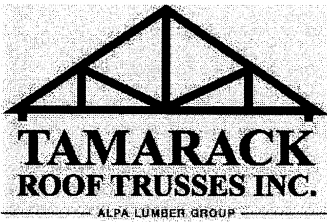


DELIVERY SHIPLIST



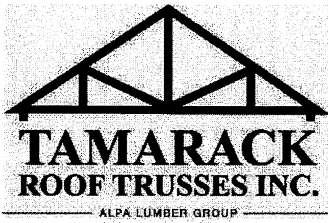
Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: CENTREFIELD
 Location: RICHMOND HILL
 Model: BLOCK 338
 Lot #:
 Elevation: A / UNIT1BLK338

Job Track: 51012
 PlanLog: 203547
 Layout ID: 413071
 Ref #
 Page: 1 of 2
 Date: 07-08-2021
 Designer: Andrew Conway
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Half Hip Girder	10 /12	19-05-08	4-01-04	2 x 4 2 x 6	1-03-08	1-07-11 4-01-04	193.91 122.67		
	1 2-ply	T1Z Half Hip Girder	10 /12	19-05-08	4-01-04	2 x 4 2 x 6	1-03-08	1-07-11 4-01-04	193.91 122.67		
	2	T2 Half Hip	10 /12	19-05-08	5-01-04	2 x 4	1-03-08	1-07-11 5-01-04	169.97 109.00		
	2	T3 Half Hip	10 /12	19-05-08	6-01-04	2 x 4	1-03-08	1-07-11 6-01-04	182.13 117.33		
	2	T4 Half Hip	10 /12	19-05-08	7-01-04	2 x 4	1-03-08	1-07-11 7-01-04	181.47 112.67		
	8	T5 Half Hip	10 /12	19-05-08	8-01-04	2 x 4	1-03-08	1-07-11 8-01-04	766.42 485.33		
	2	T6 Half Hip	10 /12	19-05-08	9-01-04	2 x 4	1-03-08	1-07-11 9-01-04	206.88 129.67		
	3	T8 Common	10 /12	9-06-00	5-07-03	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	130.39 86.50		
	2	T9S Roof Special	10 /12 6 /12	9-06-00	5-07-03	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	95.47 67.33		
	2	T10G GABLE	6 /12	22-08-08	8-01-04	2 x 4	1-05-00	1-02-00 8-01-04	243.75 153.67		
	1 2-ply	T38 Common Girder	10 /12	15-09-00	8-02-07	2 x 4 2 x 6		1-07-11 1-07-11	168.86 107.33		
	1	T39G GABLE	10 /12	15-09-00	8-02-07	2 x 4		1-07-11 1-07-11	74.85 48.00		
	1 2-ply	T40 Monopitch Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	58.39 37.67		
	2	PB01 Piggyback	10 /12	11-08-07	2-00-00	2 x 4		2-00-00	75.36 51.33		

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: CENTREFIELD
 Location: RICHMOND HILL
 Model: BLOCK 338
 Lot #:
 Elevation: A / UNIT1BLK338

Job Track: 51012
 PlanLog: 203547
 Layout ID: 413071
 Ref #
 Page: 2 of 2
 Date: 07-08-2021
 Designer: Andrew Conway
 Sales Rep: Mario DiCano

Roof Trusses

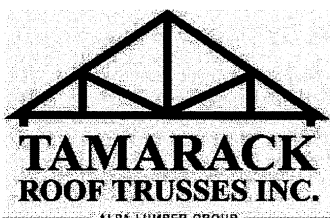
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	PB02 Piggyback	10 /12	11-08-07	3-00-00	2 x 4		3-00-00	81.28 54.67		
	2	PB03 Piggyback	10 /12	11-08-07	4-00-00	2 x 4		4-00-00	83.07 54.00		
	2	PB04G GABLE	6 /12	8-10-00	4-05-00	2 x 4		4-05-00	56.15 34.33		
	14	J01 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	235.12 149.33		
	5	J02 Jack-Open	3.5 /12	5-10-00	2-04-09	2 x 4	1-03-08	3-14 1-09-11	76.06 46.67		
	5	J03 Jack-Open	4 /12	3-07-00	1-11-03	2 x 4	1-03-08	3-15 1-03-04	50.28 33.33		

TOTAL # TRUSS= 64 TOTAL BFT OF ALL TRUSSES= 2123.5 BFT. TOTAL WEIGHT OF ALL TRSSES 3323.74 LBS

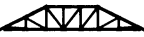
HARDWARE

QTY	TYPE	MODEL	LENGTH
3	Hardware	LUS24	
8	Hardware	LJS26DS	
1	Hardware	HGUS26-2	

TOTAL NUMBER OF ITEMS= 12

DELIVERY SHIPLIST				
 TAMARACK ROOF TRUSSES INC. ALPA LUMBER GROUP	Lumber Yard:	TAMARACK LUMBER	Job Track:	51012
	Builder:	ROYAL PINE HOMES	PlanLog:	203547
	Project:	CENTREFIELD	Layout ID:	413072
	Location:	RICHMOND HILL	Ref #	
	Model:	BLOCK 338	Page:	1 of 1
	Lot #:		Date:	07-08-2021
	Elevation:	A2 / UNIT2BLK338	Designer:	Andrew Conway
		Sales Rep:	Mario DiCano	

Roof Trusses

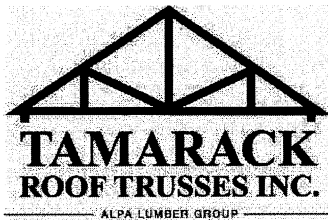
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	9	T11 Piggyback Base	6 /12	45-05-00	8-01-04	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	2302.82 1395.00		
	1	T11G GABLE	6 /12	45-05-00	8-01-04	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	272.06 169.67		
	1	T11GA GABLE	6 /12	45-05-00	8-01-04	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	265.46 167.00		
	1 3-ply	T12 Monopitch Girder	10 /12	8-05-08	8-08-04	2 x 4 2 x 8		1-07-11 8-08-04	164.13 108.50		
	2	T13S Scissor	10 /12 6 /12	8-07-00	5-02-10	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	81.56 52.00		
	1	T14 Monopitch	10 /12	8-03-08	8-06-10	2 x 4	1-03-08	1-07-11 8-06-10	45.99 28.83		
	9	PB06 Piggyback	6 /12	17-08-00	4-05-00	2 x 4			462.27 288.00		
	2	PB06G GABLE	6 /12	17-08-00	4-05-00	2 x 4			101.52 62.00		
	5	J02 Jack-Open	3.5 /12	5-10-00	2-04-09	2 x 4	1-03-08	3-14 1-09-11	76.06 46.67		

TOTAL # TRUSS= 33 TOTAL BFT OF ALL TRUSSES= 2317.67 BFT. TOTAL WEIGHT OF ALL TRSSES 3771.85 LBS

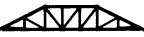
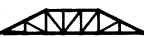
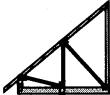
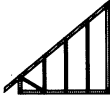
HARDWARE

QTY	TYPE	MODEL	LENGTH
4	Hardware	LJS26DS	
14	Hardware	H2.5T	
2	Hardware	LGT3	

TOTAL NUMBER OF ITEMS= 20

DELIVERY SHIPLIST				
	Lumber Yard:	TAMARACK LUMBER	Job Track:	51012
	Builder:	ROYAL PINE HOMES	PlanLog:	203547
	Project:	CENTREFIELD	Layout ID:	413073
	Location:	RICHMOND HILL	Ref #	
	Model:	BLOCK 338	Page:	1 of 1
	Lot #:		Date:	07-08-2021
	Elevation:	A1 / UNIT3BLK338	Designer:	Andrew Conway
		Sales Rep:	Mario DiCano	

Roof Trusses

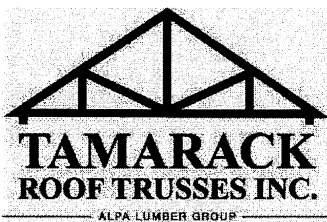
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	6	T11 Piggyback Base	6 /12	45-05-00	8-01-04	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	1535.21 930.00		
	1	T11GA GABLE	6 /12	45-05-00	8-01-04	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	265.46 167.00		
	4	T11X Piggyback Base	6 /12	45-05-00	8-01-04	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	995.26 604.00		
	1 3-ply	T15 Monopitch Girder	10 /12	8-06-08	8-09-02	2 x 4 2 x 8	1-03-08	1-07-11 8-09-02	171.4 112.50		
	1	T16G GABLE	10 /12	8-05-00	8-07-14	2 x 4	1-03-08	1-07-11 8-07-14	48.26 30.83		
	10	PB06 Piggyback	6 /12	17-08-00	4-05-00	2 x 4			513.63 320.00		
	1	PB06G GABLE	6 /12	17-08-00	4-05-00	2 x 4			50.76 31.00		
	5	J02 Jack-Open	3.5 /12	5-10-00	2-04-09	2 x 4	1-03-08	3-14 1-09-11	76.06 46.67		
	5	J03 Jack-Open	4 /12	3-07-00	1-11-03	2 x 4	1-03-08	3-15 1-03-04	50.28 33.33		

TOTAL # TRUSS= 36 TOTAL BFT OF ALL TRUSSES= 2275.33 BFT. TOTAL WEIGHT OF ALL TRSSES 3706.32 LBS

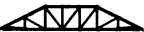
HARDWARE

QTY	TYPE	MODEL	LENGTH
4	Hardware	LJS26DS	
16	Hardware	H2.5T	
2	Hardware	LGT3	

TOTAL NUMBER OF ITEMS= 22

 TAMARACK ROOF TRUSSES INC. ALPA LUMBER GROUP	DELIVERY SHIPLIST			
	Lumber Yard: TAMARACK LUMBER Builder: ROYAL PINE HOMES Project: CENTREFIELD Location: RICHMOND HILL Model: BLOCK 338 Lot #: Elevation: A1 / UNIT4BLK338	Job Track: 51012 PlanLog: 203547 Layout ID: 413074 Ref # Page: 1 of 1 Date: 07-08-2021 Designer: Andrew Conway Sales Rep: Mario DiCano		

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	6	T11 Piggyback Base	6 /12	45-05-00	8-01-04	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	1535.21 930.00		
	1	T11GA GABLE	6 /12	45-05-00	8-01-04	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	265.46 167.00		
	4	T11X Piggyback Base	6 /12	45-05-00	8-01-04	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	995.26 604.00		
	1 3-ply	T15 Monopitch Girder	10 /12	8-06-08	8-09-02	2 x 4 2 x 8	1-03-08	1-07-11 8-09-02	171.4 112.50		
	1	T16G GABLE	10 /12	8-05-00	8-07-14	2 x 4	1-03-08	1-07-11 8-07-14	48.26 30.83		
	10	PB06 Piggyback	6 /12	17-08-00	4-05-00	2 x 4			513.63 320.00		
	1	PB06G GABLE	6 /12	17-08-00	4-05-00	2 x 4			50.76 31.00		
	5	J02 Jack-Open	3.5 /12	5-10-00	2-04-09	2 x 4	1-03-08	3-14 1-09-11	76.06 46.67		
	5	J03 Jack-Open	4 /12	3-07-00	1-11-03	2 x 4	1-03-08	3-15 1-03-04	50.28 33.33		

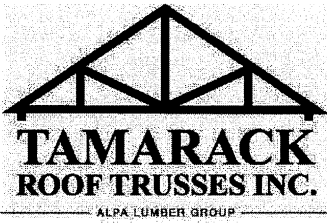
TOTAL # TRUSS= 36 TOTAL BFT OF ALL TRUSSES= 2275.33 BFT. TOTAL WEIGHT OF ALL TRSSES 3706.32 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
4	Hardware	LJS26DS	
16	Hardware	H2.5T	
2	Hardware	LGT3	

TOTAL NUMBER OF ITEMS= 22

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: CENTREFIELD
 Location: RICHMOND HILL
 Model: BLOCK 338
 Lot #:
 Elevation: A / UNIT5BLK338

Job Track: 51012
 PlanLog: 203547
 Layout ID: 413075
 Ref #
 Page: 1 of 2
 Date: 07-08-2021
 Designer: Andrew Conway
 Sales Rep: Mario DiCano

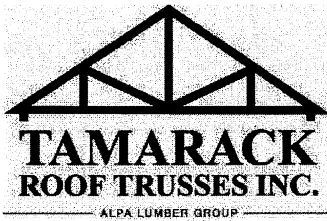
Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	3	T11 Piggyback Base	6 /12	45-05-00	8-01-04	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	767.61 465.00		
	1	T11GA GABLE	6 /12	45-05-00	8-01-04	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	265.46 167.00		
	1 3-ply	T28 Common Girder	10 /12	9-05-08	9-02-02	2 x 4 2 x 8		1-07-11 8-09-15	203.57 130.00		
	1	T29S Roof Special	10 /12 6 /12	9-03-08	9-04-10	2 x 4	1-03-08	1-07-11 9-04-10	66.46 43.83		
	6	T37 Piggyback Base	6 /12	45-05-00	8-01-04	2 x 6		6-00 6-00	1373.31 826.00		
	1	T37G GABLE	6 /12	45-05-00	8-01-04	2 x 6		6-00 6-00	258.48 161.00		
	3	PB06 Piggyback	6 /12	17-08-00	4-05-00	2 x 4			154.09 96.00		
	1	PB06G GABLE	6 /12	17-08-00	4-05-00	2 x 4			50.76 31.00		
	6	PB32 Piggyback	6 /12	15-00-00	3-09-00	2 x 4			219.15 140.00		
	1	PB32G GABLE	6 /12	15-00-00	3-09-00	2 x 4			41.61 28.33		
	5	J02 Jack-Open	3.5 /12	5-10-00	2-04-09	2 x 4	1-03-08	3-14 1-09-11	76.06 46.67		
	5	J03 Jack-Open	4 /12	3-07-00	1-11-03	2 x 4	1-03-08	3-15 1-03-04	50.28 33.33		

TOTAL # TRUSS= 36 TOTAL BFT OF ALL TRUSSES= 2168.16 BFT. TOTAL WEIGHT OF ALL TRSSES 3526.82 LBS

HARDWARE

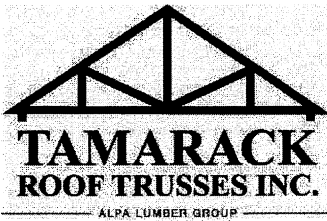
QTY	TYPE	MODEL	LENGTH
4	Hardware	LJS26DS	
14	Hardware	H2.5T	

		DELIVERY SHIPLIST	
 TAMARACK ROOF TRUSSES INC. ALPA LUMBER GROUP	Lumber Yard:	TAMARACK LUMBER	Job Track: 51012
	Builder:	ROYAL PINE HOMES	PlanLog: 203547
	Project:	CENTREFIELD	Layout ID: 413075
	Location:	RICHMOND HILL	Ref #
	Model:	BLOCK 338	Page: 2 of 2
	Lot #:		Date: 07-08-2021
	Elevation:	A / UNIT5BLK338	Designer: Andrew Conway
		Sales Rep: Mario DiCano	

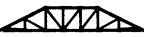
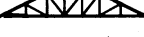
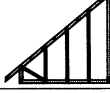
HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	LGT3	

TOTAL NUMBER OF ITEMS= 20

 TAMARACK ROOF TRUSSES INC. ALPHA LUMBER GROUP	DELIVERY SHIPLIST			
	Lumber Yard: TAMARACK LUMBER Builder: ROYAL PINE HOMES Project: CENTREFIELD Location: RICHMOND HILL Model: BLOCK 338 Lot #: Elevation: A1 / UNIT6BLK338	Job Track: 51012 PlanLog: 203547 Layout ID: 413076 Ref # Page: 1 of 1 Date: 07-08-2021 Designer: Andrew Conway Sales Rep: Mario DiCano		

Roof Trusses

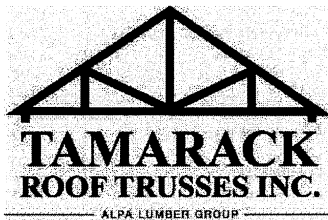
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	5	T11 Piggyback Base	6 /12	45-05-00	8-01-04	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	1279.34 775.00		
	1	T11G GABLE	6 /12	45-05-00	8-01-04	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	272.06 169.67		
	1	T11GA GABLE	6 /12	45-05-00	8-01-04	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	265.46 167.00		
	4	T11X Piggyback Base	6 /12	45-05-00	8-01-04	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	995.26 604.00		
	1 3-ply	T15 Monopitch Girder	10 /12	8-06-08	8-09-02	2 x 4 2 x 8	1-03-08	1-07-11 8-09-02	171.4 112.50		
	1	T16G GABLE	10 /12	8-05-00	8-07-14	2 x 4	1-03-08	1-07-11 8-07-14	48.26 30.83		
	9	PB06 Piggyback	6 /12	17-08-00	4-05-00	2 x 4			462.27 288.00		
	2	PB06G GABLE	6 /12	17-08-00	4-05-00	2 x 4			101.52 62.00		
	5	J02 Jack-Open	3.5 /12	5-10-00	2-04-09	2 x 4	1-03-08	3-14 1-09-11	76.06 46.67		
	5	J03 Jack-Open	4 /12	3-07-00	1-11-03	2 x 4	1-03-08	3-15 1-03-04	50.28 33.33		

TOTAL # TRUSS= 36 TOTAL BFT OF ALL TRUSSES= 2289 BFT. TOTAL WEIGHT OF ALL TRSSES 3721.91 LBS

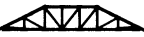
HARDWARE

QTY	TYPE	MODEL	LENGTH
4	Hardware	LJS26DS	
14	Hardware	H2.5T	
2	Hardware	LGT3	

TOTAL NUMBER OF ITEMS= 20

DELIVERY SHIPLIST				
 TAMARACK ROOF TRUSSES INC. ALPHA LUMBER GROUP	Lumber Yard:	TAMARACK LUMBER	Job Track:	51012
	Builder:	ROYAL PINE HOMES	PlanLog:	203547
	Project:	CENTREFIELD	Layout ID:	413077
	Location:	RICHMOND HILL	Ref #	
	Model:	BLOCK 338	Page:	1 of 2
	Lot #:		Date:	07-08-2021
	Elevation:	A2 / UNIT7BLK338	Designer:	Andrew Conway
		Sales Rep:	Mario DiCano	

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	3	T11 Piggyback Base	6 /12	45-05-00	8-01-04	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	767.61 465.00		
	1	T11GA GABLE	6 /12	45-05-00	8-01-04	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	265.46 167.00		
	1 3-ply	T12 Monopitch Girder	10 /12	8-05-08	8-08-04	2 x 4 2 x 8		1-07-11 8-08-04	164.13 108.50		
	2	T13S Scissor	10 /12 6 /12	8-07-00	5-02-10	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	81.56 52.00		
	1	T14 Monopitch	10 /12	8-03-08	8-06-10	2 x 4	1-03-08	1-07-11 8-06-10	45.99 28.83		
	6	T81 Piggyback Base	6 /12	45-05-00	8-01-04	2 x 6		1-02-00 1-02-00	1471 912.00		
	1	T81G GABLE	6 /12	45-05-00	8-01-04	2 x 6		2-00 2-00	250.51 155.00		
	3	PB06 Piggyback	6 /12	17-08-00	4-05-00	2 x 4			154.09 96.00		
	1	PB06G GABLE	6 /12	17-08-00	4-05-00	2 x 4			50.76 31.00		
	6	PB81 Piggyback	6 /12	13-08-00	3-05-00	2 x 4			198.2 121.00		
	1	PB81G GABLE	6 /12	13-08-00	3-05-00	2 x 4			35.84 22.17		
	5	J02 Jack-Open	3.5 /12	5-10-00	2-04-09	2 x 4	1-03-08	3-14 1-09-11	76.06 46.67		

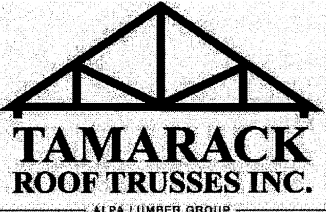
TOTAL # TRUSS= 33

TOTAL BFT OF ALL TRUSSES= 2205.17 BFT.

TOTAL WEIGHT OF ALL TRSSES 3561.2 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
3	Hardware	LJS26DS	
14	Hardware	H2.5T	

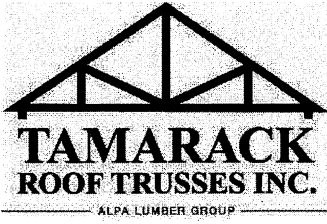
		DELIVERY SHIPLIST	
 TAMARACK ROOF TRUSSES INC. ALFA LUMBER GROUP	Lumber Yard:	TAMARACK LUMBER	Job Track: 51012
	Builder:	ROYAL PINE HOMES	PlanLog: 203547
	Project:	CENTREFIELD	Layout ID: 413077
	Location:	RICHMOND HILL	Ref #
	Model:	BLOCK 338	Page: 2 of 2
	Lot #:		Date: 07-08-2021
	Elevation:	A2 / UNIT7BLK338	Designer: Andrew Conway
		Sales Rep: Mario DiCano	

HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	LGT3	

TOTAL NUMBER OF ITEMS= 19

DELIVERY SHIPLIST

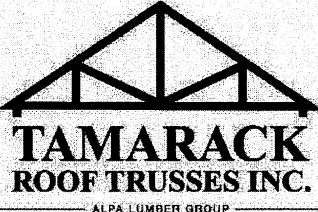


Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: CENTREFIELD
 Location: RICHMOND HILL
 Model: BLOCK 338
 Lot #:
 Elevation: A / UNIT8BLK338

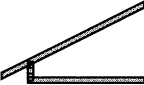
Job Track: 51012
 PlanLog: 203547
 Layout ID: 413078
 Ref #
 Page: 1 of 2
 Date: 07-08-2021
 Designer: Andrew Conway
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Half Hip Girder	10 / 12	19-05-08	4-01-04	2 x 4 2 x 6	1-03-08	1-07-11 4-01-04	193.91 122.67		
	1 2-ply	T1Z Half Hip Girder	10 / 12	19-05-08	4-01-04	2 x 4 2 x 6	1-03-08	1-07-11 4-01-04	193.91 122.67		
	2	T2 Half Hip	10 / 12	19-05-08	5-01-04	2 x 4	1-03-08	1-07-11 5-01-04	169.97 109.00		
	2	T3 Half Hip	10 / 12	19-05-08	6-01-04	2 x 4	1-03-08	1-07-11 6-01-04	182.13 117.33		
	2	T4 Half Hip	10 / 12	19-05-08	7-01-04	2 x 4	1-03-08	1-07-11 7-01-04	181.47 112.67		
	8	T5 Half Hip	10 / 12	19-05-08	8-01-04	2 x 4	1-03-08	1-07-11 8-01-04	766.42 485.33		
	2	T6 Half Hip	10 / 12	19-05-08	9-01-04	2 x 4	1-03-08	1-07-11 9-01-04	206.88 129.67		
	3	T8 Common	10 / 12	9-06-00	5-07-03	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	130.39 86.50		
	2	T9S Scissor	10 / 12 6 / 12	9-06-00	5-07-03	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	95.47 67.33		
	2	T10G GABLE	6 / 12	22-08-08	8-01-04	2 x 4	1-05-00	1-02-00 8-01-04	243.75 153.67		
	1 2-ply	T38 Common Girder	10 / 12	15-09-00	8-02-07	2 x 4 2 x 6		1-07-11 1-07-11	168.86 107.33		
	1	T39G GABLE	10 / 12	15-09-00	8-02-07	2 x 4		1-07-11 1-07-11	74.85 48.00		
	1 2-ply	T40 Monopitch Girder	6 / 12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	58.39 37.67		
	2	PB01 Piggyback	10 / 12	11-08-07	2-00-00	2 x 4		2-00-00	75.36 51.33		

 TAMARACK ROOF TRUSSES INC. ALPHA LUMBER GROUP	DELIVERY SHIPLIST				
	Lumber Yard: TAMARACK LUMBER				Job Track: 51012
	Builder: ROYAL PINE HOMES				PlanLog: 203547
	Project: CENTREFIELD				Layout ID: 413078
	Location: RICHMOND HILL				Ref #
	Model: BLOCK 338				Page: 2 of 2
Lot #:				Date: 07-08-2021	
Elevation: A / UNIT8BLK338				Designer: Andrew Conway	
				Sales Rep: Mario DiCano	

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	PB02 Piggyback	10 /12	11-08-07	3-00-00	2 x 4		3-00-00	81.28 54.67		
	2	PB03 Piggyback	10 /12	11-08-07	4-00-00	2 x 4		4-00-00	83.07 54.00		
	2	PB04G GABLE	6 /12	8-10-00	4-05-00	2 x 4		4-05-00	56.15 34.33		
	14	J01 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	235.12 149.33		
	5	J02 Jack-Open	3.5 /12	5-10-00	2-04-09	2 x 4	1-03-08	3-14 1-09-11	76.06 46.67		
	5	J03 Jack-Open	4 /12	3-07-00	1-11-03	2 x 4	1-03-08	3-15 1-03-04	50.28 33.33		

TOTAL # TRUSS= 64 TOTAL BFT OF ALL TRUSSES= 2123.5 BFT. TOTAL WEIGHT OF ALL TRSSES 3323.74 LBS

HARDWARE

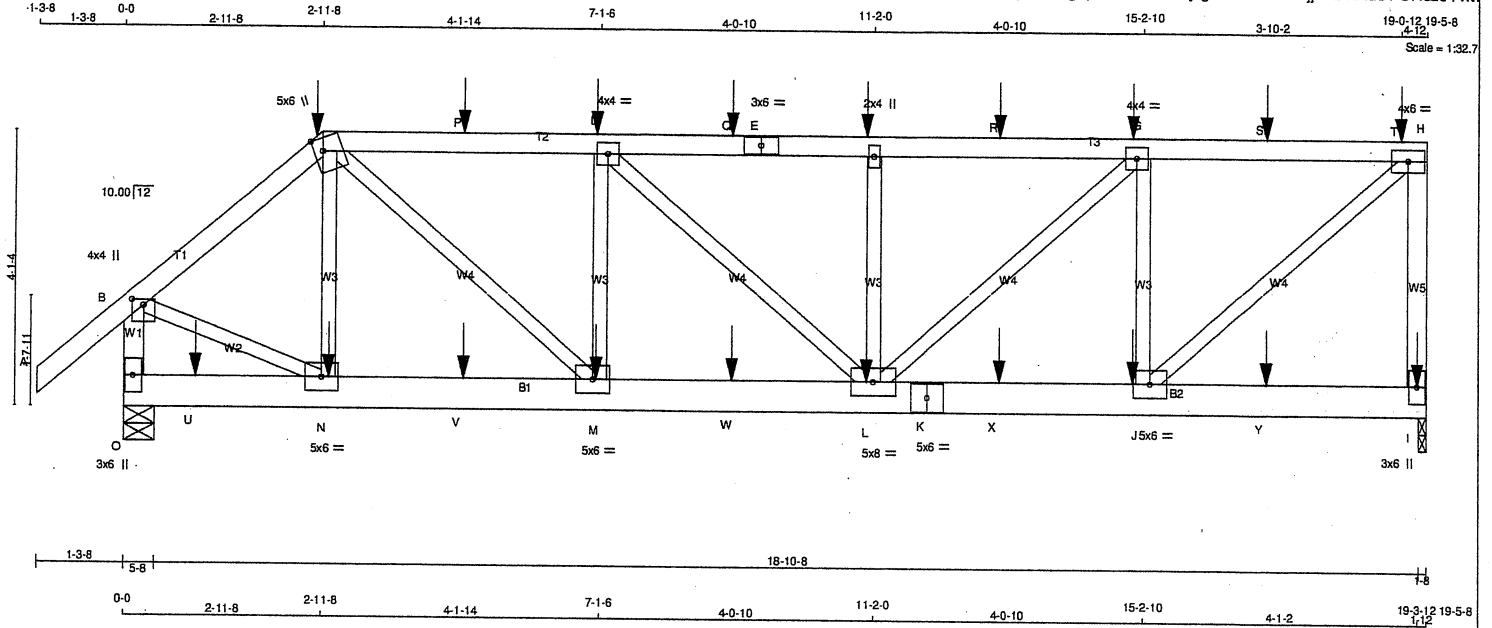
QTY	TYPE	MODEL	LENGTH
3	Hardware	LUS24	
8	Hardware	LJS26DS	
1	Hardware	HGUS26-2	

TOTAL NUMBER OF ITEMS= 12

JOB NAME 412868	TRUSS NAME T1	QUANTITY 2	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 1 10:22:31 2021 Page 1
ID:wTM7ioISByniqocTeDgf3Pyb9F8-gbpxA7Y5Ksd?lytgb5SaLkWA8jj9XBhAdDFUNcz0Tvm



LUMBER				N. L. G. A. RULES	
CHORDS	SIZE	LUMBER	DESCR.	CHORDS	SIZE
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
E - H	2x4	DRY	No.2	SPF	
O - B	2x4	DRY	No.2	SPF	
O - K	2x6	DRY	No.2	SPF	
K - I	2x6	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
I - H	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C 1	12	SIDE (61.0)
C-E 1	12	SIDE (61.0)
E-H 1	12	SIDE (61.0)
O-B 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
O-K 2	12	SIDE (183.1)
K-I 2	12	SIDE (183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	
2x4 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQD
	VERT	HORZ	DOWN	UP/LIFT
JT	1856	0	1856	0
I	1856	0	1856	0
O	1936	0	1936	0

UNFACTORED REACTIONS

	1ST CASE	MAX/MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	1314	855 / 0	0 / 0	0 / 0	0 / 0	460 / 0	0 / 0
I	1367	910 / 0	0 / 0	0 / 0	0 / 0	457 / 0	0 / 0
O	1367	910 / 0	0 / 0	0 / 0	0 / 0	457 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLINE SPACING = 5.69 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO				FR-TO			
A-B	0 / 41	-91.8	-91.8 0.07 (1)	I-H	-1775 / 0	0.16 (1)	
B-C	-1727 / 0	-91.8	-91.8 0.09 (1)	N-C	-390 / 0	0.05 (1)	
C-P	-2233 / 0	-91.8	-91.8 0.20 (1)	B-N	0 / 1423	0.18 (1)	
P-D	-2233 / 0	-91.8	-91.8 0.20 (1)	J-H	0 / 2216	0.27 (1)	
D-Q	-2268 / 0	-91.8	-91.8 0.20 (1)	C-M	0 / 1237	0.15 (1)	
Q-E	-2268 / 0	-91.8	-91.8 0.20 (1)	J-G	-1324 / 0	0.17 (1)	
E-F	-2268 / 0	-91.8	-91.8 0.20 (1)	M-D	-699 / 0	0.09 (1)	
F-R	-2268 / 0	-91.8	-91.8 0.21 (1)	L-G	0 / 915	0.11 (1)	
R-G	-2268 / 0	-91.8	-91.8 0.21 (1)	D-L	0 / 46	0.01 (1)	
G-S	-1595 / 0	-91.8	-91.8 0.20 (1)	L-F	-542 / 0	0.07 (1)	
S-T	-1595 / 0	-91.8	-91.8 0.20 (1)				
T-H	-1595 / 0	-91.8	-91.8 0.20 (1)				
O-B	-1902 / 0	0.0	0.0 0.11 (1)				
O-U	0 / 0	-18.5	-18.5 0.02 (4)				
U-N	0 / 0	-18.5	-18.5 0.02 (4)				
N-V	0 / 1311	-18.5	-18.5 0.10 (1)				
V-M	0 / 1311	-18.5	-18.5 0.10 (1)				
M-W	0 / 2234	-18.5	-18.5 0.18 (1)				
W-L	0 / 2234	-18.5	-18.5 0.18 (1)				
L-K	0 / 1595	-18.5	-18.5 0.14 (1)				
K-X	0 / 1595	-18.5	-18.5 0.14 (1)				
X-J	0 / 1595	-18.5	-18.5 0.14 (1)				
J-Y	-69 / 0	-18.5	-18.5 0.03 (4)				
Y-I	-69 / 0	-18.5	-18.5 0.03 (4)				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-11-8	-25	-25		FRONT	VERT	DEAD		C1
C	2-11-8	-104	-104		BACK	VERT	TOTAL		C1
C	2-11-8	-105	-105		FRONT	VERT	SNOW		C1
D	7-0-12	-76	-76		BACK	VERT	TOTAL		C1
F	11-0-12	-76	-76		BACK	VERT	TOTAL		C1
G	15-0-12	-76	-76		BACK	VERT	TOTAL		C1
I	19-3-12	-27	-27		BACK	VERT	TOTAL		C1
J	15-0-12	-21	-21		BACK	VERT	TOTAL		C1
L	11-0-12	-21	-21		BACK	VERT	TOTAL		C1
M	7-0-12	-21	-21		BACK	VERT	TOTAL		C1
N	3-0-12	-21	-21		BACK	VERT	TOTAL		C1
P	5-0-12	-76	-76		BACK	VERT	TOTAL		C1
Q	9-0-12	-76	-76		BACK	VERT	TOTAL		C1
R	13-0-12	-76	-76		BACK	VERT	TOTAL		C1
S	17-0-12	-76	-76		BACK	VERT	TOTAL		C1

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***

GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OSC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.64")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.03")
ALLOWABLE DEFL. (TL) = $L/360$ (0.64")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.06")

CSI: TC=0.21/1.00 (F-G:1), BC=0.18/1.00 (L-M:1), WB=0.27/1.00 (H-J:1), SS=0.14/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650 371 1747 788 1987 1873	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.85 (B) (INPUT = 0.90)
JSI METAL=0.24 (H) (INPUT = 1.00)



Structural component only
DWG# T-2121151 *in*

CONTINUED ON PAGE 2

JOB NAME 412868	TRUSS NAME T1	QUANTITY 2	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:wTM7iolSByniqocTeDgf3Pvb9F8-gbpxA?Y5Ksd?lvtgb5SaLkWA8ji9XBhAdDFUNcz0Tvm

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TMWW-t	MT20	4.0	4.0		
H	TMWW-t	MT20	4.0	6.0		
I	BMW1+w	MT20	3.0	6.0		
J, M, N						
J	BMWW-t	MT20	5.0	6.0		
K	BS-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	5.0	8.0		
O	BMV1+p	MT20	3.0	6.0		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
T	19-0-12	-95	-95	—	BACK	VERT	TOTAL	—	C1
U	1-0-12	-21	-21	—	BACK	VERT	TOTAL	—	C1
V	5-0-12	-21	-21	—	BACK	VERT	TOTAL	—	C1
W	9-0-12	-21	-21	—	BACK	VERT	TOTAL	—	C1
X	13-0-12	-21	-21	—	BACK	VERT	TOTAL	—	C1
Y	17-0-12	-21	-21	—	BACK	VERT	TOTAL	—	C1

CONNECTION REQUIREMENTS

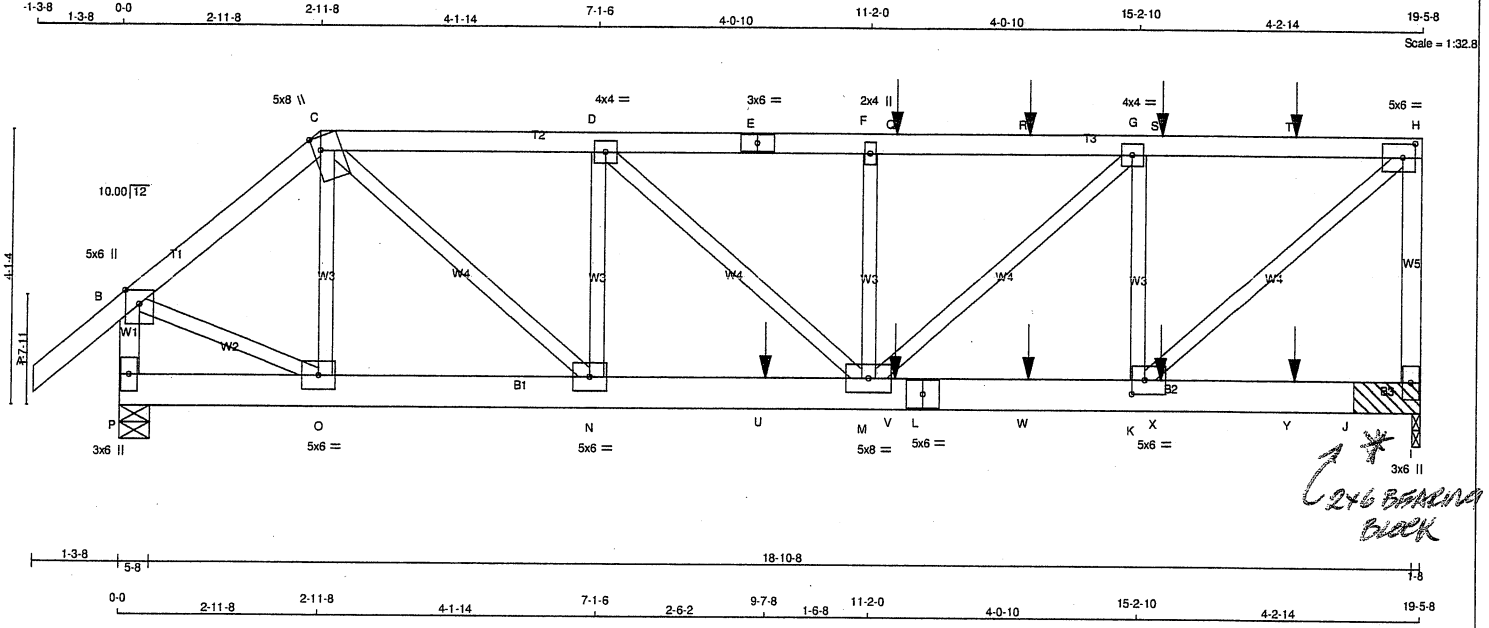
- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2121151 *m*

JOB NAME 413062	TRUSS NAME T1Z	QUANTITY 2	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 8 16:46:22 2021 Page 1
ID:wTM7ioISByniqocTeDgf3Pyb9F8-UzJz7CZSEyJ5i6gmBKfGQCt0WCQ?uUTP8qF14z_4dv



TOTAL WEIGHT = 4 X 97 = 388 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - H	2x4	DRY	No.2
I - H	2x4	DRY	No.2
P - B	2x4	DRY	No.2
P - L	2x6	DRY	No.2
L - I	2x6	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C 1	12	TOP
C-E 1	12	TOP
E-H 1	12	SIDE(61.0)
H-I 1	12	TOP
P-B 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
P-L 2	12	SIDE(0.0)
L-I 2	12	SIDE(183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
JT 2190	0	2190	0	0	1-8	1-8	& BLOCK
P 2061	0	2061	0	0	5-8	5-8	

UNFACTORED REACTIONS							
1ST LCASE	MAX.	MIN.	COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.	LIVE	WIND	DEAD
I	1546	1032 / 0	0 / 0	0 / 0	0 / 0	513 / 0	0 / 0
P	1451	988 / 0	0 / 0	0 / 0	0 / 0	462 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, P
2x6 DRY SPF No.2 BEARING BLOCK 12" LONG AT JT. I ATTACHED TO FRONT SIDE WITH 3 ROWS OF (0.122"x3") SPIRAL NAILS SPACED 3" C.C. 12 NAILS TOTAL.

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.76 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS			WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO				FR-TO	
A-B	0 / 41	-91.8 -91.8 0.07 (1)	10.00	O-C	-604 / 0
B-C	-1850 / 0	-91.8 -91.8 0.09 (1)	6.25	B-O	0 / 1525
C-D	-3065 / 0	-91.8 -91.8 0.15 (1)	5.15	K-H	0 / 2893
D-E	-3481 / 0	-91.8 -91.8 0.16 (1)	4.89	C-N	0 / 2232
E-F	-3481 / 0	-91.8 -91.8 0.16 (1)	4.89	K-G	-1912 / 0
F-Q	-3481 / 0	-91.8 -91.8 0.25 (1)	4.76	N-D	-786 / 0
Q-R	-3481 / 0	-91.8 -91.8 0.25 (1)	4.76	M-G	0 / 1778
R-G	-3481 / 0	-91.8 -91.8 0.25 (1)	4.76	D-M	0 / 566
G-S	-2172 / 0	-91.8 -91.8 0.22 (1)	5.76	M-F	-469 / 0
S-T	-2172 / 0	-91.8 -91.8 0.22 (1)	5.76		
T-H	-2172 / 0	-91.8 -91.8 0.22 (1)	5.76		
H-I	-2113 / 0	0.0 0.0 0.26 (1)	7.63		
P-B	-2018 / 0	0.0 0.0 0.12 (1)	7.76		

P-O	0 / 0	-18.5	-18.5	0.02 (1)	10.00
O-N	0 / 1399	-18.5	-18.5	0.21 (1)	10.00
N-U	0 / 3065	-18.5	-18.5	0.59 (1)	10.00
U-M	0 / 3065	-18.5	-18.5	0.59 (1)	10.00
M-V	0 / 2172	-18.5	-18.5	0.28 (1)	10.00
V-L	0 / 2172	-18.5	-18.5	0.28 (1)	10.00
L-W	0 / 2172	-18.5	-18.5	0.28 (1)	10.00
W-K	0 / 2172	-18.5	-18.5	0.28 (1)	10.00
K-X	0 / 0	-18.5	-18.5	0.04 (1)	10.00
X-Y	0 / 0	-18.5	-18.5	0.04 (1)	10.00
Y-J	0 / 0	-18.5	-18.5	0.04 (1)	10.00
J-I	0 / 0	-18.5	-18.5	0.04 (1)	10.00

SPECIFIED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.	
Q	11-6-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
R	13-6-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
S	15-6-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
T	17-6-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
U	9-7-8	-1003	-1003	---	BACK	VERT	TOTAL	---	C1
V	11-6-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
W	13-6-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
X	15-6-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
Y	17-6-12	-21	-21	---	BACK	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL) = L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/999 (0.12")

CSI: TC=0.26/1.00 (H-I:1), BC=0.59/1.00 (M-N:1), WB=0.36/1.00 (H-K:1), SS=0.31/1.00 (M-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (G) (INPUT = 0.90)
JSI METAL= 0.34 (B) (INPUT = 1.00)



Structural component only
DWG# T-2122145

JOB NAME 413062	TRUSS NAME T1Z	QUANTITY 2	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 8 16:46:22 2021 Page 2
ID:wTM7iolSByniqocTeDgf3Pyb9F8-UzJz7CZSEy5l6qmBKtGGQCt0WCQ?uUTP8qF1f4z 4dV

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	8.0	Edge	1.25
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TMWW-t	MT20	4.0	4.0		
H	TMVW-t	MT20	5.0	6.0	2.50	2.25
I	BMV1+p	MT20	3.0	6.0		
K	BMWW-t	MT20	5.0	6.0	2.50	2.25
L	BS-t	MT20	5.0	6.0		
M	BMWWW-t	MT20	5.0	8.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMWW-t	MT20	5.0	6.0		
P	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

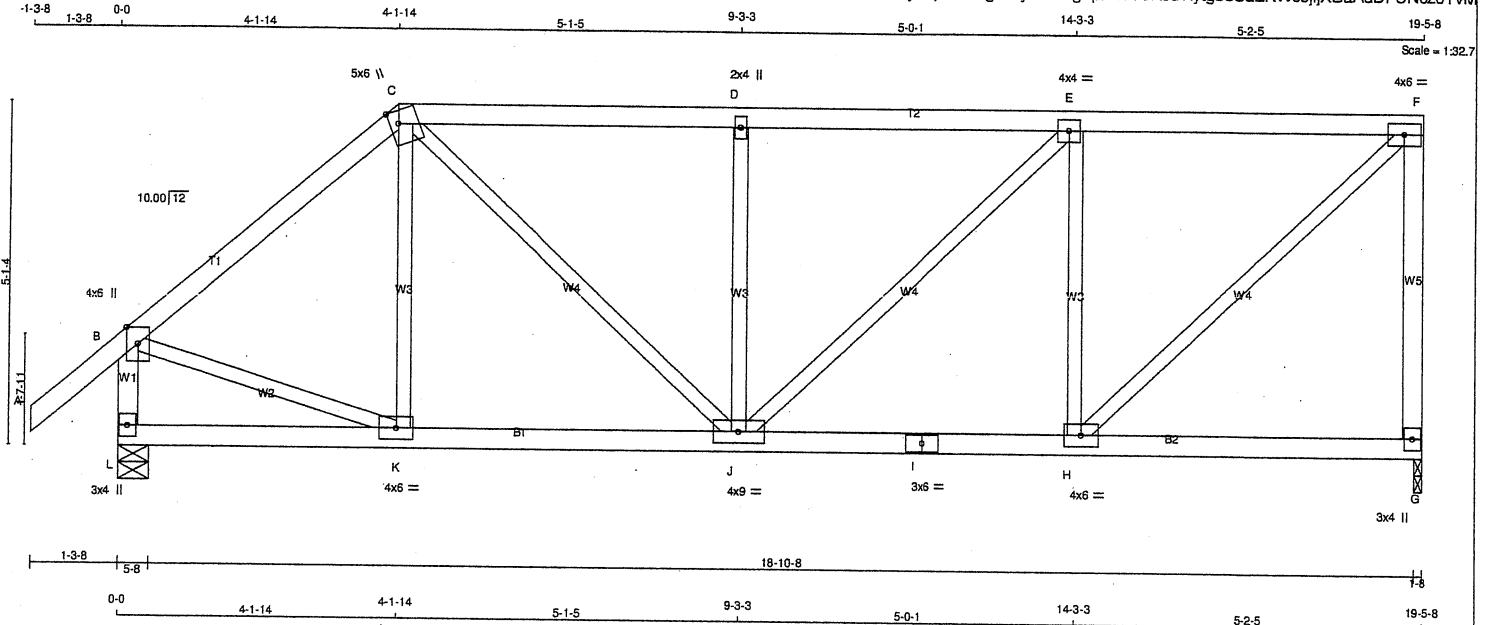


Structural component only
DWG# T-2122145 *ML*

JOB NAME 412868	TRUSS NAME T2	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

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LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
G - F	2x4	DRY	No.2
L - B	2x4	DRY	No.2
L - I	2x4	DRY	No.2
I - G	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMWV-t	MT20	4.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWV-t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWVW-t	MT20	4.0	9.0		
K	BMWV-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	VERT	HORZ	DOWN	HORZ	BRG	IN-SX	BRG	IN-SX
G	1073	0	1073	0	0	1-8	1-8		
L	1200	0	1200	0	0	5-8	5-8		

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN.	COMPONENT REACTIONS						
G	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL		
L	759	498 / 0	0 / 0	0 / 0	0 / 0	261 / 0	0 / 0		
G	846	569 / 0	0 / 0	0 / 0	0 / 0	277 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.50 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX. FACTORED		WEBS		FACTORED		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX	UNBRAC	MEMB.	FORCE (LBS)	MAX	LC1	MEMB.	FORCE (LBS)
FR-TO		FROM	TO	LENGTH	FR-TO						
A-B	0 / 41	-91.8	-91.8	0.13 (1)	10.00	K-C	-130 / 29	0.05 (1)			
B-C	-971 / 0	-91.8	-91.8	0.30 (1)	5.92	B-K	0 / 778	0.18 (1)			
C-D	-1133 / 0	-91.8	-91.8	0.34 (1)	5.52	H-F	0 / 1236	0.28 (1)			
D-E	-1133 / 0	-91.8	-91.8	0.35 (1)	5.50	C-J	0 / 537	0.12 (1)			
E-F	-906 / 0	-91.8	-91.8	0.35 (1)	5.99	H-E	-733 / 0	0.28 (1)			
G-F	-1033 / 0	0.0	0.0	0.46 (1)	7.72	J-D	-500 / 0	0.19 (1)			
L-B	-1170 / 0	0.0	0.0	0.12 (1)	7.37	J-E	0 / 315	0.07 (1)			
L-K	0 / 0	-18.5	-18.5	0.09 (4)	10.00						
K-J	0 / 741	-18.5	-18.5	0.17 (1)	10.00						
J-I	0 / 906	-18.5	-18.5	0.21 (1)	10.00						
I-H	0 / 906	-18.5	-18.5	0.21 (1)	10.00						
H-G	0 / 0	-18.5	-18.5	0.12 (4)	10.00						

TOTAL WEIGHT = 2 X 85 = 170 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT),
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/999 (0.07")

CSI: TC=0.46/1.00 (F-G:1), BC=0.21/1.00 (H-J:1), WB=0.28/1.00 (E-H:1), SSI=0.22/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

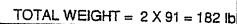
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

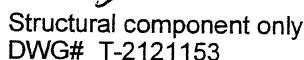
JSI GRIP= 0.79 (H) (INPUT = 0.90)
JSI METAL= 0.49 (B) (INPUT = 1.00)

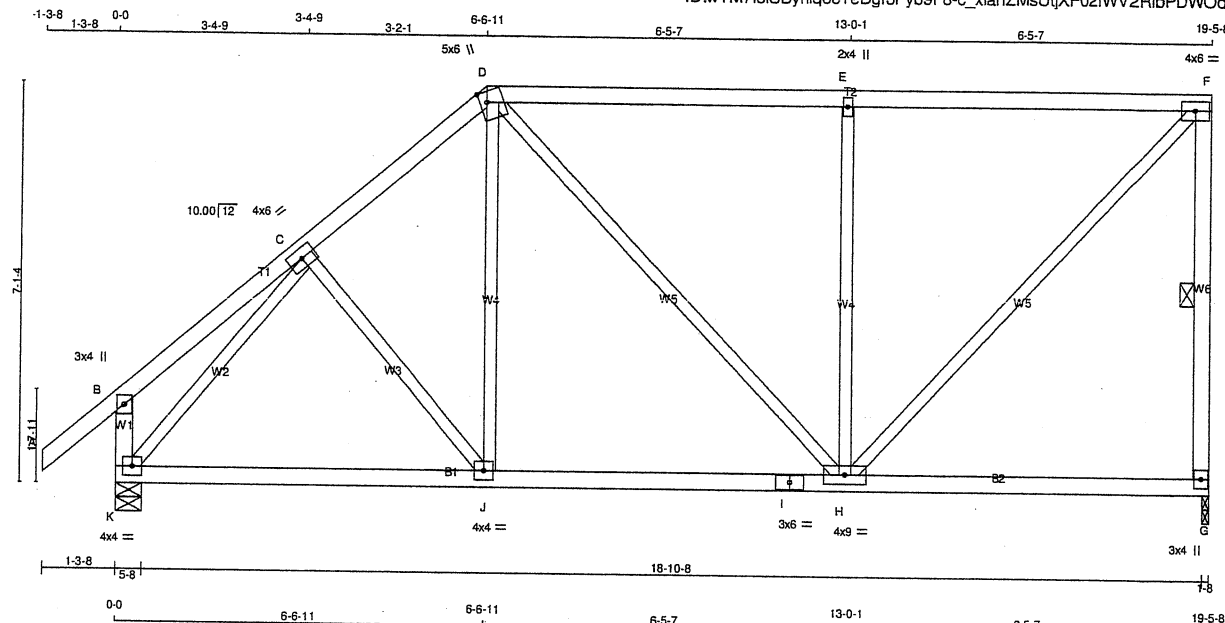


Structural component only
DWG# T-2121152



JSI GRIP= 0.87 (H) (INPUT = 0.90)
JSI METAL= 0.50 (B) (INPUT = 1.00)





LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
G	1073	0	1073	0	1-8	1-8
K	1200	0	1200	0	5-8	5-8

UNFACTORED REACTIONS						
JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
G	759	498 / 0	0 / 0	0 / 0	0 / 0	261 / 0
K	846	569 / 0	0 / 0	0 / 0	0 / 0	277 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.62 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

FR-TO	CHORDS			MAX. UNBRACED LENGTH	FR-TO	WEBS		
	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)			MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
A-B	0 / 41	-91.8	-91.8	0.13 (1)	10.00	C-J	-59 / 20	0.03 (1)
B-C	0 / 21	-91.8	-91.8	0.15 (1)	10.00	J-D	0 / 177	0.05 (4)
C-D	-930 / 0	-91.8	-91.8	0.16 (1)	6.21	D-H	0 / 89	0.02 (1)
D-E	-759 / 0	-91.8	-91.8	0.68 (1)	5.62	H-E	-734 / 0	0.64 (1)
E-F	-759 / 0	-91.8	-91.8	0.69 (1)	5.62	H-F	0 / 1101	0.25 (1)
G-F	-1023 / 0	0.0	0.0	0.23 (1)	6.20	K-C	-1167 / 0	0.50 (1)
K-B	-243 / 0	0.0	0.0	0.03 (1)	7.81			
K-J	0 / 734	-18.5	-18.5	0.24 (4)	10.00			
J-I	0 / 699	-18.5	-18.5	0.24 (4)	10.00			
I-H	0 / 699	-18.5	-18.5	0.24 (4)	10.00			
H-G	0 / 0	-18.5	-18.5	0.17 (4)	10.00			

TOTAL WEIGHT = 4 X 91 = 363 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/999 (0.07")

CSI: TC=0.69/1.00 (E-F:1), BC=0.24/1.00 (H-J:4), WB=0.64/1.00 (E-H:1), SSI=0.29/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

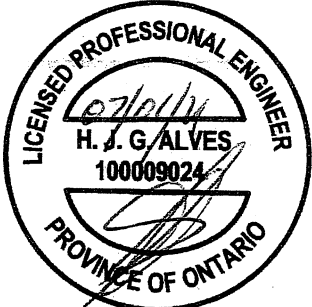
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

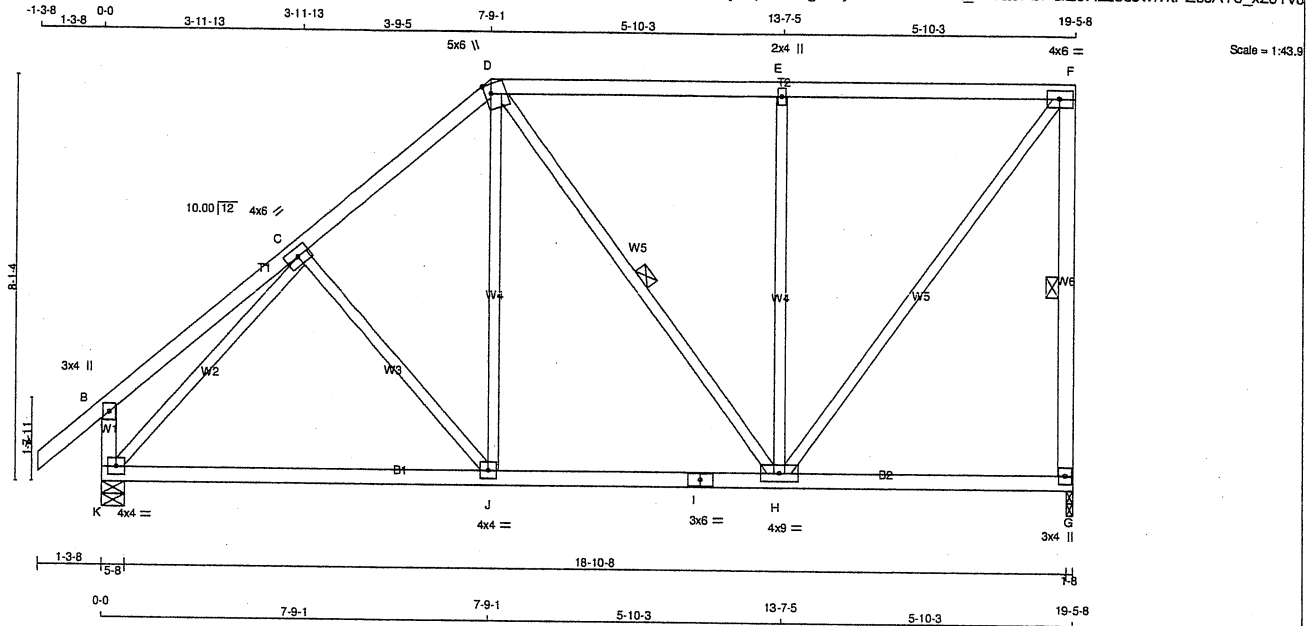
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.88 (K) (INPUT = 0.90)
JSI METAL = 0.26 (C) (INPUT = 1.00)



JOB NAME 412868	TRUSS NAME T5	QUANTITY 16	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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TOTAL WEIGHT = 16 X 96 = 1533 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
K - B	2x4	DRY	No.2	SPF	
K - I	2x4	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMVW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMVWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMVW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT		REQD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	BRG	IN-SX	BRG
G	1073	0	1073	0	0	1-8		1-8	
K	1200	0	1200	0	0	5-8		5-8	

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN	COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	759	498 / 0	0 / 0	0 / 0	0 / 0	0 / 0	261 / 0	0 / 0	0 / 0
K	846	569 / 0	0 / 0	0 / 0	0 / 0	0 / 0	277 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.24 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX LC1 (LC)	UNBRACED LENGTH
FR-TO		FROM TO		FR-TO			
A-B	0 / 41	-91.8	-91.8 0.13 (1)	C-J	-144 / 0	0.09 (1)	10.00
B-C	0 / 26	-91.8	-91.8 0.22 (1)	J-D	0 / 255	0.06 (1)	10.00
C-D	-878 / 0	-91.8	-91.8 0.24 (1)	D-H	-62 / 0	0.04 (1)	6.24
D-E	-618 / 0	-91.8	-91.8 0.55 (1)	H-E	-665 / 0	0.84 (1)	6.25
E-F	-619 / 0	-91.8	-91.8 0.55 (1)	H-F	0 / 1027	0.23 (1)	6.25
G-F	-1026 / 0	0.0	0.0 0.30 (1)	K-C	-1153 / 0	0.68 (1)	6.20
K-B	-262 / 0	0.0	0.0 0.03 (1)				7.81
K-J	0 / 748	-18.5	-18.5 0.29 (4)				10.00
J-I	0 / 656	-18.5	-18.5 0.30 (4)				10.00
I-H	0 / 656	-18.5	-18.5 0.30 (4)				10.00
H-G	0 / 0	-18.5	-18.5 0.13 (4)				10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	25.6	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/999 (0.12")

CSI: TC=0.55/1.00 (E-F:1), BC=0.30/1.00 (H-J:4), WB=0.84/1.00 (E-H:1), SS=0.28/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
	MAX	MIN	MAX MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

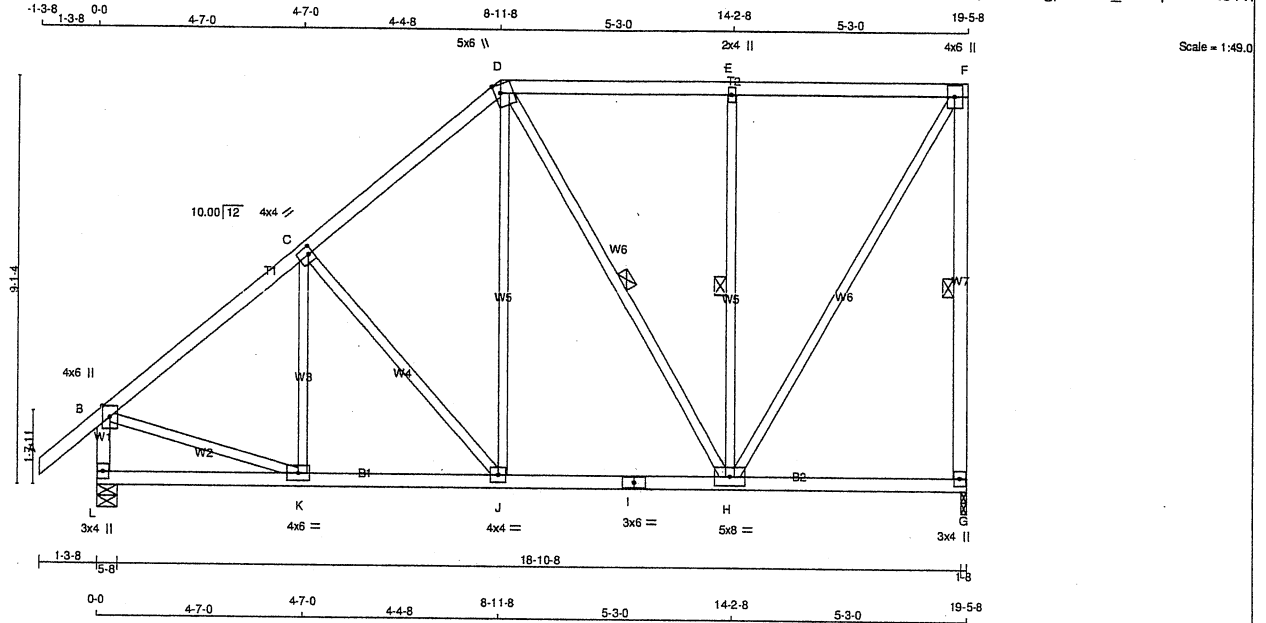
JSI GRIP= 0.88 (K) (INPUT = 0.90)
JSI METAL= 0.26 (C) (INPUT = 1.00)



Structural component only
DWG# T-2121155

JOB NAME 412868	TRUSS NAME T6	QUANTITY 4	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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TOTAL WEIGHT = 4 X 103 = 414 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
G - F	2x4	DRY	No.2
L - B	2x4	DRY	No.2
L - I	2x4	DRY	No.2
I - G	2x4	DRY	No.2

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMVW+p	MT20	4.0	6.0 Edge
C	TMVW-t	MT20	4.0	4.0 2.00 1.25
D	TTWW+m	MT20	5.0	6.0 2.25 1.50
E	TMVW+w	MT20	2.0	4.0
F	TMVW+p	MT20	4.0	6.0
G	BMV1+p	MT20	3.0	4.0
H	BMVWW-t	MT20	5.0	8.0
I	BS-t	MT20	3.0	6.0
J	BMVW-t	MT20	4.0	4.0
K	BMVW-t	MT20	4.0	6.0
L	BMV1+p	MT20	3.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
G	1073	0	1073	0	0	1-8	1-8		
L	1200	0	1200	0	0	5-8	5-8		

UNFACTORED REACTIONS		1ST LCASE		MAX/MIN. COMPONENT REACTIONS		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM.LIVE							
G	759	498 / 0	0 / 0	0 / 0	0 / 0	0 / 0	261 / 0	0 / 0			
L	846	569 / 0	0 / 0	0 / 0	0 / 0	0 / 0	277 / 0	0 / 0			

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.83 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED FORCE		FACTORED VERT. LOAD		MAX. FACTORED FORCE		W E B S		MAX. FACTORED FORCE		MAX. FACTORED FORCE	
MEMB.	FORCE (LBS)	FROM	TO	FROM	TO	MEMB.	FORCE (LBS)	MEMB.	FORCE (LBS)	MEMB.	FORCE (LBS)	MEMB.	FORCE (LBS)
FR-TO						FR-TO		FR-TO		FR-TO		FR-TO	
A-B	0 / 41	-91.8	-91.8	0.13 (1)	10.00	K-C	-132 / 29	0.06 (1)					
B-C	-988 / 0	-91.8	-91.8	0.34 (1)	5.83	C-J	-293 / 0	0.25 (1)					
C-D	-803 / 0	-91.8	-91.8	0.33 (1)	6.25	J-D	0 / 305	0.07 (1)					
D-E	-509 / 0	-91.8	-91.8	0.43 (1)	6.25	D-H	-166 / 0	0.13 (1)					
E-F	-509 / 0	-91.8	-91.8	0.43 (1)	6.25	H-E	-596 / 0	0.32 (1)					
G-F	-1033 / 0	0.0	0.0	0.40 (1)	6.18	H-F	0 / 988	0.22 (1)					
L-B	-1164 / 0	0.0	0.0	0.12 (1)	7.39	B-K	0 / 814	0.18 (1)					
L-K	0 / 0	-18.5	-18.5	0.09 (4)	10.00								
K-J	0 / 783	-18.5	-18.5	0.17 (1)	10.00								
J-I	0 / 594	-18.5	-18.5	0.18 (4)	10.00								
I-H	0 / 594	-18.5	-18.5	0.18 (4)	10.00								
H-G	0 / 0	-18.5	-18.5	0.12 (4)	10.00								

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	25.6	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.43/1.00 (E-F:1), BC=0.18/1.00 (H-J:4), WB=0.32/1.00 (E-H:1), SSI=0.23/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

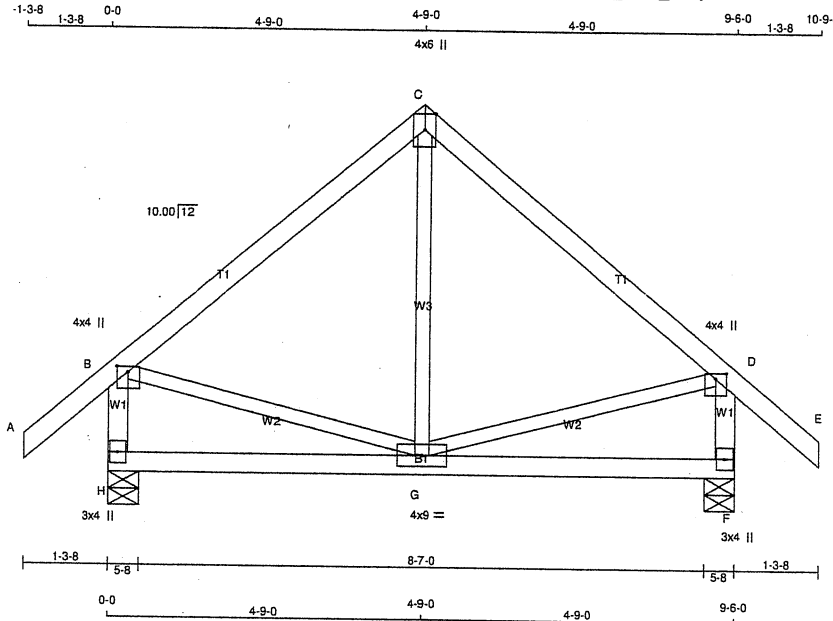
JSI GRIP= 0.77 (B) (INPUT = 0.90)
JSI METAL= 0.50 (B) (INPUT = 1.00)



Structural component only
DWG# T-2121156

JOB NAME 412868	TRUSS NAME T8	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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Scale = 1:33.2

LUMBER			
N.L.G.A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4 DRY	No.2	SPF
C - E	2x4 DRY	No.2	SPF
H - B	2x4 DRY	No.2	SPF
F - D	2x4 DRY	No.2	SPF
H - F	2x4 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMVWW-t	MT20	4.0	9.0		
H	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	HORZ	DOWN	HORZ	BRG	IN-SX	BRG	IN-SX
H	651	0	0	651	0	0	5-8	5-8	5-8
F	651	0	0	651	0	0	5-8	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
H	458	314 / 0	0 / 0	0 / 0	0 / 0	144 / 0	0 / 0
F	458	314 / 0	0 / 0	0 / 0	0 / 0	144 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX. FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. (PLF)	LC1 (LC)	MAX. (LC)	UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX. (LC)	UNBRAC LENGTH
FR-TO		FROM	TO			FR-TO			
A-B	0 / 41	-91.8	-91.8	0.13 (1)	10.00	G-C	-27 / 72	0.02 (4)	
B-C	-319 / 0	-91.8	-91.8	0.27 (1)	6.25	B-G	0 / 254	0.06 (1)	
C-D	-319 / 0	-91.8	-91.8	0.27 (1)	6.25	G-D	0 / 254	0.06 (1)	
D-E	0 / 41	-91.8	-91.8	0.13 (1)	10.00				
H-B	-617 / 0	0.0	0.0	0.07 (1)	7.81				
F-D	-617 / 0	0.0	0.0	0.07 (1)	7.81				
H-G	0 / 0	-18.5	-18.5	0.12 (4)	10.00				
G-F	0 / 0	-18.5	-18.5	0.12 (4)	10.00				

TOTAL WEIGHT = 2 X 43 = 87 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.32")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.32")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.27/1.00 (C-D:1), BC=0.12/1.00 (G-H:4), WB=0.06/1.00 (B-G:1), SSI=0.13/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.50 (B) (INPUT = 0.90)

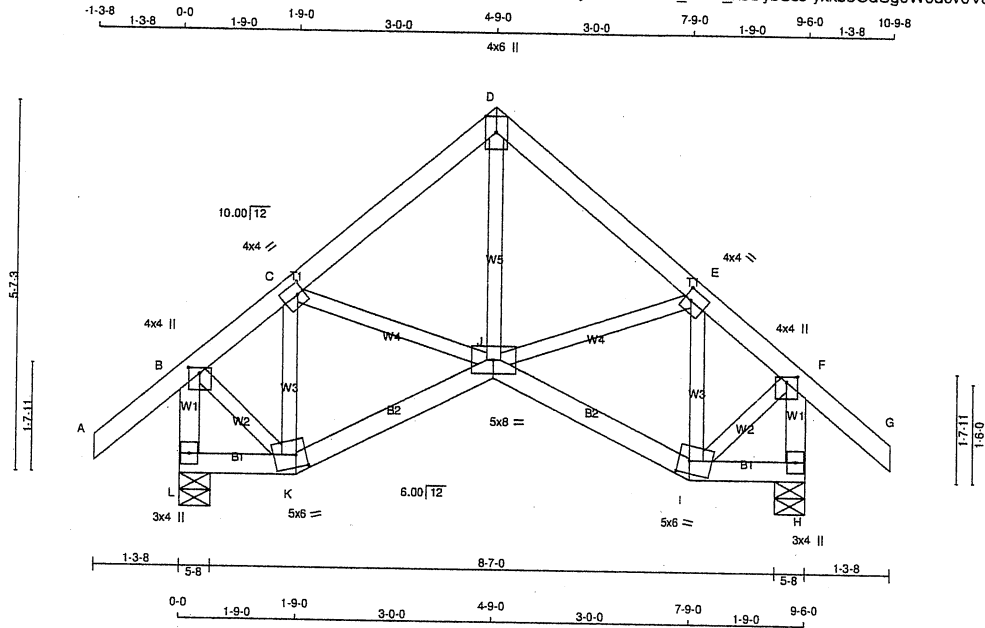
JSI METAL= 0.14 (D) (INPUT = 1.00)



Structural component only
DWG# T-2121159

JOB NAME 412868	TRUSS NAME T9S	QUANTITY 4	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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Scale = 1:33.2

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
L - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
L - K	2x4	DRY	No.2
K - J	2x4	DRY	No.2
J - I	2x4	DRY	No.2
I - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMVW-t	MT20	4.0	4.0	2.00	1.25
D	TTW+p	MT20	4.0	4.0	6.0	Edge
E	TMVW-t	MT20	4.0	4.0	2.00	1.25
F	TMVW+p	MT20	4.0	4.0	1.00	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BBWW-m	MT20	5.0	6.0	2.25	2.00
J	BBWW-p	MT20	5.0	8.0		
K	BBWW-m	MT20	5.0	6.0	2.25	2.00
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQD
JT	VERT	HORZ	DOWN	HORZ
L	651	0	651	0
H	651	0	651	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
L	458	314 / 0	0 / 0	0 / 0	144 / 0	0 / 0
H	458	314 / 0	0 / 0	0 / 0	144 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 41	-91.8 -91.8	0.13 (1)	J-D	0 / 268	0.06 (1)	
B-C	-354 / 0	-91.8 -91.8	0.09 (1)	J-E	0 / 59	0.01 (1)	
C-D	-414 / 0	-91.8 -91.8	0.11 (1)	I-E	-279 / 0	0.05 (1)	
D-E	-414 / 0	-91.8 -91.8	0.11 (1)	C-J	0 / 59	0.01 (1)	
E-F	-354 / 0	-91.8 -91.8	0.09 (1)	K-C	-279 / 0	0.05 (1)	
F-G	0 / 41	-91.8 -91.8	0.13 (1)	B-K	0 / 328	0.07 (1)	
L-B	-635 / 0	0.0 0.0	0.07 (1)	I-F	0 / 328	0.07 (1)	
H-F	-635 / 0	0.0 0.0	0.07 (1)				
L-K	0 / 0	-18.5 -18.5	0.02 (4)				
K-J	0 / 283	-18.5 -18.5	0.07 (1)				
J-I	0 / 283	-18.5 -18.5	0.07 (1)				
I-H	0 / 0	-18.5 -18.5	0.02 (4)				

TOTAL WEIGHT = 4 X 48 = 191 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
DL = 6.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.4	PSF	
TOTAL LOAD = 39.0	PSF	

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.32")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.32")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.13/1.00 (A-B:1), BC=0.07/1.00 (I-J:1), WB=0.07/1.00 (F-I:1), SS=0.10/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)
MT20	650	371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

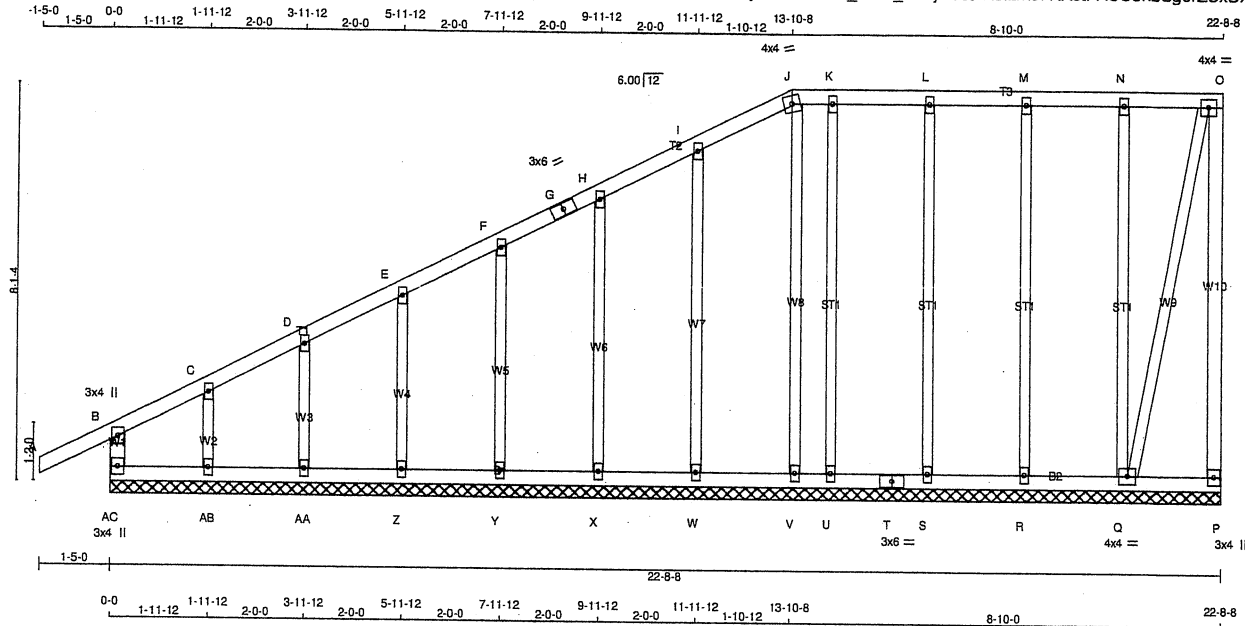
JSI GRIP= 0.44 (F) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)



Structural component only
DWG# T-2121160

JOB NAME 412868	TRUSS NAME T10G	QUANTITY 4	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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TOTAL WEIGHT = 4 X 122 = 488 lb [M]

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
AC - B	2x4 DRY	No.2	SPF
A - G	2x4 DRY	No.2	SPF
G - J	2x4 DRY	No.2	SPF
J - O	2x4 DRY	No.2	SPF
P - O	2x4 DRY	No.2	SPF
AC - T	2x4 DRY	No.2	SPF
T - P	2x4 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF
ALL GABLE WEBS	2x3 DRY	No.2	SPF
DRY: SEASONED LUMBER.			
GABLE STUDS SPACED AT 2-0-0 OC.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	MAX. FACTORED FORCE (LBS)
FR-TO				FR-TO			
AC-B	-271 / 0	0.0	0.03 (1)	Q-N	-200 / 0	7.81	0.25 (1)
A-B	0 / 30	-91.8	-91.8 0.14 (1)	R-M	-182 / 0	10.00	0.23 (1)
B-C	-21 / 0	-91.8	-91.8 0.14 (1)	S-L	-190 / 0	6.25	0.24 (1)
C-D	0 / 10	-91.8	-91.8 0.05 (1)	U-K	-149 / 0	10.00	0.19 (1)
D-E	0 / 9	-91.8	-91.8 0.05 (1)	V-J	-95 / 0	10.00	0.12 (1)
E-F	0 / 14	-91.8	-91.8 0.04 (1)	AB-C	-118 / 0	10.00	0.02 (1)
F-G	0 / 16	-91.8	-91.8 0.04 (1)	AA-D	-196 / 0	10.00	0.04 (1)
G-H	0 / 16	-91.8	-91.8 0.04 (1)	Z-E	-179 / 0	10.00	0.05 (1)
H-I	0 / 19	-91.8	-91.8 0.05 (1)	Y-F	-183 / 0	10.00	0.07 (1)
I-J	0 / 13	-91.8	-91.8 0.05 (1)	X-H	-180 / 0	10.00	0.11 (1)
J-K	0 / 20	-91.8	-91.8 0.03 (1)	W-I	-200 / 0	10.00	0.18 (1)
K-L	0 / 20	-91.8	-91.8 0.05 (1)	Q-O	-79 / 0	10.00	0.11 (1)
L-M	0 / 20	-91.8	-91.8 0.05 (1)				
M-N	0 / 20	-91.8	-91.8 0.05 (1)				
N-O	0 / 20	-91.8	-91.8 0.05 (1)				
P-O	0 / 0	0.0	0.0 0.00 (1)				
AC-AB	0 / 0	-18.5	-18.5 0.02 (4)				
AB-AA	-4 / 0	-18.5	-18.5 0.02 (4)				
AA-Z	-9 / 0	-18.5	-18.5 0.01 (4)				
Z-Y	-12 / 0	-18.5	-18.5 0.01 (4)				
Y-X	-14 / 0	-18.5	-18.5 0.01 (4)				
X-W	-16 / 0	-18.5	-18.5 0.02 (4)				
W-V	-18 / 0	-18.5	-18.5 0.02 (4)				
V-U	-20 / 0	-18.5	-18.5 0.01 (4)				
U-T	-20 / 0	-18.5	-18.5 0.02 (4)				
T-S	-20 / 0	-18.5	-18.5 0.02 (4)				
S-R	-20 / 0	-18.5	-18.5 0.02 (4)				
R-Q	-20 / 0	-18.5	-18.5 0.02 (4)				
Q-P	0 / 0	-18.5	-18.5 0.02 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (A-B:1), BC=0.02/1.00 (Q-R:4),
WB=0.25/1.00 (N-Q:1), SS=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

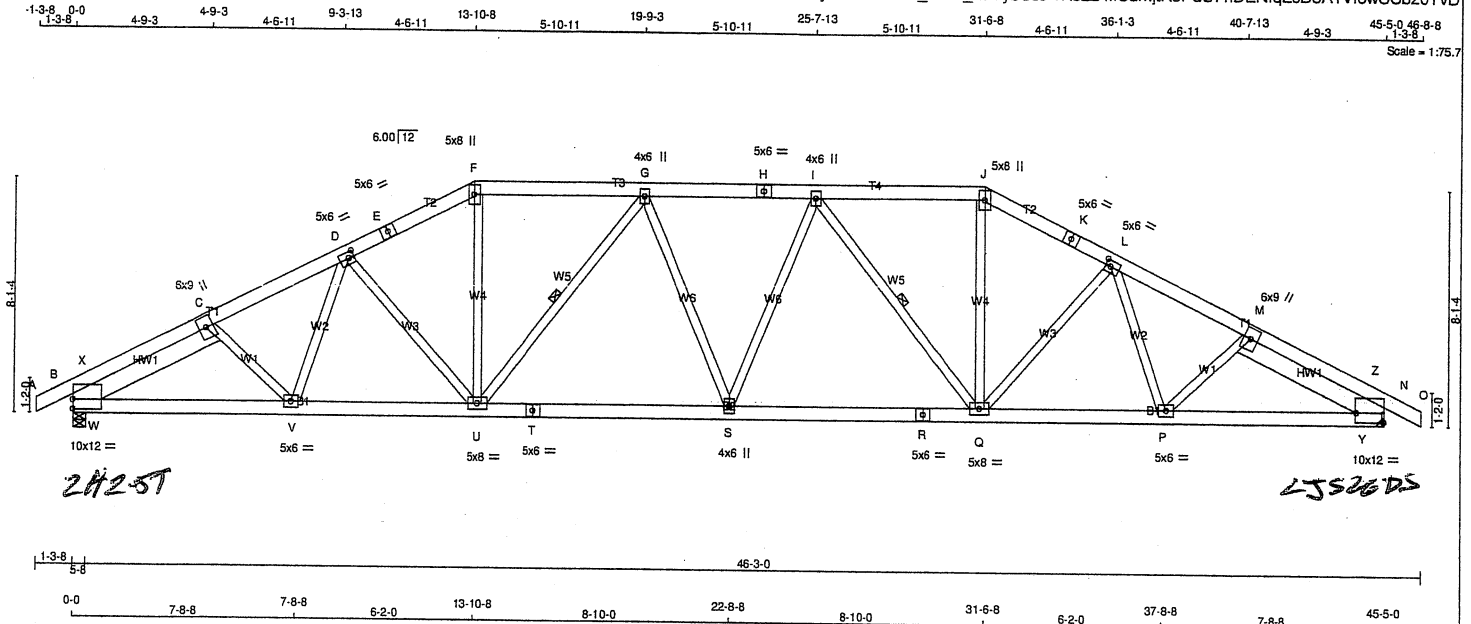
JSI GRIP = 0.59 (J) (INPUT = 0.90)
JSI METAL = 0.08 (I) (INPUT = 1.00)



Structural component only
DWG# T-2121161

JOB NAME 412868	TRUSS NAME T11	QUANTITY 15	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER		
A - E	2x6	DRY	No.2	SPF	
E - F	2x6	DRY	No.2	SPF	
F - H	2x6	DRY	No.2	SPF	
H - J	2x6	DRY	No.2	SPF	
J - K	2x6	DRY	No.2	SPF	
K - O	2x6	DRY	No.2	SPF	
B - T	2x6	DRY	No.2	SPF	
T - R	2x6	DRY	No.2	SPF	
R - N	2x6	DRY	No.2	SPF	
REINFORCING MEMBERS				DESCR.	
HW1	2x8	DRY	No.2	SPF	
HW2	2x8	DRY	No.2	SPF	
ALL WEBS				DESCR.	
2x4	DRY	No.2	SPF		
DRY: SEASONED LUMBER.					

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B TMBMW1m	MT20	10.0	12.0	4.00
C TMBWW-t	MT20	6.0	9.0	
D TMBWW-t	MT20	5.0	6.0	2.50 2.25
E H, K				
E TS-t	MT20	5.0	6.0	
F TTW+p	MT20	5.0	8.0	
G TMBWW-t	MT20	4.0	6.0	
I TMBWW-t	MT20	4.0	6.0	
J TTW+p	MT20	5.0	8.0	
L TMBWW-t	MT20	5.0	6.0	2.50 2.25
M TMBWW-t	MT20	6.0	9.0	
N TMBMW1m	MT20	10.0	12.0	4.00 Edge
P BWW-t	MT20	5.0	6.0	
Q BWWWW-t	MT20	5.0	8.0	
R BS-t	MT20	5.0	6.0	
S BWWWW-t	MT20	4.0	6.0	
T BS-t	MT20	5.0	6.0	
U BWWWW-t	MT20	5.0	8.0	
V BWWWW-t	MT20	5.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
B	3675	0	3709	195	-1082
N	3675	0	3709	0	-1082

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT N. MINIMUM BEARING LENGTH AT JOINT N = 4-0.

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 1082 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT N FOR 1082 LBS FACTORED UPLIFT

PROVIDE FOR 195 LBS FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS

JT	1ST CASE	MAX	MIN	COMPONENT REACTIONS	
	COMBINED	SNOW	LIVE	PERM LIVE	WIND DEAD SOIL
B	2713	1634 / 0	477 / 0	0 / 0	0 / -1073 625 / 0 0 / 0
N	2713	1634 / 0	477 / 0	0 / 0	59 / -1073 625 / 0 0 / 0

HORIZONTAL REACTIONS					
B	0 / 0	0 / 0	0 / 0	139 / -139	0 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 3.53 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-U, I-Q.

LOADING

TOTAL LOAD CASES: (18)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 1	C-V	-32 / 371
B-X	-4338 / 1145	D-V	-90 / 264
X-C	-2853 / 891	D-U	-826 / 383
C-D	-5520 / 1704	U-F	-479 / 1736
D-E	-5000 / 1604	U-G	-1448 / 490
E-F	-5000 / 1604	G-S	-167 / 458
F-G	-4482 / 1504	S-I	-167 / 458
G-H	-5228 / 1649	I-Q	-1448 / 490
H-I	-5228 / 1649	Q-J	-479 / 1736
I-J	-4482 / 1504	Q-L	-826 / 383
J-K	-5000 / 1604	L-P	-100 / 264
K-L	-5000 / 1604	P-M	-44 / 371
L-M	-5520 / 1705	W-X	-379 / 1962
M-Z	-2853 / 893	W-C	-2918 / 847
Z-N	-4338 / 1148	M-Y	-2918 / 846
N-O	0 / 1	Y-Z	-381 / 1962
B-W	-856 / 2550		
W-V	-1532 / 4864		
V-U	-1432 / 4893		
U-T	-1381 / 5160		
T-S	-1381 / 5160		
S-R	-1327 / 5160		
R-Q	-1327 / 5160		
Q-P	-1237 / 4893		
P-Y	-1338 / 4864		
Y-N	-664 / 2550		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 33.4	PSF
DL = 8.0	PSF	
BOT CH.	LL = 10.5	PSF
DL = 7.4	PSF	
TOTAL LOAD = 57.3	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 4 OF CBC 2018, ABC 2019
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- SLOPE REDUCTION FACTOR NOT USED
- PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED.

(80 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 33.4 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.51")
CALCULATED VERT. DEFL.(LL) = L/999 (0.24")
ALLOWABLE DEFL.(TL) = L/180 (3.03")
CALCULATED VERT. DEFL.(TL) = L/999 (0.34")

CSI: TC=0.41/1.00 (G-I:1), BC=0.76/1.00 (S-U:1), WB=0.65/1.00 (G-U:3), SS=0.24/1.00 (I-J:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX MIN
MT20	650	371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (T) (INPUT = 0.90)
JSI METAL= 0.92 (T) (INPUT = 1.00)



Structural component only
DWG# T-2121162

JOB NAME 412868	TRUSS NAME T11	QUANTITY 15	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

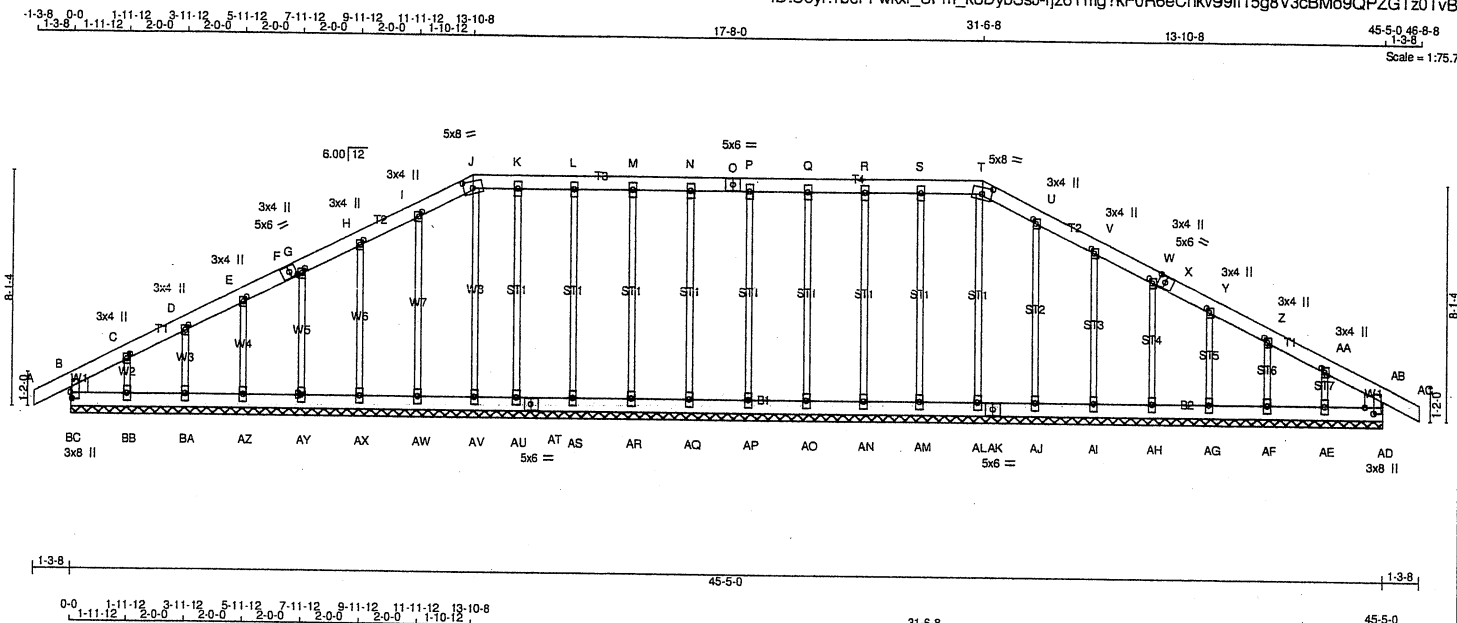
TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.2} PSF AT
{31-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK
COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL
WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON
(OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY
FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF
AND 5.0 PSF RESPECTIVELY.



JOB NAME 412868	TRUSS NAME T11G	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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LUMBER				N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.	CHORDS	SIZE	LUMBER	DESCR.
BC- B	2x8	DRY	No.2	SPF			
A- F	2x6	DRY	No.2	SPF			
F- J	2x6	DRY	No.2	SPF			
J- O	2x6	DRY	No.2	SPF			
O- T	2x6	DRY	No.2	SPF			
T- X	2x6	DRY	No.2	SPF			
X- AC	2x6	DRY	No.2	SPF			
AD- AB	2x8	DRY	No.2	SPF			
BC- AT	2x6	DRY	No.2	SPF			
AT- AK	2x6	DRY	No.2	SPF			
AK- AD	2x6	DRY	No.2	SPF			
ALL WEBS	2x3	DRY	No.2	SPF			
ALL GABLE WEBS	2x3	DRY	No.2	SPF			
DRY: SEASONED LUMBER.							

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B, AB, AD, BC					
C, D, E, G, H, I, U, V, W, Y, Z, AA					
C TMW+w	MT20	3.0	4.0	2.00	1.25
F TS-t	MT20	5.0	6.0	2.50	2.75
J TTW-m	MT20	5.0	8.0	2.75	4.00
K, L, M, N, P, Q, R, S					
K TMW+w	MT20	3.0	6.0		
O TS-t	MT20	5.0	6.0		
T TTW-m	MT20	5.0	8.0	2.75	4.00
X TS-t	MT20	5.0	6.0	2.50	2.75
AD TMBMV1+p	MT20	3.0	8.0	2.50	3.75
AE, AF, AG, AH, AI, AJ, AL, AM, AN, AO, AP, AQ, AR, AS, AU, AV, AW, AX, AY, AZ, BA, BB					
AE BMW1+w	MT20	3.0	6.0		
AK BS-t	MT20	5.0	6.0		
AT BS-t	MT20	5.0	6.0		
BC TMBMV1+p	MT20	3.0	8.0	2.50	0.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MAX. (PLF)	MEMB.	FORCE (LBS)	MAX. (PLF)	MAX. (LC)
FR-TO				FR-TO			
BC-B	-260 / 0	0.0	0.0 0.02 (1)	AL-T	-186 / 0	0.21 (1)	
A-B	0 / 29	-91.8	-91.8 0.06 (1)	AM-S	-190 / 0	0.21 (1)	
B-C	-50 / 0	-91.8	-91.8 0.05 (1)	AN-R	-183 / 0	0.21 (1)	
C-D	-24 / 0	-91.8	-91.8 0.02 (1)	AO-Q	-183 / 0	0.21 (1)	
D-E	-19 / 0	-91.8	-91.8 0.02 (1)	AP-P	-183 / 0	0.21 (1)	
E-F	-13 / 0	-91.8	-91.8 0.02 (1)	AQ-N	-183 / 0	0.21 (1)	
F-G	-13 / 0	-91.8	-91.8 0.02 (1)	AR-M	-184 / 0	0.21 (1)	
G-H	-9 / 0	-91.8	-91.8 0.02 (1)	AS-L	-184 / 0	0.21 (1)	
H-I	-5 / 0	-91.8	-91.8 0.02 (1)	AU-K	-163 / 0	0.18 (1)	
I-J	-3 / 0	-91.8	-91.8 0.02 (1)	AJ-U	-183 / 0	0.14 (1)	
J-K	0 / 0	-91.8	-91.8 0.02 (1)	AI-V	-182 / 0	0.09 (1)	
K-L	0 / 0	-91.8	-91.8 0.02 (1)	AH-W	-182 / 0	0.06 (1)	
L-M	0 / 0	-91.8	-91.8 0.02 (1)	AG-Y	-180 / 0	0.04 (1)	
M-N	0 / 0	-91.8	-91.8 0.02 (1)	AF-Z	-188 / 0	0.03 (1)	
N-O	0 / 0	-91.8	-91.8 0.02 (1)	AE-AA	-137 / 0	0.02 (1)	
O-P	0 / 0	-91.8	-91.8 0.02 (1)	AV-J	-159 / 0	0.18 (1)	
P-Q	0 / 0	-91.8	-91.8 0.02 (1)	BB-C	-137 / 0	0.02 (1)	
Q-R	0 / 0	-91.8	-91.8 0.02 (1)	BA-D	-188 / 0	0.03 (1)	
R-S	0 / 0	-91.8	-91.8 0.02 (1)	AZ-E	-180 / 0	0.04 (1)	
S-T	0 / 0	-91.8	-91.8 0.02 (1)	AY-G	-180 / 0	0.06 (1)	
T-U	-2 / 0	-91.8	-91.8 0.02 (1)	AX-H	-183 / 0	0.10 (1)	
U-V	-5 / 0	-91.8	-91.8 0.02 (1)	AW-I	-184 / 0	0.14 (1)	
V-W	-9 / 0	-91.8	-91.8 0.02 (1)				
W-X	-14 / 0	-91.8	-91.8 0.02 (1)				
X-Y	-14 / 0	-91.8	-91.8 0.02 (1)				
Y-Z	-20 / 0	-91.8	-91.8 0.02 (1)				
Z-AA	-24 / 0	-91.8	-91.8 0.02 (1)				
AA-AB	-51 / 0	-91.8	-91.8 0.05 (1)				
AB-AC	0 / 29	-91.8	-91.8 0.06 (1)				
AD-AB	-260 / 0	0.0	0.0 0.02 (1)				
BC-BB	0 / 33	-18.5	-18.5 0.03 (1)				
BB-BA	0 / 24	-18.5	-18.5 0.01 (1)				
BA-AZ	0 / 17	-18.5	-18.5 0.01 (4)				
AZ-AY	0 / 12	-18.5	-18.5 0.01 (4)				
AY-AX	0 / 8	-18.5	-18.5 0.01 (4)				
AX-AW	0 / 5	-18.5	-18.5 0.01 (4)				
AW-AV	0 / 2	-18.5	-18.5 0.01 (4)				
AV-AU	0 / 0	-18.5	-18.5 0.01 (4)				
AU-AT	0 / 0	-18.5	-18.5 0.01 (4)				
AT-AS	0 / 0	-18.5	-18.5 0.01 (4)				
AS-AR	0 / 0	-18.5	-18.5 0.01 (4)				
AR-AQ	0 / 0	-18.5	-18.5 0.01 (4)				
AQ-AP	0 / 0	-18.5	-18.5 0.01 (4)				
AP-AO	0 / 0	-18.5	-18.5 0.01 (4)				
AO-AN	0 / 0	-18.5	-18.5 0.01 (4)				
AN-AM	0 / 0	-18.5	-18.5 0.01 (4)				
AM-AL	0 / 0	-18.5	-18.5 0.01 (4)				
AL-AK	0 / 2	-18.5	-18.5 0.01 (4)				
AK-AJ	0 / 2	-18.5	-18.5 0.01 (4)				
AJ-AI	0 / 5	-18.5	-18.5 0.01 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.06/1.00 (AB-AC:1), BC=0.03/1.00 (AD-AE:1), WB=0.21/1.00 (S-AM:1), SSI=0.07/1.00 (AB-AC:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
MIN	MAX	MIN
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (J) (INPUT = 0.90)

JSI METAL= 0.09 (AD) (INPUT = 1.00)



Structural component only
DWG# T-2121163

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
412868	T11G	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

LOADING

TOTAL LOAD CASES: (4)

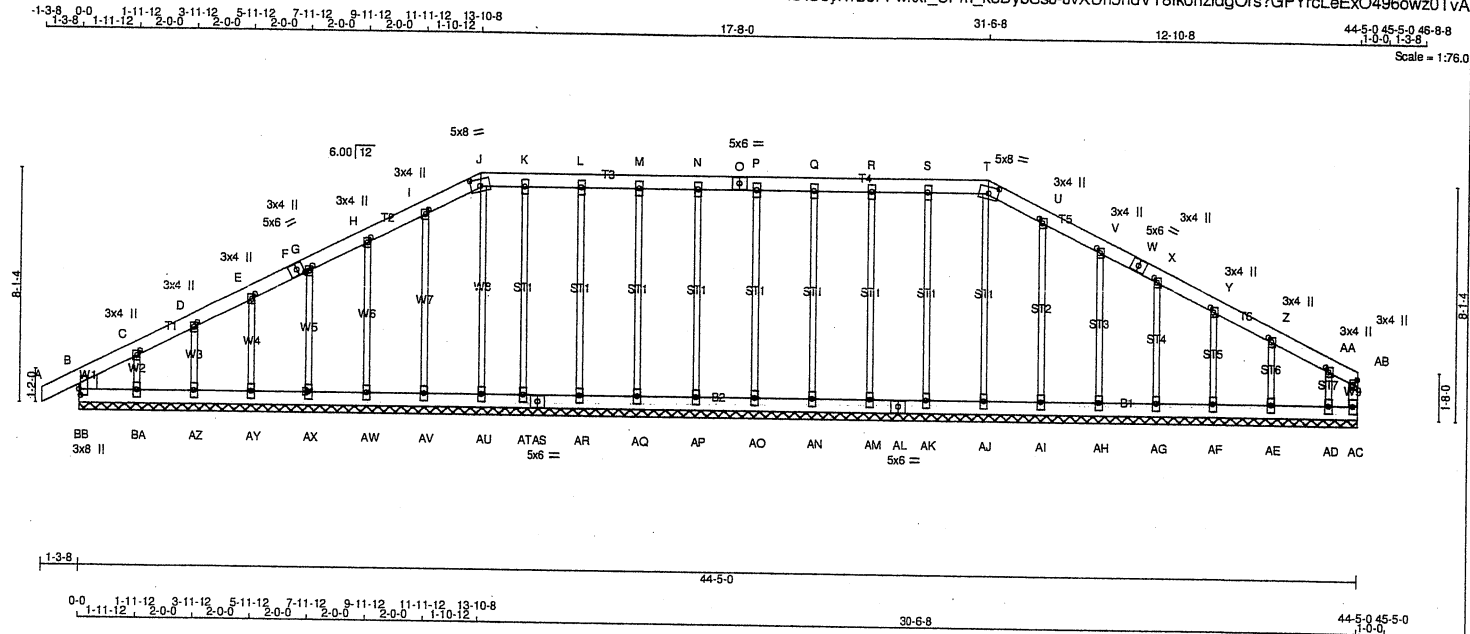
CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX UNBRAC LENGTH	FR-TO	MAX CSI (LC)
FR-TO		FROM	TO						
AI-AH	0 / 8	-18.5	-18.5	0.01 (4)			10.00		
AH-AG	0 / 12	-18.5	-18.5	0.01 (4)			10.00		
AG-AF	0 / 17	-18.5	-18.5	0.01 (4)			10.00		
AF-AE	0 / 25	-18.5	-18.5	0.01 (1)			10.00		
AE-AD	0 / 33	-18.5	-18.5	0.03 (1)			10.00		



Structural component only
DWG# T-2121163 *mm*

JOB NAME 412868	TRUSS NAME T11GA	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER DESCR.
BB- B 2x8 DRY No.2 SPF
A - F 2x6 DRY No.2 SPF
F - J 2x6 DRY No.2 SPF
J - O 2x6 DRY No.2 SPF
O - T 2x6 DRY No.2 SPF
T - W 2x6 DRY No.2 SPF
W - AB 2x6 DRY No.2 SPF
BB- AS 2x6 DRY No.2 SPF
AS- AL 2x6 DRY No.2 SPF
AL- AC 2x6 DRY No.2 SPF
ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
AC- AB 2x4 DRY No.2 SPF
ALL GABLE WEBS 2x3 DRY No.2 SPF
DRY: SEASONED LUMBER.
GABLE STUDS SPACED AT 2'-0" OC.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
PROVIDE ANCHORAGE AT BEARING JOINT AC FOR 150 LBS. FACTORED UPLIFT BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S).
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.
LOADING
TOTAL LOAD CASES: (4)

DESIGN CRITERIA
SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF
SPACING = 24.0 IN./C
LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015
THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.
(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD
CSI: TC=0.06/1.00 (A-B:1), BC=0.01/1.00 (AZ-BA:4), WB=0.24/1.00 (S-AK:1), SSI=0.07/1.00 (S-T:1)
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10
COMPANION LIVE LOAD FACTOR = 1.00
AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873
PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP= 0.29 (AB) (INPUT = 0.90)
JSI METAL= 0.06 (BB) (INPUT = 1.00)

CHORDS	MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	FACTORED MAX. (LC)	WEBS	MEMB.	FORCE (LBS)	MAX. FACTORED (LC)
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FR-TO	BB-B	-228 / 0	0.0	0.0 0.01 (1)	7.81	AJ-T	-168 / 0	0.19 (1)
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A-B	0 / 29	-91.8	-91.8 0.06 (1)	10.00	AK-S	-212 / 0	0.24 (1)
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B-C	-6 / 0	-91.8	-91.8 0.06 (1)	10.00	AM-R	-180 / 0	0.20 (1)
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C-D	0 / 3	-91.8	-91.8 0.04 (1)	10.00	AN-Q	-182 / 0	0.20 (1)
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D-E	0 / 22	-91.8	-91.8 0.02 (1)	10.00	AO-P	-184 / 0	0.21 (1)
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E-F	0 / 25	-91.8	-91.8 0.02 (1)	10.00	AP-N	-183 / 0	0.21 (1)
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F-G	0 / 25	-91.8	-91.8 0.02 (1)	10.00	AQ-M	-184 / 0	0.21 (1)
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G-H	0 / 30	-91.8	-91.8 0.02 (1)	10.00	AR-L	-190 / 0	0.21 (1)
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H-I	0 / 34	-91.8	-91.8 0.03 (1)	10.00	AT-K	-169 / 0	0.19 (1)
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I-J	0 / 30	-91.8	-91.8 0.02 (1)	10.00	AI-U	-200 / 0	0.16 (1)
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J-K	0 / 35	-91.8	-91.8 0.02 (1)	10.00	AH-V	-181 / 0	0.09 (1)
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K-L	0 / 35	-91.8	-91.8 0.02 (1)	10.00	AG-X	-181 / 0	0.06 (1)
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L-M	0 / 35	-91.8	-91.8 0.02 (1)	10.00	AF-Y	-179 / 0	0.04 (1)
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M-N	0 / 35	-91.8	-91.8 0.02 (1)	10.00	AE-Z	-183 / 0	0.03 (1)
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N-O	0 / 35	-91.8	-91.8 0.02 (1)	10.00	AD-AA	-166 / 0	0.02 (1)
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O-P	0 / 35	-91.8	-91.8 0.02 (1)	10.00	AU-J	-152 / 0	0.17 (1)
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P-Q	0 / 35	-91.8	-91.8 0.02 (1)	10.00	BA-C	-181 / 0	0.03 (1)
-----	--------	-------	----------------	-------	------	----------	----------

Q-R	0 / 35	-91.8	-91.8 0.03 (1)	10.00	AZ-D	-153 / 0	0.03 (1)
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R-S	0 / 35	-91.8	-91.8 0.03 (1)	10.00	AY-E	-187 / 0	0.04 (1)
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S-T	0 / 35	-91.8	-91.8 0.03 (1)	10.00	AX-G	-181 / 0	0.06 (1)
-----	--------	-------	----------------	-------	------	----------	----------

T-U	0 / 29	-91.8	-91.8 0.03 (1)	10.00	AW-H	-182 / 0	0.09 (1)
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U-V	0 / 34	-91.8	-91.8 0.03 (1)	10.00	AV-I	-198 / 0	0.16 (1)
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V-W	0 / 30	-91.8	-91.8 0.02 (1)	10.00	AC-AB	0 / 3	0.00 (1)
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W-X	0 / 30	-91.8	-91.8 0.02 (1)	10.00			
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Structural component only
DWG# T-2121164

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
412868	T11GA	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LOADING

TOTAL LOAD CASES: (4)

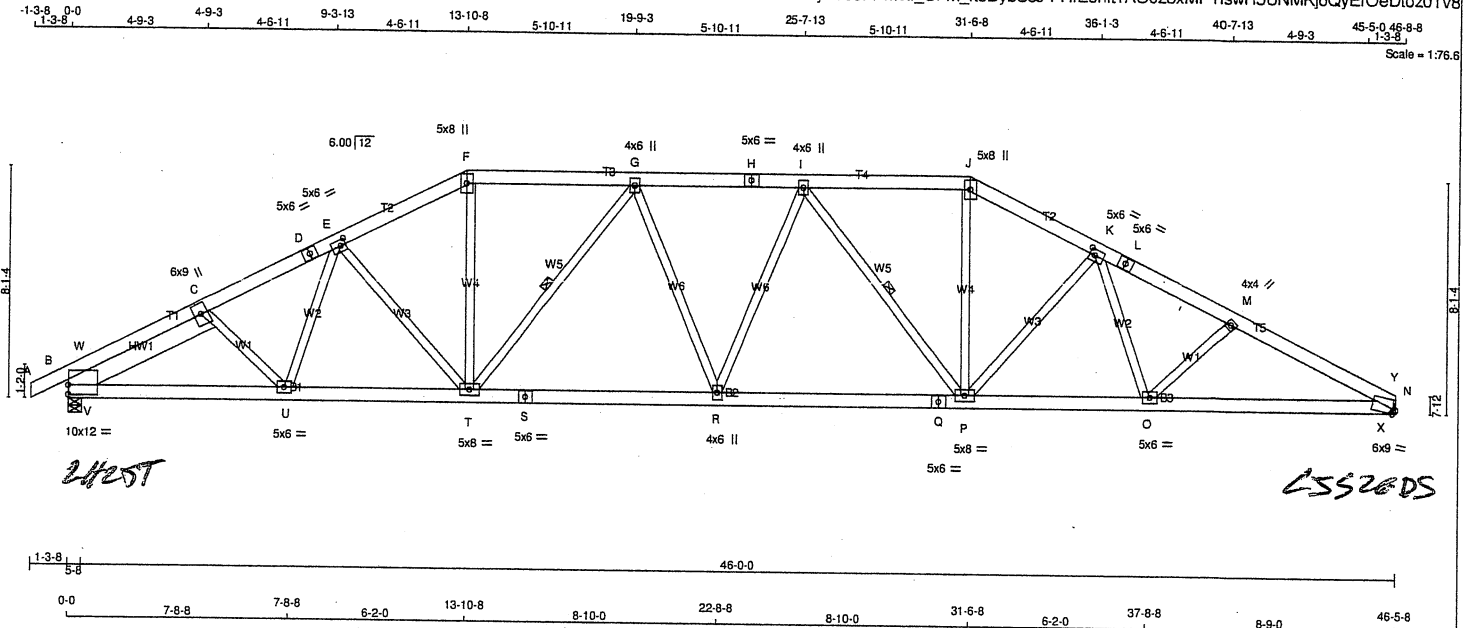
CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			FR-TO		
AH-AG	-26 / 0	-18.5 -18.5	0.01 (4)	6.25			
AG-AF	-22 / 0	-18.5 -18.5	0.01 (4)	6.25			
AF-AE	-17 / 0	-18.5 -18.5	0.01 (4)	6.25			
AE-AD	-10 / 0	-18.5 -18.5	0.01 (4)	6.25			
AD-AC	0 / 0	-18.5 -18.5	0.01 (4)	10.00			



Structural component only
DWG# T-2121164 *ML*

JOB NAME 412868	TRUSS NAME T11X	QUANTITY 3	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER		
A - D	2x6	DRY	No.2	SPF	
D - F	2x6	DRY	No.2	SPF	
F - H	2x6	DRY	No.2	SPF	
H - J	2x6	DRY	No.2	SPF	
J - L	2x6	DRY	No.2	SPF	
L - N	2x6	DRY	No.2	SPF	
B - S	2x6	DRY	No.2	SPF	
S - Q	2x6	DRY	No.2	SPF	
Q - N	2x6	DRY	No.2	SPF	

REINFORCING MEMBERS					
HW1	2x8	DRY	No.2	SPF	
ALL WEBS	2x4	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMBMW1-t	MT20	10.0	12.0	4.00
C	TMBWW-t	MT20	6.0	9.0	
D, H, L					
D	TS-t	MT20	5.0	6.0	
E	TMBWW-t	MT20	5.0	6.0	2.50 2.25
F	TTW-p	MT20	5.0	8.0	
G	TMBWW-t	MT20	4.0	6.0	
I	TMBWW-t	MT20	4.0	6.0	
J	TTW-p	MT20	5.0	8.0	
K	TMBWW-t	MT20	5.0	6.0	2.50 2.25
M	TMBWW-t	MT20	4.0	4.0	
N	TMBWW-t	MT20	6.0	9.0	Edge 1.00
O	BMWW-t	MT20	5.0	6.0	
P	BMWW-t	MT20	5.0	8.0	
Q	BS-t	MT20	5.0	6.0	
R	BMWW-t	MT20	4.0	6.0	
S	BS-t	MT20	5.0	6.0	
T	BMWW-t	MT20	5.0	8.0	
U	BMWW-t	MT20	5.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	JT	GROSS REACTION	BRG	BRG	BRG	BRG
B	VERT 3755	DOWN 3763	HORZ 195	UPLIFT -1100	IN-SX 5-8	IN-SX 5-8	MECHANICAL
N	3595	0	3622	0	-1050	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT N. MINIMUM BEARING LENGTH AT JOINT N = 3-8.

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 1100 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT N FOR 1050 LBS. FACTORED UPLIFT

PROVIDE FOR 195 LBS. FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS

1ST LC CASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	2773	1651 / 0	488 / 0	0 / 0	0 / -1093	639 / 0	0 / 0
N	2663	1571 / 0	488 / 0	0 / 0	77 / -1049	623 / 0	0 / 0

HORIZONTAL REACTIONS		MAX. FACTORED		MIN. FACTORED	
JT	REACTION	MAX. FACTORED	MIN. FACTORED	MAX. FACTORED	MIN. FACTORED
B	0 / 0	0 / 0	0 / 0	139 / -138	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 2.87 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.10 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-T, I-P.

LOADING

TOTAL LOAD CASES: (18)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 1	-115.2 -115.2	0.09 (2)	10.00	C-U	-35 / 387	0.06 (3)
B-W	-4428 / 1167	-115.2 -115.2	0.18 (2)	4.03	U-E	-101 / 257	0.04 (5)
W-C	-2902 / 905	-115.2 -115.2	0.15 (2)	4.83	E-T	-817 / 380	0.53 (2)
C-D	-5672 / 1739	-115.2 -115.2	0.33 (2)	3.50	T-F	-495 / 1807	0.39 (13)
D-E	-5672 / 1739	-115.2 -115.2	0.33 (2)	3.50	F-G	-1548 / 519	0.70 (3)
E-F	-5164 / 1642	-115.2 -115.2	0.29 (2)	3.67	G-R	-121 / 543	0.12 (9)
F-G	-4630 / 1538	-115.2 -115.2	0.37 (1)	3.76	R-I	-242 / 399	0.24 (10)
G-H	-5466 / 1719	-115.2 -115.2	0.42 (1)	3.45	I-P	-1378 / 465	0.82 (2)
H-I	-5466 / 1719	-115.2 -115.2	0.42 (1)	3.45	P-J	-528 / 1903	0.42 (14)
I-J	-4815 / 1602	-115.2 -115.2	0.38 (1)	3.69	J-K	-1143 / 470	0.74 (3)
J-K	-5371 / 1713	-115.2 -115.2	0.31 (3)	3.59	K-O	-151 / 651	0.10 (3)
K-L	-6250 / 1913	-115.2 -115.2	0.38 (3)	3.30	O-M	-532 / 311	0.11 (3)
L-M	-6250 / 1913	-115.2 -115.2	0.38 (3)	3.30	M-W	-391 / 2013	0.00 (1)
M-Y	-6641 / 1989	-115.2 -115.2	0.55 (1)	3.02	V-C	-3008 / 867	0.50 (1)
Y-N	-7145 / 1894	-115.2 -115.2	0.52 (1)	2.87	X-Y	0 / 871	0.00 (1)
B-V	-869 / 2593	-39.5 -39.5	0.33 (1)	6.25			
V-U	-1561 / 4989	-39.5 -39.5	0.71 (1)	6.25			
U-T	-1464 / 5032	-39.5 -39.5	0.72 (1)	6.25			
T-S	-1429 / 5368	-39.5 -39.5	0.78 (1)	6.25			
S-R	-1429 / 5368	-39.5 -39.5	0.78 (1)	6.25			
R-Q	-1389 / 5430	-39.5 -39.5	0.79 (1)	6.25			
Q-P	-1389 / 5430	-39.5 -39.5	0.79 (1)	6.25			
P-O	-1391 / 5427	-39.5 -39.5	0.78 (1)	6.25			
O-X	-1657 / 5910	-39.5 -39.5	0.86 (1)	6.10			
X-N	-1657 / 5910	-39.5 -39.5	0.86 (1)	6.13			

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2 (B)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	33.4	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	57.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 4 OF CBC 2018, ABC 2019
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- SLOPE REDUCTION FACTOR USED
- PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED.

(80 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 33.4 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.55")
CALCULATED VERT. DEFL.(LL) = L/999 (0.29")
ALLOWABLE DEFL.(TL) = L/180 (3.10")
CALCULATED VERT. DEFL.(TL) = L/999 (0.41")

CSI: TC=0.55/1.00 (M-Y:1), BC=0.86/1.00 (O-X:1), WB=0.74/1.00 (K-P:3), SSI=0.40/1.00 (N-Y:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)	MAX MIN	MAX MIN	MAX MIN
MT20	650	371	1747	788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (Q) (INPUT = 0.90)
JSI METAL= 0.97 (Q) (INPUT = 1.00)



Structural component only
DWG# T-2121165

JOB NAME 412868	TRUSS NAME T11X	QUANTITY 3	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

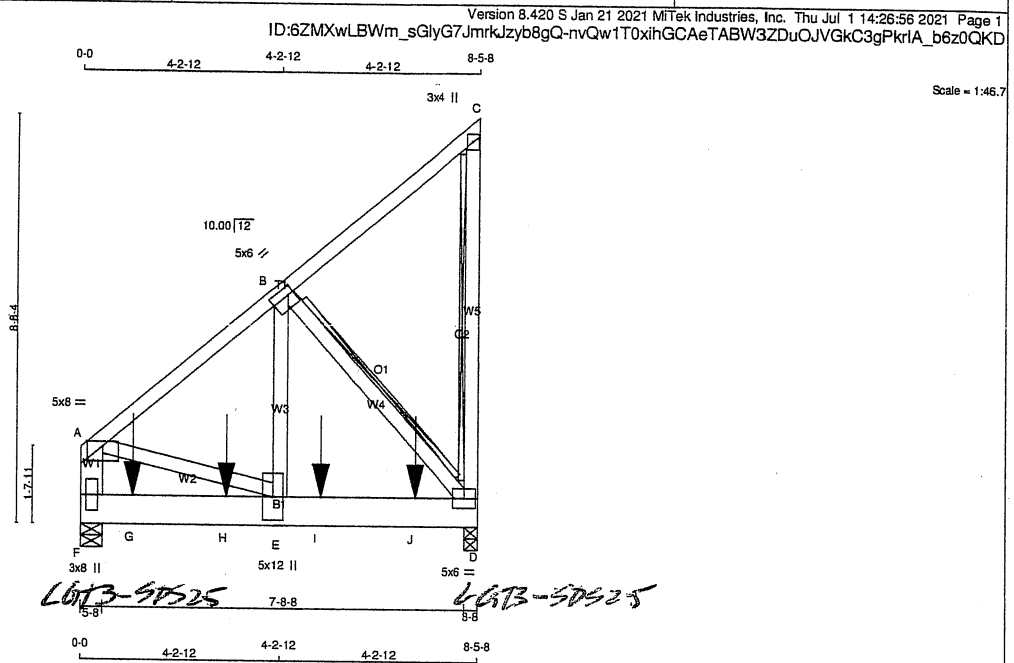
Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 1 10:22:45 2021 Page 2
ID:U6yi?rbeFFwxf UFm koDybSsJ-FHfE6nit1AO0z5xMP1iswH5UNMKioQyErOeDtoz0Tv8

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.2} PSF AT {31-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.



Structural component only
DWG# T-2121165 *ml*

JOB NAME 412868	TRUSS NAME T12	QUANTITY 1	PLY 3	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x6	DRY	No.2	SPF
F - D	2x8	DRY	No.2	SPF

ALL WEBS 2x4 DRY No.2 SPF

DRY: SEASONED LUMBER.

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C 1	12	TOP
C-D 1	12	TOP
F-A 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F-D 2	4	SIDE (957.4)
WEBS : (0.122"x3") SPIRAL NAILS		
2x4 1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLYS.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLYS FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
D	7777	0	7857	0	-2445
F	8212	0	8291	477	-2340

PROVIDE ANCHORAGE AT BEARING JOINT D FOR 2445 LBS. FACTORED UPLIFT

PROVIDE ANCHORAGE AT BEARING JOINT F FOR 2340 LBS. FACTORED UPLIFT

PROVIDE FOR 477 LBS. FACTORED HORIZONTAL REACTION AT JOINT F

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
D	5726	3507 / 0	974 / 0	0 / 0	325 / -2372	1298 / 0	0 / 0	0 / 0
F	6045	3700 / 0	1028 / 0	0 / 0	280 / -2332	1371 / 0	0 / 0	0 / 0

HORIZONTAL REACTIONS

F	0 / 0	0 / 0	0 / 0	341 / -225	0 / 0	0 / 0
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BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, F

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 4.63 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x6 DRY SPF No.2 T-BRACE AT C-D, B-D

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (18)

CHORDS		FACTORED		WEBS	
MEMB.	FORCE (LBS)	VERT. LOAD	MAX. (PLF)	MEMB.	MAX. FORCE (LBS)
FR-TO				FR-TO	
A-B	-6013 / 1767	-115.2	-115.2 0.19 (2)	E-B	-2461 / 8415
B-C	-170 / 194	-115.2	-115.2 0.12 (2)	B-D	-6928 / 2271
D-C	-223 / 159	0.0	0.0 0.13 (13)	A-E	-1409 / 4817
F-A	-5360 / 1502	0.0	0.0 0.12 (2)		
F-G	-455 / 305	-39.5	-39.5 0.54 (3)		
G-H	-455 / 305	-39.5	-39.5 0.54 (3)		
H-E	-455 / 305	-39.5	-39.5 0.54 (3)		
E-I	-1504 / 4649	-39.5	-39.5 0.68 (2)		
I-J	-1504 / 4649	-39.5	-39.5 0.68 (2)		
J-D	-1504 / 4649	-39.5	-39.5 0.68 (2)		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
G	1-1-4	-2700	-2700	615	BACK	VERT	TOTAL	C1
H	3-1-4	-2700	-2700	615	BACK	VERT	TOTAL	C1
I	5-1-4	-2700	-2700	615	BACK	VERT	TOTAL	C1
J	7-1-4	-2700	-2700	615	BACK	VERT	TOTAL	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

TOTAL WEIGHT = 3 X 59 = 178 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 33.4	PSF
	DL = 6.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 57.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 4 OF BCBC 2018, ABC 2019
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- SLOPE REDUCTION FACTOR NOT USED
- PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED.

(80 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 33.4 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.28")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL)= L/180 (0.56")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.19/1.00 (A-B-2), BC=0.68/1.00 (D-E-2), WB=0.45/1.00 (B-E-3), SSI=0.89/1.00 (D-E-3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
		788	1987

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.87 (B) (INPUT = 0.90)
JSI METAL= 0.56 (E) (INPUT = 1.00)



Structural component only
DWG# T-2121166

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
412868	T12	1	3	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
C	TMV+p	MT20	3.0	4.0		
D	BMVW1-t	MT20	5.0	6.0		
E	BMVW-t	MT20	5.0	12.0		
F	BMV1+p	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.2} PSF AT {31-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, $C_p C_g$, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.



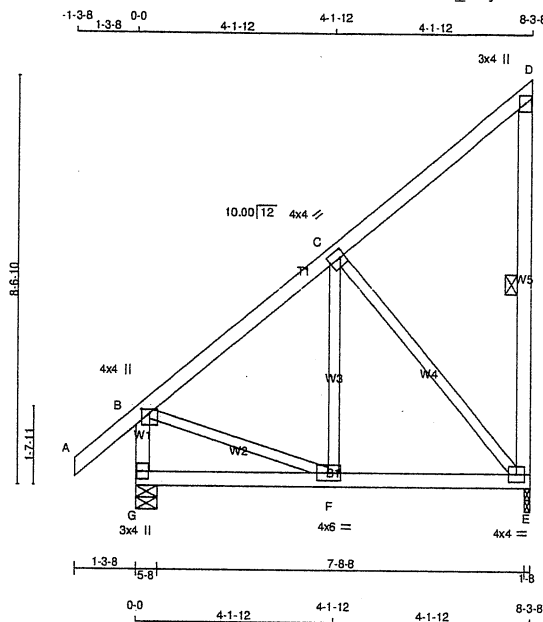
Structural component only
DWG# T-2121166 3M

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Structural component only
DWG# T-2121167

JOB NAME 412868	TRUSS NAME T14	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	



TOTAL WEIGHT = 46 lb

LUMBER				
N. L. G. A. RULES				
CHORDS		SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS		2x3	DRY	No.2
FXCFRT				SPF

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMVW-t	MT20	4.0	4.0	2.00	1.25
D	TMV+p	MT20	3.0	4.0		
E	BMVW1-t	MT20	4.0	4.0		
F	BMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	VERT	HORZ	DOWN	BRG
E	457	0	0	0	1-8
G	584	0	0	0	5-8

UNFACTORED REACTIONS

JT	1ST CASE	MAX./MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	323	212 / 0	0 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0
G	411	283 / 0	0 / 0	0 / 0	0 / 0	0 / 0	128 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		W E B S		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	MEMB.	FORCE (LBS)	MAX. FACTORED (LBS)	MAX. FACTORED (LC)
FR-TO		FROM	TO	FR-TO			
A-B	0 / 41	-91.8	-91.8 0.13 (1)	F-C	0 / 80	0.03 (4)	
B-C	-292 / 0	-91.8	-91.8 0.20 (1)	C-E	-376 / 0	0.25 (1)	
C-D	-30 / 0	-91.8	-91.8 0.20 (1)	B-F	0 / 259	0.06 (1)	
E-D	-143 / 0	0.0	0.0 0.05 (1)				
G-B	-554 / 0	0.0	0.0 0.06 (1)				
G-F	0 / 0	-18.5	-18.5 0.09 (4)				
F-E	0 / 247	-18.5	-18.5 0.11 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
DL =	6.0	PSF
BOT CH.	LL = 0.0	PSF
DL =	7.4	PSF
TOTAL LOAD =	39.0	PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.28")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.28")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.01")

CSI: TC=0.20/1.00 (B-C:1), BC=0.11/1.00 (E-F:4), WB=0.25/1.00 (C-E:1), SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

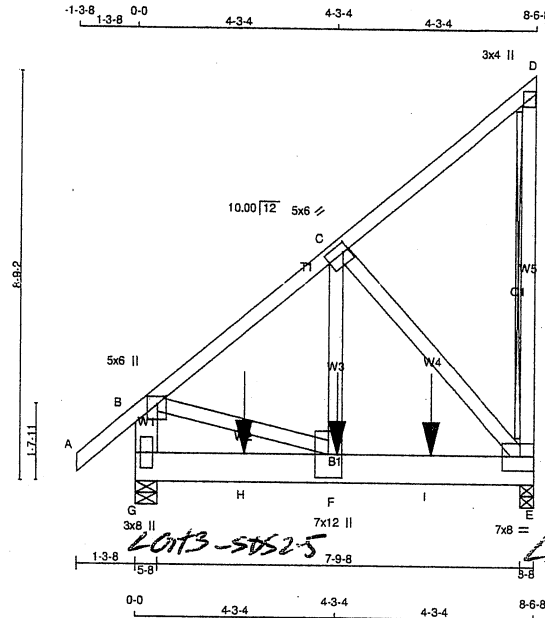
JSI GRIP = 0.45 (B) (INPUT = 0.90)
JSI METAL = 0.12 (B) (INPUT = 1.00)



Structural component only
DWG# T-2121168

JOB NAME 412868	TRUSS NAME T15	QUANTITY 1	PLY 3	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1:47.1

TOTAL WEIGHT = 3 X 62 = 185 lb [M]

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - B	2x6	DRY	No.2	SPF
G - E	2x8	DRY	No.2	SPF
ALL WEBS EXCEPT C - E	2x4	DRY	No.2	SPF
			2100F 1.8E	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - D 1	12	TOP
D - E 1	12	TOP
G - B 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
G - E 2	4	SIDE (1158.2)
WEBS : (0.122"x3") SPIRAL NAILS		
2x4 2	5	
E - C 1	6	
F - B 1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQD
JT	VERT	HORZ	DOWN	HORZ
E	6070	0	6150	0
G	6160	0	6267	510

PROVIDE ANCHORAGE AT BEARING JOINT E FOR 1933 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT G FOR 1724 LBS. FACTORED UPLIFT

PROVIDE FOR 510 LBS. FACTORED HORIZONTAL REACTION AT JOINT G

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. SNOW	MAX/MIN. LIVE	MAX/MIN. PERM. LIVE	MAX/MIN. WIND	MAX/MIN. DEAD	MAX/MIN. SOIL
JT	COMBINED					
E	4483	2710 / 0	794 / 0	0 / 0	329 / -1877	1032 / 0
G	4541	2790 / 0	785 / 0	0 / 0	252 / -1731	1037 / 0

HORIZONTAL REACTIONS

G	0 / 0	0 / 0	0 / 0	364 / -238	0 / 0	0 / 0
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BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, G

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 4.82 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x6 DRY SPF No.2 T-BRACE AT D-E

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (18)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CS (LC)	MEMB.	FORCE (LBS)	MAX. CS (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 59	-115.2 -115.2	0.06 (2)	10.00	F-C -2191 / 7523	0.40 (3)	
B-C	-5428 / 1581	-115.2 -115.2	0.19 (2)	4.82	C-E -6253 / 2074	0.61 (2)	
C-D	-171 / 196	-115.2 -115.2	0.12 (2)	6.25	B-F -1276 / 4351	0.23 (2)	
E-D	-222 / 159	0.0 0.0	0.13 (13)	7.81			
G-B	-5039 / 1395	0.0 0.0	0.11 (2)	7.63			
G-H	-488 / 322	-39.5 -39.5	0.41 (2)	6.25			
H-F	-488 / 322	-39.5 -39.5	0.41 (2)	6.25			
F-I	-1369 / 4202	-39.5 -39.5	0.55 (2)	6.25			
I-E	-1369 / 4202	-39.5 -39.5	0.55 (2)	6.25			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	HEEL	CONN.
F	4-3-12	-2645	-2645	594	FRONT	VERT	TOTAL	C1
H	2-3-12	-2645	-2645	594	FRONT	VERT	TOTAL	C1
I	6-3-12	-2645	-2645	594	FRONT	VERT	TOTAL	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 33.4	PSF
	DL = 6.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 57.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 4 OF CBC 2018, ABC 2019
- PART 4 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- SLOPE REDUCTION FACTOR NOT USED
- PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED.

(80 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 33.4 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.28")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.03")
ALLOWABLE DEFL.(TL) = $L/180$ (0.57")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.04")

CSI: TC=0.19/1.00 (B-C-2), BC=0.55/1.00 (E-F-2), WB=0.61/1.00 (C-E-2), SSI=0.45/1.00 (F-G-3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (C) (INPUT = 0.90)
JSI METAL= 0.38 (C) (INPUT = 1.00)



Structural component only
DWG# T-2121169

JOB NAME 412868	TRUSS NAME T15	QUANTITY 1	PLY 3	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TMVW-t	MT20	5.0	6.0	2.25	1.50
D	TMV+p	MT20	3.0	4.0		
E	BMVW1-t	MT20	7.0	8.0	3.75	3.50
F	BMVW-t	MT20	7.0	12.0		
G	BMV1+p	MT20	3.0	8.0		

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

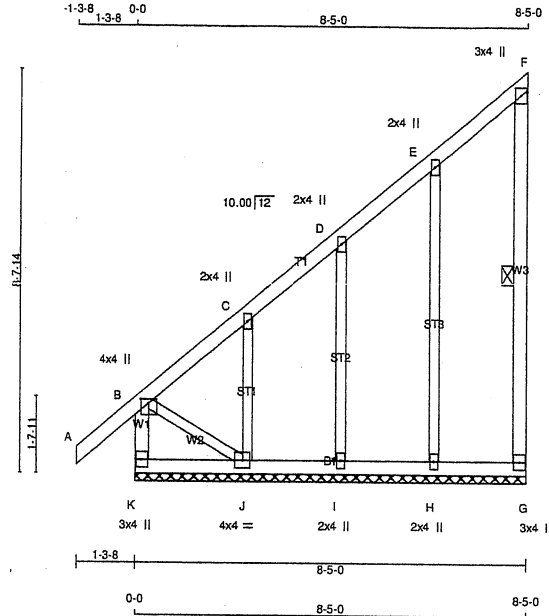
WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.2} PSF AT {31-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.



Structural component only
DWG# T-2121169 *en*

JOB NAME 412868	TRUSS NAME T16G	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1:47.1

TOTAL WEIGHT = 48 lb [M]

LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	SPF	SPF
K - B	2x4	DRY	No.2	SPF	SPF
A - F	2x4	DRY	No.2	SPF	SPF
G - F	2x4	DRY	No.2	SPF	SPF
K - G	2x4	DRY	No.2	SPF	SPF
ALL WEBS	2x3	DRY	No.2	SPF	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF	SPF
DRY: SEASONED LUMBER.					
GABLE STUDS SPACED AT 2-0-0 OC.					

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	4.0	4.0	1.00 2.00
C, D, E					
C	TMW+w	MT20	2.0	4.0	
F	TMV+p	MT20	3.0	4.0	
G	BMV1+p	MT20	3.0	4.0	
H	BMW1+w	MT20	2.0	4.0	
I	BMW1+w	MT20	2.0	4.0	
J	BMW1+w	MT20	4.0	4.0	
K	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (CSI (LC))	MEMB.	FORCE (LBS)	LC1 MAX (CSI (LC))	
FR-TO		FROM TO		FR-TO			
K-B	-230 / 0	0.0	0.0 0.02 (1)	H-E	-209 / 0	0.17 (1)	
A-B	0 / 41	-91.8	-91.8 0.13 (1)	I-D	-164 / 0	0.07 (1)	
B-C	-3 / 0	-91.8	-91.8 0.06 (1)	J-C	-224 / 0	0.05 (1)	
C-D	-15 / 0	-91.8	-91.8 0.06 (1)	B-J	0 / 14	0.00 (1)	
D-E	0 / 0	-91.8	-91.8 0.05 (1)				
E-F	-12 / 0	-91.8	-91.8 0.05 (1)				
G-F	-78 / 0	0.0	0.0 0.03 (1)				
K-J	0 / 0	-18.5	-18.5 0.02 (4)				
J-I	0 / 6	-18.5	-18.5 0.02 (4)				
I-H	0 / 3	-18.5	-18.5 0.02 (4)				
H-G	0 / 0	-18.5	-18.5 0.02 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13/1.00 (A-B:1), BC=0.02/1.00 (I-J:4), WB=0.17/1.00 (E-H:1), SSI=0.08/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

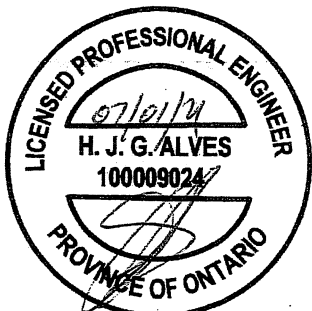
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90)
JSI METAL= 0.12 (C) (INPUT = 1.00)



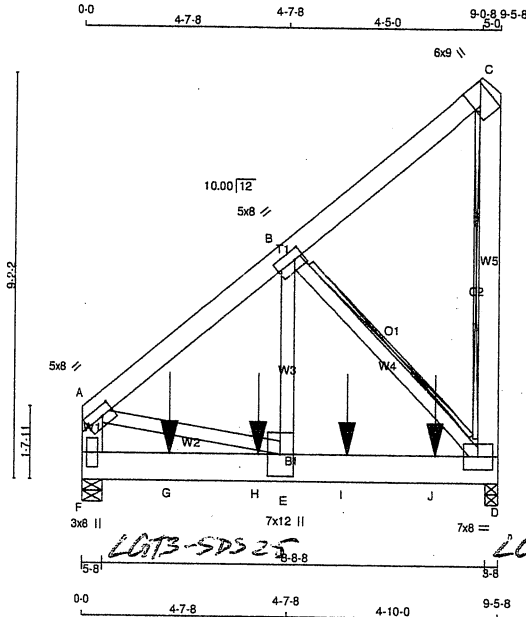
Structural component only
DWG# T-2121170

JOB NAME 412884	TRUSS NAME T28	QUANTITY 1	PLY 3	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1:49.7

LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N.L.G.A. RULES				
A - C	2x6	DRY	No.2	SPF
F - A	2x6	DRY	No.2	SPF
D - C	2x6	DRY	No.2	SPF
F - D	2x8	DRY	1950F 1.7E	DF
ALL WEBS	2x4	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C 2	12	TOP
F-A 2	12	TOP
D-C 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F-D 3	6	SIDE (957.4)
WEBS : (0.122"x3") SPIRAL NAILS		
2x4 1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	UPLIFT
F	7684	0	7773	516
D	8460	0	8548	0

PROVIDE ANCHORAGE AT BEARING JOINT F FOR 2176 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 2647 LBS. FACTORED UPLIFT

PROVIDE FOR 516 LBS. FACTORED HORIZONTAL REACTION AT JOINT F

UNFACTORED REACTIONS

1ST LC CASE	MAX./MIN. COMPONENT REACTIONS
JT COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
F	5658 3471 / 0 963 / 0 0 / 0 278 / -2173 1283 / 0 0 / 0
D	6228 3816 / 0 1059 / 0 0 / 0 351 / -2571 1412 / 0 0 / 0

HORIZONTAL REACTIONS

F	0 / 0	0 / 0	0 / 0	369 / -241	0 / 0	0 / 0
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BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, D

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 5.32 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x6 DRY SPF No.2 T-BRACE AT B-D, C-D

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (18)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	LC1 MAX	MAX. CS1 (LC)	UNBRAC LENGTH	FR-TO	WEBS	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
MEMB.							MEMB.		
FR-TO							FR-TO		
A-B	-6983 / 2034	-115.2	-115.2	0.11 (13)	5.32	E-B	-2707 / 9311	0.50 (3)	
B-C	-192 / 206	-115.2	-115.2	0.06 (2)	6.25	B-D	-7667 / 2504	0.43 (2)	
F-A	-5964 / 1674	0.0	0.0	0.13 (2)	7.16	A-E	-1600 / 5522	0.30 (2)	
D-C	-279 / 184	0.0	0.0	0.08 (13)	7.81				
F-G	-496 / 328	-39.5	-39.5	0.31 (3)	6.25				
G-H	-496 / 328	-39.5	-39.5	0.31 (3)	6.25				
H-E	-496 / 328	-39.5	-39.5	0.31 (3)	6.25				
E-I	-1726 / 5387	-39.5	-39.5	0.39 (2)	6.25				
I-J	-1726 / 5387	-39.5	-39.5	0.39 (2)	6.25				
J-D	-1726 / 5387	-39.5	-39.5	0.39 (2)	6.25				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11-12	-2700	-2700	615	FRONT	VERT	TOTAL	C1
H	3-11-12	-2700	-2700	615	FRONT	VERT	TOTAL	C1
I	5-11-12	-2700	-2700	615	FRONT	VERT	TOTAL	C1
J	7-11-12	-2700	-2700	615	FRONT	VERT	TOTAL	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2.(8)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 33.4	PSF
	DL = 6.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 57.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 4 OF CBC 2018, ABC 2019
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- SLOPE REDUCTION FACTOR NOT USED
- PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED.

(80 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 33.4 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.32")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL) = L/180 (0.63")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.13/1.00 (A-F-2), BC=0.39/1.00 (D-E-2), WB=0.50/1.00 (B-E-3), SS=0.80/1.00 (E-F-3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (B) (INPUT = 0.90)

JSI METAL= 0.46 (E) (INPUT = 1.00)



Structural component only
DWG# T-2121189

JOB NAME 412884	TRUSS NAME T28	QUANTITY 1	PLY 3	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
C	TVM-H	MT20	6.0	9.0	5.50	2.00
D	BMVW1-t	MT20	7.0	8.0		
E	BMVW-H	MT20	7.0	12.0		
F	BMV1-p	MT20	3.0	8.0		

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.2} PSF AT (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.

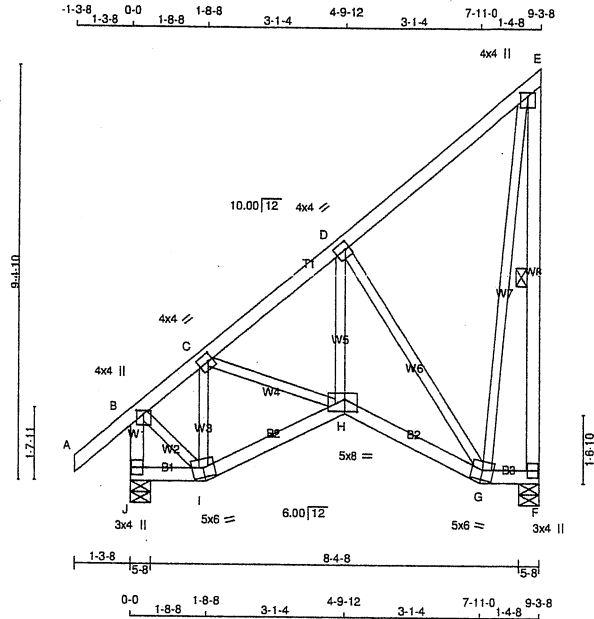


Structural component only
DWG# T-2121189

JOB NAME 412884	TRUSS NAME T29S	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1:48.7

TOTAL WEIGHT = 60 lb
[M/F]

LUMBER				DESCR.
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	
A - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	4.0	4.0	1.00 2.00
C	TMVW-l	MT20	4.0	4.0	2.00 1.25
D	TMVW-l	MT20	4.0	4.0	2.00 1.25
E	TMVW+p	MT20	4.0	4.0	1.00 2.00
F	BMV1+p	MT20	3.0	4.0	
G	BBWW-m	MT20	5.0	6.0	
H	BBWW-p	MT20	5.0	8.0	3.25 3.50
I	BBWW-m	MT20	5.0	6.0	
J	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX
F	503	0	503	0	5-8	5-8
J	649	0	649	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	WIND			
F	356	232 / 0	0 / 0	0 / 0	123 / 0	0 / 0	0 / 0
J	456	314 / 0	0 / 0	0 / 0	142 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 41	-91.8 -91.8	0.13 (1)	I-C	-254 / 0	0.05 (1)	
B-C	-339 / 0	-91.8 -91.8	0.09 (1)	G-E	0 / 368	0.08 (1)	
C-D	-430 / 0	-91.8 -91.8	0.19 (1)	B-I	0 / 297	0.07 (1)	
D-E	-95 / 0	-91.8 -91.8	0.22 (1)	H-D	0 / 313	0.07 (1)	
F-E	-490 / 0	0.0 0.0	0.20 (1)	C-H	0 / 135	0.03 (1)	
J-B	-633 / 0	0.0 0.0	0.07 (1)	D-G	-596 / 0	0.41 (1)	
J-I	0 / 0	-18.5 -18.5	0.02 (4)				
I-H	0 / 258	-18.5 -18.5	0.08 (4)				
H-G	0 / 411	-18.5 -18.5	0.10 (1)				
G-F	0 / 0	-18.5 -18.5	0.01 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
DL = 6.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.4	PSF	
TOTAL LOAD = 39.0	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.31")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.01")
ALLOWABLE DEFL. (TL) = $L/360$ (0.31")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.03")

CSI: TC=0.22/1.00 (D-E:1), BC=0.10/1.00 (G-H:1), WB=0.41/1.00 (D-G:1), SS=0.15/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.43 (B) (INPUT = 0.90)
JSI METAL = 0.16 (G) (INPUT = 1.00)

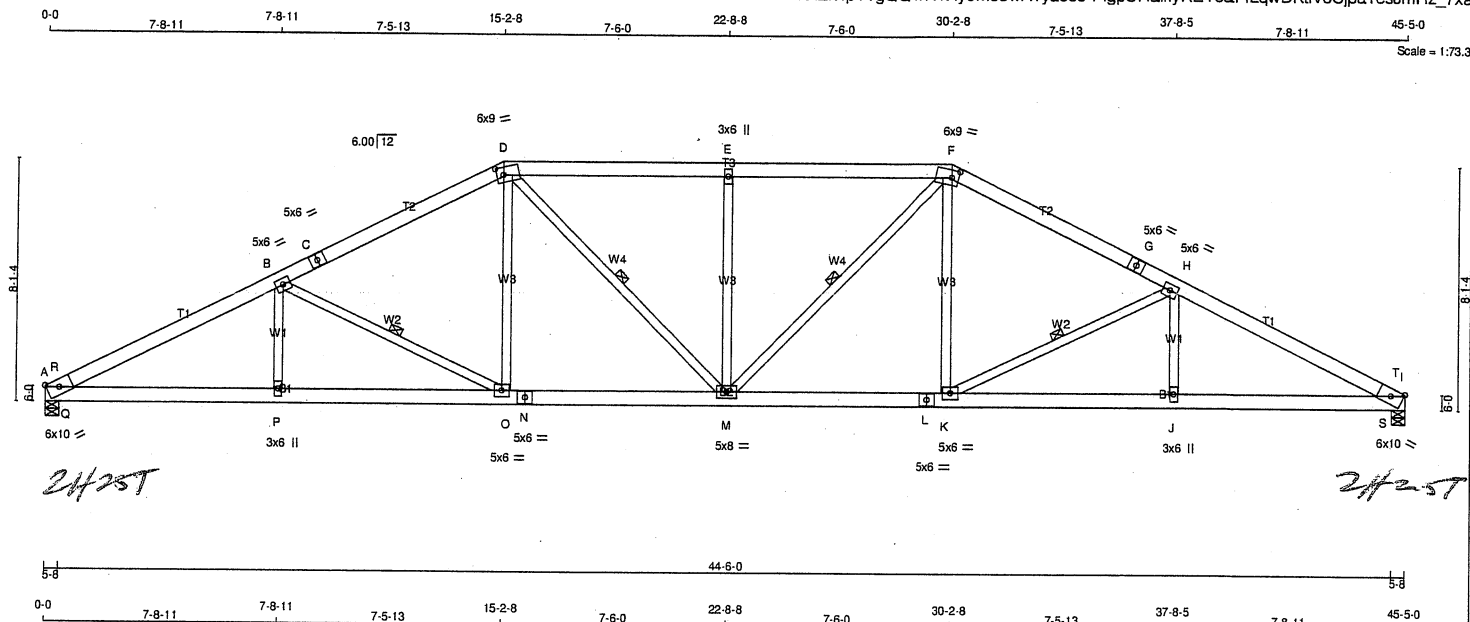


Structural component only
DWG# T-2121190

JOB NAME 413062	TRUSS NAME T37	QUANTITY 6	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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LUMBER				N. L. G. A. RULES	
CHORDS	SIZE	LUMBER	DESCR.	SPF	
A - C	2x6	DRY	No.2	SPF	
C - D	2x6	DRY	No.2	SPF	
D - F	2x6	DRY	No.2	SPF	
F - G	2x6	DRY	No.2	SPF	
G - I	2x6	DRY	No.2	SPF	
A - N	2x6	DRY	No.2	SPF	
N - L	2x6	DRY	No.2	SPF	
L - I	2x6	DRY	No.2	SPF	
ALL WEBS	2x4	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	6.0	10.0	Edge
B	TTWW-t	MT20	5.0	6.0	
C	TS-t	MT20	5.0	6.0	
D	TTWW-m	MT20	6.0	9.0	3.00 2.75
E	TMW-w	MT20	3.0	6.0	
F	TTWW-m	MT20	6.0	9.0	3.00 2.75
G	TS-t	MT20	5.0	6.0	
H	TTWW-t	MT20	5.0	6.0	
I	TBM1-h	MT20	6.0	10.0	Edge 4.75
J	BMW-w	MT20	3.0	6.0	
K	BMWW-t	MT20	5.0	6.0	
L	BS-t	MT20	5.0	6.0	
M	BMWW-t	MT20	5.0	6.0	
N	BS-t	MT20	5.0	6.0	
O	BMWW-t	MT20	5.0	6.0	
P	BMW-w	MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	HORZ	DOWN	UP	BRG	IN-SX	BRG	IN-SX	
A	3514	0	3514	193	-1003	5-8	5-8	5-8	
I	3514	0	3514	0	-1003	5-8	5-8	5-8	

PROVIDE ANCHORAGE AT BEARING JOINT A FOR 1003 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT I FOR 1003 LBS FACTORED UPLIFT

PROVIDE FOR 193 LBS FACTORED HORIZONTAL REACTION AT JOINT A

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	2603	1518 / 0	477 / 0	0 / 0	0 / -1008	609 / 0	0 / 0
I	2603	1518 / 0	477 / 0	0 / 0	80 / -1008	609 / 0	0 / 0

HORIZONTAL REACTIONS

A	0 / 0	0 / 0	0 / 0	138 / -138	0 / 0	0 / 0
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BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, I

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 2.65 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 5.86 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF B-O, D-M, F-M, H-K.

LOADING

TOTAL LOAD CASES: (18)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	
FR-TO		FROM TO	CS (LC)	FR-TO		CS (LC)	
A-R	-6772 / 1768	-115.2 -115.2	0.23 (13)	P-B	0 / 415	0.07 (5)	
R-B	-6675 / 1923	-115.2 -115.2	0.90 (2)	B-O	-1703 / 658	0.59 (2)	
B-C	-5258 / 1583	-115.2 -115.2	0.77 (2)	O-D	-234 / 1081	0.18 (13)	
C-D	-5258 / 1583	-115.2 -115.2	0.77 (2)	D-M	-427 / 1348	0.24 (14)	
D-E	-5332 / 1662	-115.2 -115.2	0.75 (1)	M-E	-1046 / 521	0.84 (2)	
E-F	-5332 / 1662	-115.2 -115.2	0.75 (1)	M-F	-427 / 1348	0.24 (13)	
F-G	-5258 / 1583	-115.2 -115.2	0.77 (3)	K-F	-234 / 1081	0.18 (14)	
G-H	-5258 / 1583	-115.2 -115.2	0.77 (3)	K-H	-1703 / 658	0.59 (3)	
H-T	-6675 / 1925	-115.2 -115.2	0.90 (3)	J-H	0 / 415	0.07 (6)	
T-I	-6772 / 1769	-115.2 -115.2	0.23 (14)	Q-R	-395 / 309	0.00 (1)	
				S-T	-396 / 309	0.00 (1)	
A-Q	-1794 / 6006	-39.5 -39.5	0.88 (2)				
Q-P	-1794 / 6006	-39.5 -39.5	0.93 (2)				
P-O	-1794 / 6006	-39.5 -39.5	0.84 (1)				
O-N	-1212 / 4685	-39.5 -39.5	0.68 (1)				
N-M	-1212 / 4685	-39.5 -39.5	0.68 (1)				
M-L	-1054 / 4685	-39.5 -39.5	0.68 (1)				
L-K	-1054 / 4685	-39.5 -39.5	0.68 (1)				
K-J	-1602 / 6006	-39.5 -39.5	0.84 (1)				
J-S	-1602 / 6006	-39.5 -39.5	0.93 (3)				
S-I	-1602 / 6006	-39.5 -39.5	0.88 (3)				

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.2) PSF AT (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpG, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	33.4	PSF
BOT CH.	LL	=	8.0	PSF
	DL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	57.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 4 OF CBC 2018, ABC 2019
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- SLOPE REDUCTION FACTOR NOT USED
- PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED.

(80 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 33.4 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.51")
CALCULATED VERT. DEFL.(LL)= L/999 (0.30")
ALLOWABLE DEFL.(TL)= L/180 (3.03")
CALCULATED VERT. DEFL.(TL)= L/999 (0.42")

CSI: TC=0.90/1.00 (H-T-3), BC=0.93/1.00 (J-S-3), WB=0.84/1.00 (E-M-2), SS=0.33/1.00 (B-D-2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371
	1747	788
	1987	1873

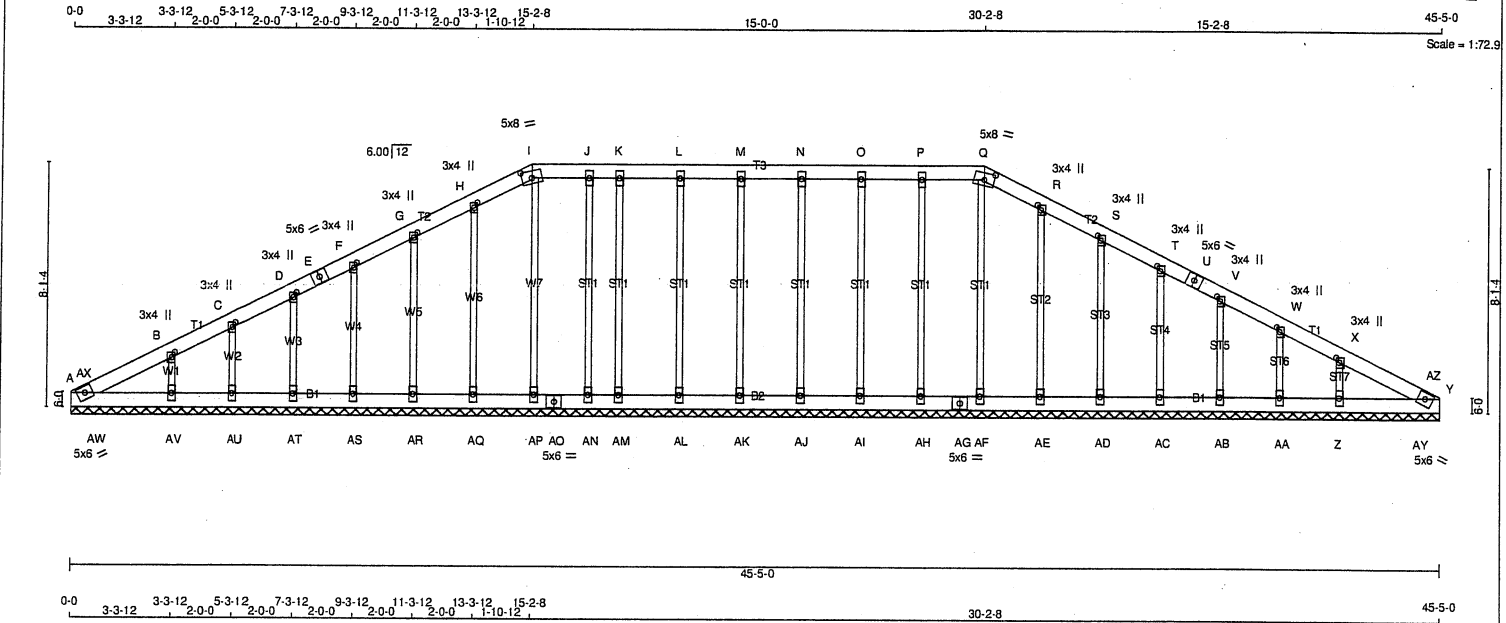
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (D) (INPUT = 0.90)
JSI METAL= 1.00 (A) (INPUT = 1.00)



Structural component only
DWG# T-2121945 1/12



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. SPF A - E 2x6 DRY No.2 E - I 2x6 DRY No.2 I - Q 2x6 DRY No.2 Q - U 2x6 DRY No.2 U - Y 2x6 DRY No.2 A - AO 2x6 DRY No.2 AO- AG 2x6 DRY No.2 AG- Y 2x6 DRY No.2 ALL WEBS 2x3 DRY No.2 ALL GABLE WEBS 2x3 DRY No.2 DRY: SEASONED LUMBER. GABLE STUDS SPACED AT 2-0-0 OC.										DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS. THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE. BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (4)										DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 25.6 PSF DL = 8.0 PSF BOT CH. LL = 0.0 PSF DL = 7.4 PSF TOTAL LOAD = 39.0 PSF SPACING = 24.0 IN. C/C LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018, ABC 2019 - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 (55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD CSI: TC=0.04/1.00 (B-C:1), BC=0.05/1.00 (Z-AY:1), WB=0.23/1.00 (P-AH:1), SSI=0.08/1.00 (B-AX:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10 COMPANION LIVE LOAD FACTOR = 1.00 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT . NAIL VALUES PLATE GRIP(DRY) (PSI) SECTION (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873 PLATE PLACEMENT TOL = 0.250 inches PLATE ROTATION TOL = 5.0 Deg. JSI GRIP= 0.25 (I) (INPUT = 0.90) JSI METAL= 0.07 (B) (INPUT = 1.00)									
PLATES (table is in inches) JT TYPE PLATES W LEN Y X A TBM1-h MT20 5.0 6.0 B, C, D, F, G, H, R, S, T, V, W, X 3.0 4.0 2.00 1.25 B TMW+w MT20 5.0 6.0 E TS-t MT20 5.0 6.0 I TTW-m MT20 5.0 8.0 2.75 4.00 J, K, L, M, N, O, P 3.0 6.0 J TMW+w MT20 5.0 8.0 2.75 4.00 Q TTW-m MT20 5.0 6.0 U TS-t MT20 5.0 6.0 Y TBM1-h MT20 5.0 6.0 Z, AA, AB, AC, AD, AE, AF, AH, AI, AJ, AK, AL, AM, AN, AP, 3.0 6.0 Z BMW1+w MT20 5.0 6.0 AG BS-t MT20 5.0 6.0 AO BS-t MT20 5.0 6.0										CHORDS MEMB. MAX. FACTORED FORCE (LBS) VERT. LOAD LC1 MAX FACTORED (PLF) CSI (LC) UNBRAC LENGTH WEBS MEMB. MAX. FACTORED FORCE (LBS) MAX CSI (LC) FR-TO -69 / 0 -91.8 -91.8 0.01 (4) 6.25 AF-Q -150 / 0 0.17 (1) AX- B -43 / 0 -91.8 -91.8 0.04 (1) 6.25 AH-P -204 / 0 0.23 (1) B- C -49 / 0 -91.8 -91.8 0.04 (1) 6.25 AI-O -184 / 0 0.21 (1) C- D -32 / 0 -91.8 -91.8 0.02 (1) 6.25 AJ-N -183 / 0 0.21 (1) D- E -27 / 0 -91.8 -91.8 0.02 (1) 6.25 AK-M -185 / 0 0.21 (1) E- F -27 / 0 -91.8 -91.8 0.02 (1) 6.25 AL-L -181 / 0 0.20 (1) F- G -23 / 0 -91.8 -91.8 0.02 (1) 6.25 AM-K -141 / 0 0.16 (1) G- H -19 / 0 -91.8 -91.8 0.02 (1) 6.25 AN-J -143 / 0 0.16 (1) H- I -23 / 0 -91.8 -91.8 0.02 (1) 6.25 AE-R -198 / 0 0.16 (1) I- J -12 / 0 -91.8 -91.8 0.02 (1) 6.25 AD-S -182 / 0 0.09 (1) J- K -12 / 0 -91.8 -91.8 0.02 (1) 6.25 AC-T -181 / 0 0.06 (1) K- L -12 / 0 -91.8 -91.8 0.02 (1) 6.25 AB-V -184 / 0 0.04 (1) L- M -12 / 0 -91.8 -91.8 0.02 (1) 6.25 AA-W -158 / 0 0.03 (1) M- N -12 / 0 -91.8 -91.8 0.02 (1) 6.25 Z- X -251 / 0 0.04 (1) N- O -12 / 0 -91.8 -91.8 0.02 (1) 6.25 AP-I -143 / 0 0.16 (1) O- P -12 / 0 -91.8 -91.8 0.02 (1) 6.25 AV-B -251 / 0 0.04 (1) P- Q -12 / 0 -91.8 -91.8 0.02 (1) 6.25 AU-C -158 / 0 0.03 (1) Q- R -23 / 0 -91.8 -91.8 0.02 (1) 6.25 AT-D -184 / 0 0.04 (1) R- S -19 / 0 -91.8 -91.8 0.02 (1) 6.25 AS-F -181 / 0 0.06 (1) S- T -23 / 0 -91.8 -91.8 0.02 (1) 6.25 AR-G -182 / 0 0.09 (1) T- U -28 / 0 -91.8 -91.8 0.02 (1) 6.25 AQ-H -198 / 0 0.15 (1) U- V -28 / 0 -91.8 -91.8 0.02 (1) 6.25 AW-AX -91 / 2 0.00 (1) V- W -32 / 0 -91.8 -91.8 0.02 (1) 6.25 AY-AZ -91 / 2 0.00 (1) W- X -49 / 0 -91.8 -91.8 0.04 (1) 6.25 X- AZ -44 / 0 -91.8 -91.8 0.04 (1) 6.25 AZ- Y -69 / 0 -91.8 -91.8 0.01 (4) 6.25										A-AW 0 / 52 -18.5 -18.5 0.05 (1) 10.00 AW-AV 0 / 52 -18.5 -18.5 0.05 (1) 10.00 AV-AU 0 / 36 -18.5 -18.5 0.03 (1) 10.00 AU-AT 0 / 30 -18.5 -18.5 0.01 (1) 10.00 AT-AS 0 / 24 -18.5 -18.5 0.01 (4) 10.00 AS-AR 0 / 20 -18.5 -18.5 0.01 (4) 10.00 AR-AQ 0 / 17 -18.5 -18.5 0.01 (4) 10.00 AQ-AP 0 / 14 -18.5 -18.5 0.01 (4) 10.00 AP-AO 0 / 12 -18.5 -18.5 0.01 (4) 10.00 AO-AN 0 / 12 -18.5 -18.5 0.01 (4) 10.00 AN-AM 0 / 12 -18.5 -18.5 0.01 (4) 10.00 AM-AL 0 / 12 -18.5 -18.5 0.01 (4) 10.00 AL-AK 0 / 12 -18.5 -18.5 0.01 (4) 10.00 AK-AJ 0 / 12 -18.5 -18.5 0.01 (4) 10.00 AJ-AI 0 / 12 -18.5 -18.5 0.01 (4) 10.00 AI-AH 0 / 12 -18.5 -18.5 0.01 (4) 10.00 AH-AG 0 / 12 -18.5 -18.5 0.01 (4) 10.00 AG-AF 0 / 12 -18.5 -18.5 0.01 (4) 10.00 AF-AE 0 / 14 -18.5 -18.5 0.01 (4) 10.00 AE-AD 0 / 17 -18.5 -18.5 0.01 (4) 10.00 AD-AC 0 / 21 -18.5 -18.5 0.01 (4) 10.00 AC-AB 0 / 25 -18.5 -18.5 0.01 (4) 10.00 AB-AA 0 / 30 -18.5 -18.5 0.01 (1) 10.00 AA- Z 0 / 36 -18.5 -18.5 0.03 (1) 10.00									

LICENSED PROFESSIONAL ENGINEER

07/09/14

H. J. G. ALVES

100009024

PROVINCE OF ONTARIO

Structural component only

DWG# T-2121946

CONTINUED ON PAGE 2



JOB NAME 413062	TRUSS NAME T37G	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 8 13:00:09 2021 Page 2
ID:Zk4pT?gQQ4nYrHy9Mo5wA1yaos6-PqpsS7laiffyREY8aFtLqwDYIr7_CtLa1esJmHz_7xa

LOADING
TOTAL LOAD CASES: (4)

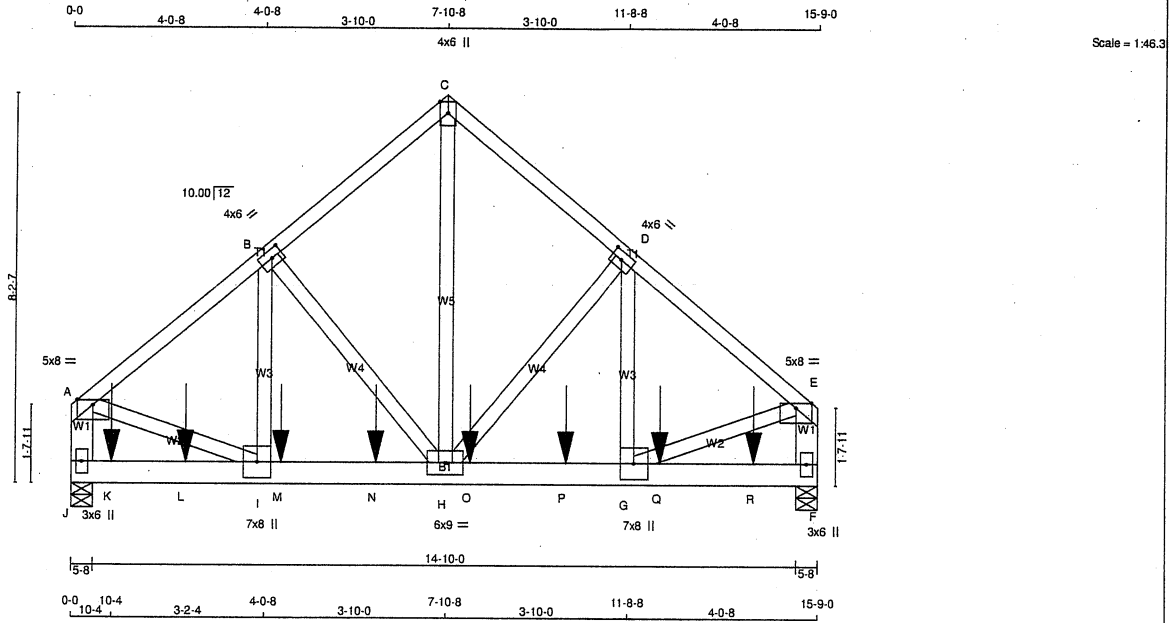
C H O R D S				W E B S			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM	TO		FR-TO		
Z-AY	0 / 52	-18.5	-18.5	0.05 (1)	10.00		
AY-Y	0 / 52	-18.5	-18.5	0.05 (1)	10.00		



Structural component only
DWG# T-2121946 *in*

JOB NAME 413062	TRUSS NAME T38	QUANTITY 2	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 Mittek Industries, Inc. Thu Jul 8 13:00:10 2021 Page 1
ID:Zk4pT?gQQ4nYrHy9Mo5wA1yaos6-txEBTJCTznp3O7KzAaM8mgDFKgxHtjGibslkz_7xZ



TOTAL WEIGHT = 4 X 96 = 382 lb [M]

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
J - A	2x6	DRY	No.2
F - E	2x6	DRY	No.2
J - F	2x6	DRY	No.2
ALL WEBS	2x4	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.
DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12	TOP
C - E	12	TOP
J - A	12	TOP
F - E	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
J - F	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x4	1	6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
A	TMVW-p	MT20	5.0	8.0 Edge
B	TMVW-t	MT20	4.0	6.0 2.00 2.75
C	TTW-p	MT20	4.0	6.0 Edge
D	TMVW-t	MT20	4.0	6.0 2.00 2.75
E	TMVW-p	MT20	5.0	8.0 Edge
F	BMV1-p	MT20	3.0	6.0
G	BMVW-t	MT20	7.0	8.0
H	BMVW-t	MT20	6.0	9.0
I	BMVW-t	MT20	7.0	8.0
J	BMV1-p	MT20	3.0	6.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
J	5848	0	5848	0	5-8
F	5340	0	5340	0	5-8

UNFACTORED REACTIONS		1ST LCASE	MAX./MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
J	4118	2800 / 0	0 / 0
F	3761	2556 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.13 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING		TOTAL LOAD CASES: (4)	
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)
FR-TO			
A-B	-5103 / 0	-91.8 -91.8 0.22 (1)	4.13
B-C	-3806 / 0	-91.8 -91.8 0.16 (1)	4.72
C-D	-3806 / 0	-91.8 -91.8 0.16 (1)	4.72
D-E	-5008 / 0	-91.8 -91.8 0.22 (1)	4.17
J-A	-4623 / 0	0.0 0.0 0.17 (1)	6.71
F-E	-4541 / 0	0.0 0.0 0.17 (1)	6.76
J-K	0 / 0	-18.5 -18.5 0.39 (1)	10.00
K-L	0 / 0	-18.5 -18.5 0.39 (1)	10.00
L-M	0 / 0	-18.5 -18.5 0.39 (1)	10.00
I-M	0 / 3936	-18.5 -18.5 0.59 (1)	10.00
M-N	0 / 3936	-18.5 -18.5 0.59 (1)	10.00
N-H	0 / 3936	-18.5 -18.5 0.59 (1)	10.00
H-O	0 / 3863	-18.5 -18.5 0.58 (1)	10.00
O-P	0 / 3863	-18.5 -18.5 0.58 (1)	10.00
P-G	0 / 3863	-18.5 -18.5 0.58 (1)	10.00
G-Q	0 / 0	-18.5 -18.5 0.39 (1)	10.00
Q-R	0 / 0	-18.5 -18.5 0.39 (1)	10.00
R-F	0 / 0	-18.5 -18.5 0.39 (1)	10.00

SPECIFIED CONCENTRATED LOADS (LBS)		JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
K	10-4	-832	-832	---	TOP	VERT	TOTAL	---	C1		
L	2-4-12	-831	-831	---	TOP	VERT	TOTAL	---	C1		
M	4-4-12	-831	-831	---	TOP	VERT	TOTAL	---	C1		
N	6-4-12	-831	-831	---	TOP	VERT	TOTAL	---	C1		
O	8-4-12	-831	-831	---	TOP	VERT	TOTAL	---	C1		
P	10-4-12	-831	-831	---	TOP	VERT	TOTAL	---	C1		
Q	12-4-12	-831	-831	---	TOP	VERT	TOTAL	---	C1		
R	14-4-12	-831	-831	---	TOP	VERT	TOTAL	---	C1		

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.52")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.52")
CALCULATED VERT. DEFL.(TL) = L/999 (0.08")

CSI: TC=0.22/1.00 (A-B:1), BC=0.59/1.00 (H-I:1),
WB=0.40/1.00 (C-H:1), SSI=0.54/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.77 (A) (INPUT = 0.90)
JSI METAL = 0.51 (G) (INPUT = 1.00)

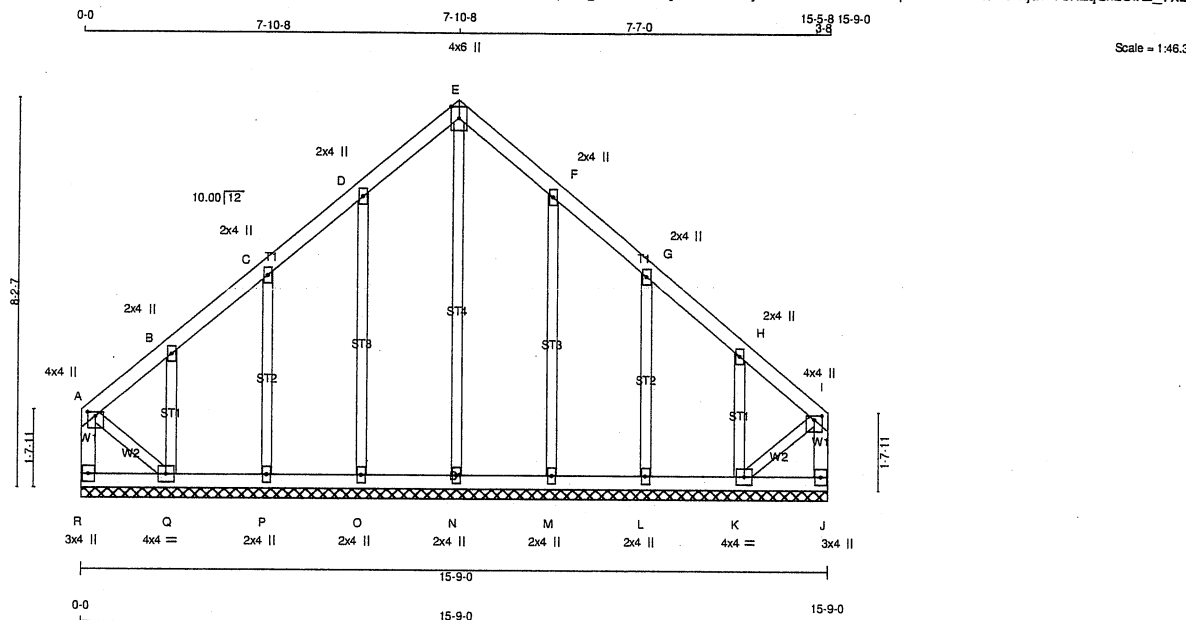


Structural component only
DWG# T-2121947

JOB NAME 413062	TRUSS NAME T39G	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 8 13:00:10 2021 Page 1
ID:Zk4pT?gQQ4nYrHy9Mo5wA1yaos6-xEBFTJCTzn3O7K7zAaM8mjuFTexLtjGlsIkz_7xZ



TOTAL WEIGHT = 2 X 75 = 150 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
R - A	2x4	DRY	No.2
A - E	2x4	DRY	No.2
E - I	2x4	DRY	No.2
J - J	2x4	DRY	No.2
R - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

GABLE STUDS SPACED AT 2'-0" O.C.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A TMVW+p	MT20	4.0	4.0	1.00	2.00
B, C, D, F, G, H					
B TMW+w	MT20	2.0	4.0		
E TTW+p	MT20	4.0	6.0	Edge	
I TMVW+p	MT20	4.0	4.0	1.00	2.00
J BMV1+p	MT20	3.0	4.0		
K BMWW1-t	MT20	4.0	4.0		
L, M, N, O, P					
L BMW1+w	MT20	2.0	4.0		
Q BMWW1-t	MT20	4.0	4.0		
R BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)
FR-TO		FROM TO		FR-TO			
R-A	-119 / 0	0.0	0.0 0.01 (1)	7.81	N-E	-116 / 0	0.15 (1)
A-B	-35 / 0	-91.8	-91.8 0.05 (1)	6.25	O-D	-209 / 0	0.15 (1)
B-C	-41 / 0	-91.8	-91.8 0.05 (1)	6.25	P-C	-173 / 0	0.06 (1)
C-D	-31 / 0	-91.8	-91.8 0.05 (1)	6.25	Q-B	-189 / 0	0.04 (1)
D-E	-42 / 0	-91.8	-91.8 0.05 (1)	6.25	M-F	-209 / 0	0.15 (1)
E-F	-42 / 0	-91.8	-91.8 0.05 (1)	6.25	L-G	-173 / 0	0.06 (1)
F-G	-31 / 0	-91.8	-91.8 0.05 (1)	6.25	K-H	-189 / 0	0.04 (1)
G-H	-41 / 0	-91.8	-91.8 0.05 (1)	6.25	A-Q	0 / 44	0.01 (1)
H-I	-35 / 0	-91.8	-91.8 0.05 (1)	6.25	K-I	0 / 44	0.01 (1)
J-I	-119 / 0	0.0	0.0 0.01 (1)	7.81			
R-Q	0 / 0	-18.5	-18.5 0.02 (4)	10.00			
Q-P	0 / 30	-18.5	-18.5 0.02 (4)	10.00			
P-O	0 / 26	-18.5	-18.5 0.02 (4)	10.00			
O-N	0 / 23	-18.5	-18.5 0.02 (4)	10.00			
N-M	0 / 23	-18.5	-18.5 0.02 (4)	10.00			
M-L	0 / 26	-18.5	-18.5 0.02 (4)	10.00			
L-K	0 / 30	-18.5	-18.5 0.02 (4)	10.00			
K-J	0 / 0	-18.5	-18.5 0.02 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.05/1.00 (E-F:1), BC=0.02/1.00 (P-Q:4), WB=0.15/1.00 (E-N:1), SSI=0.07/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.15 (D) (INPUT = 0.90)
JSI METAL = 0.11 (D) (INPUT = 1.00)

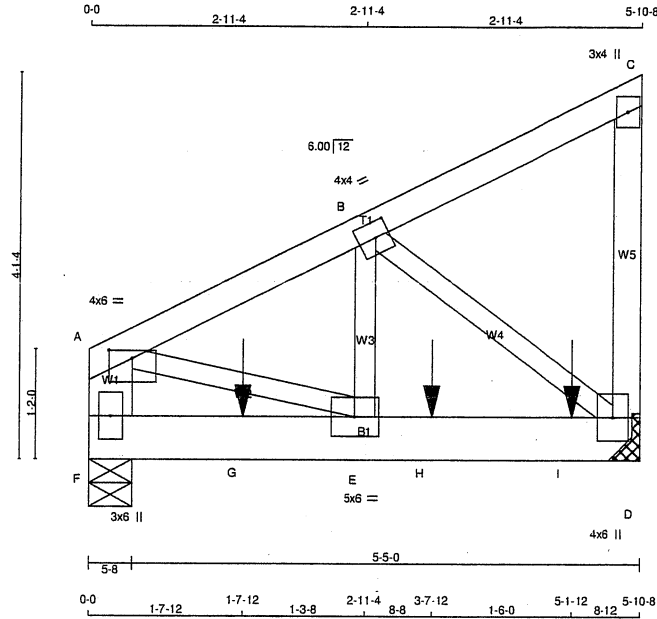


Structural component only
DWG# T-2121948

JOB NAME 413062	TRUSS NAME T40	QUANTITY 2	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 8 13:00:11 2021 Page 1
ID:Zk4pT?gQQ4nYrHy9Mo5wA1yaos6-L7oZtoJqEGwggYiXhghpVluWfmqgo0tVyLQrAz_7xY



TOTAL WEIGHT = 4 X 29 = 117 lb [M]

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x6	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C 1	12	TOP
C - D 1	12	TOP
F - A 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F - D 2	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00	3.00
B	TMVW-t	MT20	4.0	4.0	2.00	1.75
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMVW-t	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	VERT	HORZ	DOWN	UP
D	1449	0	1449	0	0
F	1098	0	1098	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
D	1018	708 / 0	0 / 0	0 / 0	0 / 0	310 / 0	0 / 0
F	771	534 / 0	0 / 0	0 / 0	0 / 0	237 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MEMB.		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX- (LC)	MAX. UNBRACED LENGTH	MEMB.	FORCE (LBS)	MAX. FACTORED (LC)
FR-TO		FROM	TO		FR-TO		
A-B	-1143 / 0	-91.8	-91.8 0.06 (1)	6.25	E-B	0 / 984	0.12 (1)
B-C	-11 / 0	-91.8	-91.8 0.06 (1)	6.25	B-D	-1299 / 0	0.15 (1)
D-C	-109 / 0	0.0	0.0 0.01 (1)	7.81	A-E	0 / 1068	0.13 (1)
F-A	-898 / 0	0.0	0.0 0.03 (1)	7.81			
F-G	0 / 0	-18.5	-18.5 0.11 (1)	10.00			
G-E	0 / 0	-18.5	-18.5 0.11 (1)	10.00			
E-H	0 / 1032	-18.5	-18.5 0.21 (1)	10.00			
H-I	0 / 1032	-18.5	-18.5 0.21 (1)	10.00			
I-D	0 / 1032	-18.5	-18.5 0.21 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-7-12	-443	-443	---	TOP	VERT	TOTAL	---	C1
H	3-7-12	-443	-443	---	TOP	VERT	TOTAL	---	C1
I	5-1-12	-445	-445	---	TOP	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.6	PSF
	DL	= 6.0	PSF
BOT CH.	LL	= 0.0	PSF
	DL	= 7.4	PSF
TOTAL LOAD		= 39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.06/1.00 (A-B:1), BC=0.21/1.00 (D-E:1),
WB=0.15/1.00 (B-D:1), SSI=0.26/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
		788	1987
		1873	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.64 (B) (INPUT = 0.90)
JSI METAL= 0.23 (D) (INPUT = 1.00)

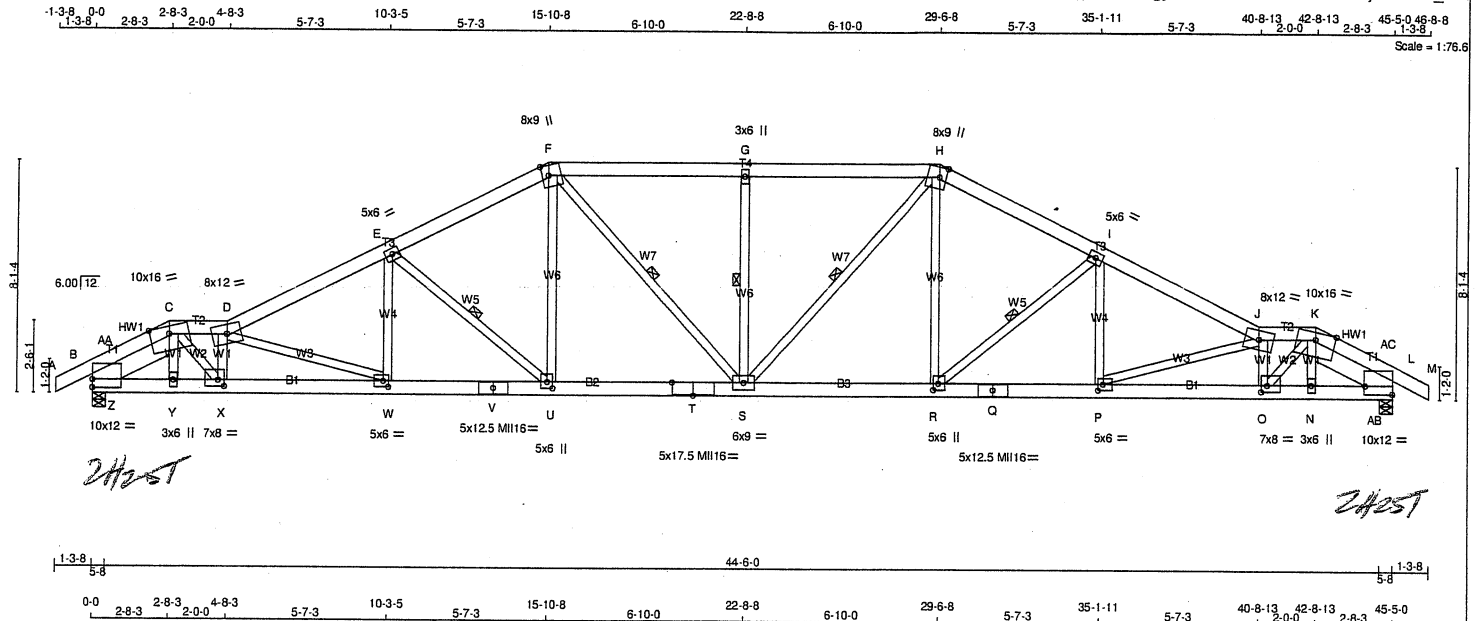


Structural component only
DWG# T-2121949

JOB NAME 413062	TRUSS NAME T81	QUANTITY 6	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 8 13:11:26 2021 Page 1
ID:wzdEniYwIMtNFy9B0RQea5yaUCB-4_4IYcUEGgyu9HoFxd6mh3GI3GKu7DjTMD1Yz_7n?



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x6	DRY	1850F 1.5E
C - D	2x6	DRY	1850F 1.5E
D - F	2x6	DRY	1850F 1.5E
F - H	2x6	DRY	1850F 1.5E
H - J	2x6	DRY	1850F 1.5E
J - K	2x6	DRY	1850F 1.5E
K - M	2x6	DRY	1850F 1.5E
B - V	2x6	DRY	2100F 1.8E
V - T	2x6	DRY	2100F 1.8E
T - Q	2x6	DRY	2100F 1.8E
Q - L	2x6	DRY	2100F 1.8E

REINFORCING MEMBERS			
HW1	2x8	DRY	No.2
HW2	2x8	DRY	No.2
ALL WEBS EXCEPT			
D - W	2x4	DRY	2100F 1.8E
P - J	2x4	DRY	2100F 1.8E

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMW1-I	MT20	10.0	12.0	3.50	
C	TTWWVW-m	MT20	10.0	16.0	Edge	
D	TTWWV-m	MT20	8.0	12.0		
E	TMWWV-t	MT20	5.0	6.0		
F	TTWWV+m	MT20	8.0	9.0	4.25	2.75
G	TMWW+w	MT20	3.0	6.0		
H	TTWWV+m	MT20	8.0	9.0	4.25	2.75
I	TMWW-t	MT20	5.0	6.0		
J	TTWWV-m	MT20	8.0	12.0		
K	TTWWVW-m	MT20	10.0	16.0	Edge	
L	TMBMW1-I	MT20	10.0	12.0	3.50	Edge
N	BMW+w	MT20	3.0	6.0		
O	BMWW-t	MT20	7.0	8.0	2.75	2.50
P	BMWW-t	MT20	5.0	6.0	2.50	2.25
Q	BS-t	MIH6	5.0	12.5		
R	BMWW-t	MT20	5.0	6.0	2.75	2.25
S	BMWWV-t	MT20	6.0	9.0		
T	BS-t	MIH6	5.0	17.5	Edge	8.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX
B	5780	0	5780	185	-881	5-8
L	5780	0	5780	0	-881	5-8

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 881 LBS. FACTORED UPLIFT

PROVIDE ANCHORAGE AT BEARING JOINT L FOR 881 LBS. FACTORED UPLIFT

PROVIDE FOR 185 LBS. FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	4123	2980 / 0	477 / 0	0 / 0	122 / -957	666 / 0	0 / 0
L	4123	2980 / 0	477 / 0	0 / 0	122 / -957	666 / 0	0 / 0

HORIZONTAL REACTIONS

B	—	0 / 0	0 / 0	0 / 0	132 / -132	0 / 0	0 / 0
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BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, L
BEARING SIZE FACTOR = 1.15 AT JNT(S) B, L (BASED ON SUPPORT DEPTH = 1-8)

BRACING

FOR SECTION F-H, MAX. UNBRACED TOP CHORD LENGTH = 2.00 FT.
FOR OTHER SECTIONS, MAX. UNBRACED TOP CHORD LENGTH = 2.81 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-U, F-S, G-S, H-S, I-R.

LOADING

TOTAL LOAD CASES: (18)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CS1 (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
A-B	0 / 2	-200.5 -200.5	0.11 (2)	10.00	Y-C	-105 / 727	0.12 (1)
B-AA	-6048 / 889	-200.5 -200.5	0.11 (1)	4.06	C-X	-792 / 5733	0.92 (1)
AA-C	-3964 / 661	-200.5 -200.5	0.08 (1)	4.85	X-D	-4565 / 708	0.49 (1)
C-D	-10819 / 1627	-200.5 -200.5	0.24 (1)	3.02	D-W	-2176 / 410	0.60 (2)
D-E	-10274 / 1521	-200.5 -200.5	0.61 (2)	2.81	W-E	-98 / 1041	0.17 (2)
E-F	-8406 / 1275	-200.5 -200.5	0.52 (2)	3.18	E-U	-2646 / 567	0.69 (2)
F-G	-8375 / 1221	-208.0 -208.0	0.76 (1)	2.00	U-F	-313 / 1954	0.31 (2)
G-H	-8375 / 1221	-208.0 -208.0	0.76 (1)	2.00	F-S	-336 / 2114	0.34 (3)
H-I	-8406 / 1275	-200.5 -200.5	0.52 (3)	3.18	S-G	-1723 / 300	0.46 (1)
I-J	-10274 / 1522	-200.5 -200.5	0.61 (3)	2.81	S-H	-336 / 2114	0.34 (2)
J-K	-10819 / 1628	-200.5 -200.5	0.24 (1)	3.02	R-H	-313 / 1954	0.31 (3)
K-AC	-3964 / 664	-200.5 -200.5	0.08 (1)	4.85	R-I	-2646 / 567	0.69 (3)
AC-L	-6048 / 891	-200.5 -200.5	0.11 (1)	4.06	P-I	-98 / 1041	0.17 (3)
L-M	0 / 2	-200.5 -200.5	0.11 (3)	10.00	P-J	-2176 / 411	0.60 (3)
					O-J	-4565 / 708	0.49 (1)
B-Z	-672 / 3512	-39.5 -39.5	0.30 (1)	6.25	O-K	-791 / 5733	0.92 (1)
Z-Y	-1142 / 6906	-39.5 -39.5	0.46 (1)	6.25	N-K	-107 / 727	0.12 (1)
Y-X	-1150 / 6958	-39.5 -39.5	0.49 (1)	6.25	C-Z	-4891 / 677	0.34 (1)
X-W	-1734 / 11146	-39.5 -39.5	0.69 (1)	6.25	K-AB	-4891 / 677	0.34 (1)
W-V	-1349 / 9245	-39.5 -39.5	0.49 (1)	6.25	Z-AA	-340 / 2818	0.00 (1)
V-U	-1349 / 9245	-39.5 -39.5	0.49 (1)	6.25	AB-AC	-340 / 2818	0.00 (1)
U-T	-920 / 7498	-39.5 -39.5	0.40 (1)	6.25			
T-S	-920 / 7498	-39.5 -39.5	0.40 (1)	6.25			
S-R	-736 / 7498	-39.5 -39.5	0.40 (1)	6.25			
R-Q	-1164 / 9245	-39.5 -39.5	0.49 (1)	6.25			
Q-P	-1164 / 9245	-39.5 -39.5	0.49 (1)	6.25			
P-O	-1550 / 11145	-39.5 -39.5	0.69 (1)	6.25			
O-N	-967 / 6958	-39.5 -39.5	0.49 (1)	6.25			
N-AB	-959 / 6906	-39.5 -39.5	0.46 (1)	6.25			
AB-L	-490 / 3512	-39.5 -39.5	0.30 (1)	6.25			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 61.9	PSF
	DL	= 6.0	PSF
BOT CH.	LL	= 10.5	PSF
	DL	= 7.4	PSF
TOTAL LOAD		= 85.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN HIGHEST FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE HEEL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

LOADING IN OTHER FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 4 OF BCBC 2018, ABC 2019
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- SLOPE REDUCTION FACTOR USED
- PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED.

(80 % OF 66.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 61.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.51")
CALCULATED VERT. DEFL.(LL) = L/999 (0.46")
ALLOWABLE DEFL.(TL) = L/180 (3.03")
CALCULATED VERT. DEFL.(TL) = L/951 (0.57")

CSI: TC=0.76/1.00 (F-G-1), BC=0.69/1.00 (W-X-1), WB=0.92/1.00 (C-X-1), SS=0.57/1.00 (F-G-2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

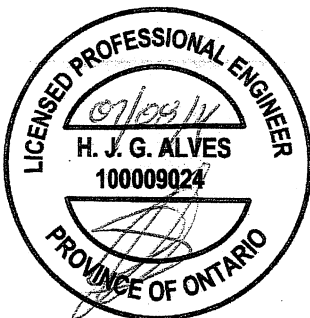
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747 788 1987 1873
MIH6	438	302	2547 1256 4283 1816

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2



Structural component only
DWG# T-2121950

JOB NAME 413062	TRUSS NAME T81	QUANTITY 6	PLY 1	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Thu Jul 8 13:11:26 2021 Page 2
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PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
U BMWW+t	MT20	5.0	6.0	2.75	2.25
V BS-t	MT16	5.0	12.5		
W BMWW-t	MT20	5.0	6.0	2.50	2.25
X BMWW-t	MT20	7.0	8.0	2.75	2.50
Y BMW-w	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (8.8) PSF AT (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpG, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.

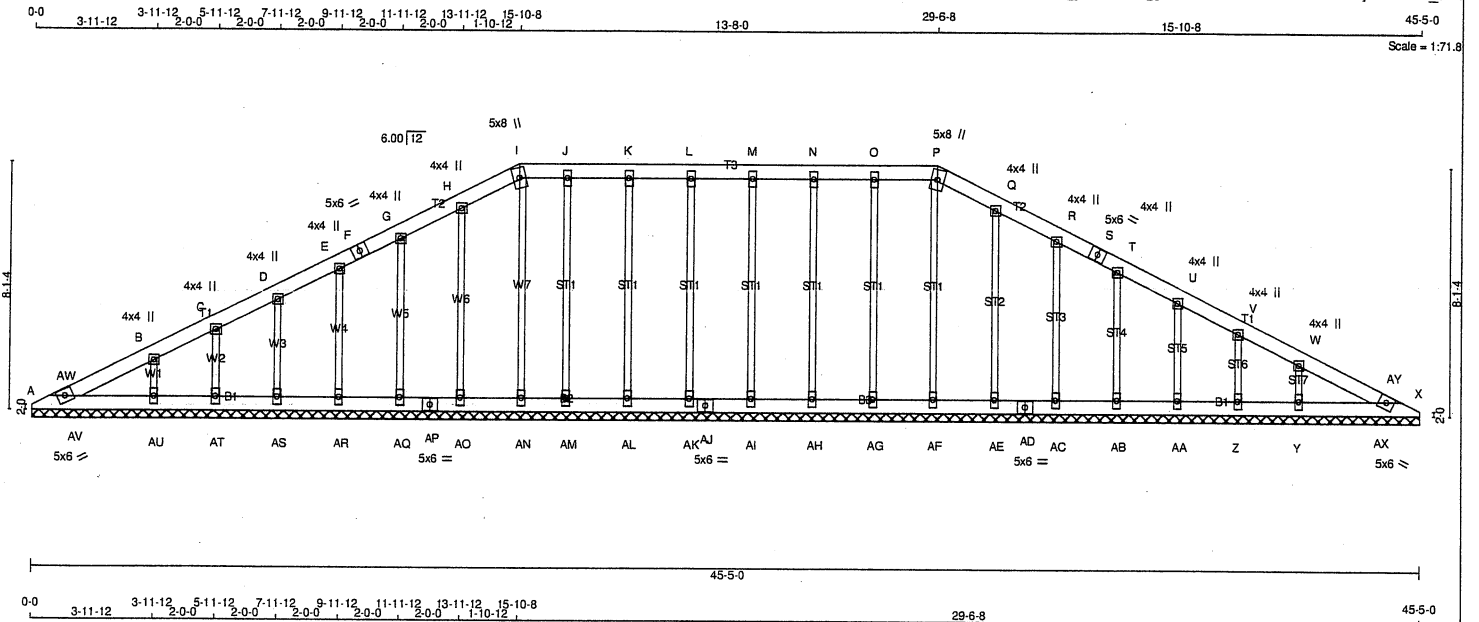
JSI GRIP= 0.90 (V) (INPUT = 0.90)
JSI METAL= 1.00 (D) (INPUT = 1.00)



Structural component only
DWG# T-2121950 *m*

JOB NAME 413062	TRUSS NAME T81G	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.420 S Jan 21 2021 MITek Industries, Inc. Thu Jul 8 13:11:26 2021 Page 1
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LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	SPF
A - F	2x6	DRY	No.2	SPF
F - I	2x6	DRY	No.2	SPF
I - P	2x6	DRY	No.2	SPF
P - S	2x6	DRY	No.2	SPF
S - X	2x6	DRY	No.2	SPF
A - AP	2x6	DRY	No.2	SPF
AP - AJ	2x6	DRY	No.2	SPF
AJ - AD	2x6	DRY	No.2	SPF
AD - X	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				
GABLE STUDS SPACED AT 2'-0" OC.				

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y
A TBM1-h	MT20	5.0	6.0	
B, C, D, E, G, H, Q, R, T, U, V, W	MT20	4.0	4.0	
F TS-t	MT20	5.0	6.0	
I TTW+m	MT20	5.0	8.0	
J, K, L, M, N, O				
J TMW+w	MT20	3.0	6.0	
P TTW+m	MT20	5.0	8.0	
S TS-t	MT20	5.0	6.0	
X TBM1-h	MT20	5.0	6.0	
Y, Z, AA, AB, AC, AE, AF, AG, AH, AI, AK, AL, AM, AN, AO, AQ, AR, AS, AT, AU				
Y BMW1+w	MT20	3.0	6.0	
AD, AJ, AP				
AD BS-t	MT20	5.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-AW	-43 / 0	A-F	-162 / 0
AW-B	-5 / 6	AG-O	-205 / 0
B-C	-24 / 0	AH-N	-184 / 0
C-D	0 / 4	AI-M	-182 / 0
D-E	0 / 5	AK-L	-184 / 0
E-F	0 / 8	AL-K	-186 / 0
F-G	0 / 8	AM-J	-176 / 0
G-H	0 / 11	AE-Q	-199 / 0
H-I	0 / 8	AC-R	-182 / 0
I-J	0 / 15	AB-T	-181 / 0
J-K	0 / 15	AA-U	-186 / 0
K-L	0 / 15	Z-V	-143 / 0
L-M	0 / 15	Y-W	-286 / 0
M-N	0 / 15	AN-I	-139 / 0
N-O	0 / 15	AT-C	-143 / 0
O-P	0 / 15	AU-B	-286 / 0
P-Q	0 / 7	AS-D	-186 / 0
Q-R	0 / 11	AR-E	-181 / 0
R-S	0 / 8	AQ-G	-183 / 0
S-T	0 / 8	AO-H	-197 / 0
T-U	0 / 5	AV-AW	-90 / 3
U-V	0 / 4	AX-AY	-90 / 3
V-W	-24 / 0		
W-AY	-5 / 6		
AY-X	-43 / 0		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.06/1.00 (V-W:1), BC=0.07/1.00 (Y-AX:1), WB=0.23/1.00 (O-AG:1), SSI=0.09/1.00 (W-AY:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.22 (P) (INPUT = 0.90)
JSI METAL = 0.06 (W) (INPUT = 1.00)



Structural component only
DWG# T-2121951

CONTINUED ON PAGE 2

JOB NAME 413062	TRUSS NAME T81G	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 8 13:11:27 2021 Page 2
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LOADING

TOTAL LOAD CASES: (4)

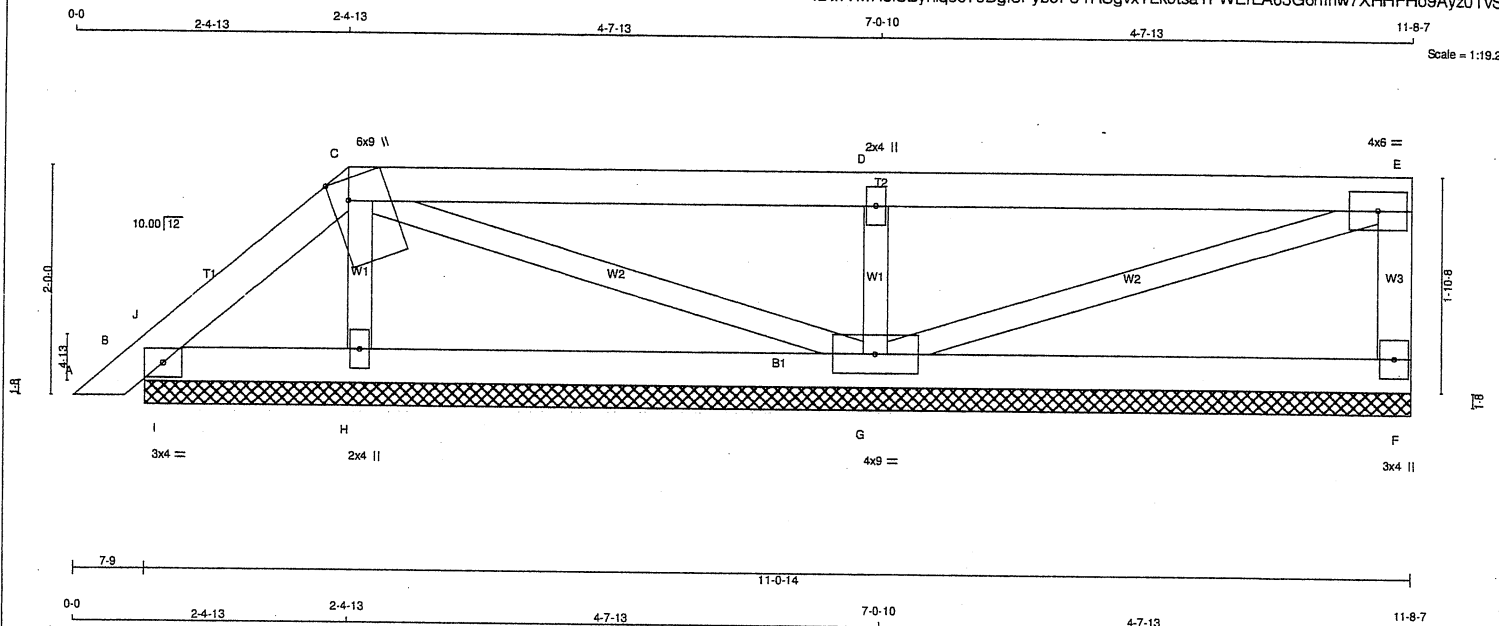
CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED UNBRAC. LENGTH	LC1 MAX. CSI (LC)
FR-TO				FR-TO			
AX-X	-1 / 15						
		FROM TO					
		-18.5 -18.5	0.06 (1)			10.00	



Structural component only
DWG# T-2121951 *me*

JOB NAME 412868	TRUSS NAME PB01	QUANTITY 4	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 1 10:22:25 2021 Page 1
ID:wTM7iolSBYniqocTeDgf3Pyb9F8-rRSgvxTLK0tsa1PWErLA63G6hIhw7XHHFHo9Ayz0TvS



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
F - H	2x4	DRY	No.2	SPF	
B - D	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-t	MT20	3.0	4.0			
C	TTWW+m	MT20	6.0	9.0	Edge	1.75	
D	TMW+w	MT20	2.0	4.0			
E	TMVW-t	MT20	4.0	6.0			
F	BMV1+p	MT20	3.0	4.0			
G	BMVWV1-t	MT20	4.0	9.0			
H	BMV1+w	MT20	2.0	4.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
F	197	0	197	0
B	170	0	170	0
H	263	0	263	0
G	641	0	641	0

UNFACTORED REACTIONS

1ST LCASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED						
F	139	91/0	0/0	0/0	0/0	48/0	0/0
B	117	91/0	0/0	0/0	0/0	26/0	0/0
H	188	110/0	0/0	0/0	0/0	78/0	0/0
G	452	302/0	0/0	0/0	0/0	151/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	MAX. FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1	MAX. CS1 (LC)	MEMB.	FORCE (LBS)
FR-TO		FROM	TO	FR-TO	
A-B	0/14	-91.8	-91.8 0.02 (1)	H-C	-181/0
B-J	-32/0	-91.8	-91.8 0.01 (1)	C-G	-28/0
J-C	-54/0	-91.8	-91.8 0.03 (1)	G-D	-533/0
C-D	0/0	-91.8	-91.8 0.34 (1)	G-E	0/0
D-E	0/0	-91.8	-91.8 0.34 (1)	I-J	-112/0
F-E	-162/0	0.0	0.0 0.02 (1)		
B-I	0/38	-18.5	-18.5 0.04 (1)		
I-H	0/38	-18.5	-18.5 0.05 (4)		
H-G	0/26	-18.5	-18.5 0.10 (4)		
G-F	0/0	-18.5	-18.5 0.10 (4)		

TOTAL WEIGHT = 4 X 38 = 151 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
	DL = 8.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CS1: TC=0.34/1.00 (D-E:1), BC=0.10/1.00 (G-H:4), WB=0.08/1.00 (D-G:1), SS=0.21/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.27 (D) (INPUT = 0.90)

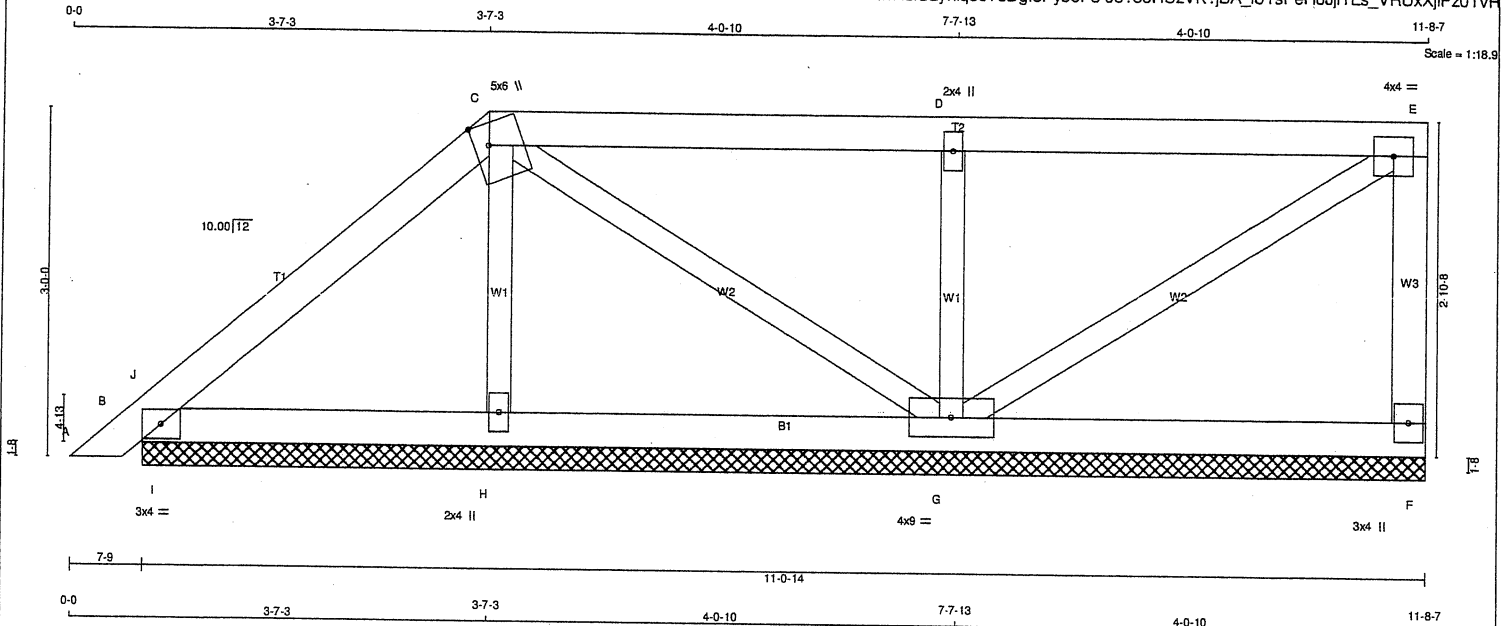
JSI METAL= 0.11 (D) (INPUT = 1.00)



Structural component only
DWG# T-2121145

JOB NAME 412868	TRUSS NAME PB02	QUANTITY 4	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 1 10:22:26 2021 Page 1
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TOTAL WEIGHT = 4 X 41 = 163 lb [M]

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
F - H	2x4	DRY	No.2
B - F	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	
C	TTWW+m	MT20	5.0	6.0	2.25 1.50
D	TMW+w	MT20	2.0	4.0	
E	TMVW-t	MT20	4.0	4.0	
F	BMV1+p	MT20	3.0	4.0	
G	BMWW1-t	MT20	4.0	9.0	
H	BMW1+w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
F	172	0	172	0	0	11-0-14	11-0-14		
B	257	0	257	0	0	11-0-14	11-0-14		
H	260	0	260	0	0	11-0-14	11-0-14		
G	582	0	582	0	0	11-0-14	11-0-14		

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	122	79 / 0	0 / 0	0 / 0	0 / 0	43 / 0	0 / 0
B	180	130 / 0	0 / 0	0 / 0	0 / 0	50 / 0	0 / 0
H	186	109 / 0	0 / 0	0 / 0	0 / 0	77 / 0	0 / 0
G	410	277 / 0	0 / 0	0 / 0	0 / 0	133 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PL)	LC1 MAX	MEMB.	FORCE (LBS)	MAX. UNBRAC LENGTH	LC1 MAX
FR-TO				FR-TO			
A-B	0 / 14	-91.8	-91.8 0.02 (1)	H-C	-163 / 0	10.00	0.03 (1)
B-J	-20 / 0	-91.8	-91.8 0.04 (1)	C-G	-69 / 0	6.25	0.02 (1)
J-C	-88 / 0	-91.8	-91.8 0.10 (1)	G-D	-484 / 0	6.25	0.08 (1)
C-D	0 / 3	-91.8	-91.8 0.25 (1)	G-E	-3 / 0	10.00	0.00 (1)
D-E	0 / 2	-91.8	-91.8 0.25 (1)	I-J	-249 / 0	10.00	0.00 (1)
F-E	-141 / 0	0.0	0.0 0.02 (1)			7.81	
B-I	0 / 82	-18.5	-18.5 0.09 (1)			10.00	
I-H	0 / 82	-18.5	-18.5 0.09 (1)			10.00	
H-G	0 / 56	-18.5	-18.5 0.08 (4)			10.00	
G-F	0 / 0	-18.5	-18.5 0.07 (4)			10.00	

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.6	PSF
	DL	= 6.0	PSF
BOT CH.	LL	= 0.0	PSF
	DL	= 7.4	PSF
TOTAL LOAD = 39.0 PSF			

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.25/1.00 (C-D:1), BC=0.09/1.00 (B-I:1), WB=0.08/1.00 (D-G:1), SS=0.19/1.00 (B-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (D) (INPUT = 0.90)

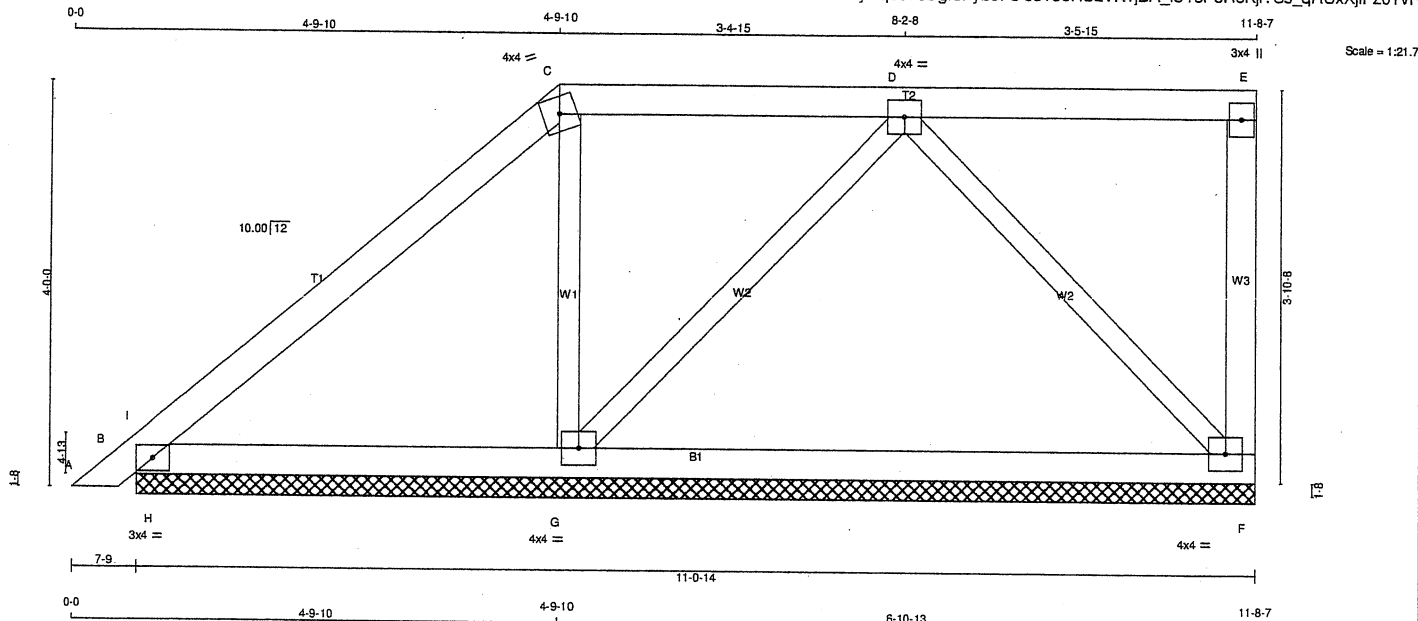
JSI METAL= 0.10 (D) (INPUT = 1.00)



Structural component only
DWG# T-2121146

JOB NAME 412868	TRUSS NAME PB03	QUANTITY 4	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:WTM7iolSBYniqocTeDgf3Pyb9F8-Je?36HUzVK?jBA_ioYsPeHoKji?Us_qRUxXjiPz0Tvr



LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMB1-I	MT20	3.0	4.0
C	TTW-m	MT20	4.0	4.0
D	TMW-w	MT20	4.0	4.0
E	TMW-p	MT20	3.0	4.0
F	BMVW1-I	MT20	4.0	4.0
G	BMVW1-I	MT20	4.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	391	0	391	0	0	11-0-14	11-0-14
B	311	0	311	0	0	11-0-14	11-0-14
G	568	0	568	0	0	11-0-14	11-0-14

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	275	190 / 0	0 / 0	0 / 0	0 / 0	85 / 0	0 / 0
B	216	162 / 0	0 / 0	0 / 0	0 / 0	54 / 0	0 / 0
G	406	242 / 0	0 / 0	0 / 0	0 / 0	163 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO		FR-TO	
A-B	0 / 14	-91.8 -91.8 0.02 (1)	10.00	G-C	-224 / 0
B-I	0 / 144	-91.8 -91.8 0.13 (1)	10.00	G-D	-233 / 0
I-C	-97 / 0	-91.8 -91.8 0.19 (1)	6.25	D-F	-309 / 0
C-D	-57 / 0	-91.8 -91.8 0.18 (1)	6.25	H-I	-519 / 0
D-E	0 / 0	-91.8 -91.8 0.18 (1)	10.00		
F-E	-122 / 0	0.0 0.0 0.03 (1)	7.81		
B-H	0 / 64	-18.5 -18.5 0.18 (1)	10.00		
H-G	0 / 64	-18.5 -18.5 0.20 (4)	10.00		
G-F	0 / 216	-18.5 -18.5 0.21 (4)	10.00		

TOTAL WEIGHT = 4 X 42 = 166 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.19/1.00 (C-I), BC=0.21/1.00 (F-G/4), WB=0.12/1.00 (D-F/1), SS=0.39/1.00 (B-H/1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

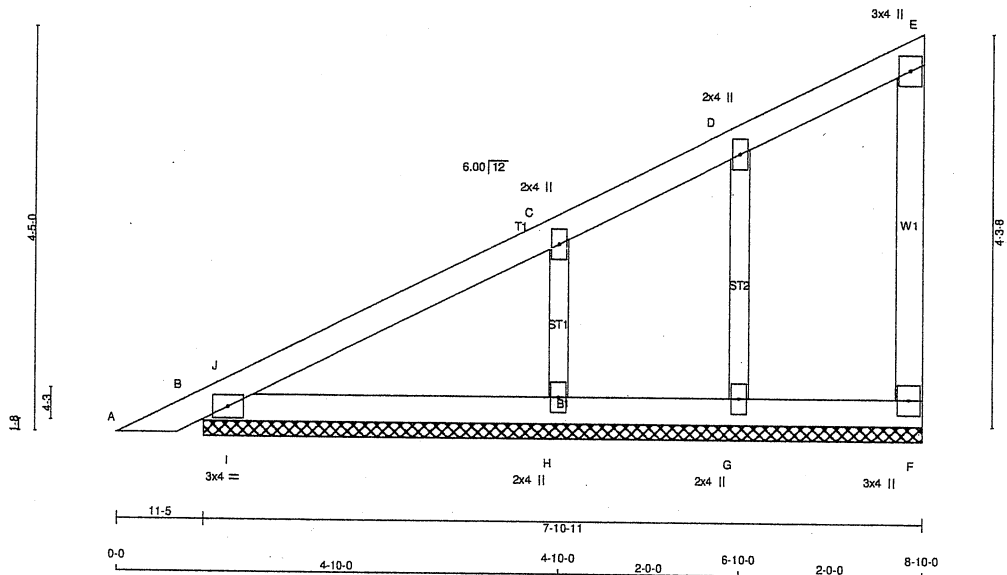
JSI GRIP= 0.26 (B) (INPUT = 0.90)
JSI METAL= 0.07 (F) (INPUT = 1.00)



Structural component only
DWG# T-2121147

JOB NAME 412868	TRUSS NAME PB04G	QUANTITY 4	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale: 1/2"=1'

TOTAL WEIGHT = 4 X 28 = 112 lb

LUMBER				
N, L, G, A. RULES	CHORDS	SIZE	DRY	LUMBER
A - E	2x4	DRY	No.2	
F - E	2x4	DRY	No.2	
B - F	2x4	DRY	No.2	
ALL WEBS	2x3	DRY	No.2	
ALL GABLE WEBS	2x3	DRY	No.2	
DRY: SEASONED LUMBER.				
GABLE STUDS SPACED AT 2-0-0 OC.				

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMB1-I	MT20	3.0	4.0
C	TMW+w	MT20	2.0	4.0
D	TMW+w	MT20	2.0	4.0
E	TMV+p	MT20	3.0	4.0
F	BMV1+p	MT20	3.0	4.0
G	BMV1+w	MT20	2.0	4.0
H	BMV1+w	MT20	2.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	UNBRAC LENGTH	MAX. FACTORED CSI (LC)
FR-TO				FR-TO			
A-B	0 / 17	-91.8	-91.8 0.05 (1)	G-D	-158 / 0	10.00	0.03 (1)
B-J	-17 / 0	-91.8	-91.8 0.02 (4)	H-C	-307 / 0	10.00	0.05 (1)
J-C	-4 / 1	-91.8	-91.8 0.13 (1)	I-J	-172 / 5	10.00	0.00 (1)
C-D	-21 / 0	-91.8	-91.8 0.13 (1)				
D-E	-5 / 0	-91.8	-91.8 0.05 (1)				
F-E	-84 / 0	0.0	0.0 0.02 (1)				
B-I	0 / 14	-18.5	-18.5 0.10 (1)				
I-H	0 / 14	-18.5	-18.5 0.10 (1)				
H-G	0 / 4	-18.5	-18.5 0.07 (1)				
G-F	0 / 0	-18.5	-18.5 0.02 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13/1.00 (C-D:1), BC=0.10/1.00 (B-I:1), WB=0.05/1.00 (C-H:1), SSI=0.14/1.00 (B-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (B) (INPUT = 0.90)
JSI METAL= 0.13 (C) (INPUT = 1.00)

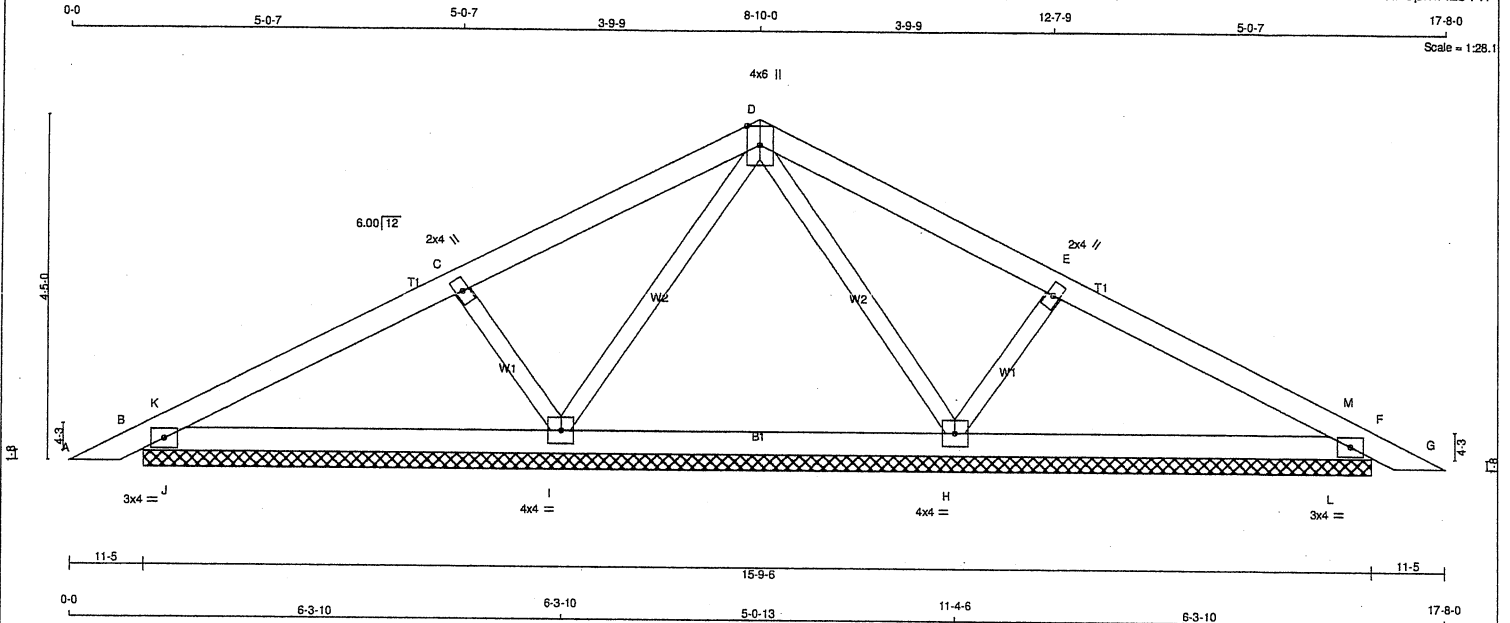


Structural component only
DWG# T-2121148

JOB NAME 412868	TRUSS NAME PB06	QUANTITY 18	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

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LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS 2x3 DRY No.2 SPF				
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	
C	TMW+w	MT20	2.0	4.0	
D	TTWW+p	MT20	4.0	6.0	Edge
E	TMW+w	MT20	2.0	4.0	
F	TMB1-I	MT20	3.0	4.0	
H	BMW1-I	MT20	4.0	4.0	
I	BMW1-I	MT20	4.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT		REQD	
JT	VERT	HORIZ	DOWN	HORIZ	UPLIFT	BRG	IN-SX
B	340	0	340	0	0	15-9-6	15-9-6
F	340	0	340	0	0	15-9-6	15-9-6
H	610	0	610	0	0	15-9-6	15-9-6
I	610	0	610	0	0	15-9-6	15-9-6

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	239	166 / 0	0 / 0	0 / 0	0 / 0	73 / 0	0 / 0
F	239	166 / 0	0 / 0	0 / 0	0 / 0	73 / 0	0 / 0
H	432	283 / 0	0 / 0	0 / 0	0 / 0	149 / 0	0 / 0
I	432	283 / 0	0 / 0	0 / 0	0 / 0	149 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, H, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	FR-TO
FR-TO				FR-TO			
A-B	0 / 17	-91.8	-91.8 0.05 (1)	D-H	-197 / 0	10.00	0.07 (1)
B-K	-199 / 0	-91.8	-91.8 0.06 (4)	H-E	-396 / 0	6.25	0.07 (1)
K-C	-143 / 0	-91.8	-91.8 0.23 (1)	I-D	-197 / 0	6.25	0.07 (1)
C-D	0 / 54	-91.8	-91.8 0.24 (1)	C-I	-396 / 0	10.00	0.07 (1)
D-E	0 / 54	-91.8	-91.8 0.24 (1)	J-K	-69 / 58	10.00	0.00 (1)
E-M	-143 / 0	-91.8	-91.8 0.23 (1)	L-M	-69 / 58	6.25	0.00 (1)
M-F	-199 / 0	-91.8	-91.8 0.06 (4)				
F-G	0 / 17	-91.8	-91.8 0.05 (1)				
B-J	0 / 149	-18.5	-18.5 0.08 (1)				
J-I	0 / 149	-18.5	-18.5 0.12 (4)				
I-H	0 / 40	-18.5	-18.5 0.11 (4)				
H-L	0 / 149	-18.5	-18.5 0.12 (4)				
L-F	0 / 149	-18.5	-18.5 0.08 (1)				

TOTAL WEIGHT = 18 X 51 = 925 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.24/1.00 (C-D:1), BC=0.12/1.00 (I-J:4), WB=0.07/1.00 (D-H:1), SSI=0.15/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.24 (B) (INPUT = 0.90)

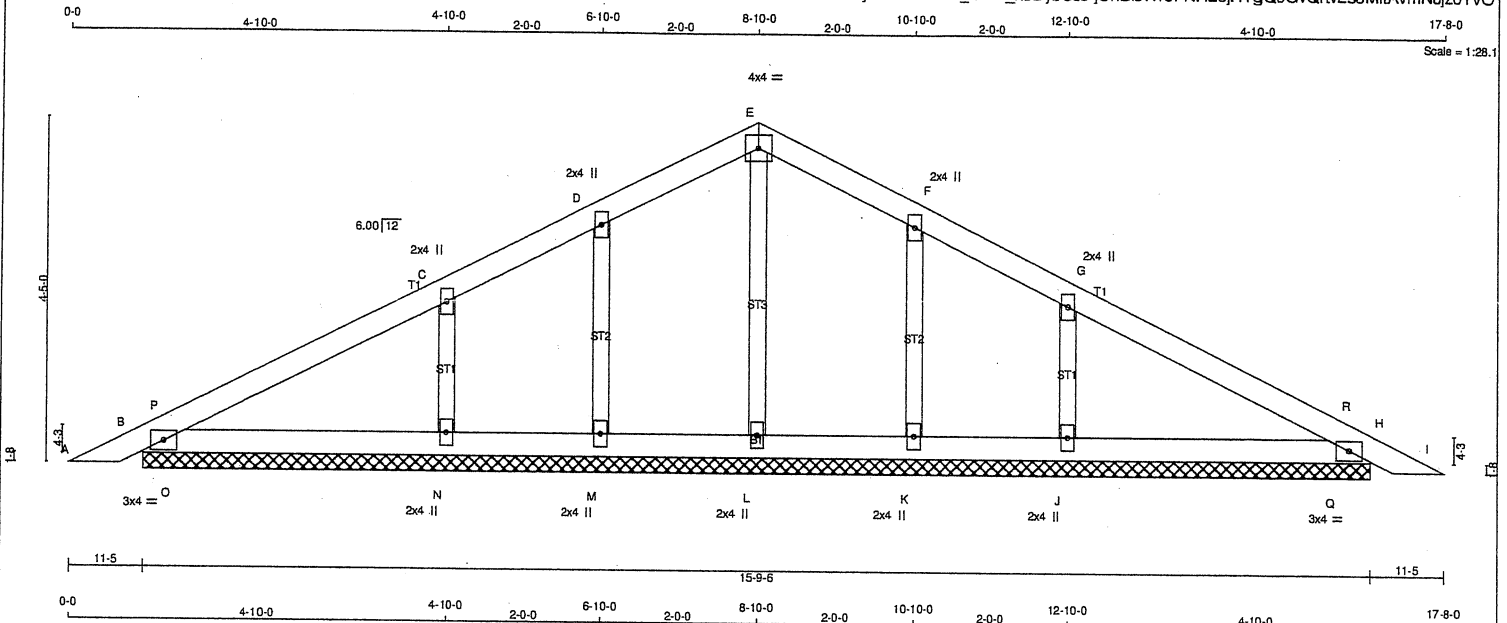
JSI METAL= 0.08 (E) (INPUT = 1.00)



Structural component only
DWG# T-2121149

JOB NAME 412868	TRUSS NAME PB06G	QUANTITY 4	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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LUMBER

N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - E	2x4	DRY	No.2
E - I	2x4	DRY	No.2
B - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

GABLE STUDS SPACED AT 2'-0" OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C, D, F, G						
C	TMW+w	MT20	2.0	4.0		
E	TTW-p	MT20	4.0	4.0		
H	TMB1-I	MT20	3.0	4.0		
J, K, L, M, N						
J	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	
FR-TO		FROM TO		FR-TO			
A-B	0 / 17	-91.8 -91.8	0.05 (1)	10.00	L-E	-131 / 0	0.04 (1)
B-P	-44 / 0	-91.8 -91.8	0.02 (4)	6.25	M-D	-160 / 0	0.03 (1)
P-C	-44 / 0	-91.8 -91.8	0.13 (1)	6.25	N-C	-306 / 0	0.05 (1)
C-D	-60 / 0	-91.8 -91.8	0.13 (1)	6.25	K-F	-160 / 0	0.03 (1)
D-E	-45 / 0	-91.8 -91.8	0.05 (1)	6.25	J-G	-306 / 0	0.05 (1)
E-F	-45 / 0	-91.8 -91.8	0.05 (1)	6.25	O-P	-171 / 5	0.00 (1)
F-G	-60 / 0	-91.8 -91.8	0.13 (1)	6.25	Q-R	-171 / 5	0.00 (1)
G-R	-44 / 0	-91.8 -91.8	0.13 (1)	6.25			
R-H	-44 / 0	-91.8 -91.8	0.02 (4)	6.25			
H-I	0 / 17	-91.8 -91.8	0.05 (1)	10.00			
B-O	0 / 49	-18.5 -18.5	0.10 (1)	10.00			
O-N	0 / 49	-18.5 -18.5	0.10 (1)	10.00			
N-M	0 / 39	-18.5 -18.5	0.07 (1)	10.00			
M-L	0 / 36	-18.5 -18.5	0.02 (4)	10.00			
L-K	0 / 36	-18.5 -18.5	0.02 (4)	10.00			
K-J	0 / 39	-18.5 -18.5	0.07 (1)	10.00			
J-Q	0 / 49	-18.5 -18.5	0.10 (1)	10.00			
Q-H	0 / 49	-18.5 -18.5	0.10 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13/1.00 (G-R:1), BC=0.10/1.00 (H-Q:1), WB=0.05/1.00 (G-J:1), SSI=0.14/1.00 (B-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

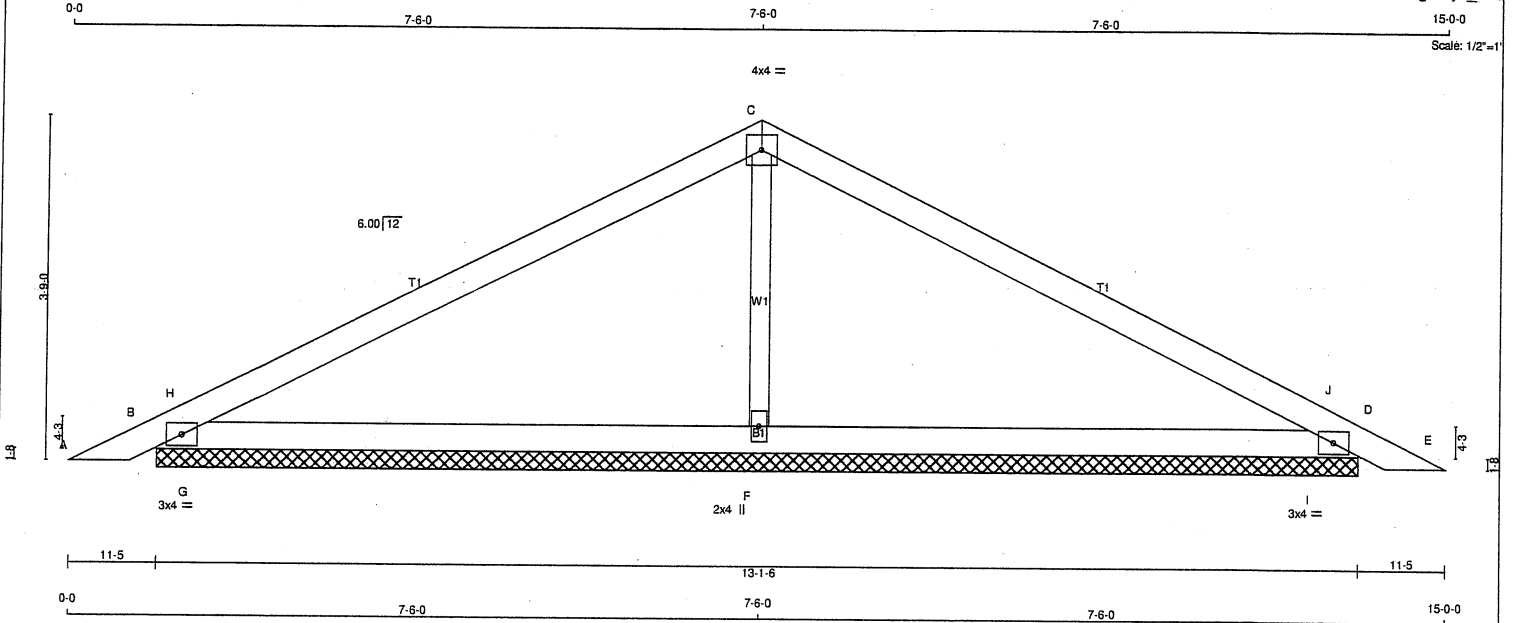
JSI GRIP = 0.21 (B) (INPUT = 0.90)
JSI METAL = 0.13 (G) (INPUT = 1.00)



Structural component only
DWG# T-2121150

JOB NAME 413062	TRUSS NAME PB32	QUANTITY 6	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
B - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	
C	TTW-p	MT20	4.0	4.0	
D	TMB1-I	MT20	3.0	4.0	
F	BMW1+w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
B	517	0	517	0	0
D	517	0	517	0	0
F	574	0	574	0	0

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	362	256 / 0	0 / 0	0 / 0	0 / 0	106 / 0	0 / 0
D	362	256 / 0	0 / 0	0 / 0	0 / 0	106 / 0	0 / 0
F	409	249 / 0	0 / 0	0 / 0	0 / 0	160 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRAC LENGTH	MAX. FACTORED FORCE (LBS)
FR-TO		FROM	TO	FR-TO			
A-B	0 / 17	-91.8	-91.8 0.05 (1)	10.00	F-C	-284 / 0	0.06 (1)
B-H	-62 / 0	-91.8	-91.8 0.24 (1)	6.25	G-H	-744 / 0	0.00 (1)
H-C	-262 / 0	-91.8	-91.8 0.49 (1)	6.25	I-J	-744 / 0	0.00 (1)
C-J	-262 / 0	-91.8	-91.8 0.49 (1)	6.25			
J-D	-62 / 0	-91.8	-91.8 0.24 (1)	6.25			
D-E	0 / 17	-91.8	-91.8 0.05 (1)	10.00			
B-G	0 / 223	-18.5	-18.5 0.43 (1)	10.00			
G-F	0 / 223	-18.5	-18.5 0.43 (1)	10.00			
F-I	0 / 223	-18.5	-18.5 0.43 (1)	10.00			
I-D	0 / 223	-18.5	-18.5 0.43 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LIVE LOAD)

CSI: TC=0.49/1.00 (C-H:1), BC=0.43/1.00 (B-G:1), WB=0.06/1.00 (C-F:1), SSL=0.56/1.00 (D-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.45 (D) (INPUT = 0.90)
JSI METAL = 0.12 (B) (INPUT = 1.00)



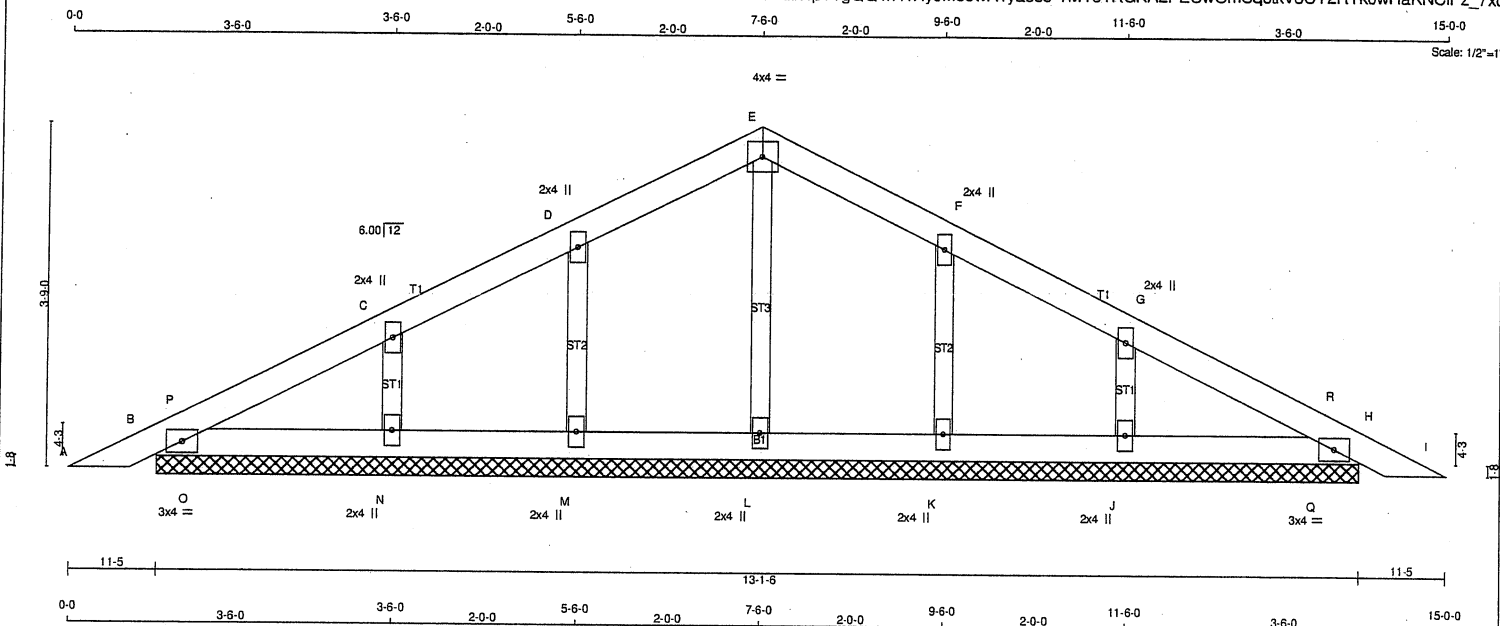
Structural component only
DWG# T-2121941

JOB NAME 413062	TRUSS NAME PB32G	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:Zk4pT?gQQ4nYrHy9Mo5wAtYaos6-TMY31RGKA2PECwOmSqctkV8CY2RTk0wHaKNCiPz_7xc



TOTAL WEIGHT = 42 lb

LUMBER				
N. L. G. A. RULES	SIZE	LUMBER	DESCR.	SPF
CHORDS				
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
B - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				
GABLE STUDS SPACED AT 2-0-0 OC.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	
C, D, F, G					
C	TMW+w	MT20	2.0	4.0	
E	TTW-p	MT20	4.0	4.0	
H	TMB1-I	MT20	3.0	4.0	
J, K, L, M, N					
J	BMW1+w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED UNBRACED LENGTH	MAX. FACTORED LC1 CSI (LC)
FR-TO		FROM TO		FR-TO			
A-B	0 / 17	-91.8 -91.8	0.05 (1)	10.00	L-E	-135 / 0	0.03 (1)
B-P	-47 / 0	-91.8 -91.8	0.01 (4)	6.25	M-D	-198 / 0	0.03 (1)
P-C	-30 / 0	-91.8 -91.8	0.06 (1)	6.25	N-C	-208 / 0	0.03 (1)
C-D	-30 / 0	-91.8 -91.8	0.06 (1)	6.25	K-F	-198 / 0	0.03 (1)
D-E	-30 / 0	-91.8 -91.8	0.05 (1)	6.25	J-G	-208 / 0	0.03 (1)
E-F	-30 / 0	-91.8 -91.8	0.05 (1)	6.25	O-P	-78 / 3	0.00 (1)
F-G	-30 / 0	-91.8 -91.8	0.06 (1)	6.25	Q-R	-78 / 3	0.00 (1)
G-R	-30 / 0	-91.8 -91.8	0.06 (1)	6.25			
R-H	-47 / 0	-91.8 -91.8	0.01 (4)	6.25			
H-I	0 / 17	-91.8 -91.8	0.05 (1)	10.00			
B-O	0 / 36	-18.5 -18.5	0.05 (1)	10.00			
O-N	0 / 36	-18.5 -18.5	0.05 (1)	10.00			
N-M	0 / 25	-18.5 -18.5	0.03 (1)	10.00			
M-L	0 / 20	-18.5 -18.5	0.02 (4)	10.00			
L-K	0 / 20	-18.5 -18.5	0.02 (4)	10.00			
K-J	0 / 25	-18.5 -18.5	0.03 (1)	10.00			
J-Q	0 / 36	-18.5 -18.5	0.05 (1)	10.00			
Q-H	0 / 36	-18.5 -18.5	0.05 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP. CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT. CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.06/1.00 (G-R:1), BC=0.05/1.00 (N-O:1), WB=0.03/1.00 (D-M:1), SSI=0.08/1.00 (G-R:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

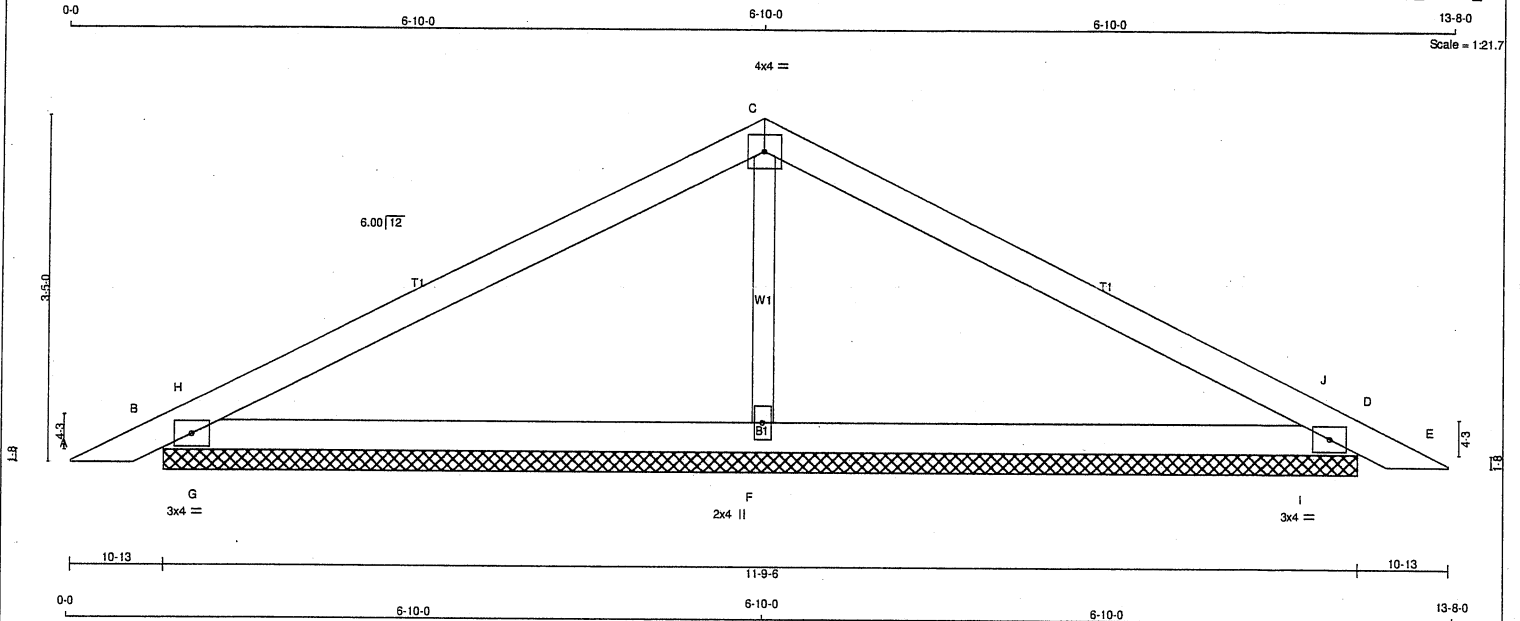
JSI GRIP= 0.15 (H) (INPUT = 0.90)
JSI METAL= 0.09 (C) (INPUT = 1.00)



Structural component only
DWG# T-2121942

JOB NAME 413062	TRUSS NAME PB81	QUANTITY 6	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.420 S Jan 21 2021 MITek Industries, Inc. Thu Jul 8 13:00:08 2021 Page 1
ID:wzdEniYwIMtNFy9B0RQea5yaUCB-xY6RFnHyxLX5p4zy0Y86HigEIReNTSMQp_6mErz_7xb



TOTAL WEIGHT = 6 X 33 = 198 lb

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	
C	TTW-p	MT20	4.0	4.0	
D	TMB1-I	MT20	3.0	4.0	
F	BMW1+w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
B	723	0	723	0	11-9-6
D	723	0	723	0	11-9-6
F	796	0	796	0	11-9-6

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN.	COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	498	403 / 0	0 / 0	0 / 0	0 / 0	95 / 0	0 / 0
D	498	403 / 0	0 / 0	0 / 0	0 / 0	95 / 0	0 / 0
F	555	409 / 0	0 / 0	0 / 0	0 / 0	147 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 26	-150.3 -150.3	0.08 (1)	10.00	F-C	-446 / 0	0.08 (1)
B-H	-53 / 0	-150.3 -150.3	0.32 (1)	6.25	G-H	-1038 / 0	0.00 (1)
H-C	-346 / 0	-150.3 -150.3	0.63 (1)	6.25	I-J	-1038 / 0	0.00 (1)
C-J	-346 / 0	-150.3 -150.3	0.63 (1)	6.25			
J-D	-53 / 0	-150.3 -150.3	0.32 (1)	6.25			
D-E	0 / 26	-150.3 -150.3	0.08 (1)	10.00			
B-G	0 / 293	-18.5 -18.5	0.58 (1)	10.00			
G-F	0 / 293	-18.5 -18.5	0.58 (1)	10.00			
F-I	0 / 293	-18.5 -18.5	0.58 (1)	10.00			
I-D	0 / 293	-18.5 -18.5	0.58 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 45.1	PSF
	DL	= 6.0	PSF
BOT CH.	LL	= 0.0	PSF
	DL	= 7.4	PSF
TOTAL LOAD	=	58.5	PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 66.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 45.1 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.63/1.00 (C-J:1), BC=0.58/1.00 (F-I:1), WB=0.08/1.00 (C-F:1), SSI=0.76/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.65 (D) (INPUT = 0.90)
JSI METAL= 0.16 (D) (INPUT = 1.00)

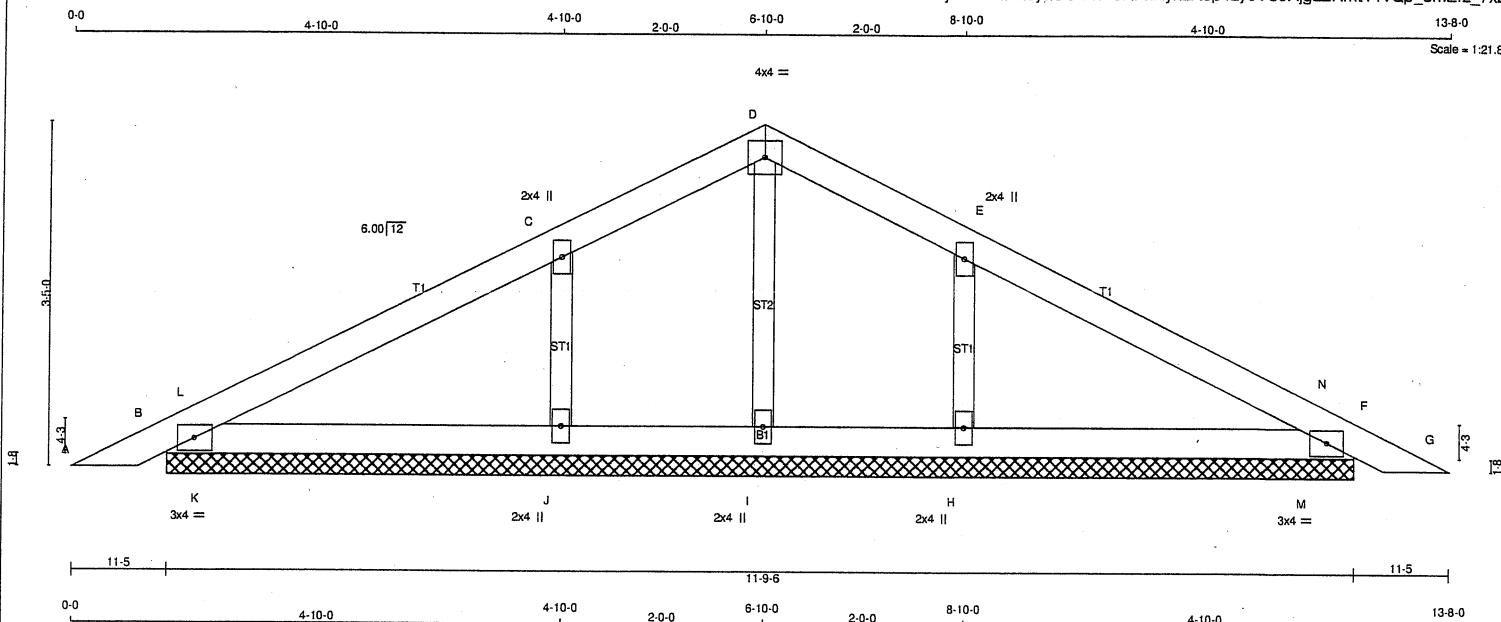


Structural component only
DWG# T-2121943

JOB NAME 413062	TRUSS NAME PB81G	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MTek Industries, Inc. Thu Jul 8 13:00:08 2021 Page 1
ID:wzdEniYwIMtNFy9B0RQea5yaUCB-xY6RFnHyyLX5p4zy0Y86HjgL2RmtTTvQp_6mErz_7xb



TOTAL WEIGHT = 36 lb

LUMBER				
N. L. G. A. RULES	SIZE	LUMBER	DESCR.	
CHORDS				
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				
GABLE STUDS SPACED AT 2-0-0 OC.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	
C	TMW+w	MT20	2.0	4.0	
D	TTW-p	MT20	4.0	4.0	
E	TMW+w	MT20	2.0	4.0	
F	TMB1-I	MT20	3.0	4.0	
H, I, J					
H	BMW1+w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX LC1 (LC)	MAX. UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX LC1 (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 17	-91.8 -91.8	0.05 (1)	10.00	I-D	-58 / 0	0.01 (1)
B-L	-38 / 0	-91.8 -91.8	0.02 (4)	6.25	J-C	-322 / 0	0.05 (1)
L-C	-37 / 0	-91.8 -91.8	0.14 (1)	6.25	H-E	-322 / 0	0.05 (1)
C-D	-59 / 0	-91.8 -91.8	0.14 (1)	6.25	K-L	-169 / 5	0.00 (1)
D-E	-59 / 0	-91.8 -91.8	0.14 (1)	6.25	M-N	-169 / 5	0.00 (1)
E-N	-37 / 0	-91.8 -91.8	0.14 (1)	6.25			
N-F	-38 / 0	-91.8 -91.8	0.02 (4)	6.25			
F-G	0 / 17	-91.8 -91.8	0.05 (1)	10.00			
B-K	0 / 44	-18.5 -18.5	0.10 (1)	10.00			
K-J	0 / 44	-18.5 -18.5	0.10 (1)	10.00			
J-I	0 / 33	-18.5 -18.5	0.08 (1)	10.00			
I-H	0 / 33	-18.5 -18.5	0.08 (1)	10.00			
H-M	0 / 44	-18.5 -18.5	0.10 (1)	10.00			
M-F	0 / 44	-18.5 -18.5	0.10 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (C-L:1), BC=0.10/1.00 (H-M:1), WB=0.05/1.00 (C-J:1), SSI=0.14/1.00 (B-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

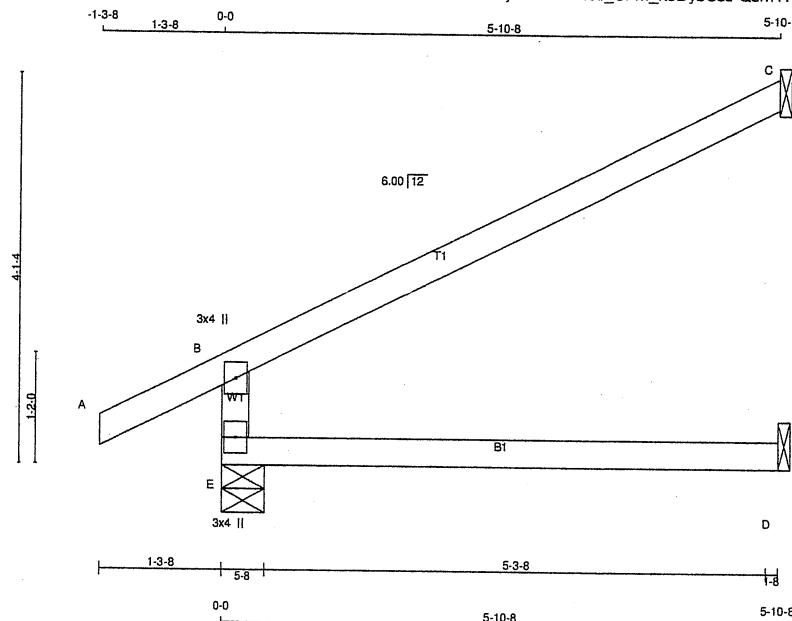
JSI GRIP= 0.21 (F) (INPUT = 0.90)
JSI METAL= 0.13 (C) (INPUT = 1.00)



Structural component only
DWG# T-2121944

JOB NAME 412868	TRUSS NAME J01	QUANTITY 20	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Thu Jul 1 10:22:22 2021 Page 1
ID:U6yi?rbeFFwxf_UFm_koDybSsJ-QsmYHwRSR5VHJZhXZioTureZH4eowBkrZJZVZdz0TvV



Scale = 1:23.1

TOTAL WEIGHT = 20 X 17 = 336 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
E - B	2x4 DRY	No.2	SPF
A - C	2x4 DRY	No.2	SPF
E - D	2x4 DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	JT	GROSS REACTION	BRG	BRG	IN-SX	IN-SX
E	525	0	525	0	5-8	5-8	5-8
C	202	0	202	0	1-8	1-8	1-8
D	45	0	50	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
E	369	257 / 0	0 / 0	0 / 0	111 / 0	0 / 0
C	139	113 / 0	0 / 0	0 / 0	26 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	MAX. FORCE (LBS)	VERT. LOAD (PL)	LC1 MAX (LC)	UNBRAC LENGTH FR-TO	MEMB.	MAX. FORCE (LBS)	MAX. (LC)
FR-TO		FROM	TO		FR-TO		
E-B	-461 / 0	0.0	0.0	0.13 (4)	7.81		
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00		
B-C	-30 / 0	-91.8	-91.8	0.54 (1)	6.25		
E-D	0 / 0	-18.5	-18.5	0.13 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.54/1.00 (B-C:1), BC=0.13/1.00 (D-E:4),
WB=0.00/1.00 (n/a:0), SSI=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.18 (E) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)



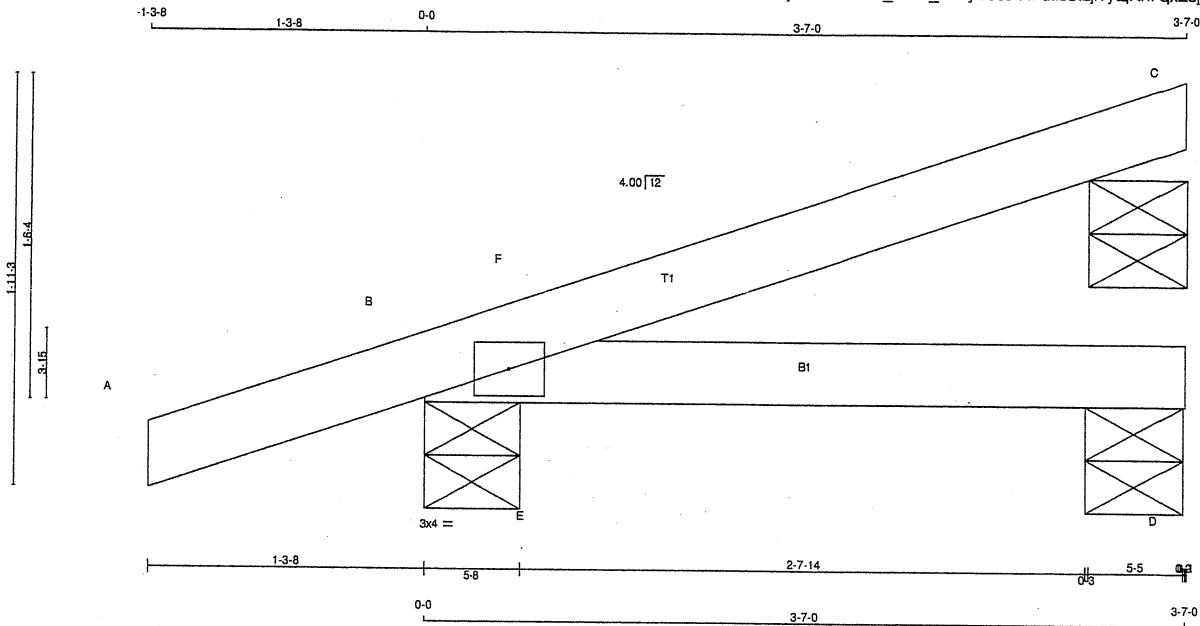
Structural component only
DWG# T-2121142

Structural component only
DWG# T-2121143

JOB NAME 412868	TRUSS NAME J03	QUANTITY 15	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MTek Industries, Inc. Thu Jul 1 10:22:24 2021 Page 1
ID:U6yi?rbeFFwkxf_UFm_koDybSsJ-NFulicSizj?ytqKh7qxZSj_uuLTO5E80d2cdWz0Tvt

Scale = 1:10.3



LUMBER
N. L. G. A. RULES
CHORDS SIZE DRY LUMBER No.2
A - C 2x4 DRY No.2
B - D 2x4 DRY No.2
DESCR. SPF
SPF

PLATES (table is in inches)
JT TYPE PLATES W LEN Y X
B TMB1-I MT20 3.0 4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
C	141	0	141	0	0
B	320	0	320	0	0
D	57	0	57	0	0

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): C

UNFACTORED REACTIONS

1ST LCASE	MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
C	98	76 / 0	0 / 0	0 / 0	0 / 0	22 / 0
B	224	160 / 0	0 / 0	0 / 0	0 / 0	64 / 0
D	42	16 / 0	0 / 0	0 / 0	0 / 0	26 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 18	A-B	0 / 18
B-F	-10 / 0	B-F	-138 / 5
F-C	0 / 2	F-C	0.00 (1)
B-E	0 / 0	B-E	0 / 0
E-D	0 / 0	E-D	0 / 0

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
DL	=	8.0	PSF	
BOT CH.	LL	=	0.0	PSF
DL	=	7.4	PSF	
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.15/1.00 (C-F:1), BC=0.12/1.00 (B-E:1),
WB=0.00/1.00 (E-F:1), SS=0.11/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (B) (INPUT = 0.90)

JSI METAL= 0.05 (B) (INPUT = 1.00)



Structural component only
DWG# T-2121144



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components

2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.

3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.

4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.

5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.

2- Lumber is to be the sizes and grade specified on the truss drawing.

3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.

4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings

5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.

6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)

7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.

8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1800213

Feb 09, 2018

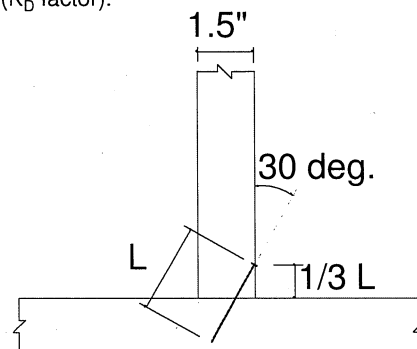
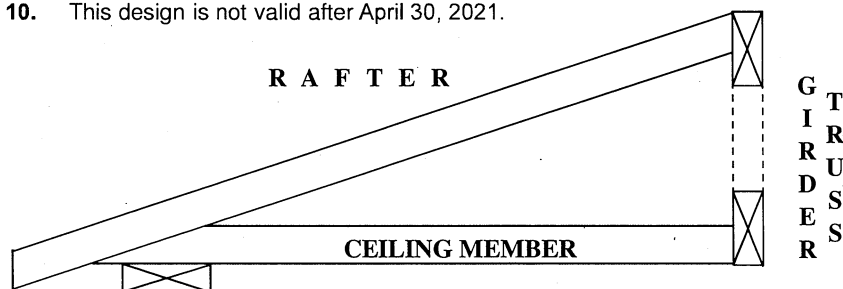
BEARING ANCHORAGE BY TOE-NAILS FOR LATERAL CAPACITY

B97791H1

NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL LATERAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	132	147
	3.25	0.144	132	147
	3.50	0.160	159	177
COMMON SPIRAL	3.00	0.122	97	108
	3.25	0.122	97	108
	3.50	0.152	145	162

NOTES:

1. Rafter and ceiling members may be anchored to top and bottom chords of girder truss by toe-nailing rafter and ceiling members to girder chords provided the reaction does not exceed the lateral capacities in the table. Hangers (specified by others) are required for reactions higher than the maximum toe-nail capacity. Reactions are based on factored loads.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.4.1.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in tables below.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See next page for nailing on bearing plate).
7. For loads due to **wind** the nail lateral capacity in this table may be multiplied by 1.15 (K_D factor).
8. Lumber must be dry (< 19% moisture content) at the time of nail installation.
9. Nail values in this table comply with CSA O86-14, section 12.9.4
10. This design is not valid after April 30, 2021.



TOE-NAIL INSTALLATION

Nail type	Common wire	Common spiral	Common wire	Common spiral
Nail dia. (in)	0.160	0.152	0.144	0.122
	(3.5" nail)		(3" and 3.25" nail)	
LUMBER SIZE	MAXIMUM NUMBER OF TOE-NAILS			
2X4 SPF	2	2	3	3
2X4 D. Fir	2	2	2	2
2X6 SPF	4	4	4	5
2X6 D. Fir	3	3	3	4

PEO
Certificate No. 10889485



MiTek® MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

April 2, 2020

BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING

B97791H2

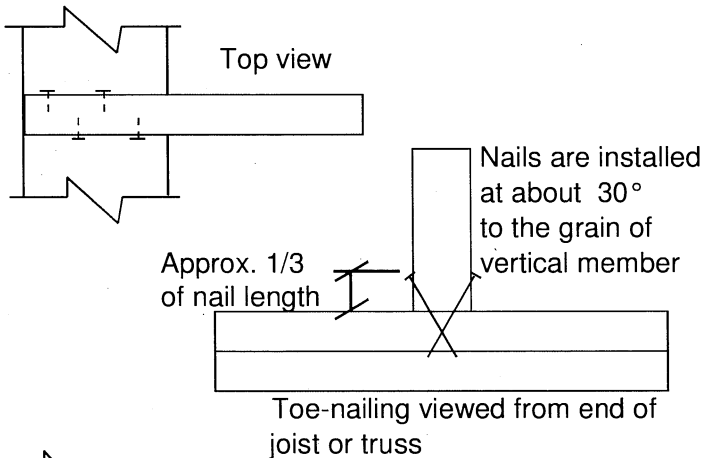
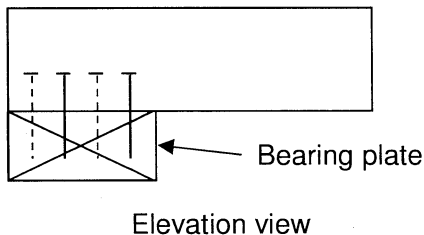
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	30	42
	3.25	0.144	32	45
	3.50	0.160	38	52
COMMON SPIRAL	3.00	0.122	26	36
	3.25	0.122	28	40
	3.50	0.152	36	50

Note: If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

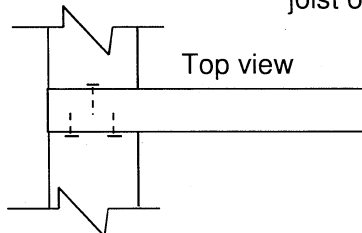
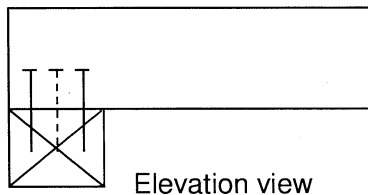
NOTES:

1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to **wind** or **earthquake** load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.5.2.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities: G = 0.42(SPF), G = 0.49(D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37579H1).
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-14, section 12.9.5
9. This design is not valid after April 30, 2021.

Toe-nailing on 2x6 Bearing Plate



Toe-nailing on 2x4 Bearing Plate



MiTek

MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

April 2, 2020

PEO
Certificate No. 10889485



HUS/LJS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

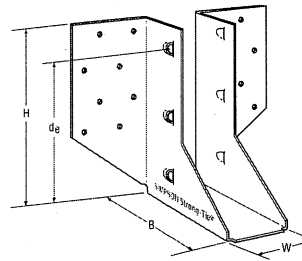
- Factored resistances are in accordance with CSA O86 -14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

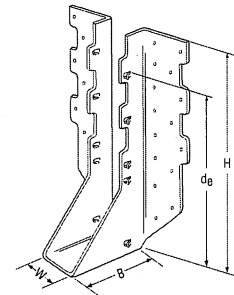
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

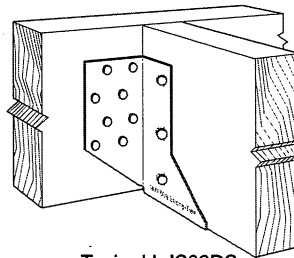
- See current catalogue for options



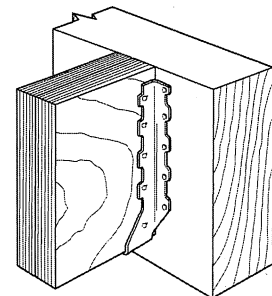
LJS26DS



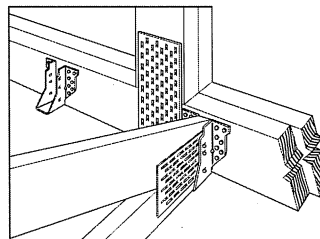
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



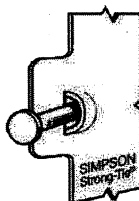
Typical HUS
Installation



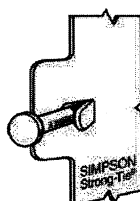
Typical HUS Installation
(Truss Designer to provide fastener
quantity for connecting multiple
members together)

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _g ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _o =1.15)	Normal (K _o =1.00)	Uplift (K _o =1.15)	Normal (K _o =1.00)
LJS26DS	18	1½	5	3½	4⅝	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1½	5⅝	3	3⅝	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1½	7½	3	6⅝	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1½	9⅝	3	7⅝	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1⅜	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

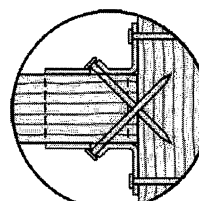
1. d_g is the distance from the seat of the hanger to the highest joist nail.



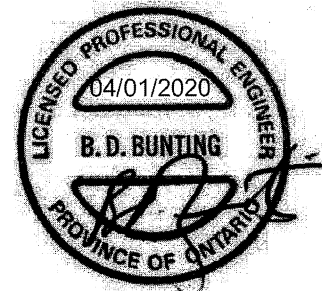
Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).
U.S. Patent
5,603,580



Double
Shear
Nailing
Side View.
Do not
bend tab
back.



Double
Shear
Nailing
Top View.



LIMIT
STATES
DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

© 2020 Simpson Strong-Tie Company Inc.

T-SPEC HUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

HGUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

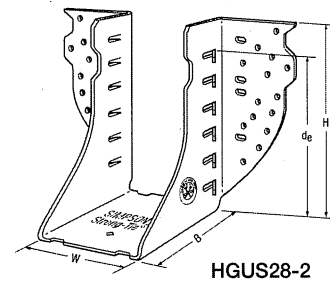
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

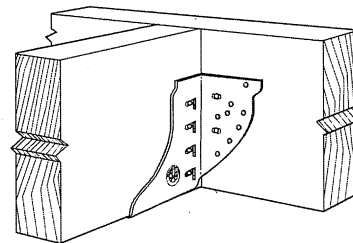
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

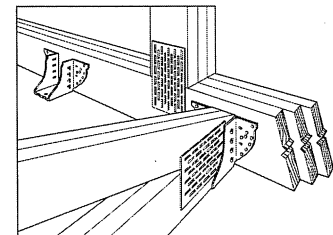
- See current catalogue for options



HGUS28-2



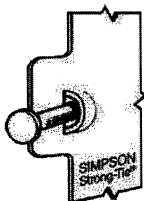
Typical HGUS Installation



Typical HGUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

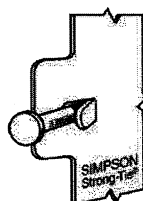
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K ₀ =1.15)	Normal (K ₀ =1.00)	Uplift (K ₀ =1.15)	Normal (K ₀ =1.00)
HGUS26	12	1⅝	5⅜	5	4⅝	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3⅝	5⅝	4	4⅝	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4⅝	5½	4	4⅝	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6⅝	5⅝	4	4⅝	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1⅝	7⅝	5	6⅝	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3⅝	7⅝	4	6⅝	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4⅝	7¼	4	6⅝	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6⅝	7⅝	4	6⅝	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210	12	1⅝	9⅝	5	7⅝	(46) 16d	(16) 16d	3535	11070	2510	8090
HGUS210-2	12	3⅝	9⅝	4	8⅝	(46) 16d	(16) 16d	6840	14015	4855	10270
HGUS210-3	12	4⅝	9¼	4	8⅝	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6⅝	9⅝	4	8⅝	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6⅝	10⅝	4	10⅝	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6⅝	12⅝	4	11⅝	(66) 16d	(22) 16d	10130	16400	7195	11645

1. d_e is the distance from the seat of the hanger to the highest joist nail.

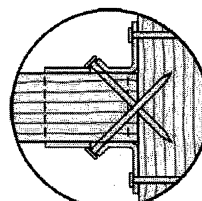


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent
5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT
STATES
DESIGN

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T-SPECHGUS20 3/20 exp. 6/22

(800) 999-5099
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H – Seismic and Hurricane Ties

SIMPSON
Strong-Tie

The H connector series provides wind and seismic ties for trusses and rafters.

Material: 18 gauge **Finish:** G90 galvanized

Design: • Factored resistances are in accordance with CSA O86-14

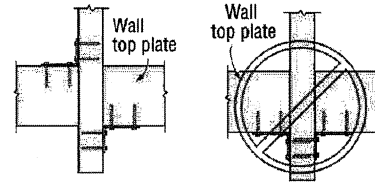
- Factored resistances have been increased 15%. No further increase is permitted.

Installation: • Use all specified fasteners

- Nails: 8d = 0.131" dia. x 2½" long common wire, 8d x 1½" = 0.131" x 1½" long, 10d x 1½" = 0.146" x 1½" long
- H1 can be installed with flanges facing outwards
- Hurricane ties do not replace solid blocking

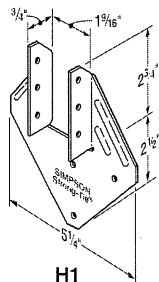
Factored resistances for more than one direction for a single connection cannot be added together. A factored load which can be divided into components in the directions given must be evaluated as follows: Factored Shear/Resisting Shear + Factored Tension/Resisting Tension ≤ 1.0.

Hurricane Tie Installations to Achieve Twice the Load (Top View)

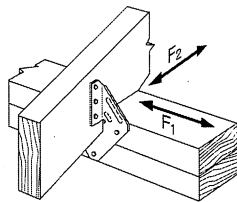


Install diagonally across from each other for minimum 2x truss.

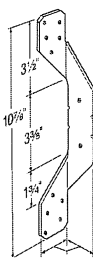
Nailing into both sides of a single ply 2x truss may cause the wood to split.



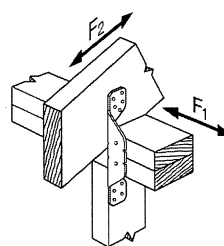
H1



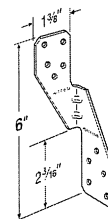
H1 Installation



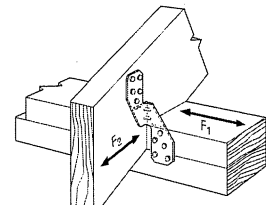
H2A



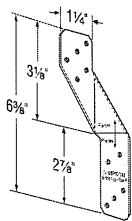
H2A Installation



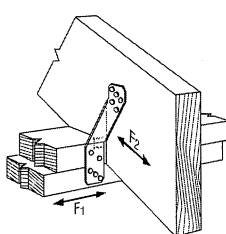
H2.5A



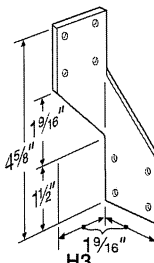
H2.5A Installation



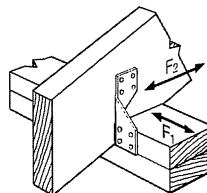
H2.5T



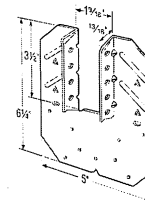
H2.5T Installation
(Nails into both top plates)



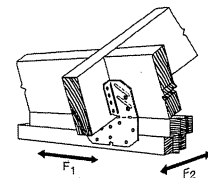
H3



H3 Installation



H10A



H10A Installation

Model No.	Ga.	Fasteners			Factored Resistance (lb.)					
					D.Fir-L			S-P-F		
		To Rafter	To Plates	To Studs	Uplift	Normal		Uplift	Normal	
						F ₁	F ₂		F ₁	F ₂
						(K ₀ =1.15)			(K ₀ =1.15)	
H1	18	(6) 8d x 1½"	(4) 8d	—	740	685	300	680	485	215
H2A	18	(5) 8d x 1½"	(2) 8d x 1½"	(5) 8d x 1½"	830	220	75	590	155	55
H2.5A	18	(5) 8d	(5) 8d	—	805	160	160	755	160	160
H2.5T	18	(5) 8d	(5) 8d	—	835	175	240	740	160	210
H3	18	(4) 8d	(4) 8d	—	740	180	265	615	125	190
H10A	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1735	795	410	1505	565	290

1. Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.
2. Factored resistances are for one anchor. A minimum rafter thickness of 2½" must be used when framing anchors are installed on each side of the joist and on the same side of the plate.

3. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.
4. Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a Continuous Load Path, connections must be on same side of the wall.



LIMIT
STATES
DESIGN

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T-SPECH20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

LUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

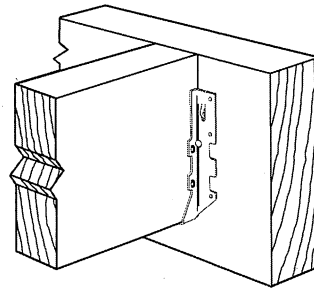
Finish: G90 galvanized

Design:

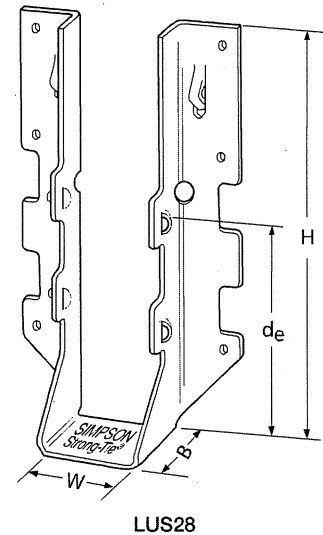
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.



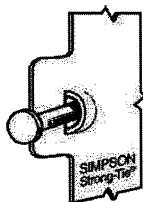
Typical LUS
Installation



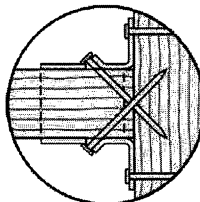
LUS28

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K ₀ =1.15)	Normal (K ₀ =1.00)	Uplift (K ₀ =1.15)	Normal (K ₀ =1.00)
LUS24	18	1⅞	3⅞	1¾	1⅞	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3⅞	3⅞	2	1⅞	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1⅞	4¾	1¾	3⅞	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3⅞	4¾	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4⅞	4¾	2	3¼	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1⅞	6⅞	1¾	3¾	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3⅞	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4⅞	6¼	2	3¼	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1⅞	7⅞	1¾	3⅞	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3⅞	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4⅞	8⅞	2	5¼	(8) 16d	(6) 16d	2580	3345	2320	2375

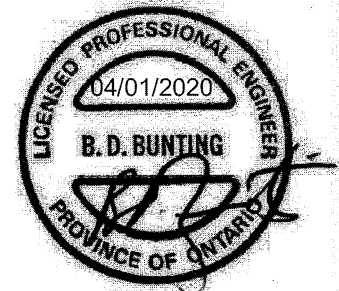
1. d_e is the distance from the seat of the hanger to the highest joist nail.



Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).
U.S. Patent
5,603,580



Double
Shear
Nailing
Top View.



LIMIT
STATES
DESIGN

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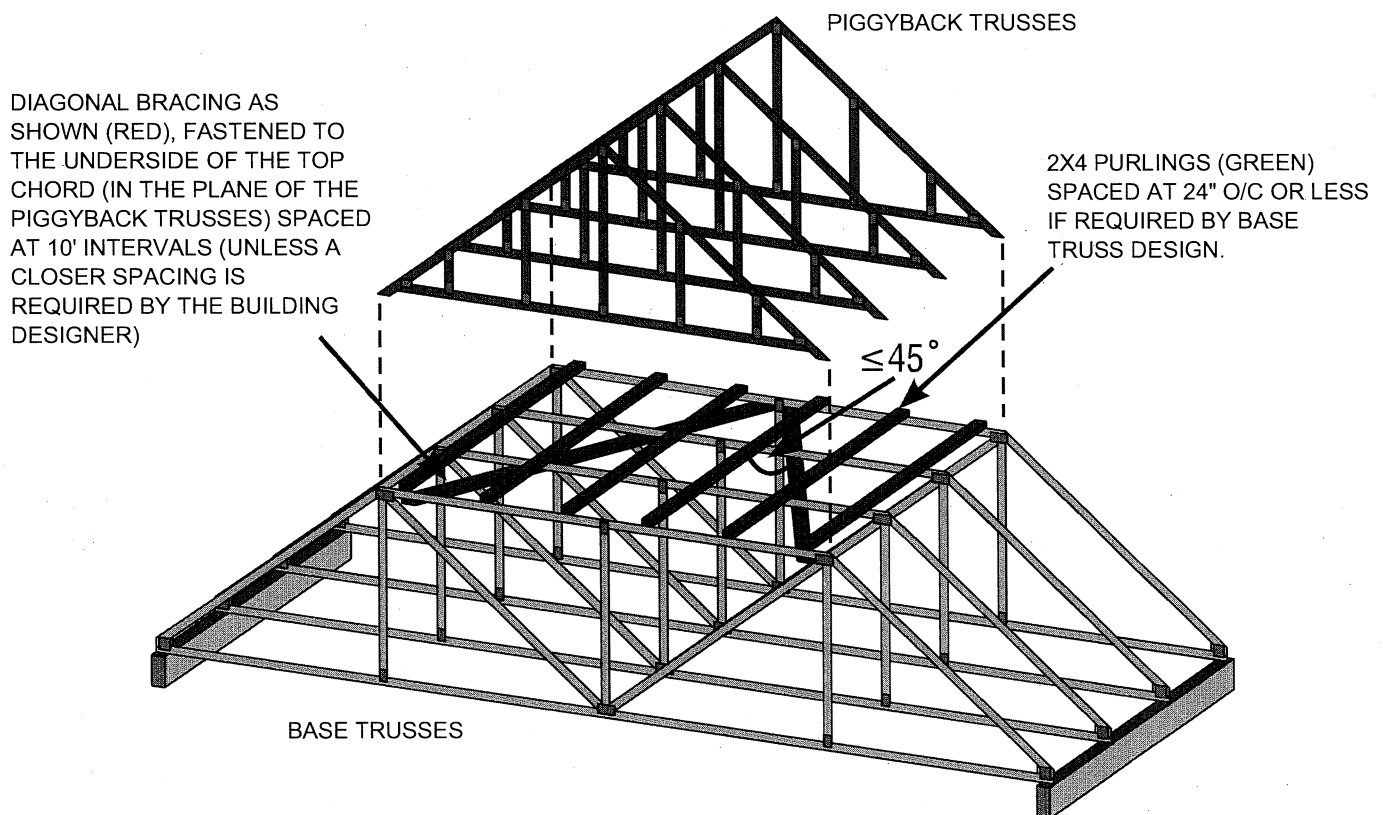


Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

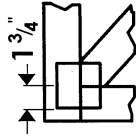
Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

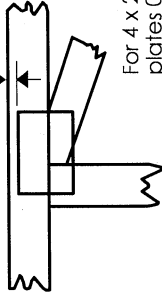
Symbols

PLATE LOCATION AND ORIENTATION

Center plate on joint unless x, y offsets are indicated.
Dimensions are in ft-in-sixteenths or mm.
Apply plates to both sides of truss and fully embed teeth.



0 1/16"



For 4 x 2 orientation, locate plates 0 1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.



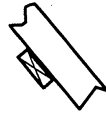
* Plate location details available in MiTek software or upon request.

PLATE SIZE

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

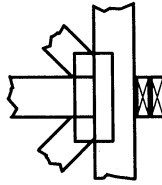
4 X 4

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use 1, I or Eliminator bracing if indicated.

BEARING



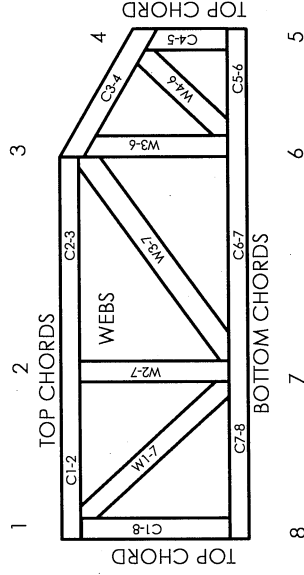
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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MiTek
POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MII-7473C rev. 10-'08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.