

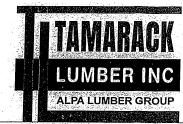
Connector Summary						
Qty	Manuf	Product				
6	H1	IUS2.56/11.88				
4	H1	IUS2.56/11.88				
1	H2	HUS1.81/10				

CITY OF RICHMOND HILL BUILDING DIVISION

08/13/2021

RECEIVED

Per: laura.dalberto



FROM PLAN DATED:

BUILDER: ROYAL PINE HOMES

SITE: CENTREFIELD WEST GORMLEY

MODEL: 2007

ELEVATION: A.B.

LOT:

CITY: RICHMAND HILL

SALESMAN: MARIO DICIANO

DESIGNER: AJ **REVISION:**

NOTES:

REFER TO THE NORDIC INSTALLATION GUIDE FOR PROPER STORAGE AND

INSTALLATION. **SQUASH BLOCKS** OF 2x4, 2x6, 2x8 #2 S.P.F REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. MULTIPLE SQUASH **BLOCKS** REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. CANTILEVERED JOISTS INCLUDING CANT' OVER BRICK REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR HOLES INCLUDING DUCT CHASE AND FIELD CUT OPENINGS SEE FIGURE 7, TABLES 1 & 2. CERAMIC TILE APPLICATION AS PER O.B.C 9.30.6.

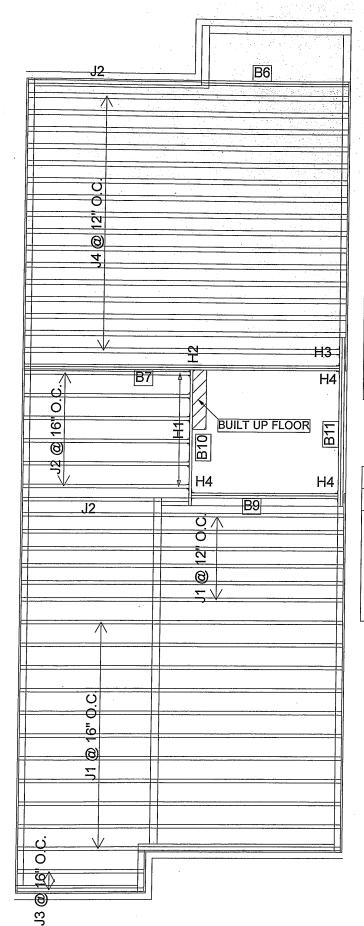
LOADING:

DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft² DEAD LOAD: 20.0 lb/ft²

SUBFLOOR: 3/4" GLUED AND NAILED

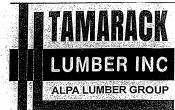
DATE: 2021-06-23

1st FLOOR



		Products		
PlotID	Length	Product	Plies	Net Qty
J1	20-00-00	11 7/8" NI-40x	1	17
J2	12-00-00	11 7/8" NI-40x	1	8
J3	8-00-00	11 7/8" NI-40x	1	2
J4	20-00-00	11 7/8" NI-80	1	16
B7	20-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2
B11	10-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2
B6	10-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2
B9	10-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2
B10	8-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1

Connector Summary						
Qty	Manuf	Product				
6	H1	IUS2.56/11.88				
1	H2	HUS1.81/10				
1	H3	IUS3.56/11.88				
1	H4	HGUS410				
2	H4	HGUS410				



FROM PLAN DATED:

BUILDER: ROYAL PINE HOMES

SITE: CENTREFIELD WEST GORMLEY

MODEL: 2007

ELEVATION: A,B

LOT:

CITY: RICHMAND HILL

SALESMAN: MARIO DICIANO

DESIGNER: AJ REVISION:

NOTES:

REFER TO THE NORDIC INSTALLATION **GUIDE** FOR PROPER STORAGE AND INSTALLATION. SQUASH BLOCKS OF 2x4. 2x6, 2x8 #2 S.P.F. REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. MULTIPLE **SQUASH BLOCKS** REQ'D UNDER CONCENTRATED LOADS, SEE FIGURE 1. CANTILEVERED JOISTS INCLUDING CANT' **OVER BRICK** REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURE 7 TABLES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR HOLES INCLUDING DUCT CHASE AND FIELD **CUT OPENINGS** SEE FIGURE 7 TABLES 1 & 2 OF THE INSTALLATION GUIDE. CERAMIC TILE APPLICATION AS PER O.B.C. 9.30.6

LOADING:

DESIGN LOADS: L/480.000

LIVE LOAD: 140.0 Hb/ft chmond HILL DEAD LOAD: 20.0 lb/ft division

SUBFLOOR: 5/8" GLUED AND NAILED

DATE: 2021-06-07

2nd FLOOR erto

NORDIC

INSTALLATION GUIDE NORDIC JOIST

NS-GI33 **■**◆ ENGLISH

Engineered Wood Products

BASIC INSTALLATION **GUIDE FOR** RESIDENTIAL **FLOORS**

JOIST

NORDIC

nordic.ca

INSTALLING NORDIC I-JOISTS

- Installation of Nordic I-joists shall be as shown in details 1. 2. Except for cutting to length, I-joist flanges should never be cut, drilled or notche
- Install I-joists so that top and bottom flanges are within 1/2 inch of true vertical alignment.
- Concentrated loads should only be applied to the top surface of the top flange. Concentrated loads should not be bottom flange with the exception of light loads, such as ceiling fans or light fixtures.
- I-joists must not be used in applications where they will be permanently exposed to weather, or will reach a moisture content of 15 percent or greater, such as in swimming pool or hot tub areas. They must not be installed where they will remain in direct contact with
- End bearing length must be at least 1-3/4 inch. For multiple-span joists, intermediate bearing length must be at least 3-1/2 inches
- 8. Ends of floor joists shall be restrained to prevent rollover. Use rim board or I-joist blocking panels.
- (cripple blocks) to transfer gravity loads from above the floor system to the wall or foundation below.
- 0. For I-joists installed directly beneath bearing walls parallel to the joists or used as rim board or blocking panels, ti using a single I-joist is 3,300 plf, and 6,600 plf if double I-joists are used.
- . Continuous (ateral support of the I-joist's compression flange is required to prevent rotation and buckling. In simple span uses, latera support of the top flange is normally supplied by the floor sheathing. In multiple-span or cantilever applications, bracing of the I-joist's bottom flange is also required at interior supports of multiple-span joists, and at the end support next to the cantilever extension. The ends of all cantilever extensions must be laterally braced as shown in details 3, 4, or 5,
- 2. Nails installed in flange face or edge shall be spaced in accordance
- with the applicable building code requirements or approved building plans, but should not be closer than those specified on page 3.3 of the Nordic Joist Technical Guide (NS-GT3). NORDIC I-JOIST SERIES
- 3. Details 1 show only I-joist-specific fastener requirements. For other fastener requirements, see the applicable building code.
- 4. For proper temporary bracing of wood I-joists and placement of temporary construction loads, see APA Technical Note: Temporary Construction Loads over I-Joist Roofs and Floors, Form J735.

All nails shown in the details are assumed to be common nails unless otherwise noted. Nails shall have a diameter not less than 0.128 inch for 2-1/2-inch nails, or 0.144 inch for 3-inch nails. Individual components not shown to scale for clarity.

SAFETY AND CONSTRUCTION PRECAUTIONS

- . Brace and nail each I-joist as it is installed, using hangers, blocking panels, rim board, and/ or cross-bridging at joist ends. When I-joists are applied continuous over interior supports and a load-bearing wall is planned at that location, blocking will be required at the interior support.
- When the building is completed, the floor sheathing will provide lateral support for the top flanges of the I-joists. Until this sheathing is applied, temporary bracing, often called struts, or temporary sheathing must be applied to prevent I-joist rollover or buckling.
- Temporary bracing or struts must be 1x4 inch minimum, at least 8 feet long and spaced no more than 8 feet on centre, and must be secured with a minimum of two 2-1/2-inch nails fastened to the top surface of each I-joist. Nail the bracing to a lateral restraint at the end of each bay. Lap ends of adjoining bracing over at least two I-loists.
- . Or, sheathing (temporary or permanent) can be nailed to the top flange of the first 4 feet
- For cantilevered I-joists, brace top and bottom flanges, and brace ends with closure panels, rim board, or cross-bridging.
- . Install and fully nail permanent sheathing to each I-joist before placing loads on the floo. system. Then, stack building materials over beams or walls only.
- . Never install a damaged I-loist.

NI-60

33 pieces per unit

RESIDENTIAL SERIES

2x8 1950f MSR 3/8 in. web

33 pieces per unit

mproper storage or installation, failure to follow applicable building codes, failure to follow pan ratings for Nordic I-joists, failure to follow allowable hole sizes and locations, or failure o use web stiffeners when required can result in serious accidents. Follow these installation

2x4 2100f MSR

23 pieces per un

WEB HOLES AND OPENINGS

WEB HOLES IN I-JOISTS

- Holes measuring 1-1/2 inch or smaller be permitted subject to verification



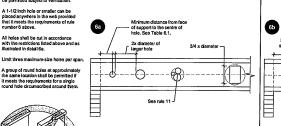
Never stack building materials over Once sheathed, de

RIM BOARDS

Width Length 1-1/8 in. 16 ft

APA Rim Board Plu

Do not walk on I-joists until fully fastened and braced, or serious injuries can result.



DUCT CHASE OPENINGS tules for Cutting Duct Chase Openings in I-jols

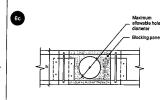
. The maximum depth of a duct chase opening that can be cut into an Hotst web shall equal the clear distance between the flanges of the Holst minus 1/4 inch. A minimum of 1/8 inch should always be maintained between the top or bottom of the opening and the adacont Holst flange.

All openings shall be cut in accordance with the restrictions listed above and as illustrated in detail 6b.

- - The maximum allowable hole size for a lateral-restraint-only blocking panel is 273 of the lesser dimension of the blocking's depth or length. Assuming th blocking panel is longer than its height (or depth), the table aside applies. For other applications, contact Norde Structures.

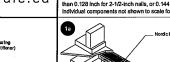
HOLES IN BLOCKING PANELS

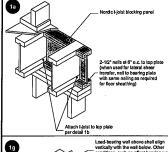
- Field-cut holes must be centred in the blocking horizontally.
- While round holes are preferred, rectangle holes may be used provided the corners are not over cut. Slightly rounding corners or pre-drilling corn with a 1-inch-diameter bit is recommended.
- All holes must be cut in a workman-like manner in accordance with the limitations listed above.

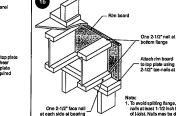


i-joist or rim board blocking depth (in.)	Maximum allowable hole diameter (in.) (*)	
9-1/2	6-1/4	
11-7/8	7-3/4	
14	9-1/4	
16	10-1/2	

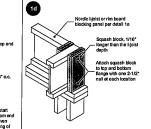


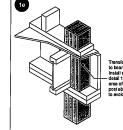






NI-20 2x8 S-P-F No. 2

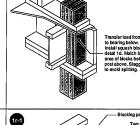


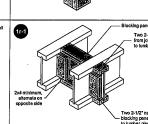


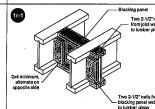
NI-90

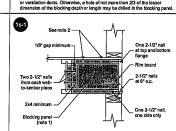
2×4 2400f MSF

23 pieces per uni









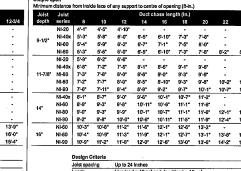
<u>→DC3</u>

	NI-20	0'-7"	1'-6"	2'-10"	4'-3"	5'-8"	6'-0"		•	-	-	•		•		_
1/2*	NI-40x	0'-7"	1'-6"	3'-0"	4'-4"	6'-0"	6'-4"	-			-	-	-		-	
172	NI-60	1'-3"	2'-6"	4'-0"	5'-4"	7'-0"	7'-5"	-		-	-				-	
	NI-80	2'-3"	3'-6"	5'-0"	6'-6"	8'-2"	8'-8"		-	-	-	-	-	-		
	NI-20	0'-7*	0'-8"	1'-0"	2'-4"	3'-8"	4'-0"	5'-0"	6'-6"	7'-9"	-		-		-	_
	NI-40x	0'-7"	0'-8"	1'-3"	2-8*	4'-0"	4'-4"	5'-5"	7'-0"	8'-4"	-	-			-	
-7/8*	NI-60	0'-7"	1'-8"	3'-0"	4'-3"	5'-9"	6'-0"	7'-3"	8'-10"	10'-0"	-	-		-	-	
	NI-B0	1'-6"	2'-10"	4'-2"	5'-6"	7'-0"	7'-5"	8'-6"	10'-3"	11'-4"				-	-	
	NI-90	0'-7"	0'-8"	1'-5"	3'-2"	4"-10"	5-4"	6'-9"	8'-9"	10'-2"	-			-		
	NI-40x	0'-7"	0'-8"	0'-8"	1'-0"	2'-4"	2-9*	3'-9"	5-2"	6'-0"	6'-6"	8'-3"	10'-2"	-		
	NI-60	0'-7"	0'-8"	1'-8"	3'-0"	4'-3"	4'-8"	5'-8"	7-2*	8'-0"	8'-8"	10'-4"	11'-9"	_	-	
	NI-80	0'-10"	2'-0"	3'-4"	4'-9"	6'-2"	6-5	7'-6"	9'-0"	10'-0"	10'-8"	12-4"	13'-9"	-		
	NI-90	0'-7"	0'-8"	0'-10"	2-5*	4'-0"	4'-5"	5'-9"	7'-5"	8'-8"	9'-4"	11'-4"	12'-11"		-	
	NI-60	0'-7*	0'-8"	0'-8"	1'-6"	2'-10"	3'-2"	4'-2"	5'-6"	6'-4"	7'-0"	8'-5"	9'-8"	10"-2"	12'-2"	1
•	NI-80	0'-7"	1'-3"	2'-6"	3'-10"	5'-3"	5-6"	6'-6"	8'-0"	9'-0"	9'-5"	11'-0"	12'-3"	12'-9"	14'-5"	1
	N1-90	0'-7"	0'-8"	0'-8"	1'-9"	3'-3"	3'-8"	4'-9"	6'-5"	7'-5"	8'-0"	9'-10"	11'-3"	11'-9"	13'-9"	1

TABLE 6.1 - LOCATION OF WEB HOLES

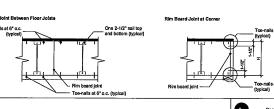
Joist spacing Up to 24 inches

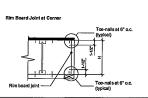
TABLE 6.2 - LOCATION OF DUCT CHASE OPENINGS

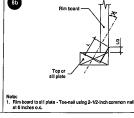


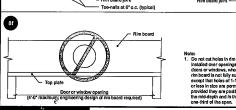
Live load = 40 psf and dead load = 15 psf limits L/480 under live load and L/240 under total loa

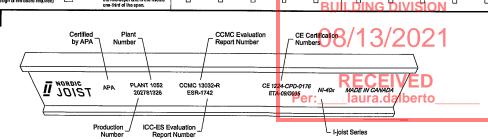
RIM BOARDS 8a

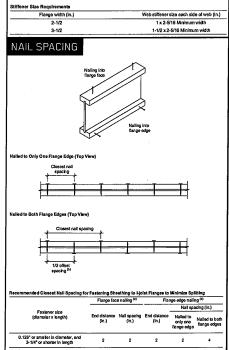


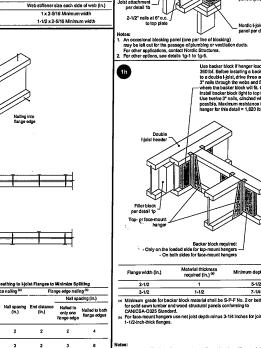


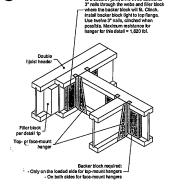




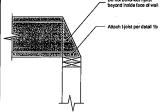


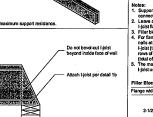




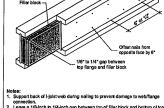


, nanger	*	
	Backer block red paded side for top-mount he oth sides for face-mount he	ingers —
Flange width (in.)	Material thickness required (in.) (a)	Minimum depth (in.) (b)
2-1/2	1	5-1/2
3-1/2	1-1/2	7-1/4
for solid sawn lumber at CAN/CSA-0325 Standa	use net joist depth minus :	onforming to









- Support cauch or spois was using insing to prevent comings or warranged to warranged to the spois of the spois of filler block and bottom of for I-Joist Range.

Filler block its required between joists for full length of span.

For flange width of 2-1/2 inches, nell joists (opether with two rows of 3-inch malls at 12 Inches or. (clinched when possible) on each side of the duble.

Filler block size (in.) Example
2-1/8 to 2-1/4 x 6 2x6 + 5/6* or 3/4* shea
2-1/8 to 2-1/4 x 8 2x6 + 5/6* or 3/4* shea
2-1/8 to 2-1/4 x 10 2x10 + 5/6* or 3/4* shea 2-1/8 to 2-1/4 x 12 2x12 + 5/8" or 3/4" sheall/in

FOR ALL construction details

COMPANY July 22, 2020 08:23 **PROJECT** J5 1ST FLOOR.wwb

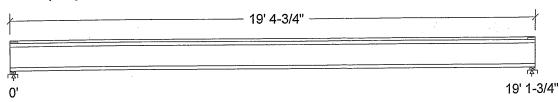
Design Check Calculation Sheet

Nordic Sizer - Canada 7.2

Loads:

Load	Туре	Distribution	Pat-	Location	[ft]	Magnitud	de	Unit
12044	-21		tern	Start	End	Start	End	
Load1	Dead	Full Area				20.00		psf
Load2	Live	Full Area				40.00		psf

Maximum Reactions (lbs) and Support Bearing (in):



191 383		191 383
814		814
2188 5573		2188 5573
0.37 0.15 #2		0.37 0.15 #2
2-3/8 1-3/4 No		2-3/8 1-3/4 No
1.00 1.00 769		1.00 1.00 769 1.09
	383 814 2188 5573 0.37 0.15 #2 2-3/8 1-3/4 No 1.00 1.00	383 814 2188 5573 0.37 0.15 #2 2-3/8 1-3/4 No 1.00 1.00 769

Nordic Joist 11-7/8" NI-80 Floor joist @ 12" o.c.

Supports: All - Lumber Sill plate, No.1/No.2 Total length: 19' 4-3/4"; Clear span: 19'; 3/4" nailed and glued OSB sheathing This section PASSES the design code check.

Limit States Design using CSA 086-14 and Vibration Criterion:

Limit States Desi	CITY OF RICHMOND HILL			
Criterion	Analysis Value	Design Value	Unit	Analysis/DesignN
Shear	Vf = 814	Vr = 2336	lbs	Vf/Vr = 0.35
Moment(+)	Mf = 3894	Mr = 11609	lbs-	ft = 0.34
Perm. Defl'n	$0.11 = \langle L/999 \rangle$	0.64 = L/360	in	0.17
Live Defl'n	0.22 = < L/999	0.48 = L/480	in	0.46
Total Defl'n	0.33 = L/690	0.96 = L/240	in	0.35
Bare Defl'n	0.25 = L/920	0.64 = L/360	in	WINDOWS ON B9
Vibration	$I_{max} = 19'-1.8$	Lv = 21'-2.7	ft	S. KATSOUI AND OF SO
Defl'n	= 0.026	= 0.033	in	0.78
L				

COMPONENT ONLY

WoodWorks® Sizer

for NORDIC STRUCTURES

J5 1ST FLOOR.wwb

Nordic Sizer - Canada 7.2

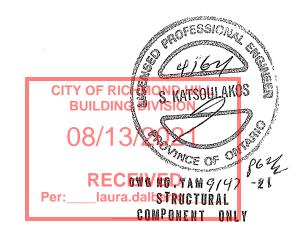
Page 2

```
Additional Data:
                                                                          LC#
                                                                    KN
FACTORS:
                     KD
                            KH
                                             KL
                                                            KS
                                                                          #2
            2336
                    1.00
                            1.00
 Vr
                                                                          #2
                                           1.000
           11609
                    1.00
                            1.00
Mr+
                                                                          #2
           547.1 million
F.T
CRITICAL LOAD COMBINATIONS:
           : LC \#2 = 1.25D + 1.5L
 Shear
 Moment(+): LC #2
                    = 1.25D + 1.5L
                            (permanent)
 Deflection: LC #1
                    = 1.0D
                    = 1.0D + 1.0L
                                   (live)
             LC #2
                    = 1.0D + 1.0L
                                   (total)
             LC #2
                   = 1.0D + 1.0L
                                    (bare joist)
             LC #2
           : Support 1 - LC \# 2 = 1.25D + 1.5L
 Bearing
             Support 2 - LC \# 2 = 1.25D + 1.5L
 Load Types: D=dead W=wind S=snow H=earth, groundwater E=earthquake
             L=live(use, occupancy) Ls=live(storage, equipment)
 Load Patterns: s=S/2 L=L+Ls _=no pattern load in this span
 All Load Combinations (LCs) are listed in the Analysis output
CALCULATIONS:
 Eleff = 625.37 lb-in^2 K = 6.18e06 lbs
 "Live" deflection is due to all non-dead loads (live, wind, snow...) CONFORMS TO OBC 2012
```

Design Notes:

AMENDED 2020

- 1. WoodWorks analysis and design are in accordance with the 2015 National Building Code of Canada (NBC), Division B, Part 4, and the CSA O86-14 Engineering Design in Wood standard, Update No. 2 (June 2017).
- 2. Please verify that the default deflection limits are appropriate for your application.
- 3. Refer to Nordic Structures technical documentation for installation guidelines and construction details.
- 4. Nordic I-joists are listed in CCMC evaluation report 13032-R.
- 5. Joists shall be laterally supported at supports and continuously along the compression edge.
- 6. The design assumptions and specifications have been provided by the client. Any damages resulting from faulty or incorrect information, specifications, and/or designs furnished, and the correctness or accuracy of this information is their responsibility. This analysis does not constitute a record of the structural integrity of the building nor suitability of the design assumptions made. Nordic Structures is responsible only for the structural adequacy of this component based on the design criteria and loadings shown.





COMPANY July 22, 2020 08:27 **PROJECT**J5 2ND FLOOR.wwb

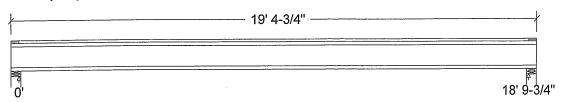
Design Check Calculation Sheet

Nordic Sizer - Canada 7.2

Loads:

Load	Type	Distribution	Pat-	Location	[ft]	Magnitude	Unit
1			tern	Start	End	Start Er	nd
Loadl	Dead	Full Area				20.00	psf
Load2	Live	Full Area				40.00	psf

Maximum Reactions (lbs) and Support Bearing (in):



Unfactored: Dead Live	188 376	188 376
Factored: Total	800	800
Bearing: Capacity Joist Support	2336 10841	2336 10841
Des ratio Joist Support Load case Length	0.34 0.07 #2 4-3/8	0.34 0.07 #2 4-3/8
Min req'd Stiffener KD	1-3/4 No 1.00	1-3/4 No 1.00
KB support	769 -	769

Bearing for wall supports is perpendicular-to-grain bearing on top plate. No stud design included.

Nordic Joist 11-7/8" NI-80 Floor joist @ 12" o.c.

Supports: All - Lumber Wall, No.1/No.2

Total length: 19' 4-3/4"; Clear span: 18' 8"; 5/8" nailed and glued OSB sheathing with 1/2" gypsum ceiling

This section PASSES the design code check.

Limit States Design using CSA 086-14 and Vibration Criterion:

Criterion	Analysis Value	Design Value	Unit
Shear	Vf = 800	Vr = 2336	lbs
Moment (+)	Mf = 3760	Mr = 11609	lbs-f
Perm. Defl'n	0.11 = < L/999	0.63 = L/360	in
Live Defl'n	0.21 = < L/999	0.47 = L/480	in
Total Defl'n	0.32 = L/712	0.94 = L/240	in
Bare Defl'n	0.23 = L/966	0.63 = L/360	in
Vibration	Lmax = 18'-9.8	Lv = 20'-5.8	ft -
Defl'n	= 0.027	= 0.033	in
Derr 11			

BUILDING DIVISION

Analysis/Design

O 34

MI/MI 0 32

P S KATSOULAKOS 137

92

CITY OF RICHMOND HILL

STRUCTURAL

Nordic Sizer - Canada 7.2

Page 2

```
Additional Data:
                                                                            LC#
                                                                      KN
FACTORS:
                      KD
                             KH
                                      KZ
                                               KL
                                                             KS
                                                                            #2
             2336
                     1.00
                            1.00
 Vr
                                                                            #2
                                             1.000
           11609
                     1.00
                            1.00
 Mr+
                                                                            #2
           547.1 million
 ET
CRITICAL LOAD COMBINATIONS:
           : LC #2 = 1.25D + 1.5L
 Moment(+) : LC #2
                    = 1.25D + 1.5L
                             (permanent)
 Deflection: LC #1
                     = 1.0D
                    = 1.0D + 1.0L
                                    (live)
              LC #2
                    = 1.0D + 1.0L
                                     (total)
              LC #2
              LC #2 = 1.0D + 1.0L (bare joist)
           : Support 1 - LC \# 2 = 1.25D + 1.5L
 Bearing
              Support 2 - LC \# 2 = 1.25D + 1.5L
 Load Types: D=dead W=wind S=snow H=earth, groundwater E=earthquake
              L=live(use,occupancy) Ls=live(storage,equipment)
 Load Patterns: s=S/2 L=L+Ls _=no pattern load in this span
 All Load Combinations (LCs) are listed in the Analysis output
CALCULATIONS:
 Eleff = 613.27 \text{ lb-in}^2 \text{ K} = 6.18e06 \text{ lbs}
 "Live" deflection is due to all non-dead loads (live, wind, snow...)
                                                                        CONFORMS TO OBC 2012
```

Design Notes:

AMENDED 2020

- 1. WoodWorks analysis and design are in accordance with the 2015 National Building Code of Canada (NBC), Division B, Part 4, and the CSA O86-14 Engineering Design in Wood standard, Update No. 2 (June 2017).
- 2. Please verify that the default deflection limits are appropriate for your application.
- 3. Refer to Nordic Structures technical documentation for installation guidelines and construction details.
- 4. Nordic I-joists are listed in CCMC evaluation report 13032-R.
- 5. Joists shall be laterally supported at supports and continuously along the compression edge.
- 6. The design assumptions and specifications have been provided by the client. Any damages resulting from faulty or incorrect information, specifications, and/or designs furnished, and the correctness or accuracy of this information is their responsibility. This analysis does not constitute a record of the structural integrity of the building nor suitability of the design assumptions made. Nordic Structures is responsible only for the structural adequacy of this component based on the design criteria and loadings shown.



BC CALC® Member Report



Single 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

PASSED

1ST FLR FRAMING\Flush Beams\B1(i416) (Flush Beam)

Dry | 1 span | No cant.

July 22, 2020 09:22:00

Build 7493

Job name:

Customer:

Address:

City, Province, Postal Code: RICHMAND HILL

File name:

UNIT 2007 EL A, B. mmdl

Description:

Wind

1ST FLR FRAMING\Flush Beams\B1(i416)

Specifier:

Designer: ΑJ

Company:

Code reports:	CCMC 12472-R

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<u> </u>																					 										
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															C	08-03-	2														ı

Total Horizontal Product Length = 08-03-12

Reaction Summary (Down / Uplift) (lbs)

I Cachon Gai	IIIII AI Y (BO O III I I O	P / \ \ /	
Bearing	Live	Dead	Snow
B1, 1-7/8"	37 / 0	43 / 0	
B2. 4-3/8"	39 / 0	45 / 0	

Loa	ad Summary						Live	Dead	Snow	Wind	Tributary
	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	08-03-12	Тор		6			00-00-00
1	FC1 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	08-03-12	Тор	9	5			n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	212 ft-lbs	17696 ft-lbs	1.2%	1	04-00-10
End Shear	78 lbs	7232 lbs	1.1%	1	01-01-12
Total Load Deflection	L/999 (0.004")	n\a	n\a ·	4	04-00-10
Live Load Deflection	L/999 (0.002")	n\a	n\a	5	04-00-10
Max Defl.	0.004"	n\a	n\a	4	04-00-10
Span / Depth	8.0				

Bearin	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Wall/Plate	1-7/8" x 1-3/4"	109 lbs	5.4%	2.7%	Spruce-Pine-Fir
B2	Wall/Plate	4-3/8" x 1-3/4"	115 lbs	2.4%	1.2%	Spruce-Pine-Fir

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

AMENDED 2020

Resistance Factor phi has been applied to all presented results per CSA O86. BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Calculations assume member is fully braced.

CONFORMS TO OBC 2012

Disclosure

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular

OWNICE OF CHANGE

OVE NO. TAM 9/49-28 STRUCTURAL COMPONENT ONLY

application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade

engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788

before installation.

BC CALC®, BC FRAMER® ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,





Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP 1ST FLR FRAMING\Flush Beams\B2(i573) (Flush Beam)

PASSED

BC CALC® Member Report

Build 7773

Dry | 1 span | No cant.

June 23, 2021 10:22:26

Job name:

Address:

City, Province, Postal Code: RICHMAND HILL

File name:

UNIT 2007 EL A, B.mmdl

Description: 1ST FLR FRAMING\Flush Beams\B2(i573)

Wind

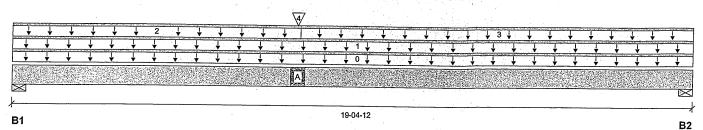
Specifier:

Designer:

Customer: Code reports:

CCMC 12472-R

AJ Company:



Total Horizontal Product Length = 19-04-12

Snow

Reaction Summary (Down / Uplift) (Ibs)

Bearing Live Dead B1, 1-7/8" 1229 / 0 745 / 0 B2, 1-7/8" 892/0 572 / 0

Loa	ad Summary	•					Live	Dead	Snow	Wind	Tributary
	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	moutary
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	19-04-12	Тор		12			00-00-00
1	FC1 Floor Decking (Plan View Fill)	Unf. Lin. (lb/ft)	L	00-00-00	19-04-12	Тор	20	10			n\a
2	FC1 Floor Decking (Plan View Fill)	Unf. Lin. (lb/ft)	L	00-00-00	08-01-02	Тор	22	11			n\a
3	FC1 Floor Decking (Plan View Fill)	Unf. Lin. (lb/ft)	L	08-01-02	19-04-12	Тор	6	3			n\a
4	B3(i574)	Conc. Pt. (lbs)	L	08-00-04	08-00-04	Тор	1488	767			n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	18632 ft-lbs	35392 ft-lbs	52.6%	1	08-00-04
End Shear	2655 lbs	14464 lbs	18.4%	1	01-01-12
Total Load Deflection	L/306 (0.753")	n\a	78.4%	4	09-02-08
Live Load Deflection	L/487 (0.473")	n\a	73.9%	5	09-02-08
Max Defl.	0.753"	n\a	n\a	4	09-02-08
Span / Depth	19.4				

Bearing	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Wall/Plate	1-7/8" x 3-1/2"	2774 lbs	68.7%	34.7%	Spruce-Pine-Fir
B2	Wall/Plate	1-7/8" x 3-1/2"	2052 lbs	50.8%	25.6%	Spruce-Pine-Fir

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA 086.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Calculations assume unbraced length of Top: 00-00-00, Bottom: 11-01-12.

CONFORMS TO OBE 2012 AMENDED 2020ITY OF

RETURNO/FAM13692-21 Jaura STRUCTURAL COMPONENT ONLY





Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP 1ST FLR FRAMING\Flush Beams\B2(i573) (Flush Beam)

PASSED

BC CALC® Member Report

Build 7773

Dry | 1 span | No cant.

June 23, 2021 10:22:26

Job name:

Address:

Customer:

Code reports:

City, Province, Postal Code: RICHMAND HILL

CCMC 12472-R

File name:

UNIT 2007 EL A, B. mmdl

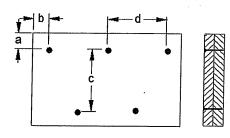
Description: 1ST FLR FRAMING\Flush Beams\B2(i573)

Specifier:

Designer:

AJ Company:

Connection Diagram: Full Length of Member



a minimum = 2"

c = 7-7/8"

b minimum = 3"

d= 100 11

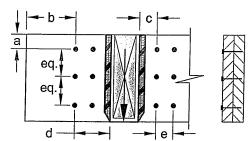
Connectors are:

- **∕** ... Nails

ARDOX SPIRAL

Connection Diagrams: Concentrated Side Loads

Connection Tag: Applies to-load tag(s): 3



a minimum = 2"

b minimum = 4"

c minimum = 4"

d maximum = 12"

e minimum = 4"

Connectors are:

Nails

ARDOX SPIRAL



OWO NO. TAM 1369221 STRUCTURAL COMPONENT ONLY

Disclosure

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on

building code-accepted design properties and analysis methods. Binstallation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

BC CALC®, BC FRAMER® , AJS™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue® , VERSA-LAM®, VERSA-RIM PLUS® ,





Single 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP 1ST FLR FRAMING\Flush Beams\B3(i574) (Flush Beam)

PASSED

BC CALC® Member Report

Dry | 1 span | No cant.

June 23, 2021 10:22:26

Build 7773

Job name:

Address:

City, Province, Postal Code: RICHMAND HILL

Customer:

Code reports:

CCMC 12472-R

File name:

UNIT 2007 EL A, B. mmdl

Description: 1ST FLR FRAMING\Flush Beams\B3(i574)

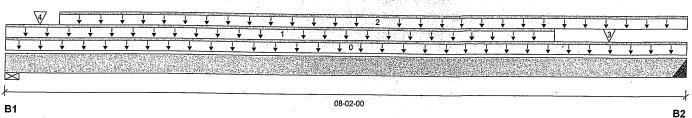
AJ

CONFORMS TO THE PAIR STATE

Specifier:

Designer:

Company:



Total Horizontal Product Length = 08-02-00

Snow

Reaction Summary (Down / Unlift) (lbs)

. togotion our	mma.j (Domin op	111 <i>t)</i> (103)
Bearing	Live	Dead
B1, 5-1/2"	1906 / 0	1029 / 0
R2 2"	1517 / 0	702 / 0

Lo	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	08-02-00	Top		6	· · · · · ·		00-00-00
1	Smoothed Load	Unf. Lin. (lb/ft)	L	00-00-00	06-06-08	Top	216	96			n\a
2	STAIR	Unf. Lin. (lb/ft)	L	00-07-08	08-02-00	Тор	240	120			n\a
3	J2(i550)	Conc. Pt. (lbs)	L	07-02-08	07-02-08	Тор	199	100		1200	n\a
4	1(i140)	Conc. Pt. (lbs)	L	00-04-12	00-04-12	Тор		126	. M	OSE	55/0/ _{/.n/a} .

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	6310 ft-lbs	17696 ft-lbs	35.7%	1	04-05-08
End Shear	2920 lbs	7232 lbs	40.4%	1	01-05-06
Total Load Deflection	L/999 (0.096")	n\a	n\a	4	04-02-08
Live Load Deflection	L/999 (0.064")	n\a	n\a	5	04-02-08
Max Defl.	0.096"	n\a	n\a	4	04-02-08
Span / Depth	7.7				

_Bearing	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Wall/Plate	5-1/2" x 1-3/4"	4145 lbs	70.0%	35.3%	Spruce-Pine-Fir
B2	Hanger	2" x 1 - 3/4"	3253 lbs	n\a	76.2%	HUS1.81/10

Header for the hanger HUS1.81/10 is a Double 1-3/4" x 11-7/8" LVL Beam.

Hanger model HUS1.81/10 and seat length were input by the user. Hanger has not been analyzed for adequate capacity.

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Hanger Manufacturer: Unassigned

AMENDED 2020 Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA 086.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Calculations assume unbraced length of Top: 00-00-00, Bottom: 01-01-08.

NOWNCE OF UWU NO. TAM 13693-21 STRUCTURAL COMPONENT ONLY

Disclosure

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a Olygualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on

building code-accepted design properties and analysis methods. Binstallation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

BC CALC®, BC FRAMER® , AJS™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®. VERSA-LAM®, VERSA-RIM PLUS®





PASSED

July 22, 2020 08:51:14

2ND FLR FRAMING\Flush Beams\B6(i267) (Flush Beam)

BC CALC® Member Report

CCMC 12472-R

Build 7493

Job name: Address:

City, Province, Postal Code:

Customer:

Code reports:

Dry | 1 span | No cant.

UNIT 2007 EL A,B.mmdl

File name: Description: 2ND FLR FRAMING\Flush Beams\B6(i267)

Specifier: Designer:

Company:

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Total Horizontal Product Length = 08-10-14

nmary (Down / Unlift) (lhs)

Reaction Sur	filliary (Down / Of	mit (ida)			
Bearing	Live	Dead	Snow	Wind	
B1, 5-1/2"	1281 / 0	1544 / 0	2490 / 0		
B2, 4-3/8"	1249 / 0	1507 / 0	2427 / 0		

۱۵	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag		Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	08-10-14	Тор		12			00-00-00
1	FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	08-10-14	Top	20	10			n\a
2	E26(i263)	Unf. Lin. (lb/ft)	L	00-00-00	03-07-00	Top		81			n\a
3	E26(i263)	Unf. Lin. (lb/ft)	L	00-00-00	03-03-00	Top	264	240	552		n\a
4	E27(i275)	Unf. Lin. (lb/ft)	L	03-07-00	06-01-00	Top		61			n\a
5	E28(i276)	Unf. Lin. (lb/ft)	L	06-01-00	08-10-14	Тор		81			n\a
6	E28(i276)	Unf. Lin. (lb/ft)	L	06-05-00	08-10-14	Top	264	240	552		n\a
7	E26(i263)	Conc. Pt. (lbs)	L	03-06-00	03-06-00	Top	426	411	891		n\a
8	E28(i276)	Conc. Pt. (lbs)	L	06-02-00	06-02-00	Тор	410	396	857		n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	12287 ft-lbs	35392 ft-lbs	34.7%	13	03-06-00
End Shear	4715 lbs	14464 lbs	32.6%	13	01-05-06
Total Load Deflection	L/999 (0.118")	n\a	n\a	35	04-05-05
Live Load Deflection	L/999 (0.084")	n\a	n\a	51	04-05-05
Max Defl.	0.118"	n\a	n\a	35	04-05-05
Snan / Denth	8.3				

Bearin	ıg Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Wall/Plate	5-1/2" x 3-1/2"	6946 lbs	58.7%	29.6%	Spruce-Pine-Fir
B2	Wall/Plate	4-3/8" x 3-1/2"	6773 lbs	71.9%	36.3%	Spruce-Pine-Fir

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86. Unbalanced snow loads determined from building geometry were used in selected product

verification.

Design based on Dry Service Condition. Importance Factor: Normal Part code: Part 9

canforms to obe

AMENDED 2020

laura.dalberto

PUNCE OF OWO NO. TAM 9/

> STRUCTURAL COMPONENT ONLY





PASSED

2ND FLR FRAMING\Flush Beams\B6(i267) (Flush Beam)

Dry | 1 span | No cant.

July 22, 2020 08:51:14

BC CALC® Member Report

Build 7493 Job name:

Address:

City, Province, Postal Code:

Customer:

Code reports:

CCMC 12472-R

File name:

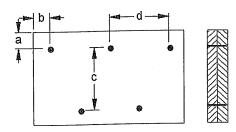
UNIT 2007 EL A, B. mmdi

Description: 2ND FLR FRAMING\Flush Beams\B6(i267)

Specifier: Designer:

Company:

Connection Diagram: Full Length of Member



a minimum = 2" b minimum = 3"

c = 7-7/8" d = 22 B

Connectors are:

Nails ARDOX SPIRAL

CONCE OF CASE

OWG NO. TAM 9/56-2 STRUCTURAL COMPONENT ONLY

Disclosure

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before installation.

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PASSED

2ND FLR FRAMING\Flush Beams\B7(i472) (Flush Beam)

BC CALC® Member Report

Build 7493

Job name: Address:

Dry | 1 span | No cant.

July 27, 2020 08:17:43

File name:

UNIT 2007 EL A,B.mmdl

Description:

2ND FLR FRAMING\Flush Beams\B7(i472)

Specifier: Designer:

Code reports:

Customer:

City, Province, Postal Code: RICHMAND HILL

CCMC 12472-R

Company:

ΑJ

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<u> </u>					_								19-0	00-06																_
B1													_																	В

Total Horizontal Product Length = 19-00-06

Reaction Sun	nmary (Down / O	piiit) (ibs)			
Bearing	Live	Dead	Snow	Wind	
B1. 4-3/8"	911 / 0	581 / 0			
B2 4"	982 / 0	617 / 0			

1	and Summary						Live	Dead	Snow	Wind	Tributary
	oad Summary Jag Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
<u>-</u>	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	19-00-06	Тор		12			00-00-00
1	FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	10-02-06	Top	27	13			n\a
	FC2 Floor Material	Unf, Lin. (lb/ft)	L	10-02-06	19-00-06	Top	23	11			n\a
-		Conc. Pt. (lbs)	ì	10-01-08	10-01-08	Тор	1422	734			n\a
- 3	B10(i465)	COHC. 1 t. (103)	-	10 01 00		. • •					

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
		35392 ft-lbs	47.7%	1	10-01-08
Pos. Moment	16887 ft-lbs				
End Shear	2161 lbs	14464 lbs	14.9%	1	17-08-08
	L/352 (0.629")	n\a	68.1%	4	09-08-09
Total Load Deflection			64.1%	5	09-08-09
Live Load Deflection	L/562 (0.394")	n\a	04.170	5	
Max Defl.	0.629"	n\a	n\a	4	09-08-09
Span / Depth	18.7				

Rearing	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Wall/Plate	4-3/8" x 3-1/2"	2093 lbs	22.2%	11.2%	Spruce-Pine-Fir
B2	Hanger	4" x 3-1/2"	2244 lbs	n\a	13.1%	HGUS410

Cautions

Header for the hanger HGUS410 is a Double 1-3/4" x 11-7/8" LVL Beam.

Hanger model HGUS410 and seat length were input by the user. Hanger has not been analyzed for adequate capacity.

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor : Normal Part code : Part 9

CONFORMS TO OBC

AMENDED

DIVICE OF laura.dalbe

> STRUCTURAL COMPONENT ONLY



PASSED

2ND FLR FRAMING\Flush Beams\B7(i472) (Flush Beam)

Dry | 1 span | No cant.

July 27, 2020 08:17:43

BC CALC® Member Report

Build 7493

Job name:

Address:

City, Province, Postal Code: RICHMAND HILL

Customer: Code reports:

CCMC 12472-R

File name:

UNIT 2007 EL A,B.mmdl

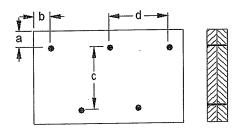
2ND FLR FRAMING\Flush Beams\B7(i472) Description:

Specifier:

ΑJ Designer:

Company:

Connection Diagram: Full Length of Member



a minimum = 2"

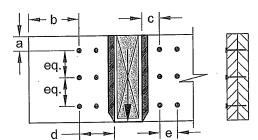
c = 7-7/8" d = 26'8''

b minimum = 3"

Connectors are: ARDOX SPIRAL

Connection Diagrams: Concentrated Side Loads

Applies to load tag(s): 2 Connection Tag. A



a minimum = 2"

b minimum = 4"

c minimum = 4"

d maximum = 12"

e minimum = 4"

ARDOX SPIRAL



Disclosure

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before installation.

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PASSED

2ND FLR FRAMING\Flush Beams\B9(i260) (Flush Beam)

Dry | 1 span | No cant. **BC CALC® Member Report**

July 22, 2020 08:51:14

Build 7493

Job name:

Address:

City, Province, Postal Code:

Customer: Code reports: CCMC 12472-R

File name:

UNIT 2007 EL A,B.mmdl

Description: 2ND FLR FRAMING\Flush Beams\B9(i260)

Specifier:

Designer:

Company:

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В1

Total Horizontal Product Length = 08-10-00

Reaction Summary (Down / Unlift) (lbs)

Neaction Su	IIIIIIaiy (Domini o	אווינין (ווייטין)			
Bearing	Live	Dead	Snow	Wind	
B1, 4"	85 / 0	360 / 0			
B2, 4"	85 / 0	360 / 0			

Los	ad Summary						Live	Dead	Snow	Wind	Tributary
	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	08-10-00	Тор		12			00-00-00
1	WALL	Unf. Lin. (lb/ft)	L	00-00-00	08-10-00	Top		60			n\a
2	FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	08-10-00	Тор	19	10			n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	982 ft-lbs	23005 ft-lbs	4.3%	0	04-05-00
End Shear	354 lbs	9401 lbs	3.8%	0	01-03-14
Total Load Deflection	L/999 (0.011")	n\a	n\a	4	04-05-00
Live Load Deflection	L/999 (0.002")	n\a	n\a	5	04-05-00
Max Defl.	0.011"	n\a	n\a	4	04-05-00
Span / Depth	8.4				

Bearing	a Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material	
B1	Hanger	4" x 3-1/2"	505 lbs	n\a	4.5%	HGUS410	
B2	Hanger	4" x 3-1/2"	505 lbs	n\a	4.5%	HGUS410	

Cautions

Header for the hanger HGUS410 is a Double 1-3/4" x 11-7/8" LVL Beam.

Hanger model HGUS410 and seat length were input by the user. Hanger has not been analyzed for adequate capacity.

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

CONFORMS TO OBC 2012

laura.dalb

18 10. TAN 9159 STRUCTURAL

COMPONENT ONLY

ACE OF





PASSED

2ND FLR FRAMING\Flush Beams\B9(i260) (Flush Beam)

Dry | 1 span | No cant.

July 22, 2020 08:51:14

BC CALC® Member Report

Build 7493 Job name:

Address: City, Province, Postal Code:

Customer:

Code reports:

File name:

UNIT 2007 EL A,B.mmdl

Description: 2ND FLR FRAMING\Flush Beams\B9(i260)

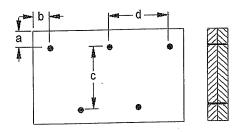
Specifier:

Designer:

Company:

Connection Diagram: Full Length of Member

CCMC 12472-R



a minimum = 2" b minimum = 3"

c = 7-7/8" d = 20 8 "

Connectors are: ...

ARDOX SPIRAL

POWNICE OF ON 18 C

DWG NO. TAM 9159 STRUCTURAL COMPONENT ONLY

Disclosure

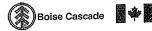
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before installation.

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PASSED

July 27, 2020 08:17:43

2ND FLR FRAMING\Flush Beams\B10(i465) (Flush Beam)

BC CALC® Member Report

Build 7493

Job name:

Address:

City, Province, Postal Code: RICHMAND HILL

Customer: Code reports:

CCMC 12472-R

Dry | 1 span | No cant.

UNIT 2007 EL A, B. mmdl

File name:

Wind

AMENDED

2020

2ND FLR FRAMING\Flush Beams\B10(i465) Description:

Specifier:

Designer: ΑJ

Company:

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251																											,	
											08-00	0-00																
B1																_											•	E

Total Horizontal Product Length = 08-00-00

Reaction Summary (Down / Uplift) (Ibs)

Bearing Live 841 / 0 995 / 0 B1, 5-1/2" 740 / 0 1435 / 0 B2, 2"

1 0	ad Summary						Live	Dead
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65
0	Self-Weight	Unf. Lin. (lb/ft)	L.	00-00-00	08-00-00	Top		6
1	Smoothed Load	Unf. Lin. (lb/ft)	L	01-09-08	07-01-08	Top	201	101
2	STAIR	Unf. Lin. (lb/ft)	L	04-05-01	08-00-00	Тор	240	120
3	B9(i345)	Conc. Pt. (lbs)	L	00-07-04	00-07-04	Тор	114	375
	J2(i477)	Conc. Pt. (lbs)	L	01-01-08	01-01-08	Тор	210	105
4 5	J2(i452)	Conc. Pt. (lbs)	L	07-09-08	07-09-08	Тор	170	85

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	4677 ft-lbs	17696 ft-lbs	26.4%	1	04-11-06
End Shear	2133 lbs	7232 lbs	29.5%	1	06-10-02
Total Load Deflection	L/999 (0.067")	n\a	n\a	4	04-03-03
Live Load Deflection	L/999 (0.044")	n\a	n\a	5	04-03-03
Max Defl.	0.067"	n\a	n\a	4	04-03-03
Span / Depth	7.6				

Rearing	Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1		5-1/2" x 1-3/4"	2543 lbs	43.0%	21.7%	Spruce-Pine-Fir
B2	Hanger	2" x 1-3/4"	3077 lbs	n\a	72.1%	HUS1.81/10

Cautions

Header for the hanger HUS1.81/10 is a Double 1-3/4" x 11-7/8" LVL Beam.

Hanger model HUS1.81/10 and seat length were input by the user. Hanger has not been analyzed for

adequate capacity.

15

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume unbraced length of Top: 00-00-00, Bottom: 00-00-00. CONFORMS TO OBC 2012

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA Q86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

DWG NO. TAM9/60 -21 STRUCTURAL COMPONENT ONLY

POPMOE OF CHILD

Disclosure

Snow

1.00

Dead

Wind

Tributary

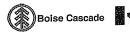
00-00-00 n\a n∖a n\a

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laubefore installation.

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PASSED

July 22, 2020 08:51:14

2ND FLR FRAMING\Flush Beams\B11(i258) (Flush Beam)

BC CALC® Member Report

Build 0

Job name: Address:

City, Province, Postal Code:

Customer:

Code reports:

Dry | 1 span | No cant.

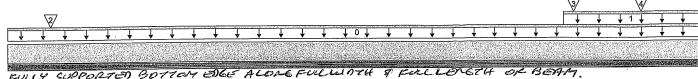
UNIT 2007 EL A, B. mmdl

File name: Description: 2ND FLR FRAMING\Flush Beams\B11(i258)

Specifier:

Designer:

Company:



Total Horizontal Product Length = 09-11-12

Los	ad Summary						Live	Dead	Snow	Wind	Tributary
	. •	Load Type	Ref.	Start	End .	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	09-11-12	Тор		12			00-00-00
1	FC2 Floor Material	Unf. Lin. (lb/ft)	L	08-00-00	09-11-12	Top	6				n\a
2	B9(i260)	Conc. Pt. (lbs)	L	00-07-04	00-07-04	Top	87	362			n\a
3	B7(i144)	Conc. Pt. (lbs)	L	08-01-12	08-01-12	Top	96	98			n\a
4	J5(i200)	Conc. Pt. (lbs)	L	09-01-08	09-01-08	Top	397	198			n\a

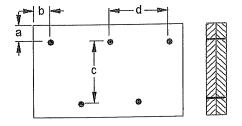
Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location	_
Dist. Load	13.28 lb/ft	57645.00 lb/ft	n\a			
Conc. Load	843 lbs	16813 lbs	5.0%			_

CONFORMS TO OBC 2012

Connection Diagram: Full Length of Member

CCMC 12472-R

AMENDED 2020



a minimum = 2" b minimum = 3"

 $c = 7 - \frac{7}{8}$ d = 100 ci

Calculated Side Load = 421.5 lb/ft Connectors are: 16d And Nails

ARDOX SPIRAL

POMANCE OF ON

DWE NO. TAM916/-28 STRUCTURAL COMPONENT ONLY

Disclosure

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Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask

R questions, please call (800)232-0788 before installation.

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Maximum Floor Spans - S2.1

Design Criteria

Spans:

Simple span

Loads: Deflection limits: Live load = 40 psf and dead load = 15 psf L/480 under live load and L/240 under total load

Sheathing:

5/8 in. nailed-glued oriented strand board (OSB) sheathing

Maximum Floor Spans

			E	Bare			1/2 in. gy	psum ceiling	
Joist depth	Joist series		On cent	re spacing			On cent	re spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	15'-1"	14'-3"	13'-10"	_	15'-7"	14'-9"	14'-3"	-
0.4/01	NI-40x	16'-2"	15'-3"	14'-8"	-	16'-7"	15'-8"	15'-1"	-
9-1/2"	NI-60	16'-4"	15'-4"	14'-10"	-	16'-9"	15'-9"	15'-3"	-
	NI-80	17'-3"	16'-3"	15'-8"	-	17'-8"	16'-7"	16'-0"	-
	NI-20	17'-0"	16'-0"	15'-6"	-	17'-6"	16'-7"	16'-0"	_
	NI-40x	18'-2"	17'-1"	16'-6"	-	18'-9"	17'-6"	16'-11"	-
11-7/8"	NI-60	18'-5"	17'-3"	16'-8"	-	19'-0"	17'-8"	17'-1"	-
	NI-80	19'-9"	18'-3"	17'-7"	-	20'-4"	18'-10"	18'-0"	-
	NI-90	20'-2"	18'-8"	17'-10"	-	20'-9"	19'-2"	18'-4"	-
	NI-40x	20'-1"	18'-8"	17'-10"	-	20'-10"	19'-4"	18'-6"	-
4 411	NI-60	20'-6"	18'-11"	18'-2"	-	21'-2"	19'-8"	18'-9"	-
14"	NI-80	21'-11"	20'-3"	19'-4"	-	22'-7"	20'-11"	20'-0"	-
	NI-90	22'-5"	20'-8"	19'-9"	-	23'-0"	21'-4"	20'-4"	-
	NI-60	22'-4"	20'-8"	19'-9"	-	23'-1"	21'-5"	20'-6"	-
16"	NI-80	23'-11"	22'-1"	21'-1"	-	24'-8"	22'-10"	21'-9"	-
	NI-90	24'-5"	22'-6"	21'-6"	-	25'-1"	23'-2"	22'-2"	-

		Mi	d-span blocking	g with 1x4 inch s	trap	Mid-s	oan blocking ar	nd 1/2 in. gypsum	ceiling
Joist depth	Joist series		On cent	re spacing			On cent	re spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	16'-8"	15'-3"	14'-5"	-	16'-8"	15'-3"	14'-5"	-
0.4/01	NI-40x	17'-11"	17'-0"	16'-1"	-	18'-5"	17'-1"	16'-1"	-
, 9-1/2"	NI-60	18'-2"	17'-1"	16'-4"	-	18'-8"	17'-4"	16'-4"	-
	NI-80	19'-5"	18'-0"	17'-5"	-	19'-10"	18'-5"	17'-8"	-
	NI-20	19'-7"	18'-2"	17'-3"	-	19'-11"	18'-3"	17'-3"	-
	NI-40x	21'-1"	19'-7"	18'-8"	-	21'-8"	20'-2"	19'-2"	-
11-7/8"	NI-60	21'-4"	19'-9"	18'-11"	-	21'-11"	20'-5"	19'-6"	-
	NI-80	22'-9"	21'-1"	20'-2"	-	23'-3"	21'-8"	20'-8"	-
	N!-90	23'-3"	21'-6"	20'-6"	-	23'-9"	22'-0"	21'-0"	-
-	NI-40x	23'-8"	21'-11"	20'-11"	-	24'-4"	22'-8"	21'-8"	-
4.411	NI-60	24'-0"	22'-3"	21'-3"	-	24'-8"	22'-11"	21'-11"	-
14"	NI-80	25'-7"	23'-9"	22'-7"	-	26'-2"	24'-4"	23'-3"	-
	NI-90	26'-1"	24'-2"	23'-0"	-	26'-8"	24'-9"	23'-7"	-
	NI-60	26'-5"	24'-6"	23'-5"	-	27'-2"	25'-3"	24'-2"	-
16"	NI-80	28'-2"	26'-1"	24'-10"	-	28'-10"	26'-9"	25'-6"	-
	NI-90	28'-8"	26'-6"	25'-3"	_	29'-3"	27'-2"	25'-11"	-

Notes

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- 1. The tabulated clear spans are based on CSA O86-14 and NBC 2015, and are applicable to residential floor construction interting the above design orderia.
- 2. For multiple-span applications, the end spans shall be 40% or more of the adjacent span.
- 3. Minimum bearing length shall be 1-3/4 inch for end bearings, and 3-1/2 inches for intermediate bearings.
- 4. Bearing stiffeners are not required when I-joists are used in accordance with this table, except as required for hangers.
- 5. Nordic I-joists are listed in CCMC Evaluation Report 13032-R and APA Product Report PR-L274C.

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Maximum Floor Spans - S4.1

Design Criteria

Spans:

Simple span

Loads:

Live load = 40 psf and dead load = 15 psf

Deflection limits:

L/480 under live load and L/240 under total load

Sheathing:

3/4 in. nailed-glued oriented strand board (OSB) sheathing

Maximum Floor Spans

			E	lare			1/2 in. gy	psum ceiling	
Joist depth	Joist series		On cent	re spacing			On cent	re spacing	
		12"	16"	19.2"	24"	12"	16"	19.2" 19.2" 14'-6" 15'-10" 16'-0" 16'-10" 17'-9" 17'-11" 19'-0" 19'-5" 19'-7" 19'-11" 21'-2" 21'-6" 21'-8"	24"
	NI-20	15'-11"	15'-0"	14'-6"	13'-5"	16'-5"	15'-5"	14'-6"	13'-5"
0.4/0"	NI-40x	17'-0"	16'-0"	15'-5"	14'-10"	17'-5"	16'-5"	15'-10"	15'-2"
9-1/2"	NI-60	17'-2"	16'-2"	15'-7"	14'-11"	17'-7"	16'-7"	16'-0"	15'-4"
	NI-80	18'-3"	17'-1"	16'-5"	15'-9"	18'-8"	17'-5"	16'-9"	16'-1"
	NI-20	17'-11"	16'-11"	16'-3"	15'-8"	18'-7"	17'-5"	16'-10"	16'-2"
	NI-40x	19'-4"	17'-11"	17'-3"	16'-7"	19'-11"	18'-6"	17'-9"	17'-0"
11-7/8"	NI-60	19'-7"	18'-2"	17'-6"	16'-9"	20'-2"	18'-9"	17'-11"	17'-2"
	NI-80	21'-1"	19'-6"	18'-6"	17'-7"	21'-7"	20'-0"	19'-0"	18'-0"
	NI-90	21'-6"	19'-10"	18'-11"	17'-11"	22'-0"	20'-4"	19'-5"	18'-4"
	NI-40x	21'-5"	19'-11"	18'-11"	18'-0"	22'-1"	20'-7"	19'-7"	18'-7"
4.411	NI-60	21'-10"	20'-2"	19'-3"	18'-3"	22'-6"	20'-10"	19'-11"	18'-10"
14"	NI-80	23'-5"	21'-7"	20'-7"	19'-5"	24'-0"	22'-3"	21'-2"	20'-0"
	NI-90	23'-10"	22'-1"	21'-0"	19'-10"	24'-5"	22'-7"	21'-6"	20'-4"
	NI-60	23'-9"	22'-0"	21'-0"	19'-10"	24'-6"	22'-9"	21'-8"	20'-7"
16"	NI-80	25'-6"	23'-7"	22'-5"	21'-2"	26'-2"	24'-3"	23'-1"	21'-10"
	NI-90	26'-0"	24'-0"	22'-10"	21'-6"	26'-7"	24'-8"	23'-5"	22'-2"

		Mi	d-span blocking	g with 1x4 inch	strap	Mid-s	oan blocking an	d 1/2 in. gypsu	m ceiling	
Joist depth	Joist series		On cent	re spacing		On centre spacing				
		12"	16"	19.2"	24"	12"	16"	d 1/2 in. gypsur re spacing 19.2" 14'-6" 16'-3" 16'-6" 18'-2" 17'-5" 19'-4" 19'-8" 21'-6" 22'-4" 24'-1" 24'-6" 24'-9" 26'-5"	24"	
	NI-20	16'-10"	15'-5"	14'-6"	13'-5"	16'-10"	15'-5"	14'-6"	13'-5"	
0.4/01	NI-40x	18'-8"	17'-2"	16'-3"	15'-2"	18'-10"	17'-2"	16'-3"	15'-2"	
9-1/2"	NI-60	18'-11"	17'-6"	16'-6"	15'-5"	19'-2"	17'-6"	16'-6"	15'-5"	
	NI-80	20'-3"	18'-10"	17'-11"	16'-10"	20'-8"	19'-3"	18'-2"	16'-10	
	NI-20	20'-1"	18'-5"	17'-5"	16'-2"	20'-1"	18'-5"	17'-5"	16'-2"	
	NI-40x	21'-10"	20'-4"	19'-4"	17'-8"	22'-5"	20'-6"	19'-4"	17'-8"	
11-7/8"	NI-60	22'-1"	20'-7"	19'-8"	18'-4"	22'-8"	20'-10"	19'-8"	18'-4"	
	NI-80	23'-8"	22'-0"	20'-11"	19'-10"	24'-1"	22'-6"	21'-6"	20'-0"	
	NI-90	24'-1"	22'-5"	21'-4"	20'-2"	24'-7"	22'-11"	21'-10"	20'-7"	
	NI-40x	24'-5"	22'-9"	21'-9"	19'-5"	25'-1"	23'-2"	21'-9"	19'-5"	
4.411	NI-60	24'-10"	23'-2"	22'-1"	20'-10"	25'-6"	23'-8"	22'-4"	20'-10	
14"	NI-80	26'-6"	24'-8"	23'-6"	22'-2"	27'-1"	25'-3"	24'-1"	22'-9"	
	NI-90	27'-0"	25'-1"	23'-11"	22'-7"	27'-6"	25'-8"	24'-6"	23'-2"	
	NI-60	27'-3"	25'-5"	24'-3"	22'-11"	28'-0"	26'-2"	24'-9"	23'-1"	
16"	NI-80	29'-1"	27'-1"	25'-9"	24'-4"	29'-8"	27'-9"	26'-5"	25'-0"	
	NI-90	29'-7"	27'-6"	26'-2"	24'-9"	30'-2"	28'-2"	26'-10"	25'-5"	

Notes:

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- 1. The tabulated clear spans are based on CSA O86-14 and NBC 2015, and are applicable to residential floor construction intecting ltie above design driteria.
- 2. For multiple-span applications, the end spans shall be 40% or more of the adjacent span.
- 3. Minimum bearing length shall be 1-3/4 inch for end bearings, and 3-1/2 inches for intermediate bearings.
- 4. Bearing stiffeners are not required when I-joists are used in accordance with this table, except as required for hangers.
- 5. Nordic I-joists are listed in CCMC Evaluation Report 13032-R and APA Product Report PR-L274C.

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Maximum Floor Spans - S6.1

Design Criteria

Spans:

Simple span

Loads:

Live load = 40 psf and dead load = 15 psf

Deflection limits:

L/480 under live load and L/240 under total load

Sheathing:

5/8 in. nailed-glued Canadian softwood plywood

Maximum Floor Spans

			E	Bare			1/2 in. gy	psum ceiling	
Joist depth	Joist series		On cent	re spacing			On cent	re spacing	
		12"	16"	19.2"	24"	12"	16"	psum ceiling re spacing 19.2" 14'-1" 14'-11" 15'-10" 15'-10" 16'-9" 16'-11" 17'-9" 18'-1" 18'-3" 18'-6" 19'-9" 20'-1" 20'-2" 21'-6" 21'-10"	24"
	NI-20	14'-11"	14'-1"	13'-7"	-	15'-4"	14'-6"	14'-1"	-
0.4/01	NI-40x	15'-11"	15'-0"	14'-6"	-	16'-4"	15'-5"	14'-11"	-
9-1/2"	NI-60	16'-1"	15'-2"	14'-8"	-	16'-6"	15'-7"	15'-1"	-
	NI-80	17'-1"	16'-1"	15'-6"	-	17'-5"	16'-5"	15'-10"	-
	NI-20	16'-9"	15'-10"	15'-4"	-	17'-4"	16'-4"	15'-10"	-
	NI-40x	17'-10"	16'-10"	16'-3"	-	18'-6"	17' -4 "	16'-9"	-
11-7/8"	NI-60	18'-1"	17'-0"	16'-5"	-	18'-9"	17'-6"	16'-11"	-
	NI-80	19'-6"	18'-0"	17'-4"	-	20'-1"	18'-7"	17'-9"	-
	NI-90	19'-11"	18'-4"	17'-8"	-	20'-5"	18'-11"	18'-1"	-
	NI-40x	19'-10"	18'-4"	17'-8"	-	20'-6"	19'-1"	18'-3"	-
14"	NI-60	20'-2"	18'-8"	17'-11"	-	20'-10"	19'-4"	18'-6"	-
14	NI-80	21'-8"	20'-0"	19'-1"	-	22'-4"	20'-8"	19'-9"	-
	NI-90	22'-1"	20'-5"	19'-6"	-	22'-9"	21'-0"	20'-1"	-
	NI-60	22'-0"	20'-4"	19'-6"	-	22'-9"	21'-1"	20'-2"	-
16"	NI-80	23'-7"	21'-10"	20'-10"	-	24'-4"	22'-6"	21'-6"	-
	NI-90	24'-1"	22'-2"	21'-2"	_	24'-9"	22'-11"	21'-10"	_

		Mi	d-span blocking	with 1x4 inch s	trap	Mid-s	pan blocking an	d 1/2 in. gypsum	ceiling
Joist depth	Joist series		On cent	re spacing			On cent	re spacing	
		12"	16"	19.2"	24"	12"	16"	19.2" 14'-3" 15'-11" 16'-2" 17'-7" 17'-1" 19'-0" 19'-3" 20'-5" 20'-10" 21'-5" 21'-8" 23'-0" 23'-4" 23'-11" 25'-3"	24"
	NI-20	16'-6"	15'-1"	14'-3"	-	16'-6"	15'-1"	14'-3"	-
0.4/01	NI-40x	17'-9"	16'-10"	15'-11"	-	18'-2"	16'-11"	15'-11"	-
9-1/2"	NI-60	17'-11"	16'-11"	16'-2"	-	18'-5"	17'-2"	16'-2"	-
	NI-80	19'-3"	17'-10"	17'-3"	-	19'-8"	18'-3"	17'-7"	-
	NI-20	19'-4"	18'-0"	17'-1"	-	19'-9"	18'-1"	17'-1"	-
	NI-40x	20'-10"	19'-4"	18'-6"	-	21'-5"	19'-11"	19'-0"	_
11-7/8"	NI-60	21'-1"	19'-7"	18'-8"	-	21'-8"	20'-2"	19'-3"	-
	NI-80	22'-6"	20'-10"	19'-11"	-	23'-1"	21'-5"	20'-5"	-
	NI-90	23'-0"	21'-3"	20'-4"	-	23'-6"	21'-10"	20'-10"	-
	NI-40x	23'-5"	21'-8"	20'-9"	-	24'-0"	22'-5"	21'-5"	-
4.411	NI-60	23'-9"	22'-0"	21'-0"	-	24'-5"	22'-8"	21'-8"	-
14"	NI-80	25'-4"	23'-6"	22'-5"	-	25'-11"	24'-1"	23'-0"	-
	NI-90	25'-10"	23'-11"	22'-9"	-	26'-5"	24'-6"	23'-4"	-
	NI-60	26'-2"	24'-3"	23'-2"	=	26'-11"	25'-0"	23'-11"	-
16"	NI-80	27'-11"	25'-10"	24'-7"	-	28'-7"	26'-6"	25'-3"	-
	NI-90	28'-5"	26'-3"	25'-0"	-	29'-0"	26'-11"	25'-8"	

Notes:

- 1. The tabulated clear spans are based on CSA O86-14 and NBC 2015, and are applicable to residential floor construction meeting the above design criteria.
- 2. For multiple-span applications, the end spans shall be 40% or more of the adjacent span.
- 3. Minimum bearing length shall be 1-3/4 inch for end bearings, and 3-1/2 inches for intermediate bearings.
- 4. Bearing stiffeners are not required when I-joists are used in accordance with this table, except as required for hangers.
- 5. Nordic I-joists are listed in CCMC Evaluation Report 13032-R and APA Product Report PR-L274C.

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Maximum Floor Spans - S7.1

Design Criteria

Spans:

Simple span

Loads:

Live load = 40 psf and dead load = 15 psf

Deflection limits:

L/480 under live load and L/240 under total load

Sheathing:

3/4 in. nailed-glued Canadian softwood plywood

Maximum Floor Spans

			E	Bare			1/2 in. gy	psum ceiling	
Joist depth	Joist series		On cent	re spacing			On cent	rpsum ceiling tre spacing 19.2" 14'-6" 15'-9" 15'-11" 16'-8" 16'-9" 17'-8" 17'-10" 18'-11" 19'-3" 19'-6" 19'-9" 21'-0" 21'-5" 22'-11"	
		12"	16"	19.2"	24"	12"	16"		24"
	NI-20	15'-10"	15'-0"	14'-5"	13'-5"	16'-4"	15'-5"	14'-6"	13'-5"
9-1/2"	NI-40x	16'-11"	15'-11"	15'-4"	14'-9"	17'-4"	16'-4"	15'-9"	15'-1"
9-1/2	NI-60	17'-1"	16'-1"	15'-6"	14'-10"	17'-6"	16'-6"	15'-11"	15'-3"
	NI-80	18'-1"	17'-0"	16'-4"	15'-8"	18'-7"	17'-4"	16'-8"	16'-0"
	NI-20	17'-10"	16'-10"	16'-2"	15'-7"	18'-5"	17'-4"	16'-9"	16'-1"
	NI-40x	19'-3"	17'-10"	17'-2"	16'-6"	19'-10"	18'-5"	17'-8"	16'-11'
11-7/8"	NI-60	19'-6"	18'-1"	17'-4"	16'-8"	20'-1"	18'-8"	17'-10"	17'-1"
	NI-80	20'-11"	19'-4"	18'-5"	17'-7"	21'-5"	19'-10"	18'-11"	17'-11'
	NI-90	21'-4"	19'-9"	18'-9"	17'-10"	21'-10"	20'-3"	19.2" 14'-6" 15'-9" 15'-11" 16'-8" 16'-9" 17'-10" 18'-11" 19'-3" 19'-6" 19'-9" 21'-0" 22'-11"	18'-3"
	NI-40x	21'-4"	19'-9"	18'-10"	17'-11"	22'-0"	20'-5"	19'-6"	18'-6"
14"	NI-60	21'-8"	20'-1"	19'-2"	18'-2"	22'-4"	20'-9"	19'-9"	18'-9"
14	NI-80	23'-3"	21'-6"	20'-5"	19'-4"	23'-10"	22'-1"	21'-0"	19'-11"
	NI-90	23'-9"	21'-11"	20'-10"	19'-8"	24'-3"	22'-6"	21'-5"	20'-3"
	NI-60	23'-7"	21'-10"	20'-10"	19'-9"	24'-4"	22'-7"	21'-7"	20'-5"
16"	NI-80	25'-4"	23'-5"	22'-3"	21'-1"	26'-0"	24'-1"	22'-11"	21'-8"
	NI-90	25'-10"	23'-10"	22'-8"	21'-5"	26'-5"	24'-6"	23'-4"	22'-0"

		Mi	d-span blockin	g with 1x4 inch	strap	Mid-s	oan blocking ar	nd 1/2 in. gypsu	ım ceiling	
Joist depth	Joist series		On cent	tre spacing		On centre spacing				
		12"	16"	19.2"	24"	12"	16"	19.2" 14'-6" 16'-3" 16'-6" 18'-2" 17'-5" 19'-4" 19'-8" 21'-4" 21'-9" 22'-4" 24'-0" 24'-5" 26'-4"	24"	
	NI-20	16'-10"	15'-5"	14'-6"	13'-5"	16'-10"	15'-5"	14'-6"	13'-5"	
9-1/2"	NI-40x	18'-7"	17'-2"	16'-3"	15'-2"	18'-10"	17'-2"	16'-3"	15'-2"	
9-1/2	NI-60	18'-10"	17'-6"	16'-6"	15'-5"	19'-1"	17'-6"	16'-6"	15'-5"	
	NI-80	20'-2"	18'-9"	17'-11"	16'-10"	20'-7"	19'-2"	18'-2"	16'-10	
	NI-20	20'-1"	18'-5"	17'-5"	16'-2"	20'-1"	18'-5"	17'-5"	16'-2"	
	NI-40x	21'-9"	20'-3"	19'-4"	17'-8"	22'-4"	20'-5"	19'-4"	17'-8"	
11-7/8"	NI-60	22'-0"	20'-6"	19'-7"	18'-4"	22'-7"	20'-10"	19'-8"	18'-4"	
	NI-80	23'-6"	21'-10"	20'-10"	19'-9"	24'-0"	22'-5"	21'-4"	20'-0"	
	NI-90	24'-0"	22'-4"	21'-3"	20'-1"	24'-6"	22'-10"	21'-9"	20'-7"	
	NI-40x	24'-4"	22'-8"	21'-8"	19'-5"	25'-0"	23'-2"	21'-9"	19'-5"	
14"	NI-60	24'-9"	23'-0"	22'-0"	20'-9"	25'-5"	23'-8"	22'-4"	20'-10'	
14"	NI-80	26'-5"	24'-6"	23'-4"	22'-1"	27'-0"	25'-2"	24'-0"	22'-8"	
	NI-90	26'-11"	25'-0"	23'-10"	22'-6"	27'-5"	25'-7"	24'-5"	23'-1"	
	NI-60	27'-2"	25'-4"	24'-2"	22'-10"	27'-11"	26'-1"	24'-9"	23'-1"	
16"	NI-80	29'-0"	26'-11"	25'-8"	24'-3"	29'-7"	27'-7"	26'-4"	24'-11'	
	NI-90	29'-6"	27'-5"	26'-1"	24'-8"	30'-1"	28'-1"	26'-9"	25'-4"	

Notes:

- 1. The tabulated clear spans are based on CSA O86-14 and NBC 2015, and are applicable to residential floor construction meeting the above design criteria.
- 2. For multiple-span applications, the end spans shall be 40% or more of the adjacent span.
- 3. Minimum bearing length shall be 1-3/4 inch for end bearings, and 3-1/2 inches for intermediate bearings.
- 4. Bearing stiffeners are not required when I-joists are used in accordance with this table, except as required for hangers
- 5. Nordic I-joists are listed in CCMC Evaluation Report 13032-R and APA Product Report PR-L274C.

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Maximum Floor Spans - M2.1

Design Criteria

Spans:

Simple span

Loads: Deflection limits:

Live load = 40 psf and dead load = 20 psf

L/480 under live load and L/240 under total load

Sheathing:

5/8 in. nailed-glued oriented strand board (OSB) sheathing

Maximum	F	loor	Sp	ans

			E	Bare			1/2 in. gyį	osum ceiling	
Joist depth	Joist series		On cent	re spacing			On cent	psum ceiling tre spacing 19.2" 14'-3" 15'-1" 15'-3" 16'-0" 16'-11" 17'-1" 18'-0" 18'-4" 18'-6" 18'-9" 20'-0" 20'-4" 20'-6" 21'-9" 22'-2"	
·		12"	16"	19.2"	24"	12"	16"		24"
	NI-20	15'-1"	14'-3"	13'-10"	-	15'-7"	14'-9"	14'-3"	-
	NI-40x	16'-2"	15'-3"	14'-8"	-	16'-7"	15'-8"	15'-1"	-
9-1/2"	NI-60	16'-4"	15'-4"	14'-10"	-	16'-9"	15'-9"	15'-3"	-
	NI-80	17'-3"	16'-3"	15'-8"	-	17'-8"	16'-7"	16'-0"	-
	NI-20	17'-0"	16'-0"	15'-6"	-	17'-6"	16'-7"	16'-0"	-
	NI-40x	18'-2"	17'-1"	16'-6"	-	18'-9"	17'-6"	16'-11"	-
11-7/8"	NI-60	18'-5"	17'-3"	16'-8"	-	19'-0"	17'-8"	17'-1"	-
	NI-80	19'-9"	18'-3"	17'-7"	_	20'-4"	18'-10"	18'-0"	-
	NI-90	20'-2"	18'-8"	17'-10"	-	20'-9"	19'-2"	18'-4"	-
	NI-40x	20'-1"	18'-8"	17'-10"	_	20'-10"	19'-4"	18'-6"	-
	NI-60	20'-6"	18'-11"	18'-2"	-	21'-2"	19'-8"	18'-9"	-
14"	NI-80	21'-11"	20'-3"	19'-4"	-	22'-7"	20'-11"	20'-0"	-
	NI-90	22'-5"	20'-8"	19'-9"	-	23'-0"	21'-4"	20'-4"	-
	NI-60	22'-4"	20'-8"	19'-9"	-	23'-1"	21'-5"	20'-6"	-
16"	NI-80	23'-11"	22'-1"	21'-1"	-	24'-8"	22'-10"	21'-9"	-
	NI-90	24'-5"	22'-6"	21'-6"	_	25'-1"	23'-2"	22'-2"	-

		Mi	d-span blocking	with 1x4 inch s	trap	Mid-sp	an blocking an	d 1/2 in. gypsum	ceiling
Joist depth	Joist series		On cent	re spacing			On cent	nd 1/2 in. gypsum htre spacing 19.2" 14'-5" 16'-1" 16'-4" 17'-8" 17'-3" 19'-0" 19'-6" 20'-8" 21'-0" 20'-11" 21'-11" 23'-3" 23'-7" 24'-2" 25'-6"	
•		12"	16"	19.2"	24"	12"	16"		24"
	NI-20	16'-8"	15'-3"	14'-5"	-	16'-8"	15'-3"	14'-5"	-
	NI-40x	17'-11"	17'-0"	16'-1"	-	18'-5"	17'-1"	16'-1"	-
9-1/2"	NI-60	18'-2"	17'-1"	16'-4"	-	18'-8"	17'-4"	16'-4"	-
	NI-80	19'-5"	18'-0"	17'-5"	-	19'-10"	18'-5"	17'-8"	
	NI-20	19'-7"	18'-2"	17'-3"	-	19'-11"	18'-3"	17'-3"	-
	NI-40x	21'-1"	19'-7"	18'-8"	-	21'-8"	20'-2"	19'-0"	-
11-7/8"	NI-60	21'-4"	19'-9"	18'-11"	-	21'-11"	20'-5"	19'-6"	-
	NI-80	22'-9"	21'-1"	20'-2"	-	23'-3"	21'-8"	20'-8"	-
	NI-90	23'-3"	21'-6"	20'-6"	-	23'-9"	22'-0"	21'-0"	-
	NI-40x	23'-8"	21'-11"	20'-11"	-	24'-4"	22'-8"	20'-11"	-
	NI-60	24'-0"	22'-3"	21'-3"	-	24'-8"	22'-11"	21'-11"	-
14"	NI-80	25'-7"	23'-9"	22'-7"	-	26'-2"	24'-4"	23'-3"	-
	NI-90	26'-1"	24'-2"	23'-0"	-	26'-8"	24'-9"	23'-7"	
	NI-60	26'-5"	24'-6"	23'-5"	-	27'-2"	25'-3"	24'-2"	-
16"	NI-80	28'-2"	26'-1"	24'-10"	-	28'-10"	26'-9"	25'-6"	-
	NI-90	28'-8"	26'-6"	25'-3"	-	29'-3"	27'-2"	25'-11"	-

Notes:

- 1. The tabulated clear spans are based on CSA O86-14 and NBC 2015, and are applicable to residential floor construction meeting the above design of teria.
- 2. For multiple-span applications, the end spans shall be 40% or more of the adjacent span.
- 3. Minimum bearing length shall be 1-3/4 inch for end bearings, and 3-1/2 inches for intermediate bearings.
- 4. Bearing stiffeners are not required when I-joists are used in accordance with this table, except as required for hangers.
- 5. Nordic I-joists are listed in CCMC Evaluation Report 13032-R and APA Product Report PR-L274C.

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Maximum Floor Spans - M4.1

Design Criteria

Simple span

Live load = 40 psf and dead load = 20 psf Loads:

Deflection limits: L/480 under live load and L/240 under total load

3/4 in. nailed-glued oriented strand board (OSB) sheathing Sheathing:

<u> </u>			В	are			1/2 in. gy _l	osum ceiling	
Joist depth	Joist series		On cent	re spacing			On cent	re spacing	
•		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	15'-11"	15'-0"	14'-6"	13'-5"	16'-5"	15'-5"	14'-6"	13'-5"
	NI-40x	17'-0"	16'-0"	15'-5"	14'-10"	17'-5"	16'-5"	15'-10"	14'-11
9-1/2"	NI-60	17'-2"	16'-2"	15'-7"	14'-11"	17'-7"	16'-7"	16'-0"	15'-4"
	NI-80	18'-3"	17'-1"	16'-5"	15'-9"	18'-8"	17'-5"	16'-9"	16'-1"
	NI-20	17'-11"	16'-11"	16'-3"	15'-8"	18'-7"	17'-5"	16'-10"	16'-1"
	NI-40x	19'-4"	17'-11"	17'-3"	16'-7"	19'-11"	18'-6"	17'-9"	17'-0"
11-7/8"	NI-60	19'-7"	18'-2"	17'-6"	16'-9"	20'-2"	18'-9"	17'-11"	17'-2"
	NI-80	21'-1"	19'-6"	18'-6"	17'-7"	21'-7"	20'-0"	19'-0"	18'-0"
	NI-90	21'-6"	19'-10"	18'-11"	17'-11"	22'-0"	20'-4"	19'-5"	18'-4"
	NI-40x	21'-5"	19'-11"	18'-11"	18'-0"	22'-1"	20'-7"	19'-7"	18'-7"
	NI-60	21'-10"	20'-2"	19'-3"	18'-3"	22'-6"	20'-10"	19'-11"	18'-10
14"	NI-80	23'-5"	21'-7"	20'-7"	19'-5"	24'-0"	22'-3"	21'-2"	20'-0"
	NI-90	23'-10"	22'-1"	21'-0"	19'-10"	24'-5"	22'-7"	21'-6"	20'-4"
	NI-60	23'-9"	22'-0"	21'-0"	19'-10"	24'-6"	22'-9"	21'-8"	20'-7"
16"	NI-80	25'-6"	23'-7"	22'-5"	21'-2"	26'-2"	24'-3"	23'-1"	21'-10
	NI-90	26'-0"	24'-0"	22'-10"	21'-6"	26'-7"	24'-8"	23'-5"	22'-2"

		Mi	d-span blocking	with 1x4 inch	strap	Mid-sp	oan blocking an	d 1/2 in. gypsu	m ceiling
Joist depth	Joist series		On cent	re spacing			On cent	re spacing	
		12"	16"	19.2"	24"	12"	16"	d 1/2 in. gypsur re spacing 19.2" 14'-6" 16'-3" 16'-6" 18'-2" 17'-5" 19'-0" 19'-8" 21'-6" 22'-4" 24'-1" 24'-6" 24'-9" 26'-5" 26'-10"	24"
	NI-20	16'-10"	15'-5"	14'-6"	13'-5"	16'-10"	15'-5"	14'-6"	13'-5"
	NI-40x	18'-8"	17'-2"	16'-3"	14'-11"	18'-10"	17'-2"	16'-3"	14'-11'
9-1/2"	NI-60	18'-11"	17'-6"	16'-6"	15'-5"	19'-2"	17'-6"	16'-6"	15'-5"
	NI-80	20'-3"	18'-10"	17'-11"	16'-10"	20'-8"	19'-3"	18'-2"	16'-10'
	NI-20	20'-1"	18'-5"	17'-5"	16'-1"	20'-1"	18'-5"	17'-5"	16'-1"
	NI-40x	21'-10"	20'-4"	19'-0"	17'-0"	22'-5"	20'-6"	19'-0"	17'-0"
11-7/8"	NI-60	22'-1"	20'-7"	19'-8"	18'-4"	22'-8"	20'-10"	19'-8"	18'-4"
	NI-80	23'-8"	22'-0"	20'-11"	19'-10"	24'-1"	22'-6"	21'-6"	20'-0"
	NI-90	24'-1"	22'-5"	21'-4"	20'-2"	24'-7"	22'-11"	re spacing 19.2" 14'-6" 16'-3" 16'-6" 18'-2" 17'-5" 19'-0" 19'-8" 21'-6" 22'-4" 24'-1" 24'-6" 24'-9" 26'-5"	20'-7"
	NI-40x	24'-5"	22'-9"	20'-11"	18'-8"	25'-1"	22'-11"	20'-11"	18'-8"
	NI-60	24'-10"	23'-2"	22'-1"	20'-10"	25'-6"	23'-8"	22'-4"	20'-10'
14"	NI-80	26'-6"	24'-8"	23'-6"	22'-2"	27'-1"	25'-3"	24'-1"	22'-9"
	NI-90	27'-0"	25'-1"	23'-11"	22'-7"	27'-6"	25'-8"	24'-6"	23'-2"
	NI-60	27'-3"	25'-5"	24'-3"	22'-11"	28'-0"	26'-2"	24'-9"	23'-1"
16"	NI-80	29'-1"	27'-1"	25'-9"	24'-4"	29'-8"	27'-9"	26'-5"	25'-0"
	NI-90	29'-7"	27'-6"	26'-2"	24'-9"	30'-2"	28'-2"	26'-10"	25'-5"

- 1. The tabulated clear spans are based on CSA O86-14 and NBC 2015, and are applicable to residential floor construction meeting the above design criteria.
- 2. For multiple-span applications, the end spans shall be 40% or more of the adjacent span.
- 3. Minimum bearing length shall be 1-3/4 inch for end bearings, and 3-1/2 inches for intermediate bearings.
- 4. Bearing stiffeners are not required when I-joists are used in accordance with this table, except as required for hangers.
- 5. Nordic I-joists are listed in CCMC Evaluation Report 13032-R and APA Product Report PR-L274C.

CITY OF RICHMOND HILL BUILDING DIVISION

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Maximum Floor Spans – M6.1

Design Criteria Spans:

Simple span

Live load = 40 psf and dead load = 20 psf Loads: Deflection limits: L/480 under live load and L/240 under total load

Sheathing:

5/8 in. nailed-glued Canadian softwood plywood

			E	Bare			1/2 in. gy	osum ceiling	
Joist depth	Joist series		On cent	re spacing			On cent	re spacing	
		12"	16"	19.2"	24"	12"	16"	re spacing 19.2" 14'-1" 14'-11" 15'-10" 15'-10" 16'-9" 16'-11" 17'-9" 18'-1" 18'-6" 19'-9" 20'-1" 20'-2" 21'-6"	24'
	NI-20	14'-11"	14'-1"	13'-7"	-	15'-4"	14'-6"	14'-1"	-
0.4/0!!	NI-40x	15'-11"	15'-0"	14'-6"	-	16'-4"	15'-5"	14'-11"	-
9-1/2"	NI-60	16'-1"	15'-2"	14'-8"	-	16'-6"	15'-7"	15'-1"	-
	NI-80	17'-1"	16'-1"	15'-6"	-	17'-5"	16'-5"	15'-10"	-
	NI-20	16'-9"	15'-10"	15'-4"	-	17'-4"	16'-4"	15'-10"	-
	NI-40x	17'-10"	16'-10"	16'-3"	-	18'-6"	17'-4"	16'-9"	-
11-7/8"	NI-60	18'-1"	17'-0"	16'-5"	-	18'-9"	17'-6"	16'-11"	-
	NI-80	19'-6"	18'-0"	17'-4"	-	20'-1"	18'-7"	17'-9"	-
	NI-90	19'-11"	18'-4"	17'-8"	-	20'-5"	18'-11"	18'-1"	-
	NI-40x	19'-10"	18'-4"	17'-8"	-	20'-6"	19'-1"	18'-3"	-
4.411	NI-60	20'-2"	18'-8"	17'-11"	-	20'-10"	19'-4"	18'-6"	-
14"	NI-80	21'-8"	20'-0"	19'-1"	-	22'-4"	20'-8"	19'-9"	-
	NI-90	22'-1"	20'-5"	19'-6"	-	22'-9"	21'-0"	20'-1"	-
	NI-60	22'-0"	20'-4"	19'-6"	-	22'-9"	21'-1"	20'-2"	-
16"	NI-80	23'-7"	21'-10"	20'-10"	-	24'-4"	22'-6"	21'-6"	-
	NII QO	24'-1"	22'-2"	21'-2"	_	24'-9"	22'-11"	21'-10"	_

Joist depth	Joist series	Mid-span blocking with 1x4 inch strap On centre spacing				Mid-span blocking and 1/2 in. gypsum ceiling			
						On centre spacing			
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
9-1/2"	NI-20	16'-6"	15'-1"	14'-3"	-	16'-6"	15'-1"	14'-3"	-
	NI-40x	17'-9"	16'-10"	15'-11"	-	18'-2"	16'-11"	15'-11"	-
	NI-60	17'-11"	16'-11"	16'-2"	-	18'-5"	17'-2"	16'-2"	-
	NI-80	19'-3"	17'-10"	17'-3"	-	19'-8"	18'-3"	17'-7"	-
11-7/8"	NI-20	19'-4"	18'-0"	17'-1"	-	19'-9"	18'-1"	17'-1"	-
	NI-40x	20'-10"	19'-4"	18'-6"	-	21'-5"	19'-11"	19'-0"	-
	NI-60	21'-1"	19'-7"	18'-8"	-	21'-8"	20'-2"	19'-3"	-
	NI-80	22'-6"	20'-10"	19'-11"	-	23'-1"	21'-5"	20'-5"	-
	NI-90	23'-0"	21'-3"	20'-4"	-	23'-6"	21'-10"	20'-10"	-
14"	NI-40x	23'-5"	21'-8"	20'-9"	-	24'-0"	22'-5"	20'-11"	-
	NI-60	23'-9"	22'-0"	21'-0"	-	24'-5"	22'-8"	21'-8"	-
	NI-80	25'-4"	23'-6"	22'-5"	-	25'-11"	24'-1"	23'-0"	-
	NI-90	25'-10"	23'-11"	22'-9"	-	26'-5"	24'-6"	23'-4"	-
16"	NI-60	26'-2"	24'-3"	23'-2"	-	26'-11"	25'-0"	23'-11"	-
	NI-80	27'-11"	25'-10"	24'-7"	-	28'-7"	26'-6"	25'-3"	-
	NI-90	28'-5"	26'-3"	25'-0"	_	29'-0"	26'-11"	25'-8"	-

- 1. The tabulated clear spans are based on CSA O86-14 and NBC 2015, and are applicable to residential floor construction meeting the above design criteria.
- 2. For multiple-span applications, the end spans shall be 40% or more of the adjacent span.
- 3. Minimum bearing length shall be 1-3/4 inch for end bearings, and 3-1/2 inches for intermediate bearings.
- 4. Bearing stiffeners are not required when I-joists are used in accordance with this table, except as required for hangers.
- 5. Nordic I-joists are listed in CCMC Evaluation Report 13032-R and APA Product Report PR-L274C.

CITY OF RICHMOND HILL BUILDING DIVISION

08/13/2021

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Maximum Floor Spans - M7.1

Design Criteria

Spans:

Simple span

Loads:

Live load = 40 psf and dead load = 20 psf

Deflection limits:

L/480 under live load and L/240 under total load

Sheathing:

3/4 in. nailed-glued Canadian softwood plywood

	Joist series	Bare On centre spacing				1/2 in. gypsum ceiling On centre spacing				
Joist depth										
		12"	16"	19.2"	24"	12"	16"	19.2"	24"	
9-1/2"	NI-20	15'-10"	15'-0"	14'-5"	13'-5"	16'-4"	15'-5"	14'-6"	13'-5"	
	NI-40x	16'-11"	15'-11"	15'-4"	14'-9"	17'-4"	16 '-4"	15'-9"	14'-11'	
	NI-60	17'-1"	16'-1"	15'-6"	14'-10"	17'-6"	16'-6"	15'-11"	15'-3"	
	NI-80	18'-1"	17'-0"	16'-4"	15'-8"	18'-7"	17'-4"	16'-8"	16'-0"	
11-7/8"	NI-20	17'-10"	16'-10"	16'-2"	15'-7"	18'-5"	17'-4"	16'-9"	16'-1"	
	NI-40x	19'-3"	17'-10"	17'-2"	16'-6"	19'-10"	18'-5"	17'-8"	16'-11	
	NI-60	19'-6"	18'-1"	17'-4"	16'-8"	20'-1"	18'-8"	17'-10"	17'-1"	
	NI-80	20'-11"	19'-4"	18'-5"	17'-7"	21'-5"	19'-10"	18'-11"	17'-11'	
	NI-90	21'-4"	19'-9"	18'-9"	17'-10"	21'-10"	20'-3"	19'-3"	18'-3"	
14"	NI-40x	21'-4"	19'-9"	18'-10"	17'-11"	22'-0"	20'-5"	19'-6"	18'-6"	
	NI-60	21'-8"	20'-1"	19'-2"	18'-2"	22'-4"	20'-9"	19'-9"	18'-9"	
	NI-80	23'-3"	21'-6"	20'-5"	19'-4"	23'-10"	22'-1"	21'-0"	19'-11'	
	NI-90	23'-9"	21'-11"	20'-10"	19'-8"	24'-3"	22'-6"	21'-5"	20'-3"	
16"	NI-60	23'-7"	21'-10"	20'-10"	19'-9"	24'-4"	22'-7"	21'-7"	20'-5"	
	NI-80	25'-4"	23'-5"	22'-3"	21'-1"	26'-0"	24'-1"	22'-11"	21'-8"	
	NI-90	25'-10"	23'-10"	22'-8"	21'-5"	26'-5"	24'-6"	23'-4"	22'-0"	

	Joist series	Mid-span blocking with 1x4 inch strap On centre spacing				Mid-span blocking and 1/2 in. gypsum ceiling On centre spacing				
Joist depth										
		12"	16"	19.2"	24"	12"	16"	19.2"	24"	
9-1/2"	NI-20	16'-10"	15'-5"	14'-6"	13'-5"	16'-10"	15'-5"	14'-6"	13'-5"	
	NI-40x	18'-7"	17'-2"	16'-3"	14'-11"	18'-10"	17'-2"	16'-3"	14'-11'	
	NI-60	18'-10"	17'-6"	16'-6"	15'-5"	19'-1"	17'-6"	16'-6"	15'-5"	
	NI-80	20'-2"	18'-9"	17'-11"	16'-10"	20'-7"	19'-2"	18'-2"	16'-10	
11-7/8"	NI-20	20'-1"	18'-5"	17'-5"	16'-1"	20'-1"	18'-5"	17'-5"	16'-1"	
	NI-40x	21'-9"	20'-3"	19'-0"	17'-0"	22'-4"	20'-5"	19'-0"	17'-0"	
	NI-60	22'-0"	20'-6"	19'-7"	18'-4"	22'-7"	20'-10"	19'-8"	18'-4"	
	NI-80	23'-6"	21'-10"	20'-10"	19'-9"	24'-0"	22'-5"	21'-4"	20'-0"	
	NI-90	24'-0"	22'-4"	21'-3"	20'-1"	24'-6"	22'-10"	21'-9"	20'-7"	
14"	NI-40x	24'-4"	22'-8"	20'-11"	18'-8"	25'-0"	22'-11"	20'-11"	18'-8"	
	NI-60	24'-9"	23'-0"	22'-0"	20'-9"	25'-5"	23'-8"	22'-4"	20'-10	
	NI-80	26'-5"	24'-6"	23'-4"	22'-1"	27'-0"	25'-2"	24'-0"	22'-8"	
	NI-90	26'-11"	25'-0"	23'-10"	22'-6"	27'-5"	25'-7"	24'-5"	23'-1"	
16"	NI-60	27'-2"	25'-4"	24'-2"	22'-10"	27'-11"	26'-1"	24'-9"	23'-1"	
	NI-80	29'-0"	26'-11"	25'-8"	24'-3"	29'-7"	27'-7"	26'-4"	24'-11	
	NI-90	29'-6"	27'-5"	26'-1"	24'-8"	30'-1"	28'-1"	26'-9"	25'-4"	

- 1. The tabulated clear spans are based on CSA O86-14 and NBC 2015, and are applicable to residential floor construction meeting the above design criteria.
- 2. For multiple-span applications, the end spans shall be 40% or more of the adjacent span.
- 3. Minimum bearing length shall be 1-3/4 inch for end bearings, and 3-1/2 inches for intermediate bearings.
- 4. Bearing stiffeners are not required when I-joists are used in accordance with this table, except as required for hangers.
- 5. Nordic I-joists are listed in CCMC Evaluation Report 13032-R and APA Product Report PR-L274C.

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