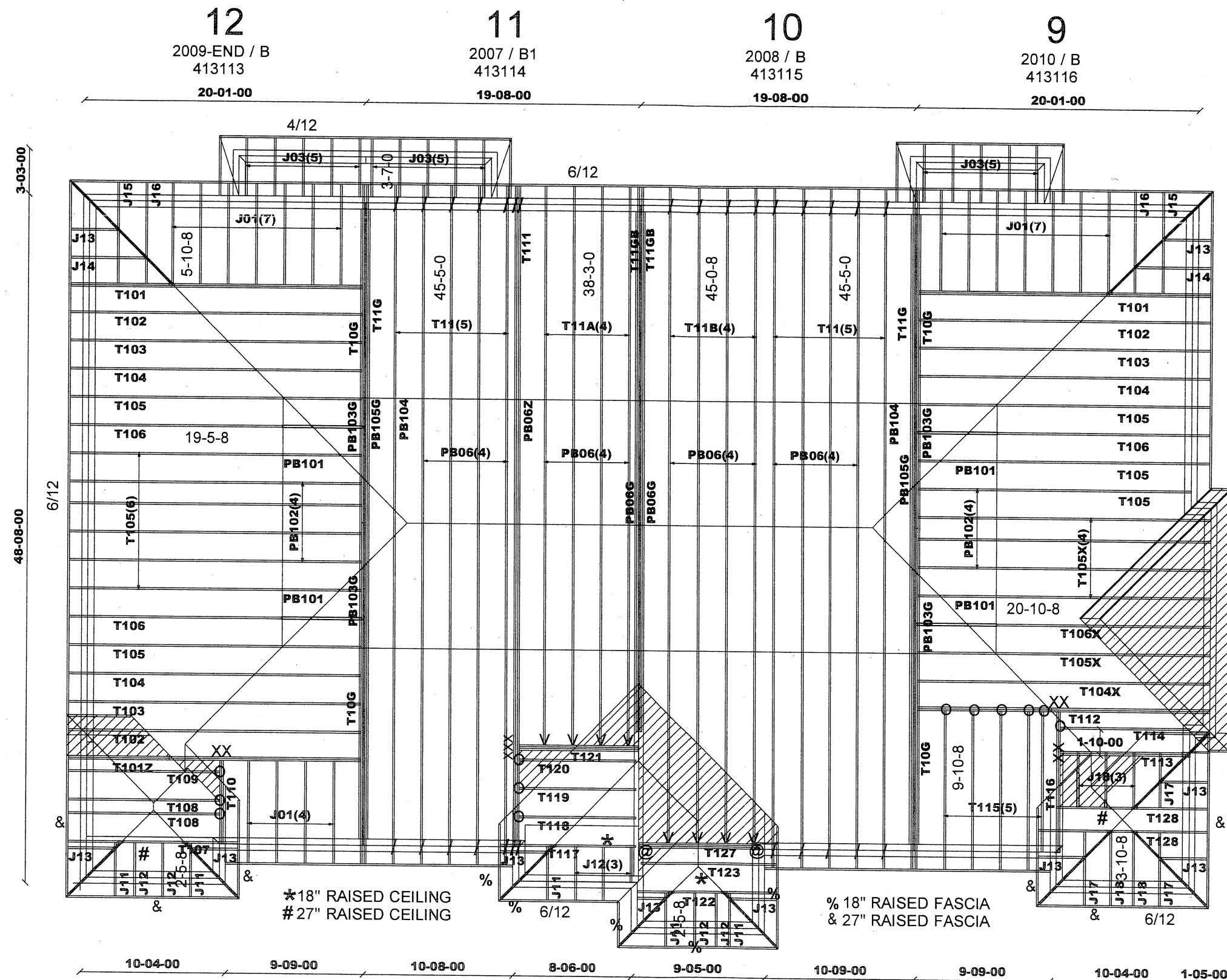




ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

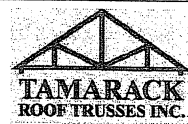
DESIGN CONFORMS WITH OBC 2012
OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 31.4 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 25.6 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

 DENOTES:
CONVENTIONAL
FRAMING

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)
HGUS26-3 - (XXX)
H2.5T - (/)
LGT3 - (@)

All conventional framing to conform with Part 9 of O.B.C. 2012. Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.



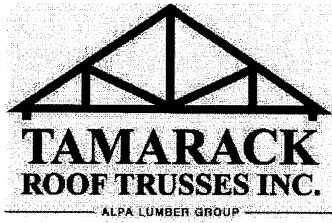
Job Track: **51012**
Plan Log: **203502**
Layout ID: **412865**

Builder / Location:
ROYAL PINE HOMES / RICHMOND HILL
Project: **CENTREFIELD**
Date: 2021-07-01 Sales: **Mario DiCano** Designer: AC

Model / Elevation:
BLOCK 56 / UNITS 9 - 12

Mitek ver 8.4.2.286

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.



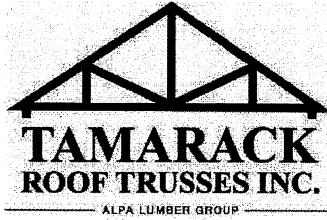
DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: CENTREFIELD
 Location: RICHMOND HILL
 Model: BLOCK 56
 Lot #:
 Elevation: B / UNIT12BLK282

Job Track: 51012
 PlanLog: 203502
 Layout ID: 413113
 Ref #
 Page: 1 of 2
 Date: 07-08-2021
 Designer: Andrew Conway
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	T10G GABLE	6 /12	22-08-08	8-01-04	2 x 4	1-05-00	1-02-00 8-01-04	243.75 153.67		
	1 2-ply	T101 Half Hip Girder	6 /12	19-05-08	4-01-04	2 x 4 2 x 6	1-03-08	1-02-00 4-01-04	182.72 116.00		
	1 2-ply	T101Z Half Hip Girder	6 /12	19-05-08	4-01-04	2 x 4 2 x 6	1-03-08	1-02-00 4-01-04	182.72 116.00		
	2	T102 Half Hip	6 /12	19-05-08	5-01-04	2 x 4	1-03-08	1-02-00 5-01-04	159.13 101.33		
	2	T103 Half Hip	6 /12	19-05-08	6-01-04	2 x 4	1-03-08	1-02-00 6-01-04	172.24 109.67		
	2	T104 Half Hip	6 /12	19-05-08	7-01-04	2 x 4	1-03-08	1-02-00 7-01-04	173.04 108.00		
	8	T105 Half Hip	6 /12	19-05-08	8-01-04	2 x 4	1-03-08	1-02-00 8-01-04	689.37 434.67		
	2	T106 Half Hip	6 /12	19-05-08	9-01-04	2 x 4	1-03-08	1-02-00 9-01-04	181.02 114.00		
	1	T107 Hip Girder	6 /12	9-06-00	2-04-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	37.9 25.50		
	2	T108 Hip	6 /12	9-06-00	4-10-12	2 x 4	1-03-08 1-03-08	2-08-00 2-08-00	99.25 67.00		
	1	T109 Hip	6 /12	9-06-00	3-10-12	2 x 4	1-03-08 1-03-08	2-08-00 2-08-00	45.71 31.67		
	1 2-ply	T110 Monopitch Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	58.39 37.67		
	2	PB101 Piggyback	6 /12	5-07-00	2-00-00	2 x 4		2-00-00	32.19 22.67		
	4	PB102 Piggyback	6 /12	5-07-00	2-09-08	2 x 4		2-09-08	58.98 40.00		



DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: CENTREFIELD
 Location: RICHMOND HILL
 Model: BLOCK 56
 Lot #:
 Elevation: B / UNIT12BLK282

Job Track: 51012
 PlanLog: 203502
 Layout ID: 413113
 Ref #
 Page: 2 of 2
 Date: 07-08-2021
 Designer: Andrew Conway
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	PB103G GABLE	6 /12	8-10-00	2-09-08	2 x 4		2-09-08	51.18 35.33		
	11	J01 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	184.74 117.33		
	5	J03 Jack-Open	4 /12	3-07-00	1-11-03	2 x 4	1-03-08	3-15 1-03-04	50.28 33.33		
	2	J11 Jack-Open	6 /12	1-10-15	2-01-08	2 x 4	1-03-08 6-09	1-02-00 2-01-08	15.69 10.67		
	2	J12 Jack-Open	6 /12	2-05-08	2-04-12	2 x 4	1-03-08	1-02-00 2-04-12	17.09 12.00		
	3	J13 Jack-Open	6 /12	1-10-15	2-01-08	2 x 4	1-03-08 1-01	1-02-00 2-01-08	21.97 14.00		
	1	J14 Jack-Open	6 /12	2-00-00	3-01-08	2 x 4	1-03-08 1-10-15	1-02-00 2-02-00	9.87 6.00		
	1	J15 Jack-Open	6 /12	1-10-15	2-01-08	2 x 4	1-03-08 3-11-09	1-02-00 2-01-08	11.74 7.33		
	1	J16 Jack-Open	6 /12	3-10-15	3-01-08	2 x 4	1-03-08 1-11-09	1-02-00 3-01-08	14.29 8.67		

TOTAL # TRUSS= 62

TOTAL BFT OF ALL TRUSSES= 1722.51

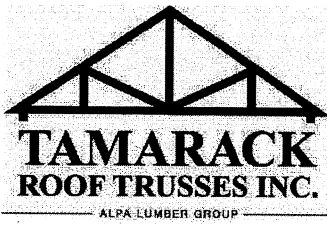
BFT.

TOTAL WEIGHT OF ALL TRSSES 2693.28 LBS

HARDWARE

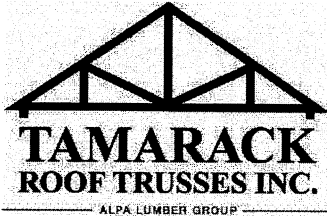
QTY	TYPE	MODEL	LENGTH
3	Hardware	LUS24	
1	Hardware	HGUS26-2	

TOTAL NUMBER OF ITEMS= 4

 TAMARACK ROOF TRUSSES INC. ALFA LUMBER GROUP	DELIVERY SHIPLIST							
	Lumber Yard: TAMARACK LUMBER Builder: ROYAL PINE HOMES Project: CENTREFIELD Location: RICHMOND HILL Model: BLOCK 56 Lot #: Elevation: B1 / UNIT11BLK282	Job Track: 51012 PlanLog: 203502 Layout ID: 413114 Ref # Page: 1 of 2 Date: 07-08-2021 Designer: Andrew Conway Sales Rep: Mario DiCano						

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	5	T11 Piggyback Base	6 /12	45-05-00	8-01-04	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	1279.34 775.00		
	4	T11A Piggyback Base	6 /12	38-03-00	8-01-04	2 x 6	1-03-08	1-02-00 1-02-00	886.16 540.00		
	1	T11G GABLE	6 /12	45-05-00	8-01-04	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	272.06 169.67		
	1	T11GB GABLE	6 /12	37-03-00	8-01-04	2 x 6	1-03-08	1-02-00 5-03-00	236.81 147.83		
	1 3-ply	T111 Piggyback Base Girder	6 /12	45-05-00	8-01-04	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	843.15 519.00		
	1	T117 Half Hip Girder	6 /12	8-04-08	2-04-12	2 x 4	1-03-08	1-02-00 2-04-12	34.08 22.83		
	1	T118 Half Hip	6 /12	8-04-08	4-10-12	2 x 4		2-08-00 4-10-12	39.12 26.00		
	1	T119 Half Hip	6 /12	8-04-08	5-10-12	2 x 4		2-08-00 5-10-12	45.3 29.67		
	1	T120 Monopitch	6 /12	8-04-08	6-10-04	2 x 4		2-08-00 6-10-04	41.6 27.33		
	1 3-ply	T121 Jack-Closed Girder	6 /12	8-06-08	6-11-04	2 x 4 2 x 6		2-08-00 6-11-04	141.44 94.00		
	8	PB06 Piggyback	6 /12	17-08-00	4-05-00	2 x 4			410.9 256.00		
	1	PB06G GABLE	6 /12	17-08-00	4-05-00	2 x 4			50.76 31.00		
	1 3-ply	PB06Z Piggyback	6 /12	17-08-00	4-05-00	2 x 4			154.09 96.00		
	1	PB104 Piggyback	6 /12	17-08-00	3-11-04	2 x 4			57.06 37.00		

 TAMARACK ROOF TRUSSES INC. ALPA LUMBER GROUP	DELIVERY SHIPLIST				
	Lumber Yard: TAMARACK LUMBER				Job Track: 51012
	Builder: ROYAL PINE HOMES				PlanLog: 203502
	Project: CENTREFIELD				Layout ID: 413114
	Location: RICHMOND HILL				Ref #
	Model: BLOCK 56				Page: 2 of 2
Lot #:				Date: 07-08-2021	
Elevation: B1 / UNIT11BLK282				Designer: Andrew Conway	
				Sales Rep: Mario DiCano	

Roof Trusses

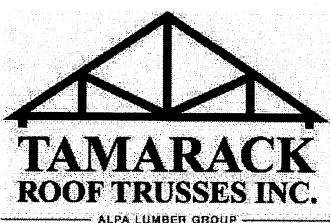
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	PB105G Piggyback	6 /12	8-10-00	2-08-00	2 x 4		2-09-08	25.27 17.67		
	5	J03 Jack-Open	4 /12	3-07-00	1-11-03	2 x 4	1-03-08	3-15 1-03-04	50.28 33.33		
	1	J11 Jack-Open	6 /12	1-10-15	2-01-08	2 x 4	1-03-08 6-09	1-02-00 2-01-08	7.85 5.33		
	3	J12 Jack-Open	6 /12	2-05-08	2-04-12	2 x 4	1-03-08	1-02-00 2-04-12	25.63 18.00		
	1	J13 Jack-Open	6 /12	1-10-15	2-01-08	2 x 4	1-03-08 1-01	1-02-00 2-01-08	7.32 4.67		

TOTAL # TRUSS= 45
TOTAL BFT OF ALL TRUSSES= 2850.33
BFT.
TOTAL WEIGHT OF ALL TRSSES 4608.21
LBS

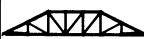
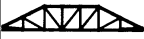
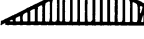
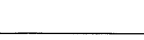
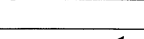
HARDWARE

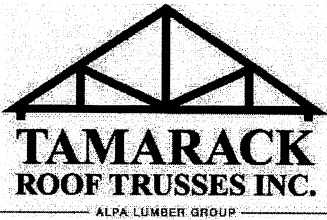
QTY	TYPE	MODEL	LENGTH
3	Hardware	LUS24	
4	Hardware	LJS26DS	
1	Hardware	HGUS26-3	
14	Hardware	H2.5T	

TOTAL NUMBER OF ITEMS= 22

DELIVERY SHIPLIST				
	Lumber Yard:	TAMARACK LUMBER	Job Track:	51012
	Builder:	ROYAL PINE HOMES	PlanLog:	203502
	Project:	CENTREFIELD	Layout ID:	413115
	Location:	RICHMOND HILL	Ref #	
	Model:	BLOCK 56	Page:	1 of 2
	Lot #:		Date:	07-08-2021
	Elevation:	B / UNIT10BLK282	Designer:	Andrew Conway
		Sales Rep:	Mario DiCano	

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	5	T11 Piggyback Base	6 /12	45-05-00	8-01-04	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	1279.34 775.00		
	4	T11B Piggyback Base	6 /12	45-00-08	8-01-04	2 x 6	1-03-08	1-02-00 1-02-00	1007.58 617.33		
	1	T11G GABLE	6 /12	45-05-00	8-01-04	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	272.06 169.67		
	1	T11GB GABLE	6 /12	37-03-00	8-01-04	2 x 6	1-03-08	1-02-00 5-03-00	236.81 147.83		
	1	T122 Hip Girder	6 /12	8-07-00	2-04-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	35.14 23.67		
	1	T123 Common	6 /12	8-03-08	3-03-12	2 x 4	1-03-08	1-02-00 1-03-12	32.19 20.83		
	1 3-ply	T127 Monopitch Girder	6 /12	8-05-08	4-09-12	2 x 4 2 x 6		2-08-00 2-08-12	124.02 84.50		
	8	PB06 Piggyback	6 /12	17-08-00	4-05-00	2 x 4			410.9 256.00		
	1	PB06G GABLE	6 /12	17-08-00	4-05-00	2 x 4			50.76 31.00		
	1	PB104 Piggyback	6 /12	17-08-00	3-11-04	2 x 4			57.06 37.00		
	1	PB105G Piggyback	6 /12	8-10-00	2-08-00	2 x 4		2-09-08	25.27 17.67		
	2	J11 Jack-Open	6 /12	1-10-15	2-01-08	2 x 4	1-03-08 6-09	1-02-00 2-01-08	15.69 10.67		
	2	J12 Jack-Open	6 /12	2-05-08	2-04-12	2 x 4	1-03-08	1-02-00 2-04-12	17.09 12.00		
	2	J13 Jack-Open	6 /12	1-10-15	2-01-08	2 x 4	1-03-08 1-01	1-02-00 2-01-08	14.65 9.33		

 TAMARACK ROOF TRUSSES INC. ALPHA LUMBER GROUP	DELIVERY SHIPLIST	
	Lumber Yard: TAMARACK LUMBER Builder: ROYAL PINE HOMES Project: CENTREFIELD Location: RICHMOND HILL Model: BLOCK 56 Lot #: Elevation: B / UNIT10BLK282	Job Track: 51012 PlanLog: 203502 Layout ID: 413115 Ref # Page: 2 of 2 Date: 07-08-2021 Designer: Andrew Conway Sales Rep: Mario DiCano

Roof Trusses

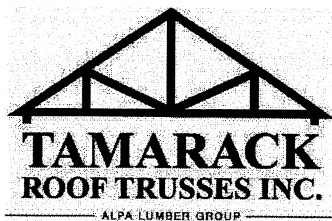
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
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TOTAL # TRUSS= 33
TOTAL BFT OF ALL TRUSSES= 2212.5
BFT.
TOTAL WEIGHT OF ALL TRSSES 3578.55
LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
4	Hardware	LJS26DS	
14	Hardware	H2.5T	
2	Hardware	LGT3	

TOTAL NUMBER OF ITEMS= 20



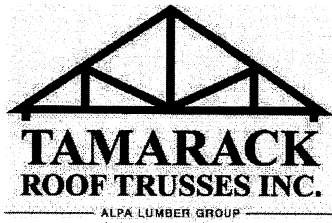
DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: CENTREFIELD
 Location: RICHMOND HILL
 Model: BLOCK 56
 Lot #:
 Elevation: B / UNIT9BLK282

Job Track: 51012
 PlanLog: 203502
 Layout ID: 413116
 Ref #
 Page: 1 of 3
 Date: 07-08-2021
 Designer: Andrew Conway
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	T10G GABLE	6 /12	22-08-08	8-01-04	2 x 4	1-05-00	1-02-00 8-01-04	243.75 153.67		
	1 2-ply	T101 Half Hip Girder	6 /12	19-05-08	4-01-04	2 x 4 2 x 6	1-03-08	1-02-00 4-01-04	182.72 116.00		
	1	T102 Half Hip	6 /12	19-05-08	5-01-04	2 x 4	1-03-08	1-02-00 5-01-04	79.56 50.67		
	1	T103 Half Hip	6 /12	19-05-08	6-01-04	2 x 4	1-03-08	1-02-00 6-01-04	86.12 54.83		
	1	T104 Half Hip	6 /12	19-05-08	7-01-04	2 x 4	1-03-08	1-02-00 7-01-04	86.52 54.00		
	1	T104X Half Hip	6 /12	19-05-08	7-01-04	2 x 4	1-03-08	1-02-00 7-01-04	88.21 54.67		
	3	T105 Half Hip	6 /12	19-05-08	8-01-04	2 x 4	1-03-08	1-02-00 8-01-04	258.51 163.00		
	5	T105X Piggyback Base	6 /12	19-05-08	8-01-04	2 x 4	1-03-08	1-02-00 8-01-04	439.32 275.00		
	1	T106 Half Hip	6 /12	19-05-08	9-01-04	2 x 4	1-03-08	1-02-00 9-01-04	90.51 57.00		
	1	T106X Half Hip	6 /12	19-05-08	9-01-04	2 x 4	1-03-08	1-02-00 9-01-04	92.67 58.17		
	1 2-ply	T112 Half Hip Girder	6 /12	20-10-08	6-01-04	2 x 4 2 x 6		5-08 6-01-04	215.22 127.67		
	1 2-ply	T113 Flat	0 /12	9-01-08	1-06-00	2 x 4		1-06-00 1-06-00	61.18 40.33		
	1	T114 Monopitch	6 /12	9-01-08	5-06-00	2 x 4		1-02-00 5-06-00	41.45 26.50		
	5	T115 Monopitch	6 /12	9-10-08	6-01-04	2 x 4	1-03-08	1-02-00 6-01-04	217.09 137.50		

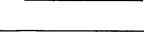


DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: CENTREFIELD
 Location: RICHMOND HILL
 Model: BLOCK 56
 Lot #:
 Elevation: B / UNIT9BLK282

Job Track: 51012
 PlanLog: 203502
 Layout ID: 413116
 Ref #
 Page: 2 of 3
 Date: 07-08-2021
 Designer: Andrew Conway
 Sales Rep: Mario DiCano

Roof Trusses

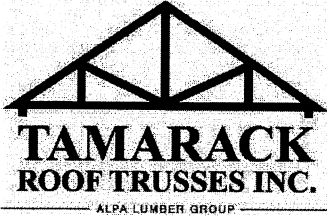
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T116 Flat Girder	0 /12	9-10-08	1-06-00	2 x 4 2 x 6		1-06-00 1-06-00	78.66 50.00		
	2	T128 Hip Girder	6 /12	9-06-00	3-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	80.58 51.67		
	2	PB101 Piggyback	6 /12	5-07-00	2-00-00	2 x 4		2-00-00	32.19 22.67		
	4	PB102 Piggyback	6 /12	5-07-00	2-09-08	2 x 4		2-09-08	58.98 40.00		
	2	PB103G GABLE	6 /12	8-10-00	2-09-08	2 x 4		2-09-08	51.18 35.33		
	7	J01 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	117.56 74.67		
	5	J03 Jack-Open	4 /12	3-07-00	1-11-03	2 x 4	1-03-08	3-15 1-03-04	50.28 33.33		
	4	J13 Jack-Open	6 /12	1-10-15	2-01-08	2 x 4	1-03-08 1-01	1-02-00 2-01-08	29.29 18.67		
	1	J14 Jack-Open	6 /12	2-00-00	3-01-08	2 x 4	1-03-08 1-10-15	1-02-00 2-02-00	9.87 6.00		
	1	J15 Jack-Open	6 /12	1-10-15	2-01-08	2 x 4	1-03-08 3-11-09	1-02-00 2-01-08	11.74 7.33		
	1	J16 Jack-Open	6 /12	3-10-15	3-01-08	2 x 4	1-03-08 1-11-09	1-02-00 3-01-08	14.29 8.67		
	3	J17 Jack-Open	6 /12	1-10-15	2-01-08	2 x 4	1-03-08 1-11-09	1-02-00 2-01-08	28.38 18.00		
	5	J18 Jack-Open	6 /12	3-10-08	3-01-04	2 x 4	1-03-08	1-02-00 3-01-04	59.82 36.67		

TOTAL # TRUSS= 67

TOTAL BFT OF ALL TRUSSES= 1772.02

BFT.

TOTAL WEIGHT OF ALL TRSSES 2805.68 LBS

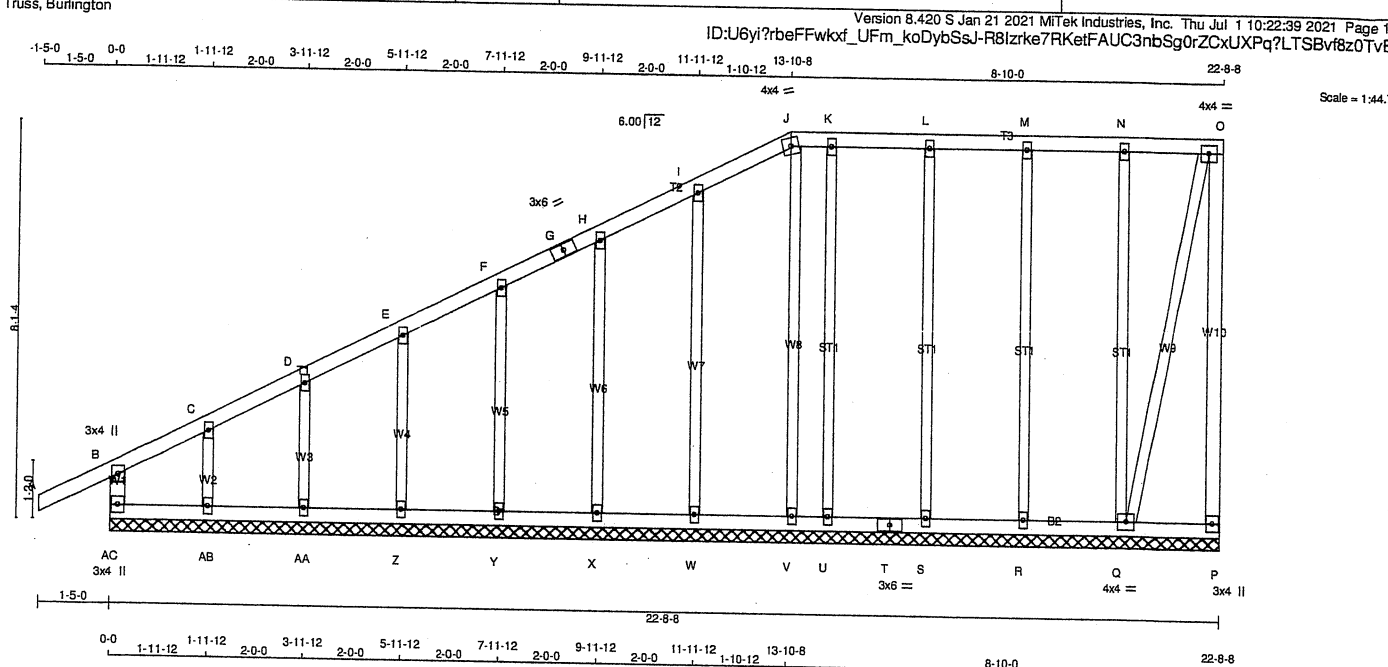
	DELIVERY SHIPLIST	
	Lumber Yard: TAMARACK LUMBER Builder: ROYAL PINE HOMES Project: CENTREFIELD Location: RICHMOND HILL Model: BLOCK 56 Lot #: Elevation: B / UNIT9BLK282	Job Track: 51012 PlanLog: 203502 Layout ID: 413116 Ref # Page: 3 of 3 Date: 07-08-2021 Designer: Andrew Conway Sales Rep: Mario DiCano

HARDWARE

QTY	TYPE	MODEL	LENGTH
6	Hardware	LUS24	
2	Hardware	HGUS26-2	

TOTAL NUMBER OF ITEMS= 8

JOB NAME 412868	TRUSS NAME T10G	QUANTITY 4	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
AC - B	2x4	DRY No.2	SPF
A - G	2x4	DRY No.2	SPF
G - J	2x4	DRY No.2	SPF
J - O	2x4	DRY No.2	SPF
P - O	2x4	DRY No.2	SPF
AC - T	2x4	DRY No.2	SPF
T - P	2x4	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

ALL GABLE WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2'-0" OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C, D, E, F, H, I, K, L, M, N						
C	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
J	TTW-m	MT20	4.0	4.0		
O	TMW-t	MT20	4.0	4.0		
P	BMV1+p	MT20	3.0	4.0		
Q	BMW1-t	MT20	4.0	4.0		
R, S, U, V, W, X, Y, Z, AA, AB						
R	BMW1+w	MT20	2.0	4.0		
T	BS-t	MT20	3.0	6.0		
AC	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD LC1 (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO				FR-TO			
AC-B	-271 / 0	0.0	0.03 (1)	Q-N	-200 / 0	0.25 (1)	
A-B	0 / 30	-91.8	-91.8 0.14 (1)	R-M	-182 / 0	0.23 (1)	
B-C	-21 / 0	-91.8	-91.8 0.14 (1)	S-L	-190 / 0	0.24 (1)	
C-D	0 / 10	-91.8	-91.8 0.05 (1)	U-K	-149 / 0	0.19 (1)	
D-E	0 / 9	-91.8	-91.8 0.05 (1)	V-J	-95 / 0	0.12 (1)	
E-F	0 / 14	-91.8	-91.8 0.04 (1)	AB-C	-118 / 0	0.02 (1)	
F-G	0 / 16	-91.8	-91.8 0.04 (1)	AA-D	-196 / 0	0.04 (1)	
G-H	0 / 16	-91.8	-91.8 0.04 (1)	Z-E	-179 / 0	0.05 (1)	
H-I	0 / 19	-91.8	-91.8 0.05 (1)	Y-F	-183 / 0	0.07 (1)	
I-J	0 / 13	-91.8	-91.8 0.05 (1)	X-H	-180 / 0	0.11 (1)	
J-K	0 / 20	-91.8	-91.8 0.03 (1)	W-I	-200 / 0	0.18 (1)	
K-L	0 / 20	-91.8	-91.8 0.05 (1)	Q-O	-79 / 0	0.11 (1)	
L-M	0 / 20	-91.8	-91.8 0.05 (1)				
M-N	0 / 20	-91.8	-91.8 0.05 (1)				
N-O	0 / 20	-91.8	-91.8 0.05 (1)				
P-O	0 / 0	0.0	0.0 0.00 (1)				
AC-AB	0 / 0	-18.5	-18.5 0.02 (4)				
AB-AA	-4 / 0	-18.5	-18.5 0.02 (4)				
AA-Z	-9 / 0	-18.5	-18.5 0.01 (4)				
Z-Y	-12 / 0	-18.5	-18.5 0.01 (4)				
Y-X	-14 / 0	-18.5	-18.5 0.01 (4)				
X-W	-16 / 0	-18.5	-18.5 0.01 (4)				
W-V	-18 / 0	-18.5	-18.5 0.02 (4)				
V-U	-20 / 0	-18.5	-18.5 0.02 (4)				
U-T	-20 / 0	-18.5	-18.5 0.01 (4)				
T-S	-20 / 0	-18.5	-18.5 0.02 (4)				
S-R	-20 / 0	-18.5	-18.5 0.02 (4)				
R-Q	-20 / 0	-18.5	-18.5 0.02 (4)				
Q-P	0 / 0	-18.5	-18.5 0.02 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (A-B:1), BC=0.02/1.00 (Q-R:4), WB=0.25/1.00 (N-Q:1), SS=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.59 (J) (INPUT = 0.90)
JSI METAL = 0.08 (I) (INPUT = 1.00)

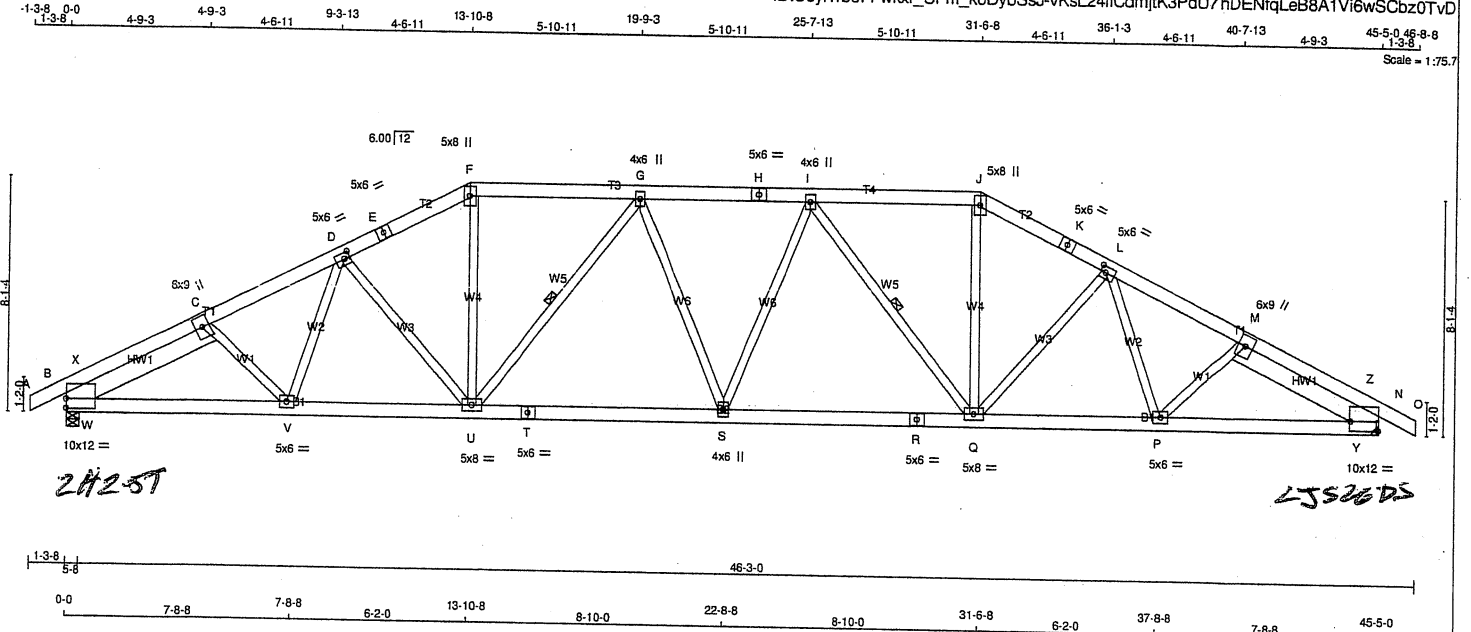


Structural component only
DWG# T-2121161

JOB NAME 412868	TRUSS NAME T11	QUANTITY 15	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

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LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - E	2x6	DRY	No.2
F - F	2x6	DRY	No.2
F - H	2x6	DRY	No.2
H - J	2x6	DRY	No.2
J - K	2x6	DRY	No.2
K - O	2x6	DRY	No.2
B - T	2x6	DRY	No.2
T - R	2x6	DRY	No.2
R - N	2x6	DRY	No.2

REINFORCING MEMBERS			
HW1	2x8	DRY	No.2
HW2	2x8	DRY	No.2

ALL WEBS			
2x4	DRY	No.2	SPF
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMBMW1m	MT20	10.0	12.0	4.00
C	TMBWW-t	MT20	6.0	9.0	
D	TMBWW-t	MT20	5.0	6.0	2.50 2.25
E, H, K					
E	TS-t	MT20	5.0	6.0	
F	TTW+p	MT20	5.0	8.0	
G	TMBWW-t	MT20	4.0	6.0	
I	TMBWW-t	MT20	4.0	6.0	
J	TTW+p	MT20	5.0	8.0	
L	TMBWW-t	MT20	5.0	6.0	2.50 2.25
M	TMBWW-t	MT20	6.0	9.0	
N	TMBMW1m	MT20	10.0	12.0	4.00 Edge
P	BMWW-t	MT20	5.0	6.0	
Q	BMWW-t	MT20	5.0	8.0	
R	BS-t	MT20	5.0	6.0	
S	BMWW-t	MT20	4.0	6.0	
T	BS-t	MT20	5.0	6.0	
U	BMWW-t	MT20	5.0	8.0	
V	BMWW-t	MT20	5.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	VERT	GROSS REACTION	DOWN	BRG	BRG
B	3675	0	3709	195	-1082
N	3675	0	3709	0	-1082

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT N. MINIMUM BEARING LENGTH AT JOINT N = 4-0.

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 1082 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT N FOR 1082 LBS. FACTORED UPLIFT

PROVIDE FOR 195 LBS. FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	2713	1634 / 0	477 / 0	0 / 0	0 / -1073	625 / 0	0 / 0
N	2713	1634 / 0	477 / 0	0 / 0	59 / -1073	625 / 0	0 / 0

HORIZONTAL REACTIONS		FACTORED	UNFACTORED
B	0 / 0	0 / 0	0 / 0
N	139 / -139	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 3.53 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-U, I-Q.

LOADING

TOTAL LOAD CASES: (18)

CHORDS		WEBS	
MEMB.	FORCE (LBS)	MEMB.	FORCE (LBS)
A-B	0 / 1	C-V	-32 / 371
B-X	-4338 / 1145	D-U	-90 / 264
C-D	-2853 / 891	E-F	-826 / 383
D-E	-5520 / 1704	F-G	-479 / 1736
E-F	-5000 / 1604	G-H	-1448 / 490
F-G	-5000 / 1604	H-I	-167 / 458
G-H	-4482 / 1504	I-J	-167 / 458
H-I	-5228 / 1649	J-K	-1448 / 490
I-J	-4482 / 1504	K-L	-479 / 1736
J-K	-5000 / 1604	L-M	-826 / 383
K-L	-5000 / 1604	M-N	-100 / 264
L-M	-5520 / 1705	N-O	-44 / 371
M-N	-2853 / 893	O-P	-379 / 1962
N-O	-4338 / 1148	P-Q	-2918 / 847
	0 / 1	Q-R	-2918 / 846
		R-S	-381 / 1962
B-W	-856 / 2550	S-T	
W-V	-1532 / 4864	T-U	
V-U	-1432 / 4893	U-T	
U-T	-1381 / 5160	T-S	
T-S	-1381 / 5160	S-R	
S-R	-1327 / 5160	R-Q	
R-Q	-1327 / 5160	Q-P	
Q-P	-1237 / 4893	P-Y	
P-Y	-1338 / 4864	Y-N	
Y-N	-864 / 2550		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	33.4	PSF
DL	=	6.0	PSF	
BOT CH.	LL	=	10.5	PSF
DL	=	7.4	PSF	
TOTAL LOAD	=	57.3	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF BCBC 2018, ABC 2019
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR NOT USED
- PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED.

(80 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR, EQUALS 33.4 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.51")
CALCULATED VERT. DEFL.(LL) = L/999 (0.24")
ALLOWABLE DEFL.(TL) = L/180 (3.03")
CALCULATED VERT. DEFL.(TL) = L/999 (0.34")

CSI: TC=0.41/1.00 (G-I-1), BC=0.78/1.00 (S-U-1), WB=0.65/1.00 (G-U-3), SS=0.24/1.00 (I-J-2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

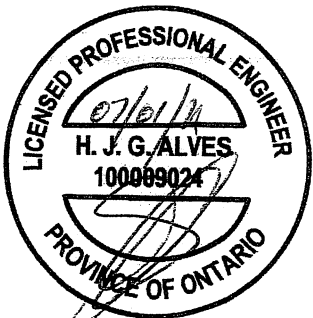
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650 371 1747 788 1987 1873	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (T) (INPUT = 0.90)
JSI METAL= 0.92 (T) (INPUT = 1.00)



Structural component only
DWG# T-2121162

JOB NAME 412868	TRUSS NAME T11	QUANTITY 15	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 1 10:22:40 2021 Page 2
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TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.2) PSF AT (31-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.



Structural component only
DWG# T-2121162 *ML*

[illegible]

JOB NAME 412868	TRUSS NAME T11G	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 1 10:22:42 2021 Page 2
 ID:U6yi?rbeFFwkxf Ufm koDvbSsJ-riz6Tmq?kF0R6eCnkV99ift5g8V3cBMo9QPZGTz0TvB

LOADING
TOTAL LOAD CASES: (4)

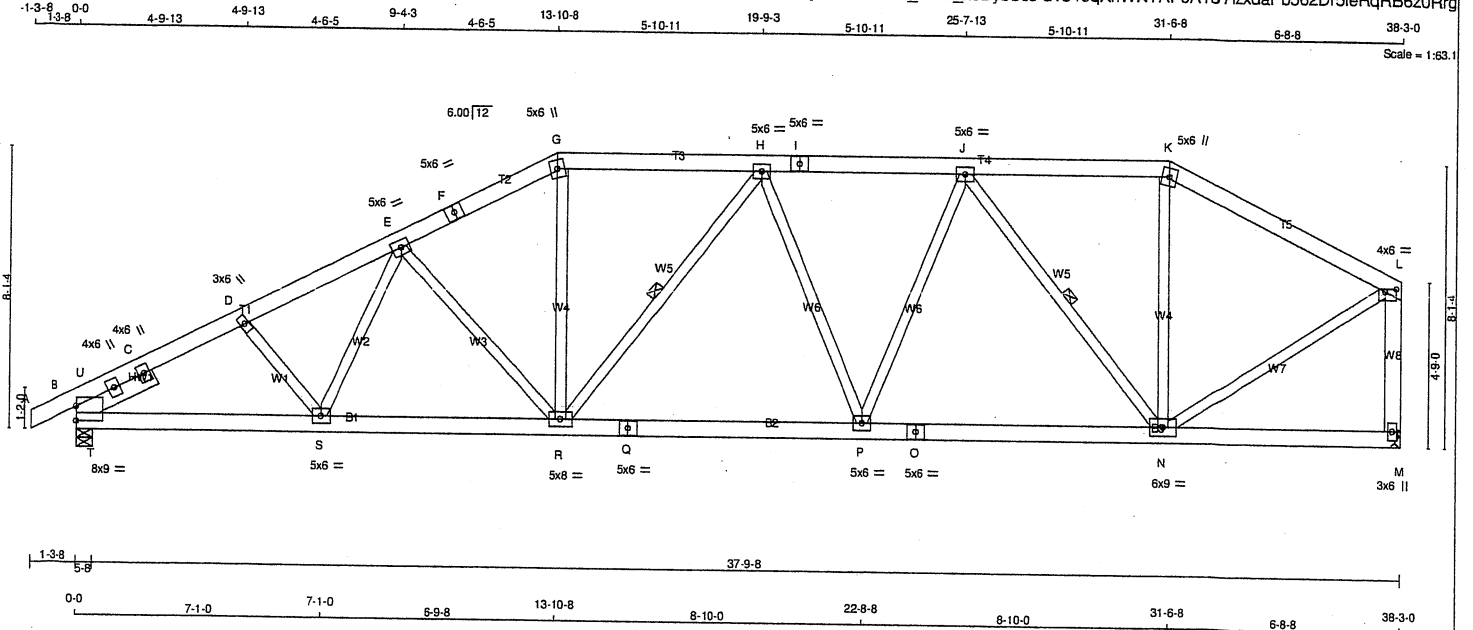
C H O R D S				W E B S			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO			FR-TO		
AI-AH	0 / 8	-18.5 -18.5	0.01 (4)	10.00			
AH-AG	0 / 12	-18.5 -18.5	0.01 (4)	10.00			
AG-AF	0 / 17	-18.5 -18.5	0.01 (4)	10.00			
AF-AE	0 / 25	-18.5 -18.5	0.01 (1)	10.00			
AE-AD	0 / 33	-18.5 -18.5	0.03 (1)	10.00			



Structural component only
 DWG# T-2121163 *me*

JOB NAME 413139	TRUSS NAME T11A	QUANTITY 8	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 M/Tek Industries, Inc. Thu Jul 1 12:42:59 2021 Page 1
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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - F	2x6 DRY	No.2	SPF		
F - G	2x6 DRY	No.2	SPF		
G - I	2x6 DRY	No.2	SPF		
I - K	2x6 DRY	No.2	SPF		
K - L	2x6 DRY	No.2	SPF		
M - L	2x6 DRY	No.2	SPF		
B - Q	2x6 DRY	No.2	SPF		
Q - O	2x6 DRY	No.2	SPF		
O - M	2x6 DRY	No.2	SPF		

REINFORCING MEMBERS				
HW1	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMBMR1-I	MT20	8.0	9.0	5.00	
B RT-t	MT20	4.0	6.0		
B RT-t	MT20	4.0	6.0		
D TMW-w	MT20	3.0	6.0		
E, H, J					
F TMW-w	MT20	5.0	6.0		
F TS-t	MT20	5.0	6.0		
G TTW-w	MT20	5.0	6.0		
I TS-t	MT20	5.0	6.0		
K TTW-w	MT20	5.0	6.0		
L TMW-w	MT20	4.0	6.0	1.00	3.75
M BMV1-p	MT20	3.0	6.0		
N BMWVW-t	MT20	6.0	9.0		
O BS-t	MT20	5.0	6.0		
P BMWVW-t	MT20	5.0	6.0		
Q BS-t	MT20	5.0	6.0		
R BMWVW-t	MT20	5.0	8.0		
S BMWVW-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT VERT	2237	0	0	0
B	2237	0	0	0
M	2109	0	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT M. MINIMUM BEARING LENGTH AT JOINT M = 3-8.

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS	1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT COMBINED	SNOW	LIVE	PERM. LIVE
B	1579	1050 / 0	0 / 0
M	1491	979 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.60 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF H-R, J-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	MAX	CS	MEMB.	MAX. FACTORED	FACTORED	VERT. LOAD	MAX	CS
MEMB.	FORCE (LBS)	FROM	TO	CS	MEMB.	FORCE (LBS)	FROM	TO	CS	MEMB.	FORCE (LBS)
FR-T	0 / 0	-91.8	-91.8	0.06 (1)	10.00	D-S	0 / 69	0.02 (4)			
B-U	-2908 / 0	-91.8	-91.8	0.05 (1)	4.91	S-E	0 / 65	0.02 (4)			
U-C	-2227 / 0	-91.8	-91.8	0.40 (1)	5.00	E-R	-550 / 0	0.36 (1)			
C-D	-2227 / 0	-91.8	-91.8	0.40 (1)	5.00	R-G	0 / 854	0.14 (1)			
D-E	-3092 / 0	-91.8	-91.8	0.23 (1)	4.89	H-P	-332 / 0	0.15 (1)			
E-F	-2766 / 0	-91.8	-91.8	0.17 (1)	4.89	P-J	0 / 506	0.08 (1)			
F-G	-2766 / 0	-91.8	-91.8	0.17 (1)	5.08	J-N	-1328 / 0	0.60 (1)			
G-H	-2465 / 0	-91.8	-91.8	0.20 (1)	5.04	N-K	0 / 252	0.04 (4)			
H-I	-2543 / 0	-91.8	-91.8	0.19 (1)	5.04	N-L	0 / 1836	0.30 (1)			
I-J	-2543 / 0	-91.8	-91.8	0.18 (1)	6.07	T-U	0 / 2337	0.00 (1)			
J-K	-1563 / 0	-91.8	-91.8	0.28 (1)	5.89	T-C	-2249 / 0	0.17 (1)			
K-L	-1744 / 0	-91.8	-91.8	0.35 (1)	10.00						
M-L	-2060 / 0	0.0	0.0	0.44 (1)	7.06						
B-T	0 / 1311	-18.5	-18.5	0.32 (1)	10.00						
T-S	0 / 2770	-18.5	-18.5	0.49 (1)	10.00						
S-R	0 / 2810	-18.5	-18.5	0.41 (1)	10.00						
R-Q	0 / 2664	-18.5	-18.5	0.39 (1)	10.00						
Q-P	0 / 2664	-18.5	-18.5	0.39 (1)	10.00						
P-O	0 / 2361	-18.5	-18.5	0.35 (1)	10.00						
O-N	0 / 2361	-18.5	-18.5	0.35 (1)	10.00						
N-M	0 / 0	-18.5	-18.5	0.12 (4)	10.00						

TOTAL WEIGHT = 8 X 238 = 1900 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.27")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL) = L/360 (1.27")
CALCULATED VERT. DEFL.(TL) = L/999 (0.21")

CSI: TC=0.44/1.00 (L-M:1), BC=0.49/1.00 (S-T:1), WB=0.60/1.00 (J-N:1), SS=0.31/1.00 (B-T:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

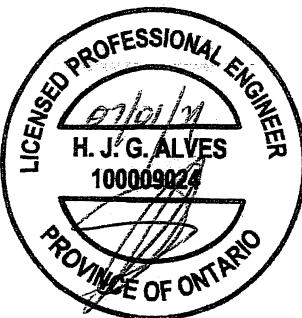
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

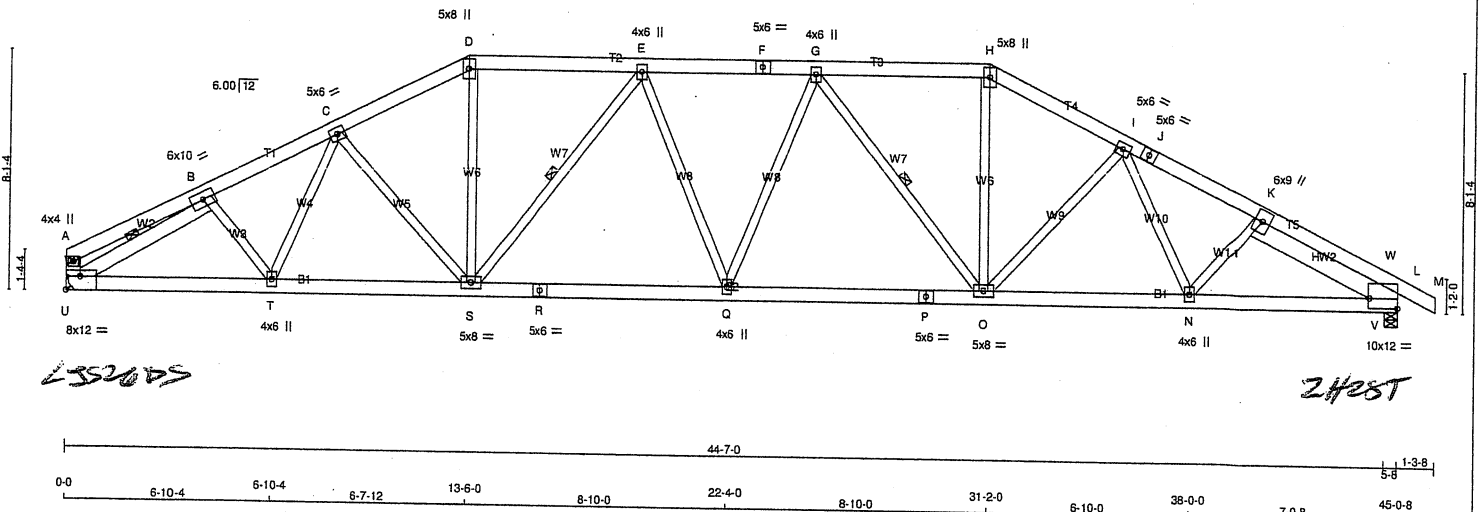
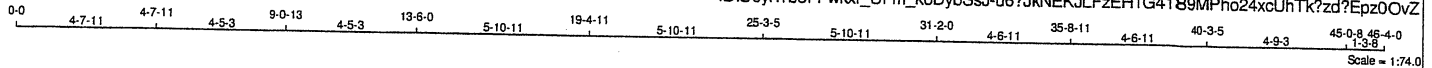
JSI GRIP = 0.82 (L) (INPUT = 0.90)
JSI METAL = 0.78 (B) (INPUT = 1.00)



Structural component only
DWG# T-2121237

JOB NAME 413139	TRUSS NAME T11B	QUANTITY 8	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x6 DRY	No.2	SPF		
D - F	2x6 DRY	No.2	SPF		
F - H	2x6 DRY	No.2	SPF		
H - J	2x6 DRY	No.2	SPF		
J - M	2x6 DRY	No.2	SPF		
U - A	2x6 DRY	No.2	SPF		
U - R	2x6 DRY	No.2	SPF		
R - P	2x6 DRY	No.2	SPF		
P - L	2x6 DRY	No.2	SPF		

REINFORCING MEMBERS	SIZE	LUMBER	DESCR.
HW2	2x8 DRY	No.2	SPF
ALL WEBS EXCEPT U - B	2x4 DRY	No.2	SPF
U - B	2x6 DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	4.0	4.0		
B	TMWW-t	MT20	6.0	10.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTW+p	MT20	5.0	8.0		
E	TMWW-t	MT20	4.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMWW-t	MT20	4.0	6.0		
H	TTW+p	MT20	5.0	8.0		
I	TMWW-t	MT20	5.0	6.0		
J	TS-t	MT20	5.0	6.0		
K	TMWW-t	MT20	6.0	9.0		
L	TMBMW1-t	MT20	10.0	12.0	4.00	Edge
N, Q, T						
N	BMWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	5.0	8.0		
P	BS-t	MT20	5.0	6.0		
R	BS-t	MT20	5.0	6.0		
S	BMWW-t	MT20	5.0	8.0		
U	BMWW-t	MT20	8.0	12.0	5.50	Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED	MAXIMUM FACTORED	INPUT	REORD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	HORZ	DOWN	HORZ
U	3485	0	3487
L	3646	0	3690

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT U. MINIMUM BEARING LENGTH AT JOINT U = 3-8.

PROVIDE ANCHORAGE AT BEARING JOINT U FOR 1004 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT L FOR 1065 LBS FACTORED UPLIFT

PROVIDE FOR 196 LBS FACTORED HORIZONTAL REACTION AT JOINT U

UNFACTORED REACTIONS

1ST CASE	MAX/MIN	COMPONENT REACTIONS
JT COMBINED	SNOW	LIVE
U	2582	1506 / 0
L	2692	1628 / 0

HORIZONTAL REACTIONS	U	0 / 0	0 / 0	0 / 0	120 / -140	0 / 0	0 / 0
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BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 3.56 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-S, G-O, B-U.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (18)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MEMB.	FORCE (LBS)
FR-TO			FR-TO	
A-B	-15 / 90	-115.2 -115.2 0.18 (2)	B-T	-48 / 520
B-C	-5244 / 1613	-115.2 -115.2 0.29 (2)	T-C	-193 / 181
C-D	-4868 / 1551	-115.2 -115.2 0.27 (2)	C-S	-695 / 340
D-E	-4364 / 1458	-115.2 -115.2 0.36 (1)	S-D	-455 / 1675
E-F	-5141 / 1615	-115.2 -115.2 0.40 (1)	S-E	-1471 / 495
F-G	-5141 / 1615	-115.2 -115.2 0.40 (1)	E-Q	-142 / 477
G-H	-4431 / 1478	-115.2 -115.2 0.36 (1)	Q-G	-185 / 424
H-I	-4943 / 1574	-115.2 -115.2 0.29 (3)	G-O	-1409 / 476
I-J	-5473 / 1687	-115.2 -115.2 0.32 (3)	O-H	-464 / 1703
J-K	-5473 / 1687	-115.2 -115.2 0.32 (3)	H-I	-813 / 384
K-W	-2810 / 889	-115.2 -115.2 0.15 (3)	I-N	-117 / 259
W-L	-4259 / 1142	-115.2 -115.2 0.18 (3)	N-K	-31 / 366
L-M	0 / 1	-115.2 -115.2 0.09 (3)	U-B	-5485 / 1539
U-A	-267 / 147	0.0 0.0 0.02 (13)	K-V	-2931 / 816
			V-W	-377 / 1904
U-T	-1416 / 4506	-39.5 -39.5 0.66 (1)		
T-S	-1357 / 4690	-39.5 -39.5 0.69 (1)		
S-R	-1339 / 5063	-39.5 -39.5 0.74 (1)		
R-Q	-1339 / 5063	-39.5 -39.5 0.74 (1)		
Q-P	-1292 / 5085	-39.5 -39.5 0.75 (1)		
P-O	-1292 / 5085	-39.5 -39.5 0.75 (1)		
O-N	-1220 / 4842	-39.5 -39.5 0.71 (1)		
N-V	-1311 / 4828	-39.5 -39.5 0.68 (1)		
V-L	-660 / 2510	-39.5 -39.5 0.30 (3)		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 33.4	PSF
DL	= 6.0	PSF	
BOT CH.	LL	= 10.5	PSF
DL	= 7.4	PSF	
TOTAL LOAD	= 57.3	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 4 OF CBC 2018, ABC 2019
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- SLOPE REDUCTION FACTOR NOT USED
- PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED.

(80 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR = EQUALS 33.4 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (1.50")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.24")
ALLOWABLE DEFL.(TL) = $L/180$ (3.00")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.34")

CSI: TC=0.40/1.00 (E-G:1), BC=0.75/1.00 (O-Q:1), WB=0.66/1.00 (E-S:3), SS=0.24/1.00 (D-E:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (B) (INPUT= 0.90)
JSI METAL= 0.93 (R) (INPUT= 1.00)



Structural component only
DWG# T-2121238

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
413139	T11B	8	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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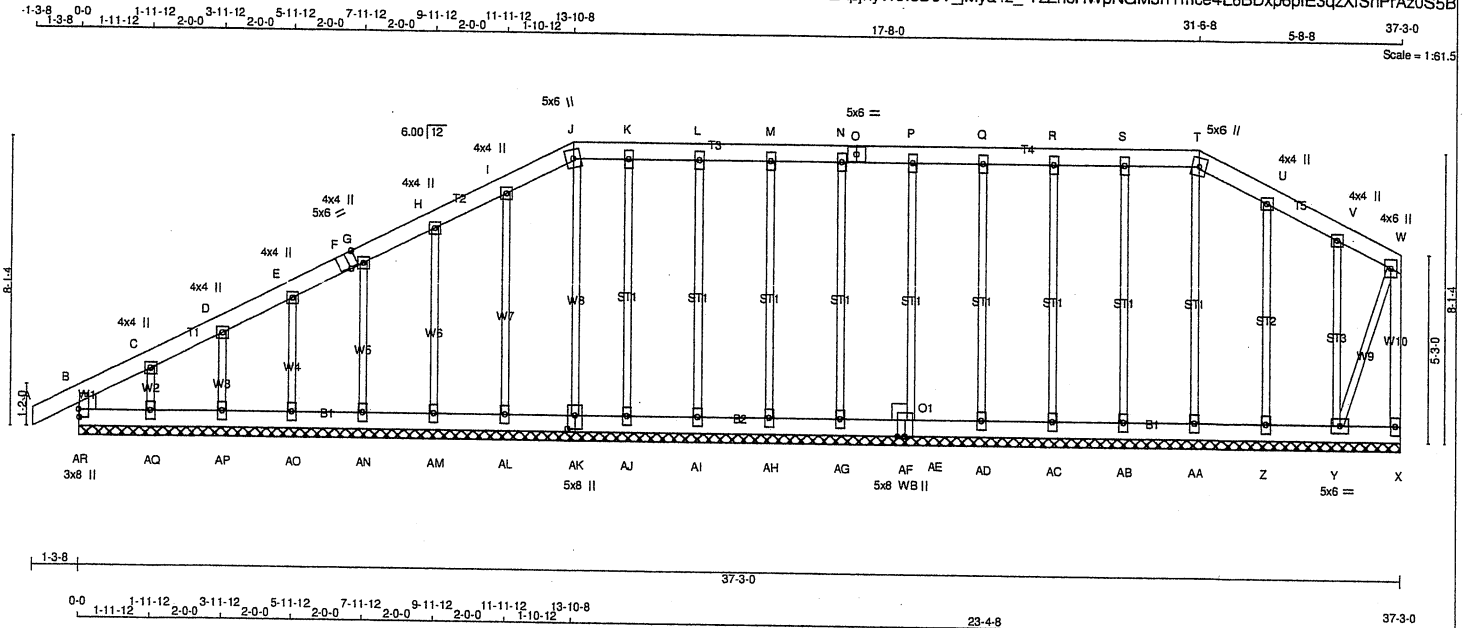
TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.2) PSF AT (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.



Structural component only
DWG# T-2121238 *mn*

JOB NAME 412865	TRUSS NAME T11GB	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 1 12:26:26 2021 Page 1	
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LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
AR - B	2x6 DRY	No.2	SPF	
A - F	2x6 DRY	No.2	SPF	
F - J	2x6 DRY	No.2	SPF	
J - O	2x6 DRY	No.2	SPF	
O - T	2x6 DRY	No.2	SPF	
T - W	2x6 DRY	No.2	SPF	
X - W	2x4 DRY	No.2	SPF	
AR- AK	2x6 DRY	No.2	SPF	
AK- AF	2x6 DRY	No.2	SPF	
AF- X	2x6 DRY	No.2	SPF	
ALL WEBS	2x3 DRY	No.2	SPF	
ALL GABLE WEBS	2x3 DRY	No.2	SPF	
DRY: SEASONED LUMBER.				
GABLE STUDS SPACED AT 2'-0" OC.				

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B, AE, AF, AR				
C, D, E, G, H, I, U, V				
C TMW+w	MT20	4.0	4.0	
F TS-t	MT20	5.0	6.0	Edge 2.75
J TTW+m	MT20	5.0	6.0	
K, L, M, N, P, Q, R, S				
K TMW+w	MT20	3.0	6.0	
O TS-t	MT20	5.0	6.0	
T TTW+m	MT20	5.0	6.0	
W TMW+w	MT20	4.0	6.0	
X BMV1+p	MT20	3.0	6.0	
Y BMVW1-t	MT20	5.0	6.0	
Z, AA, AB, AC, AD, AG, AH, AI, AJ, AL, AM, AN, AO, AP, AQ				
Z BMW1+w	MT20	3.0	6.0	
AE BSW1+l	MT20	5.0	8.0	
AK BSW1+l	MT20	5.0	8.0	4.50 2.50
AR TBMV1+p	MT20	3.0	8.0	2.75 0.25

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. (LC)	MEMB.	FORCE (LBS)	MAX. (LC)	
FR-TO		FROM	TO	FR-TO			
AR-B	-255 / 0	0.0	0.0 0.02 (1)	AA-T	-170 / 0	0.19 (1)	
A-B	0 / 29	-91.8	-91.8 0.06 (1)	AB-S	-206 / 0	0.23 (1)	
B-C	-18 / 0	-91.8	-91.8 0.06 (1)	AC-R	-183 / 0	0.21 (1)	
C-D	0 / 14	-91.8	-91.8 0.02 (1)	AD-Q	-183 / 0	0.21 (1)	
D-E	0 / 16	-91.8	-91.8 0.02 (1)	AE-P	-184 / 0	0.21 (1)	
E-F	0 / 23	-91.8	-91.8 0.02 (1)	AG-N	-183 / 0	0.21 (1)	
F-G	0 / 23	-91.8	-91.8 0.02 (1)	AH-M	-184 / 0	0.21 (1)	
G-H	0 / 27	-91.8	-91.8 0.02 (1)	AI-L	-186 / 0	0.21 (1)	
H-I	0 / 31	-91.8	-91.8 0.02 (1)	AJ-K	-178 / 0	0.20 (1)	
I-J	0 / 27	-91.8	-91.8 0.02 (1)	Z-U	-206 / 0	0.16 (1)	
J-K	0 / 33	-91.8	-91.8 0.02 (1)	Y-V	-173 / 0	0.09 (1)	
K-L	0 / 33	-91.8	-91.8 0.02 (1)	AK-J	-146 / 0	0.16 (1)	
L-M	0 / 33	-91.8	-91.8 0.02 (1)	AQ-C	-122 / 0	0.02 (1)	
M-N	0 / 33	-91.8	-91.8 0.02 (1)	AP-D	-193 / 0	0.03 (1)	
N-O	0 / 33	-91.8	-91.8 0.02 (1)	AO-E	-179 / 0	0.04 (1)	
O-P	0 / 33	-91.8	-91.8 0.02 (1)	AN-G	-181 / 0	0.06 (1)	
P-Q	0 / 33	-91.8	-91.8 0.02 (1)	AM-H	-182 / 0	0.09 (1)	
Q-R	0 / 33	-91.8	-91.8 0.03 (1)	AL-I	-198 / 0	0.15 (1)	
R-S	0 / 33	-91.8	-91.8 0.03 (1)	Y-W	-68 / 0	0.03 (1)	
S-T	0 / 33	-91.8	-91.8 0.03 (1)				
T-U	0 / 26	-91.8	-91.8 0.03 (1)				
U-V	0 / 34	-91.8	-91.8 0.03 (1)				
V-W	0 / 32	-91.8	-91.8 0.02 (1)				
X-W	0 / 9	0.0	0.0 0.00 (1)				

AR-AQ	0 / 0	-18.5	-18.5 0.01 (4)	10.00
AQ-AP	-8 / 0	-18.5	-18.5 0.01 (4)	10.00
AP-AO	-15 / 0	-18.5	-18.5 0.01 (4)	6.25
AO-AN	-20 / 0	-18.5	-18.5 0.01 (4)	6.25
AN-AM	-24 / 0	-18.5	-18.5 0.01 (4)	6.25
AM-AL	-28 / 0	-18.5	-18.5 0.01 (4)	6.25
AL-AK	-31 / 0	-18.5	-18.5 0.01 (4)	6.25
AK-AJ	-33 / 0	-18.5	-18.5 0.01 (4)	6.25
AJ-AI	-33 / 0	-18.5	-18.5 0.01 (4)	6.25
AI-AH	-33 / 0	-18.5	-18.5 0.01 (4)	6.25
AH-AG	-33 / 0	-18.5	-18.5 0.01 (4)	6.25
AG-AF	-33 / 0	-18.5	-18.5 0.01 (4)	6.25
AF-AE	-33 / 0	-18.5	-18.5 0.01 (4)	6.25
AE-AD	-33 / 0	-18.5	-18.5 0.01 (4)	6.25
AD-AC	-33 / 0	-18.5	-18.5 0.01 (4)	6.25
AC-AB	-33 / 0	-18.5	-18.5 0.01 (4)	6.25
AB-AA	-33 / 0	-18.5	-18.5 0.01 (4)	6.25
AA-Z	-30 / 0	-18.5	-18.5 0.01 (4)	6.25
Z-Y	-27 / 0	-18.5	-18.5 0.01 (4)	6.25
Y-X	0 / 0	-18.5	-18.5 0.01 (4)	10.00

TOTAL WEIGHT = 2 X 238 = 475 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
DL	=	6.0	PSF	
BOT CH.	LL	=	0.0	PSF
DL	=	7.4	PSF	
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.06/1.00 (A-B:1), BC=0.01/1.00 (AK-AL:4), WB=0.23/1.00 (S-AB:1), SSI=0.07/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.38 (Y) (INPUT = 0.90)

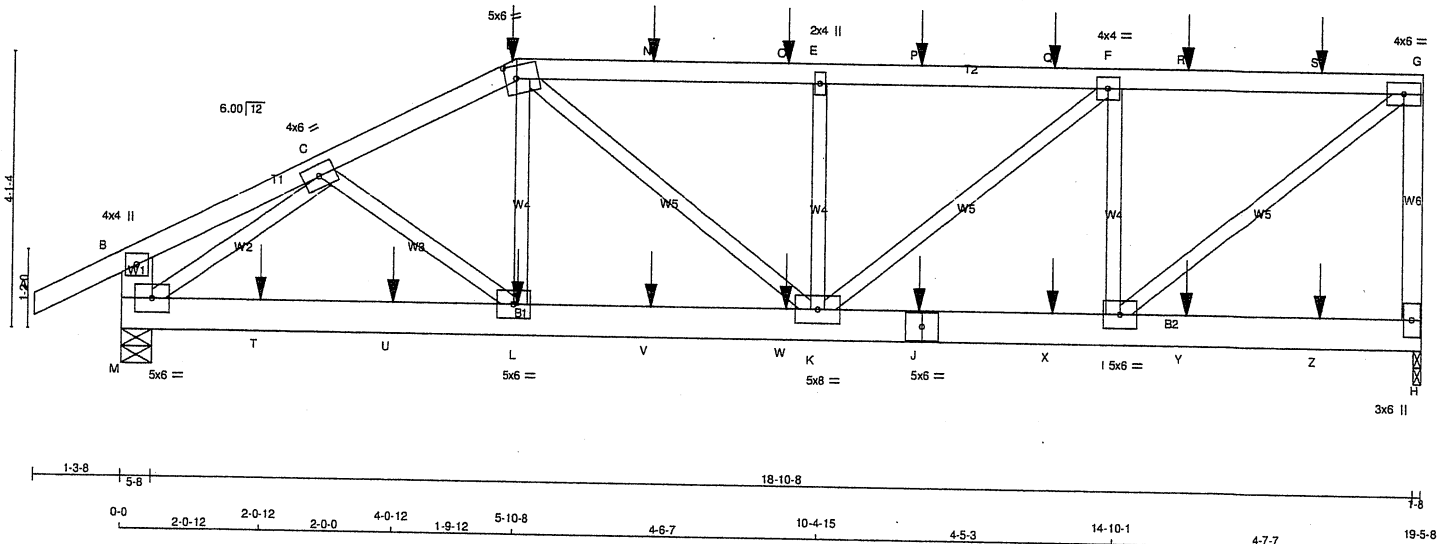
JSI METAL= 0.07 (AR) (INPUT = 1.00)



Structural component only
DWG# T-2121205

JOB NAME 412865	TRUSS NAME T101	QUANTITY 2	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 1 12:26:27 2021 Page 1 ID:hEKUVWZApjnyW9leD6V_jMya4z_-x973pdXR8aUwOikPcosLkRUwoCcwG5hX6XyNcz0SSa	

Scale = 1:32.7



LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - D 1	12	SIDE(61.0)
D - G 1	12	SIDE(61.0)
G - H 1	12	TOP
M - B 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
M - J 2	12	SIDE(183.1)
J - H 2	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
H	1774	0	1774	0	1-8
M	1855	0	1855	0	5-8

UNFACTORED REACTIONS

JT	1ST CASE	MAX./MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1255	820 / 0	0 / 0	0 / 0	0 / 0	0 / 0	435 / 0	0 / 0
M	1311	867 / 0	0 / 0	0 / 0	0 / 0	0 / 0	444 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.42 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX.	MEMB.	MAX. FACTORED	MAX
MEMB.	FORCE (LBS)	FROM	TO	PLF	CSI (LC)	UNBRAC	LENGTH	FR-TO	FORCE (LBS)	CSI (LC)
FR-TO										
A-B	0 / 28	-91.8	-91.8	0.07 (1)	10.00	C-L	0 / 214	0.03 (1)		
B-C	0 / 9	-91.8	-91.8	0.05 (1)	10.00	L-D	0 / 151	0.03 (4)		
C-D	-2468 / 0	-91.8	-91.8	0.07 (1)	5.69	M-C	-2546 / 0	0.30 (1)		
D-N	-2488 / 0	-91.8	-91.8	0.23 (1)	5.46	I-G	0 / 2332	0.29 (1)		
N-O	-2488 / 0	-91.8	-91.8	0.23 (1)	5.46	D-K	0 / 375	0.05 (1)		
O-E	-2488 / 0	-91.8	-91.8	0.23 (1)	5.46	I-F	-1302 / 0	0.17 (1)		
E-P	-2488 / 0	-91.8	-91.8	0.26 (1)	5.42	K-E	-702 / 0	0.09 (1)		
P-Q	-2488 / 0	-91.8	-91.8	0.26 (1)	5.42	K-F	0 / 880	0.11 (1)		
Q-F	-2488 / 0	-91.8	-91.8	0.26 (1)	5.42					
F-R	-1814 / 0	-91.8	-91.8	0.25 (1)	6.13					
R-S	-1814 / 0	-91.8	-91.8	0.25 (1)	6.13					
S-G	-1814 / 0	-91.8	-91.8	0.25 (1)	6.13					
H-G	-1712 / 0	0.0	0.0	0.21 (1)	7.81					
M-B	-239 / 0	0.0	0.0	0.01 (1)	7.81					
M-T	0 / 2025	-18.5	-18.5	0.17 (1)	10.00					
T-U	0 / 2025	-18.5	-18.5	0.17 (1)	10.00					
U-L	0 / 2025	-18.5	-18.5	0.17 (1)	10.00					
L-V	0 / 2198	-18.5	-18.5	0.17 (1)	10.00					
V-W	0 / 2198	-18.5	-18.5	0.17 (1)	10.00					
W-K	0 / 2198	-18.5	-18.5	0.17 (1)	10.00					
K-J	0 / 1814	-18.5	-18.5	0.15 (1)	10.00					
J-X	0 / 1814	-18.5	-18.5	0.15 (1)	10.00					
X-I	0 / 1814	-18.5	-18.5	0.15 (1)	10.00					
I-Y	0 / 0	-18.5	-18.5	0.04 (4)	10.00					
Y-Z	0 / 0	-18.5	-18.5	0.04 (4)	10.00					
Z-H	0 / 0	-18.5	-18.5	0.04 (4)	10.00					

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	5-10-8	-317	-317	---	BACK	VERT	TOTAL	---	C1
J	11-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
L	5-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
N	7-11-4	-76	-76	---	BACK	VERT	TOTAL	---	C1
O	9-11-4	-76	-76	---	BACK	VERT	TOTAL	---	C1
P	11-11-4	-76	-76	---	BACK	VERT	TOTAL	---	C1
Q	13-11-4	-76	-76	---	BACK	VERT	TOTAL	---	C1
R	15-11-4	-76	-76	---	BACK	VERT	TOTAL	---	C1
S	17-11-4	-76	-76	---	BACK	VERT	TOTAL	---	C1
T	2-0-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
U	4-0-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
V	7-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
W	9-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1

TOTAL WEIGHT = 4 X 91 = 365 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL)= L/999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL)= L/999 (0.07")

CSI: TC=0.26/1.00 (E-F:1), BC=0.17/1.00 (L-M:1), WB=0.30/1.00 (C-M:1), SS=0.18/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (G) (INPUT = 0.90)
JSI METAL= 0.30 (C) (INPUT = 1.00)



Structural component only
DWG# T-2121206 1/2

CONTINUED ON PAGE 2

JOB NAME 412865	TRUSS NAME T101	QUANTITY 2	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	6.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	5.0	8.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMVW1-t	MT20	5.0	6.0		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
X	13-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
Y	15-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
Z	17-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

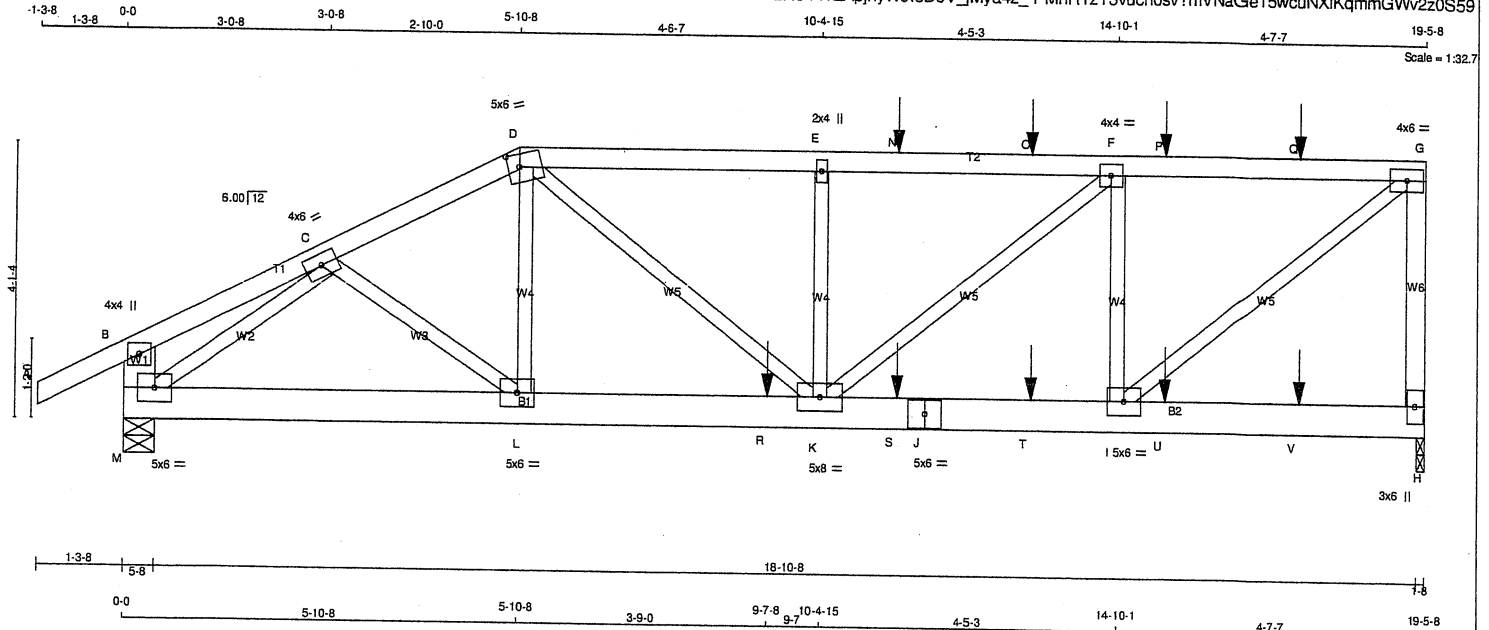


Structural component only
DWG# T-2121206 *mn*

JOB NAME 412865	TRUSS NAME T101Z	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - G	2x4	DRY	No.2	SPF	
H - M	2x4	DRY	No.2	SPF	
M - B	2x6	DRY	No.2	SPF	
M - J	2x6	DRY	No.2	SPF	
J - H	2x6	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - D 1	12	TOP
D - G 1	12	SIDE(0.0)
G - H 1	12	TOP
H - M 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
M - J 2	12	SIDE(0.0)
J - H 2	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
H	2170	0	2170	0
M	2037	0	2037	0

UNFACTORED REACTIONS

JT	1ST LOASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1531	1022 / 0	0 / 0	0 / 0	0 / 0	0 / 0	509 / 0	0 / 0
M	1434	977 / 0	0 / 0	0 / 0	0 / 0	0 / 0	458 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX.	MEMB.	MAX. FACTORED	FORCE	MAX
MEMB.	FORCE	(PLF)	FROM	TO	CSI (LC)	UNBRAC	FR-TO	FORCE	(LBS)	CSI (LC)
A-B	0 / 28	-91.8	-91.8	0.07 (1)	10.00	C-L	0 / 295	0.04 (1)		
B-C	0 / 9	-91.8	-91.8	0.06 (1)	10.00	L-D	0 / 176	0.02 (4)		
C-D	-2873 / 0	-91.8	-91.8	0.08 (1)	5.35	M-C	-2920 / 0	0.35 (1)		
D-E	-3565 / 0	-91.8	-91.8	0.19 (1)	4.80	I-G	0 / 2975	0.37 (1)		
E-N	-3565 / 0	-91.8	-91.8	0.30 (1)	4.66	D-K	0 / 1296	0.16 (1)		
N-O	-3565 / 0	-91.8	-91.8	0.30 (1)	4.66	I-F	-1812 / 0	0.23 (1)		
O-F	-3565 / 0	-91.8	-91.8	0.30 (1)	4.66	K-E	-529 / 0	0.07 (1)		
F-P	-2315 / 0	-91.8	-91.8	0.27 (1)	5.56	K-F	0 / 1635	0.20 (1)		
P-Q	-2315 / 0	-91.8	-91.8	0.27 (1)	5.56					
Q-G	-2315 / 0	-91.8	-91.8	0.27 (1)	5.56					
H-G	-2095 / 0	0.0	0.0	0.26 (1)	7.65					
M-B	-241 / 0	0.0	0.0	0.01 (1)	7.81					
M-L	0 / 2322	-18.5	-18.5	0.22 (1)	10.00					
L-R	0 / 2564	-18.5	-18.5	0.41 (1)	10.00					
R-K	0 / 2564	-18.5	-18.5	0.41 (1)	10.00					
K-S	0 / 2315	-18.5	-18.5	0.25 (1)	10.00					
S-J	0 / 2315	-18.5	-18.5	0.25 (1)	10.00					
J-T	0 / 2315	-18.5	-18.5	0.25 (1)	10.00					
T-I	0 / 2315	-18.5	-18.5	0.25 (1)	10.00					
I-U	0 / 0	-18.5	-18.5	0.04 (4)	10.00					
U-V	0 / 0	-18.5	-18.5	0.04 (4)	10.00					
V-H	0 / 0	-18.5	-18.5	0.04 (4)	10.00					

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
N	11-6-12	-76	-76	---	FRONT	VERT	TOTAL	C1
O	13-6-12	-76	-76	---	FRONT	VERT	TOTAL	C1
P	15-6-12	-76	-76	---	FRONT	VERT	TOTAL	C1
Q	17-6-12	-76	-76	---	FRONT	VERT	TOTAL	C1
R	9-7-8	-974	-974	---	FRONT	VERT	TOTAL	C1
S	11-6-12	-21	-21	---	FRONT	VERT	TOTAL	C1
T	13-6-12	-21	-21	---	FRONT	VERT	TOTAL	C1
U	15-6-12	-21	-21	---	FRONT	VERT	TOTAL	C1
V	17-6-12	-21	-21	---	FRONT	VERT	TOTAL	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 91 = 183 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
DL = 6.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.4	PSF	
TOTAL LOAD = 39.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL) = L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CSI: TC=0.30/1.00 (E-F:1), BC=0.41/1.00 (K-L:1),
WB=0.37/1.00 (G-I:1), SSI=0.40/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.88 (G) (INPUT = 0.90)
JSI METAL= 0.34 (C) (INPUT = 1.00)



Structural component only
DWG# T-2121207

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
412865	T101Z	1	2	ROYAL PINE HOMES	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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PLATES (table is in inches)

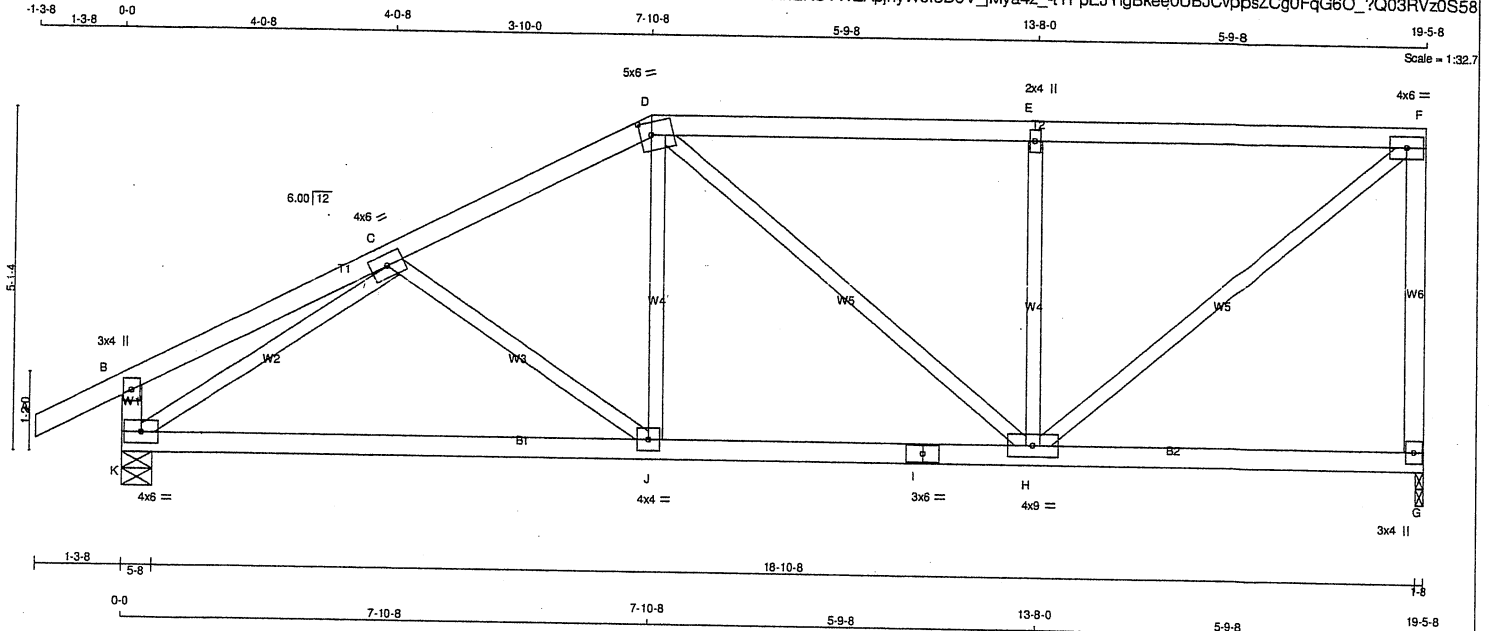
JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	6.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	5.0	8.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMVW1-t	MT20	5.0	6.0		



Structural component only
DWG# T-2121207 *m*

JOB NAME 412865	TRUSS NAME T102	QUANTITY 3	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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LUMBER				
N. L. G. A. RULES				
CHORDS		SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	4.0	6.0		
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMV+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMVWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMVW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
G	1073	0	1073	0
K	1197	0	1197	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
G	COMBINED	SNOW	LIVE	PERM.LIVE	WIND
G	759	498 / 0	0 / 0	0 / 0	261 / 0
K	844	567 / 0	0 / 0	0 / 0	277 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.39 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	CS1 (LC)	UNBRAC	MEMB.	WEBS	MAX. FACTORED	MAX	CS1 (LC)
		FORCE	VERT. LOAD	LC1	MAX	CS1 (LC)	UNBRAC	MEMB.			FORCE	MAX	CS1 (LC)
		(LBS)	(LBS)	(LBS)	(LBS)	(LBS)	(LBS)				(LBS)	(LBS)	(LBS)
FR-TO								FR-TO					
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00			C-J	-182 / 2	0.07 (1)			
B-C	0 / 18	-91.8	-91.8	0.21 (1)	10.00			J-D	0 / 253	0.06 (4)			
C-D	-1207 / 0	-91.8	-91.8	0.24 (1)	5.54			D-H	-94 / 0	0.10 (1)			
D-E	-996 / 0	-91.8	-91.8	0.55 (1)	5.39			H-E	-658 / 0	0.25 (1)			
E-F	-996 / 0	-91.8	-91.8	0.56 (1)	5.39			H-F	0 / 1291	0.29 (1)			
G-F	-1026 / 0	0.0	0.0	0.45 (1)	7.74			K-C	-1491 / 0	0.57 (1)			
K-B	-266 / 0	0.0	0.0	0.03 (1)	7.81								
K-J	0 / 1212	-18.5	-18.5	0.34 (4)	10.00								
J-I	0 / 1068	-18.5	-18.5	0.34 (4)	10.00								
I-H	0 / 1068	-18.5	-18.5	0.34 (4)	10.00								
H-G	0 / 0	-18.5	-18.5	0.13 (4)	10.00								

TOTAL WEIGHT = 3 X 80 = 239 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL)= L/999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL)= L/999 (0.15")

CSI: TC=0.58/1.00 (E-F:1), BC=0.34/1.00 (H-J:4),
WB=0.57/1.00 (C-K:1), SSI=0.28/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

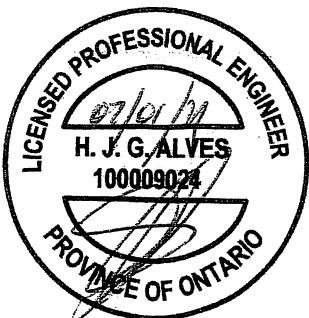
NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

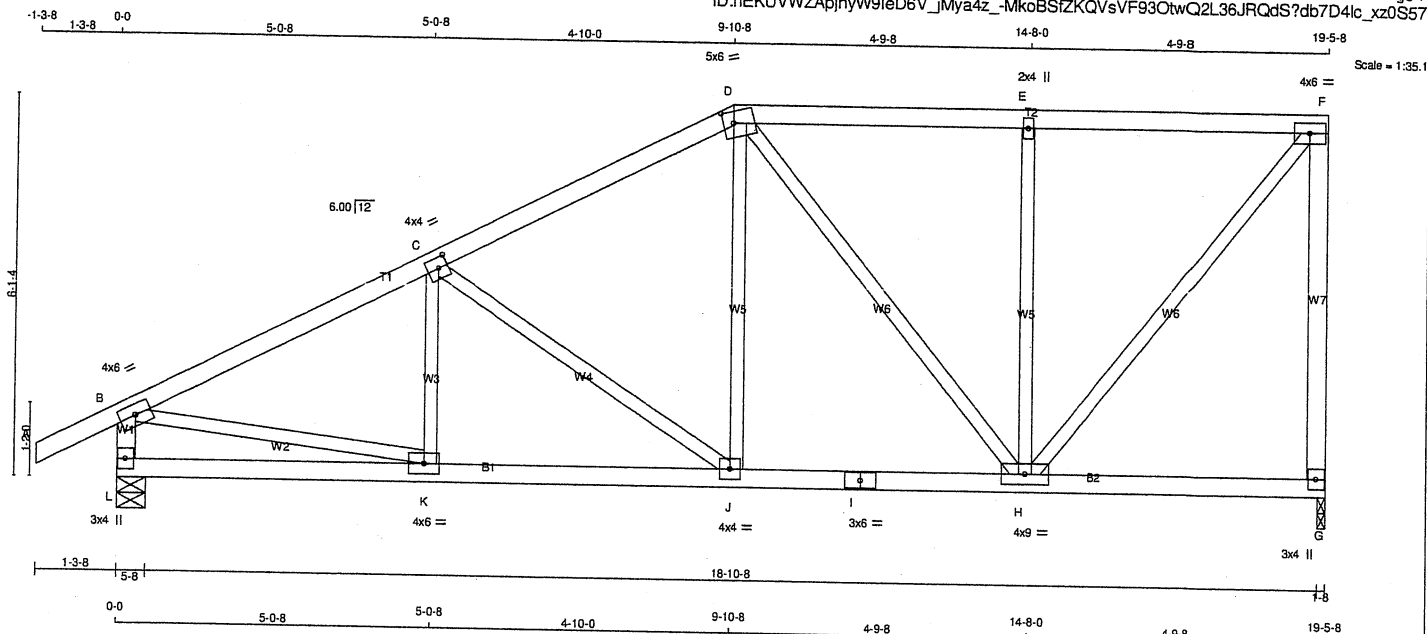
JSI GRIP= 0.78 (F) (INPUT = 0.90)
JSI METAL= 0.32 (C) (INPUT = 1.00)



Structural component only
DWG# T-2121208

JOB NAME 412865	TRUSS NAME T103	QUANTITY 3	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

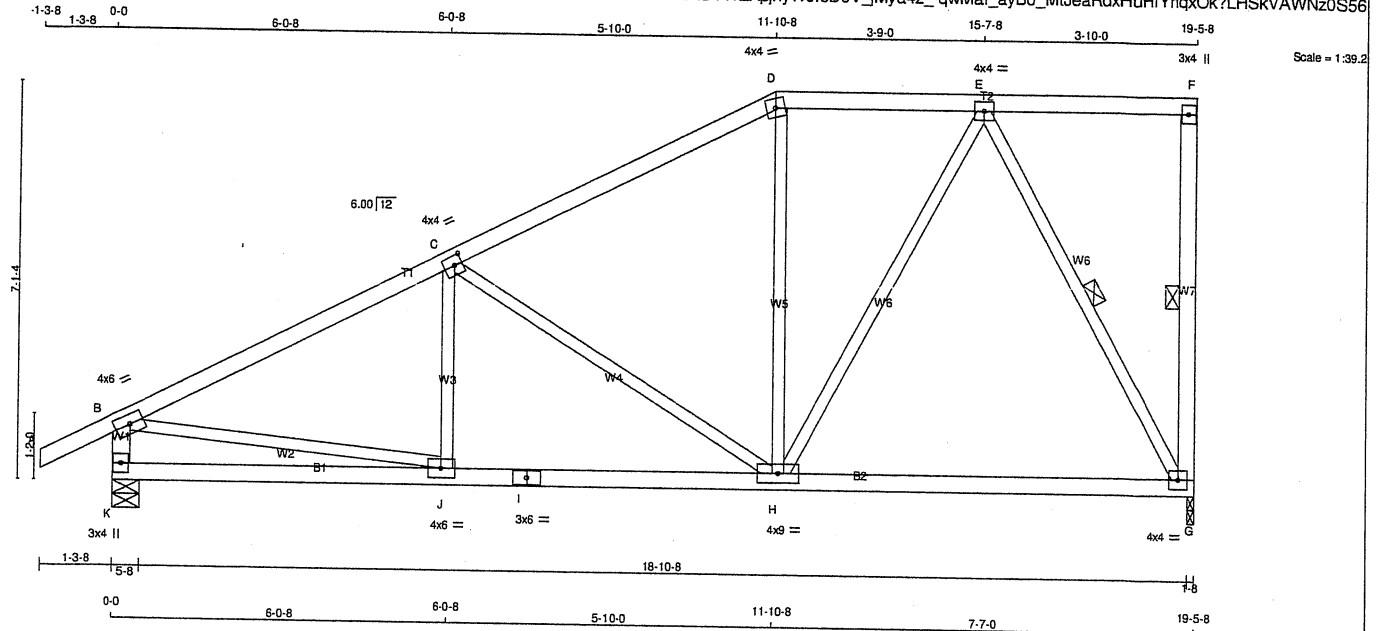
Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 1 12:26:30 2021 Page 1
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
412865	T104	3	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER					
N. L. G. A. RULES					
CHORDS		SIZE	LUMBER		DESCR
A - D		2x4	DRY	No.2	SPF
D - F		2x4	DRY	No.2	SPF
G - F		2x4	DRY	No.2	SPF
K - B		2x4	DRY	No.2	SPF
K - I		2x4	DRY	No.2	SPF
I - G		2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
G	1073	0	1073	0
K	1197	0	1197	0

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN.	COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	
G	759	498 / 0	0 / 0	0 / 0	0 / 0	261 / 0	0 / 0	
K	844	567 / 0	0 / 0	0 / 0	0 / 0	277 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.99 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, E-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS							
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)				
FR-TO		FROM	TO		FR-TO						
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	J-C	-70 / 66	0.02 (4)			
B-C	-1384 / 0	-91.8	-91.8	0.45 (1)	4.99	C-H	-629 / 0	0.61 (1)			
C-D	-855 / 0	-91.8	-91.8	0.42 (1)	6.02	H-D	0 / 69	0.02 (4)			
D-E	-738 / 0	-91.8	-91.8	0.17 (1)	6.25	H-E	0 / 512	0.12 (1)			
E-F	0 / 0	-91.8	-91.8	0.22 (1)	10.00	E-G	-1012 / 0	0.41 (1)			
G-F	-134 / 0	0.0	0.0	0.03 (1)	6.25	B-J	0 / 1276	0.29 (1)			
K-B	-1148 / 0	0.0	0.0	0.12 (1)	7.42						
K-J	0 / 0	-18.5	-18.5	0.14 (4)	10.00						
J-I	0 / 1264	-18.5	-18.5	0.33 (1)	10.00						
I-H	0 / 1264	-18.5	-18.5	0.33 (1)	10.00						
H-G	0 / 496	-18.5	-18.5	0.26 (4)	10.00						

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.65")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.04")
ALLOWABLE DEFL.(TL) = $L/360$ (0.65")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.11")

CSI: TC=0.45/1.00 (B-C:1), BC=0.33/1.00 (H-J:1), WB=0.61/1.00 (C-H:1), SSI=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)
MAX	MIN	MAX
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (J) (INPUT = 0.90)
JSI METAL= 0.37 (I) (INPUT = 1.00)

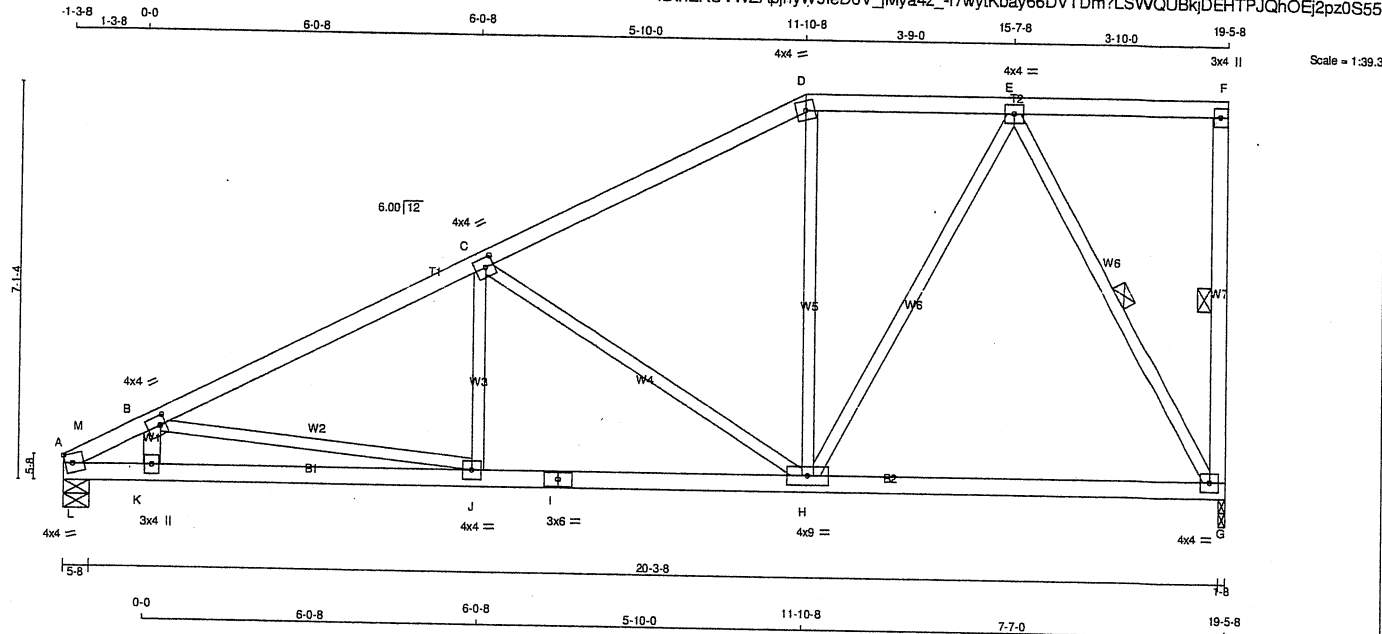


Structural component only
DWG# T-2121210

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
412865	T104X	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER					
N. L. G. A. RULES					
CHORDS		SIZE	LUMBER	DESCR.	
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
K - B	2x4	DRY	No.2	SPF	
A - I	2x4	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-m	MT20	4.0	4.0		Edge
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TMVW-t	MT20	4.0	4.0	2.00	1.75
D	TTW-m	MT20	4.0	4.0		
E	TMVW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BMVW1-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMVW-t	MT20	4.0	4.0		
K	BMV+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQRD
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
A	1151	0	1151	0	5-8
G	1151	0	1151	0	1-8

UNFACTORED REACTIONS

JT	1ST CASE	MAX./MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
A	814	534 / 0	0 / 0	280 / 0	0 / 0
G	814	534 / 0	0 / 0	280 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.24 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, E-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC1)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-M	-2504 / 0	-91.8 -91.8	0.16 (1)	J-C	0 / 169	0.05 (4)	
M-B	-2072 / 0	-91.8 -91.8	0.17 (1)	C-H	-846 / 0	0.82 (1)	
B-C	-1693 / 0	-91.8 -91.8	0.44 (1)	H-D	0 / 84	0.03 (4)	
C-D	-947 / 0	-91.8 -91.8	0.40 (1)	H-E	0 / 601	0.14 (1)	
D-E	-823 / 0	-91.8 -91.8	0.17 (1)	E-G	-1101 / 0	0.44 (1)	
E-F	0 / 0	-91.8 -91.8	0.22 (1)	B-J	-335 / 0	0.21 (1)	
G-F	-134 / 0	0.0 0.0	0.03 (1)	L-M	0 / 721	0.00 (1)	
K-B	-100 / 62	0.0 0.0	0.02 (4)				
A-L	0 / 1862	-18.5 -18.5	0.48 (1)	10.00			
L-K	0 / 1862	-18.5 -18.5	0.48 (1)	10.00			
K-J	0 / 1862	-18.5 -18.5	0.42 (1)	10.00			
J-I	0 / 1531	-18.5 -18.5	0.37 (1)	10.00			
I-H	0 / 1531	-18.5 -18.5	0.37 (1)	10.00			
H-G	0 / 540	-18.5 -18.5	0.27 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/999 (0.14")

CSI: TC=0.44/1.00 (B-C:1), BC=0.48/1.00 (A-L:1), WB=0.82/1.00 (C-H:1), SSI=0.44/1.00 (A-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

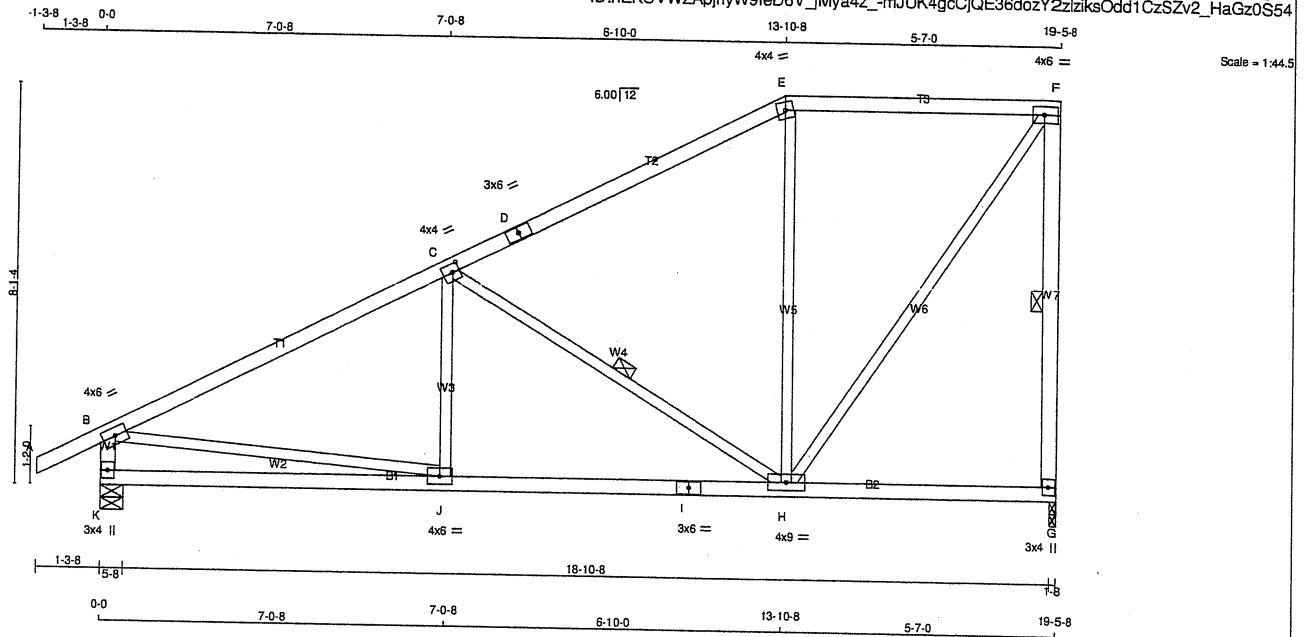
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (A) (INPUT = 0.90)
JSI METAL= 0.70 (A) (INPUT = 1.00)



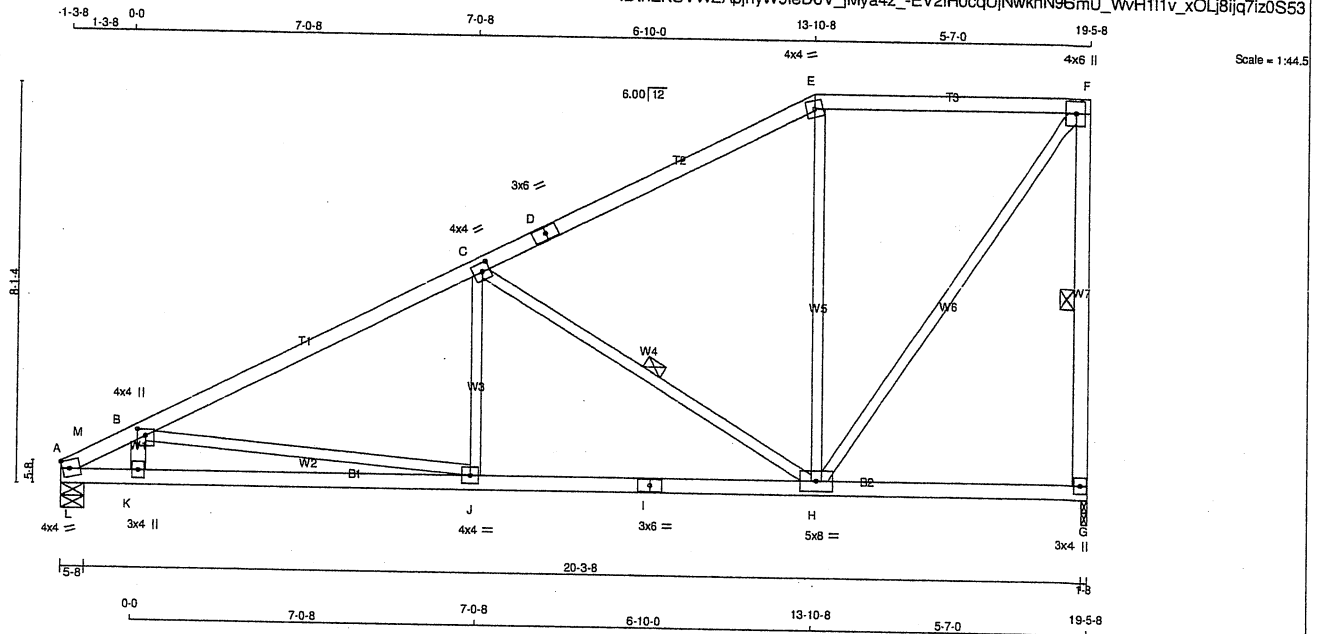
Structural component only
DWG# T-2121211

JOB NAME 412865	TRUSS NAME T105	QUANTITY 11	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



JOB NAME 412865	TRUSS NAME T105X	QUANTITY 5	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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LUMBER	CHORDS	SIZE	DRY	LUMBER	DESCR.
N. L. G. A. RULES					
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
E - F	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
K - B	2x4	DRY	No.2	SPF	
A - I	2x4	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-m	MT20	4.0	4.0		Edge
B	TMVW+p	MT20	4.0	4.0	1.50	2.00
C	TMVW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
E	TTW-m	MT20	4.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMVW-t	MT20	5.0	8.0		
I	BS-t	MT20	3.0	6.0		
J	BMVW-t	MT20	4.0	4.0		
K	BMV+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
G	1151	0	1151	0	1-8	1-8
A	1151	0	1151	0	5-8	5-8

UNFACTORED REACTIONS

1ST LOASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	814	534 / 0	0 / 0	0 / 0	0 / 0	280 / 0	0 / 0
A	814	534 / 0	0 / 0	0 / 0	0 / 0	280 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, A

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.13 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, C-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)	
FR-TO		FROM TO	CSI (LC)	FR-TO		CSI (LC)	
A-M	-2641 / 0	-91.8	-91.8 0.17 (1)	J-C	0 / 214	0.06 (4)	
M-B	-2142 / 0	-91.8	-91.8 0.25 (1)	C-H	-1010 / 0	0.46 (1)	
B-C	-1610 / 0	-91.8	-91.8 0.60 (1)	H-E	-196 / 15	0.25 (1)	
C-D	-706 / 0	-91.8	-91.8 0.55 (1)	H-F	0 / 1045	0.24 (1)	
D-E	-706 / 0	-91.8	-91.8 0.55 (1)	B-J	-496 / 0	0.46 (1)	
E-F	-600 / 0	-91.8	-91.8 0.37 (1)	L-M	0 / 845	0.00 (1)	
G-F	-1111 / 0	0.0	0.0 0.33 (1)				
K-B	-112 / 75	0.0	0.0 0.02 (4)				
A-L	0 / 1950	-18.5	-18.5 0.53 (1)	10.00			
L-K	0 / 1950	-18.5	-18.5 0.53 (1)	10.00			
K-J	0 / 1950	-18.5	-18.5 0.45 (1)	10.00			
J-I	0 / 1457	-18.5	-18.5 0.37 (1)	10.00			
I-H	0 / 1457	-18.5	-18.5 0.37 (1)	10.00			
H-G	0 / 0	-18.5	-18.5 0.16 (4)	10.00			

TOTAL WEIGHT = 5 X 88 = 439 lb
[M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL)= L/999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL)= L/999 (0.17")

CSI: TC=0.60/1.00 (B-C:1), BC=0.53/1.00 (A-L:1), WB=0.46/1.00 (C-H:1), SSI=0.52/1.00 (A-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

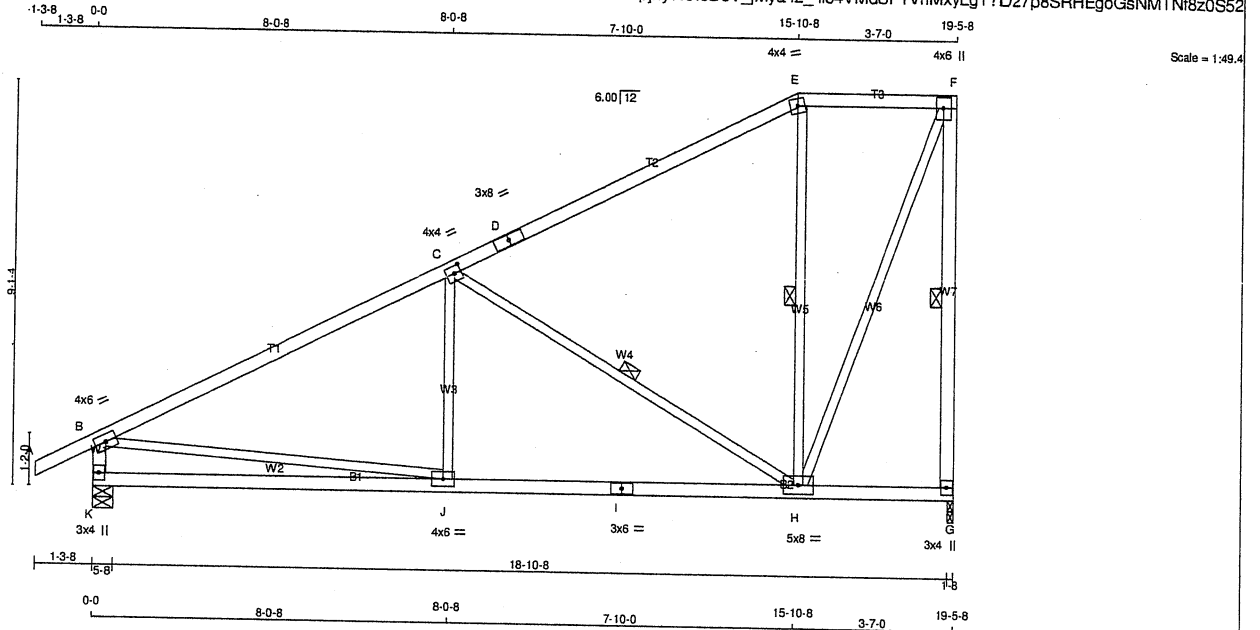
JSI GRIP = 0.86 (A) (INPUT = 0.90)
JSI METAL = 0.74 (A) (INPUT = 1.00)



Structural component only
DWG# T-2121213

JOB NAME 412865	TRUSS NAME T106	QUANTITY 3	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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TOTAL WEIGHT = 3 X 91 = 272 lb
(M/F)

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
G - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
DRY: SEASONED LUMBER.				

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	BRG	IN-SX
G	1073	0	1073	0	0	1-8	1-8		
K	1197	0	1197	0	0	5-8	5-8		

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
G	759	498 / 0	0 / 0	0 / 0	0 / 0	261 / 0	0 / 0		
K	844	567 / 0	0 / 0	0 / 0	0 / 0	277 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.15 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, C-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		W E B S		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-91.8	-91.8 0.12 (1)	J-C	0 / 149	0.05 (4)	
B-C	-1311 / 0	-91.8	-91.8 0.86 (1)	C-H	-1000 / 0	0.61 (1)	
C-D	-439 / 0	-91.8	-91.8 0.77 (1)	H-E	-255 / 0	0.14 (1)	
D-E	-439 / 0	-91.8	-91.8 0.77 (1)	H-F	0 / 961	0.22 (1)	
E-F	-353 / 0	-91.8	-91.8 0.15 (1)	B-J	0 / 1214	0.27 (1)	
G-F	-1057 / 0	0.0	0.0 0.41 (1)				
K-B	-1136 / 0	0.0	0.0 0.11 (1)				
K-J	0 / 0	-18.5	-18.5 0.30 (4)				
J-I	0 / 1208	-18.5	-18.5 0.40 (4)				
I-H	0 / 1208	-18.5	-18.5 0.40 (4)				
H-G	0 / 0	-18.5	-18.5 0.15 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/999 (0.14")

CSI: TC=0.86/1.00 (B-C:1), BC=0.40/1.00 (H-J:4), WB=0.61/1.00 (C-H:1), SS=0.32/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

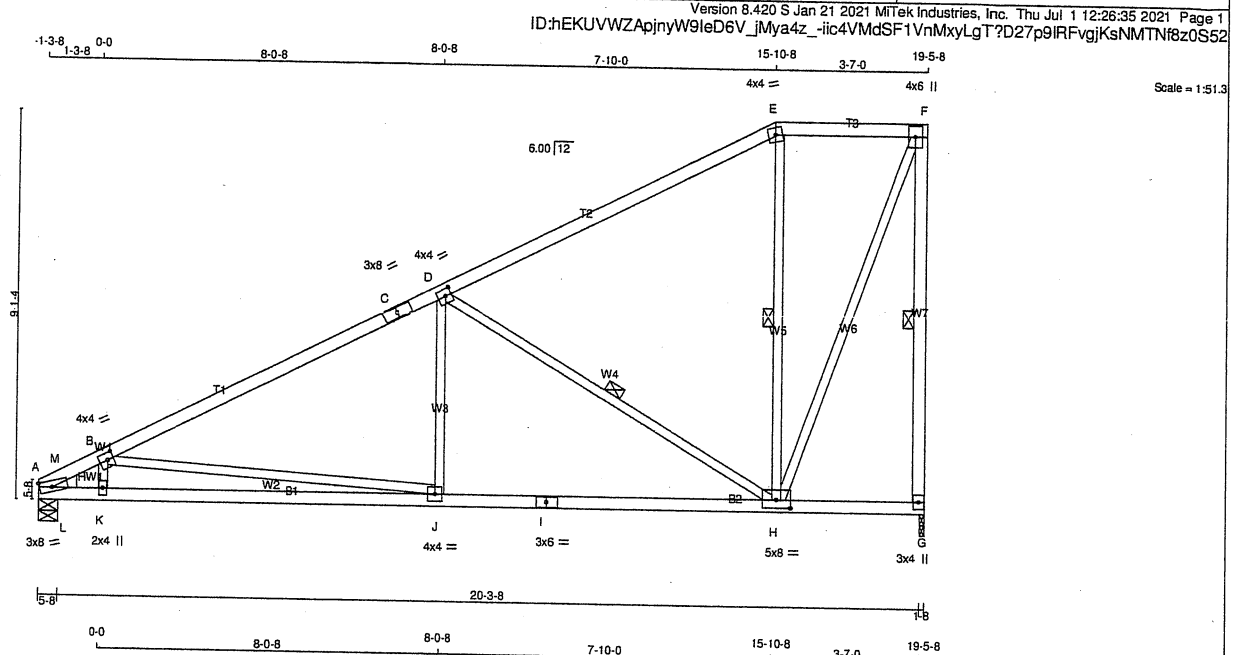
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.83 (H) (INPUT = 0.90)
JSI METAL = 0.49 (I) (INPUT = 1.00)



Structural component only
DWG# T-2121214

JOB NAME 412865	TRUSS NAME T106X	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.	
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
E - F	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
A - I	2x4	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.				SPF	

PLATES (table in inches)					
JT TYPE	PLATES	W	LEN	Y	X
A	TBMH1-m	MT20	3.0	8.0	1.75 Edge
B	TMWW-t	MT20	4.0	4.0	2.00 1.75
C	TS-t	MT20	3.0	8.0	
D	TMWW-t	MT20	4.0	4.0	2.00 1.75
E	TTW-m	MT20	4.0	4.0	
F	TMWW-p	MT20	4.0	6.0	
G	BMV1-p	MT20	3.0	4.0	
H	BMWWW-t	MT20	5.0	8.0	2.25 4.00
I	BS-t	MT20	3.0	6.0	
J	BMWW-t	MT20	4.0	4.0	
K	BMW-w	MT20	2.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG		HEEL WEDGE	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX		
G	1151	0	1151	0	0	1-8	1-8		
A	1151	0	1151	0	0	5-8	5-8	2x4 L	

UNFACTORED REACTIONS		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	814	534 / 0	0 / 0	0 / 0	0 / 0	280 / 0	0 / 0
A	814	534 / 0	0 / 0	0 / 0	0 / 0	280 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, A

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.24 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MAX. UNBRACED LENGTH (LBS)	MEMB.	FORCE (LBS)	MAX. UNBRACED LENGTH (LBS)	MAX. UNBRACED LENGTH (LBS)
FR-TO		FROM	TO	FR-TO			
A-M	-2452 / 0	-91.8	-91.8 0.15 (1)	4.28	J-D	0 / 254	0.07 (4)
M-B	-2188 / 0	-91.8	-91.8 0.34 (1)	4.27	D-H	-1163 / 0	0.71 (1)
B-C	-1519 / 0	-91.8	-91.8 0.77 (1)	4.24	H-E	-250 / 0	0.13 (1)
C-D	-1519 / 0	-91.8	-91.8 0.77 (1)	4.24	H-F	0 / 1044	0.23 (1)
D-E	-468 / 0	-91.8	-91.8 0.72 (1)	6.25	K-B	-171 / 94	0.03 (4)
E-F	-383 / 0	-91.8	-91.8 0.15 (1)	6.25	B-J	-674 / 0	0.93 (1)
G-F	-1134 / 0	0.0	0.0 0.44 (1)	5.97	L-M	0 / 486	0.00 (1)
A-L	0 / 2047	-18.5	-18.5 0.55 (1)	10.00			
L-K	0 / 2047	-18.5	-18.5 0.55 (1)	10.00			
K-J	0 / 2047	-18.5	-18.5 0.51 (1)	10.00			
J-I	0 / 1377	-18.5	-18.5 0.40 (1)	10.00			
I-H	0 / 1377	-18.5	-18.5 0.40 (1)	10.00			
H-G	0 / 0	-18.5	-18.5 0.16 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:			
TOP CH.	LL	=	25.6 PSF
	DL	=	6.0 PSF
BOT CH.	LL	=	0.0 PSF
	DL	=	7.4 PSF
TOTAL LOAD		=	39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC08 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/999 (0.21")

CSI: TC=0.77/1.00 (B-D:1), 8C=0.55/1.00 (A-L:1), WB=0.93/1.00 (B-J:1), SSI=0.31/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES		PLATE GRIP(DRY) SHEAR		SECTION	
(PSI)	(PLI)	(PSI)	(PLI)	(PSI)	(PLI)
MT20	650	371	1747	788	1987

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

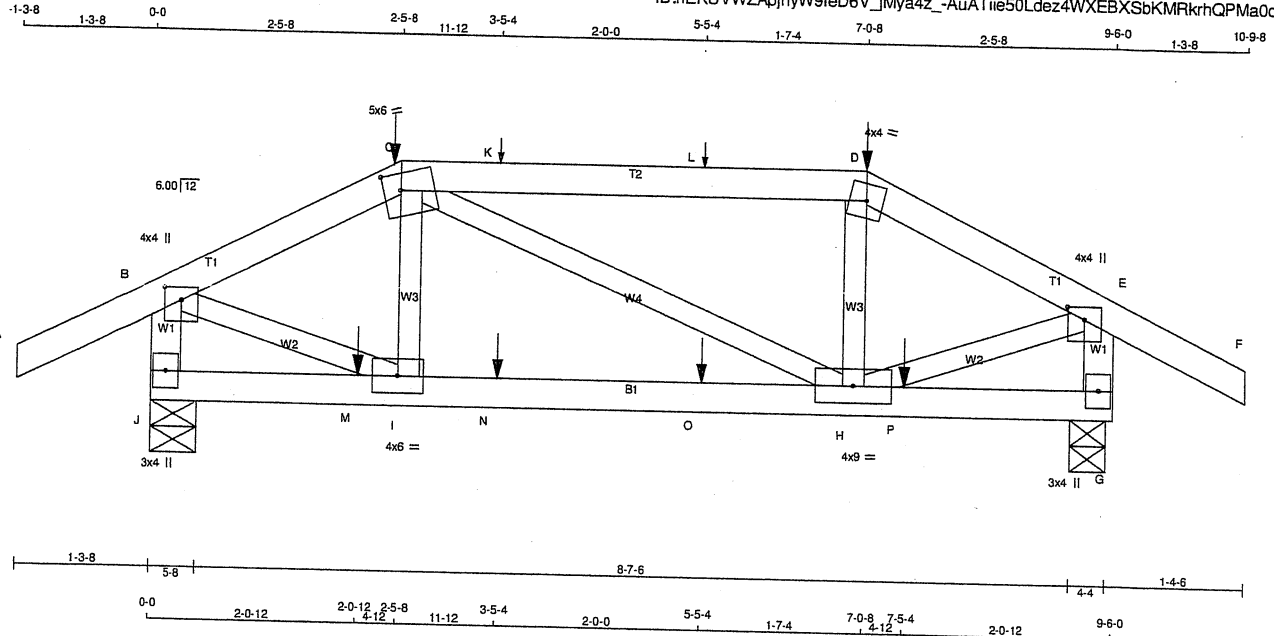
JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.44 (I) (INPUT = 1.00)



Structural component only
DWG# T-2121215

JOB NAME 412865	TRUSS NAME T107	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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Scale = 1:21.5

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
J - B	2x4	DRY	No.2
G - E	2x4	DRY	No.2
J - G	2x4	DRY	No.2

ALL WEBS EXCEPT			
2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.50	2.00
C	TTWW-m	MT20	5.0	6.0	2.00	2.00
D	TTWW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	4.0	1.50	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMVWW-t	MT20	4.0	9.0		
I	BMVWW-t	MT20	4.0	6.0		
J	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	UP
J	762	0	762	0
G	762	0	762	0

UNFACTORED REACTIONS

JT	1ST CASE	MAX./MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
J	535	373 / 0	0 / 0	0 / 0	0 / 0	0 / 0	162 / 0	0 / 0
G	535	373 / 0	0 / 0	0 / 0	0 / 0	0 / 0	162 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-91.8 -91.8	0.13 (1)	I-C	-126 / 22	0.02 (1)	
B-C	-678 / 0	-91.8 -91.8	0.11 (1)	C-H	0 / 0	0.00 (1)	
C-K	-598 / 0	-91.8 -91.8	0.37 (1)	H-D	-127 / 21	0.02 (1)	
K-L	-598 / 0	-91.8 -91.8	0.37 (1)	B-I	0 / 638	0.16 (1)	
L-D	-598 / 0	-91.8 -91.8	0.37 (1)	H-E	0 / 638	0.16 (1)	
D-E	-677 / 0	-91.8 -91.8	0.11 (1)				
E-F	0 / 28	-91.8 -91.8	0.13 (1)				
J-B	-745 / 0	0.0 0.0	0.08 (1)				
G-E	-744 / 0	0.0 0.0	0.08 (1)				
J-M	0 / 0	-18.5 -18.5	0.06 (4)				
M-I	0 / 0	-18.5 -18.5	0.06 (4)				
I-N	0 / 599	-18.5 -18.5	0.15 (1)				
N-O	0 / 599	-18.5 -18.5	0.15 (1)				
O-H	0 / 599	-18.5 -18.5	0.15 (1)				
H-P	0 / 0	-18.5 -18.5	0.06 (4)				
P-G	0 / 0	-18.5 -18.5	0.06 (4)				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-5-8	-74	-74		FRONT	VERT	TOTAL		C1
D	7-0-8	-74	-74		FRONT	VERT	TOTAL		C1
K	3-5-4	1	1		FRONT	VERT	TOTAL		C1
L	5-5-4	1	1		FRONT	VERT	TOTAL		C1
M	2-0-12	-3	-3		FRONT	VERT	TOTAL		C1
N	3-5-4	-1	-1		FRONT	VERT	TOTAL		C1
O	5-5-4	-1	-1		FRONT	VERT	TOTAL		C1
P	7-5-4	-3	-3		FRONT	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.32")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.32")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.37/1.00 (C-D:1), BC=0.15/1.00 (H-I:1), WB=0.16/1.00 (B-I:1), SS=0.18/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

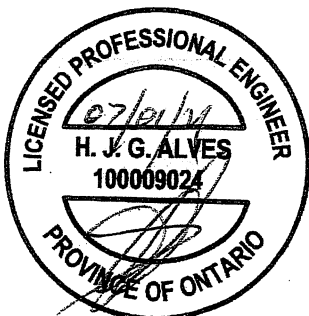
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

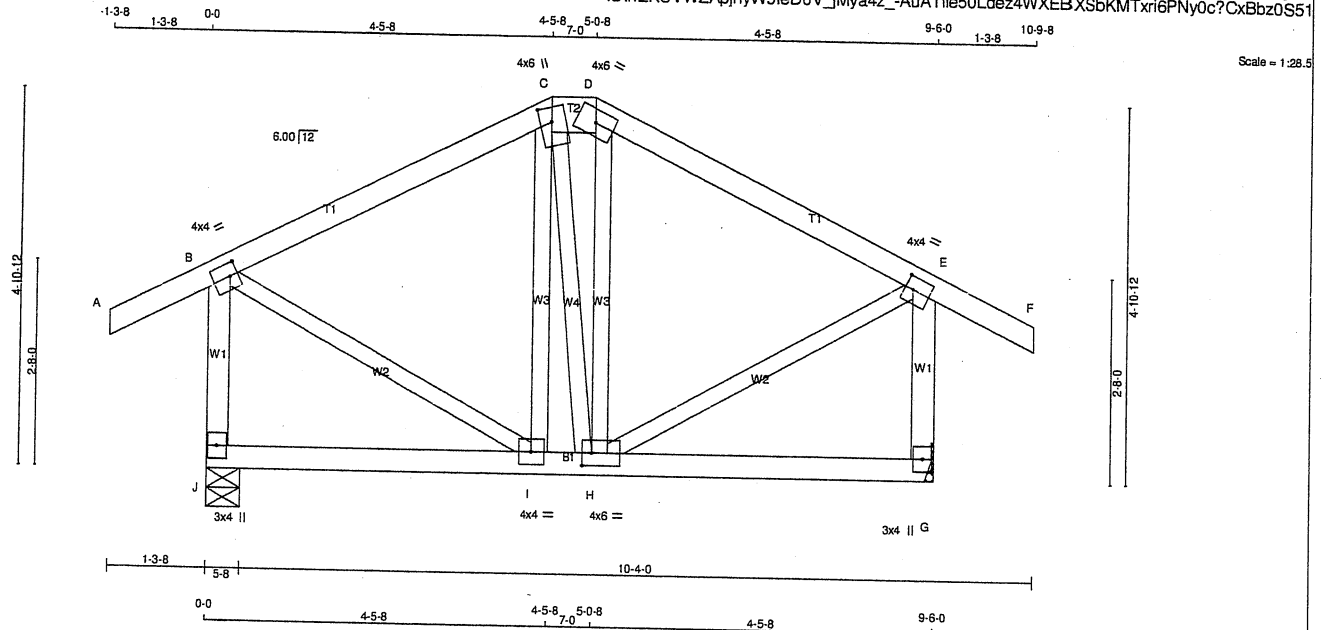
JSI GRIP= 0.65 (B) (INPUT = 0.90)
JSI METAL= 0.20 (B) (INPUT = 1.00)



Structural component only
DWG# T-2121216

JOB NAME 412865	TRUSS NAME T108	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - D	2x6	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
J - B	2x4	DRY	No.2	SPF	
G - E	2x4	DRY	No.2	SPF	
J - G	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TTWW+m	MT20	4.0	6.0	2.25	1.75
D	TTWW-h	MT20	4.0	6.0		
E	TMVW-t	MT20	4.0	4.0	2.00	1.25
G	BMV1+p	MT20	3.0	4.0		
H	BMVWW-t	MT20	4.0	6.0	2.00	1.50
I	BMVWW-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	IN-SX	IN-SX
J	648	0	5-8	5-8
G	648	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 1-8.

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	456	312 / 0	0 / 0	0 / 0	0 / 0	144 / 0	0 / 0
G	456	312 / 0	0 / 0	0 / 0	0 / 0	144 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE	VERT. LOAD	MEMB.	FORCE
	(LBS)	LC1		(LBS)
FR-TO		FROM TO	FR-TO	
A-B	0 / 28	-91.8 -91.8 0.12 (1)	I-C	-88 / 7
B-C	-309 / 0	-91.8 -91.8 0.23 (1)	H-D	-96 / 7
C-D	-278 / 0	-91.8 -91.8 0.00 (1)	B-I	0 / 315
D-E	-307 / 0	-91.8 -91.8 0.23 (1)	H-E	0 / 314
E-F	0 / 28	-91.8 -91.8 0.12 (1)	C-H	-7 / 0
J-B	-615 / 0	0.0 0.0 0.09 (1)		
G-E	-614 / 0	0.0 0.0 0.09 (1)		
J-I	0 / 0	-18.5 -18.5 0.08 (4)		
I-H	0 / 279	-18.5 -18.5 0.10 (4)		
H-G	0 / 0	-18.5 -18.5 0.08 (4)		

TOTAL WEIGHT = 2 X 50 = 99 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.32")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.32")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.23/1.00 (B-C:1), BC=0.10/1.00 (H-I:4), WB=0.07/1.00 (B-I:1), SS=0.14/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.45 (B) (INPUT = 0.90)
JSI METAL= 0.16 (B) (INPUT = 1.00)

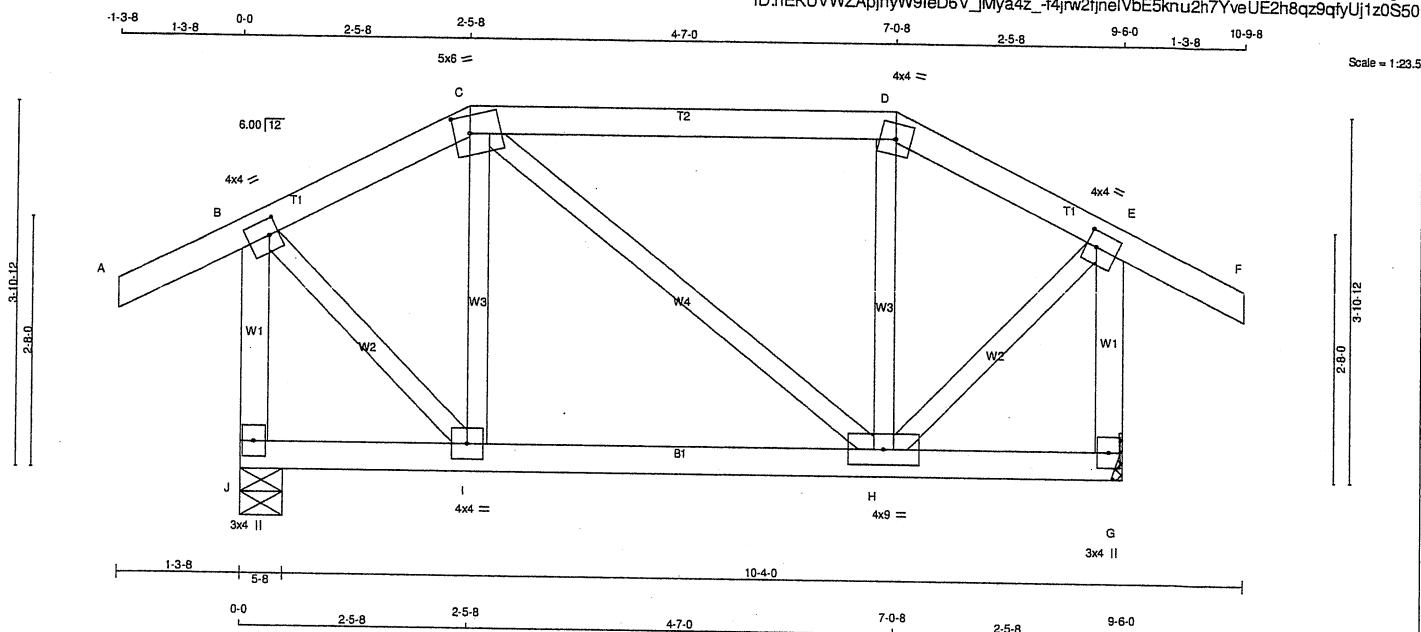


Structural component only
DWG# T-2121217

JOB NAME 412865	TRUSS NAME T109	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 46 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
J - B	2x4	DRY	No.2
G - E	2x4	DRY	No.2
J - G	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-t	MT20	4.0	4.0	2.00	1.25
C	TTWW-m	MT20	5.0	6.0	2.25	2.00
D	TTW-m	MT20	4.0	4.0		
E	TMW-t	MT20	4.0	4.0	2.00	1.25
G	BMV1-p	MT20	3.0	4.0		
H	BMVWW-t	MT20	4.0	9.0		
I	BMVW-t	MT20	4.0	4.0		
J	BMV1-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	UP
J	648	0	648	0
G	648	0	648	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 1-8.

UNFACTORED REACTIONS

1ST CASE	MAX./MIN. COMPONENT REACTIONS
JT	COMBINED
J	456
G	456

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE	FACTORED	VERT. LOAD	MEMB.	FORCE	FACTORED	VERT. LOAD
(LBS)	(LBS)	(LBS)	(LBS)	(LBS)	(LBS)	(LBS)	(LBS)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 28	-91.8	-91.8	I-C	-183 / 0	0.04	(1)
B-C	-311 / 0	-91.8	-91.8	C-H	0 / 0	0.00	(1)
C-D	-272 / 0	-91.8	-91.8	H-D	-183 / 0	0.04	(1)
D-E	-311 / 0	-91.8	-91.8	B-I	0 / 378	0.09	(1)
E-F	0 / 28	-91.8	-91.8	H-E	0 / 377	0.08	(1)
J-B	-633 / 0	0.0	0.0				
G-E	-632 / 0	0.0	0.0				
J-I	0 / 0	-18.5	-18.5				
I-H	0 / 273	-18.5	-18.5				
H-G	0 / 0	-18.5	-18.5				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
DL =	6.0	PSF	
BOT CH.	LL =	0.0	PSF
DL =	7.4	PSF	
TOTAL LOAD =	39.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.32")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.32")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.25/1.00 (C-D:1), BC=0.08/1.00 (H-I:4), WB=0.09/1.00 (B-I:1), SS=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.52 (B) (INPUT = 0.90)
JSI METAL = 0.14 (B) (INPUT = 1.00)

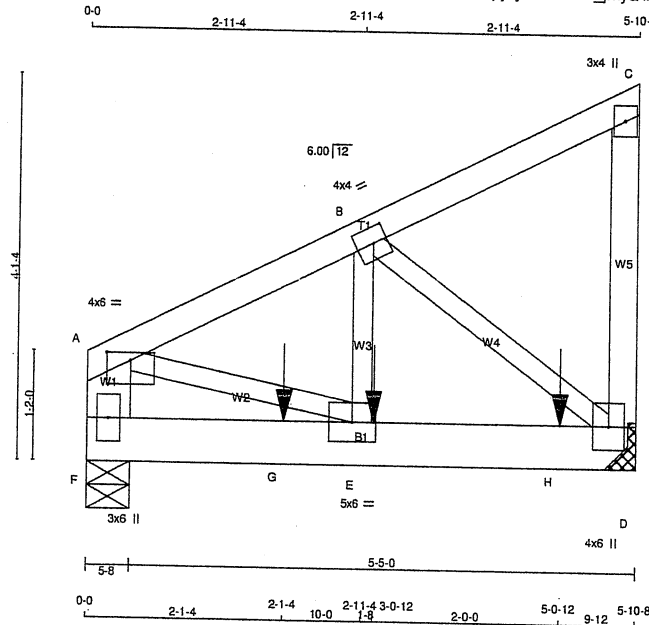


Structural component only
DWG# T-2121218

JOB NAME 412865	TRUSS NAME T110	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1:23.4



LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x6	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C 1 12		TOP
C-D 1 12		TOP
F-A 2 12		TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F-D 2 12		SIDE (183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1 6		

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00	3.00
B	TMVW-t	MT20	4.0	4.0	2.00	1.75
C	TMV-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	UP	DOWN
D	1408	0	0	MECHANICAL
F	1130	0	0	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LOOSE COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
D	989	688 / 0	0 / 0	0 / 0	0 / 0	301 / 0	0 / 0
F	794	550 / 0	0 / 0	0 / 0	0 / 0	244 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	FR-TO
A-B	-1250 / 0	-91.8	-91.8 0.06 (1)	E-B	0 / 1106	0.14 (1)	
B-C	-11 / 0	-91.8	-91.8 0.05 (1)	B-D	-1419 / 0	0.17 (1)	
D-C	-110 / 0	0.0	0.0 0.01 (1)	A-E	0 / 1166	0.14 (1)	
F-A	-971 / 0	0.0	0.0 0.03 (1)				
F-G	0 / 0	-18.5	-18.5 0.11 (1)				
G-E	0 / 0	-18.5	-18.5 0.11 (1)				
E-H	0 / 1128	-18.5	-18.5 0.20 (1)				
H-D	0 / 1128	-18.5	-18.5 0.20 (1)				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	3-0-12	-441	-441	---	BACK	VERT	TOTAL	---	C1
G	2-1-4	-441	-441	---	BACK	VERT	TOTAL	---	C1
H	5-0-12	-443	-443	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 29 = 58 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.20")
CALCULATED VERT. DEFL. (LL) = L/999 (0.01")
ALLOWABLE DEFL. (TL) = L/360 (0.20")
CALCULATED VERT. DEFL. (TL) = L/999 (0.01")

CSI: TC=0.08/1.00 (A-B:1), BC=0.20/1.00 (D-E:1),
WB=0.17/1.00 (B-D:1), SS=0.17/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.72 (B) (INPUT = 0.90)
JSI METAL = 0.25 (D) (INPUT = 1.00)



Structural component only
DWG# T-2121219 112

CONTINUED ON PAGE 2

JOB NAME 412865	TRUSS NAME T110	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
D	BMVW1+p	MT20	4.0	6.0		
E	BMWW-t	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	6.0		

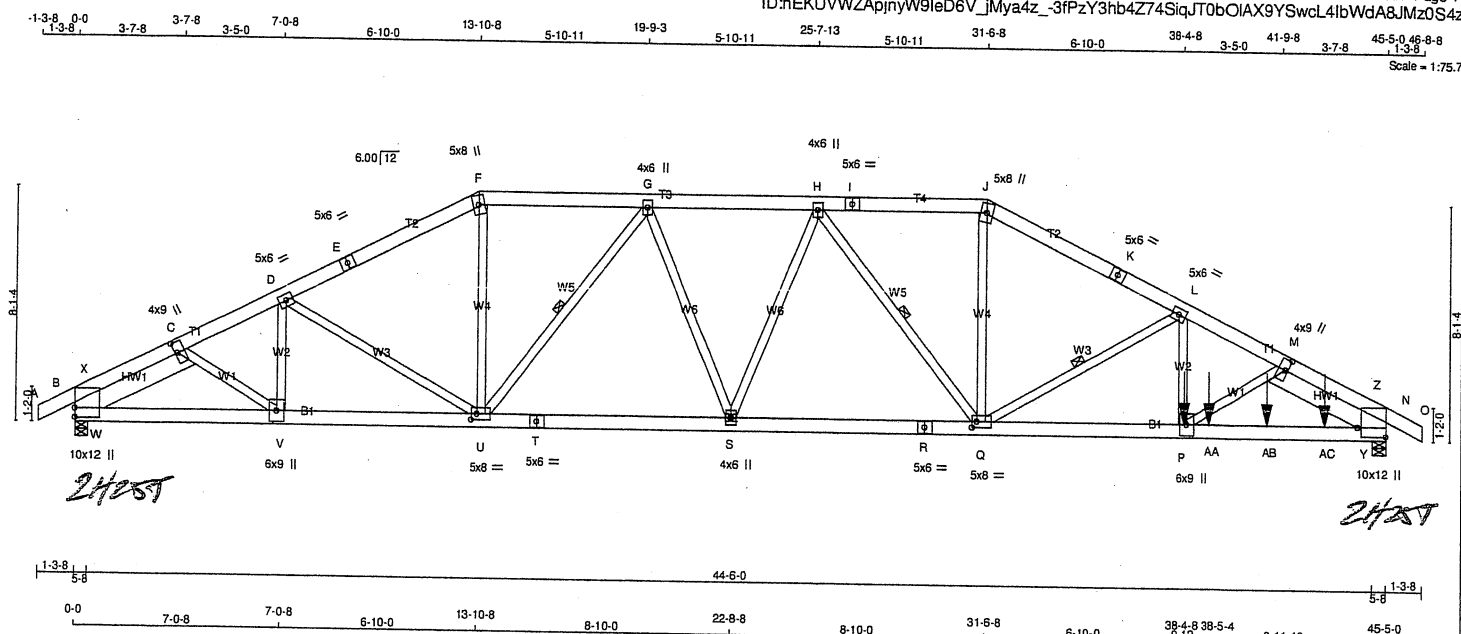


Structural component only
DWG# T-2121219 *3/11*

JOB NAME 412865	TRUSS NAME T111	QUANTITY 1	PLY 3	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - E	2x6 DRY	No.2	SPF
E - F	2x6 DRY	No.2	SPF
F - I	2x6 DRY	No.2	SPF
I - J	2x6 DRY	No.2	SPF
J - K	2x6 DRY	No.2	SPF
K - O	2x6 DRY	No.2	SPF
B - T	2x6 DRY	No.2	SPF
T - R	2x6 DRY	No.2	SPF
R - N	2x6 DRY	No.2	SPF

REINFORCING MEMBERS

MEMBER	SIZE	LUMBER	DESCR.
HW1	2x8 DRY	No.2	SPF
HW2	2x8 DRY	No.2	SPF

ALL WEBS 2x4 DRY No.2 SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-E	2 12	TOP
E-F	2 12	TOP
F-I	2 12	TOP
I-J	2 12	TOP
J-K	2 12	TOP
K-O	2 12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
B-T	2 12	TOP
T-R	2 12	TOP
R-N	2 12	SIDE(451.4)
WEBS : (0.122"x3") SPIRAL NAILS		
L-P	2 4	SIDE(1060.7)
2x4	1 6	
D-V	2 4	
2x8	2 6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	UP		
B	4431	0	4466	193	-1072 5-8	5-8
N	8350	0	8385	0	-1072 5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 1072 LBS. FACTORED UPLIFT

PROVIDE ANCHORAGE AT BEARING JOINT N FOR 1072 LBS. FACTORED UPLIFT

PROVIDE FOR 193 LBS. FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS

JT	1ST LOOSE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
B	3247	1991 / 0	477 / 0	0 / 0	0 / -1066	802 / 0	0 / 0
N	6011	3847 / 0	477 / 0	0 / 0	59 / -1066	1710 / 0	0 / 0

HORIZONTAL REACTIONS

JT	REACT	REACT	REACT	REACT	REACT	REACT
B	0 / 0	0 / 0	0 / 0	138 / -138	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, N

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 4.01 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-U, H-Q, L-Q.

LOADING

TOTAL LOAD CASES: (18)

CHORDS				FACTORED		WEBS			
MEMB.	MAX. FORCE (LBS)	VERT. (PLF)	LOAD LC1	MAX CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FORCE (LBS)	FACTORED MAX CSI (LC)	
FR-TO		FROM	TO			FR-TO			
A-B	0 / 1	-115.2	-115.2	0.03 (2)	10.00	C-V	-142 / 888	0.05 (1)	
B-X	-5083 / 1128	-115.2	-115.2	0.06 (2)	6.03	V-D	-334 / 140	0.02 (8)	
X-C	-3427 / 876	-115.2	-115.2	0.05 (2)	6.25	D-U	-793 / 418	0.25 (2)	
C-D	-7034 / 1698	-115.2	-115.2	0.14 (2)	5.26	U-F	-408 / 2301	0.12 (1)	
D-E	-6551 / 1552	-115.2	-115.2	0.16 (2)	5.40	U-G	-2385 / 487	0.36 (3)	
E-F	-6551 / 1552	-115.2	-115.2	0.16 (2)	5.40	G-S	-29 / 1258	0.07 (3)	
F-G	-5873 / 1490	-115.2	-115.2	0.10 (1)	5.67	S-H	-968 / 119	0.31 (10)	
G-H	-7469 / 1635	-115.2	-115.2	0.11 (1)	5.16	H-Q	-487 / 487	0.07 (2)	
H-I	-7589 / 1490	-115.2	-115.2	0.12 (1)	5.13	Q-J	-407 / 3131	0.17 (1)	
I-J	-7589 / 1490	-115.2	-115.2	0.12 (1)	5.13	Q-L	-5427 / 419	0.56 (3)	
J-K	-8446 / 1552	-115.2	-115.2	0.19 (3)	4.86	P-L	0 / 4135	0.22 (2)	
K-L	-8446 / 1552	-115.2	-115.2	0.19 (3)	4.86	P-M	-143 / 1628	0.09 (1)	
L-M	-13359 / 1699	-115.2	-115.2	0.19 (3)	4.01	W-X	-372 / 2258	0.00 (1)	
M-Z	-7226 / 878	-115.2	-115.2	0.11 (3)	5.22	W-C	-3479 / 777	0.12 (1)	
Z-N	-10927 / 1132	-115.2	-115.2	0.12 (3)	4.43	M-Y	-5733 / 777	0.19 (1)	
N-O	0 / 1	-115.2	-115.2	0.03 (3)	10.00	Y-Z	-375 / 5268	0.00 (1)	
B-W	-844 / 3015	-39.5	-39.5	0.14 (2)	6.25				
W-V	-1434 / 5645	-39.5	-39.5	0.27 (1)	6.25				
V-U	-1531 / 6363	-39.5	-39.5	0.30 (1)	6.25				
U-T	-1368 / 7114	-39.5	-39.5	0.33 (1)	6.25				
T-S	-1368 / 7114	-39.5	-39.5	0.33 (1)	6.25				
S-R	-1314 / 7690	-39.5	-39.5	0.35 (1)	6.25				
R-Q	-1314 / 7690	-39.5	-39.5	0.35 (1)	6.25				
Q-P	-1338 / 12033	-39.5	-39.5	0.54 (1)	6.25				
P-AA	-1243 / 10718	-39.5	-39.5	0.64 (1)	6.25				
AA-AB	-1243 / 10718	-39.5	-39.5	0.64 (1)	6.25				
AB-AC	-1243 / 10718	-39.5	-39.5	0.64 (1)	6.25				
AC-Y	-1243 / 10718	-39.5	-39.5	0.64 (1)	6.25				
Y-N	-653 / 6377	-39.5	-39.5	0.37 (3)	6.25				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 33.4 PSF

DL = 6.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.4 PSF

TOTAL LOAD = 57.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 4 OF CBC 2018, ABC 2019
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 066-14
- TPIC 2014

DESIGN ASSUMPTIONS

- SLOPE REDUCTION FACTOR NOT USED
- PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED.

(80 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 33.4 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.51")

CALCULATED VERT. DEFL.(LL) = L/999 (0.13")

ALLOWABLE DEFL.(TL)= L/180 (3.03")

CALCULATED VERT. DEFL.(TL) = L/999 (0.19")

CSI: TC=0.19/1.00 (J-L-3), BC=0.64/1.00 (P-Y-1), WB=0.56/1.00 (L-Q-3), SSI=0.17/1.00 (N-Y-2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00

WIND LOAD IMPORTANCE FACTOR = 1.00

LIVE LOAD IMPORTANCE FACTOR = 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (U) (INPUT = 0.90)

JSI METAL= 0.92 (M) (INPUT = 1.00)



Structural component only
DWG# T-2121220 1/2

JOB NAME 412865	TRUSS NAME T111	QUANTITY 1	PLY 3	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMW1+	MT20	10.0	12.0	3.75	
C	TMWW+	MT20	4.0	9.0	4.50	1.00
D	TMWW+	MT20	5.0	6.0		
E, I, K						
E	TS+	MT20	5.0	6.0		
F	TTW+m	MT20	5.0	8.0		
G	TMWW+	MT20	4.0	6.0		
H	TMWW+	MT20	4.0	6.0		
J	TTW+m	MT20	5.0	8.0		
L	TMWW+	MT20	4.0	9.0	4.50	1.00
M	TMWW+	MT20	5.0	6.0		
N	TMBMW1+	MT20	10.0	12.0	3.75	Edge
P	BMWW+	MT20	6.0	9.0		
Q	BMWW+	MT20	5.0	8.0	2.50	2.00
R	BS+	MT20	5.0	6.0		
S	BMWW+	MT20	4.0	6.0		
T	BS+	MT20	5.0	6.0		
U	BMWW+	MT20	5.0	8.0	2.50	2.50
V	BMWW+	MT20	6.0	9.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
P	38-5-4	-2896	-2896	---	BACK	VERT	TOTAL	---	C1
AA	39-3-12	-312	-312	---	BACK	VERT	TOTAL	---	C1
AB	41-3-12	-312	-312	---	BACK	VERT	TOTAL	---	C1
AC	43-3-12	-312	-312	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2.(8)

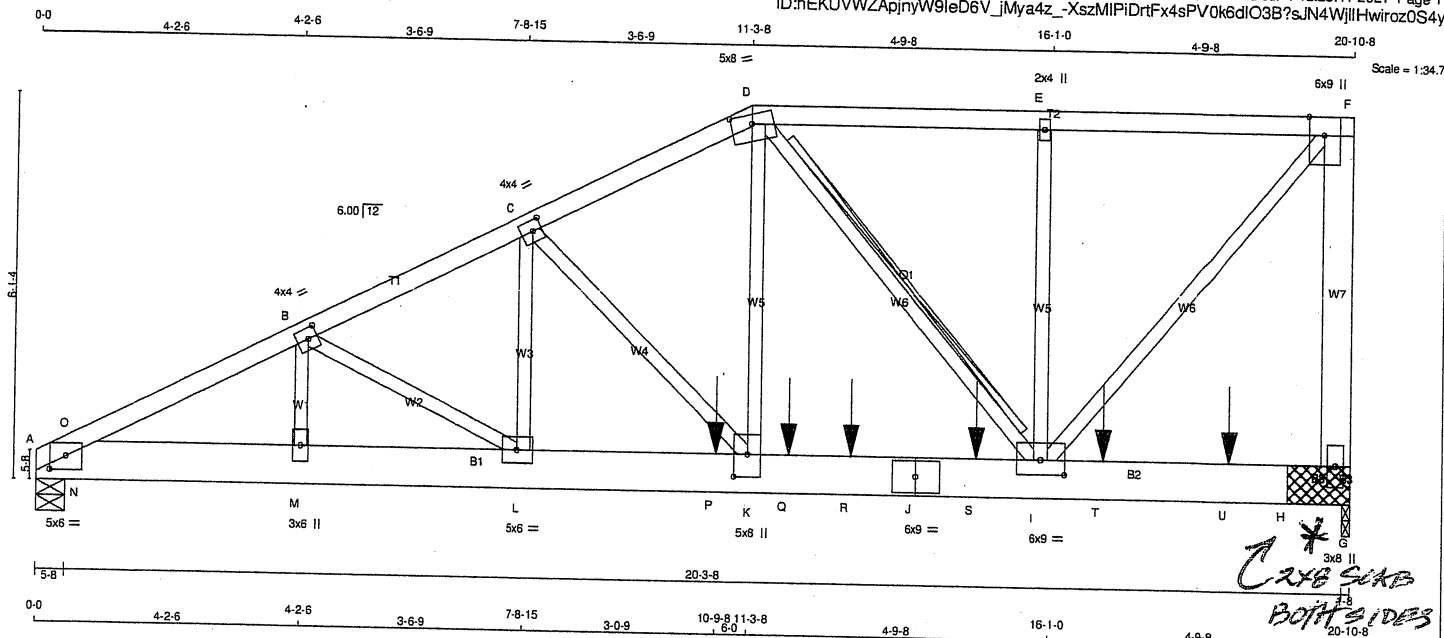
WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF [9.2] PSF AT [30-0-0] FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpG, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.



Structural component only
DWG# T-2121220 *ML*

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = $2 \times 120 = 239$ lb

ALL WEBS 2x3 DRY No.2 SPF
 DRY: SEASONED LUMBER.

CHORDS #ROWS	SURFACE	LOAD(PLF)
--------------	---------	-----------

NAILS TO BE DRIVEN FROM ONE SIDE ONLY

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED			INPUT	REQRD
JT	GROSS REACTION		GROSS REACTION		UPLIFT	BRG	BRG	
	VERT	HORZ	DOWN	HORZ		IN-SX	IN-SX	
A	3006	0	3006	0	0	5-8	5-8	
G	4270	0	4270	0	0	1-8	1-8 & BLOCK	

UNFACTORED REACTIONS

1ST CASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
A	2129	1377 / 0	0 / 0	0 / 0	0 / 0	752 / 0	0 / 0
G	3020	1981 / 0	0 / 0	0 / 0	0 / 0	1039 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A. G

2x8 DRY SPF No.2 BEARING BLOCK 12" LONG AT JT. G ATTACHED TO FRONT AND BACK SIDE WITH 4 ROWS OF (0.122"x3") SPIRAL NAILS SPACED 3" C.C. 16 NAILS TOTAL.

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.86 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED

2x4 DRY SPF No.2 T-BRACE AT D-I

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

C H O R D S				W E B S			
MAX. FACTORED		FACTORED		MAX. FACTORED			
MEMB.	FORCE	VERT. LOAD	LC1 MAX	MAX.	MEMB.	FORCE	MAX
	(LBS)	(PLF)	CSI (LC)	UNBRAC		(LBS)	CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
A-O	-5940 / 0	-91.8	-91.8 0.14 (1)	3.91	M-B	-169 / 0	0.01 (1)
O-B	-5967 / 0	-91.8	-91.8 0.20 (1)	3.86	B-L	-154 / 0	0.02 (1)
B-C	-5815 / 0	-91.8	-91.8 0.19 (1)	3.92	L-C	0 / 597	0.07 (1)
C-D	-5141 / 0	-91.8	-91.8 0.18 (1)	4.14	C-K	-904 / 0	0.21 (1)
D-E	-3253 / 0	-91.8	-91.8 0.23 (1)	4.94	K-D	0 / 3688	0.46 (1)
E-F	-3253 / 0	-91.8	-91.8 0.23 (1)	4.94	D-I	-2197 / 0	0.54 (1)
G-F	-3921 / 0	0.0	0.0 0.79 (1)	7.16	I-E	-530 / 0	0.14 (1)
					I-F	0 / 4959	0.61 (1)
A-N	0 / 5334	-18.5	-18.5 0.36 (1)	10.00	N-O	-254 / 0	0.00 (1)
N-M	0 / 5337	-18.5	-18.5 0.41 (1)	10.00			
M-L	0 / 5337	-18.5	-18.5 0.41 (1)	10.00			
L-P	0 / 5205	-18.5	-18.5 0.48 (1)	10.00			
P-K	0 / 5205	-18.5	-18.5 0.48 (1)	10.00			
K-Q	0 / 4653	-18.5	-18.5 0.39 (1)	10.00			
Q-R	0 / 4653	-18.5	-18.5 0.39 (1)	10.00			
R-J	0 / 4653	-18.5	-18.5 0.39 (1)	10.00			
J-S	0 / 4653	-18.5	-18.5 0.39 (1)	10.00			
S-I	0 / 4653	-18.5	-18.5 0.39 (1)	10.00			
I-T	0 / 0	-18.5	-18.5 0.16 (1)	10.00			
T-U	0 / 0	-18.5	-18.5 0.16 (1)	10.00			
U-H	0 / 0	-18.5	-18.5 0.16 (1)	10.00			
H-G	0 / 0	-18.5	-18.5 0.16 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
P	10-9-8	-1670	-1670	---	BACK	VERT	TOTAL	---	C1
Q	11-11-4	-370	-370	---	BACK	VERT	TOTAL	---	C1
R	12-11-4	-370	-370	---	BACK	VERT	TOTAL	---	C1
S	14-11-4	-370	-370	---	BACK	VERT	TOTAL	---	C1
T	16-11-4	-370	-370	---	BACK	VERT	TOTAL	---	C1
U	18-11-4	-370	-370	---	BACK	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	DL =	25.6	PSF
	LL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR
SMALL BUILDING REQUIREMENTS OF PART 9,
NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.79/1.00 (F-G:1), BC=0.48/1.00 (K-L:1),
WB=0.61/1.00 (F-I:1), SSI=0.61/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	187

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (I) (INPUT = 0.90)
JSI METAL= 0.76 (D) (INPUT = 1.00)



Structural component only
DWG# T-2121221 *lll*

JOB NAME 412865	TRUSS NAME T112	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
D	TTWW-m	MT20	5.0	8.0	1.75	4.00
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	6.0	9.0	Edge	
G	BMV1+p	MT20	3.0	8.0		
I	BMVWW-t	MT20	6.0	9.0	2.75	4.50
J	BS-t	MT20	6.0	9.0		
K	BMVW+t	MT20	5.0	8.0	4.25	2.50
L	BMVW-t	MT20	5.0	6.0		
M	BMV+w	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

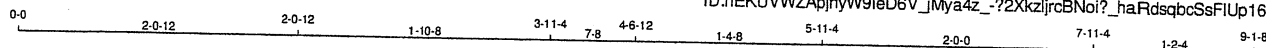


Structural component only
DWG# T-2121221 *m*

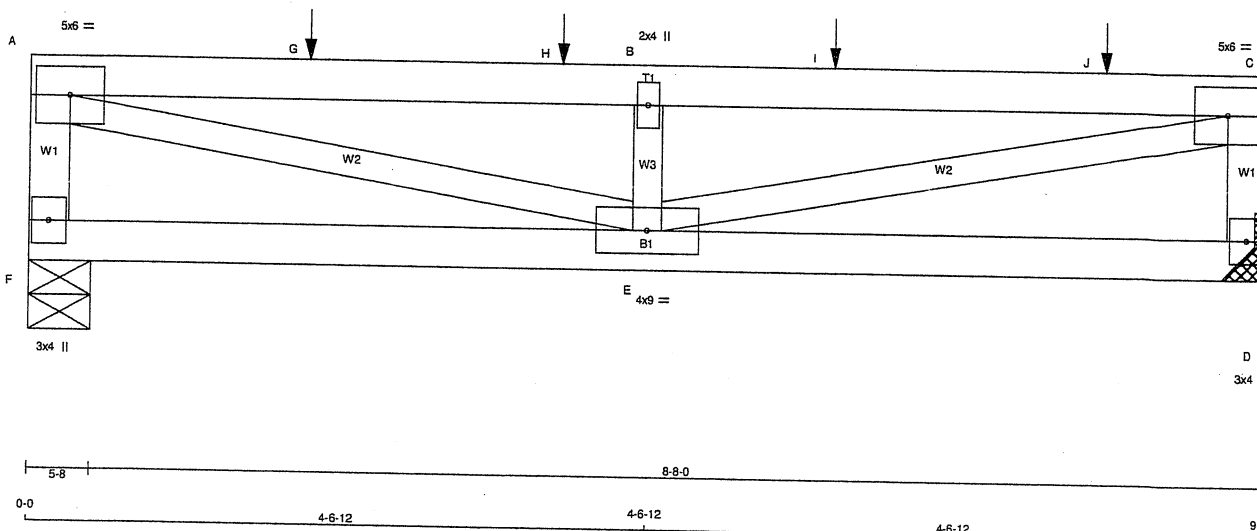
JOB NAME 412865	TRUSS NAME T113	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale: 3/4"=1'



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
F - A	2x4	DRY	No.2
A - C	2x4	DRY	No.2
D - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
F - A	12	TOP
A - C	12	TOP
C - D	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F - D	12	SIDE (14.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0		
B	TMVW-w	MT20	2.0	4.0		
C	TMVW-t	MT20	5.0	6.0		
D	BMV1-p	MT20	3.0	4.0		
E	BMVWW-t	MT20	4.0	9.0		
F	BMV1-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX	IN-SX	IN-SX
F	1069	0	1069	0	5-8	5-8		
D	1236	0	1236	0	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4'-0".

UNFACTORED REACTIONS

1ST LCASE	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	767	444 / 0	0 / 0	0 / 0	0 / 0	323 / 0	0 / 0
D	883	529 / 0	0 / 0	0 / 0	0 / 0	355 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.03 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
F-A	-968 / 0	0.0	0.05 (1)	A-E	0 / 2811	0.35 (1)	
A-G	-2718 / 0	-91.8	-91.8 0.40 (1)	E-B	-1246 / 0	0.10 (1)	
G-H	-2718 / 0	-91.8	-91.8 0.40 (1)	E-C	0 / 2811	0.35 (1)	
H-B	-2718 / 0	-91.8	-91.8 0.40 (1)				
B-I	-2718 / 0	-91.8	-91.8 0.40 (1)				
I-J	-2718 / 0	-91.8	-91.8 0.40 (1)				
J-C	-2718 / 0	-91.8	-91.8 0.40 (1)				
D-C	-1135 / 0	0.0	0.06 (1)				
F-E	0 / 0	-43.5	-43.5 0.11 (4)				
E-D	0 / 0	-43.5	-43.5 0.11 (4)				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	2-0-12	-131	-131	---	TOP	VERT	TOTAL	---	C1
H	3-11-4	-208	-208	---	TOP	VERT	TOTAL	---	C1
I	5-11-4	-208	-208	---	TOP	VERT	TOTAL	---	C1
J	7-11-4	-208	-208	---	TOP	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 31 = 61 lb

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.30")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.30")
CALCULATED VERT. DEFL.(TL) = L/999 (0.09")

CSI: TC=0.40/1.00 (A-B:1), BC=0.11/1.00 (E-F:4), WB=0.35/1.00 (A-E:1), SS=0.28/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PS) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (E) (INPUT = 0.90)
JSI METAL= 0.31 (A) (INPUT = 1.00)



Structural component only
DWG# T-2121222

Tamarack Roof Truss, Burlington



DRY: SEASONED LUMBER

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

TOTAL LOAD CASES: (4)

C H O R D S					W E B S				
MEMB.	MAX. FACTORED	FACTORED			MAX. UNBRAC	MEMB.	MAX. FACTORED	MAX	
	(LBS)	VERT. LOAD	LC1	MAX			FORCE		CSI (LC)
FR-TO		FROM	PLF	CSI (LC)	LENGTH	FR-TO			
A-B	-491 / 0	-91.8	-91.8	0.25 (1)	6.25	E-B	0 / 86	0.03 (4)	
B-C	-22 / 0	-91.8	-91.8	0.24 (1)	6.25	B-D	-551 / 0	0.26 (1)	
D-C	-166 / 0	0.0	0.0	0.07 (1)	7.81	A-E	0 / 469	0.11 (1)	
F-A	-471 / 0	0.0	0.0	0.05 (1)	7.81				
F-E	0 / 0	-18.5	-18.5	0.11 (4)	10.00				
E-D	0 / 460	-18.5	-18.5	0.14 (4)	10.00				

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR
SMALL BUILDING REQUIREMENTS OF PART 9,
NBCC 2015

- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.30")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.30")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.02")

CSI: TC=0.25/1.00 (A-B:1), BC=0.14/1.00 (D-E:4),
WB=0.26/1.00 (B-D:1), SSI=0.18/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (A) (INPUT = 0.90)
JSI METAL= 0.20 (A) (INPUT = 1.00)

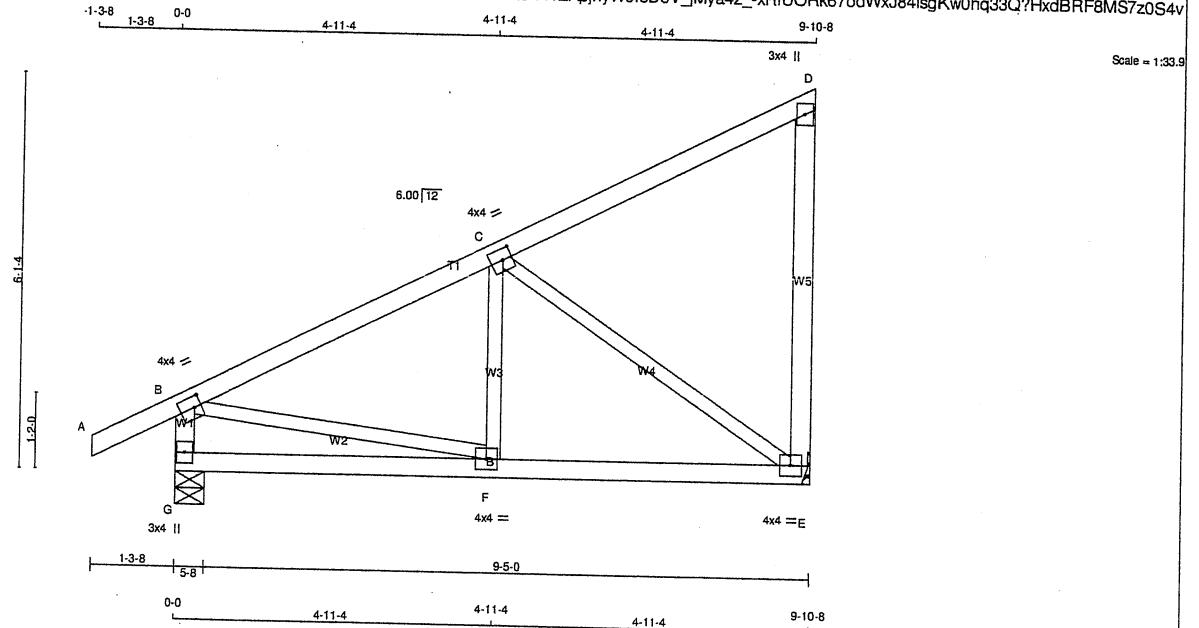


Structural component only
DWG# T-2121223

JOB NAME 412865	TRUSS NAME T115	QUANTITY 5	PLY 1	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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LUMBER	CHORDS	SIZE	DRY	LUMBER	DESCR.
N. L. G. A. RULES	A - D	2x4	DRY	No.2	SPF
	E - D	2x4	DRY	No.2	SPF
	G - B	2x4	DRY	No.2	SPF
	G - E	2x4	DRY	No.2	SPF
	ALL WEBS	2x3	DRY	No.2	SPF
	EXCEPT				

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	IN-SX	IN-SX
E	544	0	5-8	5-8
G	669	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

1ST LCASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED SNOW LIVE PERM. LIVE WIND DEAD SOIL
E	385 253 / 0 0 / 0 0 / 0 132 / 0 0 / 0
G	471 322 / 0 0 / 0 0 / 0 149 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	LC2 MAX	MAX. UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH
FR-TO							FR-TO			
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00		F-C	0 / 97	0.03 (4)	
B-C	-534 / 0	-91.8	-91.8	0.29 (1)	6.25		C-E	-602 / 0	0.35 (1)	
C-D	-25 / 0	-91.8	-91.8	0.28 (1)	6.25		B-F	0 / 507	0.11 (1)	
E-D	-172 / 0	0.0	0.0	0.12 (1)	7.81					
G-B	-633 / 0	0.0	0.0	0.06 (1)	7.81					
G-F	0 / 0	-18.5	-18.5	0.12 (4)	10.00					
F-E	0 / 499	-18.5	-18.5	0.17 (4)	10.00					

TOTAL WEIGHT = 5 X 43 = 217 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.33")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.01")
ALLOWABLE DEFL.(TL) = $L/360$ (0.33")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.02")

CSI: TC=0.29/1.00 (B-C:1), BC=0.17/1.00 (E-F:4), WB=0.35/1.00 (C-E:1), SSI=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

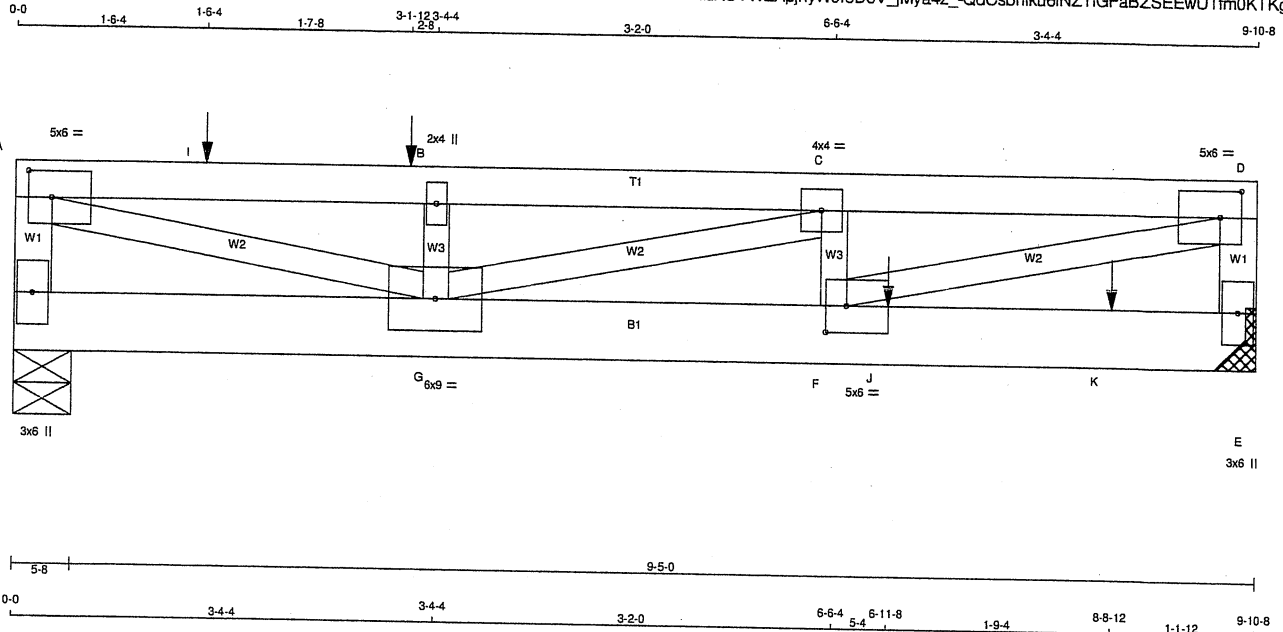
JSI GRIP= 0.87 (B) (INPUT = 0.90)
JSI METAL= 0.24 (B) (INPUT = 1.00)



Structural component only
DWG# T-2121224

JOB NAME 412865	TRUSS NAME T116	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
H - A	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - E	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
H - A 1 12		TOP
A - D 1 7		SIDE(183.1)
D - E 1 12		TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
H - E 2 12		SIDE(14.0)
WEBS : (0.122"x3") SPIRAL NAILS		
G - B 1 5		SIDE(33.1)
2x3 1 6		

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWV-1	MT20	5.0	6.0	2.50	2.25
B	TMWV-w	MT20	2.0	4.0		
C	TMWV-1	MT20	4.0	4.0		
D	TMWV-1	MT20	5.0	6.0	2.50	2.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
H	2332	0	2332	0	5-8	5-8
E	2363	0	2363	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
H	1659	1032 / 0	0 / 0	0 / 0	0 / 0	627 / 0	0 / 0
E	1685	1027 / 0	0 / 0	0 / 0	0 / 0	658 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.59 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
H-A	-2094 / 0	0.0	0.0 0.12 (1)	F-D	0 / 5075	0.63 (1)	
A-I	-4644 / 0	-91.8	-91.8 0.63 (1)	A-G	0 / 4898	0.61 (1)	
I-B	-4644 / 0	-91.8	-91.8 0.63 (1)	F-C	-112 / 38	0.01 (1)	
B-C	-4644 / 0	-91.8	-91.8 0.26 (1)	G-B	-1676 / 0	0.13 (1)	
C-D	-4811 / 0	-91.8	-91.8 0.22 (1)	G-C	-178 / 0	0.02 (4)	
E-D	-1794 / 0	0.0	0.0 0.10 (1)				
H-G	0 / 0	-43.5	-43.5 0.20 (1)				
G-F	0 / 4811	-43.5	-43.5 0.52 (1)				
F-J	0 / 0	-43.5	-43.5 0.24 (1)				
J-K	0 / 0	-43.5	-43.5 0.24 (1)				
K-E	0 / 0	-43.5	-43.5 0.24 (1)				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
B	3-1-12	-584	-584		FRONT	VERT	TOTAL		C1
I	1-6-4	-584	-584		FRONT	VERT	TOTAL		C1
J	6-11-8	-869	-869		FRONT	VERT	TOTAL		C1
K	8-8-12	-341	-341		FRONT	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 8.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.33")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.33")
CALCULATED VERT. DEFL.(TL) = L/925 (0.13")

CSI: TC=0.63/1.00 (A-B:1), BC=0.52/1.00 (F-G:1), WB=0.63/1.00 (D-F:1), SSI=0.42/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

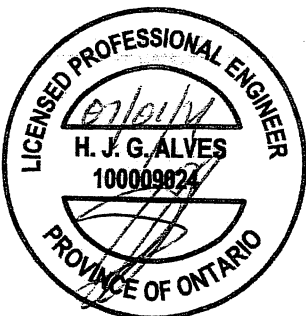
NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (A) (INPUT = 0.90)
JSI METAL= 0.55 (D) (INPUT = 1.00)



Structural component only
DWG# T-2121225 1/1

CONTINUED ON PAGE 2

JOB NAME 412865	TRUSS NAME T116	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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PLATES (table is in inches)

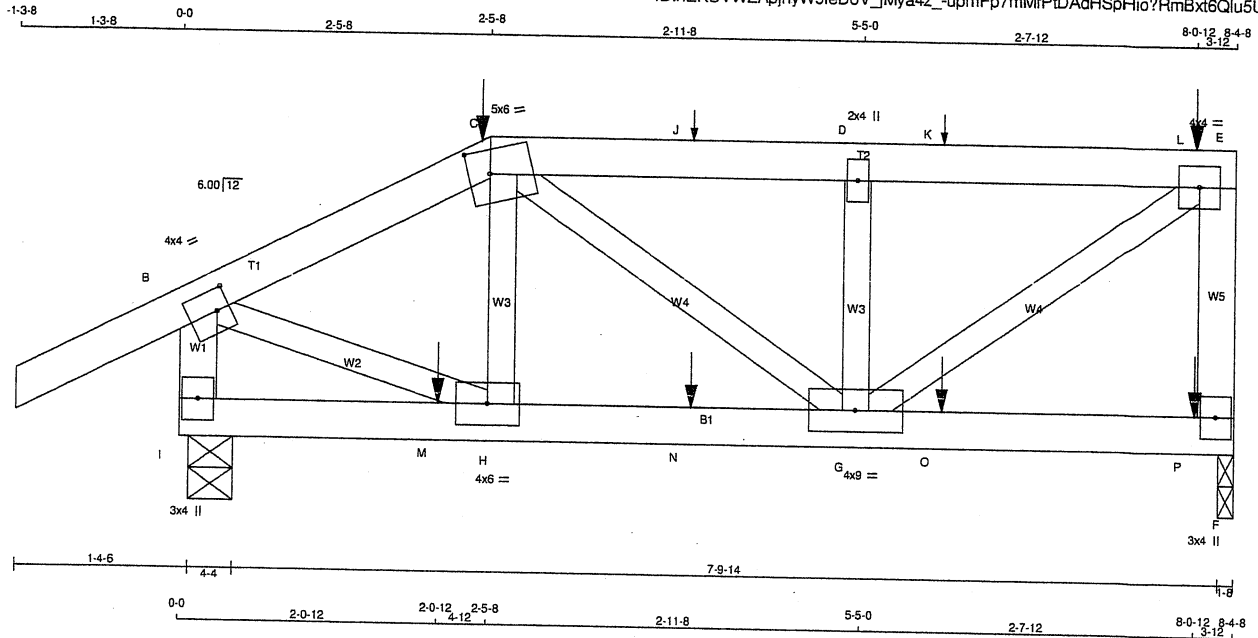
JT	TYPE	PLATES	W	LEN	Y	X
E	BMV1+p	MT20	3.0	6.0		
F	BMWW-t	MT20	5.0	6.0	2.50	2.00
G	BMWWW-t	MT20	6.0	9.0		
H	BMV1+p	MT20	3.0	6.0		



Structural component only
DWG# T-2121225 *MA*

JOB NAME 412865	TRUSS NAME T117	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 1 12:28:46 2021 Page 1
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Scale = 1:17.3

LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	SPF	
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
F - E	2x4	DRY	No.2	SPF	
I - B	2x4	DRY	No.2	SPF	
I - F	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TTWW-m	MT20	5.0	6.0	2.25	2.00
D	TMVW-w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	4.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMVWW-t	MT20	4.0	9.0		
H	BMVWW-t	MT20	4.0	6.0		
I	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	UPLIFT
F	527	0	527	0
I	668	0	668	0

UNFACTORED REACTIONS

1ST LCASE	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	373	245 / 0	0 / 0	0 / 0	0 / 0	128 / 0	0 / 0
F	469	327 / 0	0 / 0	0 / 0	0 / 0	142 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-91.8 -91.8	0.13 (1)	10.00	H-C	-106 / 11	0.02 (1)
B-C	-546 / 0	-91.8 -91.8	0.11 (1)	6.25	C-G	0 / 44	0.01 (1)
C-J	-518 / 0	-91.8 -91.8	0.14 (1)	6.25	G-D	-332 / 0	0.06 (1)
J-D	-518 / 0	-91.8 -91.8	0.14 (1)	6.25	G-E	0 / 633	0.16 (1)
D-K	-518 / 0	-91.8 -91.8	0.14 (1)	6.25	B-H	0 / 515	0.13 (1)
K-L	-518 / 0	-91.8 -91.8	0.14 (1)	6.25			
L-E	-518 / 0	-91.8 -91.8	0.14 (1)	6.25			
F-E	-494 / 0	0.0 0.0	0.07 (1)	7.81			
I-B	-646 / 0	0.0 0.0	0.07 (1)	7.81			
I-M	0 / 0	-18.5 -18.5	0.03 (4)	10.00			
M-H	0 / 0	-18.5 -18.5	0.03 (4)	10.00			
H-N	0 / 483	-18.5 -18.5	0.11 (1)	10.00			
N-G	0 / 483	-18.5 -18.5	0.11 (1)	10.00			
G-O	0 / 0	-18.5 -18.5	0.04 (4)	10.00			
O-P	0 / 0	-18.5 -18.5	0.04 (4)	10.00			
P-F	0 / 0	-18.5 -18.5	0.04 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-5-8	-74	-74		FRONT	VERT	TOTAL		C1
J	4-0-12	1	1		FRONT	VERT	TOTAL		C1
K	6-0-12	1	1		FRONT	VERT	TOTAL		C1
L	8-0-12	-17	-17		FRONT	VERT	TOTAL		C1
M	2-0-12	-3	-3		FRONT	VERT	TOTAL		C1
N	4-0-12	-1	-1		FRONT	VERT	TOTAL		C1
O	6-0-12	-1	-1		FRONT	VERT	TOTAL		C1
P	8-0-12	-6	-6		FRONT	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 34 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.28")
CALCULATED VERT. DEFL.(LL)= L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.28")
CALCULATED VERT. DEFL.(TL)= L/999 (0.01")

CSI: TC=0.14/1.00 (D-E:1), BC=0.11/1.00 (G-H:1), WB=0.16/1.00 (E-G:1), SSI=0.15/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

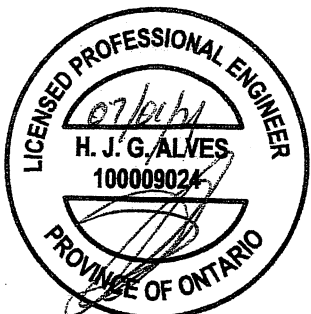
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788

PLATE PLACEMENT TOL. = 0.250 inches

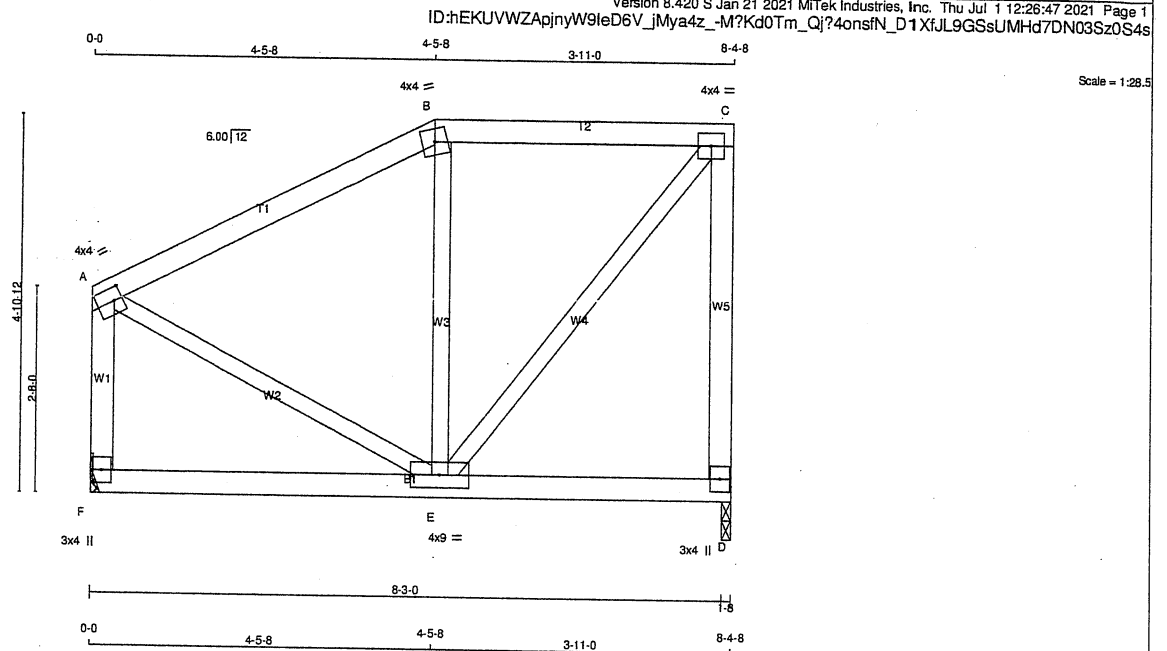
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (B) (INPUT = 0.90)
JSI METAL= 0.22 (B) (INPUT = 1.00)



Structural component only
DWG# T-2121226

JOB NAME 412865	TRUSS NAME T118	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - B	2x4	DRY	No.2
B - C	2x4	DRY	No.2
D - C	2x4	DRY	No.2
F - A	2x4	DRY	No.2
F - D	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW-t	MT20	4.0	4.0	2.00 1.25
B	TTW-m	MT20	4.0	4.0	
C	TMVW-t	MT20	4.0	4.0	
D	BMV1+p	MT20	3.0	4.0	
E	BMVWW-t	MT20	4.0	9.0	
F	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
D	462	0	462	0	1-8	1-8
F	462	0	462	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LOASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	326	214 / 0	0 / 0	0 / 0	0 / 0	112 / 0	0 / 0
F	326	214 / 0	0 / 0	0 / 0	0 / 0	112 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH	MEMB.	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	-245 / 0	-91.8	-91.8 0.23 (1)	E-B	-275 / 0	0.10 (1)	
B-C	-212 / 0	-91.8	-91.8 0.18 (1)	E-C	0 / 333	0.07 (1)	
D-C	-436 / 0	0.0	0.0 0.17 (1)	A-E	0 / 246	0.06 (1)	
F-A	-427 / 0	0.0	0.0 0.06 (1)				
F-E	0 / 0	-18.5	-18.5 0.09 (4)				
E-D	0 / 0	-18.5	-18.5 0.09 (4)				

TOTAL WEIGHT = 39 lb [M/F]

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.28")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.28")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.01")

CSI: TC=0.23/1.00 (A-B:1), BC=0.09/1.00 (E-F:4), WB=0.10/1.00 (B-E:1), SS=0.14/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

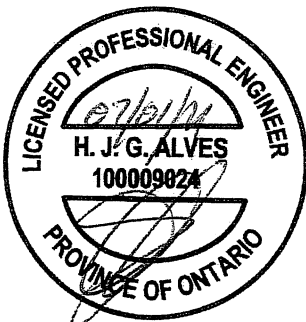
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES		PLATE GRIP(DRY)		SHEAR		SECTION	
(PSI)	(PLI)	(PSI)	(PLI)	(PSI)	(PLI)	(PSI)	(PLI)
MAX	MIN	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.36 (A) (INPUT = 0.90)
JSI METAL = 0.11 (A) (INPUT = 1.00)

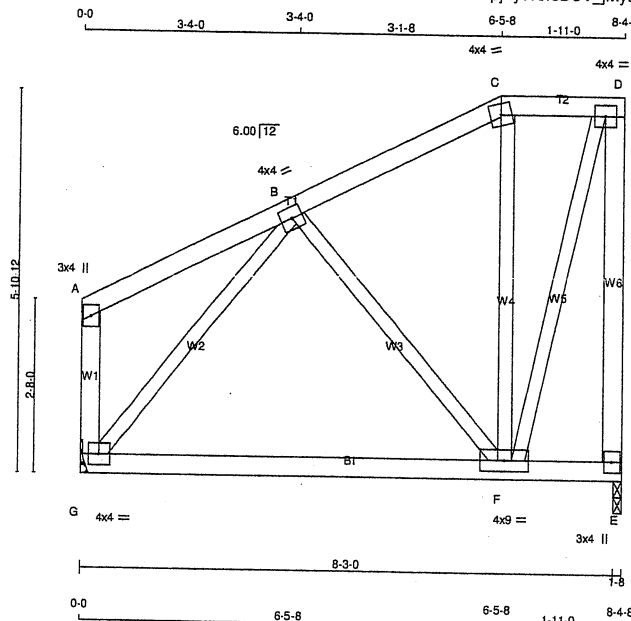


Structural component only
DWG# T-2121227

JOB NAME 412865	TRUSS NAME T119	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1:33.8

LUMBER					DESCR.
N. L. G. A. RULES	CHORDS	SIZE	LUMBER		
A - C	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	
G - A	2x4	DRY	No.2	SPF	
G - E	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWV-t	MT20	4.0	4.0		
C	TTW-m	MT20	4.0	4.0		
D	TMWV-t	MT20	4.0	4.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMWVW-t	MT20	4.0	9.0		
G	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD BRG
	VERT	HORZ	DOWN	HORZ		
E	462	0	462	0	1-8	1-8
G	462	0	462	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	326	214 / 0	0 / 0	0 / 0	0 / 0	112 / 0	0 / 0
G	326	214 / 0	0 / 0	0 / 0	0 / 0	112 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MAX. MEMB. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX LC1 (LC)
FR-TO		FROM	TO		FR-TO		
A-B	0 / 16	-91.8	-91.8 0.16 (1)	10.00	B-F	-175 / 0	0.08 (1)
B-C	-162 / 0	-91.8	-91.8 0.12 (1)	6.25	F-C	-135 / 0	0.07 (1)
C-D	-128 / 0	-91.8	-91.8 0.04 (1)	6.25	F-D	0 / 416	0.09 (1)
E-D	-463 / 0	0.0	0.0 0.32 (1)	7.81	G-B	-385 / 0	0.16 (1)
G-A	-113 / 0	0.0	0.0 0.02 (1)	7.81			
G-F	0 / 243	-18.5	-18.5 0.19 (4)	10.00			
F-E	0 / 0	-18.5	-18.5 0.18 (4)	10.00			

TOTAL WEIGHT = 45 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.28")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.01")
ALLOWABLE DEFL.(TL) = $L/360$ (0.28")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.05")

CSI: TC=0.32/1.00 (D-E:1), BC=0.19/1.00 (F-G:4), WB=0.16/1.00 (B-G:1), SSI=0.13/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PS) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.55 (D) (INPUT = 0.90)
JSI METAL= 0.12 (D) (INPUT = 1.00)

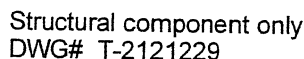


Structural component only
DWG# T-2121228

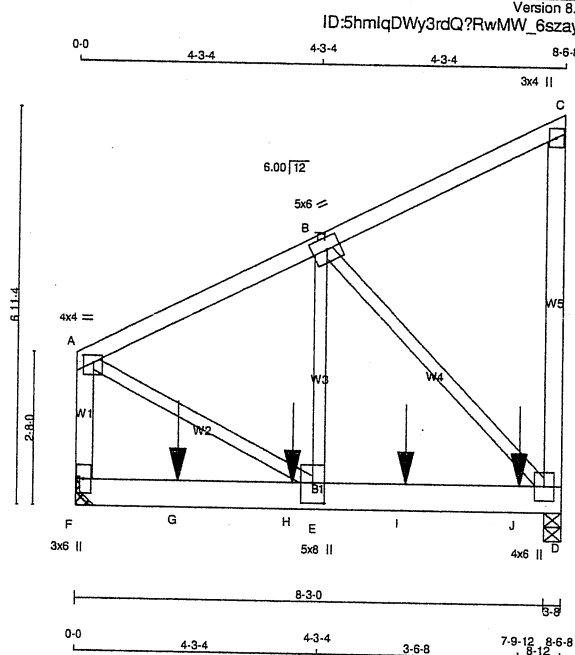
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JSI GRIP= 0.44 (A) (INPUT = 0.90)
JSI METAL= 0.12 (A) (INPUT = 1.00)



JOB NAME 412865	TRUSS NAME T121	QUANTITY 1	PLY 3	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER				DESCR.
N.L. G. A. RULES	CHORDS	SIZE	LUMBER	
F - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS 2x3 DRY No.2				SPF
DRY: SEASONED LUMBER.				

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
F-A 1	12	TOP
A-C 1	12	TOP
C-D 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F-D 2	6	SIDE(545.9)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW-p	MT20	4.0	4.0	1.50 2.00
B	TMVW-t	MT20	5.0	6.0	
C	TMV-p	MT20	3.0	4.0	
D	BMVW1-p	MT20	4.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
F	4121	0	4121	0	0	MECHANICAL
D	5184	0	5184	0	0	3-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 4-0.

UNFACTORED REACTIONS						
1ST CASE	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
JT	2911	1927 / 0	0 / 0	0 / 0	0 / 0	984 / 0
D	3662	2424 / 0	0 / 0	0 / 0	0 / 0	1238 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.99 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 MAX (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LC1 MAX (LC)	
FR-TO		FROM TO		FR-TO			
F-A	-3150 / 0	0.0	0.0 0.14 (1)	A-E	0 / 3284	0.25 (1)	
A-B	-3223 / 0	-91.8	-91.8 0.10 (1)	E-B	0 / 3972	0.30 (1)	
B-C	-18 / 0	-91.8	-91.8 0.08 (1)	B-D	-4167 / 0	0.85 (1)	
D-C	-156 / 0	0.0	0.0 0.05 (1)				
F-G	0 / 0	-18.5	-18.5 0.47 (1)				
G-H	0 / 0	-18.5	-18.5 0.47 (1)				
H-E	0 / 0	-18.5	-18.5 0.47 (1)				
E-I	0 / 2899	-18.5	-18.5 0.59 (1)				
I-J	0 / 2899	-18.5	-18.5 0.59 (1)				
J-D	0 / 2899	-18.5	-18.5 0.59 (1)				

SPECIFIED CONCENTRATED LOADS (LBS)							
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
G	1-9-12	-1476	-1476	---	BACK	VERT	TOTAL
H	3-9-12	-1476	-1476	---	BACK	VERT	TOTAL
I	5-9-12	-1476	-1476	---	BACK	VERT	TOTAL
J	7-9-12	-1476	-1476	---	BACK	VERT	TOTAL

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:	
TOP CH.	LL = 25.6 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.28")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.28")
CALCULATED VERT. DEFL.(TL) = L/999 (0.06")

CSI: TC=0.14/1.00 (A-F:1), BC=0.59/1.00 (D-E:1), WB=0.85/1.00 (B-D:1), SSI=0.66/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

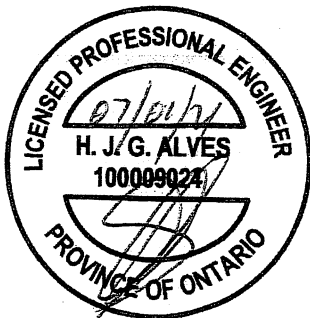
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY)	SHEAR	SECTION	(PSI)	(PLI)
MT20	650	371	1747	788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (A) (INPUT = 0.90)
JSI METAL= 0.44 (D) (INPUT = 1.00)



Structural component only
DWG# T-2121230

JOB NAME 412865	TRUSS NAME T121	QUANTITY 1	PLY 3	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:5hmlqDWy3rdQ?RwMW 6szaya40M-IOSNR8oEyKFo1401UPGVd4Oib940dy42wbXs78Kz0S4q

PLATES (table is in inches)

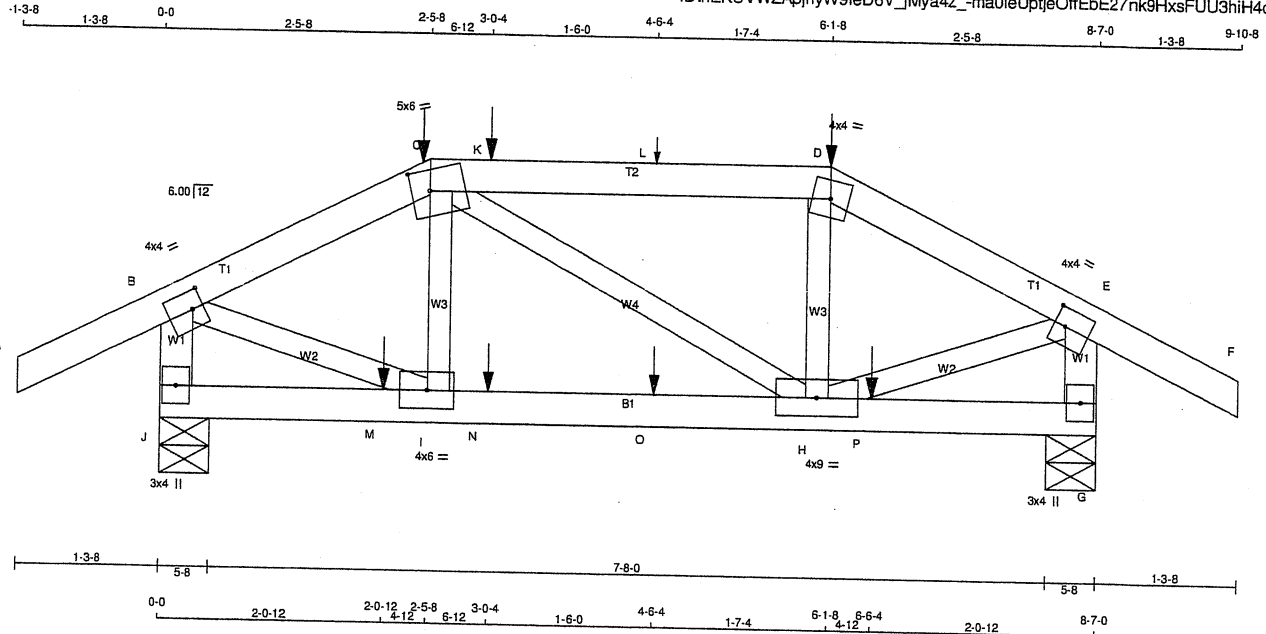
JT	TYPE	PLATES	W	LEN	Y	X
E	BMWVH	MT20	5.0	8.0	4.25	2.50
F	BMV1+p	MT20	3.0	6.0		



Structural component only
DWG# T-2121230 *mn*

JOB NAME 412865	TRUSS NAME T122	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID:hEKUVVWZApjnyW9leD6V_jMya4z_ma0leUptjeOffEbE27nk9HxsFUU3hiH4qBggnz0S4p



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
J - B	2x4	DRY	No.2	SPF	
G - E	2x4	DRY	No.2	SPF	
J - G	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TTWW-m	MT20	5.0	6.0	2.25	2.00
D	TTWW-m	MT20	4.0	4.0		
E	TMVW-t	MT20	4.0	4.0	2.00	1.25
G	BMV1+p	MT20	3.0	4.0		
H	BMVW-t	MT20	4.0	9.0		
I	BMVW-t	MT20	4.0	6.0		
J	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	IN-SX	IN-SX
J	720	0	0	5-8
G	716	0	0	5-8

UNFACTORED REACTIONS

JT	1ST CASE	MAX./MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
J	505	354 / 0	0 / 0	0 / 0	0 / 0	0 / 0	151 / 0	0 / 0
G	502	352 / 0	0 / 0	0 / 0	0 / 0	0 / 0	150 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED MEMB. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC2)	MAX. FACTORED VERT. LOAD (LC3)	MAX. FACTORED VERT. LOAD (LC4)	MAX. FACTORED VERT. LOAD (LC5)	MAX. FACTORED VERT. LOAD (LC6)	MAX. FACTORED VERT. LOAD (LC7)	MAX. FACTORED VERT. LOAD (LC8)	MAX. FACTORED VERT. LOAD (LC9)	MAX. FACTORED VERT. LOAD (LC10)	MAX. FACTORED VERT. LOAD (LC11)	MAX. FACTORED VERT. LOAD (LC12)	MAX. FACTORED VERT. LOAD (LC13)	MAX. FACTORED VERT. LOAD (LC14)	MAX. FACTORED VERT. LOAD (LC15)	MAX. FACTORED VERT. LOAD (LC16)	MAX. FACTORED VERT. LOAD (LC17)	MAX. FACTORED VERT. LOAD (LC18)	MAX. FACTORED VERT. LOAD (LC19)	MAX. FACTORED VERT. LOAD (LC20)
FR-TO																						
A-B	0 / 28	-91.8	-91.8	0.13 (1)	10.00																	
B-C	-617 / 0	-91.8	-91.8	0.11 (1)	6.25																	
C-K	-540 / 0	-91.8	-91.8	0.24 (1)	6.25																	
K-L	-540 / 0	-91.8	-91.8	0.24 (1)	6.25																	
L-D	-540 / 0	-91.8	-91.8	0.24 (1)	6.25																	
D-E	-611 / 0	-91.8	-91.8	0.11 (1)	6.25																	
E-F	0 / 28	-91.8	-91.8	0.13 (1)	10.00																	
J-B	-700 / 0	0.0	0.0	0.08 (1)	7.81																	
G-E	-695 / 0	0.0	0.0	0.08 (1)	7.81																	
J-M	0 / 0	-18.5	-18.5	0.04 (4)	10.00																	
M-I	0 / 0	-18.5	-18.5	0.04 (4)	10.00																	
I-N	0 / 546	-18.5	-18.5	0.12 (1)	10.00																	
N-O	0 / 546	-18.5	-18.5	0.12 (1)	10.00																	
O-H	0 / 546	-18.5	-18.5	0.12 (1)	10.00																	
H-P	0 / 0	-18.5	-18.5	0.04 (4)	10.00																	
P-G	0 / 0	-18.5	-18.5	0.04 (4)	10.00																	

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-5-8	-74	-74	---	BACK	VERT	TOTAL	---	C1
D	6-1-8	-74	-74	---	BACK	VERT	TOTAL	---	C1
K	3-0-4	-9	-9	---	BACK	VERT	TOTAL	---	C1
L	4-6-4	1	1	---	BACK	VERT	TOTAL	---	C1
M	2-0-12	-3	-3	---	BACK	VERT	TOTAL	---	C1
N	3-0-4	-1	-1	---	BACK	VERT	TOTAL	---	C1
O	4-6-4	-1	-1	---	BACK	VERT	TOTAL	---	C1
P	6-6-4	-3	-3	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 35 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.6	PSF
DL	= 6.0	PSF	
BOT CH.	LL	= 0.0	PSF
DL	= 7.4	PSF	
TOTAL LOAD	= 39.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.29")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.29")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.24/1.00 (C-D:1), BC=0.12/1.00 (H-I:1), WB=0.14/1.00 (B-I:1), SS=0.15/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

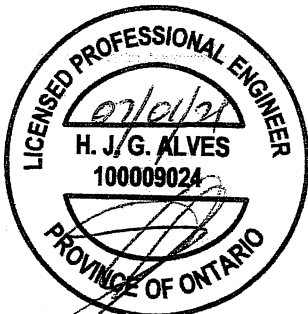
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

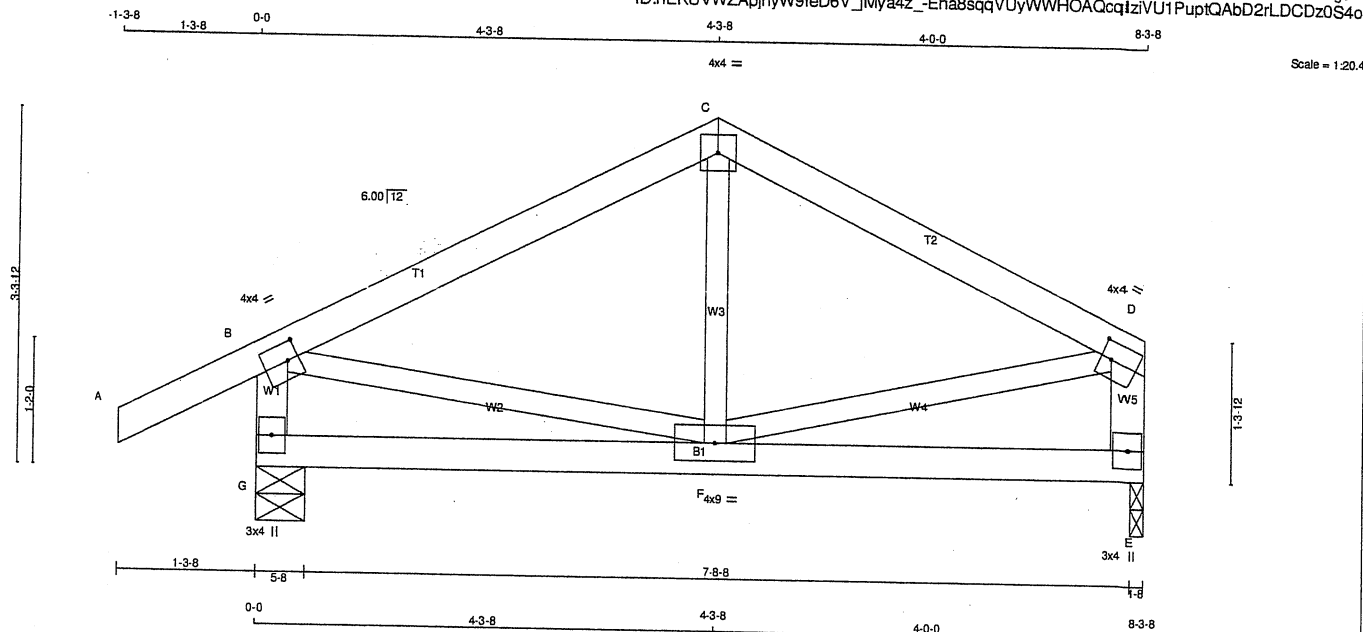
JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.25 (B) (INPUT = 1.00)



Structural component only
DWG# T-2121231

JOB NAME 412865	TRUSS NAME T123	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID:hEKUVWZApjnyW9leD6V_jMya4z_-Ena8sqvUyWWHOAcqztzVU1PuptQABD2rLDCDz0S40



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
G - B	2x4	DRY	No.2
E - D	2x4	DRY	No.2
G - E	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00 1.25
C	TTW-p	MT20	4.0	4.0	
D	TMVW-t	MT20	4.0	4.0	2.00 1.25
E	BMV1+p	MT20	3.0	4.0	
F	BMVWW-t	MT20	4.0	9.0	
G	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX	IN-SX	IN-SX
G	582	0	582	0	0	5-8	5-8	
E	457	0	457	0	0	1-8	1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM.LIVE	WIND			
G	409	282 / 0	0 / 0	0 / 0	0 / 0	127 / 0	0 / 0
E	323	212 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)		FACTORED VERT. LOAD LC1 MAX (PLF)		MAX. UNBRACED LENGTH FR-TO	MEMB.	WEBS MAX. FACTORED FORCE (LBS)	
	FR-TO	FROM TO	CSI (LC)	UNBRACED LENGTH FR-TO		FR-TO	FROM TO	CSI (LC)
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	F-C	-54 / 52	0.02 (4)
B-C	-364 / 0	-91.8	-91.8	0.22 (1)	6.25	B-F	0 / 332	0.07 (1)
C-D	-364 / 0	-91.8	-91.8	0.19 (1)	6.25	F-D	0 / 336	0.08 (1)
G-B	-550 / 0	0.0	0.0	0.06 (1)	7.81			
E-D	-428 / 0	0.0	0.0	0.04 (1)	7.81			
G-F	0 / 0	-18.5	-18.5	0.09 (4)	10.00			
F-E	0 / 0	-18.5	-18.5	0.09 (4)	10.00			

TOTAL WEIGHT = 32 lb [M/F]

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.28")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.28")
CALCULATED VERT. DEFL.(TL)= L/999 (0.01")

CSI: TC=0.22/1.00 (B-C:1), BC=0.09/1.00 (F-G:4), WB=0.08/1.00 (D-F:1), SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

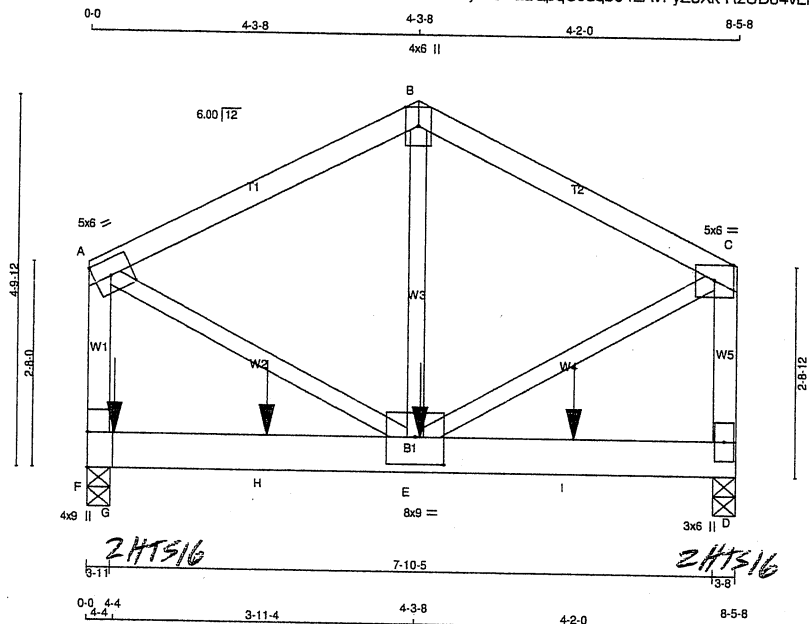
JSI GRIP= 0.56 (B) (INPUT = 0.90)
JSI METAL= 0.17 (B) (INPUT = 1.00)



Structural component only
DWG# T-2121232

JOB NAME 412865	TRUSS NAME T127	QUANTITY 1	PLY 3	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID:4yza9PaaQpQ0dqb94LAvFyZ5Xk-RzUD84vLfMFEsy9HzJKb91oF_h9r3VrOyA6FCRz0P5b



Scale = 1:28.5

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4 DRY	No.2	SPF		
B - C	2x4 DRY	No.2	SPF		
F - A	2x4 DRY	No.2	SPF		
D - C	2x4 DRY	No.2	SPF		
F - D	2x6 DRY	No.2	SPF		
ALL WEBS EXCEPT	2x3 DRY	No.2	SPF		

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-B 1 12	TOP	
B-C 1 12	TOP	
F-A 2 4	SIDE(419.6)	
D-C 1 12	TOP	
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F-D 2 4	SIDE(1383.3)	
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1 6		

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED	MAXIMUM FACTORED	INPUT	REQRD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	JT VERT	DOWN	UP
F 9505	9505	179	3-11
D 6418	6418	0	3-8

PROVIDE ANCHORAGE AT BEARING JOINT F FOR 2751 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 1848 LBS FACTORED UPLIFT

PROVIDE FOR 179 LBS FACTORED HORIZONTAL REACTION AT JOINT F

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS
JT COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
F 6997	4224 / 0 1190 / 0 0 / 0 167 / -2729 1584 / 0 0 / 0
D 4724	2852 / 0 803 / 0 0 / 0 117 / -1836 1069 / 0 0 / 0

HORIZONTAL REACTIONS

F	0 / 0	0 / 0	128 / -126	0 / 0	0 / 0
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BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, D

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 4.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (18)

CHORDS	MAX. FACTORED	FACTORED	W E B S	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1 MAX (PLF)	CSI (LC)	MEMB. MAX. FORCE (LBS)
FR-TO		FROM TO	LENGTH	FR-TO
A-B	-5261 / 1585	-115.2 -115.2	0.19 (2)	4.89
B-C	-5261 / 1587	-115.2 -115.2	0.18 (3)	4.90
F-A	-5094 / 1494	0.0 0.0	0.22 (1)	6.38
D-C	-5232 / 1532	0.0 0.0	0.23 (1)	6.31
F-G	-133 / 154	-39.5 -39.5	0.66 (1)	6.25
G-H	-133 / 154	-39.5 -39.5	0.66 (1)	6.25
H-E	-133 / 154	-39.5 -39.5	0.66 (1)	6.25
E-I	-23 / 47	-39.5 -39.5	0.65 (3)	6.25
I-D	-23 / 47	-39.5 -39.5	0.65 (3)	6.25

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
E	4-4-4	-2685	-2685	605	FRONT	VERT	TOTAL	C1
G	4-4	-2697	-2697	601	FRONT	VERT	TOTAL	C1
H	2-4-4	-2685	-2685	605	FRONT	VERT	TOTAL	C1
I	6-4-4	-2685	-2685	605	FRONT	VERT	TOTAL	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.2 } PSF AT { 30-0-0 } FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE { MAIN WIND FORCE RESISTING SYSTEM }. INTERNAL WIND PRESSURE IS BASED ON DESIGN { CATEGORY 2 }. BUILDING MAY BE LOCATED ON { OPEN TERRAIN }, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST { 0-0 } FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.

TOTAL WEIGHT = 3 X 41 = 124 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 33.4 PSF
	DL = 6.0 PSF
BOT CH.	LL = 10.5 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 57.3 PSF

SPACING = 24.0 IN C/C

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 4 OF CBC 2018, ABC 2019
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- SLOPE REDUCTION FACTOR NOT USED
- PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED.

{ 80 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD } TIMES IMPORTANCE FACTOR EQUALS 33.4 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.28")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/180 (0.56")
CALCULATED VERT. DEFL.(TL) = L/999 (0.07")

CSI: TC=0.23/1.00 (C-D:1), BC=0.66/1.00 (E-F:1), WB=0.40/1.00 (C-E:1), SS=0.89/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.87 (A) (INPUT = 0.90)
JSI METAL = 0.41 (A) (INPUT = 1.00)



Structural component only
DWG# T-2121233 112

CONTINUED ON PAGE 2

JOB NAME 412865	TRUSS NAME T127	QUANTITY 1	PLY 3	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 1 15:50:49 2021 Page 2
ID:4yza9PaaQpqU0dqb94LAvFyZ5Xk-wA2clQwzQ4N5U6kTX1sqiFLQk5U4oy5YAqrpkuZ0P5a

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0		Edge
B	TTW+p	MT20	4.0	6.0		Edge
C	TMVW-p	MT20	5.0	6.0		Edge
D	BMV1+p	MT20	3.0	6.0		
E	BMVWW-t	MT20	8.0	9.0	4.25	4.50
F	BMV1+t	MT20	4.0	9.0		5.50

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.



Structural component only
DWG# T-2121233 *3/1*

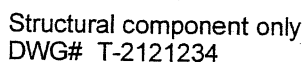
Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 1 12:26:53 2021 Page 1
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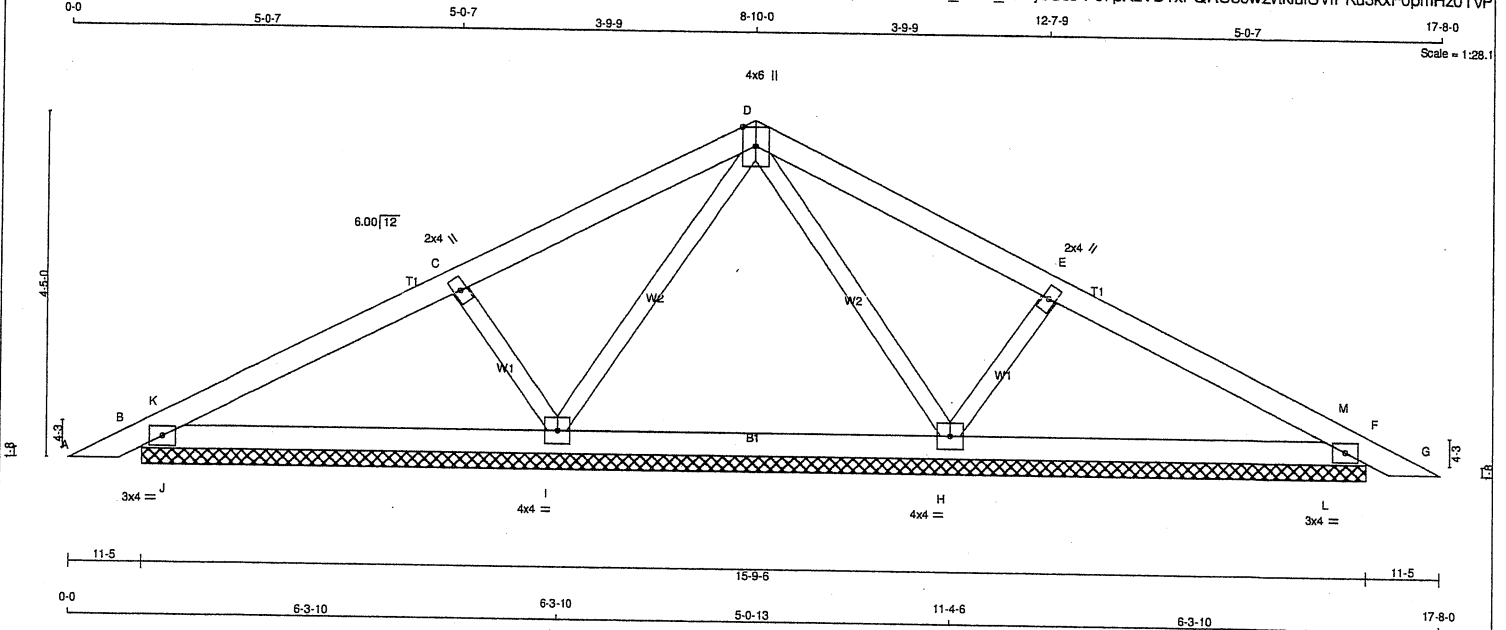
DRY: SEASONED LUMBER

JSI GRIP= 0.88 (I) (INPUT = 0.90)
JSI METAL= 0.29 (E) (INPUT = 1.00)



JOB NAME 412868	TRUSS NAME PB06	QUANTITY 18	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:U6yi?rbeFFwxf_UFm_koDybSsJ-F07pXzVD1xQRU85wzvkiufSVIPKu3kxFOpmHz0TvP



LUMBER				DESCR.
N. L. G. A. RULES	SIZE	LUMBER		
CHORDS				
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	
C	TTWW+w	MT20	2.0	4.0	
D	TTWW+p	MT20	4.0	6.0	Edge
E	TTWW+w	MT20	2.0	4.0	
F	TMB1-I	MT20	3.0	4.0	
H	BMW1-I	MT20	4.0	4.0	
I	BMW1-I	MT20	4.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT		VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
B		340	0	340	0	0	15-9-6	15-9-6	
F		340	0	340	0	0	15-9-6	15-9-6	
H		610	0	610	0	0	15-9-6	15-9-6	
I		610	0	610	0	0	15-9-6	15-9-6	

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	239	166 / 0	0 / 0	0 / 0	0 / 0	73 / 0	0 / 0
F	239	166 / 0	0 / 0	0 / 0	0 / 0	73 / 0	0 / 0
H	432	283 / 0	0 / 0	0 / 0	0 / 0	149 / 0	0 / 0
I	432	283 / 0	0 / 0	0 / 0	0 / 0	149 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, H, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 17	-91.8 -91.8	0.05 (1)	D-H	-197 / 0	10.00	0.07 (1)
B-K	-199 / 0	-91.8 -91.8	0.06 (4)	H-E	-396 / 0	6.25	0.07 (1)
K-C	-143 / 0	-91.8 -91.8	0.23 (1)	I-D	-197 / 0	6.25	0.07 (1)
C-D	0 / 54	-91.8 -91.8	0.24 (1)	C-I	-396 / 0	10.00	0.07 (1)
D-E	0 / 54	-91.8 -91.8	0.24 (1)	J-K	-69 / 58	6.25	0.00 (1)
E-M	-143 / 0	-91.8 -91.8	0.23 (1)	L-M	-69 / 58	6.25	0.00 (1)
M-F	-199 / 0	-91.8 -91.8	0.06 (4)				
F-G	0 / 17	-91.8 -91.8	0.05 (1)				
B-J	0 / 149	-18.5 -18.5	0.08 (1)				
J-I	0 / 149	-18.5 -18.5	0.12 (4)				
I-H	0 / 40	-18.5 -18.5	0.11 (4)				
H-L	0 / 149	-18.5 -18.5	0.12 (4)				
L-F	0 / 149	-18.5 -18.5	0.08 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.24/1.00 (C-D:1), BC=0.12/1.00 (I-J:4), WB=0.07/1.00 (D-H:1), SSI=0.15/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

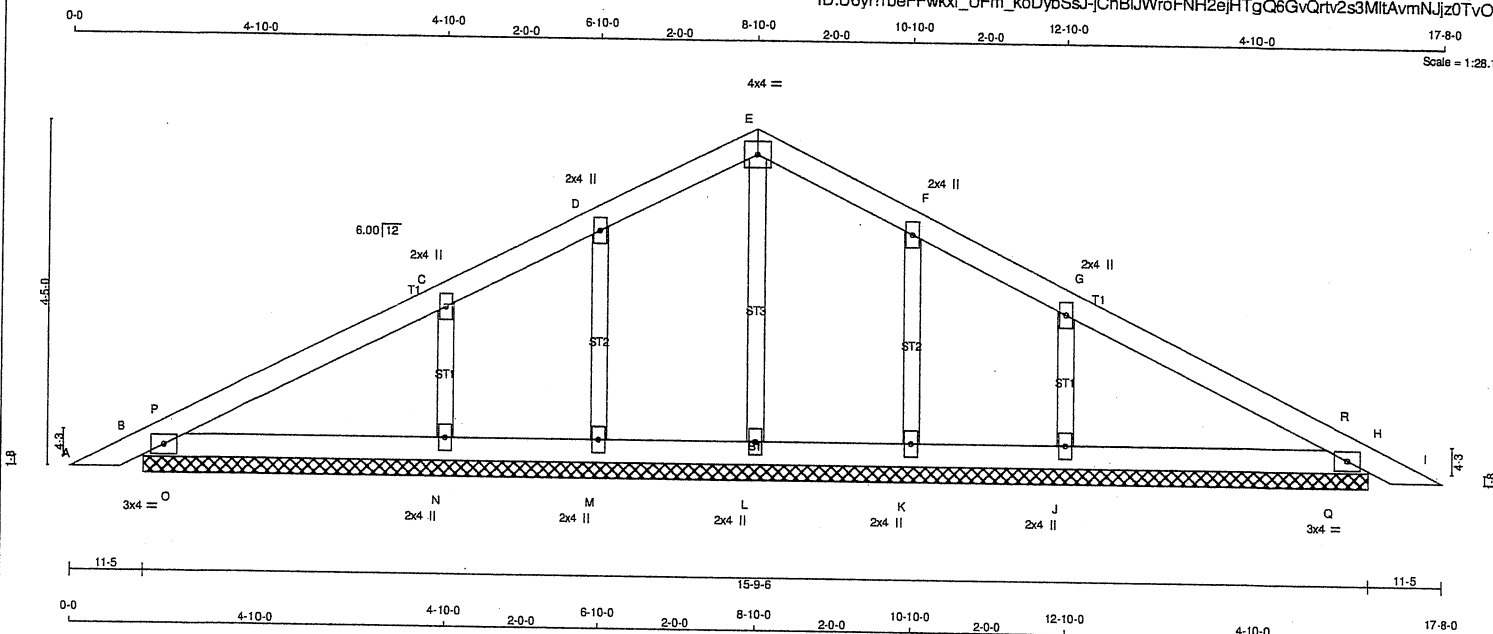
JSI GRIP= 0.24 (B) (INPUT = 0.90)
JSI METAL= 0.08 (E) (INPUT = 1.00)



Structural component only
DWG# T-2121149

JOB NAME 412868	TRUSS NAME PB06G	QUANTITY 4	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID:U6yi?rbeFFwxf_UFm_koDybSsj-jChBUWroFNH2ejHTgQ6GvQrtv2s3MitAvmNjz0TvO



LUMBER				DESCR.	
N. L. G. A. RULES	SIZE	LUMBER		SPF	
CHORDS				SPF	
A - E	2x4	DRY	No.2	SPF	
E - I	2x4	DRY	No.2	SPF	
B - H	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
ALL GABLE WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.				SPF	

GABLE STUDS SPACED AT 2-0-0 OC.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH
FR-TO		FROM TO			FR-TO		
A-B	0 / 17	-91.8 -91.8	0.05 (1)	10.00	L-E	-131 / 0	0.04 (1)
B-P	-44 / 0	-91.8 -91.8	0.02 (4)	6.25	M-D	-160 / 0	0.03 (1)
P-C	-44 / 0	-91.8 -91.8	0.13 (1)	6.25	N-C	-306 / 0	0.05 (1)
C-D	-60 / 0	-91.8 -91.8	0.13 (1)	6.25	K-F	-160 / 0	0.03 (1)
D-E	-45 / 0	-91.8 -91.8	0.05 (1)	6.25	J-G	-306 / 0	0.05 (1)
E-F	-45 / 0	-91.8 -91.8	0.05 (1)	6.25	O-P	-171 / 5	0.00 (1)
F-G	-60 / 0	-91.8 -91.8	0.13 (1)	6.25	Q-R	-171 / 5	0.00 (1)
G-R	-44 / 0	-91.8 -91.8	0.13 (1)	6.25			
R-H	-44 / 0	-91.8 -91.8	0.02 (4)	6.25			
H-I	0 / 17	-91.8 -91.8	0.05 (1)	10.00			
B-O	0 / 49	-18.5 -18.5	0.10 (1)	10.00			
O-N	0 / 49	-18.5 -18.5	0.10 (1)	10.00			
N-M	0 / 39	-18.5 -18.5	0.07 (1)	10.00			
M-L	0 / 36	-18.5 -18.5	0.02 (4)	10.00			
L-K	0 / 36	-18.5 -18.5	0.02 (4)	10.00			
K-J	0 / 39	-18.5 -18.5	0.07 (1)	10.00			
J-Q	0 / 49	-18.5 -18.5	0.10 (1)	10.00			
Q-H	0 / 49	-18.5 -18.5	0.10 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13/1.00 (G-R:1), BC=0.10/1.00 (H-Q:1), WB=0.05/1.00 (G-J:1), SSI=0.14/1.00 (B-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (B) (INPUT = 0.90)
JSI METAL= 0.13 (G) (INPUT = 1.00)

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B TMB1-I	MT20	3.0	4.0		
C, D, F, G					
C TMW-w	MT20	2.0	4.0		
E TTW-p	MT20	4.0	4.0		
H TMB1-I	MT20	3.0	4.0		
J, K, L, M, N					
J BMW1-w	MT20	2.0	4.0		

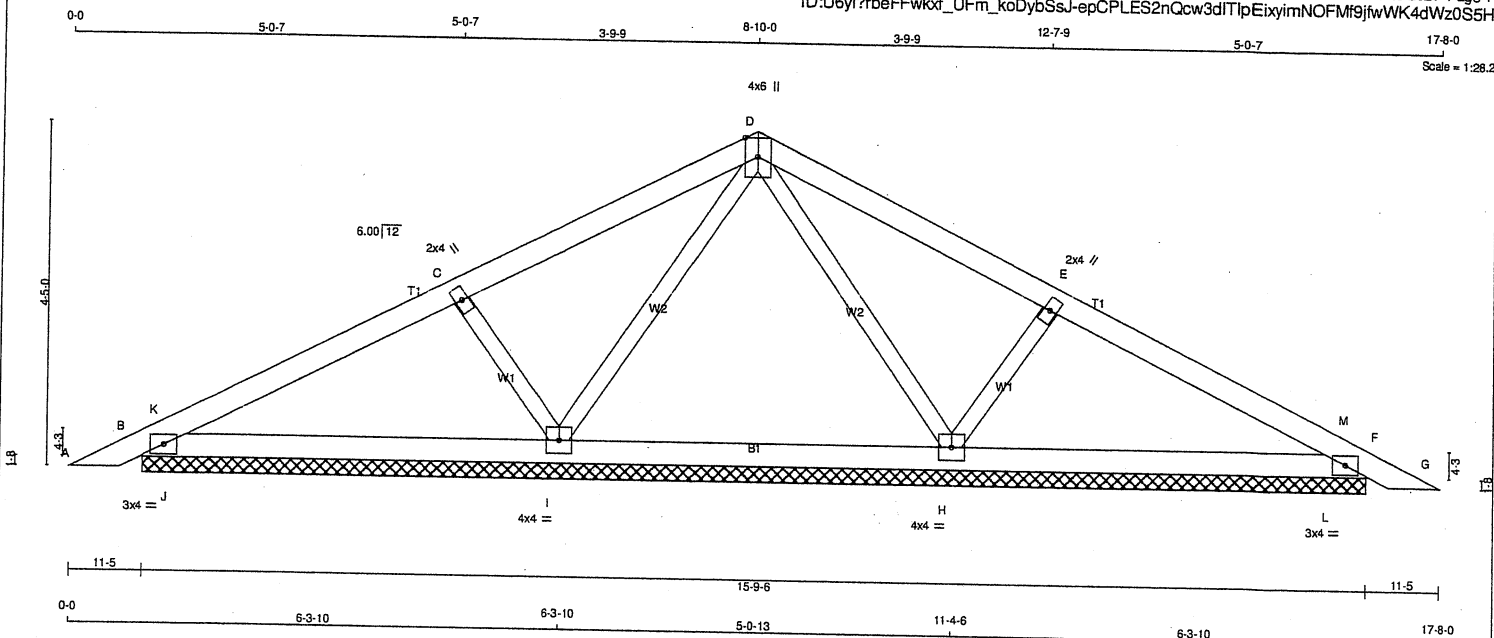


Structural component only
DWG# T-2121150

JOB NAME 412865	TRUSS NAME PB06Z	QUANTITY 1	PLY 3	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:U6yi7rbeFFwxf_UFm_koDybSsJ-epCPLES2nQcw3dITIpEixymNOFM9jfwWK4dWz0S5H



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
B - F	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
SPF

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-D 1	12	TOP
D-G 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
B-F 1	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TMW-w	MT20	2.0	4.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMW-w	MT20	2.0	4.0		
F	TMB1-I	MT20	3.0	4.0		
H	BMWW1-t	MT20	4.0	4.0		
I	BMWW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REORD BRG
	VERT	HORZ	DOWN	HORZ		
B	340	0	340	0	15-9-6	15-9-6
F	340	0	340	0	15-9-6	15-9-6
H	610	0	610	0	15-9-6	15-9-6
I	610	0	610	0	15-9-6	15-9-6

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS			
	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	239	166 / 0	0 / 0	0 / 0	73 / 0	0 / 0
F	239	166 / 0	0 / 0	0 / 0	73 / 0	0 / 0
H	432	283 / 0	0 / 0	0 / 0	149 / 0	0 / 0
I	432	283 / 0	0 / 0	0 / 0	149 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, H, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD		MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED	
		FROM	TO			CS (LC)	CS (LC)
A-B	0 / 17	-91.8	-91.8	D-H	-197 / 0	0.02 (1)	
B-K	-199 / 0	-91.8	-91.8	H-E	-396 / 0	0.02 (1)	
K-C	-144 / 0	-91.8	-91.8	I-D	-197 / 0	0.02 (1)	
C-D	0 / 54	-91.8	-91.8	C-I	-396 / 0	0.02 (1)	
D-E	0 / 54	-91.8	-91.8	J-K	-68 / 57	0.00 (1)	
E-M	-144 / 0	-91.8	-91.8	L-M	-68 / 57	0.00 (1)	
M-F	-199 / 0	-91.8	-91.8				
F-G	0 / 17	-91.8	-91.8				
B-J	0 / 149	-18.5	-18.5				
J-I	0 / 149	-18.5	-18.5				
I-H	0 / 40	-18.5	-18.5				
H-L	0 / 149	-18.5	-18.5				
L-F	0 / 149	-18.5	-18.5				

TOTAL WEIGHT = 3 X 51 = 154 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.08/1.00 (C-D:1), BC=0.04/1.00 (I-J:4), WB=0.02/1.00 (D-I:1), SSI=0.05/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.08 (F) (INPUT = 0.90)
JSI METAL = 0.03 (C) (INPUT = 1.00)

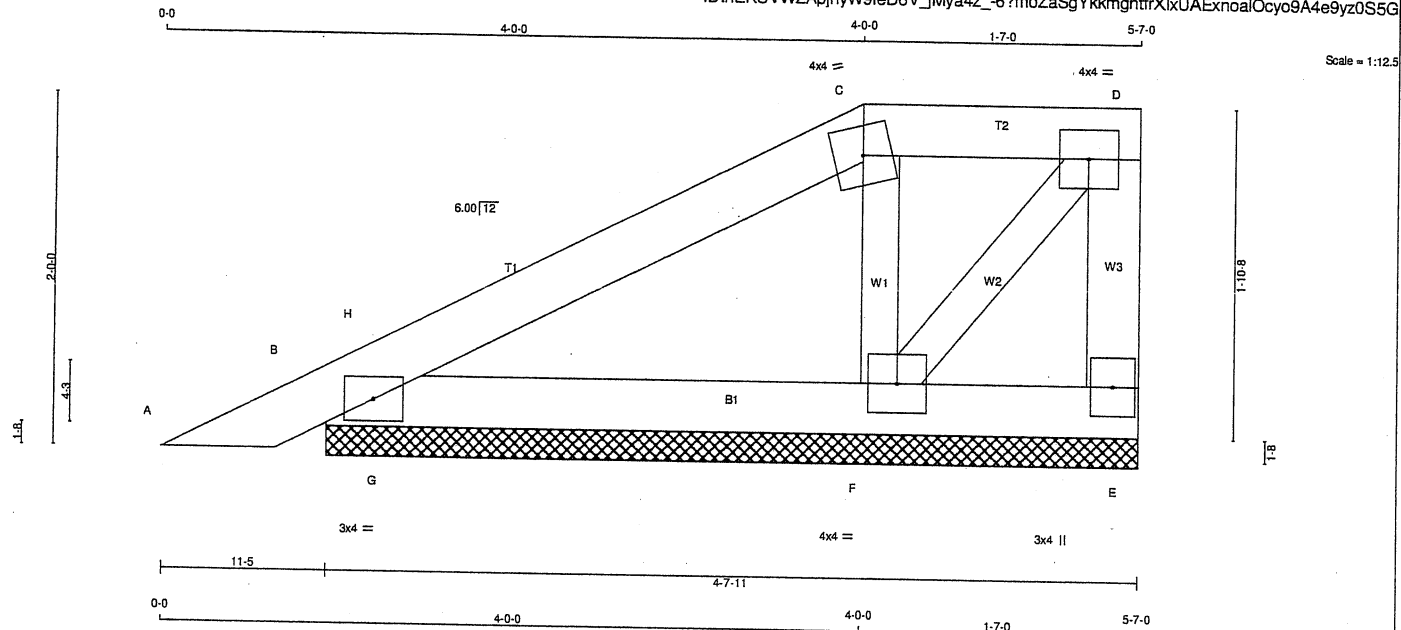


Structural component only
DWG# T-2121199

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
412865	PB101	4	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:hEKUVWZApjnyW9leD6V_jMya4z_-6?moZaSgYkmgntfrXlXUAExnoalOcyo9A4e9yz0S5G



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	
B - E	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0			
C	TTW-m	MT20	4.0	4.0			
D	TMVW-I	MT20	4.0	4.0			
E	BMV1-p	MT20	3.0	4.0			
F	BMVW1-I	MT20	4.0	4.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
E	76	0	76	0	4-7-11	4-7-11
B	252	0	252	0	4-7-11	4-7-11
F	264	0	264	0	4-7-11	4-7-11

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	53	39 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0
B	177	126 / 0	0 / 0	0 / 0	0 / 0	50 / 0	0 / 0
F	188	118 / 0	0 / 0	0 / 0	0 / 0	70 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED			MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
		VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH					
FR-TO		FROM	TO			FR-TO			
A-B	0 / 17	-91.8	-91.8	0.05 (1)	10.00	F-C	-176 / 0	0.03 (1)	
B-H	-18 / 0	-91.8	-91.8	0.02 (1)	6.25	F-D	0 / 24	0.01 (1)	
H-C	-33 / 0	-91.8	-91.8	0.10 (1)	6.25	G-H	-170 / 0	0.00 (1)	
C-D	-16 / 0	-91.8	-91.8	0.04 (1)	6.25				
E-D	-90 / 0	0.0	0.0	0.01 (1)	7.81				
B-G	0 / 28	-18.5	-18.5	0.09 (1)	10.00				
G-F	0 / 28	-18.5	-18.5	0.09 (1)	10.00				
F-E	0 / 0	-18.5	-18.5	0.06 (1)	10.00				

TOTAL WEIGHT = 4 X 16 = 64 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.10/1.00 (C-H:1), BC=0.09/1.00 (F-G:1), WB=0.03/1.00 (C-F:1), SSI=0.13/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
		788	1987

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

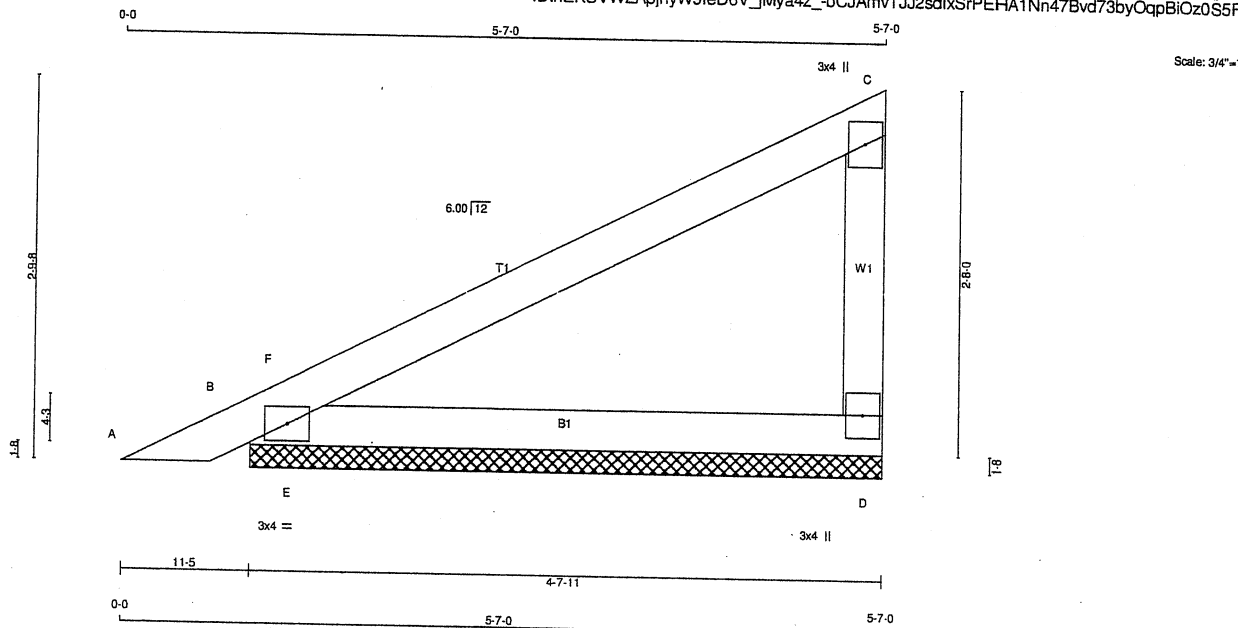
JSI GRIP= 0.20 (B) (INPUT= 0.90)
JSI METAL= 0.06 (C) (INPUT= 1.00)



Structural component only
DWG# T-2121200

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 8 X 15 = 118 lb

N. L. G. A. RULES			
CHORDS	SIZE		LUMBER
A - C	2x4	DRY	No.2
D - C	2x4	DRY	No.2
B - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TMV+p	MT20	3.0	4.0		
D	BMV1+p	MT20	3.0	4.0		

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
JT	256	0	256	0	0	4-7-11	4-7-11
D	336	0	336	0	0	4-7-11	4-7-11

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	181	119 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0
B	236	164 / 0	0 / 0	0 / 0	0 / 0	73 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D. B

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MAX. FACTORED		FACTORED		FACTORED		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 17	-91.8	-91.8	0.05 (1)			
B-F	-21 / 45	-91.8	-91.8	0.06 (1)	E-F	-310 / 7	0.00 (1)
F-C	-4 / 2	-91.8	-91.8	0.26 (1)			
D-C	-186 / 0	0.0	0.0	0.02 (1)			
				7.81			
B-E	0 / 0	-18.5	-18.5	0.18 (1)			
E-D	0 / 0	-18.5	-18.5	0.18 (1)			

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR
SMALL BUILDING REQUIREMENTS OF PART 9,
NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.26/1.00 (C-F:1), BC=0.18/1.00 (D-E:1),
WB=0.00/1.00 (E-F:1), SSI=0.25/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

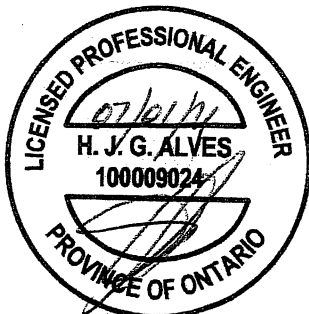
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)	(PLI)	(PSI)	(PLI)	(PSI)	(PLI)
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg

JSI GRIP= 0.26 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

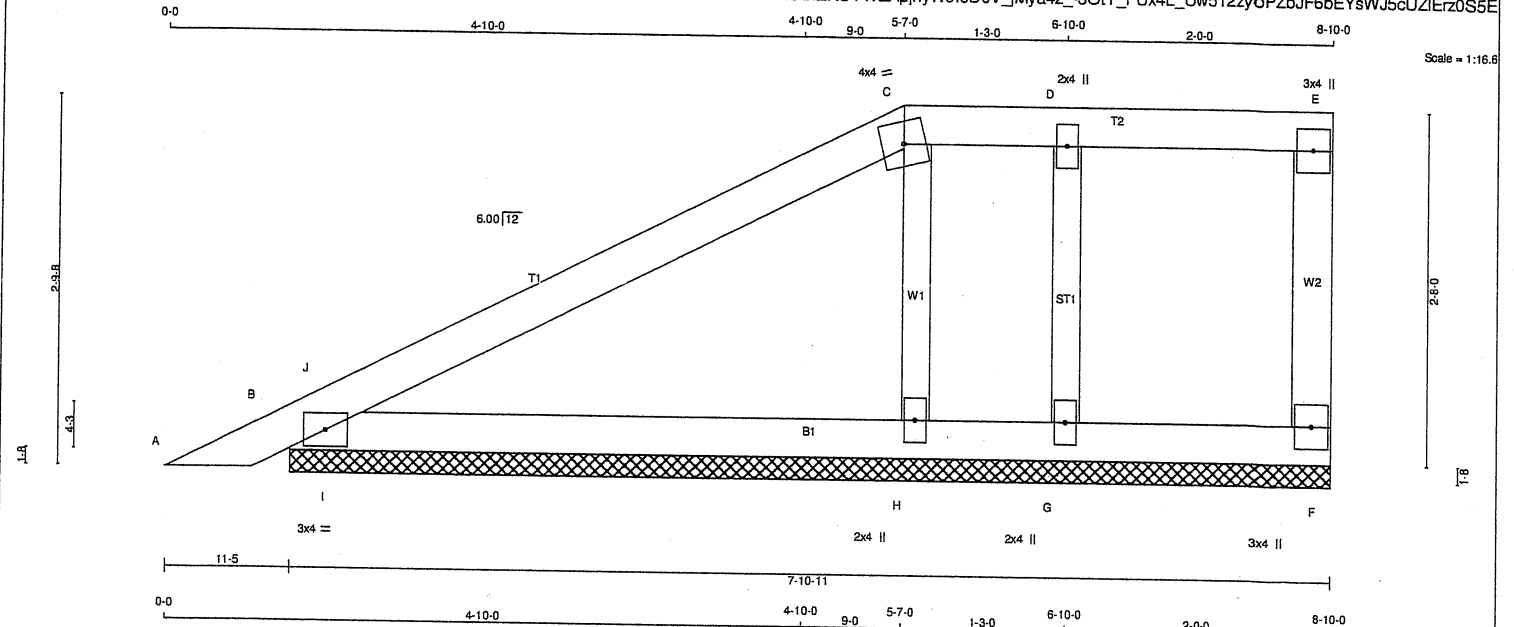


Structural component only
DWG# T-2121201

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
412865	PB103G	4	1	ROYAL PINE HOMES		

Tamarack Roof Truss, Burlington

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LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - I	2x4	DRY	No.2	SPF
I - J	2x4	DRY	No.2	SPF
J - K	2x4	DRY	No.2	SPF
K - L	2x4	DRY	No.2	SPF
L - M	2x4	DRY	No.2	SPF
M - N	2x4	DRY	No.2	SPF
N - O	2x4	DRY	No.2	SPF
O - P	2x4	DRY	No.2	SPF
P - Q	2x4	DRY	No.2	SPF
Q - R	2x4	DRY	No.2	SPF
R - S	2x4	DRY	No.2	SPF
S - T	2x4	DRY	No.2	SPF
T - U	2x4	DRY	No.2	SPF
U - V	2x4	DRY	No.2	SPF
V - W	2x4	DRY	No.2	SPF
W - X	2x4	DRY	No.2	SPF
X - Y	2x4	DRY	No.2	SPF
Y - Z	2x4	DRY	No.2	SPF
Z - A	2x4	DRY	No.2	SPF

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TM81-H	MT20	3.0	4.0
C	TTW-m	MT20	4.0	4.0
D	TMW-w	MT20	2.0	4.0
E	TMV-p	MT20	3.0	4.0
F	BMV1-p	MT20	3.0	4.0
G	BMW1-w	MT20	2.0	4.0
H	BMW1-w	MT20	2.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)	
FR-TO		FROM TO	CSI (LC)	FR-TO			
A-B	0 / 17	-91.8	-91.8 0.05 (1)	G-D	-193 / 0		0.03 (1)
B-J	-9 / 65	-91.8	-91.8 0.09 (1)	H-C	-206 / 0		0.03 (1)
J-C	-17 / 0	-91.8	-91.8 0.24 (1)	I-J	-374 / 0		0.00 (1)
C-D	0 / 0	-91.8	-91.8 0.05 (1)				
D-E	0 / 0	-91.8	-91.8 0.05 (1)				
E-F	-75 / 0	0.0	0.0 0.01 (1)				
B-I	0 / 9	-18.5	-18.5 0.20 (1)				
I-H	0 / 9	-18.5	-18.5 0.20 (1)				
H-G	0 / 0	-18.5	-18.5 0.14 (1)				
G-F	0 / 0	-18.5	-18.5 0.02 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.24/1.00 (C-J:1), BC=0.20/1.00 (B-I:1), WB=0.03/1.00 (C-H:1), SS=0.28/1.00 (B-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

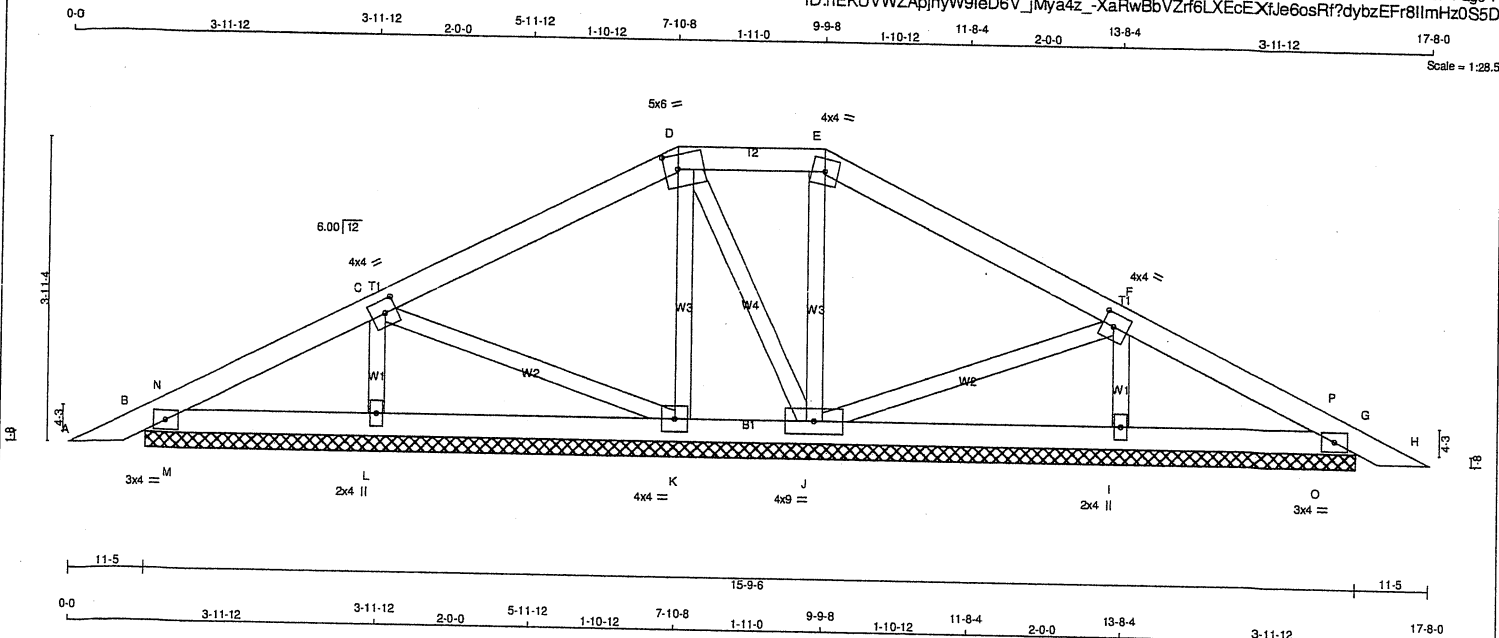
JSI GRIP= 0.26 (B) (INPUT = 0.90)
JSI METAL= 0.11 (C) (INPUT = 1.00)



Structural component only
DWG# T-2121202

JOB NAME 412865	TRUSS NAME PB104	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - H	2x4	DRY	No.2
B - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	
C	TTWW-t	MT20	4.0	4.0	2.00 1.75
D	TTWW-m	MT20	5.0	6.0	2.25 2.00
E	TTWW-m	MT20	4.0	4.0	
F	TTWW-t	MT20	4.0	4.0	2.00 1.75
G	TMB1-I	MT20	3.0	4.0	
I	BMW1-w	MT20	2.0	4.0	
J	BMW1-w	MT20	4.0	9.0	
K	BMW1-w	MT20	4.0	4.0	
L	BMW1-w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQD	
GROSS REACTION	DOWN	GROSS REACTION	UP	BRG	IN-SX	BRG	IN-SX
JT	VERT	HORZ	DOWN	UP	IN-SX	BRG	IN-SX
B	222	0	222	0	15-9-6	15-9-6	15-9-6
G	213	0	213	0	15-9-6	15-9-6	15-9-6
K	205	0	205	0	15-9-6	15-9-6	15-9-6
J	385	0	385	0	15-9-6	15-9-6	15-9-6
L	447	0	447	0	15-9-6	15-9-6	15-9-6
I	428	0	428	0	15-9-6	15-9-6	15-9-6

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	SOIL
B	155	112 / 0	0 / 0	0 / 0	0 / 0	0 / 0
G	149	108 / 0	0 / 0	0 / 0	0 / 0	0 / 0
K	147	86 / 0	0 / 0	0 / 0	0 / 0	0 / 0
J	271	187 / 0	0 / 0	0 / 0	0 / 0	0 / 0
L	316	208 / 0	0 / 0	0 / 0	0 / 0	0 / 0
I	303	197 / 0	0 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, G, K, J, L, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		MAX. FACTORED	
MEMB.	MAX. FORCE (LBS)	VERT. LOAD	CS1 (LC)	MEMB.	MAX. FORCE (LBS)	MAX. CS1 (LC)	MAX. CS1 (LC)
FR-TO		FROM	TO	FR-TO			
A-B	0 / 17	-91.8	-91.8 0.05 (1)	10.00	D-J	-86 / 0	0.02 (1)
B-N	-80 / 0	-91.8	-91.8 0.01 (1)	6.25	C-K	-37 / 0	0.01 (1)
N-C	-39 / 0	-91.8	-91.8 0.19 (1)	6.25	J-F	-59 / 0	0.02 (1)
C-D	-42 / 0	-91.8	-91.8 0.19 (1)	6.25	K-D	-143 / 0	0.03 (1)
D-E	0 / 21	-91.8	-91.8 0.06 (1)	10.00	J-E	-238 / 0	0.05 (1)
E-F	0 / 0	-91.8	-91.8 0.19 (1)	10.00	L-C	-361 / 0	0.05 (1)
F-P	-21 / 0	-91.8	-91.8 0.19 (1)	6.25	I-F	-343 / 0	0.05 (1)
P-G	-63 / 0	-91.8	-91.8 0.02 (1)	6.25	M-N	-50 / 4	0.00 (1)
G-H	0 / 17	-91.8	-91.8 0.05 (1)	10.00	O-P	-50 / 4	0.00 (1)
B-M	0 / 57	-18.5	-18.5 0.04 (1)	10.00			
M-L	0 / 57	-18.5	-18.5 0.06 (4)	10.00			
L-K	0 / 57	-18.5	-18.5 0.06 (4)	10.00			
K-J	0 / 19	-18.5	-18.5 0.04 (4)	10.00			
J-I	0 / 42	-18.5	-18.5 0.06 (4)	10.00			
I-O	0 / 42	-18.5	-18.5 0.06 (4)	10.00			
O-G	0 / 42	-18.5	-18.5 0.04 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CS1: TC=0.19/1.00 (F-P:1), BC=0.06/1.00 (K-L:4), WB=0.05/1.00 (E-J:1), SS=0.15/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

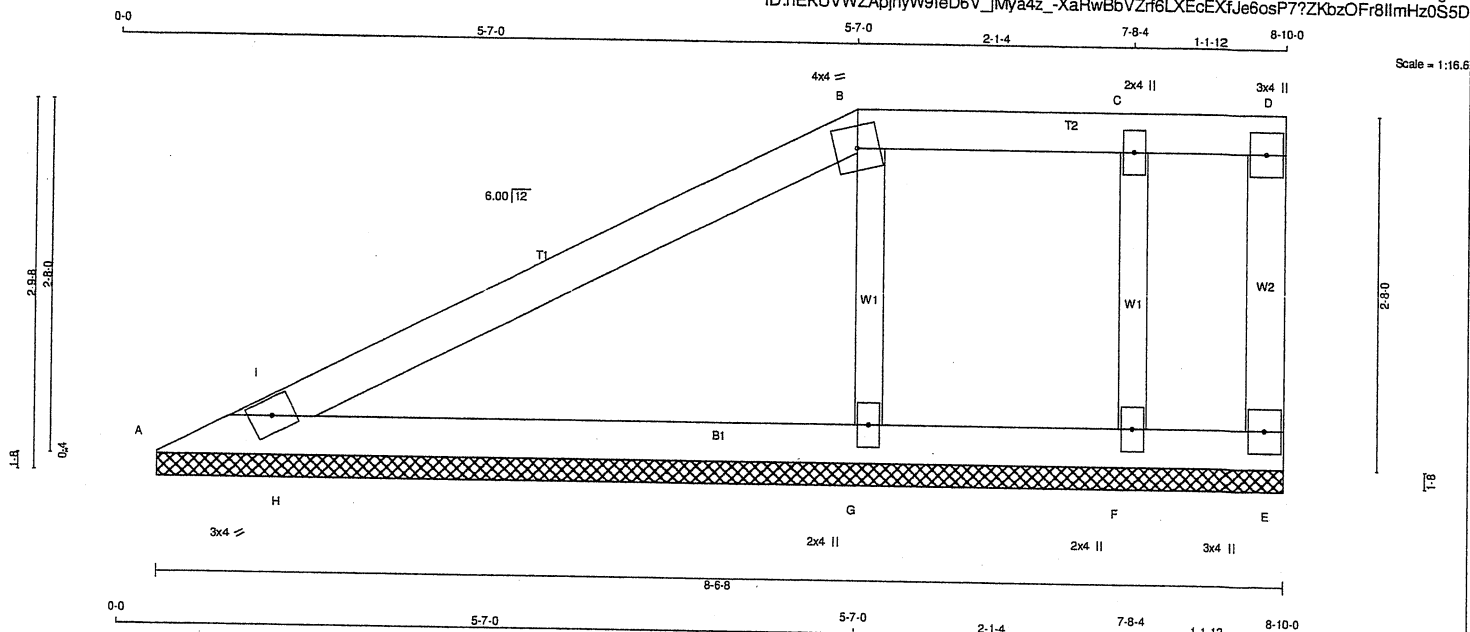
JSI GRIP = 0.30 (C) (INPUT = 0.90)
JSI METAL = 0.09 (E) (INPUT = 1.00)



Structural component only
DWG# T-2121203

JOB NAME 412865	TRUSS NAME PB105G	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - B	2x4	DRY	No.2
B - D	2x4	DRY	No.2
E - D	2x4	DRY	No.2
A - E	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TBM1-h	MT20	3.0	4.0	
B	TTW-m	MT20	4.0	4.0	
C	TMW+w	MT20	2.0	4.0	
D	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	
F	BMW1+w	MT20	2.0	4.0	
G	BMW1+w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS					
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG	
A	272	272	0	0	8-6-8
E	65	65	0	0	8-6-8
G	502	502	0	0	8-6-8
F	103	103	0	0	8-6-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
A	192	128 / 0	0 / 0	0 / 0	0 / 0	64 / 0	0 / 0
E	46	27 / 0	0 / 0	0 / 0	0 / 0	19 / 0	0 / 0
G	357	224 / 0	0 / 0	0 / 0	0 / 0	132 / 0	0 / 0
F	71	57 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, E, G, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-I	-14 / 0	-91.8 -91.8	0.05 (4)	G-B	-270 / 0	0.04 (1)	
I-B	-13 / 0	-91.8 -91.8	0.29 (1)	F-C	-199 / 0	0.03 (1)	
B-C	0 / 0	-91.8 -91.8	0.05 (1)	H-I	-238 / 0	0.00 (1)	
C-D	0 / 0	-91.8 -91.8	0.05 (1)				
E-D	-20 / 0	0.0 0.0	0.00 (1)				
A-H	-17 / 0	-18.5 -18.5	0.29 (1)				
H-G	0 / 12	-18.5 -18.5	0.29 (1)				
G-F	0 / 0	-18.5 -18.5	0.20 (1)				
F-E	0 / 0	-18.5 -18.5	0.05 (1)				

TOTAL WEIGHT = 2 X 25 = 51 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.29/1.00 (B-I:1), BC=0.29/1.00 (G-H:1), WB=0.04/1.00 (B-G:1), SSI=0.17/1.00 (A-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

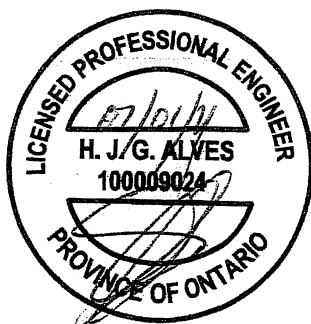
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

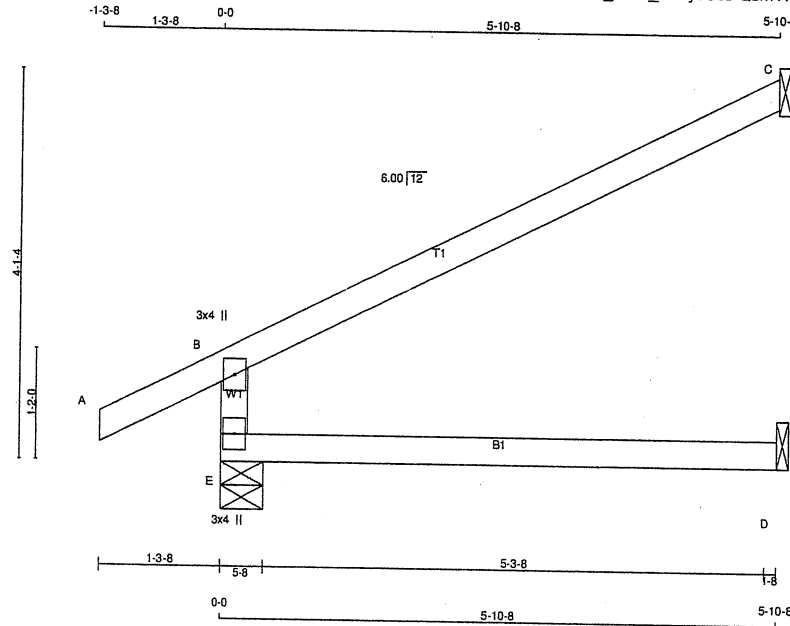
JSI GRIP= 0.22 (B) (INPUT = 0.90)
JSI METAL= 0.12 (B) (INPUT = 1.00)



Structural component only
DWG# T-2121204

JOB NAME 412868	TRUSS NAME J01	QUANTITY 20	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Thu Jul 1 10:22:22 2021 Page 1
ID:U6yi?rbeFFwxf_UFm_koDybSsJ-QsmYHwRSR5VHjZhxZioTureZH4eowBkrZJZVZdz0TvV



LUMBER			
N.L.G.A. RULES	SIZE	DRY	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B TMV+p	MT20	3.0	4.0	
E BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	525	0	525	0	0	5-8	5-8
C	202	0	202	0	0	1-8	1-8
D	45	0	50	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS		PERM. LIVE		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	COMBINED	SNOW	LIVE	PERM. LIVE
E	369	257 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0	369	257	0	0
C	139	113 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0	139	113	0	0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0	36	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX	MEMB.	FORCE (LBS)	MAX	LC1
FR-TO		FROM	TO	FR-TO		FROM	TO
E-B	-461 / 0	0.0	0.0	0.13 (4)	7.81		
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00		
B-C	-30 / 0	-91.8	-91.8	0.54 (1)	6.25		
E-D	0 / 0	-18.5	-18.5	0.13 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.03")

CSI: TO=0.54/1.00 (B-C:1), BC=0.13/1.00 (D-E:4),
WB=0.00/1.00 (n/a:0), SSI=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

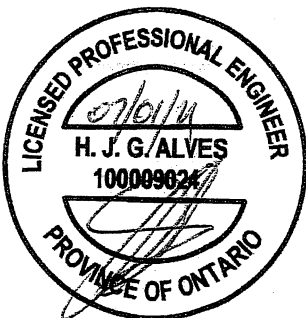
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.18 (E) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)



Structural component only
DWG# T-2121142

amarack Roof Truss, Burlington

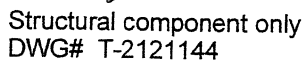
Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 1 10:22:24 2021 Page 1

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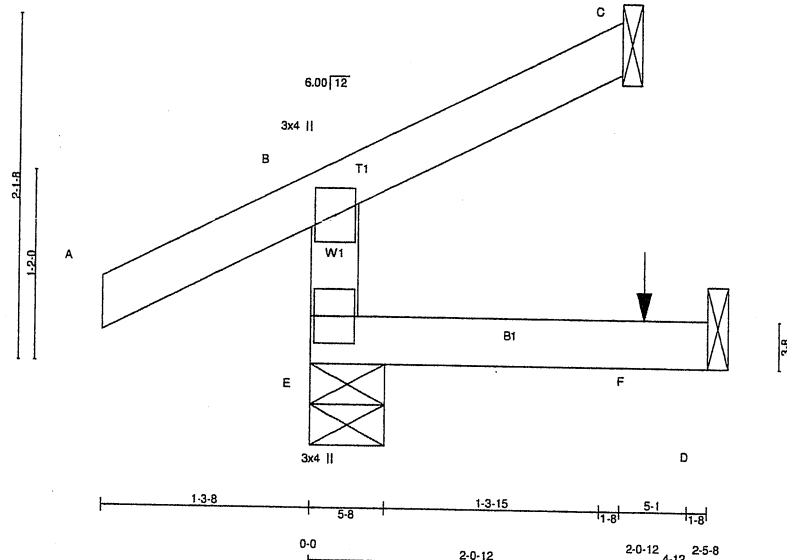
JSI GRIP= 0.20 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)



JOB NAME 412865	TRUSS NAME J11	QUANTITY 5	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Thu Jul 1 12:26:13 2021 Page 1	

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Scale = 1:13.5



LUMBER					
N. L. G. A. RULES					
CHORDS		SIZE	LUMBER		DESCR.
E - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
E	260	0	260	0	5-8	5-8
C	66	0	66	0	1-8	1-8
D	23	0	26	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS				
		COMBINED	SNOW	LIVE	PERM. LIVE	WIND
E	182	130 / 0	0 / 0	0 / 0	0 / 0	52 / 0
C	46	37 / 0	0 / 0	0 / 0	0 / 0	9 / 0
D	18	0 / 0	0 / 0	0 / 0	0 / 0	18 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS					WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO	CSI (LC)	UNBRAC LENGTH	FR-TO		MAX CSI (LC)
E-B	-234 / 0	0.0	0.0	0.02 (4)	7.81		
A-B	0 / 28	-91.8	-91.8	0.13 (5)	10.00		
B-C	-10 / 0	-91.8	-91.8	0.06 (1)	10.00		
E-F	0 / 0	-18.5	-18.5	0.03 (4)	10.00		
F-D	0 / 0	-18.5	-18.5	0.03 (4)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	2-0-12	-3	-3	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.
CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 5 X 8 = 39 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.13/1.00 (A-B:5), BC=0.03/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.09/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

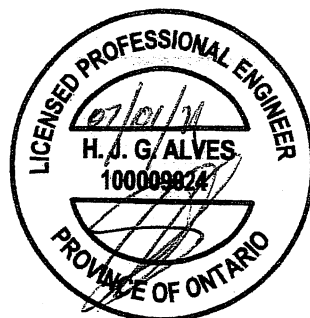
NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

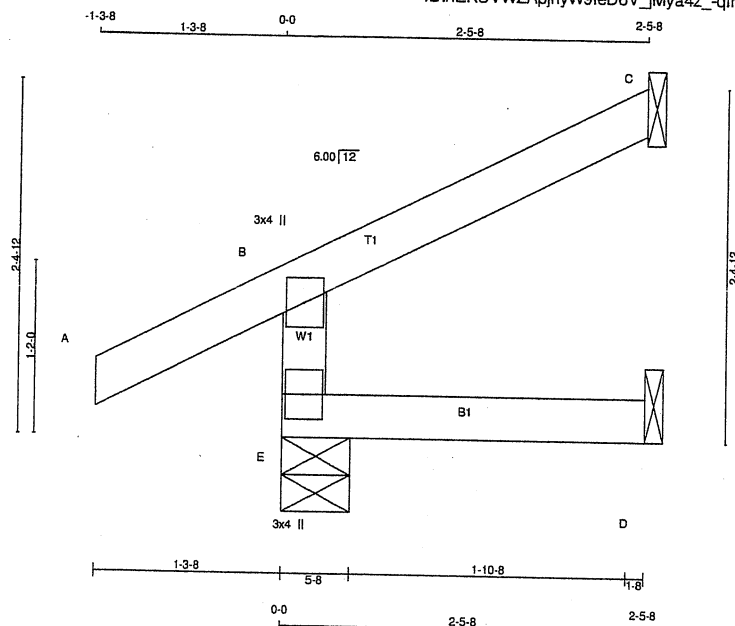
JSI GRIP = 0.09 (E) (INPUT = 0.90)
JSI METAL = 0.06 (B) (INPUT = 1.00)



Structural component only
DWG# T-2121191

JOB NAME 412865	TRUSS NAME J12	QUANTITY 7	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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ID:hEKUVWZApinyW9leD6V_jMya4z_-qfr85ANHCamLirJxZ7lihSkBzCBFScmYatmPsz0SSN



TOTAL WEIGHT = 7 X 9 = 60 lb [M]

LUMBER			
N. L. G. A. RULES	SIZE	DRY	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
E	291	0	291	0	5-8	5-8
C	85	0	85	0	1-8	1-8
D	20	0	22	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
E	203	148 / 0	0 / 0	0 / 0	0 / 0	55 / 0	0 / 0
C	58	47 / 0	0 / 0	0 / 0	0 / 0	11 / 0	0 / 0
D	16	0 / 0	0 / 0	0 / 0	0 / 0	16 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS		W E B S	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO		
E-B	-265 / 0	0.0 0.0 0.02 (4)	7.81	
A-B	0 / 28	-91.8 -91.8 0.12 (5)	10.00	
B-C	-12 / 0	-91.8 -91.8 0.09 (1)	6.25	
E-D	0 / 0	-18.5 -18.5 0.03 (4)	10.00	

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.
(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.12/1.00 (A-B:5), BC=0.03/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.10/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

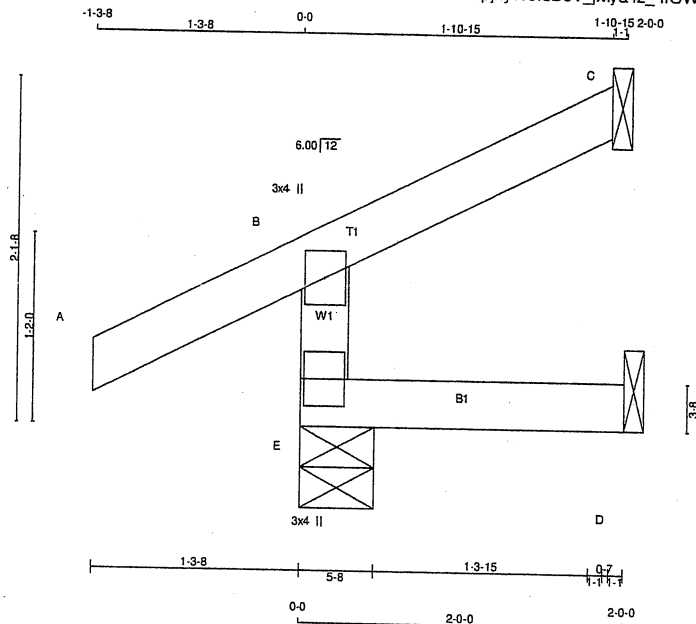
JSI GRIP= 0.11 (E) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)



Structural component only
DWG# T-2121192

JOB NAME 412865	TRUSS NAME J13	QUANTITY 10	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID:hEKUVWZApinyW9leD6V_jMya4z_-lROWIWOWzuzdysQVVGeXEv_v0NYX_vswndEdKylz0SSM



Scale = 1:13.5

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	3.0	4.0
E	BMV1+p	MT20	3.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	254	0	254	0	5-8	5-8
C	66	0	66	0	1-8	1-8
D	16	0	18	0	1-8	1-8

SEE MITTEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	177	130 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	46	37 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0
D	13	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS		FACTORED		WEBS	
	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	UNBRAC LENGTH FR-TO	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
E-B	-234 / 0	0.0	0.0	0.01 (4)	7.81	
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	
B-C	-10 / 0	-91.8	-91.8	0.06 (1)	10.00	
E-D	0 / 0	-18.5	-18.5	0.02 (4)	10.00	

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 10 X 7 = 73 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
DL = 6.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.4 PSF	
TOTAL LOAD = 39.0 PSF	

SPACING = 24.0 IN C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.00")

CSI: TC=0.12/1.00 (A-B:1), BC=0.02/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.09 (E) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

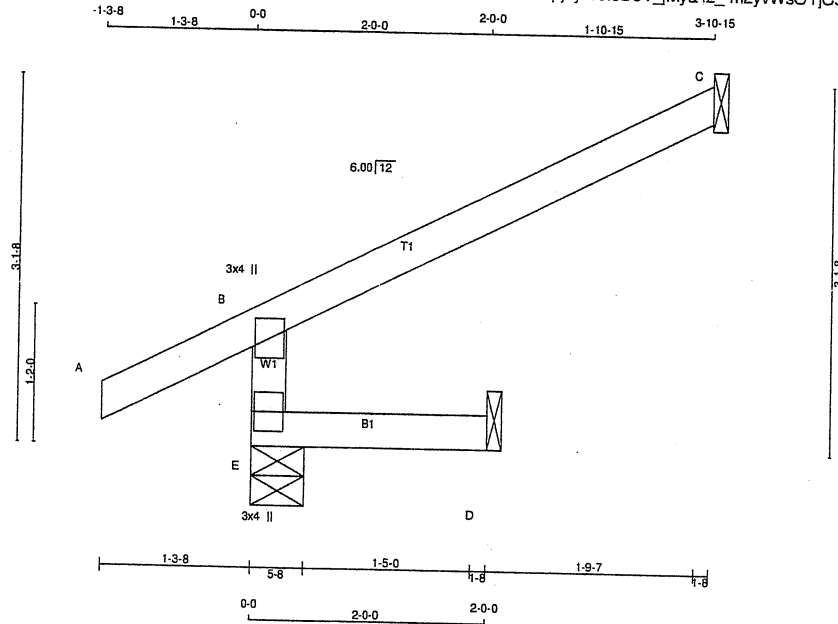


Structural component only
DWG# T-2121193

JOB NAME 412865	TRUSS NAME J14	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1:18.6

LUMBER			
N. L. G. A. RULES	SIZE	DRY	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	3.0	4.0
E	BMV1+p	MT20	3.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	369	0	369	0	5-8	5-8
C	135	0	135	0	1-8	1-8
D	16	0	16	0	1-8	1-8

SEE MITTEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	WIND			
E	256	194 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0
C	93	75 / 0	0 / 0	0 / 0	0 / 0	18 / 0	0 / 0
D	13	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS		FACTORED		WEBS	
	MAX. FACTORED FORCE (LBS)	VERT. LOAD (LBS)	LC1 MAX (PLF)	LC1 MAX (LC)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LC)
FR-TO			FROM	TO		LENGTH FR-TO
E-B	-349 / 0	0.0	0.0	0.01 (4)	7.81	
A-B	0 / 28	-91.8	-91.8	0.13 (5)	10.00	
B-C	-20 / 0	-91.8	-91.8	0.24 (1)	6.25	
E-D	0 / 0	-18.5	-18.5	0.02 (4)	10.00	

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 2 X 10 = 20 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.00")

CSI: TC=0.24/1.00 (B-C:1), BC=0.02/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

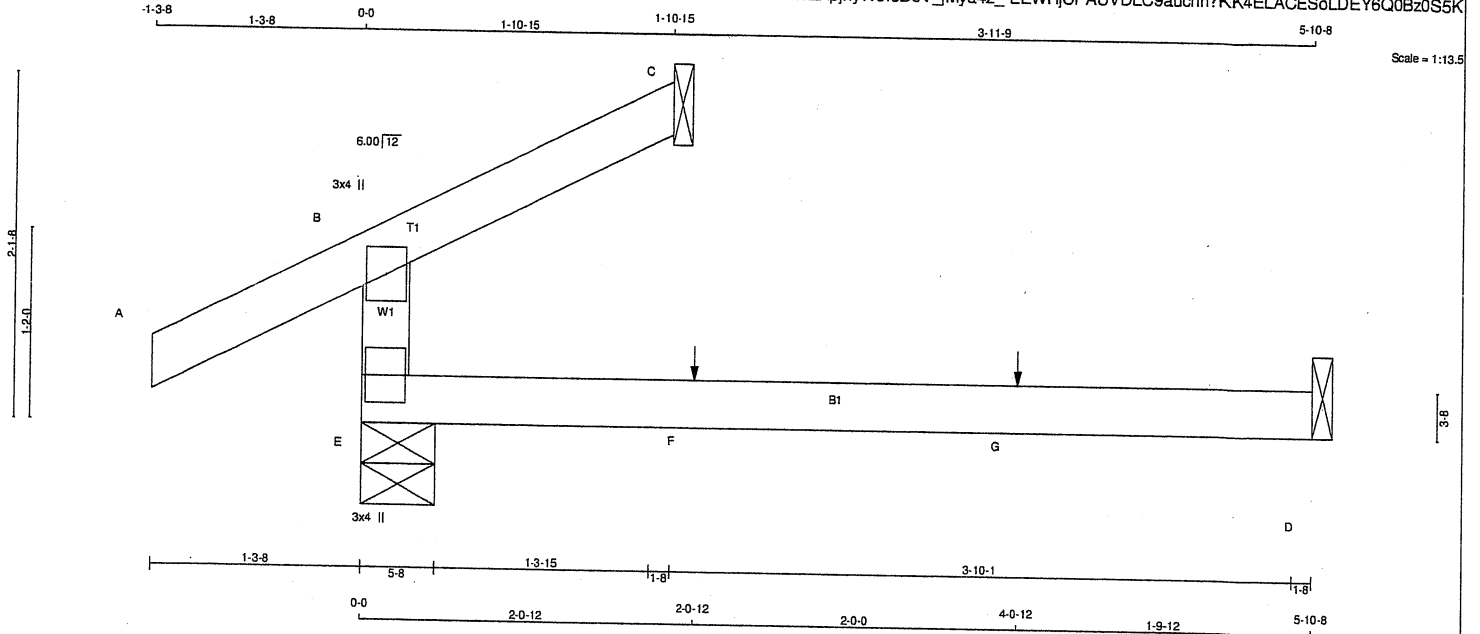
JSI GRIP= 0.14 (E) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)



Structural component only
DWG# T-2121194

JOB NAME 412865	TRUSS NAME J15	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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<u>LUMBER</u>			
N. L. G. A. RULES			
CHORDS	SIZE		LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0			
E	BMV1+p	MT20	3.0	4.0			

DESCR.
SPF
SPF
SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	297	0	297	0	5-8	5-8
C	66	0	66	0	1-8	1-8
D	45	0	50	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED		SNOW		LIVE		PERM. LIVE		WIND		DEAD		SOIL	
	212	130/0	0/0	0/0	0/0	0/0	0/0	0/0	0/0	0/0	82/0	0/0	0/0	0/0
C	46	37/0	0/0	0/0	0/0	0/0	0/0	0/0	0/0	0/0	9/0	0/0	0/0	0/0
D	36	0/0	0/0	0/0	0/0	0/0	0/0	0/0	0/0	0/0	36/0	0/0	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		WEBS		FACTORED	
	FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX	MAX.	MEMB.	FORCE (LBS)	MAX
FR-TO		FROM TO				FR-TO		
E-B	-234/0	0.0	0.0	0.13 (4)	7.81			
A-B	0/28	-91.8	-91.8	0.12 (1)	10.00			
B-C	-10/0	-91.8	-91.8	0.06 (1)	10.00			
E-F	0/0	-18.5	-18.5	0.13 (4)	10.00			
F-G	0/0	-18.5	-18.5	0.13 (4)	10.00			
G-D	0/0	-18.5	-18.5	0.13 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	2-0-12	1	1	---	FRONT	VERT	TOTAL	---	C1
G	4-0-12	1	1	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 12 = 23 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.13/1.00 (B-E:4), BC=0.13/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.09/1.00 (A-B:1)

DOL LUMBER=0.99 NAIL=0.99 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.09 (E) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)



Structural component only
DWG# T-2121195

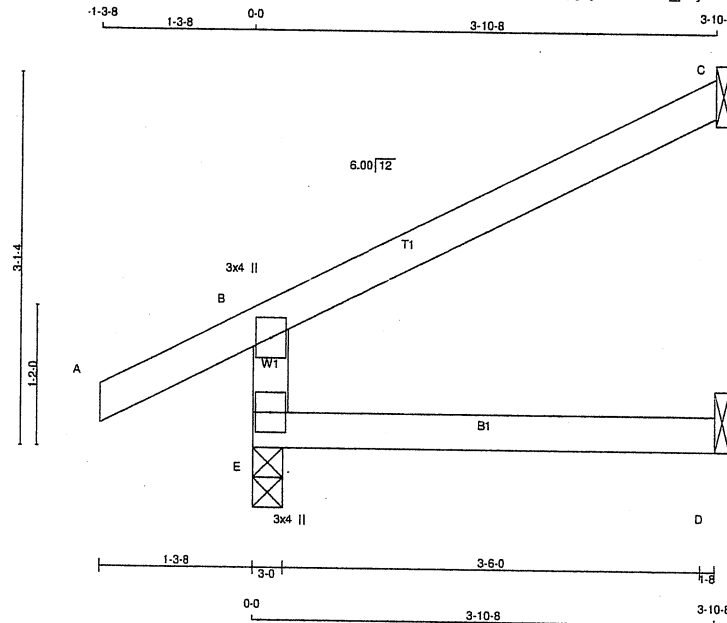
Version 8.420 5 Jan 21 2021 MiTek Industries, Inc. Thu Jul 1 12:26:18 2021 Page 1
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Structural component only
DWG# T-2121196

JOB NAME 412865	TRUSS NAME J18	QUANTITY 5	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1:18.3

LUMBER				DESCR.
N. L. G. A. RULES	SIZE	LUMBER		
CHORDS				SPF
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	3.0	4.0
E	BMV1+p	MT20	3.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
E	388	0	388	0	0	3-0	3-0	
C	133	0	133	0	0	1-8	1-8	
D	30	0	34	0	0	1-8	1-8	

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS		PERM. LIVE	WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE					
E	272	193 / 0	0 / 0	0 / 0	0 / 0	0 / 0	78 / 0	0 / 0
C	92	74 / 0	0 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	24	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	24 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS		FACTORED		WEBS		FACTORED	
	MAX. FORCE (LBS)	VERT. LOAD (LBS)	LC1 MAX (PLF)	LC1 MAX (LC)	MAX. UNBRAC LENGTH FR-TO	MEMB. FORCE (LBS)	MAX. FORCE (LBS)	MAX. CSI (LC)
FR-TO			FROM	TO				
E-B	-347 / 0	0.0	0.0	0.05 (4)	7.81			
A-B	0 / 28	-91.8	-91.8	0.14 (5)	10.00			
B-C	-20 / 0	-91.8	-91.8	0.23 (1)	6.25			
E-D	0 / 0	-18.5	-18.5	0.06 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 5 X 12 = 60 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.01")

CSI: TC=0.23/1.00 (B-C:1), BC=0.06/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (E) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)



Structural component only
DWG# T-2121198



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1900213

Feb 09, 2018

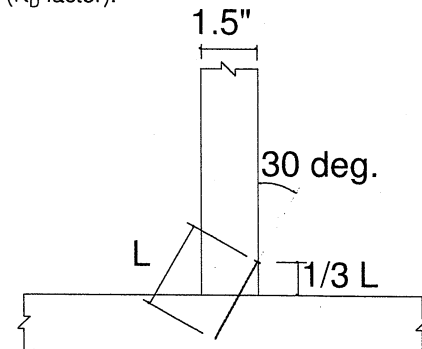
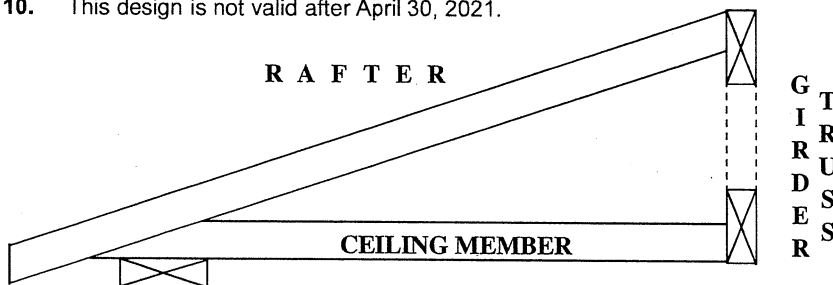
BEARING ANCHORAGE BY TOE-NAILS FOR LATERAL CAPACITY

B97791H1

NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL LATERAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	132	147
	3.25	0.144	132	147
	3.50	0.160	159	177
COMMON SPIRAL	3.00	0.122	97	108
	3.25	0.122	97	108
	3.50	0.152	145	162

NOTES:

1. Rafter and ceiling members may be anchored to top and bottom chords of girder truss by toe-nailing rafter and ceiling members to girder chords provided the reaction does not exceed the lateral capacities in the table. Hangers (specified by others) are required for reactions higher than the maximum toe-nail capacity. Reactions are based on factored loads.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.4.1.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in tables below.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See next page for nailing on bearing plate).
7. For loads due to **wind** the nail lateral capacity in this table may be multiplied by 1.15 (K_D factor).
8. Lumber must be dry (< 19% moisture content) at the time of nail installation.
9. Nail values in this table comply with CSA O86-14, section 12.9.4
10. This design is not valid after April 30, 2021.



TOE-NAIL INSTALLATION

Nail type	Common wire	Common spiral	Common wire	Common spiral
Nail dia. (in)	0.160	0.152	0.144	0.122
	(3.5" nail)		(3" and 3.25" nail)	
LUMBER SIZE	MAXIMUM NUMBER OF TOE-NAILS			
2X4 SPF	2	2	3	3
2X4 D. Fir	2	2	2	2
2X6 SPF	4	4	4	5
2X6 D. Fir	3	3	3	4

MiTek® MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

April 2, 2020

PEO
Certificate No. 10889485



BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING

B97791H2

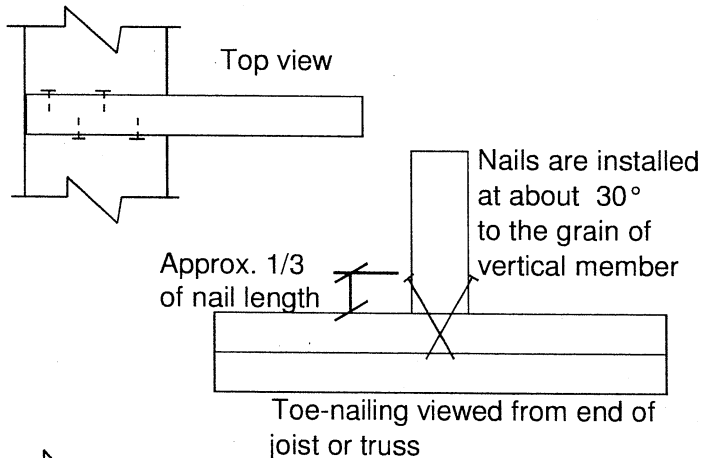
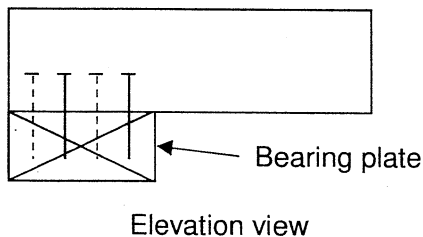
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	30	42
	3.25	0.144	32	45
	3.50	0.160	38	52
COMMON SPIRAL	3.00	0.122	26	36
	3.25	0.122	28	40
	3.50	0.152	36	50

Note: If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

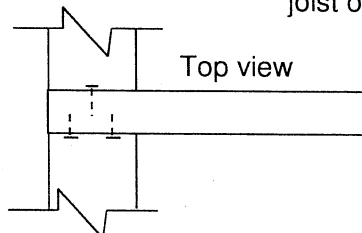
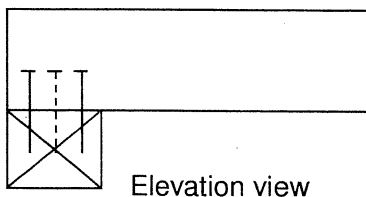
NOTES:

1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to **wind** or **earthquake** load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.5.2.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37579H1).
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-14, section 12.9.5
9. This design is not valid after April 30, 2021.

Toe-nailing on 2x6 Bearing Plate



Toe-nailing on 2x4 Bearing Plate



MiTek®

MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

April 2, 2020

PEO
Certificate No. 10889485



HUS/LJS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

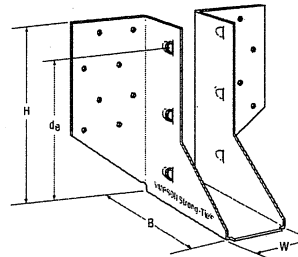
- Factored resistances are in accordance with CSA O86 -14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

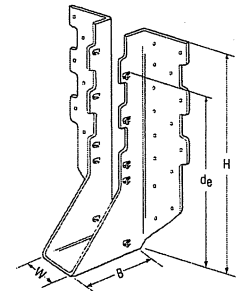
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

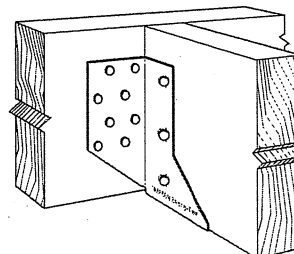
- See current catalogue for options



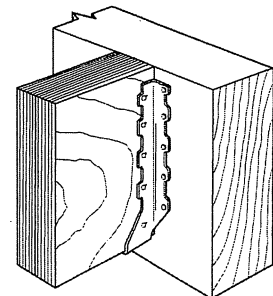
LJS26DS



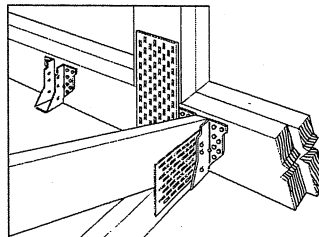
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



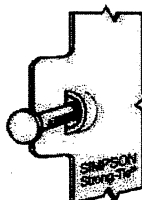
Typical HUS
Installation



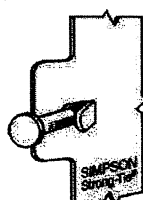
Typical HUS Installation
(Truss Designer to provide fastener
quantity for connecting multiple
members together)

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d ₈ ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K ₀ =1.15) lb.	Normal (K ₀ =1.00) lb.	Uplift (K ₀ =1.15) lb.	Normal (K ₀ =1.00) lb.
LJS26DS	18	1⅞	5	3½	4⅞	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1⅞	5⅞	3	3⅞	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1⅞	7⅞	3	6⅞	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1⅞	9⅞	3	7⅞	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1⅞	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

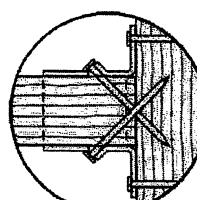
1. d₈ is the distance from the seat of the hanger to the highest joist nail.



Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).
U.S. Patent
5,603,580



Double
Shear
Nailing
Side View.
Do not
bend tab
back.



Double
Shear
Nailing
Top View.



(800) 999-5099
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HGUS – Double Shear Joist Hangers

SIMPSON

Strong-Tie

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

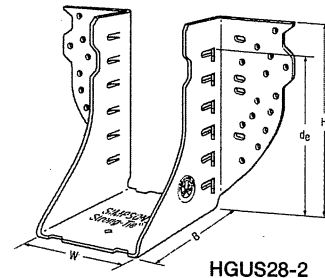
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%.
No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

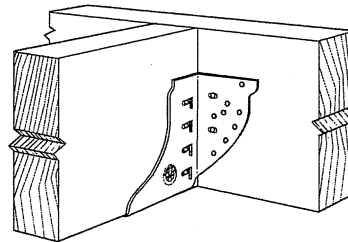
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

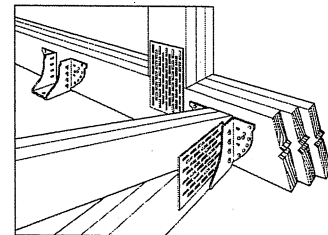
- See current catalogue for options



HGUS28-2



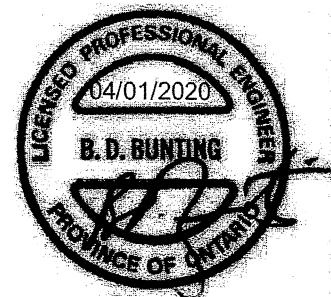
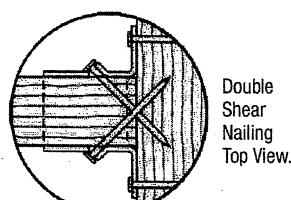
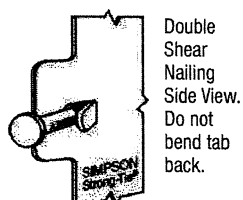
Typical HGUS Installation



Typical HGUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d ₀ ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K ₀ =1.15)	Normal (K ₀ =1.00)	Uplift (K ₀ =1.15)	Normal (K ₀ =1.00)
HGUS26	12	1⅞	5⅞	5	4⅝ ₃₂	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3⅞	5⅞	4	4⅝	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4⅞	5⅞	4	4⅝	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6⅞	5⅞	4	4⅝	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1⅞	7⅞	5	6⅝	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3⅞	7⅞	4	6⅝	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4⅞	7⅞	4	6⅝	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6⅞	7⅞	4	6⅝	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210	12	1⅞	9⅞	5	7⅞	(46) 16d	(16) 16d	3535	11070	2510	8090
HGUS210-2	12	3⅞	9⅞	4	8⅝	(46) 16d	(16) 16d	6840	14015	4855	10270
HGUS210-3	12	4⅞	9⅞	4	8⅝	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6⅞	9⅞	4	8⅝	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6⅞	10⅞	4	10⅝	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6⅞	12⅞	4	11⅝	(66) 16d	(22) 16d	10130	16400	7195	11645

1. d₀ is the distance from the seat of the hanger to the highest joist nail.



(800) 999-5099
strongtie.com

H – Seismic and Hurricane Ties

SIMPSON

Strong-Tie

The H connector series provides wind and seismic ties for trusses and rafters.

Material: 18 gauge **Finish:** G90 galvanized

Design: • Factored resistances are in accordance with CSA O86-14

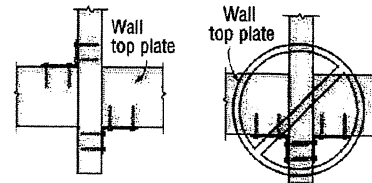
- Factored resistances have been increased 15%. No further increase is permitted.

Installation: • Use all specified fasteners

- Nails: 8d = 0.131" dia. x 2½" long common wire, 8d x 1½" = 0.131" x 1½" long, 10d x 1½" = 0.146" x 1½" long
- H1 can be installed with flanges facing outwards
- Hurricane ties do not replace solid blocking

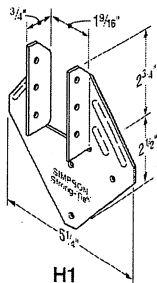
Factored resistances for more than one direction for a single connection cannot be added together. A factored load which can be divided into components in the directions given must be evaluated as follows: Factored Shear/Resisting Shear + Factored Tension/Resisting Tension ≤ 1.0.

Hurricane Tie Installations to Achieve Twice the Load (Top View)

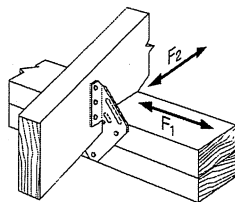


Install diagonally across from each other for minimum 2x truss.

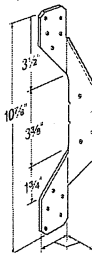
Nailing into both sides of a single ply 2x truss may cause the wood to split.



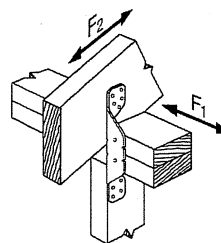
H1



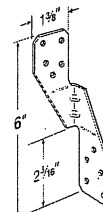
H1 Installation



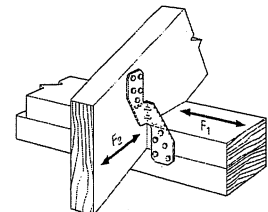
H2A



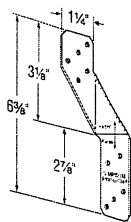
H2A Installation



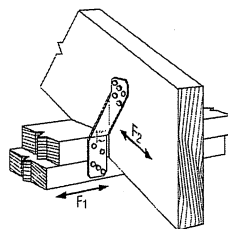
H2.5A



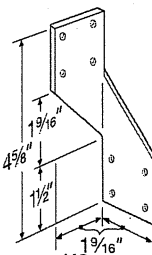
H2.5A Installation



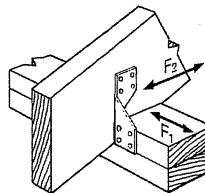
H2.5T



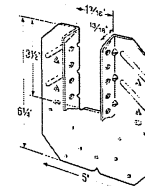
H2.5T Installation
(Nails into both top plates)



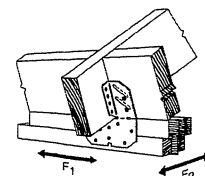
H3



H3 Installation



H10A



H10A Installation

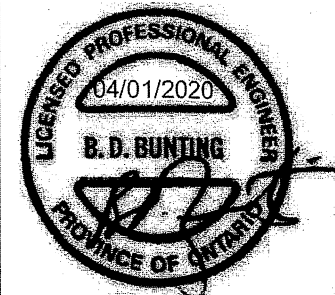
Model No.	Ga.	Fasteners			Factored Resistance (lb.)					
					D.Fir-L			S-P-F		
		To Rafter	To Plates	To Studs	Uplift	Normal		Uplift	Normal	
						F ₁	F ₂		F ₁	F ₂
						(K ₀ =1.15)			(K ₀ =1.15)	
H1	18	(6) 8d x 1½"	(4) 8d	—	740	685	300	680	485	215
H2A	18	(5) 8d x 1½"	(2) 8d x 1½"	(5) 8d x 1½"	830	220	75	590	155	55
H2.5A	18	(5) 8d	(5) 8d	—	805	160	160	755	160	160
H2.5T	18	(5) 8d	(5) 8d	—	835	175	240	740	160	210
H3	18	(4) 8d	(4) 8d	—	740	180	265	615	125	190
H10A	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1735	795	410	1505	565	290

1. Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.

2. Factored resistances are for one anchor. A minimum rafter thickness of 2½" must be used when framing anchors are installed on each side of the joist and on the same side of the plate.

3. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.

4. Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a Continuous Load Path, connections must be on same side of the wall.



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LUS – Double Shear Joist Hangers

SIMPSON

Strong-Tie

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

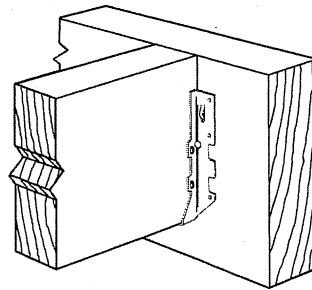
Finish: G90 galvanized

Design:

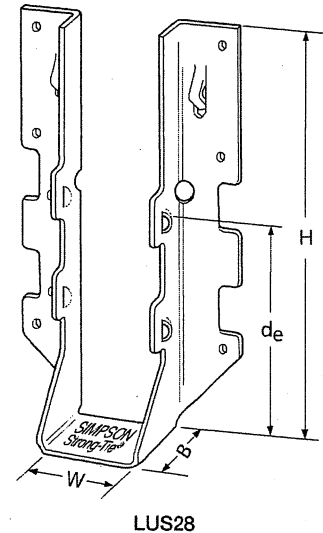
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.



Typical LUS Installation



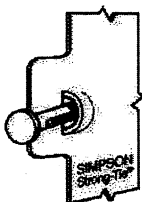
LUS28

Options:

- These hangers cannot be modified

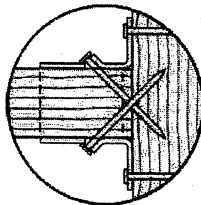
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _o =1.15)	Normal (K _o =1.00)	Uplift (K _o =1.15)	Normal (K _o =1.00)
LUS24	18	1 1/16	3 1/8	1 3/4	1 15/16	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 13/16	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1 1/16	4 3/4	1 3/4	3 5/8	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 7/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4 5/8	4 3/16	2	3 1/4	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 1/16	6 5/8	1 3/4	3 3/4	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4 5/8	6 1/4	2	3 1/4	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 1/16	7 13/16	1 3/4	3 7/8	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4 5/8	8 3/16	2	5 1/4	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Top View.



HAUT
MONT
DESIGN

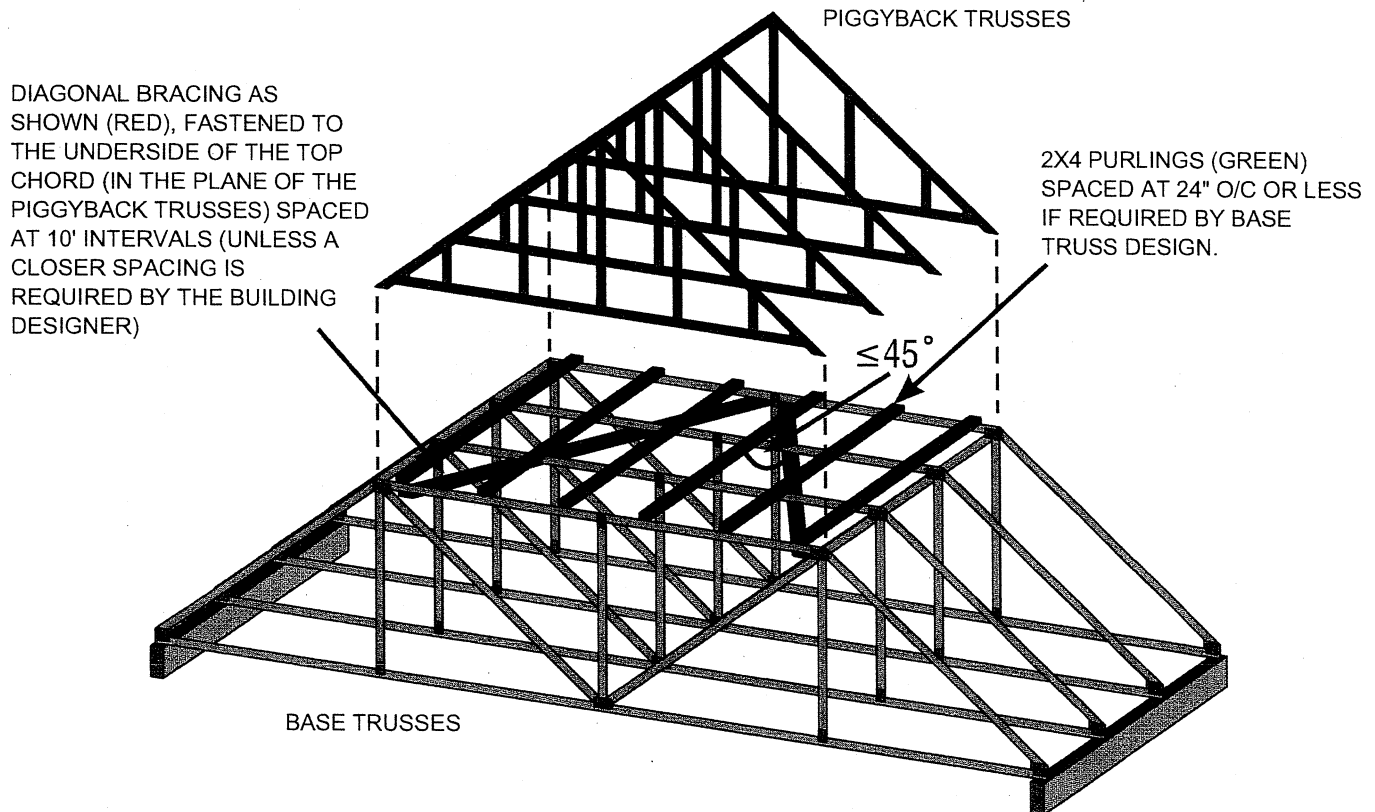
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Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

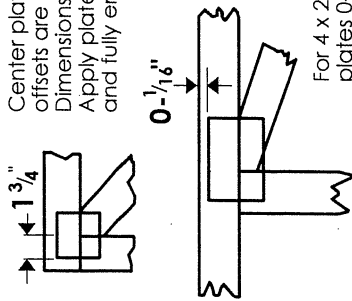
Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

Symbols

PLATE LOCATION AND ORIENTATION

Center plate on joint unless x, y offsets are indicated.
Dimensions are in ft-in-sixteenths or mm.
Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/8" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.



* Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

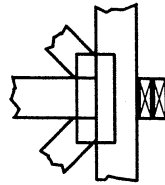
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



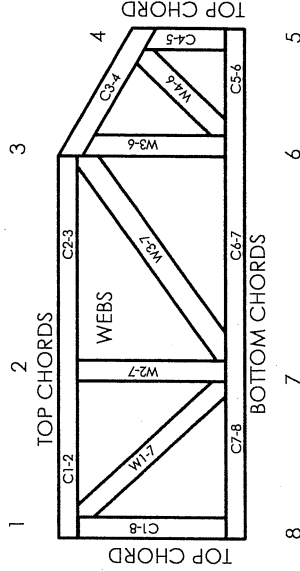
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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MiTek Engineering Reference Sheet: MIL-7473C rev. 10-'08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.