 / 0	0.0 0.0	0.40 (1)	6.18	H-F	0 / 988	0.22 (1)
L-B	-1164 / 0	0.0 0.0	0.12 (1)	7.39	B-K	0 / 814	0.18 (1)
L-K	0 / 0	-18.5 -18.5	0.09 (4)	10.00			
K-J	0 / 783	-18.5 -18.5	0.17 (1)	10.00			
J-I	0 / 594	-18.5 -18.5	0.18 (4)	10.00			
I-H	0 / 594	-18.5 -18.5	0.18 (4)	10.00			
H-G	0 / 0	-18.5 -18.5	0.12 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN/C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.43/1.00 (E-F:1), BC=0.18/1.00 (H-J:4), WB=0.32/1.00 (E-H:1), SSI=0.23/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.77 (B) (INPUT = 0.90)
JSI METAL = 0.50 (B) (INPUT = 1.00)

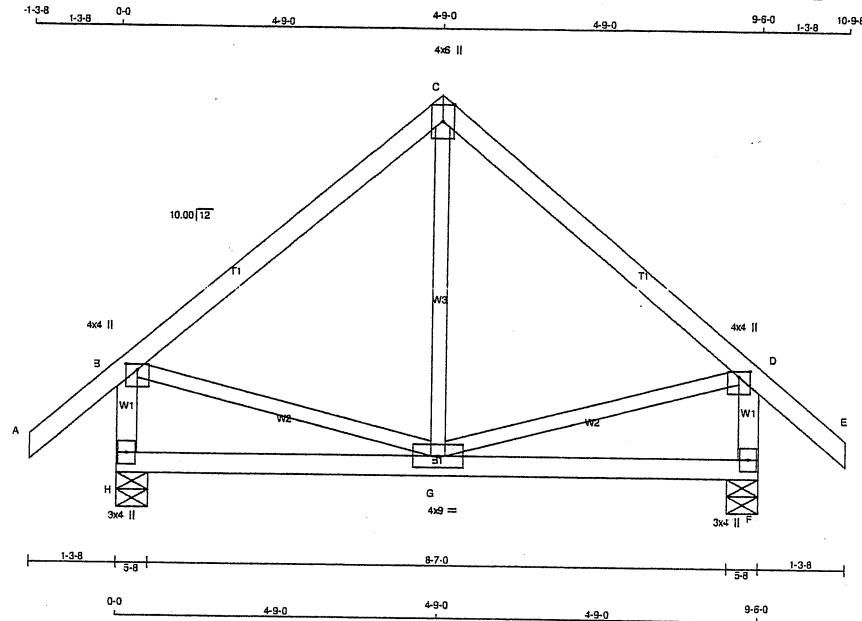


Structural component only
DWG# T-2115907

JOB NAME 412982	TRUSS NAME T8	QUANTITY 3	PLY 1	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1/32"

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4 DRY	No.2	SPF		
C - E	2x4 DRY	No.2	SPF		
H - B	2x4 DRY	No.2	SPF		
F - D	2x4 DRY	No.2	SPF		
H - F	2x4 DRY	No.2	SPF		
ALL WEBS EXCEPT	2x3 DRY	No.2	SPF		

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMVWW-t	MT20	4.0	9.0		
H	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
H	651	0	651	0
F	651	0	651	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
H	COMBINED	SNOW	LIVE	PERM. LIVE	
H	458	314 / 0	0 / 0	0 / 0	144 / 0
F	458	314 / 0	0 / 0	0 / 0	144 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 41	-91.8 -91.8	0.13 (1)	10.00	G-C	-27 / 72	0.02 (4)
B-C	-319 / 0	-91.8 -91.8	0.27 (1)	6.25	B-G	0 / 254	0.06 (1)
C-D	-319 / 0	-91.8 -91.8	0.27 (1)	6.25	G-D	0 / 254	0.06 (1)
D-E	0 / 41	-91.8 -91.8	0.13 (1)	10.00			
H-B	-617 / 0	0.0 0.0	0.07 (1)	7.81			
F-D	-617 / 0	0.0 0.0	0.07 (1)	7.81			
H-G	0 / 0	-18.5 -18.5	0.12 (4)	10.00			
G-F	0 / 0	-18.5 -18.5	0.12 (4)	10.00			

TOTAL WEIGHT = 3 X 43 = 130 lb [M/F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 8.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.32")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.32")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.27/1.00 (C-D:1), BC=0.12/1.00 (G-H:4),
WB=0.06/1.00 (B-G:1), SSI=0.13/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

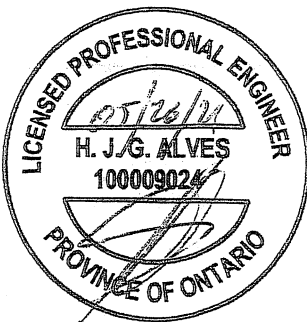
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.50 (B) (INPUT = 0.90)
JSI METAL= 0.14 (D) (INPUT = 1.00)

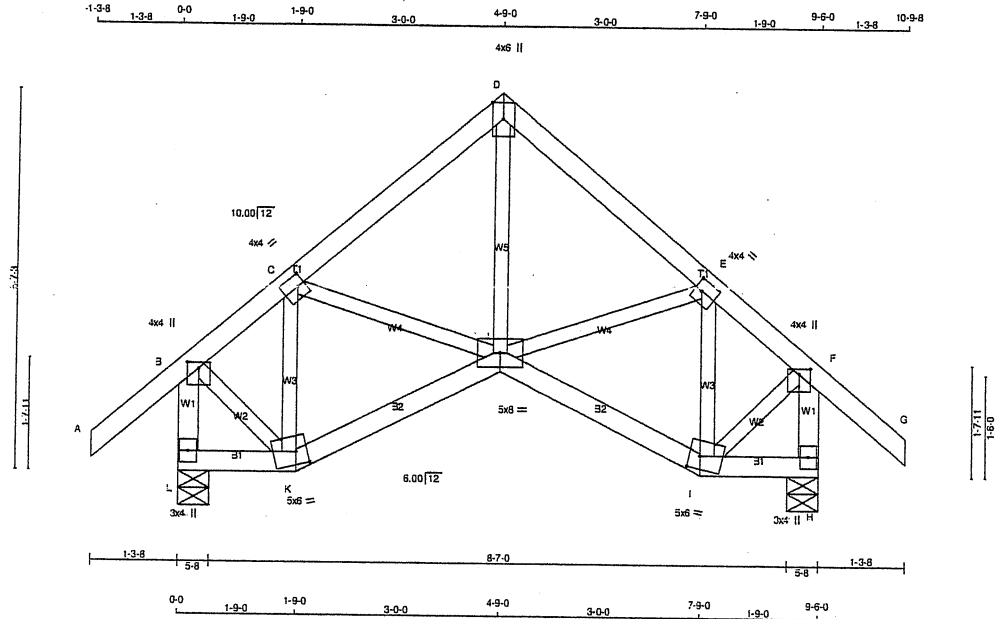


Structural component only
DWG# T-2115908

JOB NAME 412982	TRUSS NAME T9S	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1:32.2

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
L - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
L - K	2x4	DRY	No.2
K - J	2x4	DRY	No.2
J - I	2x4	DRY	No.2
I - H	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT
SPF
DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
		GROSS REACTION		GROSS REACTION		BRG		BRG	
JT		VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
L	651	0	651	0	0	5-8	5-8	5-8	
H	651	0	651	0	0	5-8	5-8	5-8	

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	458	314 / 0	0 / 0	0 / 0	0 / 0	144 / 0	0 / 0
H	458	314 / 0	0 / 0	0 / 0	0 / 0	144 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MAX. FACTORED				MAX. FACTORED			
MEMB.	FORCE (LBS)	VERT. LOAD (PL)	CS1 (LC)	MEMB.	FORCE (LBS)	VERT. LOAD (PL)	CS1 (LC)
FR-TO		FROM TO		FR-TO		FROM TO	
A-B	0 / 41	-91.8	-91.8 0.13 (1)	10.00	J-D	0 / 268	0.06 (1)
B-C	-354 / 0	-91.8	-91.8 0.09 (1)	6.25	J-E	0 / 59	0.01 (1)
C-D	-414 / 0	-91.8	-91.8 0.11 (1)	6.25	I-E	-279 / 0	0.05 (1)
D-E	-414 / 0	-91.8	-91.8 0.11 (1)	6.25	C-J	0 / 59	0.01 (1)
E-F	-354 / 0	-91.8	-91.8 0.09 (1)	6.25	K-C	-279 / 0	0.05 (1)
F-G	0 / 41	-91.8	-91.8 0.13 (1)	10.00	B-K	0 / 328	0.07 (1)
L-B	-635 / 0	0.0	0.0 0.07 (1)	7.81	I-F	0 / 328	0.07 (1)
H-F	-635 / 0	0.0	0.0 0.07 (1)	7.81			
L-K	0 / 0	-18.5	-18.5 0.02 (4)	10.00			
K-J	0 / 283	-18.5	-18.5 0.07 (1)	10.00			
J-I	0 / 283	-18.5	-18.5 0.07 (1)	10.00			
I-H	0 / 0	-18.5	-18.5 0.02 (4)	10.00			

TOTAL WEIGHT = 2 X 48 = 95 lb [M/F]

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.32")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.32")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.13/1.00 (A-B:1), BC=0.07/1.00 (I-J:1),
WB=0.07/1.00 (F-I:1), SS=0.10/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.44 (F) (INPUT = 0.90)
JSI METAL = 0.13 (B) (INPUT = 1.00)

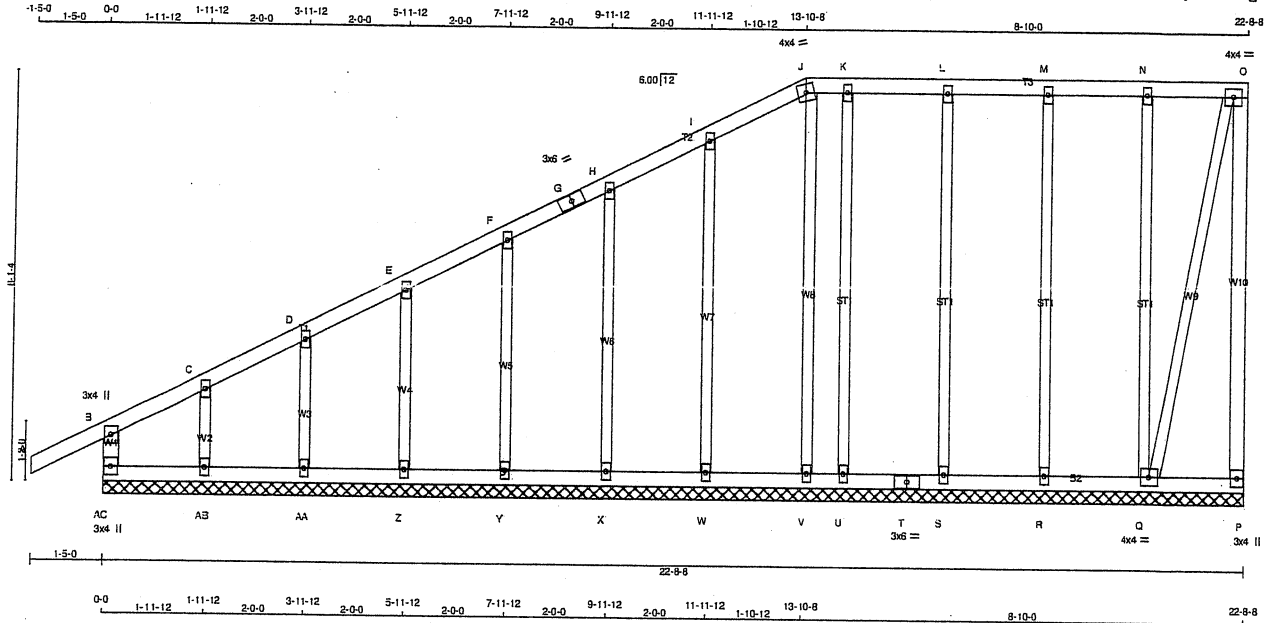


Structural component only
DWG# T-2115909

JOB NAME 412982	TRUSS NAME T10G	QUANTITY 4	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

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LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
AC- B	2x4	DRY	No.2 SPF
A - G	2x4	DRY	No.2 SPF
G - J	2x4	DRY	No.2 SPF
J - O	2x4	DRY	No.2 SPF
P - O	2x4	DRY	No.2 SPF
AC- T	2x4	DRY	No.2 SPF
T - P	2x4	DRY	No.2 SPF
ALL WEBS	2x3	DRY	No.2 SPF
ALL GABLE WEBS	2x3	DRY	No.2 SPF
DRY: SEASONED LUMBER.			
GABLE STUDS SPACED AT 2'-0-0 OC.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS (LC)	
FR-TO		FROM TO		FR-TO			
AC-B	-271 / 0	0.0	0.0 0.03 (1)	Q-N	-200 / 0	0.25 (1)	
A-B	0 / 30	-91.8	-91.8 0.14 (1)	R-M	-182 / 0	0.23 (1)	
B-C	-21 / 0	-91.8	-91.8 0.14 (1)	S-L	-190 / 0	0.24 (1)	
C-D	0 / 10	-91.8	-91.8 0.05 (1)	U-K	-149 / 0	0.19 (1)	
D-E	0 / 9	-91.8	-91.8 0.05 (1)	V-J	-95 / 0	0.12 (1)	
E-F	0 / 14	-91.8	-91.8 0.04 (1)	AB-C	-118 / 0	0.02 (1)	
F-G	0 / 16	-91.8	-91.8 0.04 (1)	AA-D	-196 / 0	0.04 (1)	
G-H	0 / 16	-91.8	-91.8 0.04 (1)	Z-E	-179 / 0	0.05 (1)	
H-I	0 / 19	-91.8	-91.8 0.05 (1)	Y-F	-183 / 0	0.07 (1)	
I-J	0 / 13	-91.8	-91.8 0.05 (1)	X-H	-180 / 0	0.11 (1)	
J-K	0 / 20	-91.8	-91.8 0.03 (1)	W-I	-200 / 0	0.18 (1)	
K-L	0 / 20	-91.8	-91.8 0.05 (1)	Q-O	-79 / 0	0.11 (1)	
L-M	0 / 20	-91.8	-91.8 0.05 (1)				
M-N	0 / 20	-91.8	-91.8 0.05 (1)				
N-O	0 / 20	-91.8	-91.8 0.05 (1)				
P-O	0 / 0	0.0	0.0 0.00 (1)				
AC-AB	0 / 0	-18.5	-18.5 0.02 (4)				
AB-AA	-4 / 0	-18.5	-18.5 0.02 (4)				
AA-Z	-9 / 0	-18.5	-18.5 0.01 (4)				
Z-Y	-12 / 0	-18.5	-18.5 0.01 (4)				
Y-X	-14 / 0	-18.5	-18.5 0.01 (4)				
X-W	-16 / 0	-18.5	-18.5 0.02 (4)				
W-V	-18 / 0	-18.5	-18.5 0.02 (4)				
V-U	-20 / 0	-18.5	-18.5 0.01 (4)				
U-T	-20 / 0	-18.5	-18.5 0.02 (4)				
T-S	-20 / 0	-18.5	-18.5 0.02 (4)				
S-R	-20 / 0	-18.5	-18.5 0.02 (4)				
R-Q	-20 / 0	-18.5	-18.5 0.02 (4)				
Q-P	0 / 0	-18.5	-18.5 0.02 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (A-B:1), BC=0.02/1.00 (Q-R:4), WB=0.25/1.00 (N-Q:1), SS=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

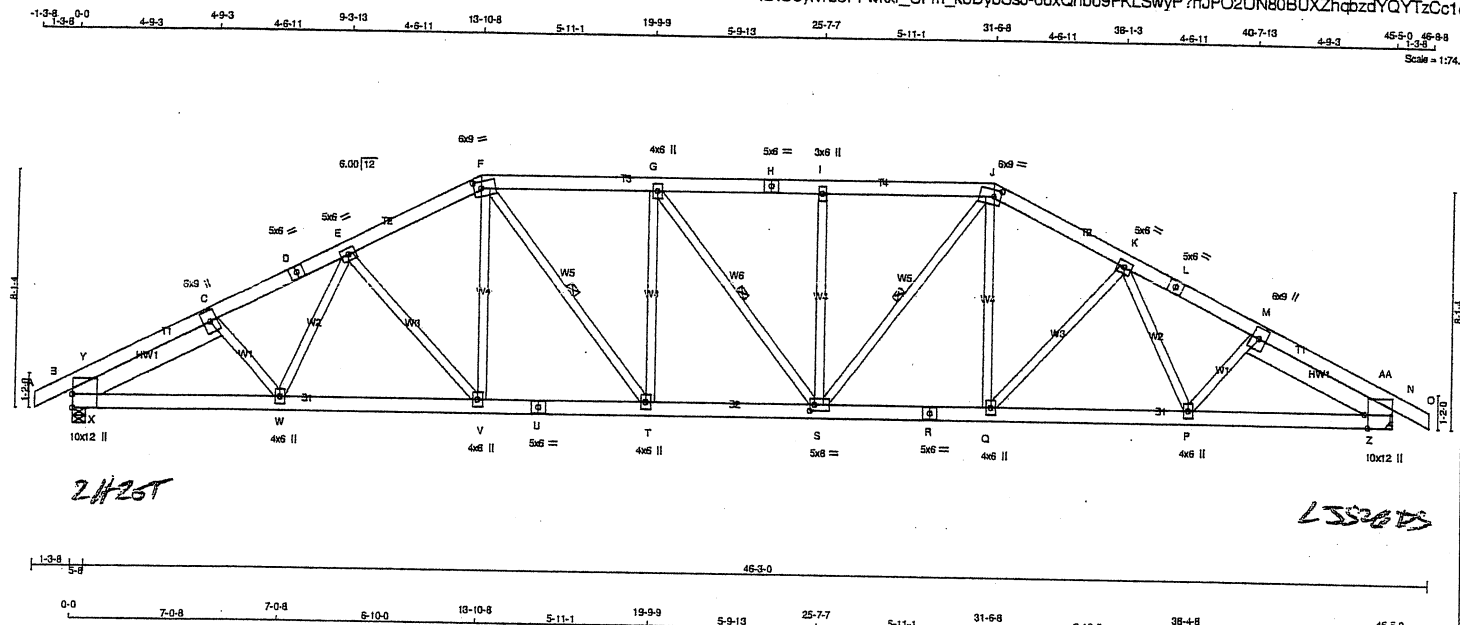
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.59 (J) (INPUT = 0.90)
JSI METAL= 0.08 (I) (INPUT = 1.00)



Structural component only
DWG# T-2115910

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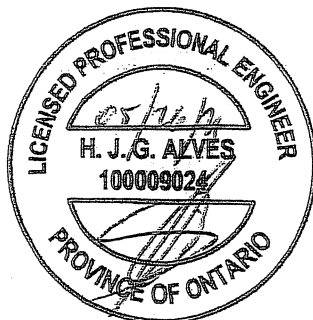
LUMBER				
N. L. G. A. RULES				
CHORDS		SIZE	LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
J - L	2x6	DRY	No.2	SPF
L - O	2x6	DRY	No.2	SPF
O - B	2x6	DRY	No.2	SPF
U - R	2x6	DRY	No.2	SPF
R - N	2x6	DRY	No.2	SPF

REINFORCING MEMBERS				
HW1	2x8	DRY	No.2	SPF
HW2	2x8	DRY	No.2	SPF

ALL WEBS 2x4 DRY No.2 SPF
 DRY: SEASONED LUMBER.

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMW+1	MT20	10.0	12.0	5.50	
C	TMMW+1	MT20	6.0	9.0		
D, H, L						
D	TS-t	MT20	5.0	6.0		
E	TMMW-t	MT20	5.0	6.0		
F	TTWW-m	MT20	6.0	9.0	2.75	3.00
G	TMMW-t	MT20	4.0	6.0		
I	TMMW-t	MT20	3.0	6.0		
J	TTWW-m	MT20	6.0	9.0	2.75	3.00
K	TMMW-t	MT20	5.0	6.0		
M	TMMW-t	MT20	6.0	9.0		
N	TMBMW+1	MT20	10.0	12.0	Edge	1.50
P, Q, T, V, W						
P	BMWW-t	MT20	4.0	6.0		
R	BS-t	MT20	5.0	6.0		
S	TMMWW-t	MT20	5.0	8.0	2.50	2.00
U	BS-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.



Structural component only
DWG# T-2115850 *1/2*

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BUILDING BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT	REQ'D
	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG IN-SX	BRG IN-SX
B	3675	0	3709	195	-1082	5-8	5-8
N	3675	0	3709	0	-1082	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT N. MINIMUM BEARING LENGTH AT JOINT N = 3-8.

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 1082 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT N FOR 1082 LBS FACTORED UPLIFT

PROVIDE FOR 195 LBS FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	2713	1634 / 0	477 / 0	0 / 0	0 / -1073	625 / 0	0 / 0
N	2713	1634 / 0	477 / 0	0 / 0	59 / -1073	625 / 0	0 / 0

HORIZONTAL REACTIONS

B	—	0 / 0	0 / 0	0 / 0	139 / -139	0 / 0	0 / 0
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BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 3.53 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-T, G-S, J-S.

LOADING

TOTAL LOAD CASES: (18)

C H O R D S				W E B S				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM TO			FR-TO			
A-B	0 / 1	-115.2	-115.2	0.09 (2)	10.00	C-W	-30 / 380	0.06 (3)
B-Y	-4270 / 1161	-115.2	-115.2	0.18 (2)	4.08	W-E	-108 / 274	0.04 (5)
C-D	-2519 / 902	-115.2	-115.2	0.15 (2)	4.88	E-V	-825 / 384	0.54 (2)
D-E	-5532 / 1713	-115.2	-115.2	0.33 (2)	3.53	V-F	-234 / 885	0.18 (13)
E-F	-5532 / 1713	-115.2	-115.2	0.33 (2)	3.53	F-T	-482 / 1515	0.24 (3)
F-G	-4993 / 1604	-115.2	-115.2	0.29 (2)	3.73	T-G	-977 / 432	0.78 (3)
G-H	-5190 / 1888	-115.2	-115.2	0.41 (1)	3.53	G-S	-326 / 310	0.14 (2)
H-I	-5185 / 1886	-115.2	-115.2	0.41 (1)	3.53	S-I	-721 / 361	0.58 (2)
I-J	-5185 / 1886	-115.2	-115.2	0.41 (1)	3.53	S-J	-477 / 1506	0.24 (2)
J-K	-5185 / 1885	-115.2	-115.2	0.41 (1)	3.55	Q-J	-237 / 890	0.19 (14)
K-L	-4994 / 1605	-115.2	-115.2	0.29 (3)	3.73	K-K	-824 / 384	0.54 (3)
L-L	-5532 / 1713	-115.2	-115.2	0.33 (3)	3.53	Q-P	-120 / 274	0.05 (9)
M-M	-5532 / 1713	-115.2	-115.2	0.33 (3)	3.53	P-M	-24 / 380	0.06 (2)
MA-AA	-2820 / 904	-115.2	-115.2	0.15 (3)	4.88	X-Y	-385 / 1912	0.00 (1)
AA-N	-4270 / 1164	-115.2	-115.2	0.18 (3)	4.08	X-C	-2971 / 828	0.50 (1)
N-O	0 / 1	-115.2	-115.2	0.09 (3)	10.00	M-Z	-2970 / 827	0.50 (1)
						Z-AA	-387 / 1912	0.00 (1)
B-X	-866 / 2518	-39.5	-39.5	0.30 (2)	6.25			
X-W	-1526 / 4875	-39.5	-39.5	0.68 (1)	6.25			
W-V	-1438 / 4893	-39.5	-39.5	0.67 (1)	6.25			
V-U	-1189 / 4457	-39.5	-39.5	0.62 (1)	6.25			
U-T	-1189 / 4457	-39.5	-39.5	0.62 (1)	6.25			
T-S	-1376 / 5190	-39.5	-39.5	0.68 (1)	6.25			
S-R	-1028 / 4458	-39.5	-39.5	0.62 (1)	6.25			
R-Q	-1028 / 4458	-39.5	-39.5	0.62 (1)	6.25			
Q-P	-1244 / 4893	-39.5	-39.5	0.67 (1)	6.25			
P-Z	-1333 / 4874	-39.5	-39.5	0.68 (1)	6.25			
Z-N	-673 / 2518	-39.5	-39.5	0.30 (3)	6.25			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	33.4	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	10.5	PSF
		DL =	7.4	PSF
TOTAL LOAD		=	57.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 2.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL
OR INDUSTRIAL BUILDING REQUIREMENTS OF
PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 4 OF BCBC 2018 , ABC 2019
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR NOT USED
- PERCENTAGE OF GROUND SNOW LOAD IS
USER-DEFINED.

(80 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 33.4 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL(LL)= L/360 (1.51")
CALCULATED VERT. DEFL(LL) = L/ 999 (0.23")
ALLOWABLE DEFL(TL)= L/180 (3.03")
CALCULATED VERT. DEFL(TL) = L/ 999 (0.32")

CSI: TC=0.41/1.00 (F-G:1), BC=0.68/1.00 (P-Z:1),
WB=0.78/1.00 (G-T:3), SSI=0.24/1.00 (F-G:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY)		SHEAR		SECTION
	(PSI)		(PLI)		(PLI)
	MAX	MIN	MAX	MIN	MAX
MT20	650	371	1747	788	1987

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (F) (INPUT = 0.90)
JSI METAL= 0.86 (C) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME 413226	TRUSS NAME T11	QUANTITY 10	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.8.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.2) PSF AT (31-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCq, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.

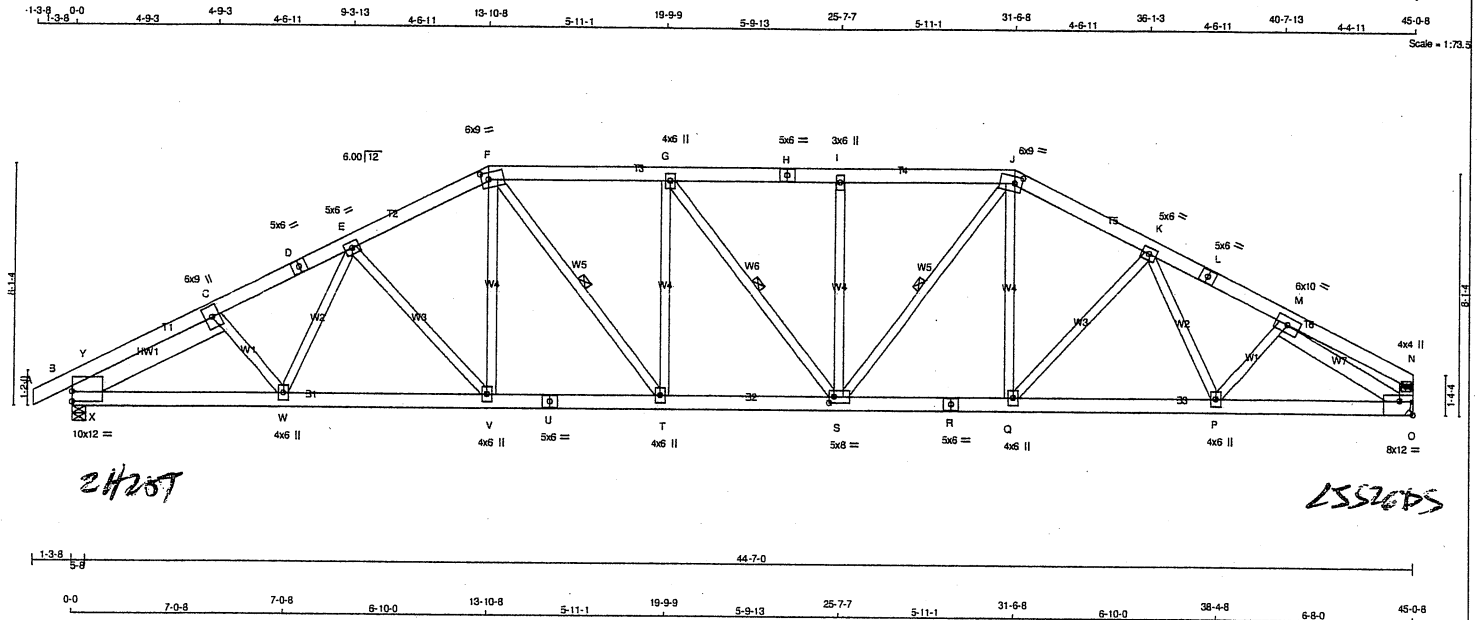


Structural component only
DWG# T-2115850 *2/2*

JOB NAME 413226	TRUSS NAME T11B	QUANTITY 4	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x6	DRY	No.2
D - F	2x6	DRY	No.2
F - H	2x6	DRY	No.2
H - J	2x6	DRY	No.2
J - L	2x6	DRY	No.2
L - N	2x6	DRY	No.2
O - N	2x6	DRY	No.2
B - U	2x6	DRY	No.2
U - R	2x6	DRY	No.2
R - O	2x6	DRY	No.2
REINFORCING MEMBERS			
HW1	2x8	DRY	No.2
ALL WEBS			
EXCEPT	2x4	DRY	No.2
M - O	2x6	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMBMW-t	MT20	10.0	12.0	4.00
C	TMBMW-t	MT20	6.0	9.0	
D, H, L					
D	TS-t	MT20	5.0	6.0	
E	TMBMW-t	MT20	5.0	6.0	
F	TTWW-t	MT20	6.0	9.0	2.75 3.00
G	TMBMW-t	MT20	4.0	6.0	
I	TMBW-w	MT20	3.0	6.0	
J	TTWW-t	MT20	6.0	9.0	2.75 3.00
K	TMBW-t	MT20	5.0	6.0	
M	TMBW-t	MT20	6.0	10.0	
N	TMBW-p	MT20	4.0	4.0	
O	BMVW-t	MT20	8.0	12.0	5.50 Edge
P, Q, T, V, W					
P	BMVW-t	MT20	4.0	6.0	
R	BS-t	MT20	5.0	6.0	
S	BMVW-t	MT20	5.0	8.0	2.50 2.00
U	BS-t	MT20	5.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
B	3646	0	3690	196	-1065	5-8	
O	3485	0	3487	0	-1004	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT O. MINIMUM BEARING LENGTH AT JOINT O = 3-8.

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 1065 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT O FOR 1004 LBS. FACTORED UPLIFT

PROVIDE FOR 196 LBS. FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	2692	1628 / 0	473 / 0	0 / 0	0 / -1059	620 / 0	0 / 0
O	2582	1506 / 0	473 / 0	0 / 0	65 / -1007	604 / 0	0 / 0

HORIZONTAL REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	—	0 / 0	0 / 0	0 / 0	140 / -120	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 3.55 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-T, G-S, J-S.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (18)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 1	-115.2 -115.2	0.09 (2)	10.00	C-W	-29 / 374	0.06 (3)
B-Y	-4246 / 1142	-115.2 -115.2	0.18 (2)	4.09	W-E	-104 / 278	0.04 (5)
Y-C	-2804 / 889	-115.2 -115.2	0.15 (2)	4.89	E-V	-827 / 382	0.54 (2)
C-D	-5478 / 1685	-115.2 -115.2	0.33 (2)	3.55	V-F	-232 / 887	0.18 (13)
D-E	-5478 / 1685	-115.2 -115.2	0.33 (2)	3.55	F-T	-468 / 1479	0.24 (3)
E-F	-4934 / 1576	-115.2 -115.2	0.29 (2)	3.75	T-G	-948 / 420	0.76 (3)
F-G	-5115 / 1656	-115.2 -115.2	0.41 (1)	3.56	G-S	-350 / 274	0.16 (2)
G-H	-5088 / 1642	-115.2 -115.2	0.41 (1)	3.57	S-I	-722 / 358	0.58 (2)
H-I	-5088 / 1642	-115.2 -115.2	0.41 (1)	3.57	S-J	-480 / 1532	0.25 (2)
I-J	-5088 / 1641	-115.2 -115.2	0.41 (1)	3.58	Q-J	-210 / 803	0.17 (14)
J-K	-4862 / 1552	-115.2 -115.2	0.29 (3)	3.78	K-Q	-707 / 342	0.47 (3)
K-L	-5236 / 1603	-115.2 -115.2	0.32 (3)	3.64	K-P	-220 / 176	0.08 (9)
L-M	-5236 / 1603	-115.2 -115.2	0.32 (3)	3.64	P-M	-22 / 556	0.09 (2)
M-N	-16 / 85	-115.2 -115.2	0.16 (3)	6.25	M-O	-5457 / 1535	1.00 (1)
O-N	-253 / 139	0.0	0.02 (14)	7.81	X-Y	-377 / 1894	0.00 (1)
					X-C	-2940 / 814	0.49 (1)
B-X	-856 / 2504	-39.5	-39.5	0.30 (2)	6.25		
X-W	-1505 / 4830	-39.5	-39.5	0.67 (1)	6.25		
W-V	-1416 / 4843	-39.5	-39.5	0.66 (1)	6.25		
V-U	-1169 / 4404	-39.5	-39.5	0.61 (1)	6.25		
U-T	-1169 / 4404	-39.5	-39.5	0.61 (1)	6.25		
T-S	-1349 / 5115	-39.5	-39.5	0.67 (1)	6.25		
S-R	-993 / 4338	-39.5	-39.5	0.61 (1)	6.25		
R-Q	-993 / 4338	-39.5	-39.5	0.61 (1)	6.25		
Q-P	-1197 / 4699	-39.5	-39.5	0.65 (1)	6.25		
P-O	-1238 / 4460	-39.5	-39.5	0.64 (1)	6.25		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	33.4	PSF
DL =	6.0	PSF	
BOT CH.	LL =	10.5	PSF
DL =	7.4	PSF	
TOTAL LOAD =	57.3	PSF	

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF CBC 2015, ABC 2019
- PART 4 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- SLOPE REDUCTION FACTOR NOT USED
- PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED.

(80 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 33.4 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.50")
CALCULATED VERT. DEFL.(LL) = L/999 (0.23")
ALLOWABLE DEFL.(TL) = L/180 (3.00")
CALCULATED VERT. DEFL.(TL) = L/999 (0.32")

CSI: TC=0.41/1.00 (F-G:1), BC=0.67/1.00 (W-X:1),
WB=1.00/1.00 (M-O:1), SSI=0.24/1.00 (F-G:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

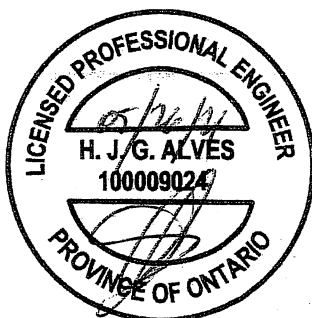
NAIL VALUES

PLATE GRIP(DRY)	PLI	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (J) (INPUT = 0.90)
JSI METAL= 0.85 (C) (INPUT = 1.00)



Structural component only
DWG# T-2115852

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
413226	T11B	4	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MITek Industries, Inc. Wed May 26 17:26:09 2021 Page 2
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TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2.(8)

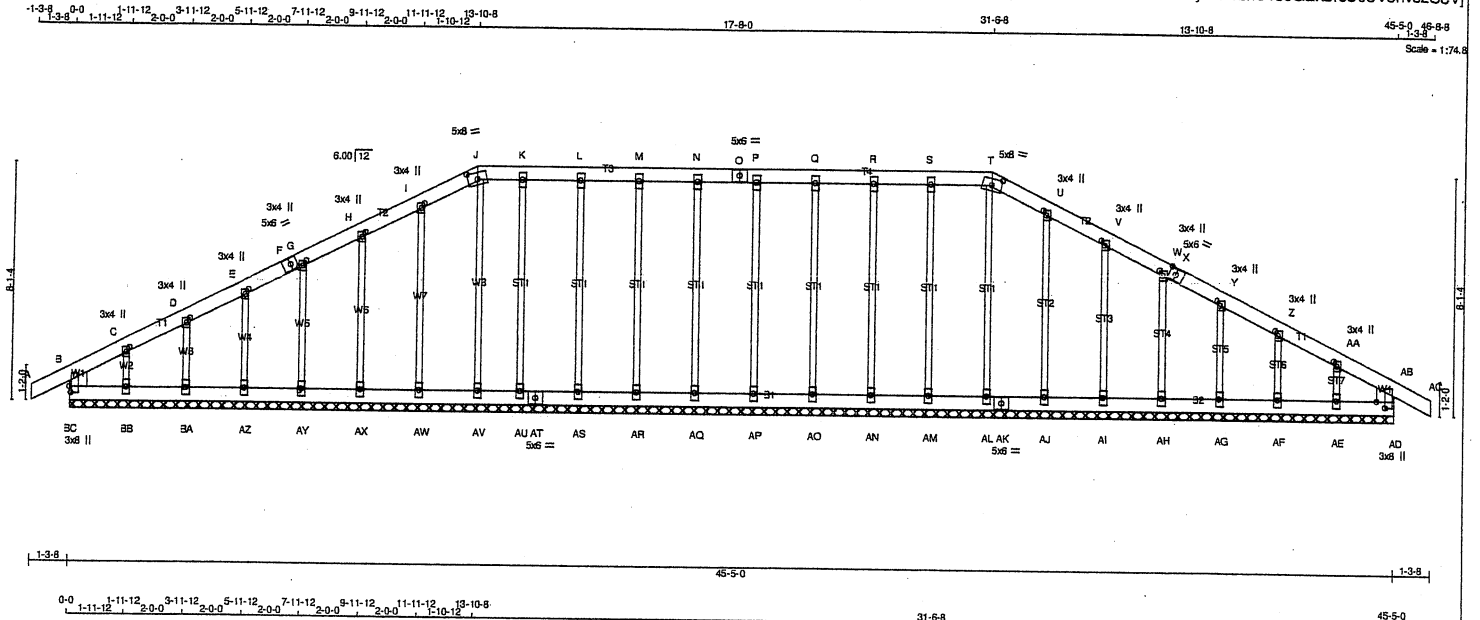
WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.2) PSF AT (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.



Structural component only
DWG# T-2115852 *W*

JOB NAME 413226	TRUSS NAME T11G	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID:U6y1?rbeFFwxf_UFm_koDybSsJ-JsrEHUeCmZKuwyfK72bn04eJG2k8re803VehvszCcVj



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
BC - B	2x8 DRY	No.2	SPF		
A - F	2x6 DRY	No.2	SPF		
F - J	2x6 DRY	No.2	SPF		
J - O	2x6 DRY	No.2	SPF		
O - X	2x6 DRY	No.2	SPF		
T - X	2x6 DRY	No.2	SPF		
X - AC	2x6 DRY	No.2	SPF		
AD - AB	2x8 DRY	No.2	SPF		
BC - AT	2x6 DRY	No.2	SPF		
AT - AK	2x6 DRY	No.2	SPF		
AK - AD	2x6 DRY	No.2	SPF		
ALL WEBS	2x3 DRY	No.2	SPF		
ALL GABLE WEBS	2x3 DRY	No.2	SPF		
DRY: SEASONED LUMBER.					

GABLE STUDS SPACED AT 2'-0" OC.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B, AB, AD, BC					
C, D, E, G, H, I, U, V, W, Y, Z, AA					
C TMW+w	MT20	3.0	4.0	2.00	1.25
F TS-t	MT20	5.0	6.0	2.50	2.75
J TTW-m	MT20	5.0	8.0	2.75	4.00
K, L, M, N, P, Q, R, S					
K TMW+w	MT20	3.0	6.0		
O TS-t	MT20	5.0	6.0		
T TTW-m	MT20	5.0	8.0	2.75	4.00
X TS-t	MT20	5.0	6.0	2.50	2.75
AD TMBMV1+p	MT20	3.0	8.0	2.50	3.75
AE, AF, AG, AH, AI, AJ, AL, AM, AN, AO, AP, AQ, AR, AS, AU, AV, AW, AX, AY, AZ, BA, BB					
AE BMW1+w	MT20	3.0	6.0		
AK BS-t	MT20	5.0	6.0		
AT BS-t	MT20	5.0	6.0		
BC TMBMV1+p	MT20	3.0	8.0	2.50	0.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSF (LC)	MEMB.	WEBS MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSF (LC)
FR-TO		FROM TO		FR-TO		
BC-B	-260 / 0	0.0	0.0 0.02 (1)	AL-T	-186 / 0	0.21 (1)
A-B	0 / 29	-91.8	-91.8 0.06 (1)	AM-S	-190 / 0	0.21 (1)
B-C	-50 / 0	-91.8	-91.8 0.05 (1)	AN-R	-183 / 0	0.21 (1)
C-D	-24 / 0	-91.8	-91.8 0.02 (1)	AO-Q	-183 / 0	0.21 (1)
D-E	-19 / 0	-91.8	-91.8 0.02 (1)	AQ-N	-183 / 0	0.21 (1)
E-F	-13 / 0	-91.8	-91.8 0.02 (1)	AR-M	-184 / 0	0.21 (1)
F-G	-13 / 0	-91.8	-91.8 0.02 (1)	AS-L	-184 / 0	0.21 (1)
G-H	-9 / 0	-91.8	-91.8 0.02 (1)	AT-K	-183 / 0	0.18 (1)
H-I	-5 / 0	-91.8	-91.8 0.02 (1)	AJ-U	-183 / 0	0.14 (1)
I-J	-3 / 0	-91.8	-91.8 0.02 (1)	AI-V	-182 / 0	0.09 (1)
J-K	0 / 0	-91.8	-91.8 0.02 (1)	AH-W	-182 / 0	0.06 (1)
K-L	0 / 0	-91.8	-91.8 0.02 (1)	AG-Y	-180 / 0	0.04 (1)
L-M	0 / 0	-91.8	-91.8 0.02 (1)	AF-Z	-188 / 0	0.03 (1)
M-N	0 / 0	-91.8	-91.8 0.02 (1)	AE-AA	-137 / 0	0.02 (1)
N-O	0 / 0	-91.8	-91.8 0.02 (1)	AV-J	-159 / 0	0.18 (1)
O-P	0 / 0	-91.8	-91.8 0.02 (1)	BB-C	-137 / 0	0.02 (1)
P-Q	0 / 0	-91.8	-91.8 0.02 (1)	BA-D	-188 / 0	0.03 (1)
Q-R	0 / 0	-91.8	-91.8 0.02 (1)	AZ-E	-180 / 0	0.04 (1)
R-S	0 / 0	-91.8	-91.8 0.02 (1)	AY-G	-181 / 0	0.06 (1)
S-T	0 / 0	-91.8	-91.8 0.02 (1)	AX-H	-183 / 0	0.10 (1)
T-U	-2 / 0	-91.8	-91.8 0.02 (1)	AW-I	-184 / 0	0.14 (1)
U-V	-5 / 0	-91.8	-91.8 0.02 (1)			
V-W	-9 / 0	-91.8	-91.8 0.02 (1)			
W-X	-14 / 0	-91.8	-91.8 0.02 (1)			
X-Y	-14 / 0	-91.8	-91.8 0.02 (1)			
Y-Z	-20 / 0	-91.8	-91.8 0.02 (1)			
Z-AA	-24 / 0	-91.8	-91.8 0.02 (1)			
AA-AB	-51 / 0	-91.8	-91.8 0.05 (1)			
AB-AC	0 / 29	-91.8	-91.8 0.06 (1)			
AD-AB	-260 / 0	0.0	0.0 0.02 (1)			

BC-BB	0 / 33	-18.5	-18.5 0.03 (1)	10.00
BB-BA	0 / 24	-18.5	-18.5 0.01 (1)	10.00
BA-AZ	0 / 17	-18.5	-18.5 0.01 (4)	10.00
AZ-AY	0 / 12	-18.5	-18.5 0.01 (4)	10.00
AY-AX	0 / 8	-18.5	-18.5 0.01 (4)	10.00
AX-AW	0 / 5	-18.5	-18.5 0.01 (4)	10.00
AW-AV	0 / 2	-18.5	-18.5 0.01 (4)	10.00
AV-AU	0 / 0	-18.5	-18.5 0.01 (4)	10.00
AU-AT	0 / 0	-18.5	-18.5 0.01 (4)	10.00
AT-AS	0 / 0	-18.5	-18.5 0.01 (4)	10.00
AS-AR	0 / 0	-18.5	-18.5 0.01 (4)	10.00
AR-AQ	0 / 0	-18.5	-18.5 0.01 (4)	10.00
AQ-AP	0 / 0	-18.5	-18.5 0.01 (4)	10.00
AP-AO	0 / 0	-18.5	-18.5 0.01 (4)	10.00
AO-AN	0 / 0	-18.5	-18.5 0.01 (4)	10.00
AN-AM	0 / 0	-18.5	-18.5 0.01 (4)	10.00
AM-AL	0 / 0	-18.5	-18.5 0.01 (4)	10.00
AL-AK	0 / 2	-18.5	-18.5 0.01 (4)	10.00
AK-AJ	0 / 2	-18.5	-18.5 0.01 (4)	10.00
AJ-AI	0 / 5	-18.5	-18.5 0.01 (4)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.
(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSF: TC=0.06/1.00 (AB-AC:1), BC=0.03/1.00 (AD-AE:1), WB=0.21/1.00 (S-AM:1), SS=0.07/1.00 (AB-AC:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (J) (INPUT = 0.90)
JSI METAL= 0.09 (AD) (INPUT = 1.00)



Structural component only
DWG# T-2115853

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
413226	T11G	2	1	TRUSS DESC.		

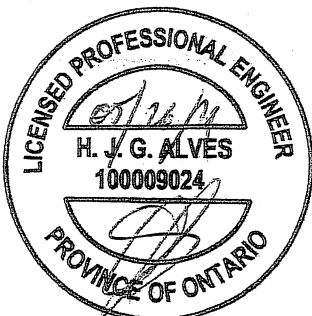
Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed May 26 14:46:40 2021 Page 2
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LOADING

TOTAL LOAD CASES: (4)

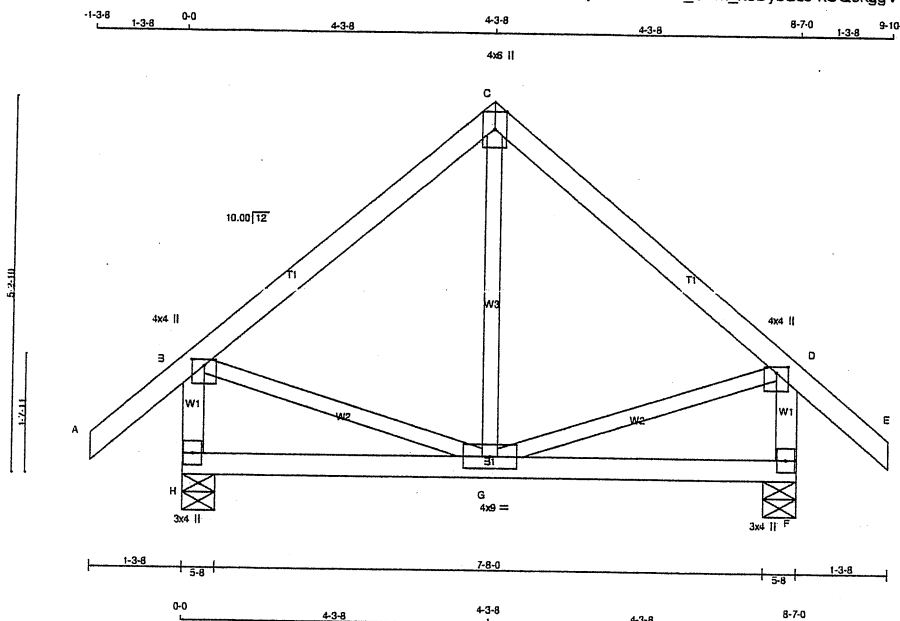
CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX CSI (LC)	MEMB.	MAX. FACTORED UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO				FR-TO			
AI-AH	0 / 8	-18.5	-18.5	0.01 (4)	10.00				
AH-AG	0 / 12	-18.5	-18.5	0.01 (4)	10.00				
AG-AF	0 / 17	-18.5	-18.5	0.01 (4)	10.00				
AF-AE	0 / 25	-18.5	-18.5	0.01 (1)	10.00				
AE-AD	0 / 33	-18.5	-18.5	0.03 (1)	10.00				



Structural component only
DWG# T-2115853

JOB NAME 412982	TRUSS NAME T13	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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Scale = 1/30.5

TOTAL WEIGHT = 40 lb [M/F]

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
H - B	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMVWW-t	MT20	4.0	9.0		
H	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
H	600	0	0	5-8
F	600	0	0	5-8

UNFACTORED REACTIONS

JT	COMBINED	1ST LCASE	SNOW/MIN. COMPONENT REACTIONS	LIVE	PERM. LIVE	WIND	DEAD	SOIL
H	422	290 / 0	0 / 0	0 / 0	0 / 0	0 / 0	132 / 0	0 / 0
F	422	290 / 0	0 / 0	0 / 0	0 / 0	0 / 0	132 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	MEMB.
FR-TO				FR-TO			
A-B	0 / 41	-91.8	-91.8 0.13 (1)	G-C	-34 / 62	0.02 (4)	
B-C	-281 / 0	-91.8	-91.8 0.22 (1)	B-G	0 / 226	0.05 (1)	
C-D	-281 / 0	-91.8	-91.8 0.22 (1)	G-D	0 / 226	0.05 (1)	
D-E	0 / 41	-91.8	-91.8 0.13 (1)				
H-B	-569 / 0	0.0	0.0 0.06 (1)				
F-D	-569 / 0	0.0	0.0 0.06 (1)				
H-G	0 / 0	-18.5	-18.5 0.10 (4)				
G-F	0 / 0	-18.5	-18.5 0.10 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.6	PSF
	DL	= 6.0	PSF
BOT CH.	LL	= 0.0	PSF
	DL	= 7.4	PSF
TOTAL LOAD		= 39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.29")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.29")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.01")

CSI: TC=0.22/1.00 (B-C:1), BC=0.10/1.00 (F-G:4), WB=0.05/1.00 (D-G:1), SS=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.46 (D) (INPUT = 0.90)
JSI METAL = 0.12 (D) (INPUT = 1.00)

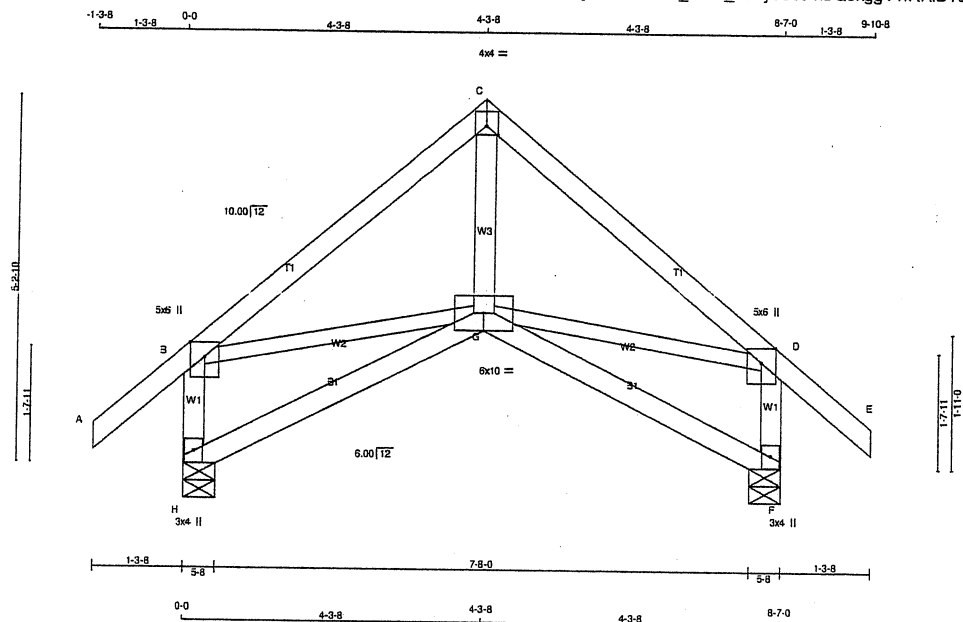


Structural component only
DWG# T-2115911

JOB NAME 412982	TRUSS NAME T13S	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:U6yi?rbeFFwxf_UFm_koDybSsJ-kUQ6kggV1IRRIDTaRVdKVjWkRAlexOicsRpFL?zCaIL



Scale = 1/31.2

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4 DRY	No.2		
C - E	2x4 DRY	No.2		
H - B	2x4 DRY	No.2		
F - D	2x4 DRY	No.2		
H - G	2x4 DRY	No.2		
G - F	2x4 DRY	No.2		

ALL WEBS	2x4 DRY	No.2
EXCEPT		
B - G	2x3 DRY	No.2
G - D	2x3 DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TMVW+p	MT20	5.0	6.0	Edge	
F	BMV1+p	MT20	3.0	4.0		
G	BBWW-p	MT20	6.0	10.0		
H	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORIZ
H	600	0	0	5-8
F	600	0	0	5-8

UNFACTORED REACTIONS

1ST CASE	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	422	290 / 0	0 / 0	0 / 0	0 / 0	132 / 0	0 / 0
F	422	290 / 0	0 / 0	0 / 0	0 / 0	132 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 41	-91.8 -91.8	0.13 (1)	10.00	G-C	0 / 180	0.03 (4)
B-C	-448 / 0	-91.8 -91.8	0.22 (1)	6.25	B-G	0 / 348	0.08 (1)
C-D	-448 / 0	-91.8 -91.8	0.22 (1)	6.25	G-D	0 / 348	0.08 (1)
D-E	0 / 41	-91.8 -91.8	0.13 (1)	10.00			
H-B	-561 / 0	0.0 0.0	0.06 (1)	7.81			
F-D	-561 / 0	0.0 0.0	0.06 (1)	7.81			
H-G	0 / 0	-18.5 -18.5	0.10 (4)	10.00			
G-F	0 / 0	-18.5 -18.5	0.10 (4)	10.00			

TOTAL WEIGHT = 2 X 41 = 82 lb [M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.29")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.29")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CS1: TC=0.22/1.00 (B-C:1), BC=0.10/1.00 (G-H:4), WB=0.08/1.00 (D-G:1), SS1=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

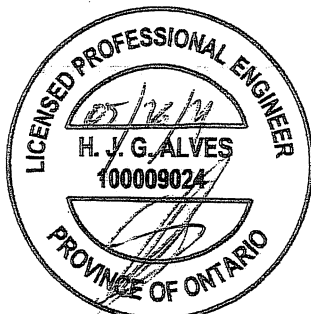
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
		788	1987

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

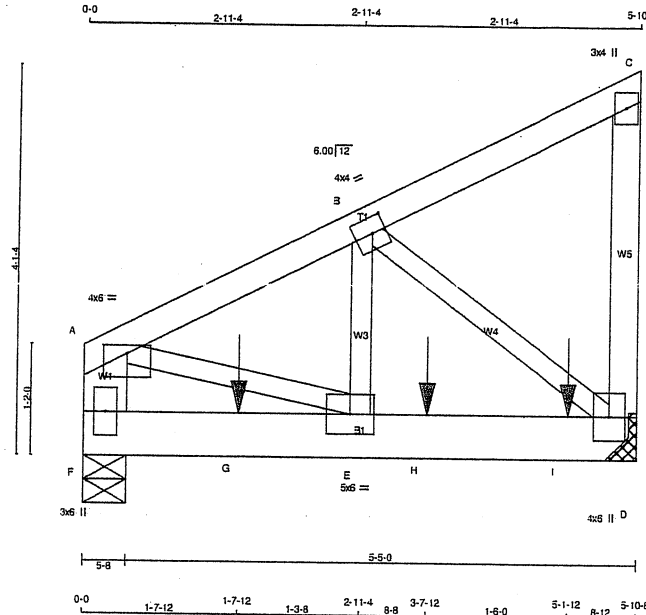
JSI GRIP= 0.60 (C) (INPUT = 0.90)
JSI METAL= 0.22 (B) (INPUT = 1.00)



Structural component only
DWG# T-2115912

JOB NAME 412982	TRUSS NAME T40	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID:PrpSyr_45RAIf3mJ8AgI49yaIqE-gfYs9MIIzWh8?XdzYwfoa8Bjo_KPH0vKIIMPtzCal



Scale = 1/2" = 1'

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x6	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C 1	12	TOP
C - D 1	12	TOP
F - A 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F - D 2	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00	3.00
B	TMVW-t	MT20	4.0	4.0	2.00	1.75
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMVW-t	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
D	1449	0	1449	0
F	1098	0	1098	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4'-0."

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
D	1018	708 / 0	0 / 0	0 / 0	0 / 0	310 / 0	0 / 0
F	771	534 / 0	0 / 0	0 / 0	0 / 0	237 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
A-B	-1143 / 0	-91.8	-91.8 0.06 (1)	E-B	0 / 984	0.12 (1)	
B-C	-11 / 0	-91.8	-91.8 0.06 (1)	B-D	-1299 / 0	0.15 (1)	
C-D	-109 / 0	0.0	0.0 0.01 (1)	A-E	0 / 1068	0.13 (1)	
F-A	-898 / 0	0.0	0.0 0.03 (1)				
F-G	0 / 0	-18.5	-18.5 0.11 (1)				
G-E	0 / 0	-18.5	-18.5 0.11 (1)				
E-H	0 / 1032	-18.5	-18.5 0.21 (1)				
H-I	0 / 1032	-18.5	-18.5 0.21 (1)				
I-D	0 / 1032	-18.5	-18.5 0.21 (1)				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-7-12	-443	-443	---	TOP	VERT	TOTAL	---	C1
H	3-7-12	-443	-443	---	TOP	VERT	TOTAL	---	C1
I	5-1-12	-445	-445	---	TOP	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 29 = 58 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.06/1.00 (A-B:1), BC=0.21/1.00 (D-E:1),
WB=0.15/1.00 (B-D:1), SSI=0.26/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

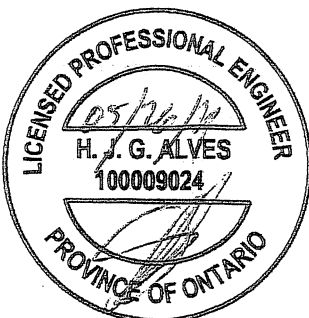
PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
			788
			1987
			1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.64 (B) (INPUT = 0.90)

JSI METAL = 0.23 (D) (INPUT = 1.00)

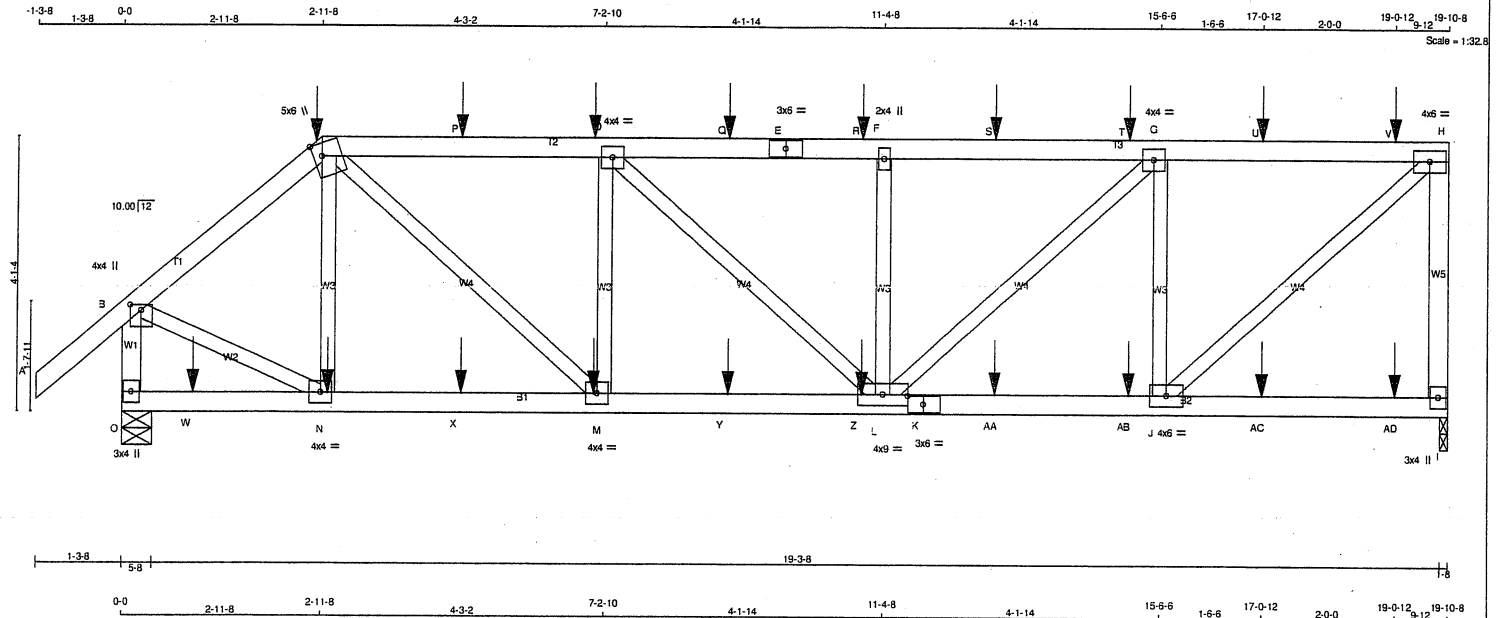


Structural component only
DWG# T-2115913

JOB NAME 412982	TRUSS NAME T52	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:PrpSyr_45RAIF3mJ8AgI49yjqE-835EMijNKDp?cgC96dA17M8rOKA8il3ZP2wyKzCall



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - H	2x4	DRY	No.2
H - I	2x4	DRY	No.2
O - B	2x4	DRY	No.2
O - K	2x4	DRY	No.2
K - I	2x4	DRY	No.2

ALL WEBS	2x3	DRY	No.2	SPF
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EXCEPT

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	1	12
C - E	1	12
E - H	1	12
H - I	1	12
O - B	1	12
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
O - K	1	12
K - I	1	12
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	1	6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
I	1827	0	1827	0	0	1-8	1-8	1-8	1-8
O	1989	0	1989	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1294	841 / 0	0 / 0	0 / 0	0 / 0	453 / 0	0 / 0
O	1405	934 / 0	0 / 0	0 / 0	0 / 0	471 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.54 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		W E B S		FACTORED	
MEMB.	MAX. FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FORCE (LBS)	MAX. CSI (LC)	MAX. CSI (LC)
FR-TO		FROM	TO	FR-TO			
A - B	0 / 41	-91.8	-91.8 0.07 (1)	10.00	N - C	-440 / 0	0.06 (1)
B - C	-1746 / 0	-91.8	-91.8 0.09 (1)	6.25	B - N	0 / 1452	0.18 (1)
C - P	-2313 / 0	-91.8	-91.8 0.21 (1)	5.65	J - H	0 / 2266	0.28 (1)
P - D	-2313 / 0	-91.8	-91.8 0.21 (1)	5.65	C - M	0 / 1324	0.16 (1)
D - Q	-2382 / 0	-91.8	-91.8 0.21 (1)	5.58	J - G	-1341 / 0	0.18 (1)
Q - E	-2382 / 0	-91.8	-91.8 0.21 (1)	5.58	M - D	-748 / 0	0.10 (1)
E - R	-2382 / 0	-91.8	-91.8 0.21 (1)	5.58	L - G	0 / 921	0.11 (1)
R - F	-2382 / 0	-91.8	-91.8 0.21 (1)	5.58	D - L	0 / 94	0.01 (1)
F - S	-2382 / 0	-91.8	-91.8 0.24 (1)	5.54	L - F	-563 / 0	0.08 (1)
S - T	-2382 / 0	-91.8	-91.8 0.24 (1)	5.54			
T - G	-2382 / 0	-91.8	-91.8 0.24 (1)	5.54			
G - U	-1703 / 0	-91.8	-91.8 0.22 (1)	6.25			
U - V	-1703 / 0	-91.8	-91.8 0.22 (1)	6.25			
V - H	-1703 / 0	-91.8	-91.8 0.22 (1)	6.25			
I - H	-1766 / 0	0.0	0.0 0.24 (1)	7.81			
O - B	-1956 / 0	0.0	0.0 0.11 (1)	7.81			

O - W	0 / 0	-18.5	-18.5 0.05 (4)	10.00
W - N	0 / 0	-18.5	-18.5 0.05 (4)	10.00
N - X	0 / 1324	-18.5	-18.5 0.14 (1)	10.00
X - M	0 / 1324	-18.5	-18.5 0.14 (1)	10.00
M - Y	0 / 2313	-18.5	-18.5 0.24 (1)	10.00
Y - Z	0 / 2313	-18.5	-18.5 0.24 (1)	10.00
Z - L	0 / 2313	-18.5	-18.5 0.24 (1)	10.00
L - K	0 / 1703	-18.5	-18.5 0.19 (1)	10.00
K - AA	0 / 1703	-18.5	-18.5 0.19 (1)	10.00
AA - AB	0 / 1703	-18.5	-18.5 0.19 (1)	10.00
AB - J	0 / 1703	-18.5	-18.5 0.19 (1)	10.00
J - AC	0 / 0	-18.5	-18.5 0.07 (4)	10.00
AC - AD	0 / 0	-18.5	-18.5 0.07 (4)	10.00
AD - I	0 / 0	-18.5	-18.5 0.07 (4)	10.00

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-11-8	-25	-25	---	FRONT	VERT	DEAD	---	C1
C	2-11-8	-104	-104	---	FRONT	VERT	TOTAL	---	C1
C	2-11-8	-105	-105	---	FRONT	VERT	SNOW	---	C1
D	7-0-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
M	7-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
N	3-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
P	5-0-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
Q	9-0-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
R	11-0-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1

TOTAL WEIGHT = 2 X 85 = 170 lb

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/999 (0.07")

CSI: TC=0.24/1.00 (H-I:1), BC=0.24/1.00 (L-M:1), WB=0.28/1.00 (H-J:1), SS=0.16/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 788 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.86 (B) (INPUT = 0.90)
JSI METAL=0.25 (H) (INPUT = 1.00)



Structural component only
DWG# T-2115914

CONTINUED ON PAGE 2

JOB NAME 412982	TRUSS NAME T52	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:ProSyr 45RAfF3mJ8Agi49yajqE-835EMiNKDp?cgC96dA17M8riOKA8iI3ZP2wyKzCall

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMVW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMVW+w	MT20	2.0	4.0		
G	TMVW-t	MT20	4.0	4.0		
H	TMVW-t	MT20	4.0	6.0		
I	BMV1+p	MT20	3.0	4.0		
J	BMVW-t	MT20	4.0	6.0		
K	BS-t	MT20	3.0	6.0	1.50	2.75
L	BMVWW-t	MT20	4.0	9.0		
M	BMVW-t	MT20	4.0	4.0		
N	BMVW-t	MT20	4.0	4.0		
O	BMV1+p	MT20	3.0	4.0		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
S	13-0-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
T	15-0-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
U	17-0-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
V	19-0-12	-82	-82	---	FRONT	VERT	TOTAL	---	C1
W	1-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
X	5-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
Y	9-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
Z	11-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AA	13-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AB	15-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AC	17-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AD	19-0-12	-23	-23	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

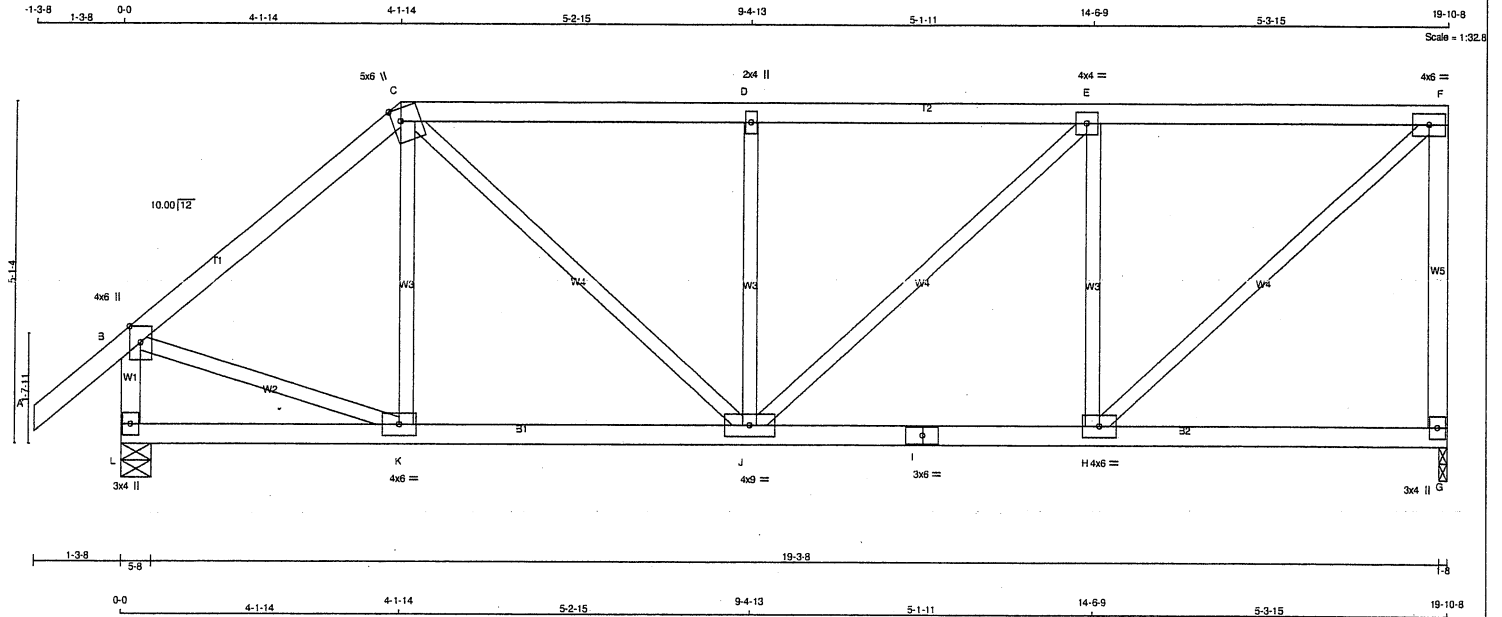


Structural component only
DWG# T-2115914 *ML*

JOB NAME 412982	TRUSS NAME T53	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 86 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
G - F	2x4	DRY	No.2
L - B	2x4	DRY	No.2
L - I	2x4	DRY	No.2
I - G	2x4	DRY	No.2

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMVW+p	MT20	4.0	6.0 Edge
C	TTWW+m	MT20	5.0	6.0 2.25 1.50
D	TMW+w	MT20	2.0	4.0
E	TMWW-t	MT20	4.0	4.0
F	TMVW-t	MT20	4.0	6.0
G	BMV1+p	MT20	3.0	4.0
H	BMWW-t	MT20	4.0	6.0
I	BS-t	MT20	3.0	6.0
J	BMWWW-t	MT20	4.0	9.0
K	BMWW-t	MT20	4.0	6.0
L	BMV1+p	MT20	3.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	GROSS REACTION	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	BRG	BRG
G	1096	0	1096	0	0	1-8	1-8		
L	1223	0	1223	0	0	5-8	5-8		

UNFACTORED REACTIONS							
1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	775	508 / 0	0 / 0	0 / 0	0 / 0	266 / 0	0 / 0
L	862	579 / 0	0 / 0	0 / 0	0 / 0	283 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.38 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. VERT. LOAD LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. VERT. LOAD LC1 (LC)	MAX. VERT. LOAD LC1 (LC)
FR-TO		FROM TO		FR-TO			
A-B	0 / 41	-91.8 -91.8	0.13 (1)	10.00	K-C	-135 / 28	0.05 (1)
B-C	-998 / 0	-91.8 -91.8	0.30 (1)	5.86	B-K	0 / 800	0.18 (1)
C-D	-1182 / 0	-91.8 -91.8	0.36 (1)	5.40	H-F	0 / 1279	0.29 (1)
D-E	-1182 / 0	-91.8 -91.8	0.38 (1)	5.38	C-J	0 / 570	0.13 (1)
E-F	-949 / 0	-91.8 -91.8	0.37 (1)	5.85	H-E	-747 / 0	0.29 (1)
G-F	-1055 / 0	0.0 0.0	0.47 (1)	7.66	J-D	-514 / 0	0.20 (1)
L-B	-1193 / 0	0.0 0.0	0.13 (1)	7.32	J-E	0 / 319	0.07 (1)
L-K	0 / 0	-18.5 -18.5	0.10 (4)	10.00			
K-J	0 / 762	-18.5 -18.5	0.18 (1)	10.00			
J-I	0 / 949	-18.5 -18.5	0.22 (1)	10.00			
I-H	0 / 949	-18.5 -18.5	0.22 (1)	10.00			
H-G	0 / 0	-18.5 -18.5	0.12 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL)= L/999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL)= L/999 (0.07")

CSI: TC=0.47/1.00 (F-G:1), BC=0.22/1.00 (H-J:1), WB=0.29/1.00 (F-H:1), SSI=0.23/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

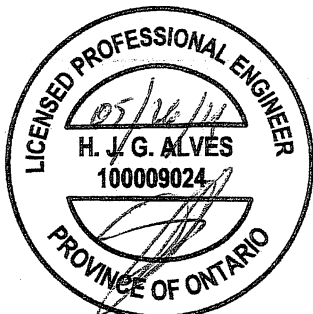
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (H) (INPUT = 0.90)
JSI METAL= 0.50 (B) (INPUT = 1.00)

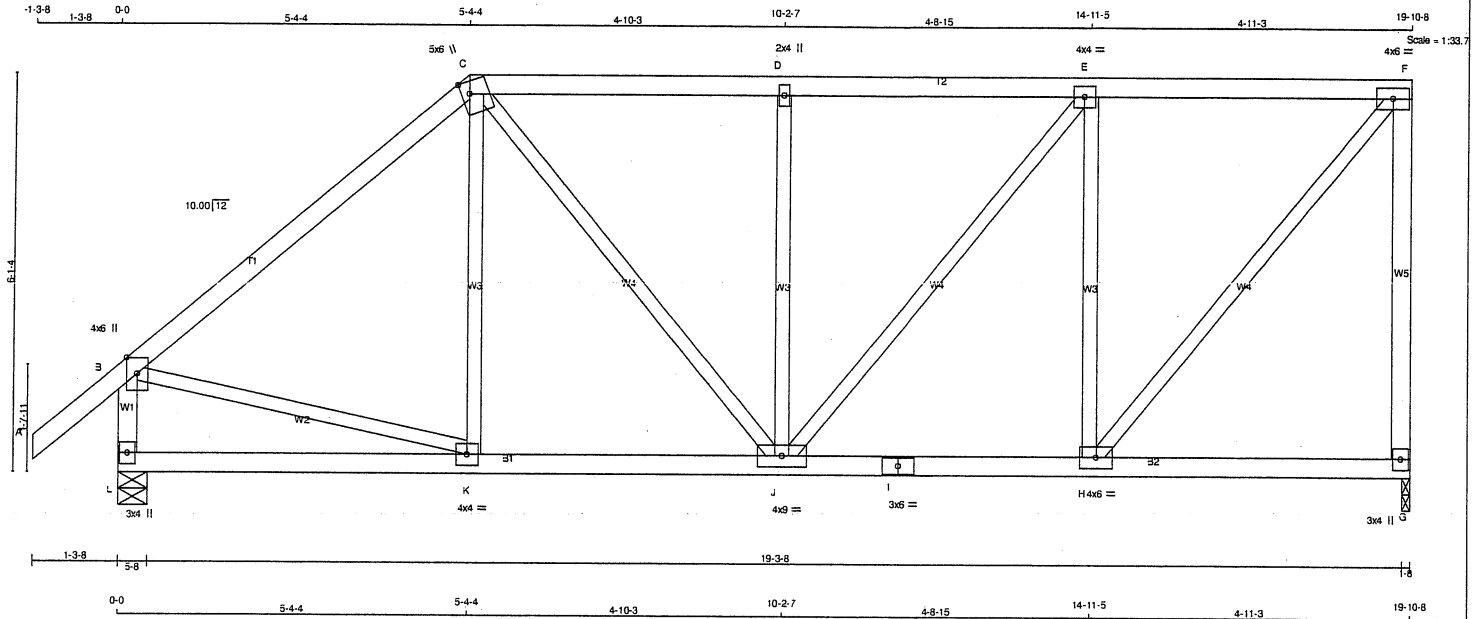


Structural component only
DWG# T-2115915

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
412982	T54	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - F	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
L - B	2x4	DRY	No.2	SPF	
L - I	2x4	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMVW+w	MT20	2.0	4.0		
E	TMVW+t	MT20	4.0	4.0		
F	TMVW+t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMVW+t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMVWW-t	MT20	4.0	9.0		
K	BMVW-t	MT20	4.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	UP
G	1096	0	1096	0
L	1223	0	1223	0

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	775	508 / 0	0 / 0	0 / 0	0 / 0	266 / 0	0 / 0
L	862	579 / 0	0 / 0	0 / 0	0 / 0	283 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.55 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED			
MEMB.	FORCE	VERT. LOAD	LC1 MAX	MAX. UNBRAC	MEMB.	FORCE	MAX
	(LBS)	(PLF)	CSI (LC)			(LBS)	CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
A-B	0 / 41	-91.8 -91.8	0.13 (1)	10.00	K-C	-68 / 61	0.04 (1)
B-C	-980 / 0	-91.8 -91.8	0.51 (1)	5.55	B-K	0 / 773	0.17 (1)
C-D	-973 / 0	-91.8 -91.8	0.30 (1)	5.91	H-F	0 / 1148	0.26 (1)
D-E	-973 / 0	-91.8 -91.8	0.31 (1)	5.88	C-J	0 / 347	0.08 (1)
E-F	-742 / 0	-91.8 -91.8	0.30 (1)	6.25	H-E	-772 / 0	0.45 (1)
G-F	-1059 / 0	0.0 0.0	0.76 (1)	7.65	J-D	-474 / 0	0.28 (1)
L-B	-1182 / 0	0.0 0.0	0.13 (1)	7.34	J-E	0 / 365	0.08 (1)
L-K	0 / 0	-18.5 -18.5	0.13 (4)	10.00			
K-J	0 / 750	-18.5 -18.5	0.19 (1)	10.00			
J-I	0 / 742	-18.5 -18.5	0.18 (1)	10.00			
I-H	0 / 742	-18.5 -18.5	0.18 (1)	10.00			
H-G	0 / 0	-18.5 -18.5	0.11 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL)= L/999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL)= L/999 (0.06")

CSI: TC=0.76/1.00 (F-G:1), BC=0.19/1.00 (J-K:1),
WB=0.45/1.00 (E-H:1), SSI=0.21/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (F) (INPUT = 0.90)
JSI METAL= 0.51 (B) (INPUT = 1.00)

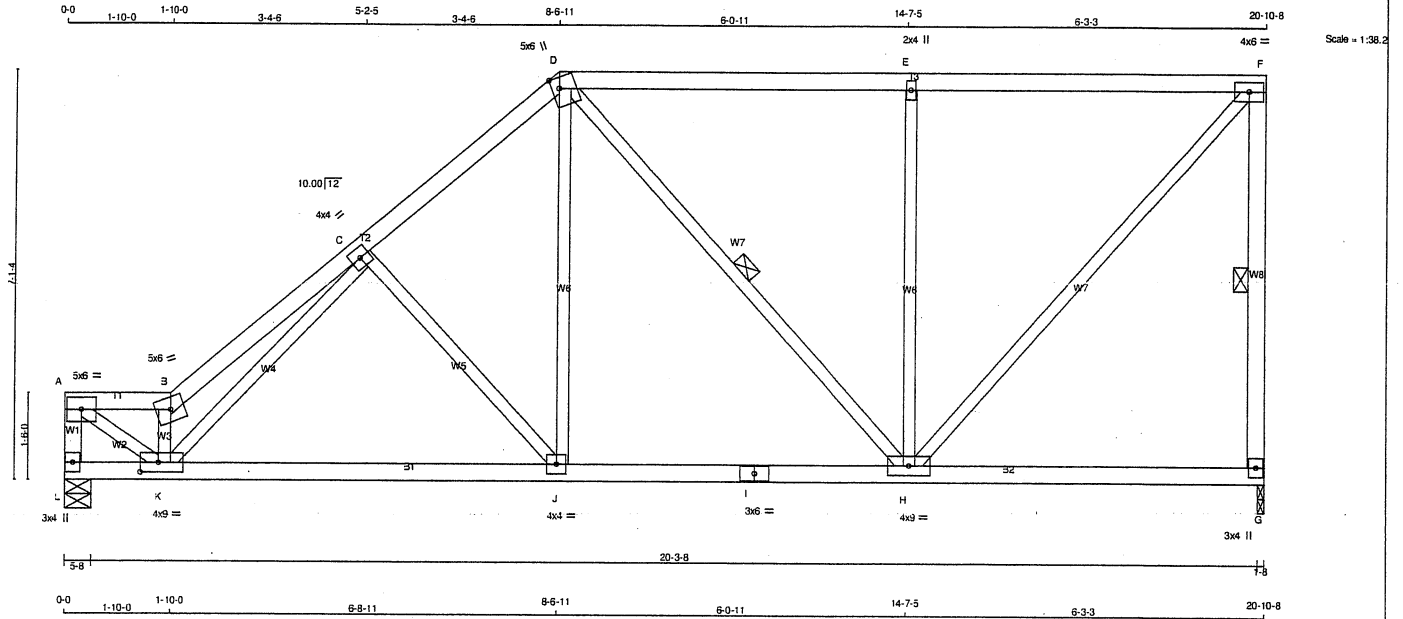


Structural component only
DWG# T-2115916

JOB NAME 412982	TRUSS NAME T55	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

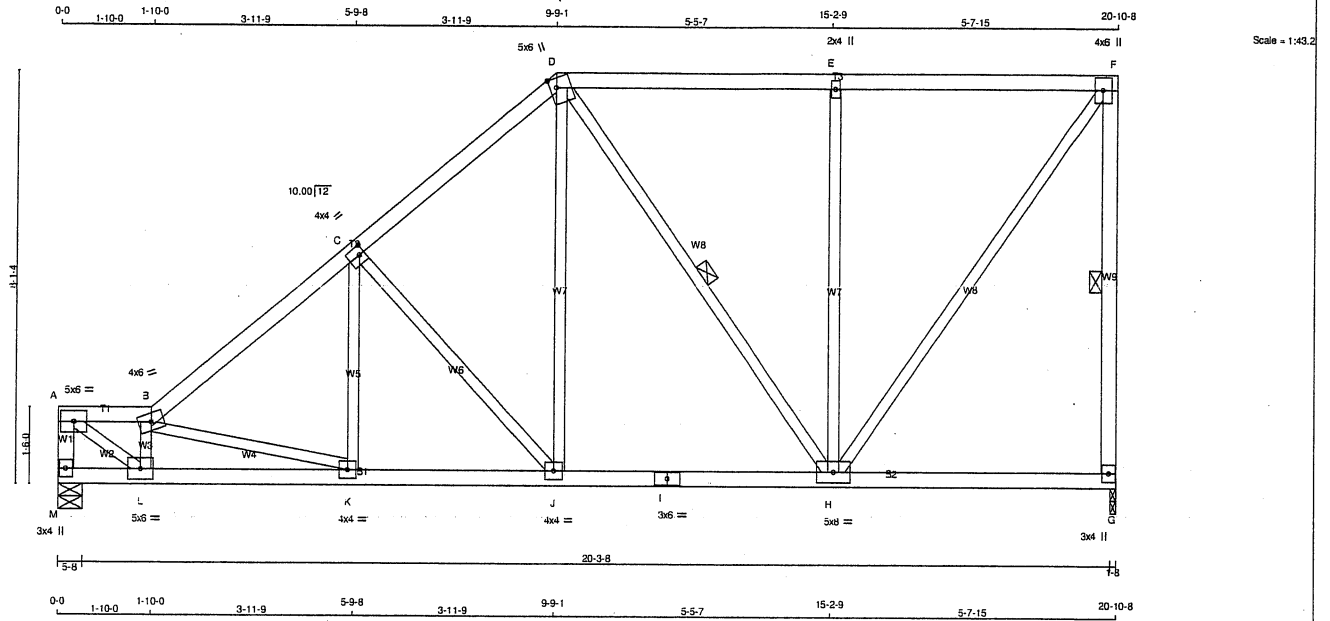
Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed May 26 16:46:38 2021 Page 1
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JOB NAME 412982	TRUSS NAME T56	QUANTITY 5	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 5 X 101 = 504 lb

LUMBER		LUMBER		DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	DRY	No.2	SPF
M - A	2x4	DRY	No.2	SPF	
A - B	2x4	DRY	No.2	SPF	
B - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
M - I	2x4	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW-t	MT20	5.0	6.0	
B	TTWW-m	MT20	4.0	6.0	
C	TMWW-t	MT20	4.0	4.0	2.00 1.25
D	TTWW+m	MT20	5.0	6.0	2.25 1.50
E	TMW+w	MT20	2.0	4.0	
F	TMVW+p	MT20	4.0	6.0	
G	BMV1+p	MT20	3.0	4.0	
H	BMVWW-t	MT20	5.0	8.0	
I	BS-t	MT20	3.0	6.0	
J	BMVW-t	MT20	4.0	4.0	
K	BMVW-t	MT20	4.0	4.0	
L	BMVW-t	MT20	5.0	6.0	
M	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	VERT	GROSS REACTION	GROSS REACTION	DOWN	HORZ	UPLIFT	IN-SX	BRG	BRG
M	1151	0	1151	0	0	5-8	5-8		
G	1151	0	1151	0	0	1-8	1-8		

UNFACTORED REACTIONS							
1ST LC CASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	814	534 / 0	0 / 0	0 / 0	0 / 0	280 / 0	0 / 0
G	814	534 / 0	0 / 0	0 / 0	0 / 0	280 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.15 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM	TO	FR-TO			
M-A	-1112 / 0	0.0	0.0 0.11 (1)	A-L	0 / 1825		0.41 (1)
A-B	-1495 / 0	-91.8	-91.8 0.08 (1)	L-B	-1035 / 0		0.15 (1)
B-C	-1430 / 0	-91.8	-91.8 0.28 (1)	B-K	-484 / 0		0.15 (1)
C-D	-1027 / 0	-91.8	-91.8 0.26 (1)	K-C	0 / 230		0.05 (1)
D-E	-654 / 0	-91.8	-91.8 0.49 (1)	C-J	-530 / 0		0.34 (1)
E-F	-655 / 0	-91.8	-91.8 0.49 (1)	J-D	0 / 475		0.11 (1)
G-F	-1109 / 0	0.0	0.0 0.33 (1)	D-H	-203 / 0		0.13 (1)
				H-E	-631 / 0		0.80 (1)
M-L	0 / 0	-18.5	-18.5 0.05 (1)	H-F	0 / 1123		0.25 (1)
L-K	0 / 1586	-18.5	-18.5 0.32 (1)				
K-J	0 / 1123	-18.5	-18.5 0.22 (1)				
J-I	0 / 772	-18.5	-18.5 0.21 (4)				
I-H	0 / 772	-18.5	-18.5 0.21 (4)				
H-G	0 / 0	-18.5	-18.5 0.14 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.70")
CALCULATED VERT. DEFL. (LL) = L/999 (0.04")
ALLOWABLE DEFL. (TL) = L/360 (0.70")
CALCULATED VERT. DEFL. (TL) = L/999 (0.08")

CSI: TC=0.49/1.00 (E-F:1), BC=0.32/1.00 (K-L:1), WB=0.80/1.00 (E-H:1), SSI=0.25/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

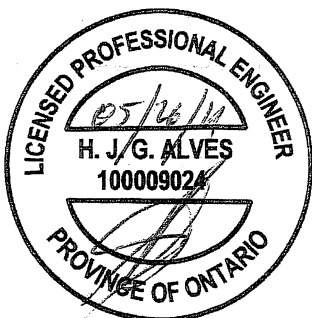
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.83 (A) (INPUT = 0.90)
JSI METAL = 0.42 (B) (INPUT = 1.00)

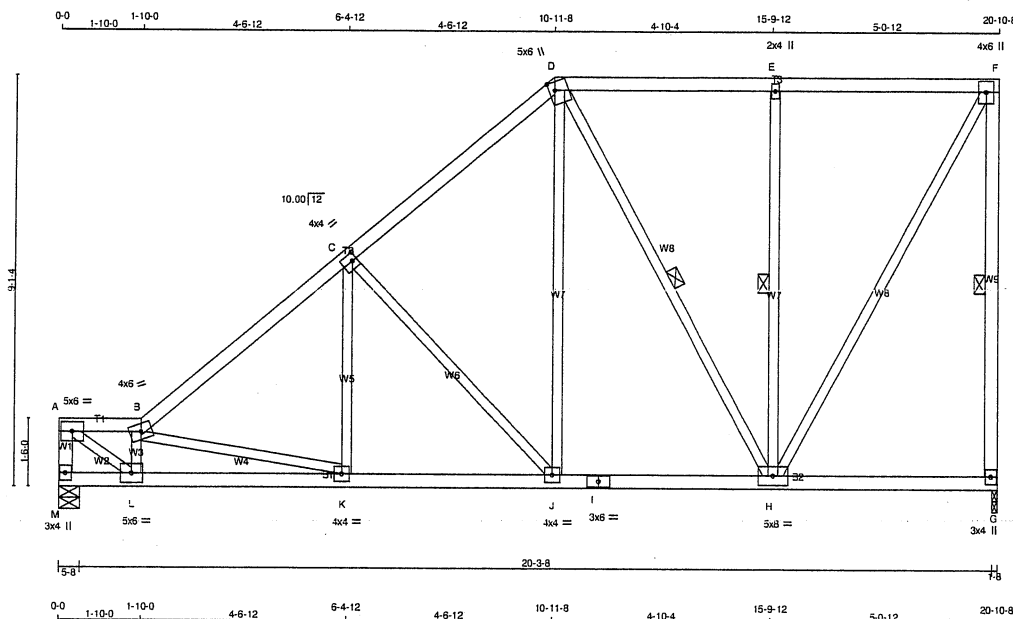


Structural component only
DWG# T-2115918

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
412982	T57	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed May 26 16:46:40 2021 Page 1
ID:PrpSyr_45RAfF3mJ8AgI49yajqE-V1v7PPnW8mSljS46vBmCqPrfWP0xpwoihhdXzCaId



TOTAL WEIGHT = 106 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	DRY	DESCR.
M - A	2x4	DRY	No.2
A - B	2x4	DRY	No.2
B - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
G - F	2x4	DRY	No.2
M - I	2x4	DRY	No.2
I - G	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0		
B	TTWW-m	MT20	4.0	6.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	5.0	8.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
M	1151	0	1151	0	5-8	5-8
G	1151	0	1151	0	1-8	1-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
M	814	534 / 0	0 / 0	0 / 0	0 / 0	280 / 0	0 / 0
G	814	534 / 0	0 / 0	0 / 0	0 / 0	280 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.10 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED			
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			FR-TO		
M-A	-1115 / 0	0.0	0.0 0.11 (1)	7.51	A-L	0 / 1830	0.41 (1)
A-B	-1500 / 0	-91.8	-91.8 0.08 (1)	5.30	L-B	-1030 / 0	0.15 (1)
B-C	-1373 / 0	-91.8	-91.8 0.38 (1)	5.10	B-K	-524 / 0	0.20 (1)
C-D	-918 / 0	-91.8	-91.8 0.35 (1)	5.97	K-C	0 / 238	0.05 (4)
D-E	-532 / 0	-91.8	-91.8 0.38 (1)	6.25	C-J	-589 / 0	0.51 (1)
E-F	-532 / 0	-91.8	-91.8 0.38 (1)	6.25	J-D	0 / 510	0.11 (1)
G-F	-1113 / 0	0.0	0.0 0.43 (1)	6.01	D-H	-313 / 0	0.23 (1)
					H-E	-561 / 0	0.30 (1)
M-L	0 / 0	-18.5	-18.5 0.05 (1)	10.00	H-F	0 / 1078	0.24 (1)
L-K	0 / 1590	-18.5	-18.5 0.33 (1)	10.00			
K-J	0 / 1082	-18.5	-18.5 0.23 (1)	10.00			
J-I	0 / 685	-18.5	-18.5 0.17 (4)	10.00			
I-H	0 / 685	-18.5	-18.5 0.17 (4)	10.00			
H-G	0 / 0	-18.5	-18.5 0.11 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.70")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.05")
ALLOWABLE DEFL.(TL) = $L/360$ (0.70")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.09")

CSI: TC=0.43/1.00 (F-G:1), BC=0.33/1.00 (K-L:1), WB=0.51/1.00 (C-J:1), SSI=0.22/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

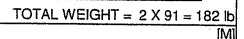
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.83 (A) (INPUT = 0.90)
JSI METAL = 0.42 (B) (INPUT = 1.00)



Structural component only
DWG# T-2115919

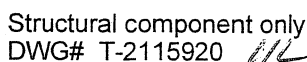
Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed May 26 16:46:42 2021 Page 1
ID:PrpSyr_45RAfF3mJ8Agi49yaiqE-RP1uq5omgNi0yEV0boqvqx1ACiXHto5A?EnhQzCaIb



SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

J	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
N	7-4-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
O	9-4-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
P	11-4-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
Q	13-4-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
R	15-4-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
S	17-4-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
T	6-4	-490	-490	---	BACK	VERT	TOTAL	---	C1
U	2-6-4	-307	-307	---	BACK	VERT	TOTAL	---	C1
V	3-4-12	-307	-307	---	BACK	VERT	TOTAL	---	C1
W	5-4-12	-352	-352	---	BACK	VERT	TOTAL	---	C1

JSI GRIP= 0.75 (F) (INPUT = 0.90)
JSI METAL= 0.43 (A) (INPUT = 1.00)



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
412982	T58	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:PrpSyr 45RAfF3mJ8Agi49yaiqE-RP1uq5omgNi0yIEV0bogvqx1ACiXHto5A?EnhQzCaIB

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	Edge	
B	TTWW+m	MT20	5.0	6.0	2.25	1.50
C	TMWW-t	MT20	4.0	4.0		
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	6.0		
H, K, L						
H	BMWW-t	MT20	5.0	6.0		
I	BS-t	MT20	5.0	6.0		
J	BMWWW-t	MT20	5.0	8.0		
M	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
X	7-4-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
Y	9-4-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
Z	11-4-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
AA	13-4-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
AB	15-4-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
AC	17-4-12	-21	-21	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

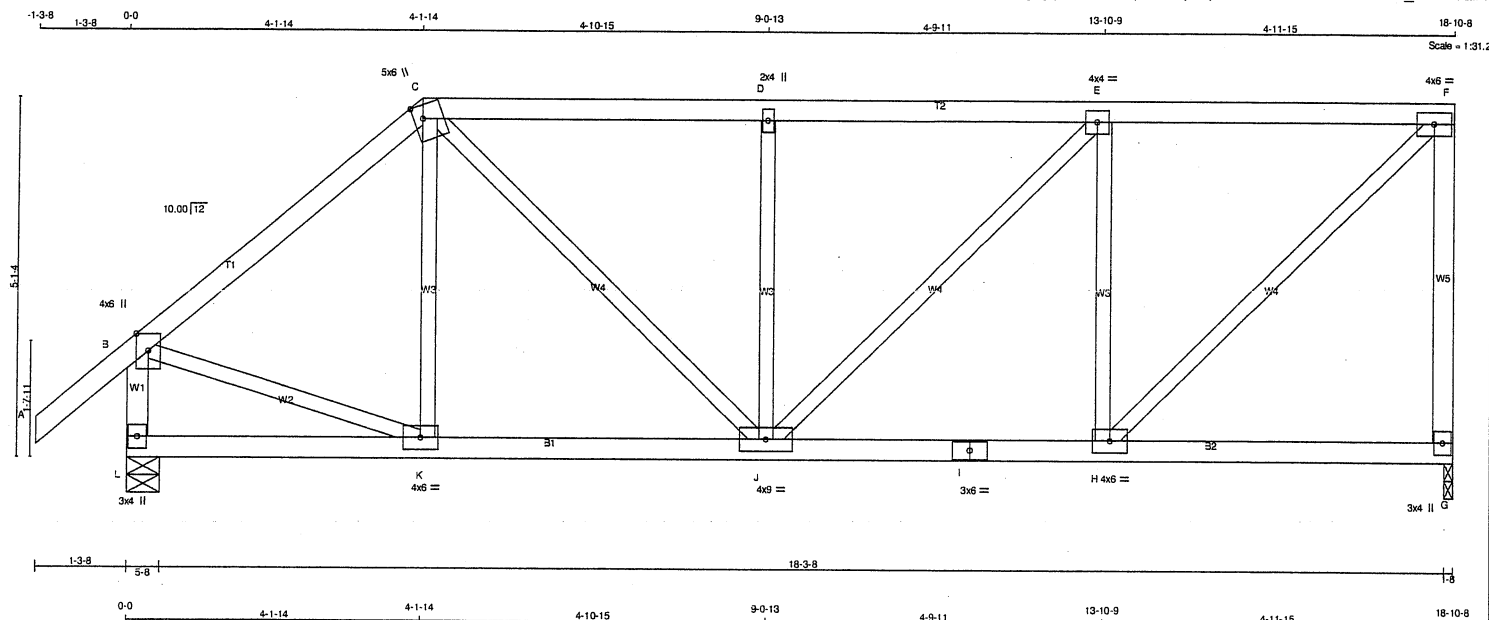
- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2115920

JOB NAME 412982	TRUSS NAME T59	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:PrpSyr_45RAf3mJ8AgI49yajqE-vcaG1RpORhqtavphaJvR2T9Zc3m0JNEOf_LEszCalA



TOTAL WEIGHT = 83 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
G - F	2x4	DRY	No.2
L - B	2x4	DRY	No.2
L - I	2x4	DRY	No.2
I - G	2x4	DRY	No.2

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMVW+p	MT20	4.0	6.0 Edge
C	TTWW+m	MT20	5.0	6.0 2.25 1.50
D	TMW+w	MT20	2.0	4.0
E	TMWW-t	MT20	4.0	4.0
F	TMVW-t	MT20	4.0	6.0
G	BMV1+p	MT20	3.0	4.0
H	BMWW-t	MT20	4.0	6.0
I	BS-t	MT20	3.0	6.0
J	BMWW-t	MT20	4.0	9.0
K	BMWW-t	MT20	4.0	6.0
L	BMV1+p	MT20	3.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	DOWN	HORZ	UPLIFT	IN-SX
G	1041	0	1041	0	1-8
L	1168	0	1168	0	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
G	736	483 / 0	0 / 0	0 / 0	0 / 0	253 / 0	0 / 0
L	823	554 / 0	0 / 0	0 / 0	0 / 0	270 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.67 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX UNBRACED LENGTH	FR-TO
FR-TO		FROM TO		FR-TO			
A-B	0 / 41	-91.8 -91.8	0.13 (1)	10.00	K-C	-123 / 30	0.05 (1)
B-C	-934 / 0	-91.8 -91.8	0.30 (1)	6.01	B-K	0 / 749	0.17 (1)
C-D	-1066 / 0	-91.8 -91.8	0.31 (1)	5.70	H-F	0 / 1177	0.26 (1)
D-E	-1066 / 0	-91.8 -91.8	0.32 (1)	5.67	C-J	0 / 493	0.11 (1)
E-F	-848 / 0	-91.8 -91.8	0.32 (1)	6.19	H-E	-713 / 0	0.27 (1)
G-F	-1003 / 0	0.0 0.0	0.44 (1)	7.81	J-D	-481 / 0	0.18 (1)
L-B	-1137 / 0	0.0 0.0	0.12 (1)	7.45	J-E	0 / 308	0.07 (1)
L-K	0 / 0	-18.5 -18.5	0.09 (4)	10.00			
K-J	0 / 713	-18.5 -18.5	0.17 (1)	10.00			
J-I	0 / 848	-18.5 -18.5	0.19 (1)	10.00			
I-H	0 / 848	-18.5 -18.5	0.19 (1)	10.00			
H-G	0 / 0	-18.5 -18.5	0.11 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.63")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.03")
ALLOWABLE DEFL.(TL) = $L/360$ (0.63")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.06")

CSI: TC=0.44/1.00 (F-G:1), BC=0.19/1.00 (H-J:1), WB=0.27/1.00 (E-H:1), SSI=0.21/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (F) (INPUT = 0.90)

JSI METAL= 0.47 (B) (INPUT = 1.00)

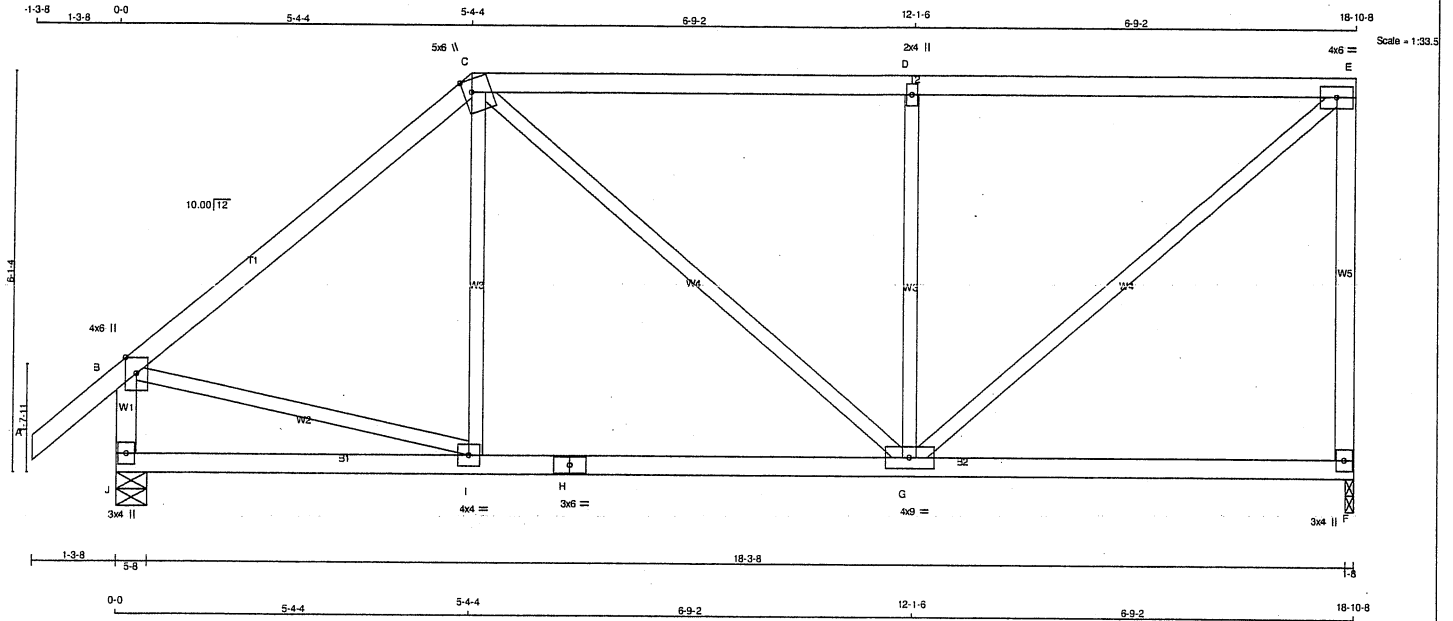


Structural component only
DWG# T-2115921

JOB NAME 412982	TRUSS NAME T60	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:PrpSyr_45RAIF3mJ8Agi49yaiqE-No8eFnq0C_yk3Ou80q8_F0EC0OvjItNdJumizCaI9



TOTAL WEIGHT = 82 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
F - E	2x4	DRY	No.2
J - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
H - F	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	4.0	6.0	Edge
C	TTWW+m	MT20	5.0	6.0	2.25 1.50
D	TMVW+w	MT20	2.0	4.0	
E	TMVW-t	MT20	4.0	6.0	
F	BMV1+p	MT20	3.0	4.0	
G	BMVWW-t	MT20	4.0	9.0	
H	BS-t	MT20	3.0	6.0	
I	BMVWW-t	MT20	4.0	4.0	
J	BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED		MAXIMUM FACTORED		INPUT	REQRD
	GROSS REACTION	GROSS REACTION	DOWN	UP		
F	1041	0	1041	0	1-8	1-8
J	1168	0	1168	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
F	736	483 / 0	0 / 0	0 / 0	0 / 0	253 / 0	0 / 0
J	823	554 / 0	0 / 0	0 / 0	0 / 0	270 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.07 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	
FR-TO				FR-TO			
A-B	0 / 41	-91.8	10.00	I-C	-48 / 75	10.03 (1)	
B-C	-915 / 0	-91.8	5.70	C-G	0 / 237	0.05 (1)	
C-D	-880 / 0	-91.8	5.08	G-D	-769 / 0	0.45 (1)	
D-E	-880 / 0	-91.8	5.07	G-E	0 / 1158	0.26 (1)	
F-E	-989 / 0	0.0	7.81	B-I	0 / 721	0.16 (1)	
J-B	-1128 / 0	0.0	7.48				
J-I	0 / 0	-18.5	10.00				
I-H	0 / 701	-18.5	10.00				
H-G	0 / 701	-18.5	10.00				
G-F	0 / 0	-18.5	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
DL = 6.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.4	PSF	
TOTAL LOAD = 39.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL)= L/999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL)= L/999 (0.08")

CSI: TC=0.77/1.00 (D-E:1), BC=0.26/1.00 (G-I:4),
WB=0.45/1.00 (D-G:1), SSI=0.30/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (I) (INPUT = 0.90)
JSI METAL= 0.48 (B) (INPUT = 1.00)

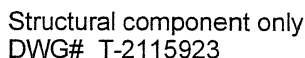


Structural component only
DWG# T-2115922

Tamarack Roof Truss, Burlington

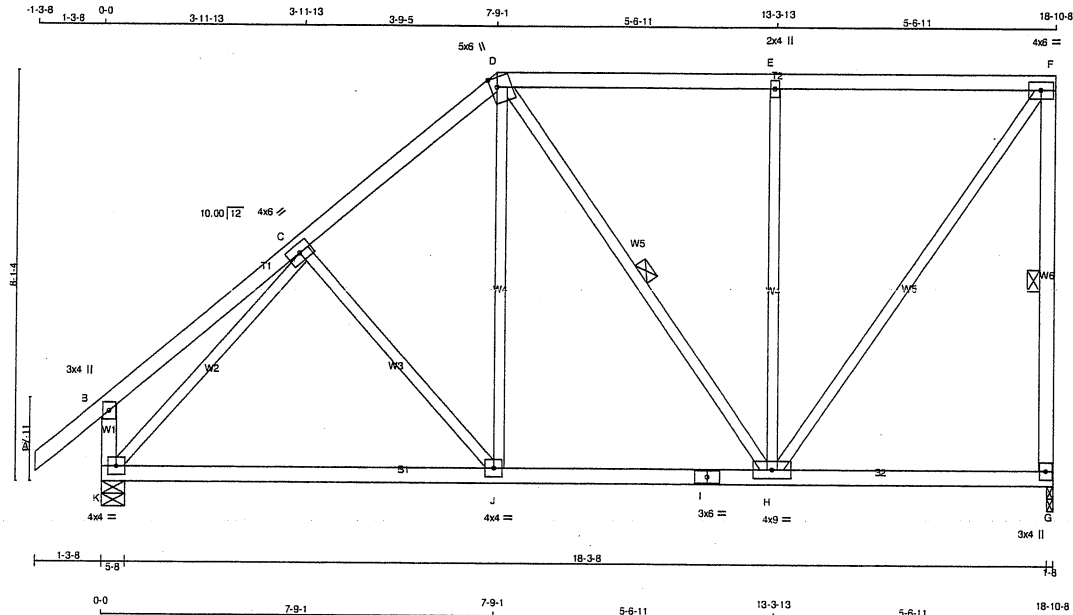


JSI GRIP= 0.85 (K) (INPUT = 0.90)
JSI METAL= 0.26 (I) (INPUT = 1.00)



JOB NAME 412982	TRUSS NAME T62	QUANTITY 3	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed May 26 16:46:46 2021 Page 1
ID:PrpSyr_45RAff3mJ8Agi49yajE-JAGOGsRrHkcCRRNXGFRtc3g5f4q4wDYvg5dC?qBzCal7



TOTAL WEIGHT = 3 X 94 = 283 lb

LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	SPF	
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
K - B	2x4	DRY	No.2	SPF	
K - I	2x4	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
DRY: SEASONED LUMBER.					

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
C	TMWW-t	MT20	4.0	6.0	
D	TTWW+w	MT20	5.0	6.0	2.25 1.50
E	TMV+w	MT20	2.0	4.0	
F	TMVW-t	MT20	4.0	6.0	
G	BMV1+p	MT20	3.0	4.0	
H	BMWWW-t	MT20	4.0	9.0	
I	BS-t	MT20	3.0	6.0	
J	BMWW-t	MT20	4.0	4.0	
K	BMVW1-t	MT20	4.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT		VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
G	1041	0	1041	0	0	1-8	1-8	1-8	
K	1168	0	1168	0	0	5-8	5-8	5-8	

UNFACTORED REACTIONS

1ST CASE		MAX./MIN. COMPONENT REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
G	736	483 / 0	0 / 0	0 / 0	0 / 0	253 / 0	0 / 0		
K	823	554 / 0	0 / 0	0 / 0	0 / 0	270 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	
FR-TO		FROM TO		FR-TO			
A-B	0 / 41	-91.8 -91.8 0.13 (1)	10.00	C-J	-152 / 0	0.10 (1)	
B-C	0 / 27	-91.8 -91.8 0.22 (1)	10.00	J-D	0 / 260	0.06 (1)	
C-D	-836 / 0	-91.8 -91.8 0.24 (1)	6.25	D-H	-87 / 0	0.06 (1)	
D-E	-573 / 0	-91.8 -91.8 0.49 (1)	6.25	H-E	-631 / 0	0.80 (1)	
E-F	-573 / 0	-91.8 -91.8 0.49 (1)	6.25	H-F	0 / 984	0.22 (1)	
G-F	-995 / 0	0.0 0.0 0.29 (1)	6.25	K-C	-1111 / 0	0.66 (1)	
K-B	-262 / 0	0.0 0.0 0.03 (1)	7.81				
K-J	0 / 721	-18.5 -18.5 0.29 (4)	10.00				
J-I	0 / 624	-18.5 -18.5 0.29 (4)	10.00				
I-H	0 / 624	-18.5 -18.5 0.29 (4)	10.00				
H-G	0 / 0	-18.5 -18.5 0.12 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD				= 39.0 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/999 (0.12")

CSI: TC=0.49/1.00 (E-F:1), BC=0.29/1.00 (J-K:4), WB=0.80/1.00 (E-H:1), SS=0.25/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
MIN	MAX	MIN
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (H) (INPUT = 0.90)
JSI METAL= 0.25 (C) (INPUT = 1.00)

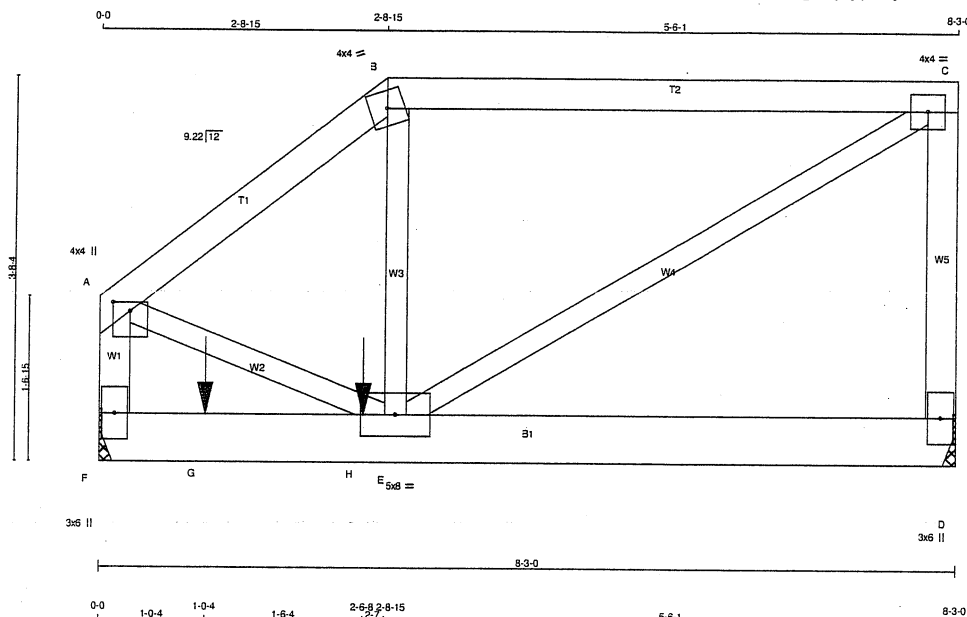


Structural component only
DWG# T-2115924

Structural component only
DWG# T-2115925

JOB NAME 412982	TRUSS NAME T64	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:PrpSyr_45RAfF3mJ8Ag49yjqE-klyXIUu91Xa0lqGrxaQJhlj9a19FQ377naRfRWzCal4



TOTAL WEIGHT = 39 lb

LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	SPF	
A - B	2x4	DRY	No.2	SPF	
B - C	2x4	DRY	No.2	SPF	
D - C	2x4	DRY	No.2	SPF	
F - A	2x4	DRY	No.2	SPF	
F - D	2x6	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TTW-m	MT20	4.0	4.0		
C	TMVW-t	MT20	4.0	4.0		
D	BMV1+p	MT20	3.0	6.0		
E	BMVWW-t	MT20	5.0	8.0		
F	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
D	568	0	568	0	MECHANICAL	
F	711	0	711	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D, F. MINIMUM BEARING LENGTH AT JOINT D = 1-8, JOINT F = 1-8.

UNFACTORED REACTIONS

JT	COMBINED	MAX./MIN. COMPONENT REACTIONS					
		1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD
D	401	267 / 0	0 / 0	0 / 0	0 / 0	133 / 0	0 / 0
F	501	338 / 0	0 / 0	0 / 0	0 / 0	164 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED				WEBS			
	MAX. FACTORED	FORCE	VERT. LOAD	LC1	MAX	MAX.	MEMB.	MAX. FACTORED	FORCE	MAX
FR-TO		(LBS)		(PLF)	CSI (LC)	UNBRAC			(LBS)	CSI (LC)
A-B	-566 / 0		-91.8	-91.8	0.14 (1)	6.25	E-B	-33 / 48		0.02 (4)
B-C	-447 / 0		-91.8	-91.8	0.54 (1)	6.25	E-C	0 / 524		0.13 (1)
D-C	-525 / 0		0.0	0.0	0.11 (1)	7.81	A-E	0 / 484		0.12 (1)
F-A	-656 / 0		0.0	0.0	0.07 (1)	7.81				
F-G	0 / 0		-18.5	-18.5	0.06 (4)	10.00				
G-H	0 / 0		-18.5	-18.5	0.06 (4)	10.00				
H-E	0 / 0		-18.5	-18.5	0.06 (4)	10.00				
E-D	0 / 0		-18.5	-18.5	0.06 (4)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-0-4	-3	-3	---	BACK	VERT	TOTAL	---	C1
H	2-6-8	-255	-255	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.27")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.27")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.54/1.00 (B-C:1), BC=0.06/1.00 (D-E:4), WB=0.13/1.00 (C-E:1), SSI=0.24/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

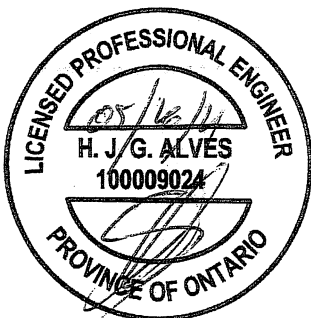
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	650	371	1747
		788	1987
			1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.60 (A) (INPUT = 0.90)
JSI METAL= 0.16 (A) (INPUT = 1.00)

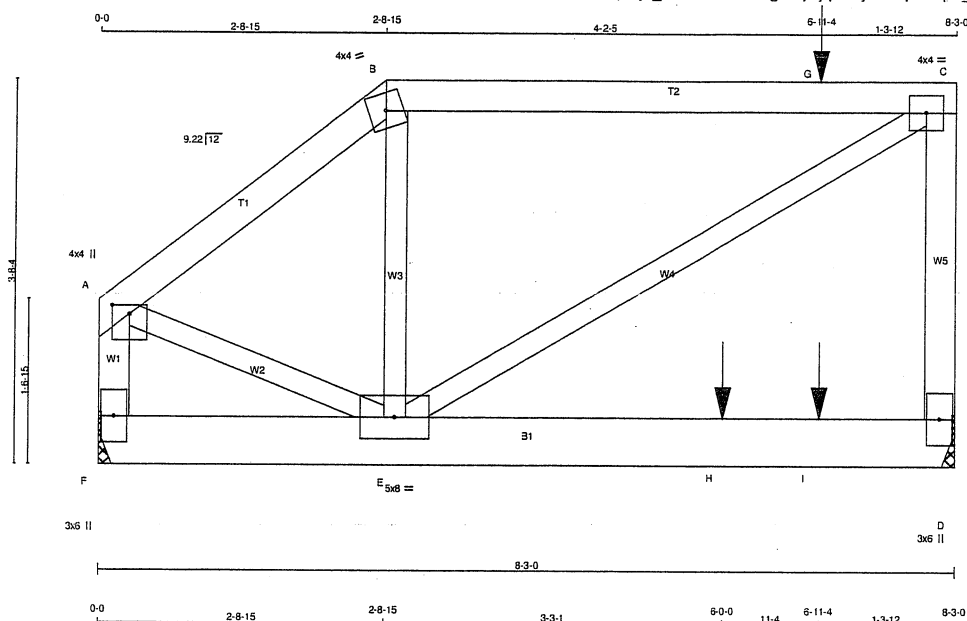


Structural component only
DWG# T-2115926

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
412982	T64Z	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:PrpSyr_45RAfF3mJ8Agi49yjqE-CyVvVqunoqitv_r1UHxYEWGK2RSy9XnG?EACzyzCa3



TOTAL WEIGHT = 39 lb

LUMBER				DESCR.			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	SPF	SPF	SPF	SPF
A - B	2x4	DRY	No.2	SPF	SPF	SPF	SPF
B - C	2x4	DRY	No.2	SPF	SPF	SPF	SPF
D - C	2x4	DRY	No.2	SPF	SPF	SPF	SPF
F - A	2x4	DRY	No.2	SPF	SPF	SPF	SPF
F - D	2x6	DRY	No.2	SPF	SPF	SPF	SPF
ALL WEBS	2x3	DRY	No.2	SPF	SPF	SPF	SPF
EXCEPT							

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW+p	MT20	4.0	4.0	1.00 2.00
B	TTW-m	MT20	4.0	4.0	
C	TMVW-t	MT20	4.0	4.0	
D	BMV1+p	MT20	3.0	6.0	
E	BMVWW-t	MT20	5.0	8.0	
F	BMV1+p	MT20	3.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQRD
JT	GROSS REACTION	GROSS REACTION	DOWN	UP	BRG
D	640	0	640	0	MECHANICAL
F	520	0	520	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D, F. MINIMUM BEARING LENGTH AT JOINT D = 1-8, JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
D	451	306 / 0	0 / 0	0 / 0	144 / 0	0 / 0
F	367	245 / 0	0 / 0	0 / 0	122 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED	FACTORED	MAX	MAX	MEMB.	MAX. FACTORED	MAX
MEMB.	FORCE (LBS)	VERT. LOAD	LC1	MAX	UNBRAC	MEMB.	FORCE (LBS)	MAX
FR-TO		FROM	TO	LENGTH	FR-TO			
A-B	-451 / 0	-91.8	-91.8	0.13 (1)	6.25	E-B	-108 / 25	0.02 (1)
B-G	-353 / 0	-91.8	-91.8	0.55 (1)	6.25	E-C	0 / 414	0.10 (1)
G-C	-353 / 0	-91.8	-91.8	0.55 (1)	6.25	A-E	0 / 385	0.10 (1)
D-C	-484 / 0	0.0	0.0	0.10 (1)	7.81			
F-A	-548 / 0	0.0	0.0	0.06 (1)	7.81			
F-E	0 / 0	-18.5	-18.5	0.11 (1)	10.00			
E-H	0 / 0	-18.5	-18.5	0.22 (1)	10.00			
H-I	0 / 0	-18.5	-18.5	0.22 (1)	10.00			
I-D	0 / 0	-18.5	-18.5	0.22 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	6-11-4	-14	-14	---	FRONT	VERT	TOTAL	---	C1
H	6-0-0	-157	-157	---	FRONT	VERT	TOTAL	---	C1
I	6-11-4	-3	-3	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

- C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.27")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.02")
ALLOWABLE DEFL.(TL) = $L/360$ (0.27")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.04")

CSI: TC=0.55/1.00 (B-C:1), BC=0.22/1.00 (D-E:1), WB=0.10/1.00 (C-E:1), SSI=0.23/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

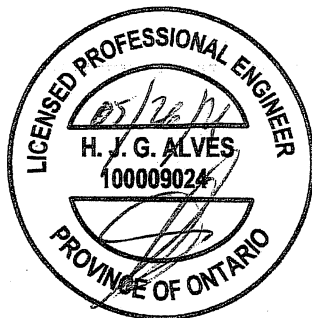
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.49 (A) (INPUT = 0.90)
JSI METAL= 0.13 (A) (INPUT = 1.00)

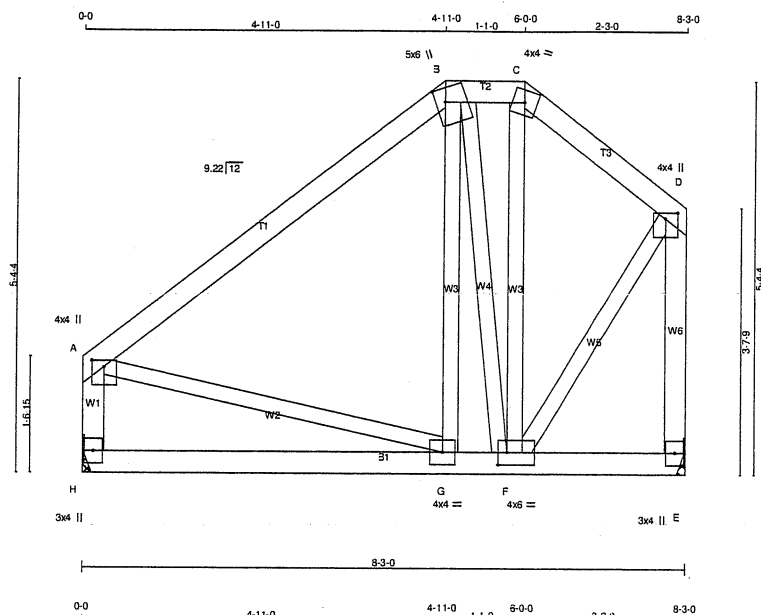


Structural component only
DWG# T-2115927

JOB NAME 412982	TRUSS NAME T65	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:PrpSyr_45RAfF3mJ8Agi49yajqE-g83HjAvPZ8qkX8QE2_SnmjoZ1rqu_NPEuwmWOzCal2



TOTAL WEIGHT = 2 X 45 = 90 lb [M/F]

LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TTWV+m	MT20	5.0	6.0	2.25	1.50
C	TTW-m	MT20	4.0	4.0		
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMV1+p	MT20	3.0	4.0		
F	BMVWW-t	MT20	4.0	6.0	2.00	1.50
G	BMVW-t	MT20	4.0	4.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
H	455	0	455	0
E	455	0	455	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H, E. MINIMUM BEARING LENGTH AT JOINT H = 1-8, JOINT E = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	DEAD	SOIL
H	322	211 / 0	0 / 0	0 / 0
E	322	211 / 0	0 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	-236 / 0	-91.8 -91.8	0.28 (1)	G-B	0 / 95	0.03 (4)	
B-C	-153 / 0	-91.8 -91.8	0.01 (1)	B-F	-185 / 0	0.08 (1)	
C-D	-194 / 0	-91.8 -91.8	0.06 (1)	F-C	-34 / 14	0.01 (1)	
H-A	-416 / 0	0.0 0.0	0.04 (1)	A-G	0 / 192	0.04 (1)	
E-D	-437 / 0	0.0 0.0	0.10 (1)	F-D	0 / 284	0.06 (1)	
H-G	0 / 0	-18.5 -18.5	0.10 (4)	10.00			
G-F	0 / 188	-18.5 -18.5	0.12 (4)	10.00			
F-E	0 / 0	-18.5 -18.5	0.02 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
DL = 8.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.4	PSF	
TOTAL LOAD = 39.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.28")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.28")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.28/1.00 (A-B:1), BC=0.12/1.00 (F-G:4), WB=0.08/1.00 (B-F:1), SS=0.14/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.36 (A) (INPUT = 0.90)
JSI METAL = 0.09 (A) (INPUT = 1.00)

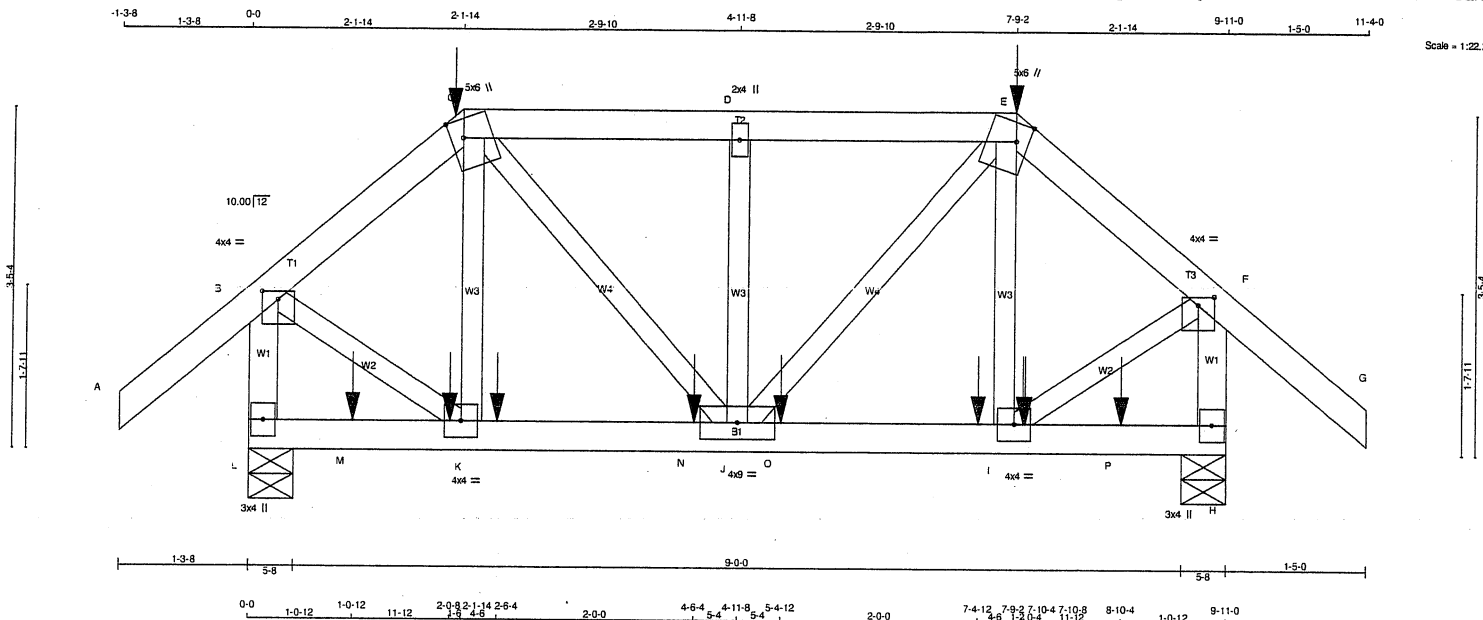


Structural component only
DWG# T-2115928

JOB NAME 412982	TRUSS NAME T66	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:PrpSyr_45RAIF3mJ8Agi49yajqE-8KdgwWw1K5yb9I?Qciz0JxLm7E8ldPhZTYfJ2rzCa1l



TOTAL WEIGHT = 2 X 48 = 96 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
L - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
L - H	2x4	DRY	No.2

ALL WEBS EXCEPT	2x3	DRY	No.2
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DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12	SIDE(61.0)
C - E	12	SIDE(61.0)
E - G	12	SIDE(61.0)
L - B	12	TOP
H - F	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
L - H	12	SIDE(61.0)
WEBS : (0.122"x3") SPIRAL NAILS		
K - C	6	SIDE(101.1)
I - E	6	SIDE(82.3)
2x3	1	6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
L	2165	0	2165	0	5-8	5-8
H	2295	0	2295	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
L	1520	1059 / 0	0 / 0	0 / 0	0 / 0	461 / 0	0 / 0
H	1611	1122 / 0	0 / 0	0 / 0	0 / 0	489 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED HORZ. LOAD (LC1)	MAX. FACTORED UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORZ. LOAD (LC)
FR-TO					FR-TO		
A - B	0 / 41	-91.8	-91.8	0.07 (1)	K - C	0 / 240	0.03 (1)
B - C	-1767 / 0	-91.8	-91.8	0.05 (1)	C - J	0 / 834	0.10 (1)
C - D	-1916 / 0	-91.8	-91.8	0.06 (1)	J - D	-268 / 0	0.03 (1)
D - E	-1916 / 0	-91.8	-91.8	0.06 (1)	J - E	0 / 704	0.09 (1)
E - F	-1874 / 0	-91.8	-91.8	0.05 (1)	J - E	0 / 405	0.05 (1)
F - G	0 / 44	-91.8	-91.8	0.08 (1)	B - K	0 / 1580	0.19 (1)
L - B	-2146 / 0	0.0	0.0	0.12 (1)	I - F	0 / 1654	0.20 (1)
H - F	-2273 / 0	0.0	0.0	0.13 (1)			
L - M	0 / 0	-18.5	-18.5	0.01 (4)			
M - K	0 / 0	-18.5	-18.5	0.01 (4)			
K - N	0 / 1359	-18.5	-18.5	0.22 (1)			
N - J	0 / 1359	-18.5	-18.5	0.22 (1)			
J - O	0 / 1446	-18.5	-18.5	0.23 (1)			
O - I	0 / 1446	-18.5	-18.5	0.23 (1)			
I - P	0 / 0	-18.5	-18.5	0.01 (4)			
P - H	0 / 0	-18.5	-18.5	0.01 (4)			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-1-14	-10	-10	---	FRONT	VERT	DEAD	---	C1
E	2-1-14	-30	-30	---	FRONT	VERT	SNOW	---	C1
C	7-9-2	-10	-10	---	FRONT	VERT	DEAD	---	C1
E	7-9-2	-30	-30	---	FRONT	VERT	SNOW	---	C1
I	7-4-12	-386	-386	---	BACK	VERT	TOTAL	---	C1
I	7-10-4	-203	-203	---	TOP	VERT	TOTAL	---	C1
I	7-10-8	-225	-225	---	FRONT	VERT	TOTAL	---	C1
K	2-0-8	-225	-225	---	FRONT	VERT	TOTAL	---	C1
K	2-6-4	-436	-436	---	BACK	VERT	TOTAL	---	C1
M	1-0-12	-3	-3	---	FRONT	VERT	TOTAL	---	C1
N	4-6-4	-307	-307	---	BACK	VERT	TOTAL	---	C1
O	5-4-12	-307	-307	---	BACK	VERT	TOTAL	---	C1
P	8-10-4	-3	-3	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.33")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.33")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.13/1.00 (F-H:1), BC=0.23/1.00 (I-J:1), WB=0.20/1.00 (F-I:1), SS=0.19/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.26 (B) (INPUT = 1.00)



Structural component only
DWG# T-2115929 1/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
412982	T66	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	4.0	1.00	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMVW-p	MT20	4.0	4.0	1.00	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMWW-t	MT20	4.0	9.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

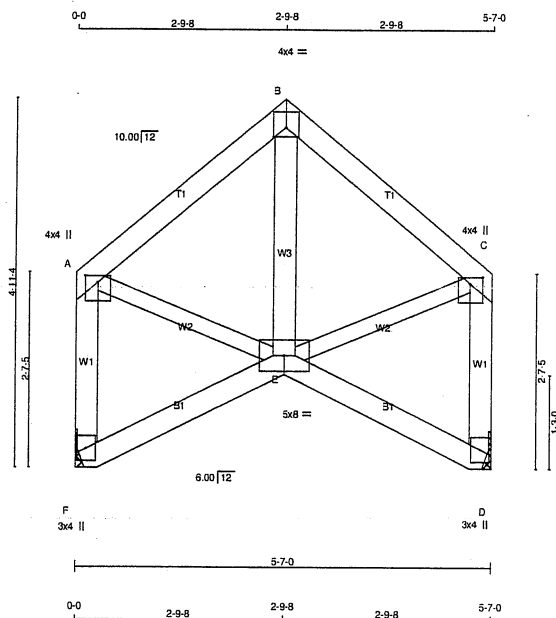


Structural component only
DWG# T-2115929 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
412982	T67S	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:PrpSyr_45RAfF3mJ8Agi49yjqE-cXB28sXg5l4SmRacAPVFr8uyWeXRMvfiCPtaHzCal0



TOTAL WEIGHT = 2 X 29 = 58 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - B	2x4	DRY	No.2
B - C	2x4	DRY	No.2
F - A	2x4	DRY	No.2
D - C	2x4	DRY	No.2
F - E	2x4	DRY	No.2
E - D	2x4	DRY	No.2
ALL WEBS EXCEPT	2x4	DRY	No.2
A - E	2x3	DRY	No.2
E - C	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW+p	MT20	4.0	4.0	1.00 2.00
B	TTW-p	MT20	4.0	4.0	1.50 2.00
C	TMVW+p	MT20	4.0	4.0	1.00 2.00
D	BMV1+p	MT20	3.0	4.0	
E	BBWW+p	MT20	5.0	8.0	
F	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT REQD	
	VERT	HORZ	DOWN	HORZ	BRG	BRG
F	308	0	308	0	0	MECHANICAL
D	308	0	308	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F, D. MINIMUM BEARING LENGTH AT JOINT F = 1-8, JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	218	143 / 0	0 / 0	0 / 0	0 / 0	75 / 0	0 / 0
D	218	143 / 0	0 / 0	0 / 0	0 / 0	75 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS			MAX. UNBRAC LENGTH	MEMB.	WEBS		
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)			MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)
FR-TO		FROM	TO		FR-TO		FROM	TO
A-B	-168 / 0	-91.8	-91.8	0.09 (1)	6.25	E-B	-41 / 26	0.01 (4)
B-C	-168 / 0	-91.8	-91.8	0.09 (1)	6.25	A-E	0 / 137	0.03 (1)
F-A	-282 / 0	0.0	0.0	0.04 (1)	7.81	E-C	0 / 137	0.03 (1)
D-C	-282 / 0	0.0	0.0	0.04 (1)	7.81			
F-E	0 / 0	-18.5	-18.5	0.04 (4)	10.00			
E-D	0 / 0	-18.5	-18.5	0.04 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/999 (0.00")

CSI: TC=0.09/1.00 (A-B:1), BC=0.04/1.00 (D-E:4), WB=0.03/1.00 (A-E:1), SSI=0.08/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

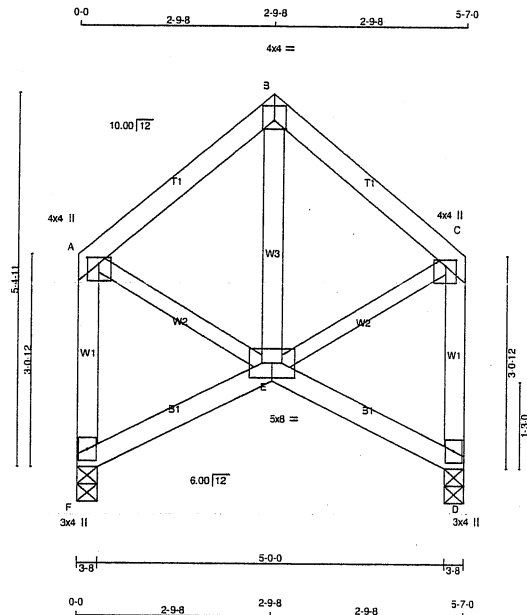
JSI GRIP= 0.23 (A) (INPUT = 0.90)
JSI METAL= 0.08 (D) (INPUT = 1.00)



Structural component only
DWG# T-2115930

JOB NAME 412982	TRUSS NAME T68S	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed May 26 16:46:54 2021 Page 1
ID:PrpSyr_45RAfF3mJ8AgI49yjqE-4jQLCxl3CJOb9p70UOMQ7G2sg5Mxsws8Q7zCal?



Scale: 3/8"=1'

TOTAL WEIGHT = 2 X 31 = 62 lb [M/F]

LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER		
A - B	2x4	DRY	No.2	SPF	
B - C	2x4	DRY	No.2	SPF	
F - A	2x4	DRY	No.2	SPF	
D - C	2x4	DRY	No.2	SPF	
F - E	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF	
A - E	2x3	DRY	No.2	SPF	
E - C	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW+p	MT20	4.0	4.0	1.00 2.00
B	TTW-p	MT20	4.0	4.0	1.50 2.00
C	TMVW+p	MT20	4.0	4.0	1.00 2.00
D	BMV1+p	MT20	3.0	4.0	
E	BBVWW-p	MT20	5.0	8.0	
F	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	UPLIFT
F	308	0	308	0
D	308	0	308	0

UNFACTORED REACTIONS

1ST LCASE	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT							
F	218	143 / 0	0 / 0	0 / 0	0 / 0	75 / 0	0 / 0
D	218	143 / 0	0 / 0	0 / 0	0 / 0	75 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MAX. MEMB. UNBRACED LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX LC1 (LC)
A-B	-148 / 0	-91.8	-91.8 0.09 (1)	6.25	E-B	-67 / 17	0.01 (1)
B-C	-148 / 0	-91.8	-91.8 0.09 (1)	6.25	A-E	0 / 128	0.03 (1)
F-A	-282 / 0	0.0	0.0 0.05 (1)	7.81	E-C	0 / 128	0.03 (1)
D-C	-282 / 0	0.0	0.0 0.05 (1)	7.81			
F-E	0 / 0	-18.5	-18.5 0.04 (4)	10.00			
E-D	0 / 0	-18.5	-18.5 0.04 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/999 (0.00")

CSI: TC=0.09/1.00 (A-B:1), BC=0.04/1.00 (D-E:4),
WB=0.03/1.00 (A-E:1), SSI=0.08/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (C) (INPUT = 0.90)
JSI METAL= 0.08 (D) (INPUT = 1.00)

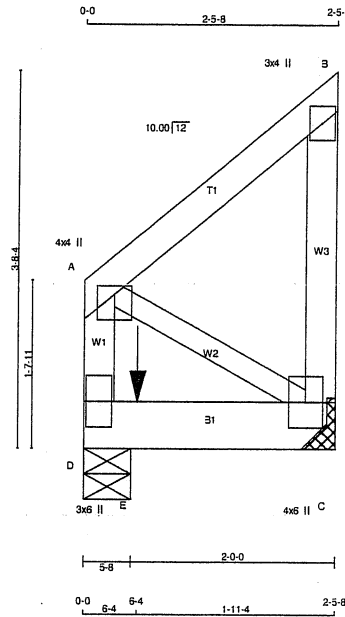


Structural component only
DWG# T-2115931

JOB NAME 412982	TRUSS NAME T69	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed May 26 16:46:55 2021 Page 1
ID:PrpSyr_45RAIF3mJ8AgI49yaiqE-YvJoZYwcnKA0Ij?HqXjxZzJfSBQqpd?9WuzfAzCal



TOTAL WEIGHT = 2 X 15 = 29 lb
(M/F)

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - B	2x4	DRY	No.2	SPF
C - B	2x4	DRY	No.2	SPF
D - A	2x4	DRY	No.2	SPF
D - C	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

DESIGN CONSISTS OF **2** TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-B 1	12	TOP
B-C 1	12	TOP
D-A 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
D-C 2	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TMV+p	MT20	3.0	4.0		
C	BMVW1+p	MT20	4.0	6.0		
D	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
C	244	0	244	0	MECHANICAL	
D	538	0	538	0	5-8	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 4'-0".

UNFACTORED REACTIONS

JT	MAX./MIN. COMPONENT REACTIONS						
	1ST LCASE COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
C	172	117 / 0	0 / 0	0 / 0	0 / 0	54 / 0	0 / 0
D	377	266 / 0	0 / 0	0 / 0	0 / 0	112 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)
FR-TO		FROM TO		FR-TO		FROM TO	
A-B	0 / 0	-91.8 -91.8	0.05 (1)	A-C	0 / 0	0.00 (1)	
C-B	-113 / 0	0.0 0.0	0.01 (1)				
D-A	-113 / 0	0.0 0.0	0.01 (1)				
D-E	0 / 0	-18.5 -18.5	0.07 (1)				
E-C	0 / 0	-18.5 -18.5	0.07 (1)				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	6-4	-357			TOP	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = **24.0** IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.05/1.00 (A-B:1), BC=0.07/1.00 (C-D:1), WB=0.00/1.00 (A-C:1), SSI=0.13/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

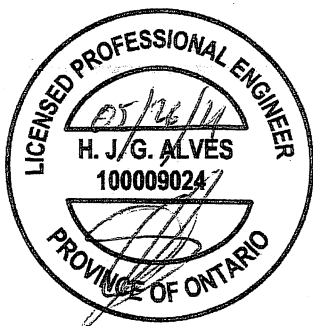
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.04 (A) (INPUT = 0.90)
JSI METAL= 0.02 (B) (INPUT = 1.00)

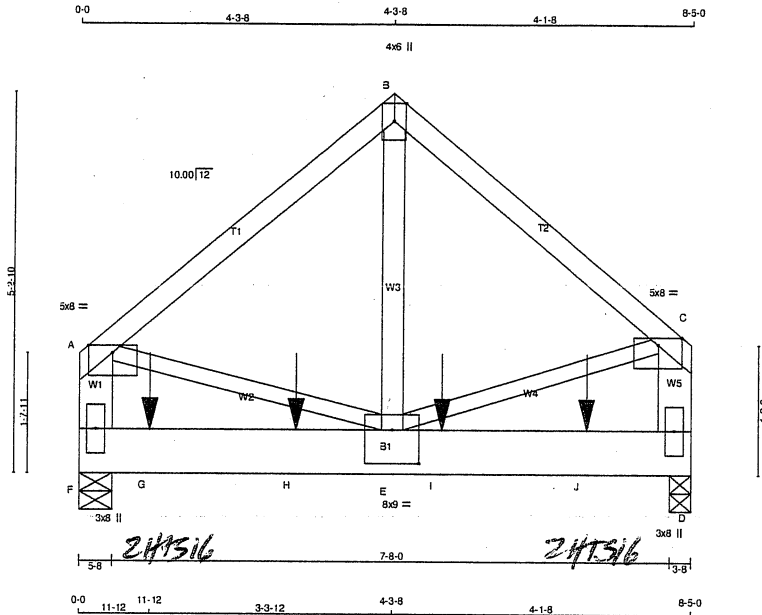


Structural component only
DWG# T-2115932

JOB NAME 412982	TRUSS NAME T510	QUANTITY 1	PLY 3	JOB DESC. ROYAL PINE HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed May 26 17:12:23 2021 Page 1
ID:PrpSyr_45RAfF3mJ8AgI49yajqE-pteSF0SNmrrMKaXgokx2soF0bObLroOvWF1qHbzCaN6



TOTAL WEIGHT = 3 X 48 = 144 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF	
B - C	2x4	DRY	No.2	SPF	
F - A	2x6	DRY	No.2	SPF	
D - C	2x6	DRY	No.2	SPF	
F - D	2x8	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
E - B	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - B	1	12
B - C	1	12
F - A	2	12
D - C	2	12
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F - D	2	4
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	1	6
2x4	1	6

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQD
JT	VERT	HORZ	DOWN	UPLIFT
F	7913	0	7913	202
D	7162	0	7162	0

PROVIDE ANCHORAGE AT BEARING JOINT F FOR 2286 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 2068 LBS. FACTORED UPLIFT

PROVIDE FOR 202 LBS. FACTORED HORIZONTAL REACTION AT JOINT F

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	5848	3454 / 0	1041 / 0	0 / 0	193 / -2284	1353 / 0	0 / 0
D	5293	3126 / 0	942 / 0	0 / 0	181 / -2066	1225 / 0	0 / 0

HORIZONTAL REACTIONS

F	---	0 / 0	0 / 0	0 / 0	144 / -142	0 / 0	0 / 0
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BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, D

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 4.81 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (18)

CHORDS	MAX. FACTORED	FACTORED	MAX.	MEMB.	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MAX. (PLF)	FORCE (LBS)	MAX. (PLF)
FR-TO					
A - B	-5480 / 1642	-115.2	-115.2	0.21 (2)	4.81
B - C	-5480 / 1646	-115.2	-115.2	0.19 (3)	4.82
F - A	-4883 / 1431	0.0	0.0	0.11 (1)	7.71
D - C	-5060 / 1484	0.0	0.0	0.11 (1)	7.61
F - G	-181 / 189	-39.5	-39.5	0.49 (3)	6.25
G - H	-181 / 189	-39.5	-39.5	0.49 (3)	6.25
H - E	-181 / 189	-39.5	-39.5	0.49 (3)	6.25
E - I	-11 / 24	-39.5	-39.5	0.49 (3)	6.25
I - J	-11 / 24	-39.5	-39.5	0.49 (3)	6.25
J - D	-11 / 24	-39.5	-39.5	0.49 (3)	6.25

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	11-12	-2544	-2544	566	BACK	VERT	TOTAL	---	C1
H	2-11-12	-2544	-2544	566	BACK	VERT	TOTAL	---	C1
I	4-11-12	-2544	-2544	566	BACK	VERT	TOTAL	---	C1
J	6-11-12	-2544	-2544	566	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.2} PSF AT {30-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_{pe}, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}. INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 33.4	PSF
DL = 6.0	PSF	
BOT CH.	LL = 10.5	PSF
DL = 7.4	PSF	
TOTAL LOAD = 57.3	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF BCBC 2018, ABC 2019
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR NOT USED
- PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED.

(80 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 33.4 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.28")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/180 (0.56")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.21/1.00 (A-B-2), BC=0.49/1.00 (D-E-3),
WB=0.35/1.00 (B-E-3), SS=0.88/1.00 (D-E-3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371 1747 788 1987 1873	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (E) (INPUT = 0.90)
JSI METAL= 0.43 (B) (INPUT = 1.00)



Structural component only
DWG# T-2115933

CONTINUED ON PAGE 2

JOB NAME 412982	TRUSS NAME T510	QUANTITY 1	PLY 3	JOB DESC. ROYAL PINE HOMES TRUSS DESC.	DRWG NO.
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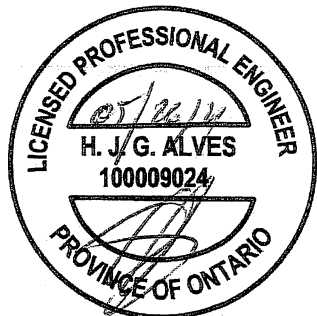
Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed May 26 17:12:23 2021 Page 2
ID:PrpSyr 45RAfF3mJ8Agi49vajqE-pteSF0SNmrrMKaXgokx2soFObObLroOvWF1qHbzCaN6

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	8.0	Edge	
B	TTW+p	MT20	4.0	6.0	Edge	
C	TMVW-p	MT20	5.0	8.0	Edge	
D	BMV1+p	MT20	3.0	8.0		
E	BMVWW-t	MT20	8.0	9.0	5.50	4.50
F	BMV1+p	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

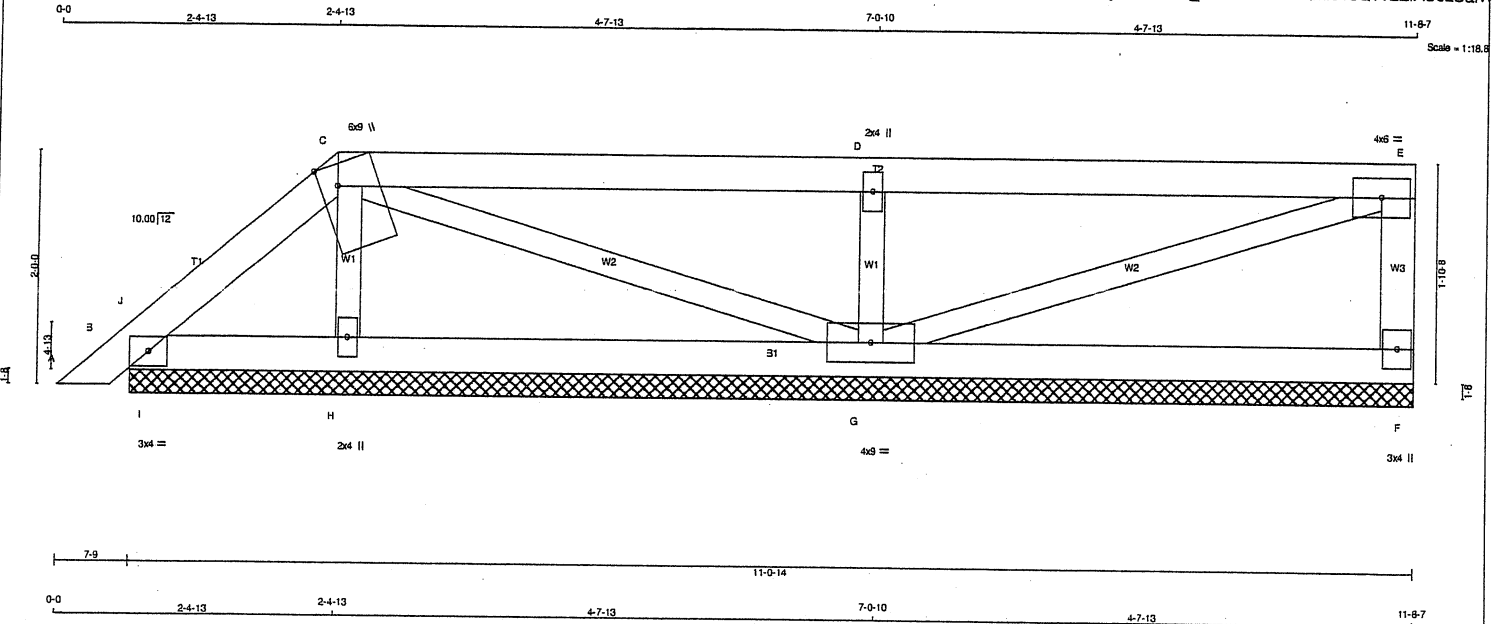


Structural component only
DWG# T-2115933 *211*

JOB NAME 412982	TRUSS NAME PB01	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed May 26 16:46:21 2021 Page 1
ID:wTM7iolSByniqocTeDgf3Pyb9F8-YNFyQvYbdw2_xx7TihwIYPZQmb?SQ??LEfAS8zCalW



LUMBER				
N. L. G. A. RULES				
CHORDS		SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS 2x3 DRY			No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-i	MT20	3.0	4.0	
C	TTWW+m	MT20	6.0	9.0	Edge 1.75
D	TMW+w	MT20	2.0	4.0	
E	TMVW-t	MT20	4.0	6.0	
F	BMV1+p	MT20	3.0	4.0	
G	BMVWW1-t	MT20	4.0	9.0	
H	BMV1+w	MT20	2.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	VERT	GROSS REACTION	HORZ	DOWN	HORZ	BRG	IN-SX	BRG	IN-SX
F	197	0	197	0	0	11-0-14	11-0-14		
B	170	0	170	0	0	11-0-14	11-0-14		
H	263	0	263	0	0	11-0-14	11-0-14		
G	641	0	641	0	0	11-0-14	11-0-14		

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	139	91/0	0/0	0/0	0/0	48/0	0/0
B	117	91/0	0/0	0/0	0/0	26/0	0/0
H	188	110/0	0/0	0/0	0/0	78/0	0/0
G	452	302/0	0/0	0/0	0/0	151/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0/14	-91.8 -91.8	0.02 (1)	10.00	H-C	-181/0	0.03 (1)
B-J	-32/0	-91.8 -91.8	0.01 (1)	6.25	C-G	-28/0	0.01 (1)
J-C	-54/0	-91.8 -91.8	0.03 (1)	6.25	G-D	-533/0	0.08 (1)
C-D	0/0	-91.8 -91.8	0.34 (1)	10.00	D-E	0/0	0.00 (1)
D-E	0/0	-91.8 -91.8	0.34 (1)	10.00	E-F	-112/0	0.00 (1)
F-E	-162/0	0.0 0.0	0.02 (1)	7.81			
B-I	0/38	-18.5 -18.5	0.04 (1)	10.00			
I-H	0/38	-18.5 -18.5	0.05 (4)	10.00			
H-G	0/26	-18.5 -18.5	0.10 (4)	10.00			
G-F	0/0	-18.5 -18.5	0.10 (4)	10.00			

TOTAL WEIGHT = 2 X 38 = 75 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.34/1.00 (C-D:1), BC=0.10/1.00 (G-H:4), WB=0.08/1.00 (D-G:1), SSI=0.21/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

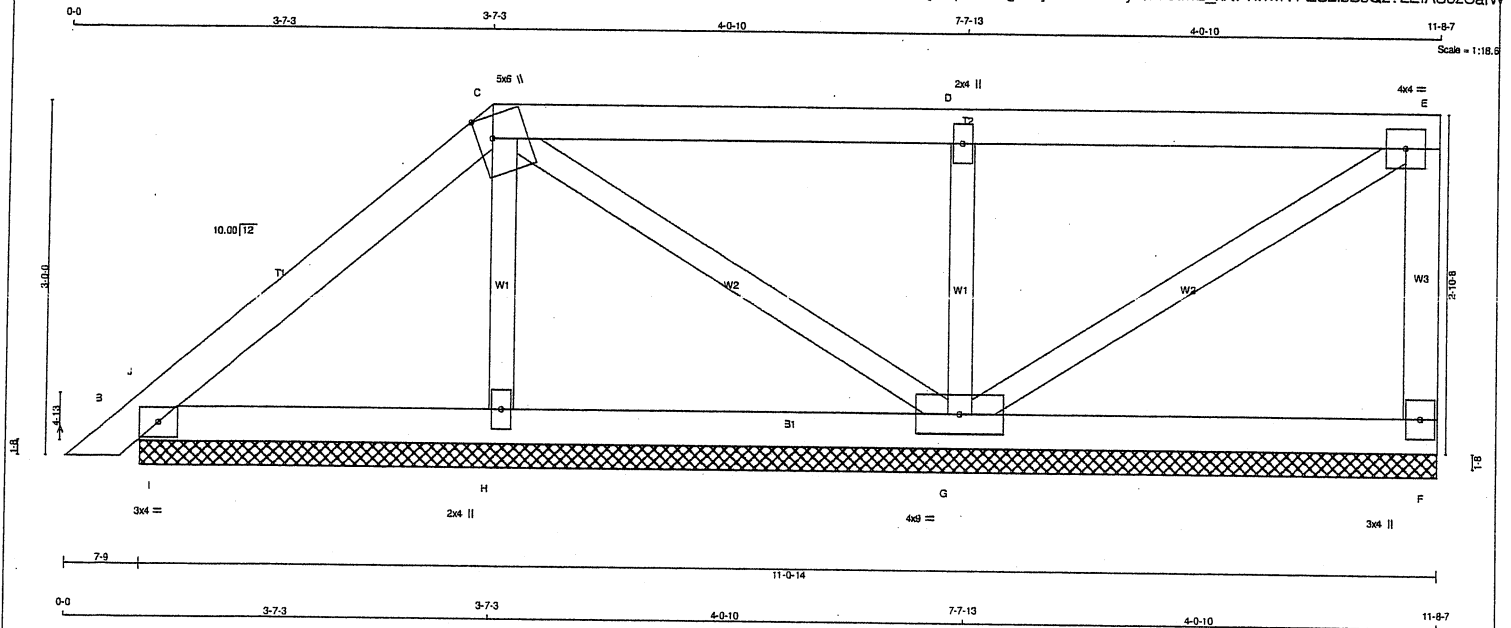
JSI GRIP= 0.27 (D) (INPUT = 0.90)
JSI METAL= 0.11 (D) (INPUT = 1.00)



Structural component only
DWG# T-2115894

JOB NAME 412982	TRUSS NAME PB02	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID:wTM7ioISByniqocTeDgr3Pyb9F8-YNFyQvYbdw2_xX7TThwiYPZS2IbBsQz?LEfAS8zCaV



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
F - H	2x4	DRY	No.2
B - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	
C	TTWW+m	MT20	5.0	6.0	2.25 1.50
D	TMW+w	MT20	2.0	4.0	
E	TMVW-t	MT20	4.0	4.0	
F	BMV1+p	MT20	3.0	4.0	
G	BMVWW1-t	MT20	4.0	9.0	
H	BMV1+w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	HORZ	BRG	IN-SX	BRG	IN-SX
F	172	0	172	0	0	11-0-14	11-0-14		
B	257	0	257	0	0	11-0-14	11-0-14		
H	260	0	260	0	0	11-0-14	11-0-14		
G	582	0	582	0	0	11-0-14	11-0-14		

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	
F	122	79 / 0	0 / 0	0 / 0	0 / 0	43 / 0	0 / 0	
B	180	130 / 0	0 / 0	0 / 0	0 / 0	50 / 0	0 / 0	
H	186	109 / 0	0 / 0	0 / 0	0 / 0	77 / 0	0 / 0	
G	410	277 / 0	0 / 0	0 / 0	0 / 0	133 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 14	-91.8 -91.8	0.02 (1)	H-C	-163 / 0	0.03 (1)	
B-J	-20 / 0	-91.8 -91.8	0.04 (1)	C-G	-69 / 0	0.02 (1)	
J-C	-88 / 0	-91.8 -91.8	0.10 (1)	G-D	-464 / 0	0.08 (1)	
C-D	0 / 3	-91.8 -91.8	0.25 (1)	D-E	-3 / 0	0.00 (1)	
D-E	0 / 2	-91.8 -91.8	0.25 (1)	E-F	-249 / 0	0.00 (1)	
E-F	-141 / 0	0.0 0.0	0.02 (1)				
B-I	0 / 62	-18.5 -18.5	0.09 (1)				
I-H	0 / 62	-18.5 -18.5	0.09 (1)				
H-G	0 / 56	-18.5 -18.5	0.08 (4)				
G-F	0 / 0	-18.5 -18.5	0.07 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(5% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.25/1.00 (C-D:1), BC=0.09/1.00 (B-I:1), WB=0.08/1.00 (D-G:1), SSI=0.19/1.00 (B-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

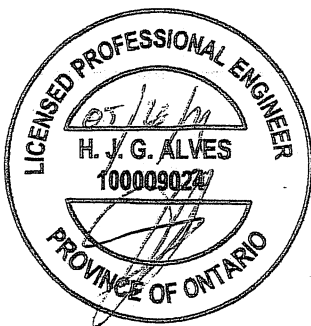
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (D) (INPUT = 0.90)
JSI METAL= 0.10 (D) (INPUT = 1.00)

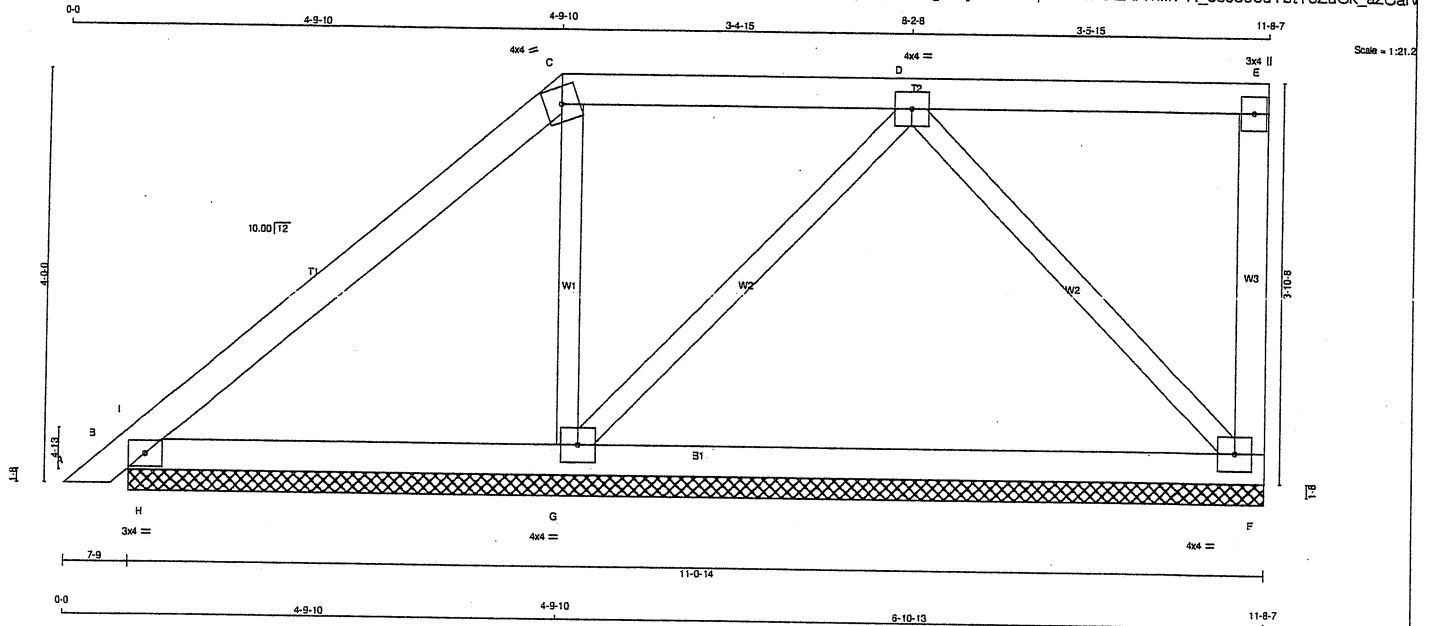


Structural component only
DWG# T-2115895

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
412982	PB03	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER				DESCR.	
N. L. G. A. RULES	SIZE	LUMBER		SPF	
CHORDS				SPF	
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
F - E	2x4	DRY	No.2	SPF	
B - F	2x4	DRY	No.2	SPF	
ALL WEBS 2x3 DRY				SPF	
DRY: SEASONED LUMBER.					

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	IN-SX	BRG	IN-SX
F	391	0	391	0	0	11-0-14	11-0-14		
B	311	0	311	0	0	11-0-14	11-0-14		
G	568	0	568	0	0	11-0-14	11-0-14		

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	275	190 / 0	0 / 0	0 / 0	0 / 0	85 / 0	0 / 0
B	216	162 / 0	0 / 0	0 / 0	0 / 0	54 / 0	0 / 0
G	406	242 / 0	0 / 0	0 / 0	0 / 0	163 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX LC1 (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 14	-91.8	-91.8 0.02 (1)	10.00	G-C	-224 / 0	0.05 (1)
B-I	0 / 144	-91.8	-91.8 0.13 (1)	10.00	G-D	-233 / 0	0.09 (1)
I-C	-97 / 0	-91.8	-91.8 0.19 (1)	6.25	D-F	-309 / 0	0.12 (1)
C-D	-57 / 0	-91.8	-91.8 0.18 (1)	6.25	H-I	-519 / 0	0.00 (1)
D-E	0 / 0	-91.8	-91.8 0.18 (1)	10.00			
F-E	-122 / 0	0.0	0.0 0.03 (1)	7.81			
B-H	0 / 64	-18.5	-18.5 0.18 (1)	10.00			
H-G	0 / 64	-18.5	-18.5 0.20 (4)	10.00			
G-F	0 / 216	-18.5	-18.5 0.21 (4)	10.00			

TOTAL WEIGHT = 2 X 42 = 83 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.19/1.00 (C-I), BC=0.21/1.00 (F-G:4), WB=0.12/1.00 (D-F:1), SSI=0.39/1.00 (B-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

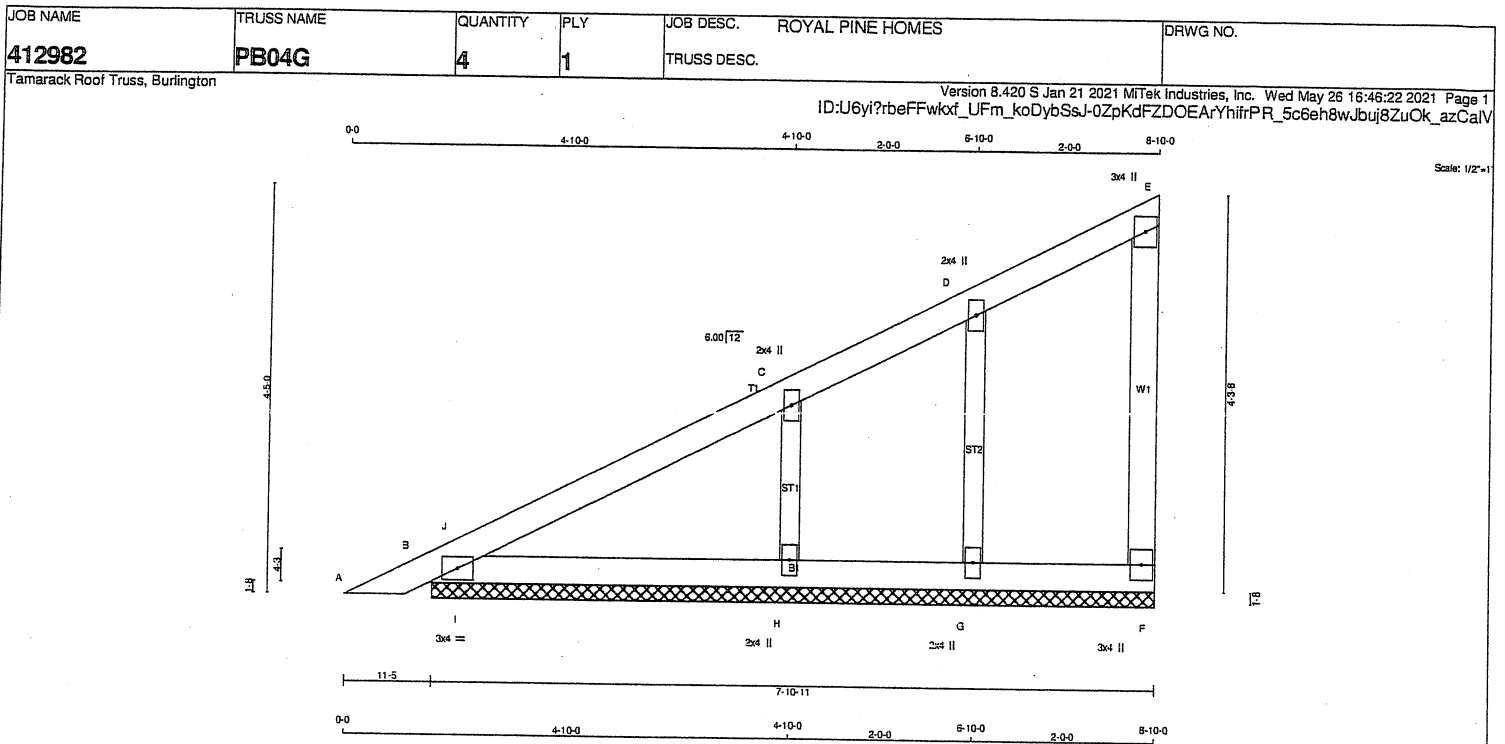
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.26 (B) (INPUT = 0.90)
JSI METAL= 0.07 (F) (INPUT = 1.00)



Structural component only
DWG# T-2115896



LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER		DESCR.
A - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3		DRY	No.2
DRY: SEASONED LUMBER.				SPF

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TMW+w	MT20	2.0	4.0		
D	TMW+w	MT20	2.0	4.0		
E	TMV+p	MT20	3.0	4.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMW1+w	MT20	2.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. VERT. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. VERT. LOAD (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 17	-91.8	-91.8 0.05 (1)	G-D	-158 / 0	0.03 (1)	
B-J	-17 / 0	-91.8	-91.8 0.02 (4)	H-C	-307 / 0	0.05 (1)	
J-C	-4 / 1	-91.8	-91.8 0.13 (1)	I-J	-172 / 5	0.00 (1)	
C-D	-21 / 0	-91.8	-91.8 0.13 (1)				
D-E	-5 / 0	-91.8	-91.8 0.05 (1)				
F-E	-84 / 0	0.0	0.0 0.02 (1)				
B-I	0 / 14	-18.5	-18.5 0.10 (1)				
I-H	0 / 14	-18.5	-18.5 0.10 (1)				
H-G	0 / 4	-18.5	-18.5 0.07 (1)				
G-F	0 / 0	-18.5	-18.5 0.02 (4)				

TOTAL WEIGHT = 4 X 28 = 112 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13/1.00 (C-D:1), BC=0.10/1.00 (B-I:1), WB=0.05/1.00 (C-H:1), SSI=0.14/1.00 (B-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (B) (INPUT = 0.90)
JSI METAL= 0.13 (C) (INPUT = 1.00)

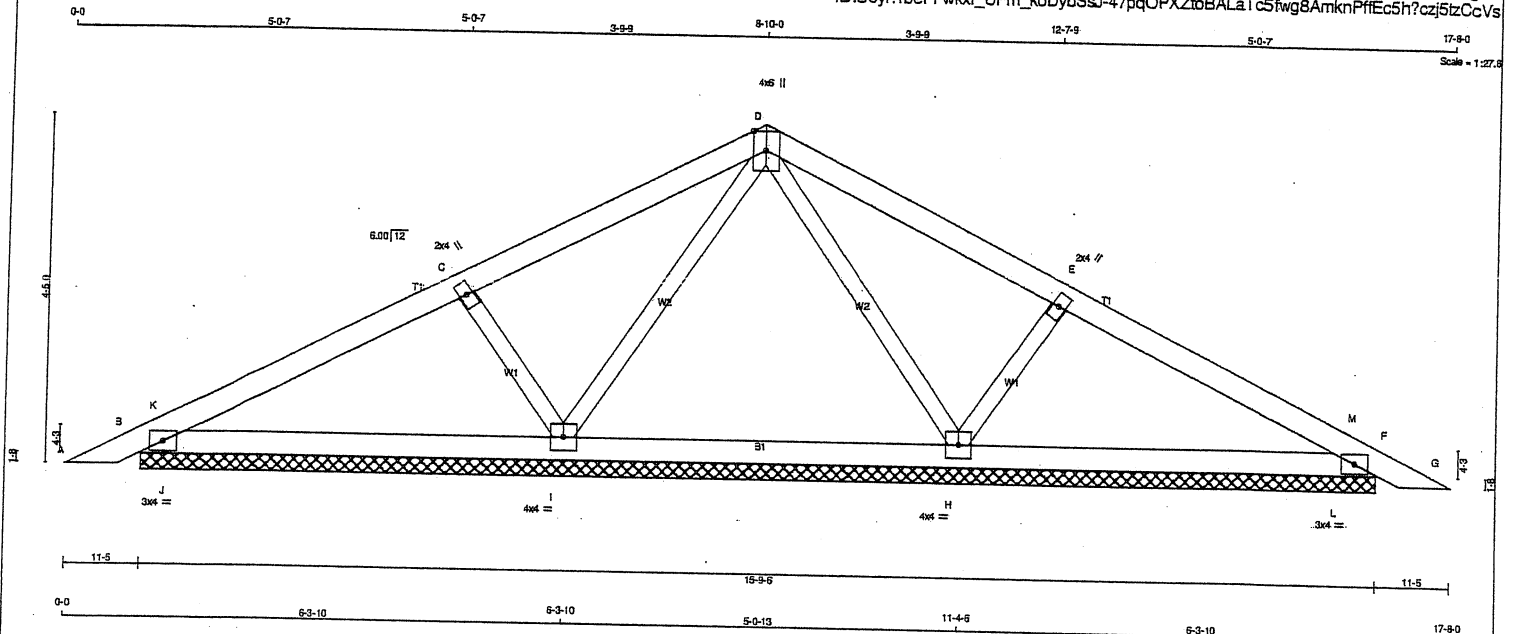


Structural component only
DWG# T-2115897

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
413226	PB06	17	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER			
N. L. & A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
B - F	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	
C	TMW+w	MT20	2.0	4.0	
D	TTWW+p	MT20	4.0	6.0	Edge
E	TMW+w	MT20	2.0	4.0	
F	TMB1-I	MT20	3.0	4.0	
H	BMWW1-t	MT20	4.0	4.0	
I	BMWW1-t	MT20	4.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
B	340	0	340	0	15-9-6	15-9-6
F	340	0	340	0	15-9-6	15-9-6
H	610	0	610	0	15-9-6	15-9-6
I	610	0	610	0	15-9-6	15-9-6

UNFACTORED REACTIONS							
1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	239	166 / 0	0 / 0	0 / 0	0 / 0	73 / 0	0 / 0
F	239	166 / 0	0 / 0	0 / 0	0 / 0	73 / 0	0 / 0
H	432	283 / 0	0 / 0	0 / 0	0 / 0	149 / 0	0 / 0
I	432	283 / 0	0 / 0	0 / 0	0 / 0	149 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, H, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO	UNBRAC LENGTH	FR-TO			
A-B	0 / 17	-91.8 -91.8 0.05 (1)	10.00	D-H	-197 / 0	0.07 (1)	
B-K	-199 / 0	-91.8 -91.8 0.06 (4)	6.25	H-E	-396 / 0	0.07 (1)	
K-C	-143 / 0	-91.8 -91.8 0.23 (1)	6.25	I-D	-197 / 0	0.07 (1)	
C-D	0 / 54	-91.8 -91.8 0.24 (1)	10.00	C-I	-396 / 0	0.07 (1)	
D-E	0 / 54	-91.8 -91.8 0.24 (1)	10.00	J-K	-69 / 58	0.00 (1)	
E-M	-143 / 0	-91.8 -91.8 0.23 (1)	6.25	L-M	-69 / 58	0.00 (1)	
M-F	-199 / 0	-91.8 -91.8 0.06 (4)	6.25				
F-G	0 / 17	-91.8 -91.8 0.05 (1)	10.00				
B-J	0 / 149	-18.5 -18.5 0.08 (1)	10.00				
J-I	0 / 149	-18.5 -18.5 0.12 (4)	10.00				
I-H	0 / 40	-18.5 -18.5 0.11 (4)	10.00				
H-L	0 / 149	-18.5 -18.5 0.12 (4)	10.00				
L-F	0 / 149	-18.5 -18.5 0.08 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.24/1.00 (C-D:1), BC=0.12/1.00 (I-J:4), WB=0.07/1.00 (D-I:1), SS=0.15/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

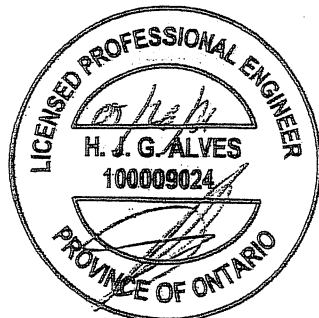
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

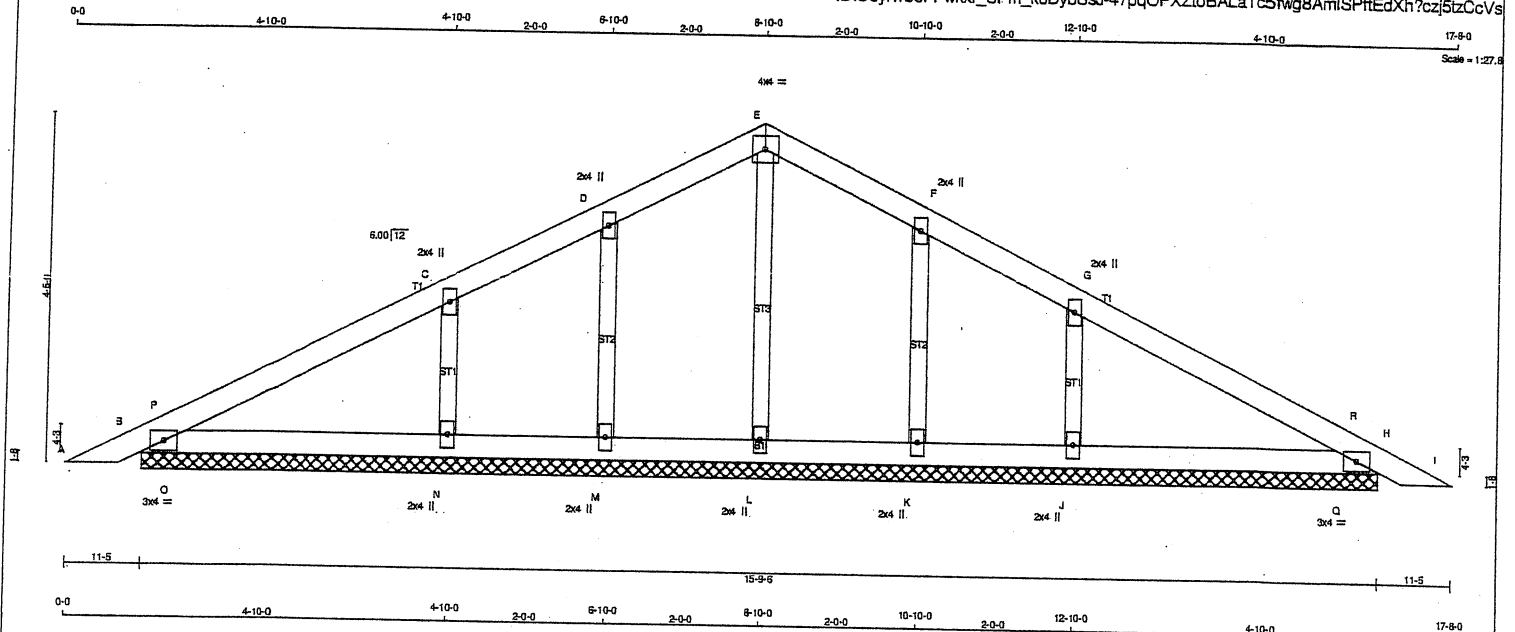
JSI GRIP= 0.24 (F) (INPUT = 0.90)
JSI METAL= 0.08 (C) (INPUT = 1.00)



Structural component only
DWG# T-2115839

JOB NAME 413226	TRUSS NAME PB06G	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - E	2x4	DRY	No.2
E - I	2x4	DRY	No.2
B - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			
GABLE STUDS SPACED AT 2-0-0 OC.			

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B TMB1-H	MT20	3.0	4.0	
C, D, F, G				
C TMW-w	MT20	2.0	4.0	
E TTW-p	MT20	4.0	4.0	
H TMB1-H	MT20	3.0	4.0	
J, K, L, M, N				
J BMW1-w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 17	-91.8 -91.8	0.05 (1)	L-E	-131 / 0	0.04 (1)	
B-P	-44 / 0	-91.8 -91.8	0.02 (4)	M-D	-160 / 0	0.03 (1)	
P-C	-44 / 0	-91.8 -91.8	0.13 (1)	N-C	-306 / 0	0.05 (1)	
C-D	-60 / 0	-91.8 -91.8	0.13 (1)	K-F	-160 / 0	0.03 (1)	
D-E	-45 / 0	-91.8 -91.8	0.05 (1)	J-G	-306 / 0	0.05 (1)	
E-F	-45 / 0	-91.8 -91.8	0.05 (1)	O-P	-171 / 5	0.00 (1)	
F-G	-60 / 0	-91.8 -91.8	0.13 (1)	Q-R	-171 / 5	0.00 (1)	
G-R	-44 / 0	-91.8 -91.8	0.13 (1)				
H-H	-44 / 0	-91.8 -91.8	0.02 (4)				
H-I	0 / 17	-91.8 -91.8	0.05 (1)				
B-O	0 / 49	-18.5 -18.5	0.10 (1)				
O-N	0 / 49	-18.5 -18.5	0.10 (1)				
N-M	0 / 39	-18.5 -18.5	0.07 (1)				
M-L	0 / 36	-18.5 -18.5	0.02 (4)				
L-K	0 / 36	-18.5 -18.5	0.02 (4)				
K-J	0 / 39	-18.5 -18.5	0.07 (1)				
J-Q	0 / 49	-18.5 -18.5	0.10 (1)				
Q-H	0 / 49	-18.5 -18.5	0.10 (1)				

TOTAL WEIGHT = 2 X 51 = 102 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BOBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13/1.00 (C-P:1), BC=0.10/1.00 (B-O:1), WB=0.05/1.00 (C-N:1), SSI=0.14/1.00 (H-Q:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
	MAX	MIN	MAX MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

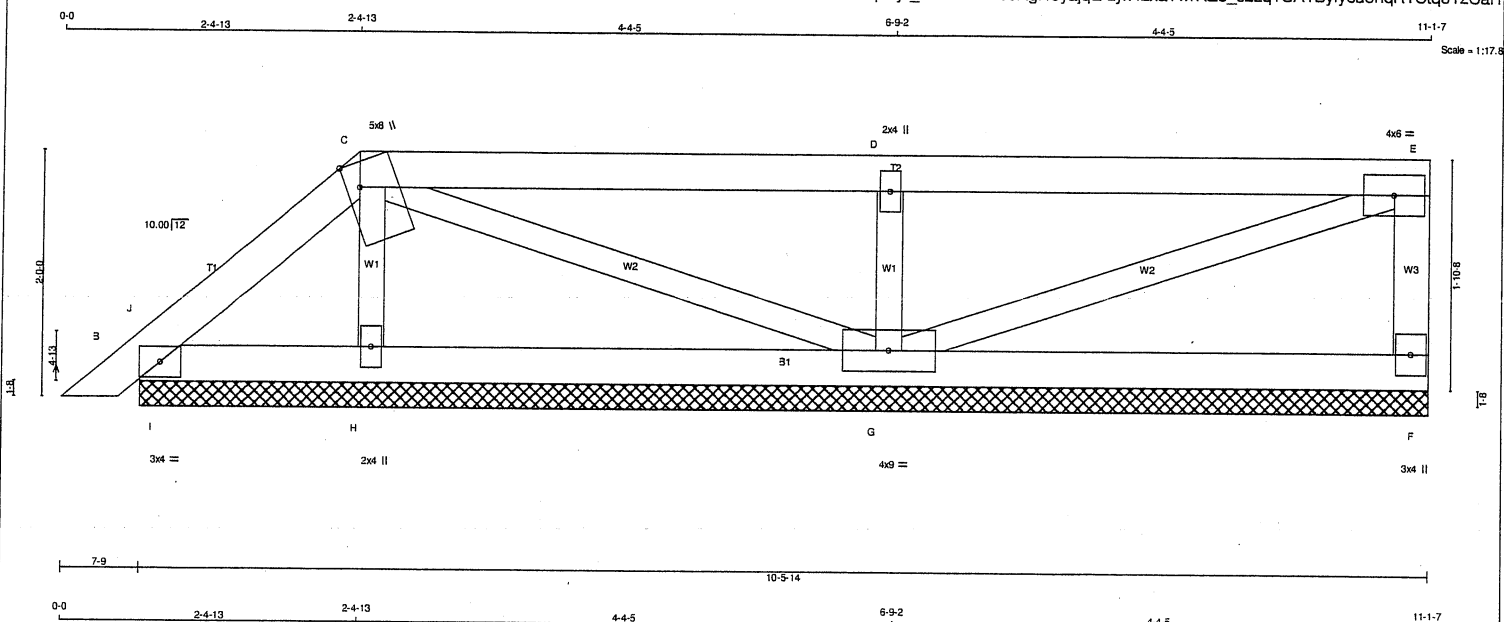
JSI GRIP= 0.21 (H) (INPUT = 0.90)
JSI METAL= 0.13 (G) (INPUT = 1.00)



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
412982	PB51	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed May 26 16:46:24 2021 Page 1
ID:PrpSyr_45RAIf3mJ8Agi49yjqE-zyx42xaTwrZ0_s2zqTSA1Byfycu3nqR1Ctq3TzCaIT



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
F - H	2x4	DRY	No.2
B - F	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	
C	TTWW+m	MT20	5.0	8.0	Edge 1.25
D	TMW+w	MT20	2.0	4.0	
E	TMVW-t	MT20	4.0	6.0	
F	BMV1+p	MT20	3.0	4.0	
G	BMWWV1-t	MT20	4.0	9.0	
H	BMW1+w	MT20	2.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
F	184	0	184	0	10-5-14	10-5-14
B	170	0	170	0	10-5-14	10-5-14
H	249	0	249	0	10-5-14	10-5-14
G	602	0	602	0	10-5-14	10-5-14

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	131	85 / 0	0 / 0	0 / 0	0 / 0	45 / 0	0 / 0
B	118	91 / 0	0 / 0	0 / 0	0 / 0	28 / 0	0 / 0
H	178	105 / 0	0 / 0	0 / 0	0 / 0	73 / 0	0 / 0
G	425	283 / 0	0 / 0	0 / 0	0 / 0	141 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE	FACTORED	MAX.	MEMB.	FORCE	FACTORED	MAX.
	(LBS)	VERT. LOAD	LC1		(LBS)	VERT. LOAD	LC1
		(PLF)	CSI (LC)			(PLF)	CSI (LC)
FR-TO		FROM TO		FR-TO			
A-B	0 / 14	-91.8	-91.8	0.02 (1)	10.00	H-C	-171 / 0
B-J	-33 / 0	-91.8	-91.8	0.01 (1)	6.25	C-G	-29 / 0
J-C	-53 / 0	-91.8	-91.8	0.03 (1)	6.25	G-D	-500 / 0
C-D	0 / 1	-91.8	-91.8	0.30 (1)	10.00	G-E	0 / 0
D-E	0 / 0	-91.8	-91.8	0.30 (1)	10.00	I-J	-109 / 0
F-E	-152 / 0	0.0	0.0	0.02 (1)	7.81		
B-I	0 / 38	-18.5	-18.5	0.04 (1)	10.00		
I-H	0 / 38	-18.5	-18.5	0.04 (4)	10.00		
H-G	0 / 27	-18.5	-18.5	0.09 (4)	10.00		
G-F	0 / 0	-18.5	-18.5	0.09 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD = 39.0 PSF				

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.30/1.00 (C-D:1), BC=0.09/1.00 (G-H:4), WB=0.07/1.00 (D-G:1), SS=0.20/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX MIN
MT20	650	371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (D) (INPUT = 0.90)

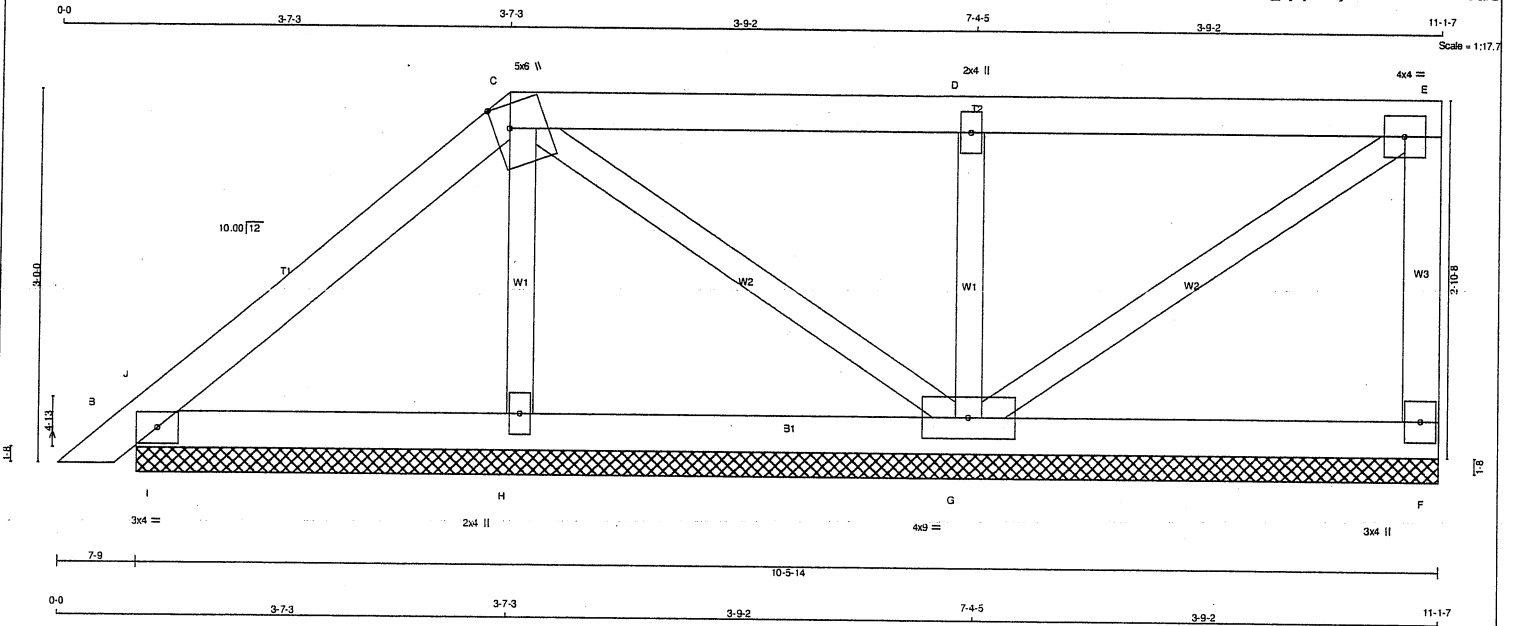
JSI METAL= 0.10 (D) (INPUT = 1.00)



Structural component only
DWG# T-2115898

JOB NAME 412982	TRUSS NAME PB52	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:PrpSyr_45RAIf3mJ8AgI49yajqE-R8VSGHb6h9ZQP8REXX_hjFj7cMy8oD2bFsdObvzCaIS



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
F - H	2x4	DRY	No.2
B - F	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-t	MT20	3.0	4.0	
C	TTWW+m	MT20	5.0	6.0	2.25 1.50
D	TMW+w	MT20	2.0	4.0	
E	TMW-t	MT20	4.0	4.0	
F	BMV1+p	MT20	3.0	4.0	
G	BMWWV1-t	MT20	4.0	9.0	
H	BMW1+w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX
F	159	0	159	0	10-5-14	10-5-14
B	256	0	256	0	10-5-14	10-5-14
H	246	0	246	0	10-5-14	10-5-14
G	544	0	544	0	10-5-14	10-5-14

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS				WIND	DEAD	SOIL
	SNOW	LIVE	PERM.	LIVE	WIND	DEAD			
F	113	73 / 0	0 / 0	0 / 0	0 / 0	40 / 0	0 / 0	0 / 0	0 / 0
B	179	129 / 0	0 / 0	0 / 0	0 / 0	50 / 0	0 / 0	0 / 0	0 / 0
H	176	103 / 0	0 / 0	0 / 0	0 / 0	73 / 0	0 / 0	0 / 0	0 / 0
G	383	259 / 0	0 / 0	0 / 0	0 / 0	124 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 14	-91.8 -91.8	0.02 (1)	10.00	H-C	-151 / 0	0.03 (1)
B-J	-21 / 0	-91.8 -91.8	0.04 (1)	6.25	C-G	-70 / 0	0.02 (1)
J-C	-87 / 0	-91.8 -91.8	0.10 (1)	6.25	G-D	-430 / 0	0.07 (1)
C-D	0 / 3	-91.8 -91.8	0.22 (1)	10.00	G-E	-3 / 0	0.00 (1)
D-E	0 / 3	-91.8 -91.8	0.22 (1)	10.00	I-J	-248 / 0	0.00 (1)
F-E	-130 / 0	0.0 0.0	0.02 (1)	7.81			
B-I	0 / 61	-18.5 -18.5	0.09 (1)	10.00			
I-H	0 / 61	-18.5 -18.5	0.09 (1)	10.00			
H-G	0 / 55	-18.5 -18.5	0.07 (4)	10.00			
G-F	0 / 0	-18.5 -18.5	0.06 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.22/1.00 (C-D:1), BC=0.09/1.00 (B-I:1), WB=0.07/1.00 (D-G:1), SS=0.19/1.00 (B-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

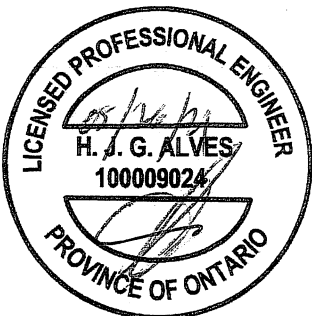
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

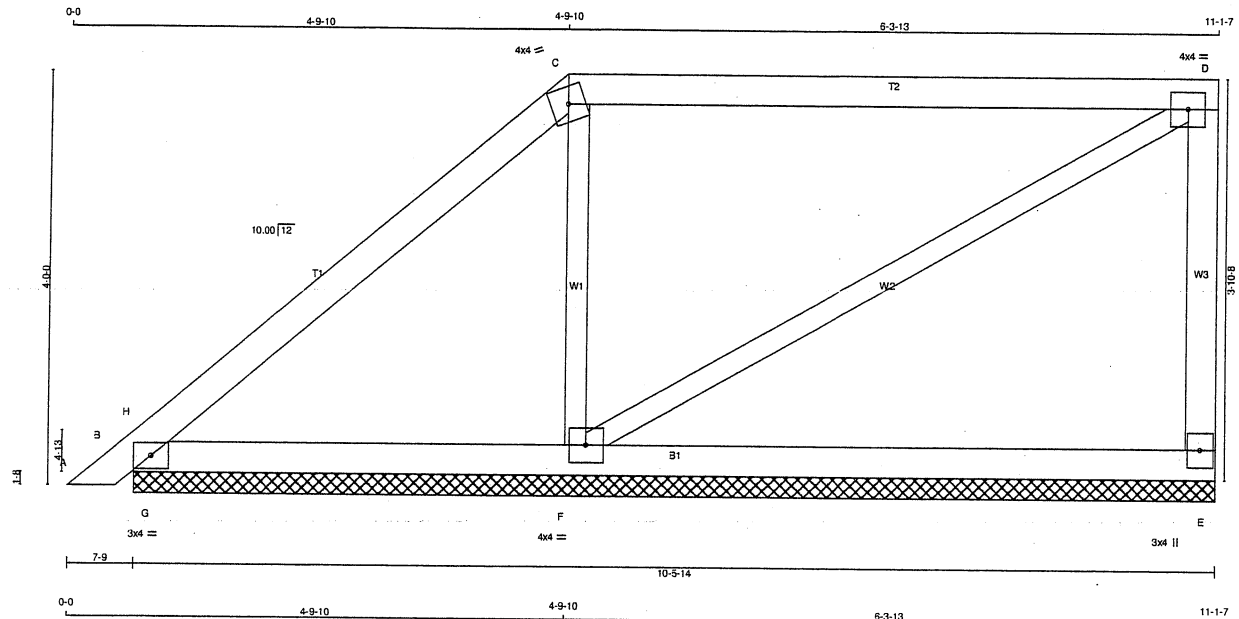
JSI GRIP= 0.21 (D) (INPUT = 0.90)
JSI METAL= 0.09 (D) (INPUT = 1.00)



Structural component only
DWG# T-2115899

JOB NAME 412982	TRUSS NAME PB53	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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TOTAL WEIGHT = 2 X 38 = 76 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4 DRY	No.2	SPF
C - D	2x4 DRY	No.2	SPF
E - D	2x4 DRY	No.2	SPF
B - E	2x4 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF
DRY: SEASONED LUMBER.			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW-m	MT20	4.0	4.0		
D	TMVW-t	MT20	4.0	4.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMWW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
	VERT	DOWN	IN-SX	IN-SX
E	369	0	10-5-14	10-5-14
B	323	0	10-5-14	10-5-14
F	514	0	10-5-14	10-5-14

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	259	179 / 0	0 / 0	0 / 0	0 / 0	80 / 0	0 / 0
B	225	167 / 0	0 / 0	0 / 0	0 / 0	59 / 0	0 / 0
F	367	218 / 0	0 / 0	0 / 0	0 / 0	149 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 14	-91.8	-91.8 0.02 (1)	10.00	F-C	-387 / 0	0.09 (1)
B-H	0 / 122	-91.8	-91.8 0.12 (1)	10.00	F-D	0 / 74	0.02 (1)
H-C	-111 / 0	-91.8	-91.8 0.19 (1)	6.25	G-H	-508 / 0	0.00 (1)
C-D	-64 / 0	-91.8	-91.8 0.63 (1)	6.25			
E-D	-327 / 0	0.0	0.0 0.08 (1)	7.81			
B-G	0 / 76	-18.5	-18.5 0.18 (1)	10.00			
G-F	0 / 76	-18.5	-18.5 0.18 (4)	10.00			
F-E	0 / 0	-18.5	-18.5 0.17 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
DL = 6.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.4 PSF	
TOTAL LOAD = 39.0 PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCO 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.63/1.00 (C-D:1), BC=0.18/1.00 (F-G:4), WB=0.09/1.00 (C-F:1), SSI=0.39/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	650 371	1747 788	1967 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

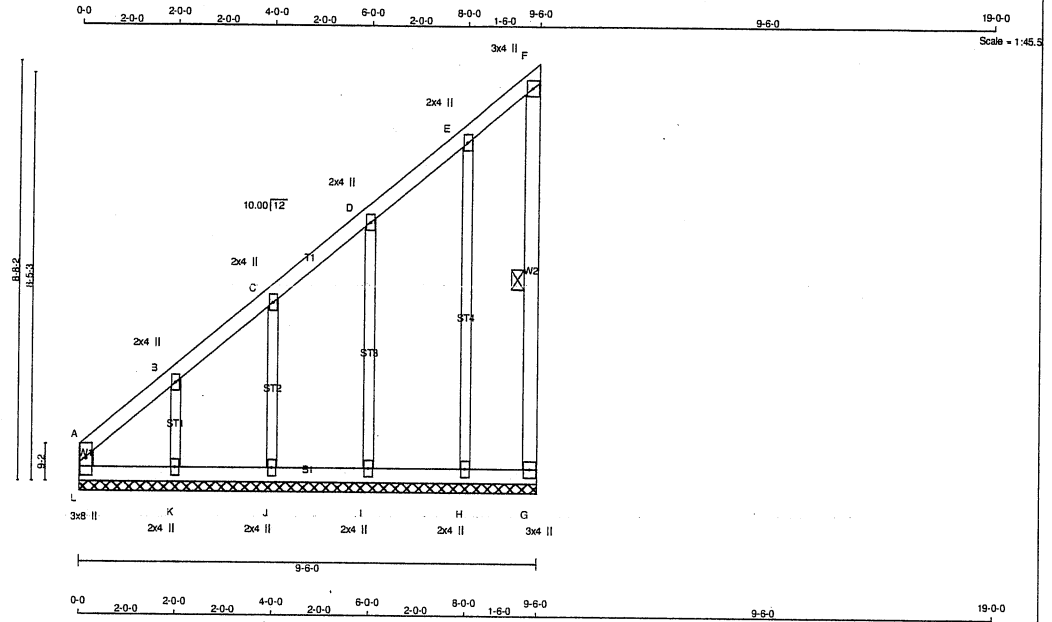
JSI GRIP = 0.26 (B) (INPUT = 0.90)
JSI METAL = 0.06 (B) (INPUT = 1.00)



Structural component only
DWG# T-2115900

JOB NAME 412982	TRUSS NAME V01	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:PrpSyr_45RArF3mJ8Agi49yajqE-d04jaZ2gq5GDSDlIhcmhOQ3BB74FwA?dpyT2CuzCaIT



LUMBER				DESCR.
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	
L - A	2x4	DRY	No.2	SPF
A - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A					
B, C, D, E					
B	TMW+w	MT20	2.0	4.0	
F	TMV+p	MT20	3.0	4.0	
G	BMV1+p	MT20	3.0	4.0	
H, I, J, K					
H	BMW1+w	MT20	2.0	4.0	
L					
L	TMBV1+p	MT20	3.0	8.0	4.25 1.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG					
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX		
L	105	0	105	0	0	9'-6"	9'-6"		
G	60	0	60	0	0	9'-6"	9'-6"		
K	230	0	230	0	0	9'-6"	9'-6"		
J	211	0	211	0	0	9'-6"	9'-6"		
I	227	0	227	0	0	9'-6"	9'-6"		
H	190	0	190	0	0	9'-6"	9'-6"		

UNFACTORED REACTIONS							
JT	1ST CASE COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
L	74	51 / 0	0 / 0	0 / 0	0 / 0	24 / 0	0 / 0
G	42	29 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0
K	163	105 / 0	0 / 0	0 / 0	0 / 0	-58 / 0	0 / 0
J	149	98 / 0	0 / 0	0 / 0	0 / 0	51 / 0	0 / 0
I	160	105 / 0	0 / 0	0 / 0	0 / 0	55 / 0	0 / 0
H	134	87 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G, K, J, I, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)
FR-TO					FR-TO				
L-A	-94 / 0	0.0	0.0	0.02 (1)	K-B	-184 / 0	0.03 (1)	0.03 (1)	0.03 (1)
A-B	-20 / 0	-91.8	-91.8	0.04 (1)	J-C	-177 / 0	0.04 (1)	0.04 (1)	0.04 (1)
B-C	-16 / 0	-91.8	-91.8	0.04 (1)	I-D	-188 / 0	0.10 (1)	0.10 (1)	0.10 (1)
C-D	-8 / 0	-91.8	-91.8	0.04 (1)	H-E	-158 / 0	0.16 (1)	0.16 (1)	0.16 (1)
D-E	-8 / 0	-91.8	-91.8	0.04 (1)					
E-F	-9 / 0	-91.8	-91.8	0.03 (1)					
G-F	-49 / 0	0.0	0.0	0.01 (1)					
L-K	0 / 20	-18.5	-18.5	0.02 (4)					
K-J	0 / 11	-18.5	-18.5	0.02 (4)					
J-I	0 / 7	-18.5	-18.5	0.02 (4)					
I-H	0 / 4	-18.5	-18.5	0.02 (4)					
H-G	0 / 1	-18.5	-18.5	0.01 (4)					

DESIGN CRITERIA			
SPECIFIED LOADS:			
TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD = 39.0 PSF			
SPACING = 24.0 IN. C/C			

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.04/1.00 (A-B:1), BC=0.02/1.00 (K-L:4), WB=0.16/1.00 (E-H:1), SSI=0.06/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES							
PLATE	GRIP (DRY)	SHEAR	SECTION				
	(PSI)	(PLI)	(PLI)	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1967	1873	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (D) (INPUT = 0.90)
JSI METAL= 0.10 (D) (INPUT = 1.00)

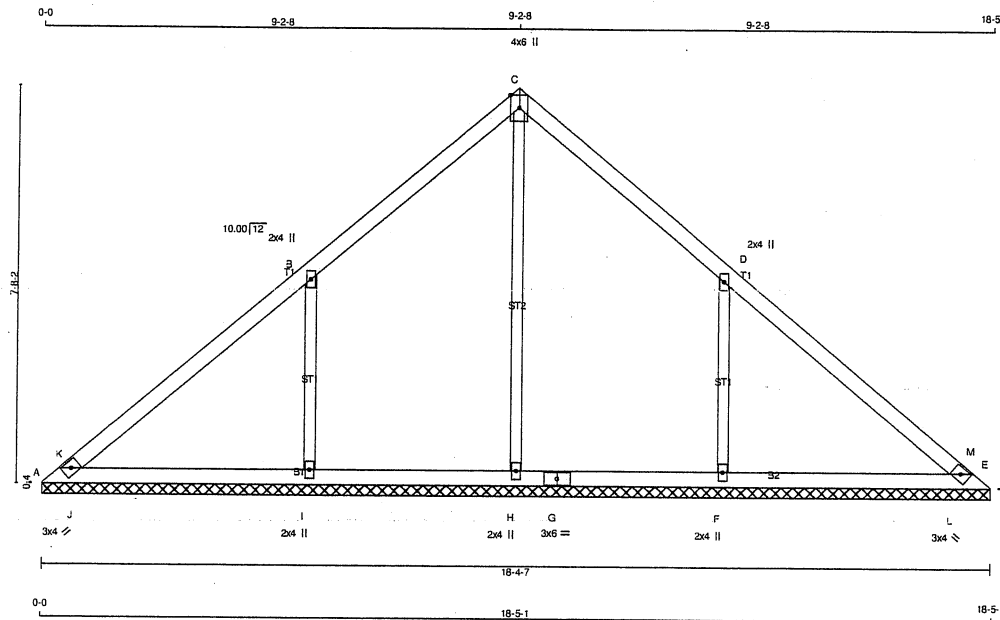


Structural component only
DWG# T-2115934

JOB NAME 412982	TRUSS NAME V02	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:PrpSyr_45RAff3mJ8AgI49yajqE-RhYJOv?QgbrbUM1mWgcf5P8wT3YBmWfb48sBnxzCakw



TOTAL WEIGHT = 59 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	DRY	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
A - G	2x4	DRY	No.2
G - E	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
A	TBM1-h	MT20	3.0	4.0
B	TMW+w	MT20	2.0	4.0
C	TTW+p	MT20	4.0	6.0 Edge
D	TMW+w	MT20	2.0	4.0
E	TBM1-h	MT20	3.0	4.0
F, H, I				
F	BMW1+w	MT20	2.0	4.0
G	BS-t	MT20	3.0	6.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	UPLIFT		
A	135	0	135	0	18-4-7 (8-4-11)B-4-7	
E	135	0	135	0	18-4-7 (8-4-11)B-4-7	
H	517	0	517	0	18-4-7 (8-4-11)B-4-7	
I	620	0	620	0	18-4-7 (8-4-11)B-4-7	
F	620	0	620	0	18-4-7 (8-4-11)B-4-7	

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS				
		SNOW	LIVE	PERM.LIVE	WIND	DEAD
A	94	66 / 0	0 / 0	0 / 0	0 / 0	28 / 0
E	94	66 / 0	0 / 0	0 / 0	0 / 0	28 / 0
H	369	226 / 0	0 / 0	0 / 0	0 / 0	143 / 0
I	437	291 / 0	0 / 0	0 / 0	0 / 0	147 / 0
F	437	291 / 0	0 / 0	0 / 0	0 / 0	147 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, E, H, I, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MAX UNBRACED LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX LC1 (LC)
FR-TO		FROM TO			FR-TO		
A-K	0 / 147	-91.8 -91.8	0.04 (1)	10.00	H-C	-486 / 0	0.51 (1)
K-B	0 / 180	-91.8 -91.8	0.32 (1)	10.00	I-B	-478 / 0	0.13 (1)
B-C	0 / 134	-91.8 -91.8	0.31 (1)	10.00	F-D	-478 / 0	0.13 (1)
C-D	0 / 134	-91.8 -91.8	0.31 (1)	10.00	J-K	-183 / 5	0.00 (1)
D-M	0 / 180	-91.8 -91.8	0.32 (1)	10.00	L-M	-183 / 5	0.00 (1)
M-E	0 / 147	-91.8 -91.8	0.04 (1)	10.00			
A-J	-154 / 0	-18.5 -18.5	0.15 (1)	6.25			
J-I	-117 / 0	-18.5 -18.5	0.15 (1)	6.25			
I-H	-128 / 0	-18.5 -18.5	0.11 (1)	6.25			
H-G	-128 / 0	-18.5 -18.5	0.11 (1)	6.25			
G-F	-128 / 0	-18.5 -18.5	0.11 (1)	6.25			
F-L	-117 / 0	-18.5 -18.5	0.15 (1)	6.25			
L-E	-154 / 0	-18.5 -18.5	0.15 (1)	6.25			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.32/1.00 (D-M:1), BC=0.15/1.00 (F-L:1), WB=0.51/1.00 (C-H:1), SSI=0.15/1.00 (D-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.38 (D) (INPUT = 0.90)
JSI METAL= 0.25 (B) (INPUT = 1.00)

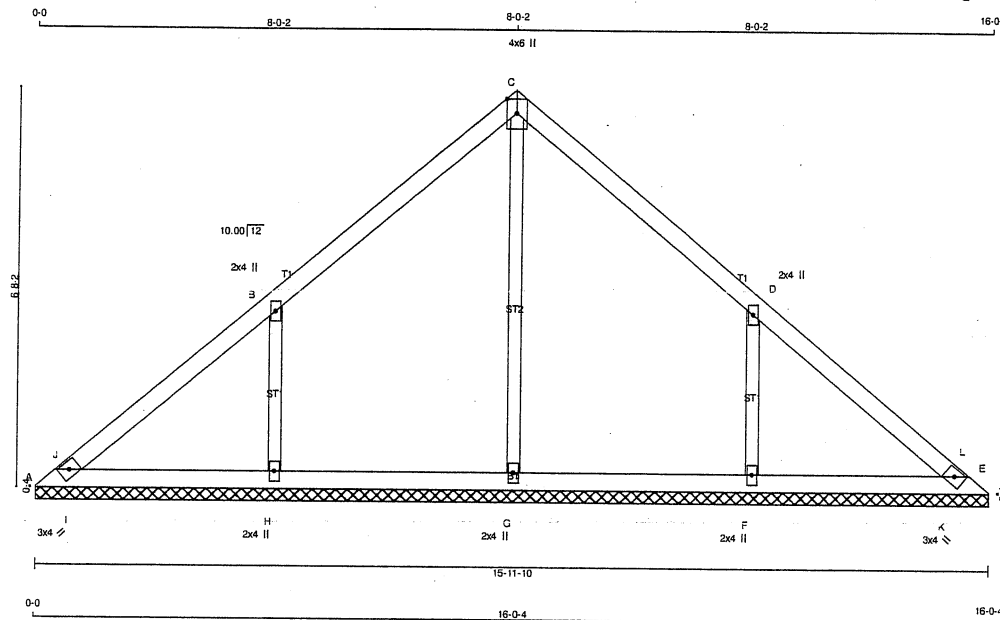


Structural component only
DWG# T-2115935

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
412982	V03	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:PrpSyr_45RAIF3mJ8Ag49yajqE-N3g3pb1hCD5JkgB9e5e7AqDHDtEgETJuXSLlrpzCaku



Scale = 1:36.7

TOTAL WEIGHT = 50 lb

LUMBER	CHORDS	SIZE	DRY	LUMBER	DESCR.
N. L. G. A. RULES					
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
A - E	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	4.0		
B	TMW+w	MT20	2.0	4.0		
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMW+w	MT20	2.0	4.0		
E	TBM1-h	MT20	3.0	4.0		
F, G, H						
F	BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
A	149	0	149	0	15-11-10	15-11-10
E	149	0	149	0	15-11-10	15-11-10
G	398	0	398	0	15-11-10	15-11-10
H	532	0	532	0	15-11-10	15-11-10
F	532	0	532	0	15-11-10	15-11-10

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS				
		SNOW	LIVE	PERM. LIVE	WIND	DEAD
A	105	73 / 0	0 / 0	0 / 0	0 / 0	32 / 0
E	105	73 / 0	0 / 0	0 / 0	0 / 0	32 / 0
G	284	168 / 0	0 / 0	0 / 0	0 / 0	116 / 0
H	375	251 / 0	0 / 0	0 / 0	0 / 0	124 / 0
F	375	251 / 0	0 / 0	0 / 0	0 / 0	124 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, E, G, H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD			MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
			FROM	TO	CS (LC)				
FR-TO									
A-J	0 / 10	-91.8	-91.8	0.03 (4)	10.00	G-C	-341 / 0	0.24 (1)	
J-B	0 / 56	-91.8	-91.8	0.24 (1)	10.00	H-B	-423 / 0	0.08 (1)	
B-C	0 / 20	-91.8	-91.8	0.23 (1)	10.00	F-D	-423 / 0	0.08 (1)	
C-D	0 / 20	-91.8	-91.8	0.23 (1)	10.00	I-J	-91 / 6	0.00 (1)	
D-L	0 / 56	-91.8	-91.8	0.24 (1)	10.00	K-L	-91 / 6	0.00 (1)	
L-E	0 / 10	-91.8	-91.8	0.03 (4)	10.00				
A-I	-35 / 0	-18.5	-18.5	0.08 (1)	6.25				
I-H	-17 / 0	-18.5	-18.5	0.08 (1)	6.25				
H-G	-28 / 0	-18.5	-18.5	0.07 (4)	6.25				
G-F	-28 / 0	-18.5	-18.5	0.07 (4)	6.25				
F-K	-17 / 0	-18.5	-18.5	0.08 (1)	6.25				
K-E	-35 / 0	-18.5	-18.5	0.08 (1)	6.25				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.24/1.00 (D-L:1), BC=0.08/1.00 (F-K:1), WB=0.24/1.00 (C-G:1), SSI=0.14/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

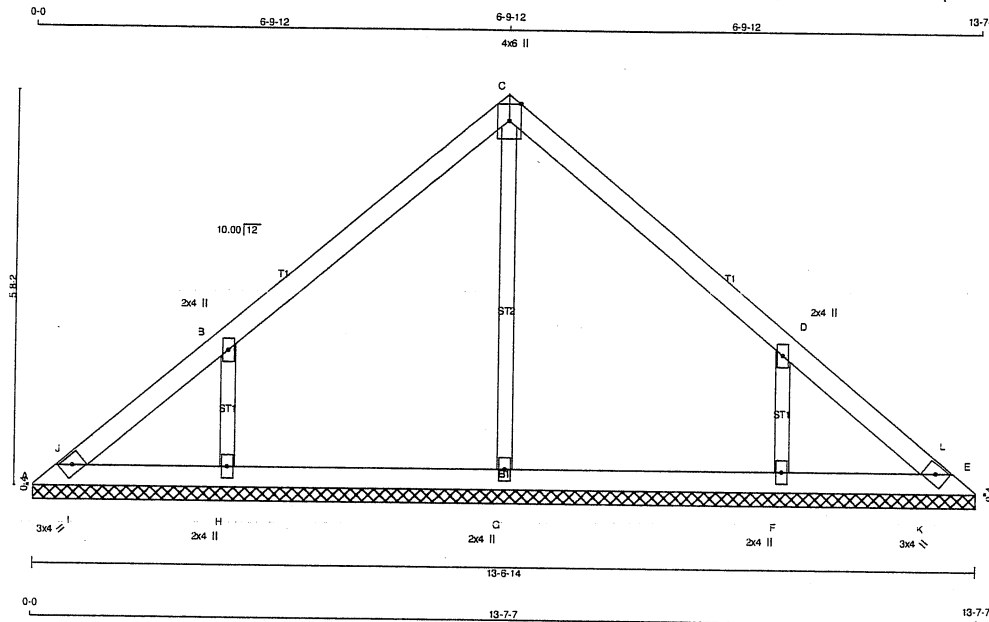
JSI GRIP= 0.30 (B) (INPUT = 0.90)
JSI METAL= 0.23 (D) (INPUT = 1.00)



Structural component only
DWG# T-2115936

JOB NAME 412982	TRUSS NAME V04	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed May 26 16:47:02 2021 Page 1
ID:PrpSyr_45RAfF3mJ8AgI49yajqE-rFES1x2JzWDALqmLBo9Mj2mSUGbDzyd1m64rOGzCakt



TOTAL WEIGHT = 42 lb

LUMBER				DESCR.	
N. L. G. A. RULES					
CHORDS	SIZE	LUMBER		SPF	
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
A - E	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TBM1-h	MT20	3.0	4.0	
B	TMW+w	MT20	2.0	4.0	
C	TTW+p	MT20	4.0	6.0	Edge
D	TMW+w	MT20	2.0	4.0	
E	TBM1-h	MT20	3.0	4.0	
F, G, H					
F	BMW1+w	MT20	2.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
A	127	0	127	0	13-6-14	13-6-14
E	127	0	127	0	13-6-14	13-6-14
G	311	0	311	0	13-6-14	13-6-14
H	465	0	465	0	13-6-14	13-6-14
F	465	0	465	0	13-6-14	13-6-14

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	WIND			
A	89	62 / 0	0 / 0	0 / 0	27 / 0	0 / 0	0 / 0
E	89	62 / 0	0 / 0	0 / 0	27 / 0	0 / 0	0 / 0
G	223	127 / 0	0 / 0	0 / 0	97 / 0	0 / 0	0 / 0
H	328	222 / 0	0 / 0	0 / 0	107 / 0	0 / 0	0 / 0
F	328	222 / 0	0 / 0	0 / 0	107 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, E, G, H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-J	-90 / 0	-91.8 -91.8	0.04 (1)	G-C	-237 / 0	0.11 (1)	
J-B	-20 / 3	-91.8 -91.8	0.20 (1)	H-B	-390 / 0	0.06 (1)	
B-C	-75 / 0	-91.8 -91.8	0.20 (1)	F-D	-390 / 0	0.06 (1)	
C-D	-75 / 0	-91.8 -91.8	0.20 (1)	I-J	-6 / 7	0.00 (1)	
D-L	-20 / 3	-91.8 -91.8	0.20 (1)	K-L	-6 / 7	0.00 (1)	
L-E	-90 / 0	-91.8 -91.8	0.04 (1)				
A-I	0 / 55	-18.5 -18.5	0.03 (1)				
I-H	0 / 58	-18.5 -18.5	0.05 (4)				
H-G	0 / 40	-18.5 -18.5	0.06 (4)				
G-F	0 / 40	-18.5 -18.5	0.06 (4)				
F-K	0 / 58	-18.5 -18.5	0.05 (4)				
K-E	0 / 55	-18.5 -18.5	0.03 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.20/1.00 (C-D:1), BC=0.06/1.00 (G-H:4), WB=0.11/1.00 (C-G:1), SSI=0.13/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
			788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

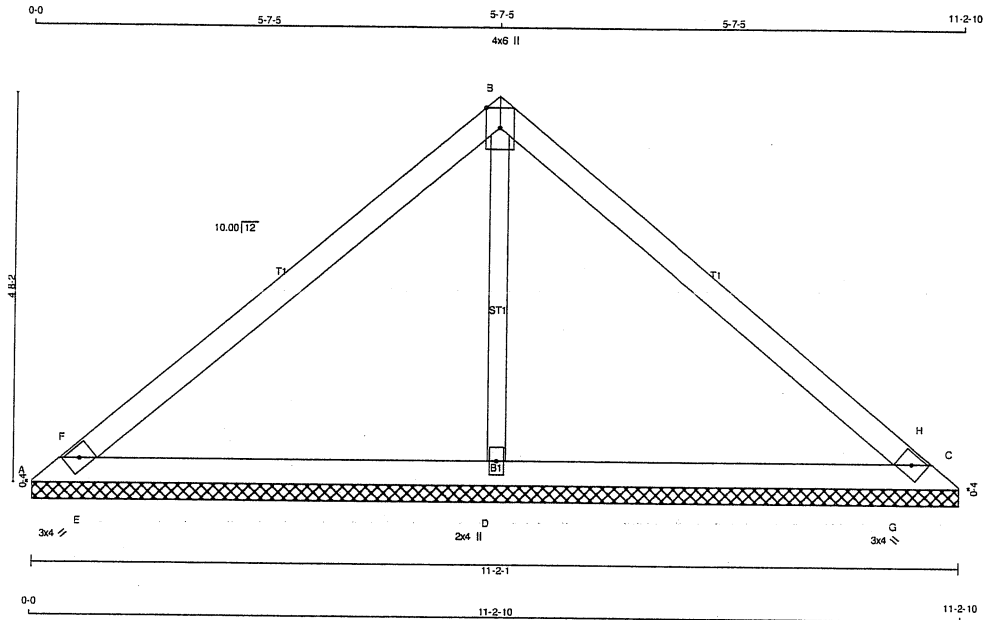
JSI GRIP= 0.28 (D) (INPUT = 0.90)
JSI METAL= 0.21 (D) (INPUT = 1.00)



Structural component only
DWG# T-2115937

JOB NAME 412982	TRUSS NAME V05	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed May 26 16:47:03 2021 Page 1
ID:PrpSyr_45RAfF3mJ8Ag49yjqE-JSoqEH2xqL1z_LXIWgbFFlbdgtQiMgA?mqOwizCaks



Scale = 1/25.4

TOTAL WEIGHT = 32 lb

LUMBER				DESCR.	SPF
N. L. G. A. RULES	CHORDS	SIZE	LUMBER		
A - B	2x4	DRY	No.2	SPF	
B - C	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TBM1-h	MT20	3.0	4.0	
B	TTW+p	MT20	4.0	6.0	Edge
C	TBM1-h	MT20	3.0	4.0	
D	BMW1+w	MT20	2.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT		VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX		
A	-12	0	0	0	-12	11-2-1	11-2-1		
C	-12	0	0	0	-12	11-2-1	11-2-1		
D	1255	0	1255	0	0	11-2-1	11-2-1		

PROVIDE ANCHORAGE AT BEARING JOINT A FOR 150 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
A	-8	0 / -7	0 / 0	0 / 0	0 / 0	0 / -1	0 / 0
C	-8	0 / -7	0 / 0	0 / 0	0 / 0	0 / -1	0 / 0
D	887	585 / 0	0 / 0	0 / 0	0 / 0	302 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, C, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	
FR-TO		FROM TO		FR-TO			
A-F	0 / 526	-91.8 -91.8	0.26 (1)	D-B	-1020 / 0	0.32 (1)	
F-B	0 / 468	-91.8 -91.8	0.37 (1)	E-F	-379 / 0	0.00 (1)	
B-H	0 / 468	-91.8 -91.8	0.37 (1)	G-H	-379 / 0	0.00 (1)	
H-C	0 / 526	-91.8 -91.8	0.26 (1)				
A-E	-440 / 0	-18.5 -18.5	0.25 (1)				
E-D	-372 / 0	-18.5 -18.5	0.26 (1)				
D-G	-372 / 0	-18.5 -18.5	0.26 (1)				
G-C	-440 / 0	-18.5 -18.5	0.25 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.37/1.00 (B-H:1), BC=0.26/1.00 (D-G:1),
WB=0.32/1.00 (B-D:1), SSI=0.22/1.00 (A-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.51 (D) (INPUT = 0.90)
JSI METAL= 0.21 (D) (INPUT = 1.00)

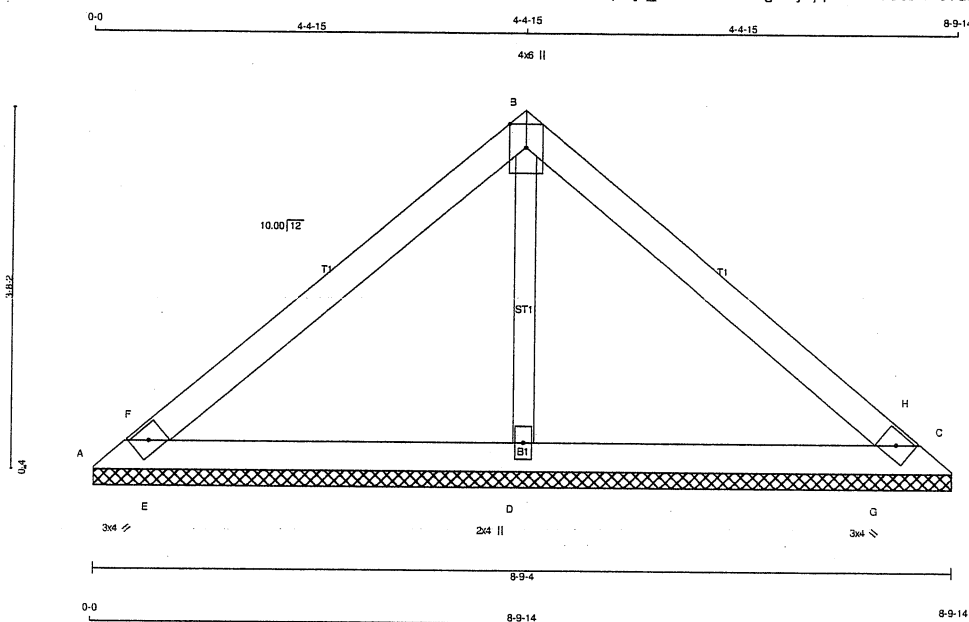


Structural component only
DWG# T-2115938

JOB NAME 412982	TRUSS NAME V06	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed May 26 16:47:04 2021 Page 1
ID:PrpSyr_45RAfF3mJ8Agi49yjqE-oeMCSc3ZV8Tub8wkJDBqoTroj4F4RrcKDQZyS8zCakr



TOTAL WEIGHT = 24 lb

LUMBER				DESCR.
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TBM1-h	MT20	3.0	4.0	Edge
B	TTW+p	MT20	4.0	6.0	
C	TBM1-h	MT20	3.0	4.0	
D	BMW1+w	MT20	2.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
A	48	0	48	0	8-9-4	8-9-4
C	48	0	48	0	8-9-4	8-9-4
D	870	0	870	0	8-9-4	8-9-4

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
A	34	23 / 0	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0
C	34	23 / 0	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0
D	615	403 / 0	0 / 0	0 / 0	0 / 0	212 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, C, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX LC1 (LC)
FR-TO		FROM	TO		FR-TO		
A-F	0 / 300	-91.8	-91.8 0.13 (1)	10.00	D-B	-676 / 0	0.14 (1)
F-B	0 / 277	-91.8	-91.8 0.22 (1)	10.00	E-F	-252 / 0	0.00 (1)
B-H	0 / 277	-91.8	-91.8 0.22 (1)	10.00	G-H	-252 / 0	0.00 (1)
H-C	0 / 300	-91.8	-91.8 0.13 (1)	10.00			
A-E	-265 / 0	-18.5	-18.5 0.17 (1)	6.25			
E-D	-221 / 0	-18.5	-18.5 0.17 (1)	6.25			
D-G	-221 / 0	-18.5	-18.5 0.17 (1)	6.25			
G-C	-265 / 0	-18.5	-18.5 0.17 (1)	6.25			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.22/1.00 (B-F:1), BC=0.17/1.00 (D-E:1), WB=0.14/1.00 (B-D:1), SSI=0.14/1.00 (A-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
		788	1987
			1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.34 (D) (INPUT = 0.90)

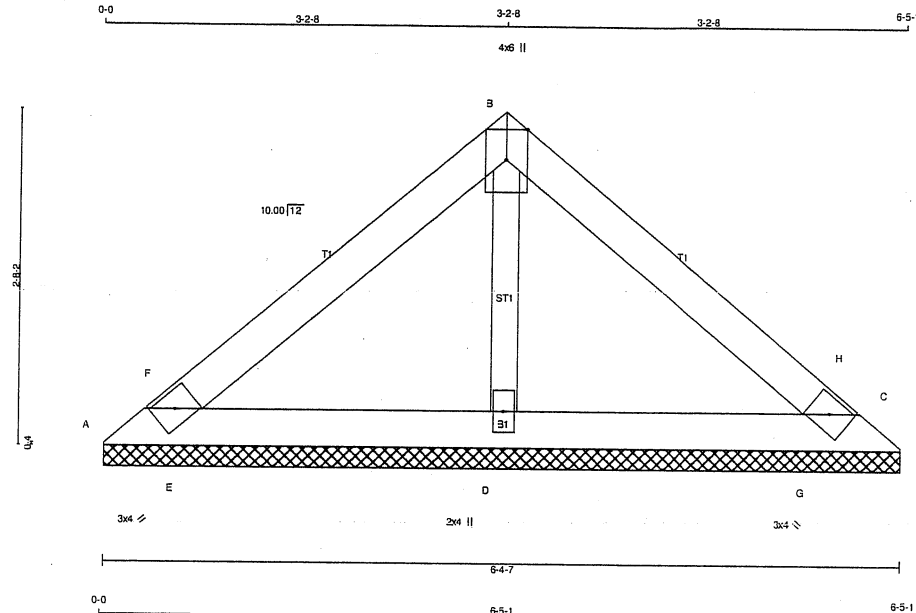
JSI METAL= 0.14 (D) (INPUT = 1.00)



Structural component only
DWG# T-2115939

JOB NAME 412982	TRUSS NAME V07	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:PrpSyr_45RAIf3mJ8Agi49yajqE-Gqvafy4BGRbIDHUwxi4KgO?EUcQAK6TS4JV_azCakq



TOTAL WEIGHT = 17 lb

LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER
A - B 2x4 DRY No.2
B - C 2x4 DRY No.2
A - C 2x4 DRY No.2
ALL WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	4.0		
B	TTW+p	MT20	4.0	6.0	Edge	
C	TBM1-h	MT20	3.0	4.0		
D	BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
A	77	0	77	0	6-4-7	6-4-7
C	77	0	77	0	6-4-7	6-4-7
D	548	0	548	0	6-4-7	6-4-7

UNFACTORED REACTIONS

JT	COMBINED	MAX./MIN. COMPONENT REACTIONS		PERM.LIVE	WIND	DEAD	SOIL
		SNOW	LIVE				
A	54	37 / 0	0 / 0	0 / 0	0 / 0	18 / 0	0 / 0
C	54	37 / 0	0 / 0	0 / 0	0 / 0	18 / 0	0 / 0
D	388	252 / 0	0 / 0	0 / 0	0 / 0	136 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, C, D
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	FORCE (LBS)	FACTORED MAX. (LC)	MAX. (LC)
FR-TO		FROM TO		FR-TO			
A-F	0 / 136	-91.8 -91.8 0.05 (1)	10.00	D-B	-394 / 0	0.06 (1)	
F-B	0 / 134	-91.8 -91.8 0.11 (1)	10.00	E-F	-153 / 0	0.00 (1)	
B-H	0 / 134	-91.8 -91.8 0.11 (1)	10.00	G-H	-153 / 0	0.00 (1)	
H-C	0 / 136	-91.8 -91.8 0.05 (1)	10.00				
A-E	-131 / 0	-18.5 -18.5 0.10 (1)	6.25				
E-D	-106 / 0	-18.5 -18.5 0.10 (1)	6.25				
D-G	-106 / 0	-18.5 -18.5 0.10 (1)	6.25				
G-C	-131 / 0	-18.5 -18.5 0.10 (1)	6.25				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11/1.00 (B-F:1), BC=0.10/1.00 (D-E:1), WB=0.06/1.00 (B-D:1), SSI=0.08/1.00 (A-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (D) (INPUT = 0.90)
JSI METAL= 0.08 (D) (INPUT = 1.00)

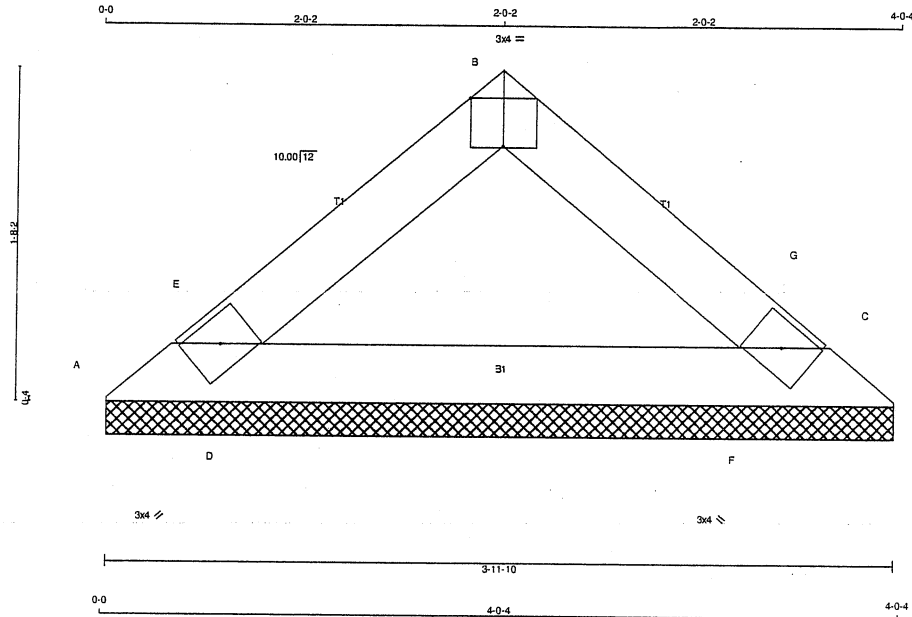


Structural component only
DWG# T-2115940

JOB NAME 412982	TRUSS NAME V08	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:PrpSyr_45RAfF3mJ8Agi49yajqE-k1Tysi5p1JcqR36QeEJtuwAJuxuvnLdhk23X1zCakp



TOTAL WEIGHT = 9 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - B	2x4	DRY	No.2
B - C	2x4	DRY	No.2
A - C	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TBM1-h	MT20	3.0	4.0	
B	TT-p	MT20	3.0	4.0	Edge 2.00
C	TBM1-h	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
A	219	0	219	0	0	3-11-10	3-11-10		
C	219	0	219	0	0	3-11-10	3-11-10		

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
A	155	102 / 0	0 / 0	0 / 0	0 / 0	0 / 0	53 / 0	0 / 0
C	155	102 / 0	0 / 0	0 / 0	0 / 0	0 / 0	53 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CS (LC)	MEMB.	FORCE (LBS)	MAX. CS (LC)	MEMB.
FR-TO		FROM	TO	FR-TO		FROM	TO
A-E	-197 / 0	-91.8	-91.8 0.09 (1)	D-E	0 / 37	0.00 (1)	
E-B	-138 / 0	-91.8	-91.8 0.09 (1)	F-G	0 / 37	0.00 (1)	
B-G	-138 / 0	-91.8	-91.8 0.09 (1)				
G-C	-197 / 0	-91.8	-91.8 0.09 (1)				
A-D	0 / 124	-18.5	-18.5 0.06 (1)				
D-F	0 / 130	-18.5	-18.5 0.08 (1)				
F-C	0 / 124	-18.5	-18.5 0.06 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.09/1.00 (B-E:1), BC=0.08/1.00 (D-F:1), WB=0.00/1.00 (D-E:1), SSI=0.08/1.00 (C-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

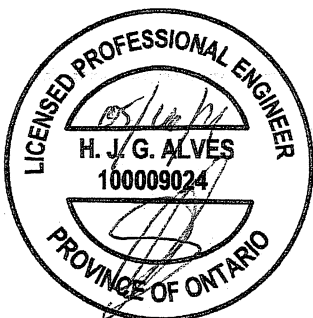
PLATE	GRIP (DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.11 (A) (INPUT = 0.90)

JSI METAL= 0.06 (A) (INPUT = 1.00)

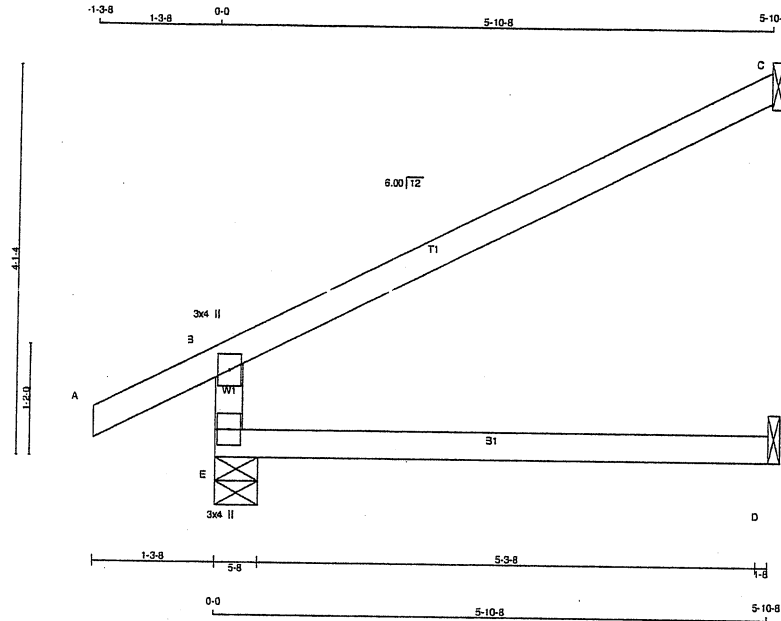


Structural component only
DWG# T-2115941

JOB NAME 412982	TRUSS NAME J01	QUANTITY 30	PLY 1	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed May 26 16:46:17 2021 Page 1
ID:U6yi?rbeFFwxf_UFm_koDybSsJ-gb0RaYV4ZhYZsvqj3rrpOZOnc7CewdDPQchzJMzCala



Scale = 1/20

TOTAL WEIGHT = 30 X 17 = 504 lb [M]

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

DESCR.
SPF
SPF
SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ
E	525	0	525	0
C	202	0	202	0
D	45	0	50	0

SEE MITTEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
	COMBINED							
E	369	257 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0	0 / 0
C	139	113 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRACED LENGTH FR-TO	WEBS	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
E-B		-461 / 0	0.0	0.0	0.13 (4)	7.81		
A-B		0 / 28	-91.8	-91.8	0.12 (1)	10.00		
B-C		-30 / 0	-91.8	-91.8	0.54 (1)	6.25		
E-D		0 / 0	-18.5	-18.5	0.13 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.54/1.00 (B-C:1), BC=0.13/1.00 (D-E:4),
WB=0.00/1.00 (n/a:0), SSI=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
		788	1987

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (E) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)



Structural component only
DWG# T-2115888

Tamarack Roof Truss, Burlington



TOTAL WEIGHT = $5 \times 10 = 50$ lb

DRY: SEASONED LUMBER.

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
C	141	0	141	0	0	5-8 (5-7)	5-8
B	320	0	320	0	0	5-8	5-8
D	57	0	57	0	0	5-8	5-8

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS
CHORD AT JT(S): C

UNFACTORED REACTIONS							
JT	1ST CASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	98	76 / 0	0 / 0	0 / 0	0 / 0	22 / 0	0 / 0
B	224	160 / 0	0 / 0	0 / 0	0 / 0	64 / 0	0 / 0
D		16 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED

LOADING

TOTAL LOAD CASES: (5)

C H O R D S					W E B S				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)		
FR-TO		FROM TO			FR-TO				
A-B	0 / 18	-91.8	-91.8	0.13 (5)	10.00	E-F	-138 / 5		
B-F	-10 / 0	-91.8	-91.8	0.04 (4)	6.25		0.00 (1)		
F-C	0 / 2	-91.8	-91.8	0.15 (1)	10.00				
B-E	0 / 0	-18.5	-18.5	0.12 (1)	10.00				
E-D	0 / 0	-18.5	-18.5	0.12 (1)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:				
TOP	CH.	LL =	25.6	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR
SMALL BUILDING REQUIREMENTS OF PART 9,
NBC 2015

THIS DESIGN COMPLIES WITH:

- THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 086-14
 - TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.15/1.00 (C-F:1) , BC=0.12/1.00 (B-E:1) ,
WB=0.00/1.00 (E-F:1) , SSI=0.11/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	187

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

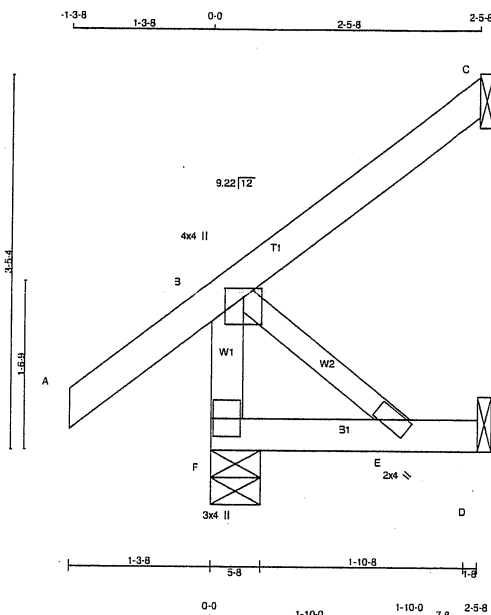


Structural component only
DWG# T-2115890

JOB NAME 412982	TRUSS NAME J04	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed May 26 16:46:19 2021 Page 1
ID:PrpSyr_45RAff3mJ8Agi49yajqE-c_8B?EWL5JoHhDz4AGuHT_U7VxweOXitwA3NFzCaY



Scale = 1:20.2

TOTAL WEIGHT = 2 X 11 = 23 lb

LUMBER					
N. L. G. A. RULES					
CHORDS	SIZE	LUMBER		DESCR.	
F - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	4.0	4.0	1.00 2.00
E	BMV+w	MT20	2.0	4.0	
F	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
F	262	0	262	0	5-8	5-8
C	113	0	113	0	1-8	1-8
D	23	0	25	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	WIND			
F	183	134 / 0	0 / 0	0 / 0	0 / 0	50 / 0	0 / 0
C	78	63 / 0	0 / 0	0 / 0	0 / 0	15 / 0	0 / 0
D	18	0 / 0	0 / 0	0 / 0	0 / 0	18 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)		FACTORED VERT. LOAD LC1 MAX (PLF)		MAX. UNBRAC LENGTH	MEMB.	WEBS MAX. FACTORED FORCE (LBS)		MAX. CSI (LC)
	FR-TO	FROM TO	FR-TO	FROM TO			FR-TO	FROM TO	
F-B	-239 / 0	0.0	0.0	0.03 (1)	7.81	B-E	0 / 0	0.00 (1)	
A-B	0 / 39	-91.8	-91.8	0.13 (5)	10.00				
B-C	0 / 0	-91.8	-91.8	0.09 (1)	10.00				
F-E	0 / 0	-18.5	-18.5	0.03 (4)	10.00				
E-D	0 / 0	-18.5	-18.5	0.03 (4)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD = 39.0 PSF				

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/999 (0.00")

CSI: TC=0.13/1.00 (A-B:5), BC=0.03/1.00 (E-F:4), WB=0.00/1.00 (B-E:1), SS=0.08/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

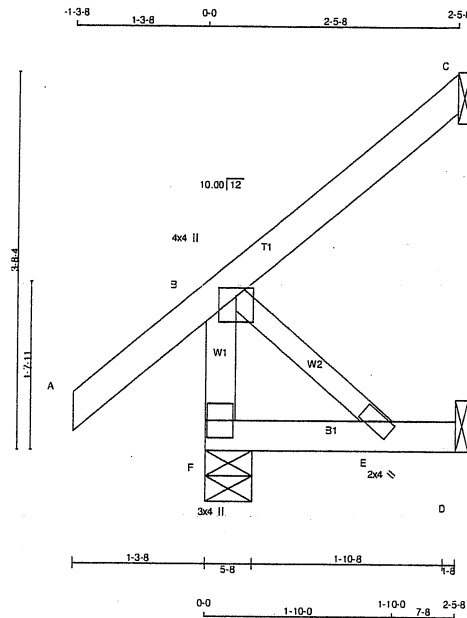


Structural component only
DWG# T-2115891

JOB NAME 412982	TRUSS NAME J05	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed May 26 16:46:20 2021 Page 1
ID:PrpSyr_45RAfF3mJ8Agi49yajqE-4AhZDaXzscw8JNYGk_PW0B01ELGt7_yr6avdwhzCalX



Scale = 1/2" = 1'-0"

TOTAL WEIGHT = 2 X 12 = 23 lb

LUMBER					DESCR.
N. L. G. A. RULES	CHORDS	SIZE	LUMBER		
F - B	2x4	DRY	No.2		SPF
A - C	2x4	DRY	No.2		SPF
F - D	2x4	DRY	No.2		SPF
ALL WEBS	2x3	DRY	No.2		SPF
DRY: SEASONED LUMBER.					

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	4.0	4.0	1.00 2.00
E	BMVW+w	MT20	2.0	4.0	
F	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
F	263	0	263	0	5-8	5-8
C	113	0	113	0	1-8	1-8
D	23	0	25	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LOASE		MAX./MIN. COMPONENT REACTIONS				DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND			
F	183	134 / 0	0 / 0	0 / 0	0 / 0	50 / 0	0 / 0	0 / 0
C	78	63 / 0	0 / 0	0 / 0	0 / 0	15 / 0	0 / 0	0 / 0
D	18	0 / 0	0 / 0	0 / 0	0 / 0	18 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS		FACTORED				WEBS		FACTORED	
	MAX.	FORCE	VERT.	LOAD	LC1	MAX	MEMB.	FORCE	MAX	
		(LBS)		(PLF)	CSI (LC)	UNBRAC		(LBS)	CSI (LC)	
FR-TO			FROM	TO		LENGTH	FR-TO			
F-B	-240	/ 0	0.0	0.0	0.03 (1)	7.81	B-E	0 / 0	0.00 (1)	
A-B	0 / 41		-91.8	-91.8	0.13 (5)	10.00				
B-C	0 / 0		-91.8	-91.8	0.09 (1)	10.00				
F-E	0 / 0		-18.5	-18.5	0.03 (4)	10.00				
E-D	0 / 0		-18.5	-18.5	0.03 (4)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.13/1.00 (A-B:5), BC=0.03/1.00 (E-F:4), WB=0.00/1.00 (B-E:1), SSI=0.08/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

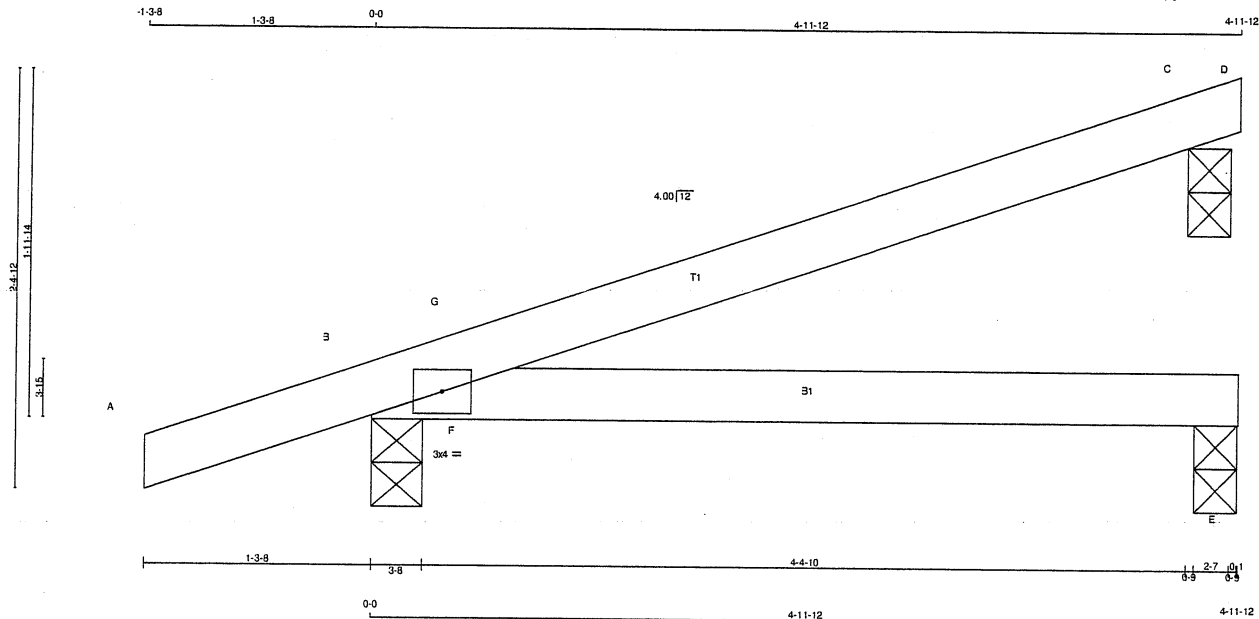


Structural component only
DWG# T-2115892

JOB NAME 412982	TRUSS NAME J07	QUANTITY 8	PLY 1	JOB DESC. ROYAL PINE HOMES TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed May 26 16:58:36 2021 Page 1
ID:PrpSyr_45RAfF3mJ8AgI49yajqE-XmAphERdXlficUpjrYc6rOV2E1PXar9BL4CZ11zCaa1



TOTAL WEIGHT = 8 X 13 = 107 lb [M]

LUMBER				DESCR.
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	
A - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMB1-I	MT20	3.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
B	390	0	390	0	3-8	3-8
E	73	0	73	0	3-0	3-0
C	196	0	196	0	3-0	3-0

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): C

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	274	192 / 0	0 / 0	0 / 0	0 / 0	82 / 0	0 / 0
E	54	19 / 0	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0
C	136	103 / 0	0 / 0	0 / 0	0 / 0	33 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 18	-91.8 -91.8	0.11 (1)	10.00	F-G	-217 / 15 0.00 (1)	
B-G	-17 / 0	-91.8 -91.8	0.06 (4)	6.25			
G-C	0 / 2	-91.8 -91.8	0.28 (1)	10.00			
C-D	0 / 0	-15.0 -15.0	0.00 (4)	10.00			
B-F	0 / 0	-18.5 -18.5	0.19 (1)	10.00			
F-E	0 / 0	-18.5 -18.5	0.19 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.03")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/849$ (0.07")

CSI: TC=0.28/1.00 (C-G:1), BC=0.19/1.00 (E-F:1), WB=0.00/1.00 (F-G:1), SSI=0.18/1.00 (B-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

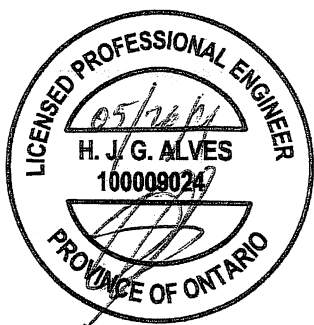
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)



Structural component only
DWG# T-2115893



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1800213

Feb 09, 2018

TOE-NAIL CAPACITY DETAILS

LATERAL AND WITHDRAWAL RESISTANCE OF BEARING ANCHORAGE BY TOE-NAILS

			SPF	D. FIR	SPF	D. FIR
COMMON WIRE	3.00	0.144	122	139	30	42
	3.25	0.144	127	144	32	45
	3.50	0.160	152	173	38	52
COMMON SPIRAL	3.00	0.122	96	108	26	36
	3.25	0.122	97	108	28	40
	3.50	0.152	142	161	36	50
3.25" Gun nail	3.25	0.120	94	105	28	39

Note: If using truss with D. Fir lumber and SPF bearing plate, use tabulated SPF values in table.

Nail type:	Common wire	Common spiral	Common wire	Common spiral	Gun Nail
Diameter (in.)	0.160	0.152	0.144	0.122	0.120
Length (in.)	3.50	3.50	3.00	3.00	3.25
MAXIMUM NUMBER OF TOE-NAILS					
2x4 SPF	2	2	3	3	3
2x6 SPF	4	4	4	5	5
2x4 D. FIR	2	2	2	2	2
2x6 D. FIR	3	3	3	4	4

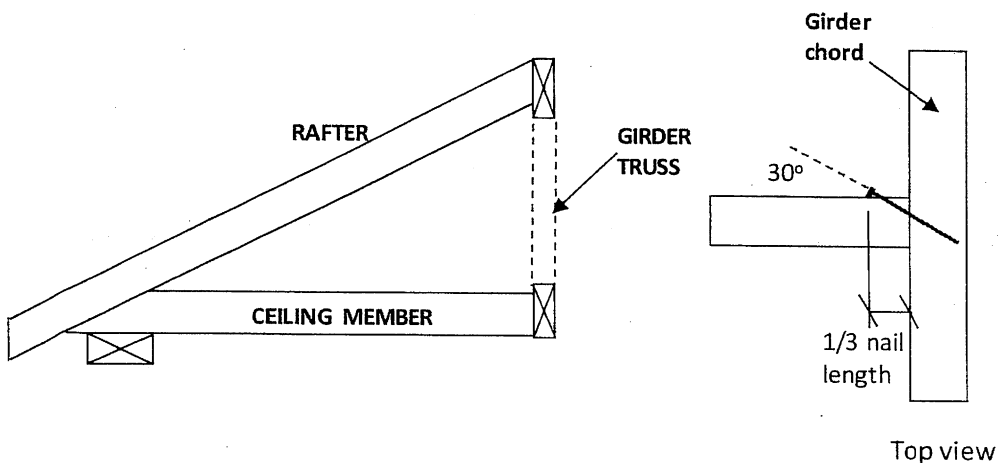
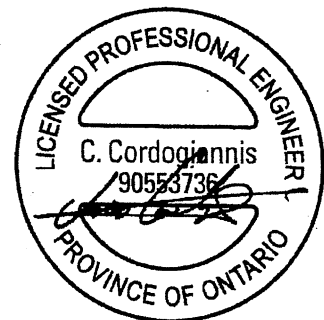


Figure 1: Toe-Nailing Rafter / Ceiling Member to Girder Truss

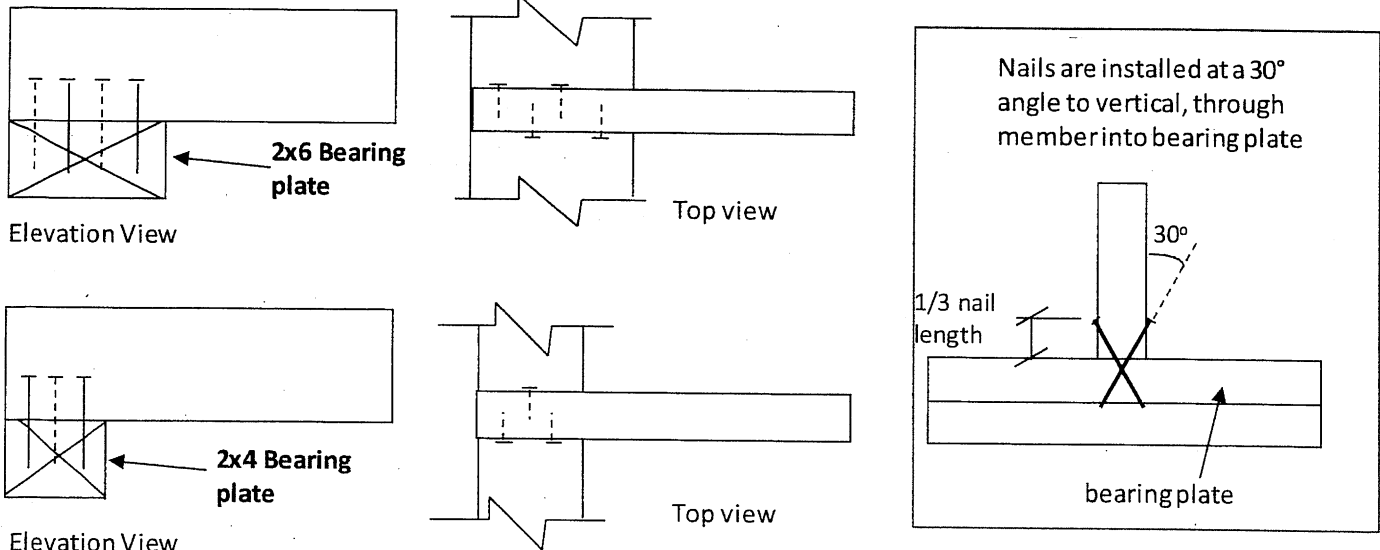
PEO
Certificate No. 10889485



December 21, 2020

TOE-NAIL CAPACITY DETAILS

Figure 2: Toe-Nail Anchorage to Bearing Plate for Uplift



NOTES:

1. Rafter and ceiling members may be connected to top and bottom chords of girder truss by toe-nailing the members into the girder chords (see fig. 1), provided the factored vertical reactions of the supported members do not exceed the lateral resistance of the toe-nails. Mechanical connectors (hangers) are required if factored vertical reactions exceed the toe-nail capacity, or if the connection must resist horizontal loads (loads perpendicular to the face of girder or rafter).
2. Trusses, rafters or ceiling members may be anchored to the bearing plate with toe-nails (see fig. 2), provided that the factored uplift reactions due to **wind or earthquake loads** do not exceed the **withdrawal resistance of the toe-nails**. Mechanical anchors (tie-downs) are required for reactions that exceed the toe-nail withdrawal capacity. Toe-nail anchorage to bearing plates is **NOT** permitted if uplift reactions are generated from gravity loads (snow, floor live, dead).
3. Tabulated toe-nail resistances on page 1 are for **one** toe-nail. Multiply unit values by the number of nails used in the connection. Maximum number of nails in a connection shall not exceed the tabulated limits shown on page 1 for a given lumber size /species.
4. Nail values are based on specific gravity of $G = 0.42$ (SPF) and $G = 0.49$ (D. Fir).
5. Toe-nails shall be driven at approximately $1/3$ the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member.
6. For wind / earthquake loads, tabulated lateral resistances may be multiplied by 1.15 (K_b factor). No increases are permitted for tabulated withdrawal resistances.
7. Lumber must be dry ($< 19\%$ moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-19, Clause 12.9.

PEO
Certificate No. 10889485



LUL/LUS/LJS/HUS/HHUS/HGUS

Standard and Double-Shear Joist Hangers



This product is preferable to similar connectors because of
a) easier installation, b) higher capacities, c) lower installed
cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 217–218.

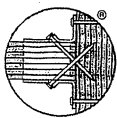
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 18–20.

Installation:

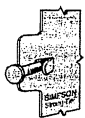
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1 1/2" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

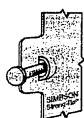
- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on pp. 105–107.



Double-Shear
Nailing
Top View

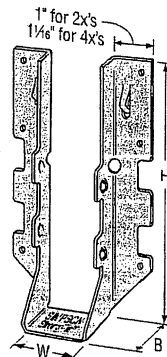
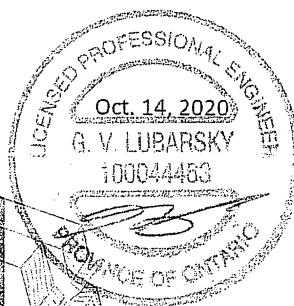
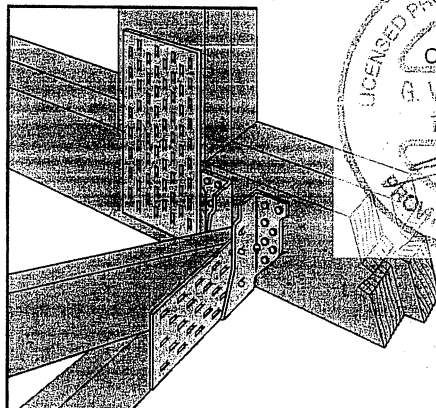


Double-Shear
Nailing
Side View;
Do not
bend tab

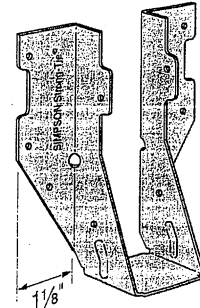


Dome Double-Shear
Nailing
Side View
(available on
some models)

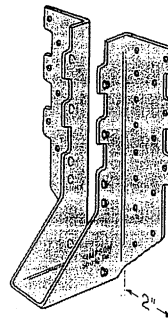
Typical HUS26
Installation
with Reduced
Heel Height
(Truss Designer
to provide
fastener quantity
for connecting
multiple members
together)



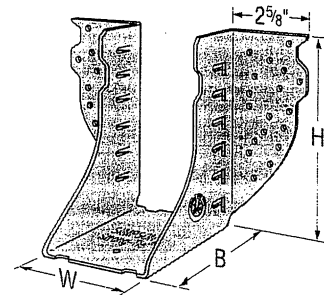
✓ LUS28



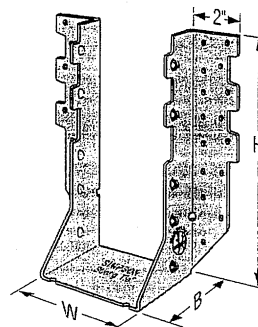
LU26L



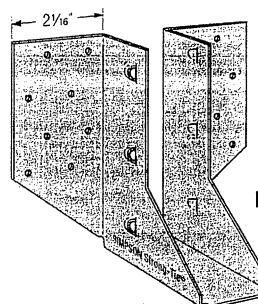
✓ HUS210
(HUS26, HUS28,
and HHUS similar)



✓ HGUS28-2



✓ HHUS210-2



LJS26DS

LUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

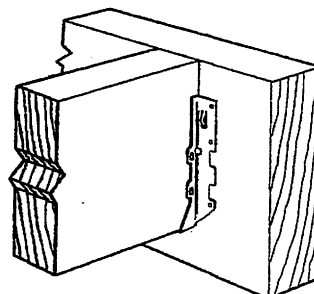
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

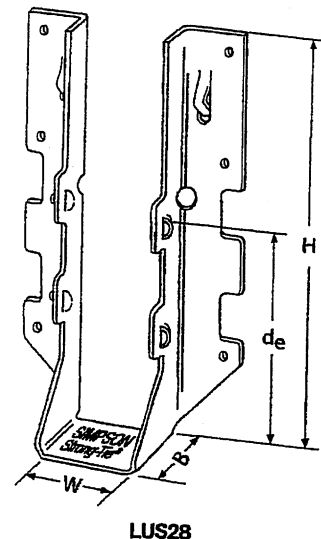
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire,
10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified



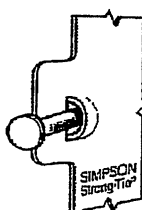
Typical LUS
Installation



LUS28

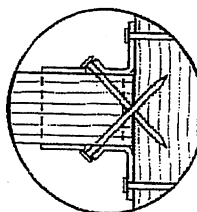
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _g ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _g =1.15)	Normal (K _g =1.00)	Uplift (K _g =1.15)	Normal (K _g =1.00)
LUS24	18	1½	3½	1¾	1 15/16	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3½	3½	2	1 15/16	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1½	4¾	1¾	3½	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3½	4¾	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4¾	4¾	2	3¼	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1½	6¾	1¾	3¾	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3½	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4¾	6¼	2	3¼	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1½	7 15/16	1¾	3¾	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3½	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4¾	8¾	2	5¼	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_g is the distance from the seat of the hanger to the highest joist nail.

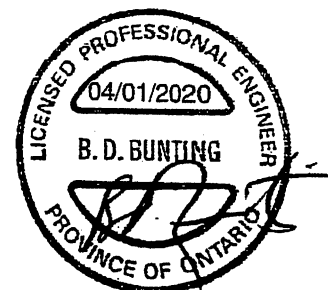


Double Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent
5,603,580



Double Shear Nailing
Top View.



**LIMIT
STATES
DESIGN**

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECLUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

HUS/LJS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

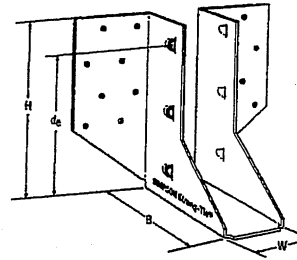
- Factored resistances are in accordance with CSA O86 -14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

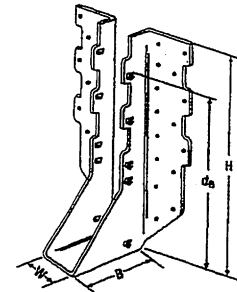
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

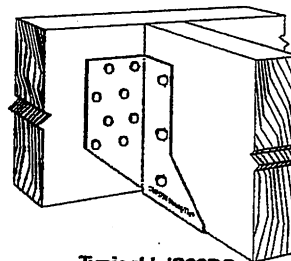
- See current catalogue for options



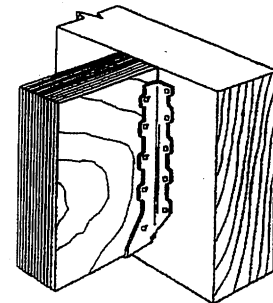
LJS26DS



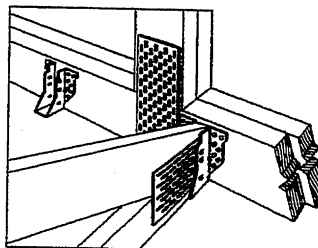
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



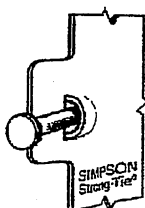
Typical HUS
Installation



Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

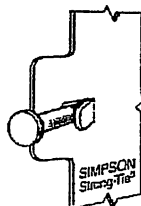
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D-Fir-L		S-P-F	
								Uplift (K _u =1.15)	Normal (K _o =1.00)	Uplift (K _u =1.15)	Normal (K _o =1.00)
LJS26DS	18	1½	5	3½	4⅝	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1½	5⅝	3	3⅞	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1½	7⅞	3	6⅞	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1½	9⅞	3	7⅞	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1½	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

1. d_e is the distance from the seat of the hanger to the highest joist nail.

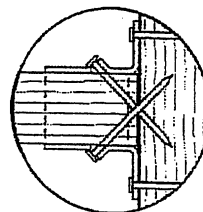


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



**LIMIT
STATES
DESIGN**

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECHUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

HGUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

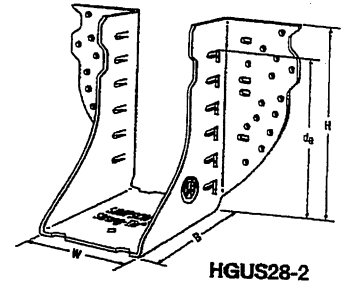
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%.
No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

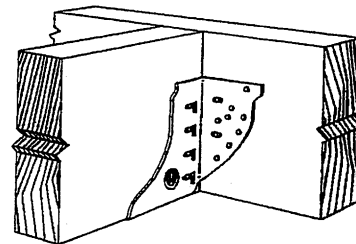
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

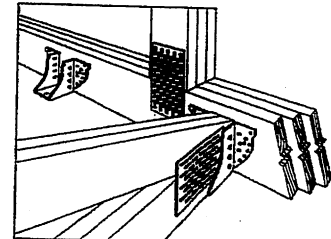
- See current catalogue for options



HGUS28-2



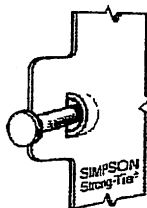
Typical HGUS
Installation



Typical HGUS Installation
(Truss Designer to
provide fastener quantity
for connecting multiple
members together)

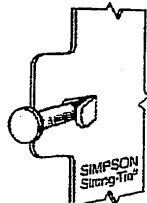
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _g ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _u =1.15)	Normal (K _n =1.00)	Uplift (K _u =1.15)	Normal (K _n =1.00)
HGUS26	12	1½	5½	5	4½	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3½	5½	4	4½	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4½	5½	4	4½	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6½	5½	4	4½	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1½	7½	5	6½	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3½	7½	4	6½	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4½	7½	4	6½	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6½	7½	4	6½	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210	12	1½	9½	5	7½	(46) 16d	(16) 16d	3535	11070	2510	8090
HGUS210-2	12	3½	9½	4	8½	(46) 16d	(16) 16d	6840	14015	4855	10270
HGUS210-3	12	4½	9½	4	8½	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6½	9½	4	8½	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6½	10½	4	10½	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6½	12½	4	11½	(66) 16d	(22) 16d	10130	16400	7195	11645

1. d_g is the distance from the seat of the hanger to the highest joist nail.

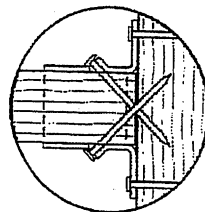


Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).

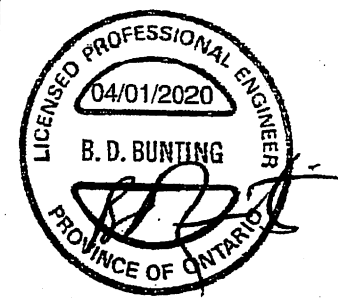
U.S. Patent
5,603,580



Double
Shear
Nailing
Side View.
Do not
bend tab
back.



Double
Shear
Nailing
Top View.



**LIMIT
STATES
DESIGN**

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020.
This information is updated periodically and should not be relied upon after June 30, 2022.
Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

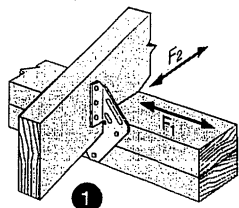
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T-SPECHGUS20 3/20 exp. 6/22

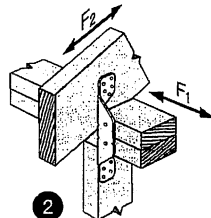
(800) 999-5099
strongtie.com

H/TSP

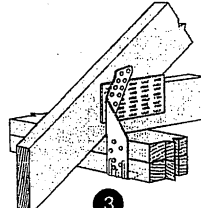
Seismic and Hurricane Ties (cont.)



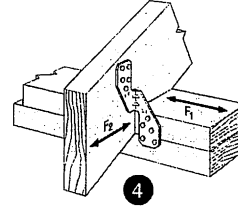
1 H1 Installation



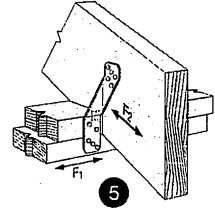
2 H2A Installation



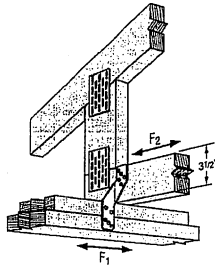
3 TSP Installation



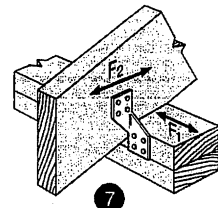
4 H2.5A Installation
(Nails into both top plates)



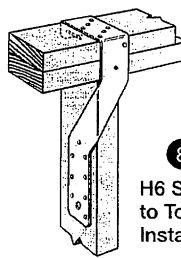
5 H2.5T Installation
(Nails into both top plates)



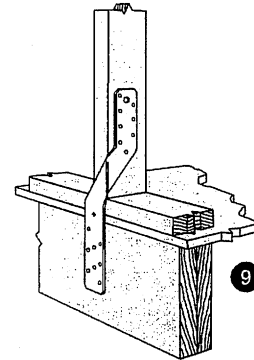
6 H2.5T Installation



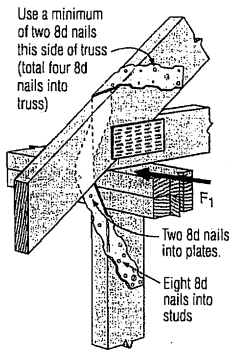
7 H3 Installation
(Nails into upper top plate)



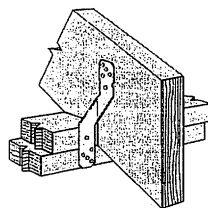
8 H6 Stud to Top Plate Installation



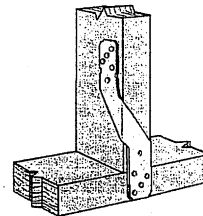
9 H6 Stud to Band Joist Installation



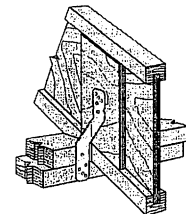
10 H7Z Installation



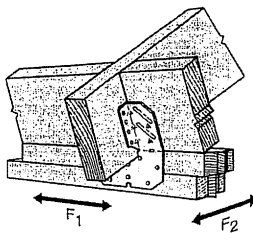
11 H8 Attaching Rafter to Double Top Plates



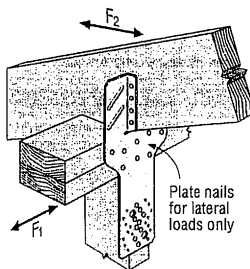
12 H8 attaching Stud to Sill
(4 8d into plate, 5 8d into stud)



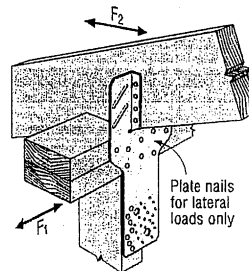
13 H8 attaching I-Joist to Double Top Plates



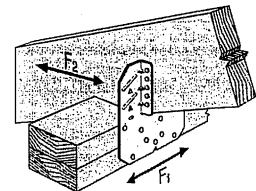
14 H10A Field-Bent Installation



15 H10S Installation

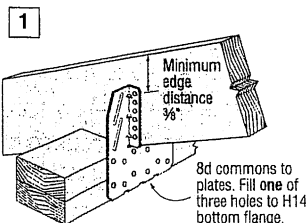


16 H10S Installation with Stud Offset

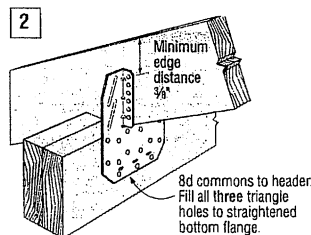


17 H10A Installation

H10A optional positive angle nailing connects shear blocking to rafter. Use 8d common nails. Slot allows maximum field-bending up to a pitch of 6/12, use 75% of the table uplift value; bend one time only.

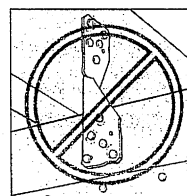


18 H14 Installation to Double Top Plates



19 H14 Installation to Double 2x Header

Avoid a Misinstallation



Do not make new holes or overdrive nails.

H/TSP

Seismic and Hurricane Ties

Simpson Strong-Tie® hurricane ties provide a positive connection between truss/rafter and the wall of the structure to resist wind and seismic forces. New additions to the line provide even more options.

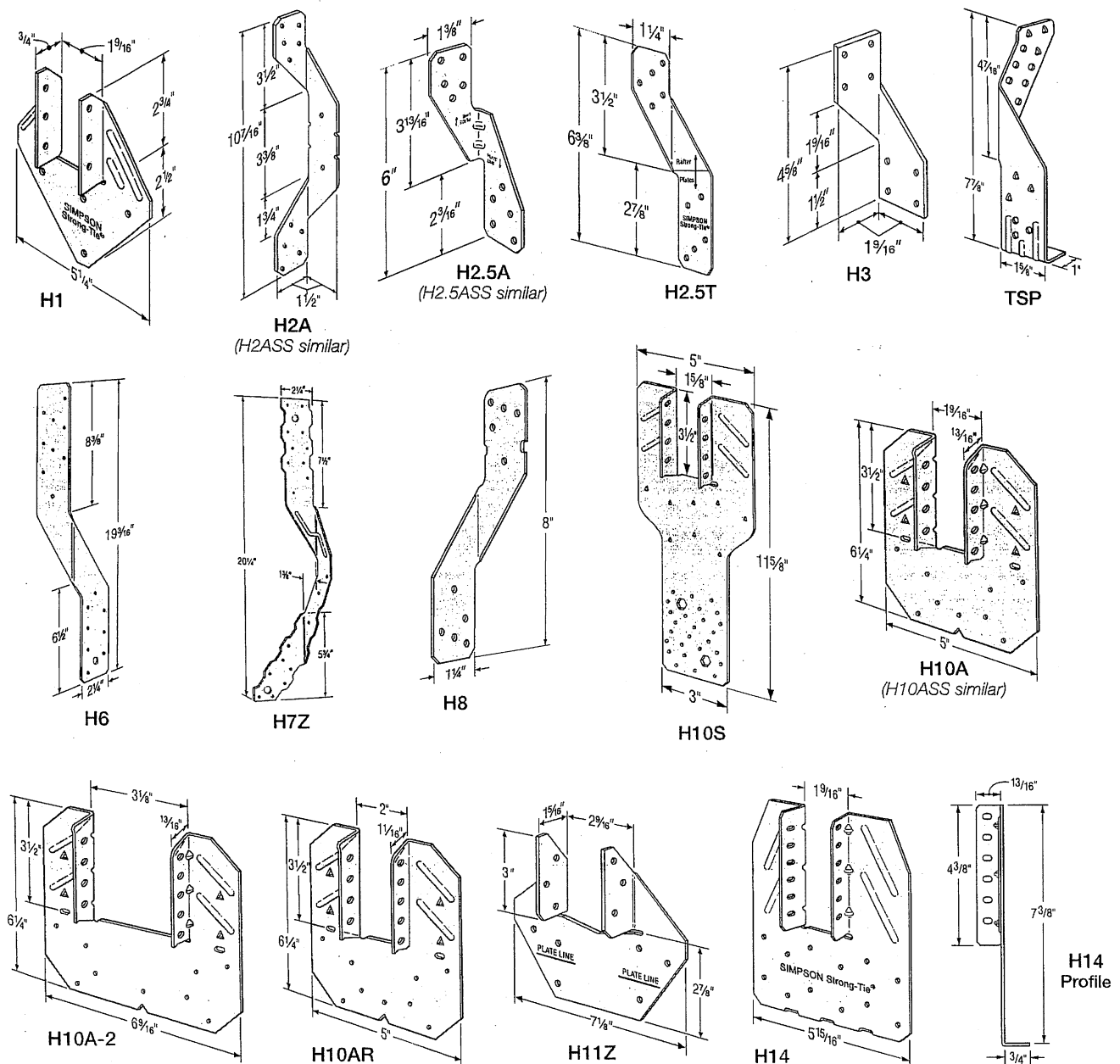
- H10AR — The heavy-duty design of the H10A available with a 2" wide throat to accommodate rough lumber
- H10A-2 — The H10A design with a 3" throat for double 2x members
- H2ASS, H2.5ASS and H10ASS — Popular ties now available in stainless steel

Material: See table

Finish: Galvanized. H7Z and H11Z — ZMAX® coating. Some models available in stainless steel or ZMAX; see Corrosion Information, pp.20-24 or visit strongtie.com.

Installation:

- Use all specified fasteners; see General Notes.
- H1 can be installed with flanges facing inward (reverse of H1 installation drawing; number 1).
- H2.5T, H3 and H6 ties are shipped in equal quantities of right and left versions (right versions shown).
- Hurricane ties do not replace solid blocking.
- When installing ties on plated trusses (on the side opposite the truss plate) do not fasten through the truss plate from behind. This can force the truss plate off of the truss and compromise truss performance.
- H10A optional nailing to connect shear blocking, use 8d nails. Slots allow maximum field bending up to a pitch of 6:12, use H10A sloped loads for field bent installation.



H – Seismic and Hurricane Ties

SIMPSON
Strong-Tie

The H connector series provides wind and seismic ties for trusses and rafters.

Material: 18 gauge **Finish:** G90 galvanized

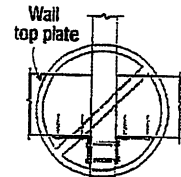
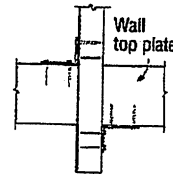
Design: • Factored resistances are in accordance with CSA O86-14
• Factored resistances have been increased 15%. No further increase is permitted.

Installation: • Use all specified fasteners

- Nails: 8d = 0.131" dia. x 2½" long common wire, 8d x 1½" = 0.131" x 1½" long, 10d x 1½" = 0.146" x 1½" long
- H1 can be installed with flanges facing outwards
- Hurricane ties do not replace solid blocking

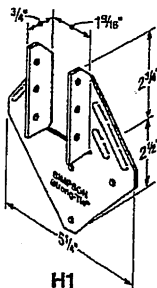
Factored resistances for more than one direction for a single connection cannot be added together. A factored load which can be divided into components in the directions given must be evaluated as follows: Factored Shear/Resisting Shear + Factored Tension/Resisting Tension ≤ 1.0.

Hurricane Tie Installations to Achieve Twice the Load (Top View)

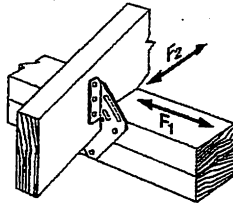


Install diagonally across from each other for minimum 2x truss.

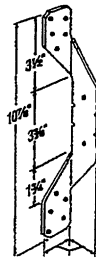
Nailing into both sides of a single ply 2x truss may cause the wood to split.



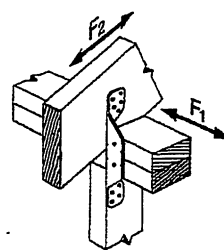
H1



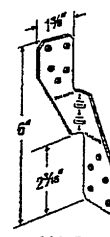
H1 Installation



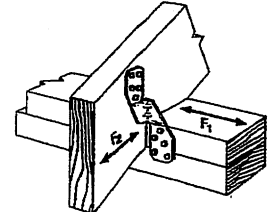
H2A



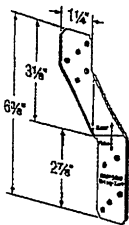
H2A Installation



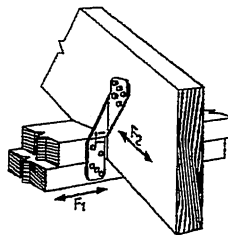
H2.5A



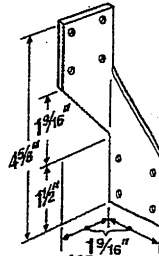
H2.5A Installation



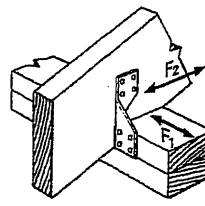
H2.5T



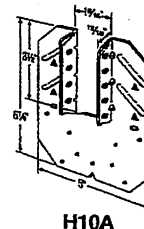
H2.5T Installation
(Nails into both top plates)



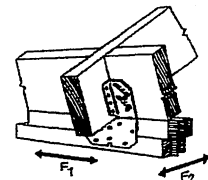
H3



H3 Installation



H10A

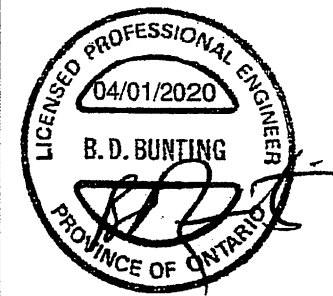


H10A Installation

Model No.	Ga.	Fasteners			Factored Resistance (lb.)					
		To Rafter	To Plates	To Studs	D.Fir-L			S-P-F		
					Uplift	Normal		Uplift	Normal	
						F ₁	F ₂		F ₁	F ₂
H1	18	(6) 8d x 1½"	(4) 8d	—	740	685	300	680	485	215
H2A	18	(5) 8d x 1½"	(2) 8d x 1½"	(5) 8d x 1½"	830	220	75	590	155	55
H2.5A	18	(5) 8d	(5) 8d	—	805	160	160	755	160	160
H2.5T	18	(5) 8d	(5) 8d	—	835	175	240	740	160	210
H3	18	(4) 8d	(4) 8d	—	740	180	265	615	125	190
H10A	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1735	795	410	1505	565	290

1. Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.
2. Factored resistances are for one anchor. A minimum rafter thickness of 2½" must be used when framing anchors are installed on each side of the joist and on the same side of the plate.

3. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.
4. Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a Continuous Load Path, connections must be on same side of the wall.



**LIMIT
STATES
DESIGN**

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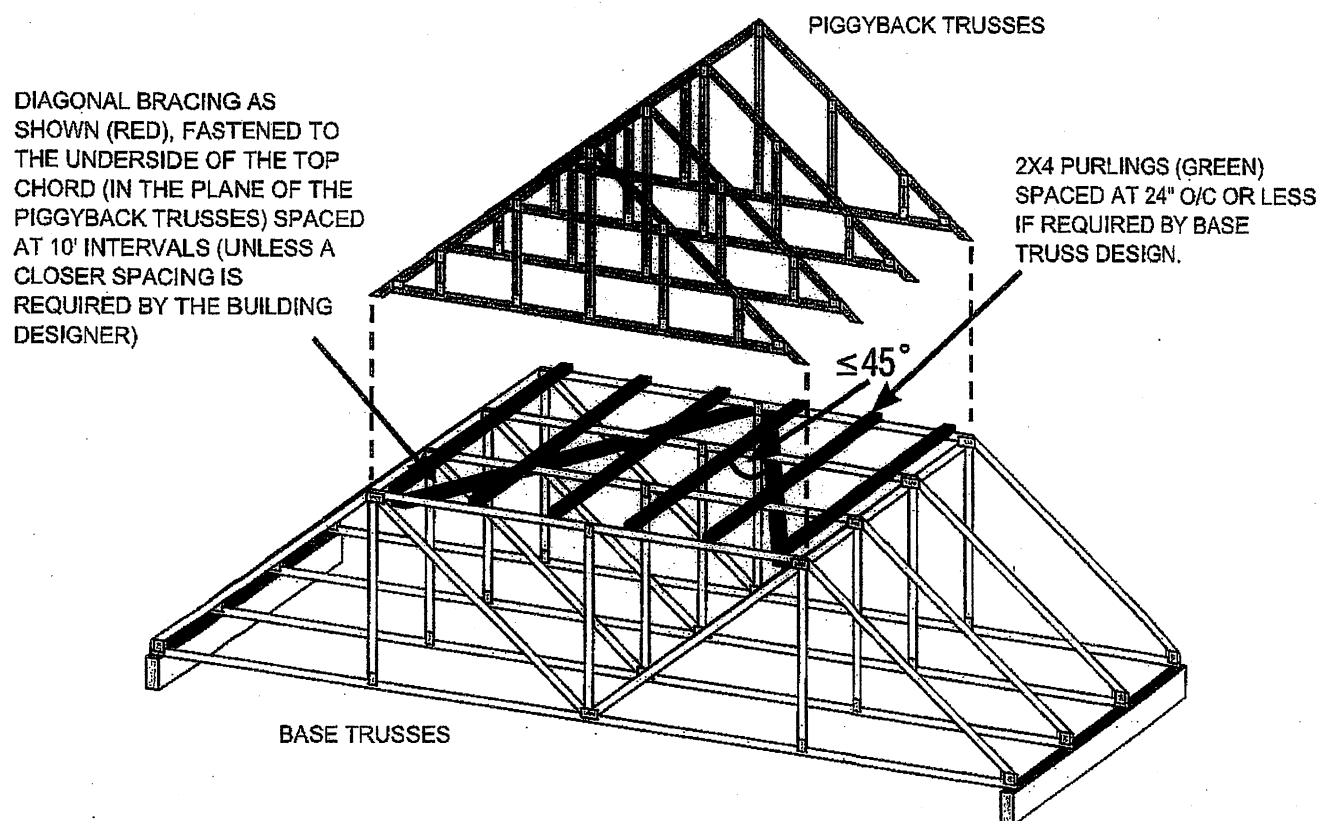
(800) 999-5099
strongtie.com

Overview:

Where piggybacks are connected ovetop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

TS/LTS/MTS/HTS

Twist Straps

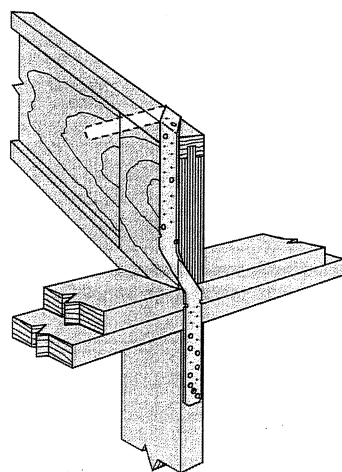
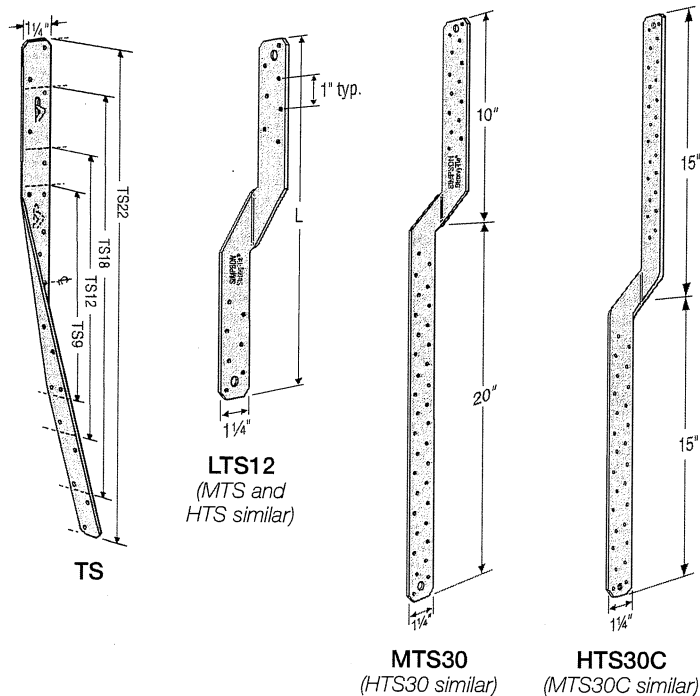
Twist straps provide a tension connection between two wood members. They resist uplift at the heel of a truss economically. The 3"-bend section eliminates interference at the transition points between wood members. TS twist straps come with an equal number of left and right hand units in each carton.

Material: LTS — 18 gauge; MTS/TS — 16 gauge; HTS — 14 gauge

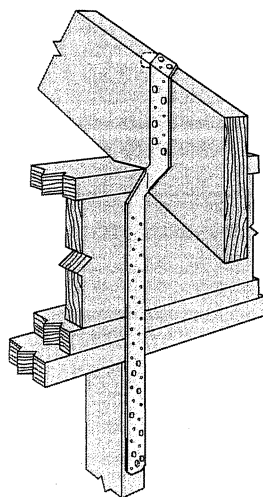
Finish: Galvanized. Some products available in stainless steel and ZMAX® coating; see Corrosion Information, pp.20–24.

Installation:

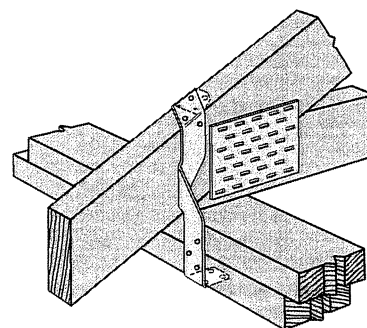
- Use all specified fasteners; see General Notes
- TS should be installed in pairs to reduce eccentricity
- When LTS/MTS is installed as truss-to-top plate tie, install (3) 10d x 1½" nails to the underside of the plate and (3) 10d x 1½" nails into the edge of the double top plate
- LTS, MTS and HTS are available with the bend reversed. Specify "-REV" after model number, such as MTS16-REV



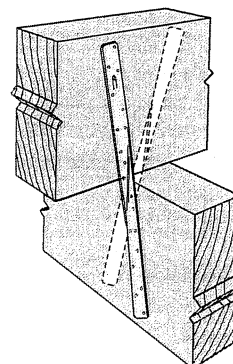
MTS30 Installation
with I-Joist Rafter



Typical MTS30
Installation



MTS Installation as a
Truss-to-Top Plate Tie



Typical TS
Installation

TS/LTS/MTS/HTS

Twist Straps (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

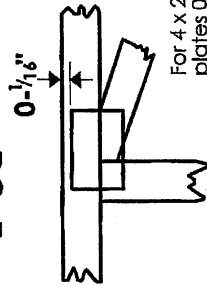
Model No.	L (in.)	Fasteners (Total)	Factored Resistance ($K_D = 1.15$)	
			D.Fir-L	S-P-F
			lb.	lb.
			kN	kN
TS9	9	(8) 16d	1125	1040
			5.00	4.63
TS12	11 3/4	(10) 16d	1410	1300
			6.27	5.78
TS18	17 3/4	(14) 16d	1970	1820
			8.76	8.10
TS22	21 3/4	(18) 16d	2125	2125
			9.45	9.45
SS LTS12	12	(12) 10d x 1 1/2"	1015	720
			4.52	3.20
LTS16	16	(12) 10d x 1 1/2"	1015	720
			4.52	3.20
SS LTS18	18	(12) 10d x 1 1/2"	1015	720
			4.52	3.20
LTS20	20	(12) 10d x 1 1/2"	1015	720
			4.52	3.20
MTS12	12	(14) 10d x 1 1/2"	1570	1180
			6.98	5.25
MTS16	16	(14) 10d x 1 1/2"	1570	1180
			6.98	5.25
MTS18	18	(14) 10d x 1 1/2"	1570	1180
			6.98	5.25
SS MTS20	20	(14) 10d x 1 1/2"	1570	1180
			6.98	5.25
MTS30	30	(14) 10d x 1 1/2"	1570	1180
			6.98	5.25
MTS24C	24	(14) 10d x 1 1/2"	1570	1180
			6.98	5.25
MTS30C	30	(14) 10d x 1 1/2"	1570	1180
			6.98	5.25
HTS16	16	(16) 10d x 1 1/2"	2050	1455
			9.12	6.47
HTS20	20	(24) 10d x 1 1/2"	2050	1455
			9.12	6.47
HTS24	24	(24) 10d x 1 1/2"	2050	1455
			9.12	6.47
HTS28	28	(24) 10d x 1 1/2"	2050	1455
			9.12	6.47
HTS30	30	(24) 10d x 1 1/2"	2050	1455
			9.12	6.47
HTS30C	30	(24) 10d x 1 1/2"	2050	1455
			9.12	6.47

1. LTS12 through LTS20, MTS16 through MTS30, HTS24 through HTS30C (except HTS30) have additional nail holes.
2. Install half of the fasteners on each end of strap to achieve maximum factored resistance.
3. Factored resistances have been increased 15% for earthquake or wind loading. No further increase allowed; reduce where other loads govern.
4. All straps except the MTS30 and HTS30 have the twist in the centre of the strap.
5. Twist straps do not have to be wrapped over the truss to achieve the load.
6. Optional nail holes are provided on some straps.
7. When used as a truss-to-top plate tie multiply the tabulated values by 0.95 for LTS and 0.74 for MTS. HTS cannot be used in this application.
8. **Nails:** 10d = 0.148" dia. x 3" long, 10d x 1 1/2" = 0.148" dia. x 1 1/2" long. See pp. 27–28 for other nail sizes and information.

Symbols

PLATE LOCATION AND ORIENTATION

Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

* Plate location details available in Mitek software or upon request.

PLATE SIZE

4 X 4

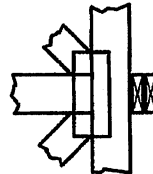
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



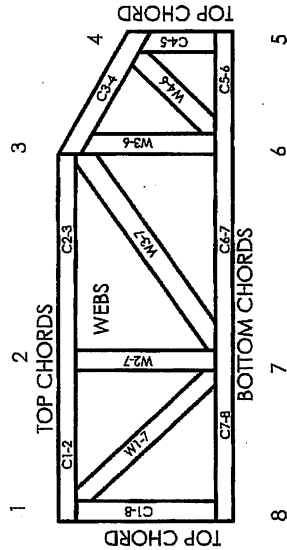
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

- TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
- DSB-89: Design Standard for Bracing.
- BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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Mitek
POWER TO PERFORM.™

Mitek Engineering Reference Sheet: MIL-7473C rev. 10-'08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.