

FROM PLAN DATED:

BUILDER: ROYAL PINE HOMES

SITE: CENTREFIELD WEST GORMLEY

MODEL: 2009 END

ELEVATION: A,B

LOT:

CITY: RICHMOND HILL

SALESMAN: MARIO DICIANO

DESIGNER: AJ

REVISION:

NOTES:

REFER TO THE **NORDIC INSTALLATION** GUIDE FOR PROPER STORAGE AND INSTALLATION.

**SQUASH BLOCKS** OF 2x4, 2x6, 2x8 #2 S.P.F REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. **MULTIPLE SQUASH BLOCKS** REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. **CANTILEVERED JOISTS** INCLUDING **CANT' OVER BRICK** RE I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR **HOLES** INCLUDING **DUCT CHASE** AND **FIELD CUT OPENINGS** SEE FIGURE 7, TABLES 1 & 2. **CERAMIC TIL** APPLICATION AS PER O.B.C 9.30.6.

LOADING:

DESIGN LOADS: L/480.000

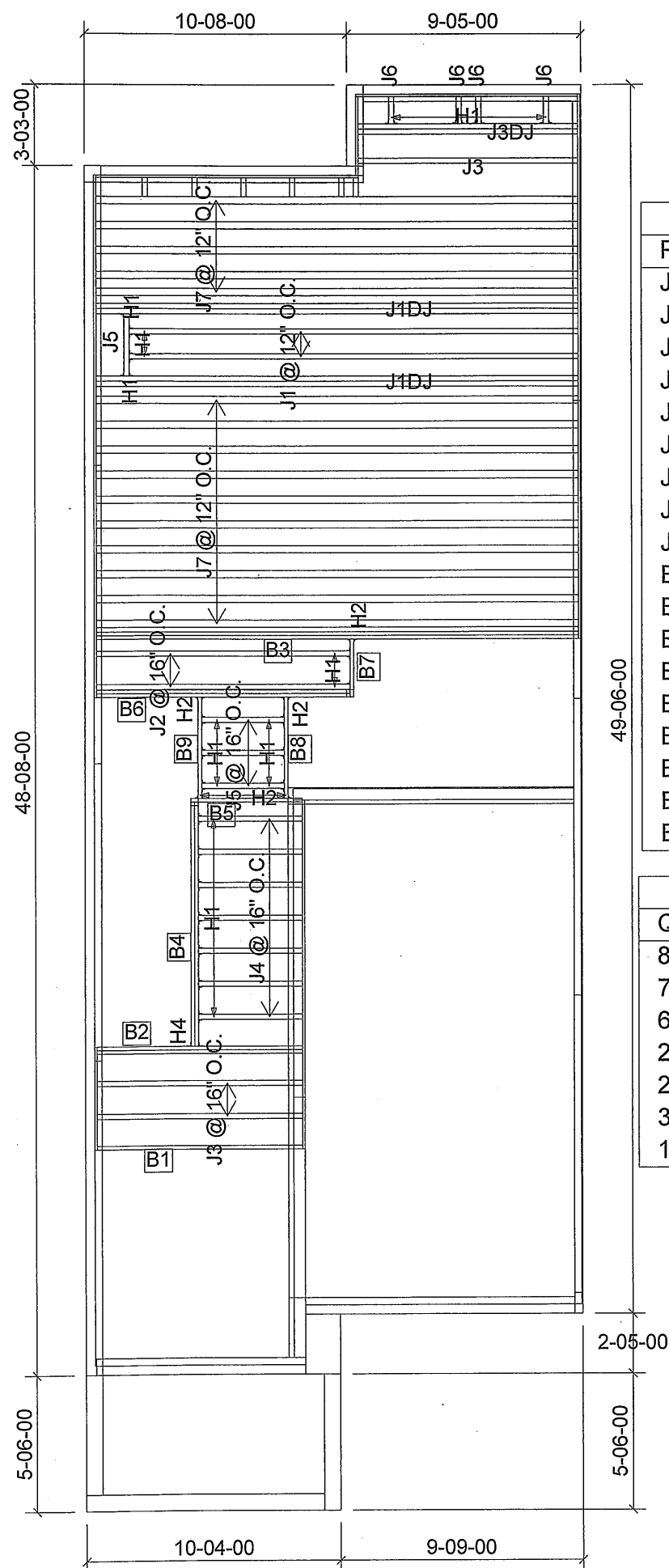
LIVE LOAD: 40.0 lb/ft<sup>2</sup>

DEAD LOAD: 20.0 lb/ft<sup>2</sup>

SUBFLOOR: 3/4" GLUED AND NAILED

DATE: 2020-08-06

1st FLOOR



| Products |          |                                         |       |         |
|----------|----------|-----------------------------------------|-------|---------|
| PlotID   | Length   | Product                                 | Plies | Net Qty |
| J1       | 20-00-00 | 11 7/8" NI-40x                          | 1     | 2       |
| J1DJ     | 20-00-00 | 11 7/8" NI-40x                          | 2     | 4       |
| J2       | 12-00-00 | 11 7/8" NI-40x                          | 1     | 2       |
| J3       | 10-00-00 | 11 7/8" NI-40x                          | 1     | 3       |
| J3DJ     | 10-00-00 | 11 7/8" NI-40x                          | 2     | 2       |
| J4       | 6-00-00  | 11 7/8" NI-40x                          | 1     | 7       |
| J5       | 4-00-00  | 11 7/8" NI-40x                          | 1     | 4       |
| J6       | 2-00-00  | 11 7/8" NI-40x                          | 1     | 4       |
| J7       | 20-00-00 | 11 7/8" NI-80                           | 1     | 15      |
| B3       | 20-00-00 | 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP | 2     | 2       |
| B6       | 12-00-00 | 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP | 2     | 2       |
| B1       | 10-00-00 | 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP | 1     | 1       |
| B2       | 10-00-00 | 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP | 2     | 2       |
| B4       | 10-00-00 | 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP | 2     | 2       |
| B5       | 6-00-00  | 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP | 1     | 1       |
| B8       | 6-00-00  | 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP | 1     | 1       |
| B9       | 6-00-00  | 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP | 1     | 1       |
| B7       | 4-00-00  | 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP | 1     | 1       |

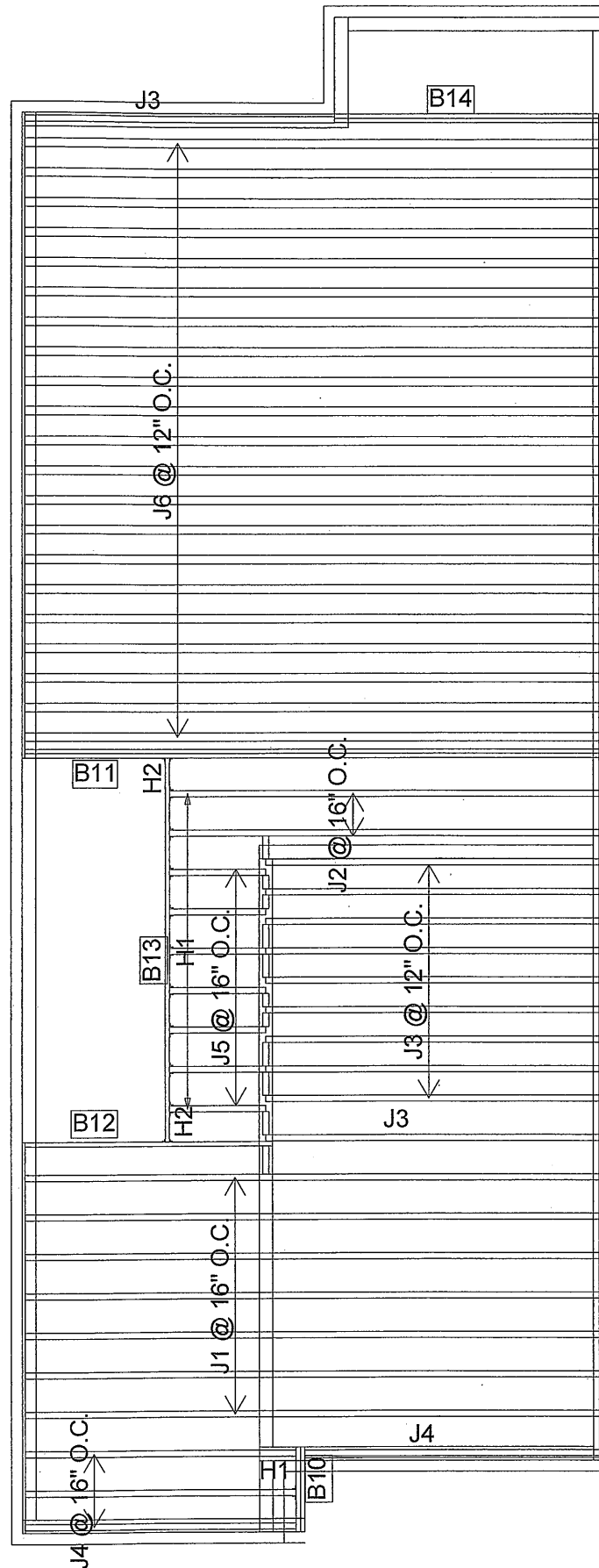
| Connector Summary |       |               |
|-------------------|-------|---------------|
| Qty               | Manuf | Product       |
| 8                 | H1    | IUS2.56/11.88 |
| 7                 | H1    | IUS2.56/11.88 |
| 6                 | H1    | IUS2.56/11.88 |
| 2                 | H1    | IUS2.56/11.88 |
| 2                 | H2    | HUS1.81/10    |
| 3                 | H2    | HUS1.81/10    |
| 1                 | H4    | HGUS410       |

CITY OF RICHMOND HILL  
BUILDING DIVISION

07/20/2021

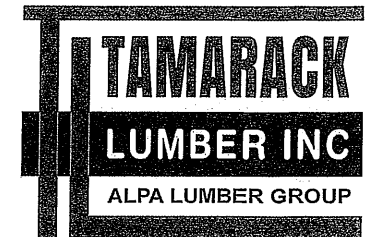
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| Products |          |                                         |       |         |
|----------|----------|-----------------------------------------|-------|---------|
| PlotID   | Length   | Product                                 | Plies | Net Qty |
| J1       | 20-00-00 | 11 7/8" NI-40x                          | 1     | 7       |
| J2       | 16-00-00 | 11 7/8" NI-40x                          | 1     | 2       |
| J3       | 12-00-00 | 11 7/8" NI-40x                          | 1     | 11      |
| J4       | 10-00-00 | 11 7/8" NI-40x                          | 1     | 4       |
| J5       | 4-00-00  | 11 7/8" NI-40x                          | 1     | 7       |
| J6       | 20-00-00 | 11 7/8" NI-80                           | 1     | 21      |
| B11      | 20-00-00 | 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP | 2     | 2       |
| B13      | 14-00-00 | 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP | 1     | 1       |
| B12      | 10-00-00 | 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP | 1     | 1       |
| B14      | 10-00-00 | 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP | 2     | 2       |
| B10      | 4-00-00  | 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP | 2     | 2       |

| Connector Summary |       |               |
|-------------------|-------|---------------|
| Qty               | Manuf | Product       |
| 9                 | H1    | IUS2.56/11.88 |
| 1                 | H1    | IUS2.56/11.88 |
| 1                 | H2    | HUS1.81/10    |
| 1                 | H2    | HUS1.81/10    |



FROM PLAN DATED:

BUILDER: ROYAL PINE HOMES

SITE: CENTREFIELD WEST GORMLEY

MODEL: 2009 END

ELEVATION: A,B

LOT:

CITY: RICHMOND HILL

SALESMAN: MARIO DICIANO

DESIGNER: AJ

REVISION:

NOTES:

REFER TO THE NORDIC **INSTALLATION GUIDE** FOR PROPER STORAGE AND INSTALLATION. **SQUASH BLOCKS** OF 2x4, 2x6, 2x8 #2 S.P.F. REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. **MULTIPLE SQUASH BLOCKS** REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. **CANTILEVERED JOISTS** INCLUDING **CANT' OVER BRICK** REQ. I-JOIST BLOCKING ALO BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURE 7 TABLES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR **HOLES** INCLUDING **DUCT CHASE** AND **FIEL CUT OPENINGS** SEE FIGURE 7 TABLES 1 & OF THE INSTALLATION GUIDE. **CERAMIC T** APPLICATION AS PER O.B.C. 9.30.6

LOADING:

DESIGN LOADS: L/480.000

LIVE LOAD: 40.0 lb/ft²

DEAD LOAD: 20.0 lb/ft²

SUBFLOOR: 5/8" GLUED AND NAILED

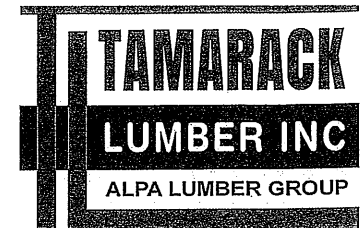
DATE: 2020-08-06

2nd FLOOR

CITY OF RICHMOND HILL  
BUILDING DIVISION

07/20/2021

RECEIVED  
Per: jocelyn.aguilar



FROM PLAN DATED:

BUILDER: ROYAL PINE HOMES

SITE: CENTREFIELD WEST GORMLEY

MODEL: 2009

ELEVATION: A,B

LOT:

CITY: RICHMOND HILL

SALESMAN: MARIO DICIANO

DESIGNER: AJ

REVISION:

NOTES:

REFER TO THE **NORDIC INSTALLATION** GUIDE FOR PROPER STORAGE AND INSTALLATION.

**SQUASH BLOCKS** OF 2x4, 2x6, 2x8 #2 S.P. REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. **MULTIPLE SQUASH BLOCKS** REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. **CANTILEVERED JOISTS** INCLUDING **CANT' OVER BRICK** RI I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR **HOLES** INCLUDING **DUCT CHASE** AND **FIELD CUT OPENINGS** SEE FIGURE 7, TABLES 1 & 2. **CERAMIC T** APPLICATION AS PER O.B.C 9.30.6.

LOADING:

DESIGN LOADS: L/480.000

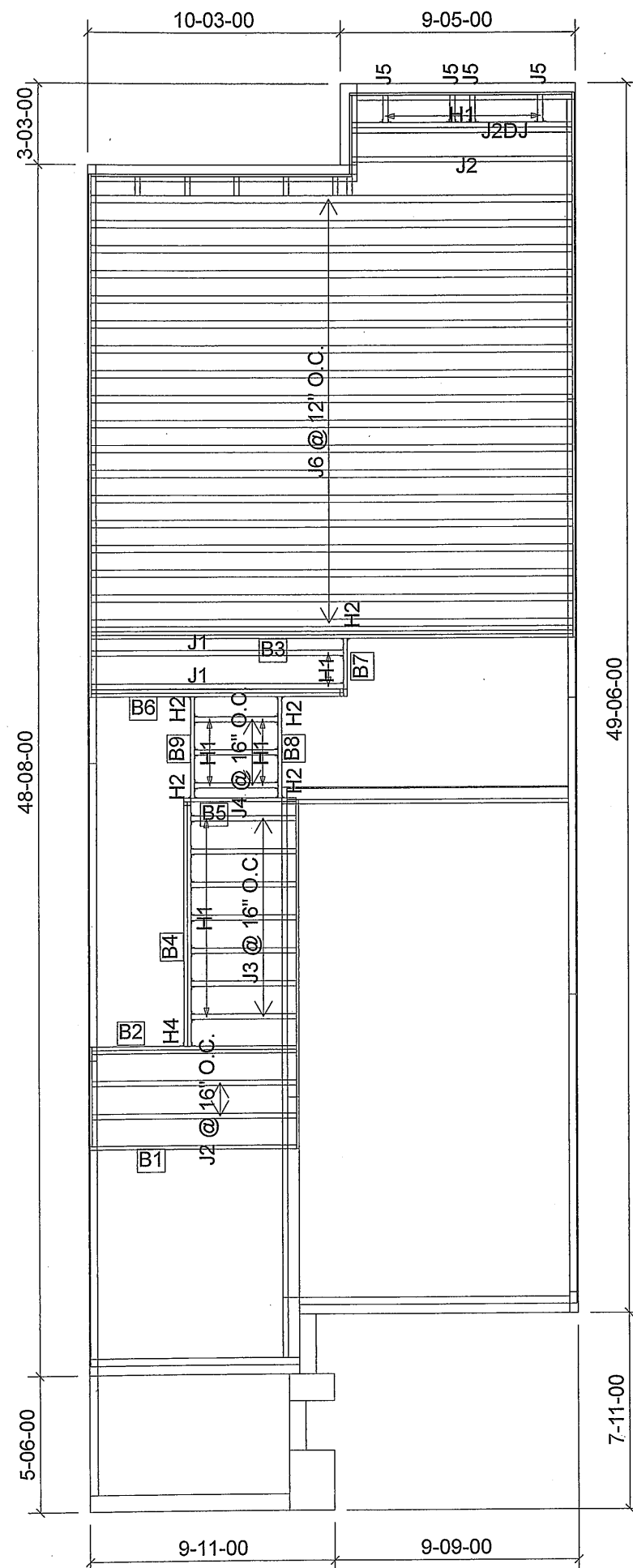
LIVE LOAD: 40.0 lb/ft<sup>2</sup>

DEAD LOAD: 20.0 lb/ft<sup>2</sup>

SUBFLOOR: 3/4" GLUED AND NAILED

DATE: 2020-07-22

1st FLOOR



| Products |          |                                         |       |         |
|----------|----------|-----------------------------------------|-------|---------|
| PlotID   | Length   | Product                                 | Plies | Net Qty |
| J1       | 12-00-00 | 11 7/8" NI-40x                          | 1     | 2       |
| J2       | 10-00-00 | 11 7/8" NI-40x                          | 1     | 3       |
| J2DJ     | 10-00-00 | 11 7/8" NI-40x                          | 2     | 2       |
| J3       | 6-00-00  | 11 7/8" NI-40x                          | 1     | 7       |
| J4       | 4-00-00  | 11 7/8" NI-40x                          | 1     | 3       |
| J5       | 2-00-00  | 11 7/8" NI-40x                          | 1     | 4       |
| J6       | 20-00-00 | 11 7/8" NI-80                           | 1     | 18      |
| B3       | 20-00-00 | 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP | 2     | 2       |
| B6       | 12-00-00 | 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP | 2     | 2       |
| B1       | 10-00-00 | 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP | 1     | 1✓      |
| B2       | 10-00-00 | 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP | 2     | 2✓      |
| B4       | 10-00-00 | 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP | 2     | 2✓      |
| B5       | 6-00-00  | 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP | 1     | 1       |
| B8       | 6-00-00  | 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP | 1     | 1       |
| B9       | 6-00-00  | 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP | 1     | 1       |
| B7       | 4-00-00  | 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP | 1     | 1       |

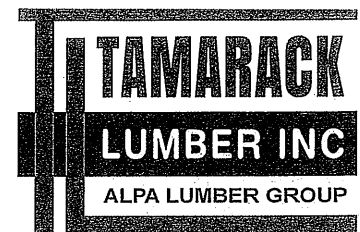
| Connector Summary |       |               |
|-------------------|-------|---------------|
| Qty               | Manuf | Product       |
| 8                 | H1    | IUS2.56/11.88 |
| 7                 | H1    | IUS2.56/11.88 |
| 4                 | H1    | IUS2.56/11.88 |
| 2                 | H2    | HUS1.81/10    |
| 3                 | H2    | HUS1.81/10    |
| 1                 | H4    | HGUS410       |

CITY OF RICHMOND HILL  
BUILDING DIVISION

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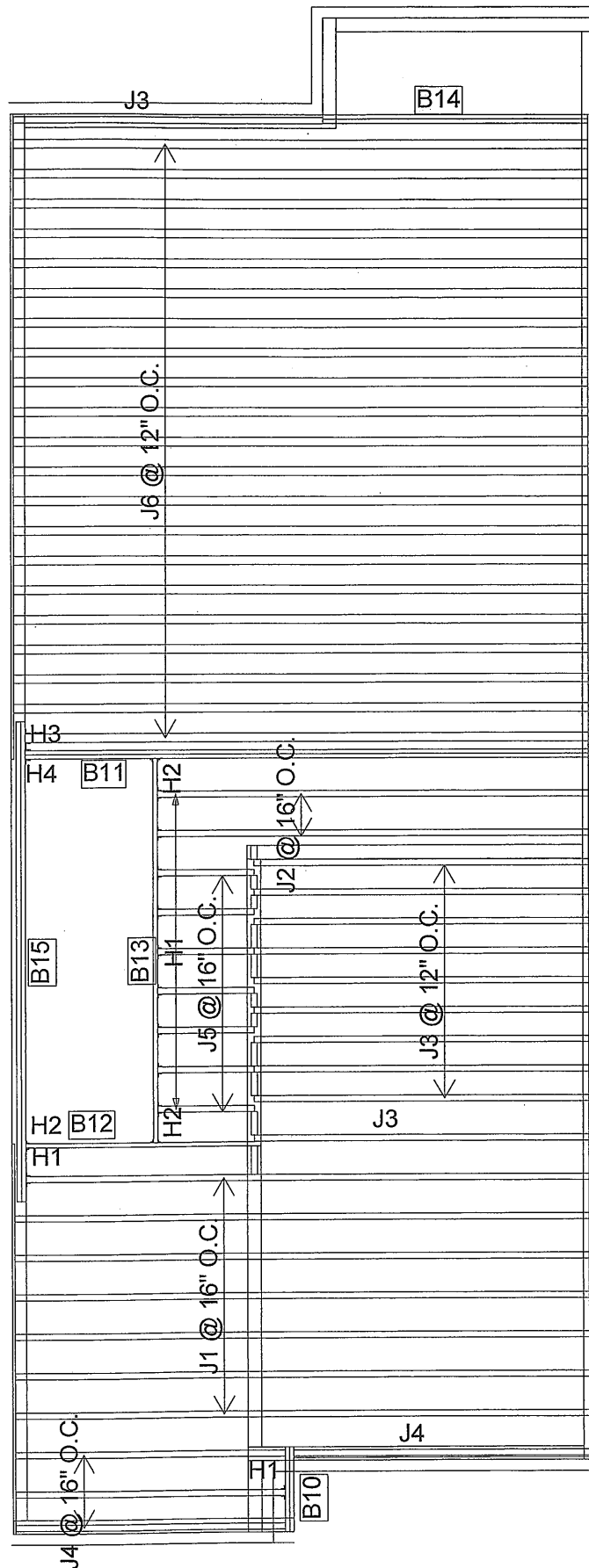
FROM PLAN DATED:  
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SITE: CENTREFIELD WEST GORMLEY  
MODEL: 2009  
ELEVATION: A,B  
LOT:  
CITY: RICHMOND HILL  
SALESMAN: MARIO DICIANO  
DESIGNER: AJ  
REVISION:

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DESIGN LOADS: L/480.000  
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DEAD LOAD: 20.0 lb/ft²  
SUBFLOOR: 5/8" GLUED AND NAILED

DATE: 2020-07-27

2nd FLOOR



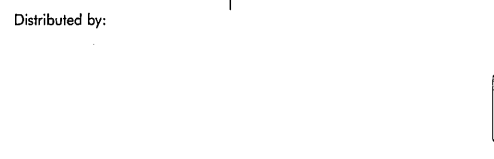
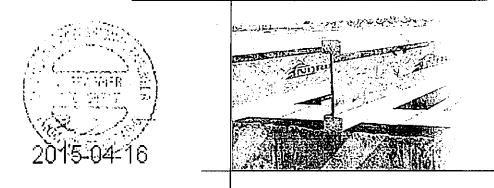
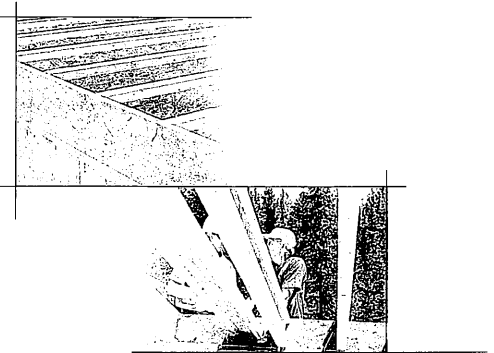
| Products |          |                                         |       |         |
|----------|----------|-----------------------------------------|-------|---------|
| PlotID   | Length   | Product                                 | Plies | Net Qty |
| J1       | 20-00-00 | 11 7/8" NI-40x                          | 1     | 7       |
| J2       | 16-00-00 | 11 7/8" NI-40x                          | 1     | 2       |
| J3       | 12-00-00 | 11 7/8" NI-40x                          | 1     | 11      |
| J4       | 10-00-00 | 11 7/8" NI-40x                          | 1     | 4       |
| J5       | 4-00-00  | 11 7/8" NI-40x                          | 1     | 7       |
| J6       | 20-00-00 | 11 7/8" NI-80                           | 1     | 21      |
| B11      | 20-00-00 | 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP | 2     | 2       |
| B15      | 18-00-00 | 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP | 2     | 2       |
| B13      | 14-00-00 | 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP | 1     | 1       |
| B14      | 10-00-00 | 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP | 2     | 2       |
| B12      | 8-00-00  | 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP | 1     | 1       |
| B10      | 4-00-00  | 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP | 2     | 2       |

| Connector Summary |       |               |
|-------------------|-------|---------------|
| Qty               | Manuf | Product       |
| 9                 | H1    | IUS2.56/11.88 |
| 2                 | H1    | IUS2.56/11.88 |
| 1                 | H2    | HUS1.81/10    |
| 2                 | H2    | HUS1.81/10    |
| 1                 | H3    | IUS3.56/11.88 |
| 1                 | H4    | HGUS410       |

CITY OF RICHMOND HILL  
BUILDING DIVISION  
07/20/2021  
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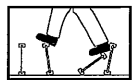


## INSTALLATION GUIDE FOR RESIDENTIAL FLOORS

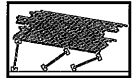


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### SAFETY AND CONSTRUCTION PRECAUTIONS



Do not walk on I-joists until fully fastened and braced, or serious injuries can result.

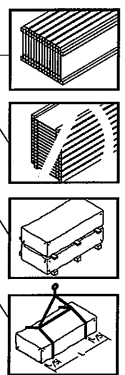


Never stack building materials over unshelved I-joists. Once sheathed, do not over-stress I-joist with concentrated loads from building materials.

Improper storage or installation, failure to follow applicable building codes, failure to follow span ratings for Nordic I-joists, failure to follow allowable hole sizes and locations, or failure to use web stiffeners when required can result in serious accidents. Follow these installation guidelines carefully.

### STORAGE AND HANDLING GUIDELINES

- Bundle wrap can be slippery when wet. Avoid walking on wrapped bundles.
- Store, stack, and handle I-joists vertically and level only.
- Always stack and handle I-joists in the upright position only.
- Do not store I-joists in direct contact with the ground and/or flatwise.
- Protect I-joists from weather, and use spacers to separate bundles.
- Bundled units should be kept intact until time of installation.
- When handling I-joists with a crane on the job site, take a few simple precautions to prevent damage to the I-joists and injury to your work crew.
  - Pick I-joists in bundles as shipped by the supplier.
  - Orient the bundles so that the webs of the I-joists are vertical.
  - Pick the bundles at the 5th points, using a spreader bar if necessary.
- Do not handle I-joists in a horizontal orientation.
- NEVER USE OR TRY TO REPAIR A DAMAGED I-JOIST.



### MAXIMUM FLOOR SPANS

- Maximum clear spans applicable to simple-span or multiple-span residential floor construction with a design live load of 40 psf and dead load of 15 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration and a live load deflection limit of L/480. For multiple-span applications, the end spans shall be 40% or more of the adjacent span.
- Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 5/8 inch for a joist spacing of 19.2 inches or less, or 3/4 inch for joist spacing of 24 inches. Adhesive shall meet the requirements given in CGS-71.26 Standard. No concrete topping or bridging element was assumed. Increased spans may be achieved with the use of gypsum and/or a row of blocking at mid-span.
- Minimum bearing length shall be 1-3/4 inches for the end bearings, and 3-1/2 inches for the intermediate bearings.
- Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.
- This span chart is based on uniform loads. For applications with other than uniform loads, an engineering analysis may be required based on the use of the design properties.
- Tables are based on Limit States Design per CAN/CSA C086-09 Standard, and NBC 2010.
- SI units conversion: 1 inch = 25.4 mm, 1 foot = 0.305 m

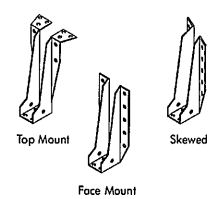
### MAXIMUM FLOOR SPANS FOR NORDIC I-JOISTS SIMPLE AND MULTIPLE SPANS

| Joist Size | Joist Spacing | Simple Span |        |        |        | Multiple Span |        |         |        |
|------------|---------------|-------------|--------|--------|--------|---------------|--------|---------|--------|
|            |               | 12"         | 16"    | 19.2"  | 24"    | 12"           | 16"    | 19.2"   | 24"    |
| 9-1/2"     | NI-20         | 15'-1"      | 14'-2" | 13'-9" | 13'-5" | 16'-3"        | 15'-4" | 14'-10" | 14'-7" |
|            | NI-40x        | 16'-1"      | 15'-2" | 14'-8" | 14'-9" | 17'-5"        | 16'-5" | 15-10"  | 15-5"  |
|            | NI-60         | 16'-3"      | 15'-4" | 14-10" | 14-11" | 17'-7"        | 16-7"  | 16-0"   | 16-1"  |
|            | NI-70         | 17-1"       | 16-1"  | 15-6"  | 15-7"  | 18-7"         | 17-4"  | 16-9"   | 16-10" |
| 11-7/8"    | NI-80         | 17-3"       | 16-3"  | 15-8"  | 15-9"  | 18-10"        | 17-6"  | 16-11"  | 17-0"  |
|            | NI-20         | 16-11"      | 16-0"  | 15-5"  | 15-6"  | 18-4"         | 17-3"  | 16-8"   | 16-7"  |
|            | NI-40x        | 18-1"       | 17-0"  | 16-5"  | 16-6"  | 20-0"         | 18-6"  | 17-9"   | 17-7"  |
|            | NI-60         | 18-4"       | 17-3"  | 16-7"  | 16-9"  | 20-3"         | 18-9"  | 18-0"   | 18-1"  |
| 14"        | NI-70         | 19-6"       | 18-0"  | 17-4"  | 17-5"  | 21-4"         | 19-11" | 19-0"   | 19-1"  |
|            | NI-80         | 19-9"       | 18-3"  | 17-6"  | 17-7"  | 21-9"         | 20-2"  | 19-3"   | 19-4"  |
|            | NI-90         | 20-2"       | 18-7"  | 17-10" | 17-11" | 22-3"         | 20-7"  | 19-8"   | 19-9"  |
|            | NI-90x        | 20-4"       | 18-9"  | 17-11" | 18-0"  | 22-5"         | 20-9"  | 19-10"  | 19-11" |
| 16"        | NI-40x        | 20-1"       | 18-7"  | 17-10" | 17-11" | 22-2"         | 20-6"  | 19-8"   | 19-4"  |
|            | NI-60         | 20-5"       | 18-11" | 18-1"  | 18-2"  | 22-7"         | 20-11" | 20-0"   | 20-1"  |
|            | NI-70         | 21-7"       | 20-0"  | 19-1"  | 19-2"  | 23-10"        | 21-1"  | 21-2"   | 21-2"  |
|            | NI-80         | 21-11"      | 20-3"  | 19-4"  | 19-5"  | 24-3"         | 22-5"  | 21-5"   | 21-6"  |
| 18"        | NI-90         | 22-5"       | 20-8"  | 19-9"  | 19-10" | 24-9"         | 23-10" | 21-10"  | 21-10" |
|            | NI-90x        | 22-7"       | 20-11" | 19-11" | 20-0"  | 25-0"         | 23-1"  | 22-0"   | 22-2"  |
| 20"        | NI-60         | 22-3"       | 20-8"  | 19-9"  | 19-10" | 24-7"         | 22-9"  | 21-9"   | 21-10" |
|            | NI-70         | 23-6"       | 21-9"  | 20-9"  | 20-10" | 26-0"         | 24-0"  | 22-11"  | 23-0"  |
|            | NI-80         | 23-11"      | 22-1"  | 21-1"  | 21-2"  | 26-5"         | 24-5"  | 23-4"   | 23-4"  |
|            | NI-90         | 24-5"       | 22-6"  | 21-5"  | 21-6"  | 26-11"        | 24-10" | 23-9"   | 23-9"  |
| 22"        | NI-90x        | 24-8"       | 22-9"  | 21-9"  | 21-10" | 27-3"         | 25-2"  | 24-0"   | 24-1"  |

CCMC EVALUATION REPORT 13032-R

### I-JOIST HANGERS

- Hangers shown illustrate the three most commonly used metal hangers to support I-joists.
- All nailing must meet the hanger manufacturer's recommendations.
- Hangers should be selected based on the joist depth, flange width and load capacity based on the maximum spans.
- Web stiffeners are required when the sides of the hangers do not laterally brace the top flange of the I-joist.



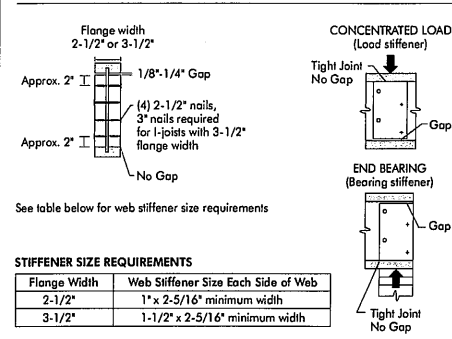
### WEB STIFFENERS

#### RECOMMENDATIONS:

- A bearing stiffener is required in all engineered applications with factored reactions greater than shown in the I-joist properties table found in the I-joist Construction Guide (C101). The gap between the stiffener and the flange is at the top.
- A bearing stiffener is required when the I-joist is supported in a hanger and the sides of the hanger do not extend up to, and support, the top flange. The gap between the stiffener and flange is at the top.
- A load stiffener is required at locations where a factored concentrated load greater than 2,370 lbs is applied to the top flange between supports, or in the case of a cantilever, anywhere between the cantilever tip and the support. These values are for standard term load duration, and may be adjusted for other load durations as permitted by the code. The gap between the stiffener and the flange is at the bottom.

SI units conversion: 1 inch = 25.4 mm

#### FIGURE 2 WEB STIFFENER INSTALLATION DETAILS



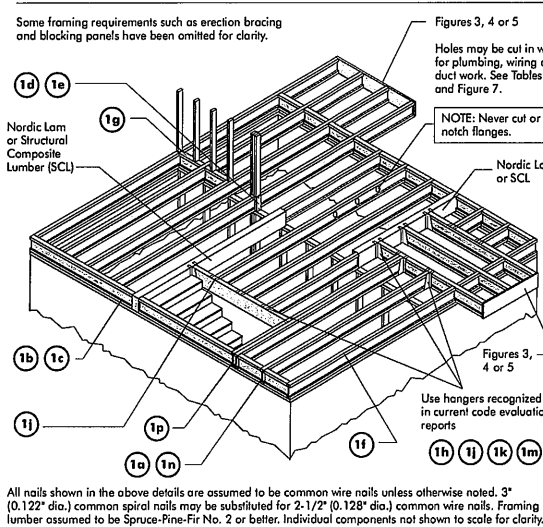
#### STIFFENER SIZE REQUIREMENTS

| Flange Width | Web Stiffener Size Each Side of Web |
|--------------|-------------------------------------|
| 2-1/2"       | 1" x 2-5/16" minimum width          |
| 3-1/2"       | 1-1/2" x 2-5/16" minimum width      |

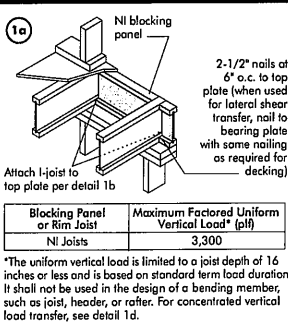
### INSTALLING NORDIC I-JOISTS

- Before laying out floor system components, verify that I-joist flange widths match hanger widths. If not, consult your supplier.
- Except for cutting to length, I-joist flanges should never be cut, drilled, or notched.
- Install I-joists so that top and bottom flanges are within 1/2 inch of true vertical alignment.
- I-joists must be anchored securely to supports before floor sheathing is attached, and supports for multiple-span applications must be level.
- Minimum bearing lengths: 1-3/4 inches for end bearings and 3-1/2 inches for intermediate bearings.
- When using hangers, seal I-joists firmly in hanger bottoms to minimize settlement.
- Leave a 1/16-inch gap between the I-joist end and a header.
- Concentrated loads greater than those that can normally be expected in residential construction should only be applied to the top surface of the top flange. Normal concentrated loads include track lighting fixtures, audio equipment and security cameras. Never suspend unusual or heavy loads from the I-joist's bottom flange. Whenever possible, suspend all concentrated loads from the top of the I-joist. Or, attach the load to blocking that has been securely fastened to the I-joist webs.
- Never install I-joists where they will be permanently exposed to weather, or where they will remain in direct contact with concrete or masonry.
- Restrain ends of floor joists to prevent rollover. Use rim board, rim joists or I-joist blocking panels.
- For I-joists installed over and beneath bearing walls, use full depth blocking panels, rim board, or squash blocks (cripple members) to transfer gravity loads through the floor system to the wall or foundation below.
- Due to shrinkage, common framing lumber set on edge may never be used as blocking or rim boards. I-joist blocking panels or other engineered wood products – such as rim board – must be cut to fit between the I-joists, and an I-joist-compatible depth selected.
- Provide permanent lateral support of the bottom flange of all I-joists at interior supports of multiple-span joists. Similarly, support the bottom flange of all cantilevered I-joists at the end support next to the cantilever extension. In the completed structure, the gypsum wallboard ceiling provides this lateral support. Until the final finished ceiling is applied, temporary bracing or struts must be used.
- If square-edge panels are used, edges must be supported between I-joists with 2x4 blocking. Glue panels to blocking to minimize squeaks. Blocking is not required under structural finish flooring, such as wood strip flooring, or if a separate underlayment layer is installed.
- Nail spacing: Space nails installed to the flange's top face in accordance with the applicable building code requirements or approved building plans.

#### FIGURE 1 TYPICAL NORDIC I-JOIST FLOOR FRAMING AND CONSTRUCTION DETAILS

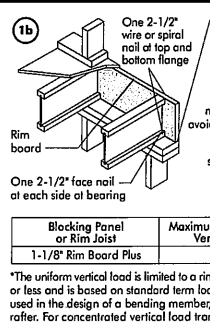


All nails shown in the above details are assumed to be common wire nails unless otherwise noted. 3" (0.122" dia.) common spiral nails may be substituted for 2-1/2" (0.128" dia.) common wire nails. Framing lumber assumed to be Spruce-Pine-Fir No. 2 or better. Individual components not shown to scale for clarity.



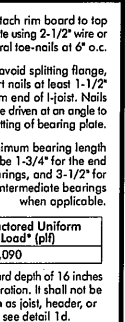
| Blocking Panel or Rim Joist | Maximum Factored Uniform Vertical Load* (plf) |
|-----------------------------|-----------------------------------------------|
| NI Joists                   | 3,300                                         |

\*The uniform vertical load is limited to a joist depth of 16 inches or less and is based on standard term load duration. It shall not be used in the design of a bending member, such as joist, header, or rafter. For concentrated vertical load transfer, see detail 1d.



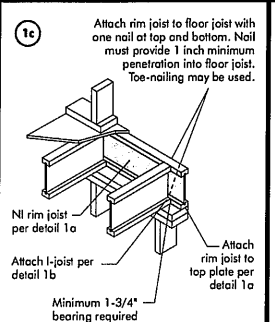
| Blocking Panel or Rim Joist | Maximum Factored Uniform Vertical Load* (plf) |
|-----------------------------|-----------------------------------------------|
| 1-1/8" Rim Board Plus       | 8,090                                         |

\*The uniform vertical load is limited to a rim board depth of 16 inches or less and is based on standard term load duration. It shall not be used in the design of a bending member, such as joist, header, or rafter. For concentrated vertical load transfer, see detail 1d.



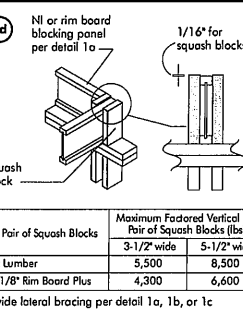
| Pair of Squash Blocks | Maximum Factored Vertical Load per Pair of Squash Blocks (lbs) |
|-----------------------|----------------------------------------------------------------|
| 2x Lumber             | 5,500                                                          |
| 1-1/8" Rim Board Plus | 4,300                                                          |

Provide lateral bracing per detail 1a, 1b, or 1c



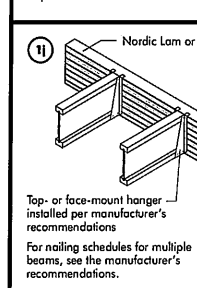
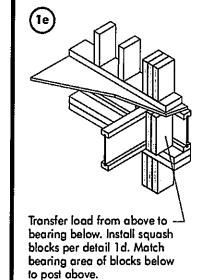
| Pair of Squash Blocks | Maximum Factored Vertical Load per Pair of Squash Blocks (lbs) |
|-----------------------|----------------------------------------------------------------|
| 2x Lumber             | 5,500                                                          |
| 1-1/8" Rim Board Plus | 4,300                                                          |

Provide lateral bracing per detail 1a, 1b, or 1c



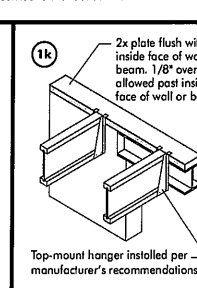
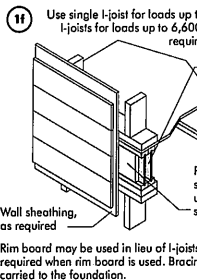
| Pair of Squash Blocks | Maximum Factored Vertical Load per Pair of Squash Blocks (lbs) |
|-----------------------|----------------------------------------------------------------|
| 2x Lumber             | 5,500                                                          |
| 1-1/8" Rim Board Plus | 4,300                                                          |

Provide lateral bracing per detail 1a, 1b, or 1c

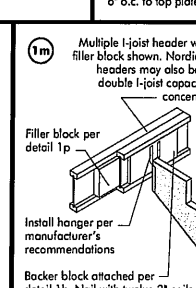
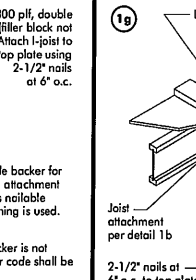


For nailing schedules for multiple bearings, see the manufacturer's recommendations.

Note: Unless hanger sides laterally support the top flange, bearing stiffeners shall be used.

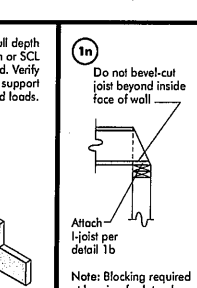
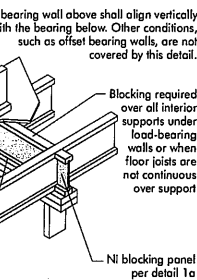


Note: Unless hanger sides laterally support the top flange, bearing stiffeners shall be used.



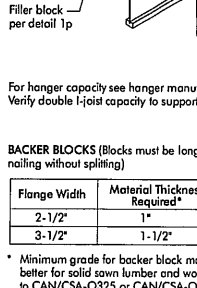
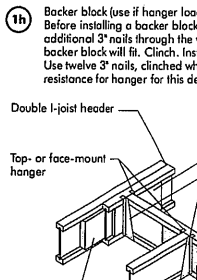
Backer block attached per detail 1h. Nail with twelve 3" nails, clinch when possible.

Maximum support capacity = 1,620 lbs.



Backer block attached per detail 1h. Nail with twelve 3" nails, clinch when possible.

Maximum support capacity = 1,620 lbs.



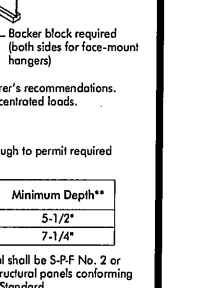
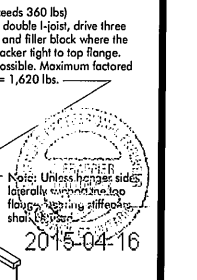
For hanger capacity see hanger manufacturer's recommendations. Verify double I-joist capacity to support concentrated loads.

Backer blocks (Blocks must be long enough to permit required nailing without splitting)

| Flange Width | Material Thickness Required | Minimum Depth** |
|--------------|-----------------------------|-----------------|
| 2-1/2"       | 1"                          | 5-1/2"          |
| 3-1/2"       | 1-1/2"                      | 7-1/4"          |

\* Minimum grade for backer block material shall be S-P-F No. 2 or better for solid sawn lumber and wood structural panels conforming to CAN/CSA-O375 or CAN/CSA-O437 Standard.

\*\* For face-mount hangers use net joist depth minus 3-1/4" for joists with 1-1/2" thick flanges. For 2" thick flanges use net depth minus 4-1/4".



For hanger capacity see hanger manufacturer's recommendations. Verify double I-joist capacity to support concentrated loads.

Backer blocks (Blocks must be long enough to permit required nailing without splitting)

| Flange Width | Material Thickness Required | Minimum Depth** |
|--------------|-----------------------------|-----------------|
| 2-1/2"       | 1"                          | 5-1/2"          |
| 3-1/2"       | 1-1/2"                      | 7-1/4"          |

\* Minimum grade for backer block material shall be S-P-F No. 2 or better for solid sawn lumber and wood structural panels conforming to CAN/CSA-O375 or CAN/CSA-O437 Standard.

\*\* For face-mount hangers use net joist depth minus 3-1/4" for joists with 1-1/2" thick flanges. For 2" thick flanges use net depth minus 4-1/4".



For hanger capacity see hanger manufacturer's recommendations. Verify double I-joist capacity to support concentrated loads.

Backer blocks (Blocks must be long enough to permit required nailing without splitting)

| Flange Width | Material Thickness Required | Minimum Depth** |
|--------------|-----------------------------|-----------------|
| 2-1/2"       | 1"                          | 5-1/2"          |
| 3-1/2"       | 1-1/2"                      | 7-1/4"          |

\* Minimum grade for backer block material shall be S-P-F No. 2 or better for solid sawn lumber and wood structural panels conforming to CAN/CSA-O375 or CAN/CSA-O437 Standard.

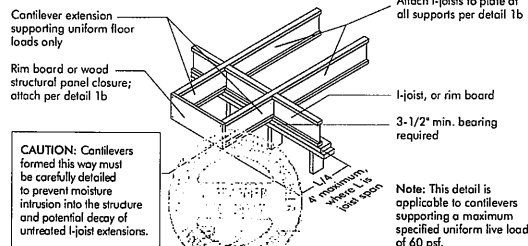
\*\* For face-mount hangers use net joist depth minus 3-1/4" for joists with 1-1/2" thick flanges. For 2" thick flanges use net depth minus 4-1/4".

Note: In some local codes, blocking is prescriptively required in the starter joist space (or first and second joist space) next to the first joist. Where required, see local code requirements for spacing of the blocking.

All nails are common spiral in this detail.

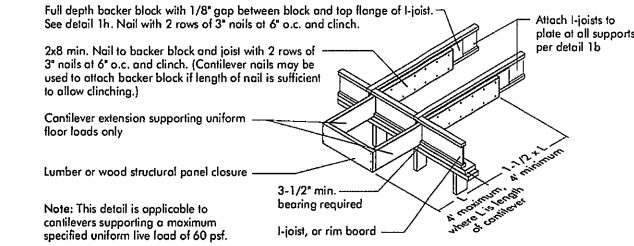
## CANTILEVER DETAILS FOR BALCONIES (NO WALL LOAD)

### 3a) 1-JOIST CANTILEVER DETAIL FOR BALCONIES (No Wall Load)



CAUTION: Cantilevers formed this way must be carefully detailed to prevent moisture intrusion into the structure and potential decay of untreated I-joist extensions.

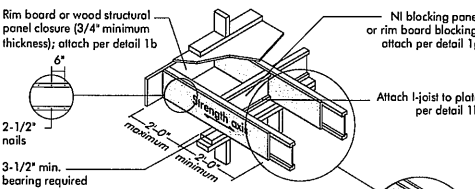
### 3b) LUMBER CANTILEVER DETAIL FOR BALCONIES (No Wall Load)



Note: This detail is applicable to cantilevers supporting a maximum specified uniform live load of 60 psf.

## CANTILEVER DETAILS FOR VERTICAL BUILDING OFFSET (CONCENTRATED WALL LOAD)

### 4a) Method 1 — SHEATHING REINFORCEMENT ONE SIDE

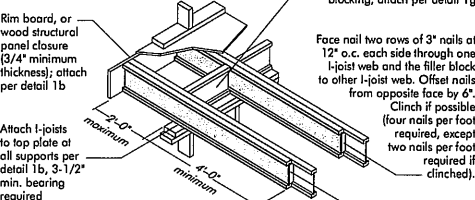


### Method 2 — SHEATHING REINFORCEMENT TWO SIDES

- Use same installation as Method 1 but reinforce both sides of I-joist with sheathing.
- Use nailing pattern shown for Method 1 with opposite face nailing offset by 3\".

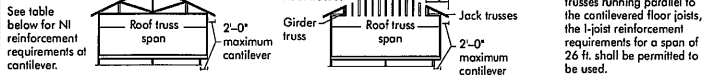
Note: Canadian softwood plywood sheathing or equivalent (minimum thickness 3/4\"/>

### 4b) Alternate Method 2 — DOUBLE I-JOIST



Block I-joists together with filler blocks for the full length of the reinforcement. For I-joist flange widths greater than 3 inches place an additional row of 3\"/>

### FIGURE 4 (continued)



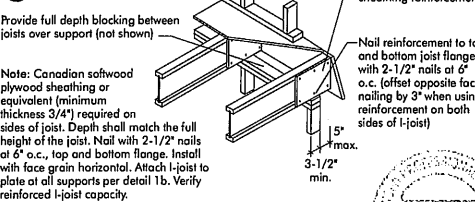
### CANTILEVER REINFORCEMENT METHODS ALLOWED

| JOIST<br>DEPTH<br>(in) | ROOF<br>TRUSS<br>SPAN<br>(ft) | ROOFLOADING (UNFACED)     |    |      |    |                           |    |      |    |                           |    |      |    |   |
|------------------------|-------------------------------|---------------------------|----|------|----|---------------------------|----|------|----|---------------------------|----|------|----|---|
|                        |                               | L = 30 psf, D.L. = 15 psf |    |      |    | L = 40 psf, D.L. = 15 psf |    |      |    | L = 60 psf, D.L. = 15 psf |    |      |    |   |
|                        |                               | JOIST SPACING (in)        |    |      |    | JOIST SPACING (in)        |    |      |    | JOIST SPACING (in)        |    |      |    |   |
|                        |                               | 12                        | 16 | 19/2 | 24 | 12                        | 16 | 19/2 | 24 | 12                        | 16 | 19/2 | 24 |   |
| 9-1/2"                 | 26                            | N                         | N  | N    | 1  | 2                         | N  | 1    | 2  | X                         | N  | 2    | X  | X |
|                        | 28                            | N                         | N  | N    | 1  | X                         | N  | 1    | 2  | X                         | N  | 2    | X  | X |
|                        | 30                            | N                         | N  | 1    | 1  | X                         | N  | 1    | 2  | X                         | 1  | 2    | X  | X |
|                        | 32                            | N                         | N  | 1    | 2  | X                         | N  | 2    | X  | X                         | 1  | X    | X  | X |
|                        | 34                            | N                         | N  | 1    | 2  | X                         | N  | 2    | X  | X                         | 1  | X    | X  | X |
| 11-7/8"                | 36                            | N                         | N  | 1    | 2  | X                         | 1  | 2    | X  | X                         | 1  | X    | X  | X |
|                        | 26                            | N                         | N  | N    | N  | 1                         | N  | N    | 1  | 2                         | N  | N    | 1  | 2 |
|                        | 28                            | N                         | N  | N    | 1  | N                         | N  | N    | 1  | 2                         | N  | 1    | 1  | X |
|                        | 30                            | N                         | N  | N    | 1  | N                         | N  | N    | 1  | 2                         | N  | 1    | 2  | X |
|                        | 32                            | N                         | N  | N    | 1  | 1                         | N  | N    | N  | 1                         | 2  | N    | 1  | 2 |
| 14"                    | 34                            | N                         | N  | N    | 2  | 2                         | N  | 1    | 1  | X                         | 1  | 1    | 2  | X |
|                        | 36                            | N                         | N  | N    | 1  | 2                         | N  | 1    | 2  | X                         | N  | 1    | 2  | X |
|                        | 38                            | N                         | N  | N    | 1  | 2                         | N  | 1    | 2  | X                         | N  | 2    | X  | X |
|                        | 26                            | N                         | N  | N    | N  | N                         | N  | N    | N  | 1                         | N  | N    | N  | 1 |
|                        | 28                            | N                         | N  | N    | N  | N                         | N  | N    | N  | 1                         | N  | N    | N  | 1 |
| 16"                    | 30                            | N                         | N  | N    | N  | N                         | N  | N    | N  | 1                         | N  | N    | N  | 1 |
|                        | 32                            | N                         | N  | N    | N  | 1                         | N  | N    | N  | 1                         | N  | N    | N  | 1 |
|                        | 34                            | N                         | N  | N    | N  | 1                         | N  | N    | N  | 1                         | N  | N    | N  | 1 |
|                        | 36                            | N                         | N  | N    | N  | 1                         | N  | N    | N  | 1                         | 2  | N    | 1  | 2 |
|                        | 38                            | N                         | N  | N    | N  | 1                         | N  | N    | N  | 1                         | 2  | N    | 1  | 2 |
| 16"                    | 40                            | N                         | N  | N    | N  | 1                         | N  | N    | N  | 1                         | 2  | N    | 1  | 2 |
|                        | 26                            | N                         | N  | N    | N  | N                         | N  | N    | N  | 1                         | N  | N    | N  | 1 |
|                        | 28                            | N                         | N  | N    | N  | N                         | N  | N    | N  | 1                         | N  | N    | N  | 1 |
|                        | 30                            | N                         | N  | N    | N  | N                         | N  | N    | N  | 1                         | N  | N    | N  | 1 |
|                        | 32                            | N                         | N  | N    | N  | N                         | N  | N    | N  | 1                         | N  | N    | N  | 1 |
| 16"                    | 34                            | N                         | N  | N    | N  | N                         | N  | N    | N  | 1                         | N  | N    | N  | 1 |
|                        | 36                            | N                         | N  | N    | N  | 1                         | N  | N    | N  | 1                         | N  | N    | N  | 1 |
|                        | 38                            | N                         | N  | N    | N  | 1                         | N  | N    | N  | 1                         | N  | N    | N  | 1 |
|                        | 40                            | N                         | N  | N    | N  | 1                         | N  | N    | N  | 1                         | 2  | N    | 1  | 2 |
|                        | 42                            | N                         | N  | N    | N  | 1                         | N  | N    | N  | 1                         | 2  | N    | 1  | 2 |

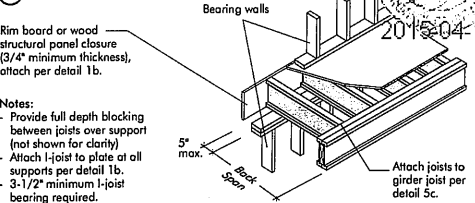
- N = No reinforcement required.
- 1 = NI reinforced with 3/4\"/>

## BRICK CANTILEVER DETAILS FOR VERTICAL BUILDING OFFSET (CONCENTRATED WALL LOAD)

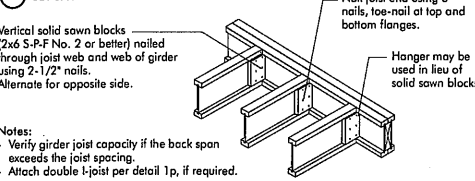
### 5a) SHEATHING REINFORCEMENT



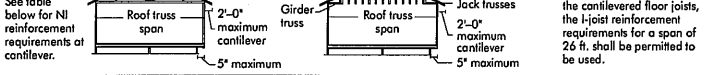
### 5b) SET-BACK DETAIL



### 5c) SET-BACK CONNECTION



### FIGURE 5 (continued)



### BRICK CANTILEVER REINFORCEMENT METHODS ALLOWED

| JOIST DEPTH<br>(in) | ROOF TRUSS SPAN<br>(ft) | ROOFLOADING (UNFACED)     |    |      |    |                           |    |      |    |                           |    |      |    |
|---------------------|-------------------------|---------------------------|----|------|----|---------------------------|----|------|----|---------------------------|----|------|----|
|                     |                         | L = 30 psf, D.L. = 15 psf |    |      |    | L = 40 psf, D.L. = 15 psf |    |      |    | L = 60 psf, D.L. = 15 psf |    |      |    |
|                     |                         | JOIST SPACING (in)        |    |      |    | JOIST SPACING (in)        |    |      |    | JOIST SPACING (in)        |    |      |    |
|                     |                         | 12                        | 16 | 19/2 | 24 | 12                        | 16 | 19/2 | 24 | 12                        | 16 | 19/2 | 24 |
| 9-1/2"              | 26                      | N                         | N  | N    | 1  | N                         | N  | N    | 1  | N                         | N  | N    | 1  |
|                     | 28                      | N                         | N  | N    | 1  | N                         | N  | N    | 1  | N                         | N  | N    | 1  |
|                     | 30                      | N                         | N  | N    | 1  | N                         | N  | N    | 1  | N                         | N  | N    | 1  |
|                     | 32                      | N                         | N  | N    | 1  | N                         | N  | N    | 1  | N                         | N  | N    | 1  |
|                     | 34                      | N                         | N  | N    | 1  | N                         | N  | N    | 1  | N                         | N  | N    | 1  |
| 11-7/8"             | 26                      | N                         | N  | N    | 1  | N                         | N  | N    | 1  | N                         | N  | N    | 1  |
|                     | 28                      | N                         | N  | N    | 1  | N                         | N  | N    | 1  | N                         | N  | N    | 1  |
|                     | 30                      | N                         | N  | N    | 1  | N                         | N  | N    | 1  | N                         | N  | N    | 1  |
|                     | 32                      | N                         | N  | N    | 1  | N                         | N  | N    | 1  | N                         | N  | N    | 1  |
|                     | 34                      | N                         | N  | N    | 1  | N                         | N  | N    | 1  | N                         | N  | N    | 1  |
| 14"                 | 26                      | N                         | N  | N    | 1  | N                         | N  | N    | 1  | N                         | N  | N    | 1  |
|                     | 28                      | N                         | N  | N    | 1  | N                         | N  | N    | 1  | N                         | N  | N    | 1  |
|                     | 30                      | N                         | N  | N    | 1  | N                         | N  | N    | 1  | N                         | N  | N    | 1  |
|                     | 32                      | N                         | N  | N    | 1  | N                         | N  | N    | 1  | N                         | N  | N    | 1  |
|                     | 34                      | N                         | N  | N    | 1  | N                         | N  | N    | 1  | N                         | N  | N    | 1  |
| 16"                 | 26                      | N                         | N  | N    | 1  | N                         | N  | N    | 1  | N                         | N  | N    | 1  |
|                     | 28                      | N                         | N  | N    | 1  | N                         | N  | N    | 1  | N                         | N  | N    | 1  |
|                     | 30                      | N                         | N  | N    | 1  | N                         | N  | N    | 1  | N                         | N  | N    | 1  |
|                     | 32                      | N                         | N  | N    | 1  | N                         | N  | N    | 1  | N                         | N  | N    | 1  |
|                     | 34                      | N                         | N  | N    | 1  | N                         | N  | N    | 1  | N                         | N  | N    | 1  |

- N = No reinforcement required.
- 1 = NI reinforced with 3/4\"/>

## WEB HOLES

### RULES FOR CUTTING HOLES AND DUCT CHASE OPENINGS:

- The distance between the inside edge of the support and the centreline of any hole or duct chase opening shall be in compliance with the requirements of Table 1 or 2, respectively.
- The top and bottom flanges must NEVER be cut, notched, or otherwise modified.
- Whenever possible, field-cut holes should be centred on the middle of the web.
- The maximum size hole or the maximum depth of a duct chase opening that can be cut into an I-joist web shall equal the clear distance between the flanges of the I-joist minus 1/4 inch. A minimum of 1/8 inch should always be maintained between the top or bottom of the hole or opening and the adjacent I-joist flange.
- The sides of square holes or longest sides of rectangular holes should not exceed 3/4 of the diameter of the maximum round hole permitted at that location.
- Where more than one hole is necessary, the distance between adjacent hole edges shall exceed twice the diameter of the largest round hole or twice the size of the largest square hole (or twice the length of the longest side of the longest rectangular hole or duct chase opening) and each hole and duct chase opening shall be sized and located in compliance with the requirements of Tables 1 and 2, respectively.
- A knockout is NOT considered a hole, may be utilized anywhere it occurs, and may be ignored for purposes of calculating minimum distances between holes and/or duct chase openings.
- Holes measuring 1-1/2 inches or smaller shall be permitted anywhere in a cantilevered section of a joist. Holes of greater size may be permitted subject to verification.
- A 1-1/2 inch hole or smaller can be placed anywhere in the web provided that it meets the requirements of rule number 6 above.
- All holes and duct chase openings shall be cut in a workman-like manner in accordance with the restrictions listed above and as illustrated in Figure 7.
- Limit three maximum size holes per span, of which one may be a duct chase opening.
- A group of round holes at approximately the same location shall be permitted if they meet the requirements for a single round hole circumscribed around them.

TABLE 1  
LOCATION OF CIRCULAR HOLES IN JOIST WEBS  
Simple or Multiple Span for Dead Loads up to 15 psf and Live Loads up to 40 psf

| Joist Depth (in) | Joist Spacing (in) | Minimum distance from face of any support to centre of hole (in) |      |       |      |      |      |     |     |     |     |     |     |
|------------------|--------------------|------------------------------------------------------------------|------|-------|------|------|------|-----|-----|-----|-----|-----|-----|
|                  |                    | 0                                                                | 10   | 12    | 16   | 18   | 20   | 22  | 24  | 26  | 28  | 30  | 32  |
| 9-1/2"           | NI-20              | 0-7"                                                             | 1-6" | 2-10" | 4-3" | 5-8" | 6-0" | ... | ... | ... | ... | ... | ... |
|                  | NI-40              | 0-7"                                                             | 1-6" | 2-10" | 4-3" | 5-8" | 6-0" | ... | ... | ... | ... | ... | ... |
|                  | NI-60              | 0-7"                                                             | 1-6" | 2-10" | 4-3" | 5-8" | 6-0" | ... | ... | ... | ... | ... | ... |
|                  | NI-80              | 0-7"                                                             | 1-6" | 2-10" | 4-3" | 5-8" | 6-0" | ... | ... | ... | ... | ... | ... |
|                  | NI-100             | 0-7"                                                             | 1-6" | 2-10" | 4-3" | 5-8" | 6-0" | ... | ... | ... | ... | ... | ... |
| 11-7/8"          | NI-20              | 0-7"                                                             | 1-6" | 2-10" | 4-3" | 5-8" | 6-0" | ... | ... | ... | ... | ... | ... |
|                  | NI-40              | 0-7"                                                             | 1-6" | 2-10" | 4-3" | 5-8" | 6-0" | ... | ... | ... | ... | ... | ... |
|                  | NI-60              | 0-7"                                                             | 1-6" | 2-10" | 4-3" | 5-8" | 6-0" | ... | ... | ... | ... | ... | ... |
|                  | NI-80              | 0-7"                                                             | 1-6" | 2-10" | 4-3" | 5-8" | 6-0" | ... | ... | ... | ... | ... | ... |
|                  | NI-100             | 0-7"                                                             | 1-6" | 2-10" | 4-3" | 5-8" | 6-0" | ... | ... | ... | ... | ... | ... |
| 14"              | NI-20              | 0-7"                                                             | 1-6" | 2-10" | 4-3" | 5-8" | 6-0" | ... | ... | ... | ... | ... | ... |
|                  | NI-40              | 0-7"                                                             | 1-6" | 2-10" | 4-3" | 5-8" | 6-0" | ... | ... | ... | ... | ... | ... |
|                  | NI-60              | 0-7"                                                             | 1-6" | 2-10" | 4-3" | 5-8" | 6-0" | ... | ... | ... | ... | ... | ... |
|                  | NI-80              | 0-7"                                                             | 1-6" | 2-10" | 4-3" | 5-8" | 6-0" | ... | ... | ... | ... | ... | ... |
|                  | NI-100             | 0-7"                                                             | 1-6" | 2-10" | 4-3" | 5-8" | 6-0" | ... | ... | ... | ... | ... | ... |
| 16"              | NI-20              | 0-7"                                                             | 1-6" | 2-10" | 4-3" | 5-8" | 6-0" | ... | ... | ... | ... | ... | ... |
|                  | NI-40              | 0-7"                                                             | 1-6" | 2-10" | 4-3" | 5-8" | 6-0" | ... | ... | ... | ... | ... | ... |
|                  | NI-60              | 0-7"                                                             | 1-6" | 2-10" | 4-3" | 5-8" | 6-0" | ... | ... | ... | ... | ... | ... |
|                  | NI-80              | 0-7"                                                             | 1-6" | 2-10" | 4-3" | 5-8" | 6-0" | ... | ... | ... | ... | ... | ... |
|                  | NI-100             | 0-7"                                                             | 1-6" | 2-10" | 4-3" | 5-8" | 6-0" | ... | ... | ... | ... | ... | ... |

- Above table may be used for I-joist spacing of 24 inches on centre or less.
- Hole location distance is measured from inside face of supports to centre of hole.
- Distances in this chart are based on uniformly loaded joists.

### OPTIONAL:

The above table is based on the I-joists used at their maximum span. If the I-joists are placed at less than their full maximum span (see Maximum Span Table), the minimum distance from the centreline of the hole to the face of any support (D) as given above may be reduced as follows:

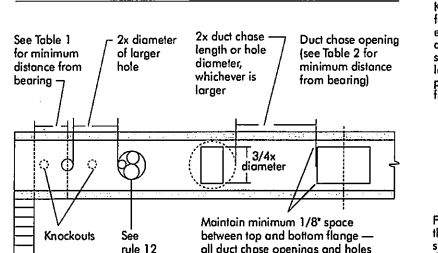
Reduced =  $\frac{\text{Actual } x \text{ D}}{\text{SAF}}$

Where:

- Actual = Distance from the inside face of any support to centre of hole, reduced for less-than-maximum span application (ft). The reduced distance shall not be less than 6 inches from the face of the support to edge of the hole.
- SAF = The actual measured span distance between the inside faces of supports (ft).
- D = The span adjustment factor given in this table.
- Minimum distance from the inside face of any support to centre of hole from this table.

If Actual is greater than 1, use 1 in the above calculation for Actual.

### FIGURE 7 FIELD-CUT HOLE LOCATOR



A knockout is NOT considered a hole, may be utilized wherever it occurs and may be ignored for purposes of calculating minimum distances between holes.

Knockouts are predrilled holes provided for the contractor's convenience to install electrical or small plumbing lines. They are 1-1/2 inches in diameter, and are spaced 15 inches on centre along the length of the I-joist. Where possible, it is preferable to use knockouts instead of field-cut holes.

Never drill, cut or notch the flange, or over-cut the web.

Holes in webs should be cut with a sharp saw.

For rectangular holes, avoid over-cutting the corners, as this can cause unnecessary stress concentrations. Slightly rounding the corners is recommended. Starting the rectangular hole by drilling a 1-inch diameter hole in each of the four corners and then making the cuts between the holes is another good method to minimize damage to the I-joist.

## INSTALLING THE GLUED FLOOR SYSTEM

- Wipe any mud, dirt, water, or ice from I-joist flanges before gluing.
- Snap a chalk line across the I-joists four feet in from the wall for panel edge alignment and as a boundary for spreading glue.
- Spread only enough glue to lay one or two panels at a time, or follow specific recommendations from the glue manufacturer.
- Lay the first panel with tongue side to the wall, and nail in place. This protects the tongue of the next panel from damage when topped into place with a block and sledgehammer.
- Apply a continuous line of glue (about 1/4-inch diameter) to the top flange of a single I-joist. Apply glue in a winding pattern on wide areas, such as with double I-joists.
- Apply two lines of glue on I-joists where panel ends butt to assure proper gluing of each end.
- After the first row of panels is in place, spread glue in the groove of one or two panels at a time before laying the next row. Glue line may be continuous or spaced, but avoid squeeze-out by applying a thinner line (1/8 inch) than used on I-joist flanges.
- Top the second row of panels into place, using a block to protect groove edges.
- Stagger end joints in each succeeding row of panels. A 1/8-inch space between all end joints and 1/8-inch at all edges, including T&G edges, is recommended. (Use a spacer tool or an 2-1/2" common nail to assure accurate and consistent spacing.)
- Complete all nailing of each panel before glue sets. Check the manufacturer's recommendations for cure time. (Warm weather accelerates glue setting.) Use 2" ring- or screw-shank nails for panels 3/4-inch thick or less, and 2-1/2" ring- or screw-shank nails for thicker panels. Space nails per the table below. Closer nail spacing may be required by some codes, or for diaphragm construction. The finished deck can be walked on right away and will carry construction loads without damage to the glue bond.

### FASTENERS FOR SHEATHING AND SUBFLOORING<sup>(1)</sup>

| Maximum Joist Spacing (in) | Minimum Panel Thickness (in) | Nail Size and Spacing |                              |         |  | Maximum Spacing (in) |                  |
|----------------------------|------------------------------|-----------------------|------------------------------|---------|--|----------------------|------------------|
|                            |                              | Common Nail Spacing   | Ring/Threaded Nail or Screws | Staples |  | Edges                | Minimum Supports |
| 16                         | 5/8                          | 2"                    | 1-3/4"                       | 2"      |  | 6"                   | 12"              |
| 20                         | 5/8                          | 2"                    | 1-3/4"                       | 2"      |  | 6"                   | 12"              |
| 24                         | 3/4                          | 2"                    | 1-3/4"                       | 2"      |  | 6"                   | 12"              |

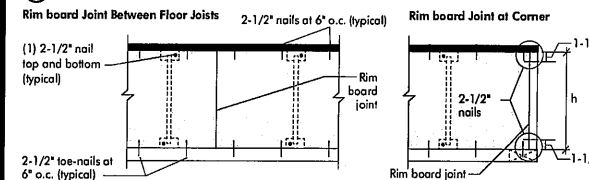
- Fasteners of sheathing and subflooring shall conform to the above table.
- Staples shall not be less than 1/16-inch in diameter or thickness, with not less than a 3/8-inch crown driven with the crown parallel to framing.
- Flooring screws shall not be less than 1/8-inch in diameter.
- Special conditions may impose heavy traffic and concentrated loads that require construction in excess of the minimums shown.
- Use only adhesives conforming to CAN/CSG-71.26 Standard, Adhesives for Field-Gluing Plywood to Lumber Framing for Floor System, applied in accordance with the manufacturer's recommendations. If OSB panels with sealed surfaces and edges are to be used, use only solvent-based glues; check with panel manufacturer.

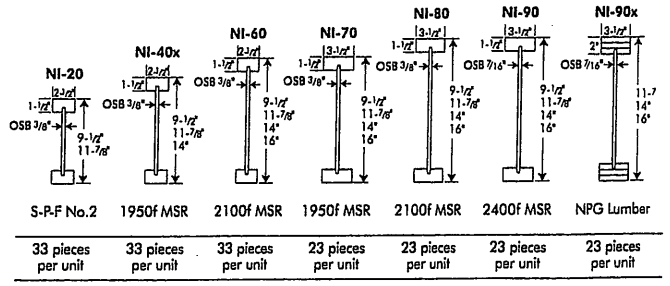
Ref.: NRC-CNRC, National Building Code of Canada 2010, Table 9.23.3.5.

IMPORTANT NOTE:  
Floor sheathing must be field glued to the I-joist flanges in order to achieve the maximum spans shown in this document. If sheathing is nailed only, I-joist spans must be verified with your local distributor.

## RIM BOARD INSTALLATION DETAILS

### 8a) ATTACHMENT DETAILS WHERE RIM BOARDS ABUT





Refer to the *Installation Guide for Residential Floors* for additional information.  
CCMC EVALUATION REPORT 13032-R

### WEB HOLE SPECIFICATIONS

RULES FOR CUTTING HOLES AND DUCT CHASE OPENINGS:

1. The distance between the inside edge of the support and the centreline of any hole or duct chase opening shall be in compliance with the requirements of Table 1 or 2, respectively.

2. I-joist top and bottom flanges must NEVER be cut, notched, or otherwise modified.

3. Whenever possible, field-cut holes should be centred on the middle of the web.

4. The maximum size hole or the maximum depth of a duct chase opening that can be cut into an I-joist web shall equal the clear distance between the flanges of the I-joist minus 1/4 inch. A minimum of 1/8 inch should always be maintained between the top or bottom of the hole or opening and the adjacent I-joist flange.
5. The sides of square holes or longest sides of rectangular holes should not exceed 3/4 of the diameter of the maximum round hole permitted at that location.

6. Where more than one hole is necessary, the distance between adjacent hole edges shall exceed twice the diameter of the largest round hole or twice the size of the largest square hole (or twice the length of the longest side of the longest rectangular hole or duct chase opening) and each hole and duct chase opening shall be sized and located in compliance with the requirements of Tables 1 and 2, respectively.

7. A knockout is not considered a hole, may be utilized anywhere it occurs, and may be ignored for purposes of calculating minimum distances between holes and/or duct chase openings.

8. Holes measuring 1-1/2 inches or smaller are permitted anywhere in a cantilevered section of a joist. Holes of greater size may be permitted subject to verification.
9. A 1-1/2 inch hole or smaller can be placed anywhere in the web provided that it meets the requirements of rule number 6 above.

10. All holes and duct chase openings shall be cut in a workman-like manner in accordance with the restrictions listed above and as illustrated in Figure 7.

11. Limit three maximum size holes per span, of which one may be a duct chase opening.

12. A group of round holes at approximately the same location shall be permitted if they meet the requirements for a single round hole circumscribed around them.

TABLE 1  
LOCATION OF CIRCULAR HOLES IN JOIST WEBS  
Simple or Multiple Span for Dead Loads up to 15 psf and Live Loads up to 40 psf

| Joist Depth | Joist Series | Minimum Distance from Inside Face of Any Support to Centre of Hole (ft - in.) |       |       |      |       |       |      |       |       |      |       |        |
|-------------|--------------|-------------------------------------------------------------------------------|-------|-------|------|-------|-------|------|-------|-------|------|-------|--------|
|             |              | Round Hole Diameter (in.)                                                     |       |       |      |       |       |      |       |       |      |       |        |
|             |              | 2                                                                             | 3     | 4     | 5    | 6     | 6-1/4 | 7    | 8     | 8-5/8 | 9    | 10    | 10-3/4 |
| 9-1/2"      | NI-20        | 0-7"                                                                          | 1-6"  | 2-10" | 4-3" | 5-8"  | 6-0"  | ---  | ---   | ---   | ---  | ---   | ---    |
|             | NI-40x       | 0-7"                                                                          | 1-6"  | 3-0"  | 4-4" | 6-0"  | 6-4"  | ---  | ---   | ---   | ---  | ---   | ---    |
|             | NI-60        | 1-3"                                                                          | 2-6"  | 4-0"  | 5-4" | 7-0"  | 7-5"  | ---  | ---   | ---   | ---  | ---   | ---    |
|             | NI-70        | 2-0"                                                                          | 3-4"  | 4-9"  | 6-3" | 8-0"  | 8-4"  | ---  | ---   | ---   | ---  | ---   | ---    |
|             | NI-80        | 2-3"                                                                          | 3-6"  | 5-0"  | 6-6" | 8-2"  | 8-8"  | ---  | ---   | ---   | ---  | ---   | ---    |
| 11-7/8"     | NI-20        | 0-7"                                                                          | 0-8"  | 1-0"  | 2-4" | 3-8"  | 4-0"  | 5-0" | 6-6"  | 7-9"  | ---  | ---   | ---    |
|             | NI-40x       | 0-7"                                                                          | 0-8"  | 1-3"  | 2-8" | 4-0"  | 4-4"  | 5-5" | 7-0"  | 8-4"  | ---  | ---   | ---    |
|             | NI-60        | 0-7"                                                                          | 1-8"  | 3-0"  | 4-3" | 5-9"  | 6-0"  | 7-3" | 8-10" | 10-0" | ---  | ---   | ---    |
|             | NI-70        | 1-3"                                                                          | 2-6"  | 4-0"  | 5-4" | 6-9"  | 7-2"  | 8-4" | 10-0" | 11-2" | ---  | ---   | ---    |
|             | NI-80        | 1-6"                                                                          | 2-10" | 4-2"  | 5-6" | 7-0"  | 7-5"  | 8-6" | 10-3" | 11-4" | ---  | ---   | ---    |
| 14"         | NI-20        | 0-7"                                                                          | 0-8"  | 1-5"  | 3-2" | 4-10" | 5-4"  | 6-9" | 8-9"  | 10-2" | ---  | ---   | ---    |
|             | NI-40x       | 0-7"                                                                          | 0-8"  | 0-9"  | 2-5" | 4-4"  | 4-9"  | 6-3" | ---   | ---   | ---  | ---   | ---    |
|             | NI-60        | 0-7"                                                                          | 0-8"  | 0-8"  | 1-0" | 2-4"  | 2-9"  | 3-9" | 5-2"  | 6-0"  | 6-6" | 8-3"  | 10-2"  |
|             | NI-70        | 0-7"                                                                          | 0-8"  | 1-8"  | 3-0" | 4-3"  | 4-8"  | 5-8" | 7-2"  | 8-0"  | 8-8" | 10-4" | 11-9"  |
|             | NI-80        | 0-7"                                                                          | 0-8"  | 1-10" | 3-0" | 4-5"  | 5-10" | 6-2" | 7-3"  | 8-9"  | 9-9" | 10-4" | 12-0"  |
| 16"         | NI-20        | 0-7"                                                                          | 0-8"  | 0-8"  | 1-6" | 2-10" | 3-2"  | 4-2" | 5-6"  | 6-4"  | 7-0" | 8-5"  | 9-8"   |
|             | NI-40x       | 0-7"                                                                          | 0-8"  | 0-8"  | 1-6" | 2-10" | 3-2"  | 4-2" | 5-6"  | 6-4"  | 7-0" | 8-5"  | 9-8"   |
|             | NI-60        | 0-7"                                                                          | 0-8"  | 1-8"  | 3-0" | 4-3"  | 4-8"  | 5-8" | 7-2"  | 8-0"  | 8-8" | 10-4" | 11-9"  |
|             | NI-70        | 0-7"                                                                          | 0-8"  | 1-10" | 3-0" | 4-5"  | 5-10" | 6-2" | 7-3"  | 8-9"  | 9-9" | 10-4" | 12-0"  |
|             | NI-80        | 0-7"                                                                          | 0-8"  | 1-10" | 3-0" | 4-5"  | 5-10" | 6-2" | 7-3"  | 8-9"  | 9-9" | 10-4" | 12-0"  |

1. Above table may be used for I-joist spacing of 24 inches on centre or less.

2. Hole location distance is measured from inside face of supports to centre of hole.

3. Distances in this chart are based on uniformly loaded joists.

4. The above table is based on the I-joists being used at their maximum spans. The minimum distance as given above may be reduced for shorter spans; contact your local distributor.
1. Above table may be used for I-joist spacing of 24 inches on centre or less.

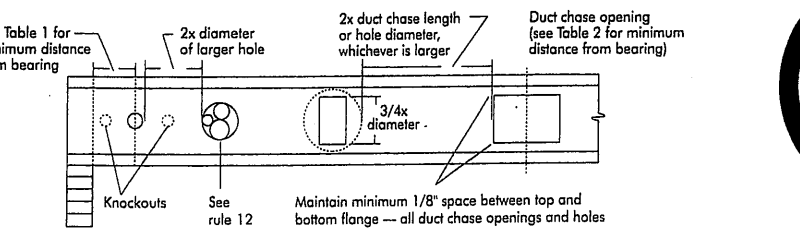
2. Above table opening location distance is measured from inside face of supports to centre of opening.

3. The above table is based on simple-span joists only. For other applications, contact your local distributor.

4. Distances are based on uniformly loaded floor joists that meet the span requirements for a design live load of 40 psf and dead load of 15 psf, and a live load deflection limit of L/480.

5. The above table is based on the I-joists being used at their maximum spans. The minimum distance as given above may be reduced for shorter spans; contact your local distributor.

FIGURE 7  
FIELD-CUT HOLE LOCATOR



### SAFETY AND CONSTRUCTION PRECAUTIONS

- WARNING:** I-joists are not stable until completely installed, and will not carry any load until fully braced and sheathed.

**AVOID ACCIDENTS BY FOLLOWING THESE IMPORTANT GUIDELINES:**

  - Brace and nail each I-joist as it is installed, using hangers, blocking panels, rim board, and/or cross-bridging at joist ends. When I-joists are applied continuously over interior supports and a load-bearing wall is planned at that location, blocking will be required at the interior support.
  - When the building is completed, the floor sheathing will provide lateral support for the top flanges of the I-joists. Until this sheathing is applied, temporary bracing, often called struts, or temporary sheathing must be applied to prevent I-joist rollover or buckling.
    - Temporary bracing or struts must be 1x4 inch minimum, at least 8 feet long and spaced no more than 8 feet on centre, and must be secured with a minimum of two 2-1/2" nails fastened to the top surface of each I-joist. Nail the bracing to a lateral restraint at the end of each bay. Lap ends of adjoining bracing over at least two I-joists.
    - Or, sheathing (temporary or permanent) can be nailed to the top flange of the first 4 feet of I-joists at the end of the bay.
  - For cantilevered I-joists, brace top and bottom flanges, and brace ends with closure panels, rim board, or cross-bridging.
  - Install and fully nail permanent sheathing to each I-joist before placing loads on the floor system. Then, stack building materials over beams or walls only.
  - Never install a damaged I-joist.

Improper storage or installation, failure to follow applicable building codes, failure to follow span ratings for Nordic I-joists, failure to follow allowable hole sizes and locations, or failure to use web stiffeners when required can result in serious accidents. Follow these installation guidelines carefully.
- Do not walk on I-joists until fully fastened and braced, or serious injuries can result.

Never stack building materials over unsheathed I-joists. Once sheathed, do not over-stress I-joists with concentrated loads from building materials.

Knockouts are prescored holes provided for the contractor's convenience to install electrical or small plumbing lines. They are 1-1/2 inches in diameter, and are spaced 15 inches on centre along the length of the I-joist. Where possible, it is preferable to use knockouts instead of field-cut holes.

Never drill, cut or notch the flange, or over-cut the web.

Holes in webs should be cut with a sharp saw.

For rectangular holes, avoid over-cutting the corners, as this can cause unnecessary stress concentrations. Slightly rounding the corners is recommended. Starting the rectangular hole by drilling a 1-inch diameter hole in each of the four corners and then making the cuts between the holes is another good method to minimize damage to the I-joist.

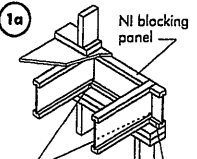


## PRODUCT WARRANTY

Chantiers Chibougamau guarantees that, in accordance with our specifications, Nordic products are free from manufacturing defects in material and workmanship.

Furthermore, Chantiers Chibougamau warrants that our products, when utilized in accordance with our handling and installation instructions, will meet or exceed our specifications for the lifetime of the structure.

**1a**



Blocking Panel or Rim Joist

Maximum Factored Uniform Vertical Load\* (plf)

NI Joists

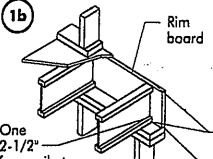
3,300

\*The uniform vertical load is limited to a joist depth of 16 inches or less and is based on standard term load duration. It shall not be used in the design of a bending member, such as joist, header, or rafter. For concentrated vertical load transfer, see detail 1d.

2-1/2" nails at 6" o.c. to top plate (when used for lateral shear transfer, nail to bearing plate with same nailing as required for decking)

Attach I-joist to top plate per detail 1b

**1b**



Rim board

One 2-1/2" face nail at each side at bearing

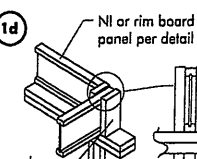
One 2-1/2" wire or spiral nail at top and bottom flange

Attach rim board to top plate using 2-1/2" wire or spiral toe-nails at 6" o.c.

To avoid splitting flange, start nails at least 1-1/2" from end of I-joist. Nails may be driven at an angle to avoid splitting of bearing plate.

Minimum bearing length shall be 1-3/4" for the end bearings, and 3-1/2" for the intermediate bearings when applicable.

**1d**



Pair of Squash Blocks

Maximum Factored Uniform Vertical Load per Pair of Squash Blocks (lbs)

3-1/2" wide

5-1/2" wide

2x Lumber

5,500

8,500

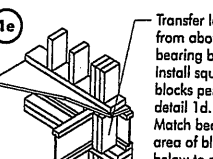
1-1/8" Rim Board Plus

4,300

6,600

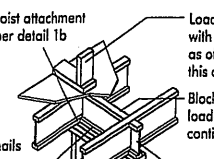
Provide lateral bracing per detail 1a or 1b

**1e**



Transfer load from above to bearing below. Install squash blocks per detail 1d. Match bearing area of blocks below to post above.

**1g**



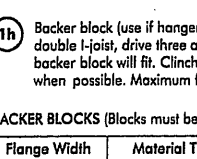
Joist attachment per detail 1b

Load bearing wall above shall align vertically with the bearing below. Other conditions, such as offset bearing walls, are not covered by this detail.

Blocking required over all interior supports under load-bearing walls or when floor joists are not continuous over support

NI blocking panel per detail 1a

**1h**



Backer block (use if hanger load exceeds 360 lbs). Before installing a backer block to a double I-joist, drive three additional 3" nails through the webs and filler block where the backer block will fit. Clinch. Install backer tight to top flange. Use twelve 3" nails, clinched when possible. Maximum factored resistance for hanger for this detail = 1,620 lbs.

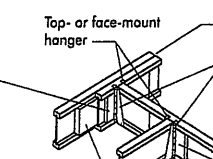
**BACKER BLOCKS** (Blocks must be long enough to permit required nailing without splitting)

| Flange Width | Material Thickness Required* | Minimum Depth** |
|--------------|------------------------------|-----------------|
| 2-1/2"       | 1"                           | 5-1/2"          |
| 3-1/2"       | 1-1/2"                       | 7-1/4"          |

\* Minimum grade for backer block material shall be S-RF No. 2 or better for solid sawn lumber and wood structural panels conforming to CAN/CSA-O325 or CAN/CSA-O437 Standard.

\*\* For face-mount hangers use net joist depth minus 3-1/4" for joists with 1-1/2" thick flanges. For 2" thick flanges use net depth minus 4-1/4".

**1i**



Top- or face-mount hanger

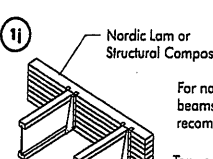
Double I-joist header

NOTE: Unless hanger sides laterally support the top flange, bearing stiffeners shall be used.

Backer block required (both sides for face-mount hangers)

For hanger capacity see hanger manufacturer's recommendations. Verify double I-joist capacity to support concentrated loads.

**1j**



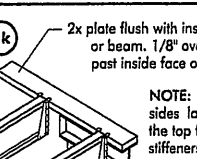
Nordic Lam or Structural Composite Lumber (SCL)

For nailing schedules for multiple beams, see the manufacturer's recommendations.

Top- or face-mount hanger installed per manufacturer's recommendations

NOTE: Unless hanger sides laterally support the top flange, bearing stiffeners shall be used.

**1k**

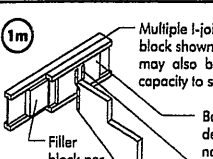


2x plate flush with inside face of wall or beam. 1/8" overhang allowed past inside face of wall or beam.

NOTE: Unless hanger sides laterally support the top flange, bearing stiffeners shall be used.

Top-mount hanger installed per manufacturer's recommendations

**1m**



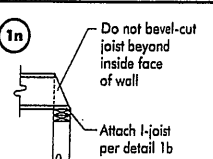
Multiple I-joist header with full depth filler block shown. Nordic Lam or SCL headers may also be used. Verify double I-joist capacity to support concentrated loads.

Backer block attached per detail 1h. Nail with twelve 3" nails, clinch when possible.

Install hanger per manufacturer's recommendations

Maximum support capacity = 1,620 lbs.

**1n**

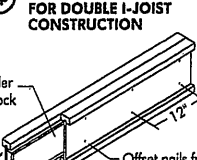


Do not bevel-cut joist beyond inside face of wall.

Attach I-joist per detail 1b

NOTE: Blocking required at bearing for lateral support, not shown for clarity.

**1p**



Filler block

Offset nails from opposite face by 6"

1/8" to 1/4" gap between top flange and filler block

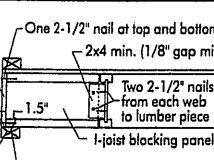
**1p FILLER BLOCK REQUIREMENTS FOR DOUBLE I-JOIST CONSTRUCTION**

NOTES:

- Support back of I-joist web during nailing to prevent damage to web/flange connection.
- Leave a 1/8 to 1/4-inch gap between top of filler block and bottom of top I-joist flange.
- Filler block is required between joists for full length of span.
- Nail joists together with two rows of 3" nails at 12 inches o.c. (clinched when possible) on each side of the double I-joist. Total of four nails per foot required. If nails can be clinched, only two nails per foot are required.
- The maximum factored load that may be applied to one side of the double joist using this detail is 860 lb/ft. Verify double I-joist capacity.

| Flange Size     | Net Depth | Filler Block Size |
|-----------------|-----------|-------------------|
| 2-1/2" x 1-1/2" | 9-1/2"    | 2-1/8" x 6"       |
|                 | 11-7/8"   | 2-1/8" x 8"       |
|                 | 14"       | 2-1/8" x 10"      |
|                 | 16"       | 2-1/8" x 12"      |
| 3-1/2" x 1-1/2" | 9-1/2"    | 3" x 6"           |
|                 | 11-7/8"   | 3" x 8"           |
|                 | 14"       | 3" x 10"          |
|                 | 16"       | 3" x 12"          |
| 3-1/2" x 2"     | 11-7/8"   | 3" x 7"           |
|                 | 14"       | 3" x 9"           |
|                 | 16"       | 3" x 11"          |

**1s**



One 2-1/2" nail at top and bottom flange

2x4 min. (1/8" gap minimum)

Two 2-1/2" nails from each web to lumber piece

1-joist blocking panel

One 2-1/2" nail one side only

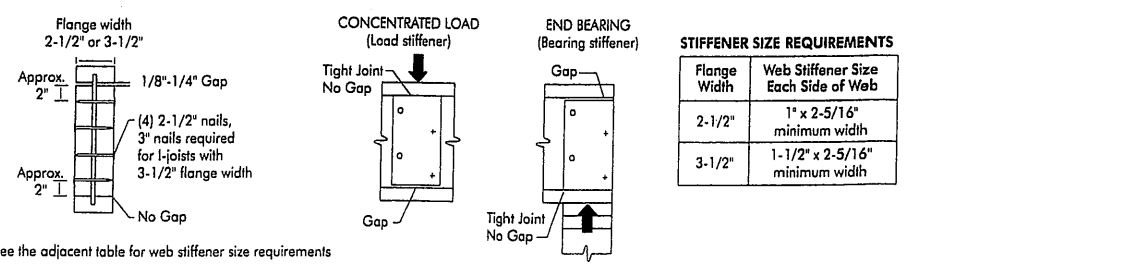
NOTE: In some local codes, blocking is prescriptively required in the first joist space (or first and second joist space) next to the starter joist. Where required, see local code requirements for spacing of the blocking. All nails are common spiral in this detail.

All nails shown in the above details are assumed to be common wire nails unless otherwise noted. 3" (0.122" dia.) common spiral nails may be substituted for 2-1/2" (0.128" dia.) common wire nails. Framing lumber assumed to be Spruce-Pine-Fir No. 2 or better. Individual components not shown to scale for clarity.

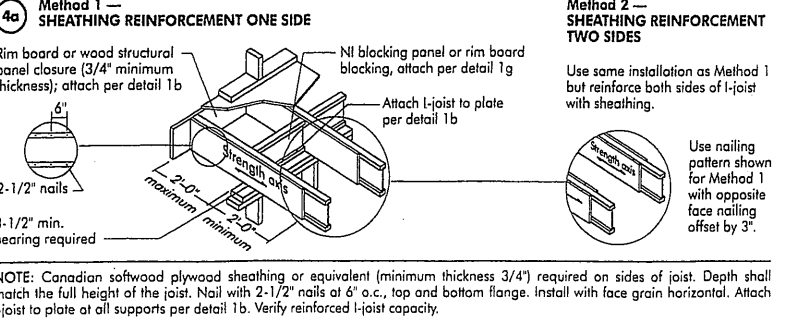
### WEB STIFFENERS

- RECOMMENDATIONS:**
- A **bearing stiffener** is required in all engineered applications with factored reactions greater than shown in the I-joist properties table found of the I-joist Construction Guide (C101). The gap between the stiffener and the flange is at the top.
  - A **bearing stiffener** is required when the I-joist is supported in a hanger and the sides of the hanger do not extend up to, and support, the top flange. The gap between the stiffener and flange is at the top.
  - A **load stiffener** is required at locations where a factored concentrated load greater than 2,370 lbs is applied to the top flange between supports, or in the case of a cantilever, anywhere between the cantilever tip and the support. These values are for standard term load duration, and may be adjusted for other load durations as permitted by the code. The gap between the stiffener and the flange is at the bottom.

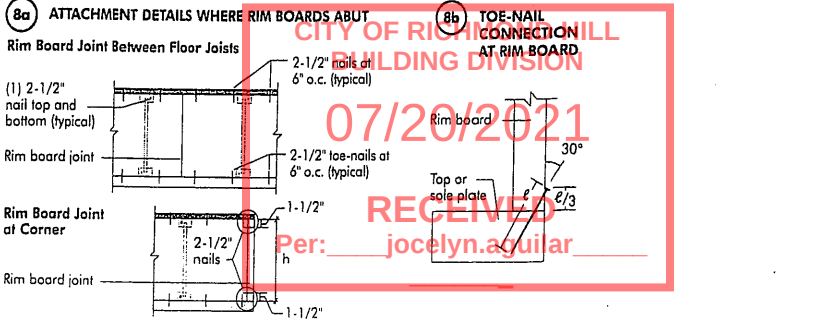
FIGURE 2  
WEB STIFFENER INSTALLATION DETAILS



### CANTILEVER DETAILS FOR VERTICAL BUILDING OFFSET



### RIM BOARD INSTALLATION DETAILS



# NORDIC STRUCTURES

**COMPANY**  
July 22, 2020 16:57

**PROJECT**  
J6 1ST FLOOR.wwb

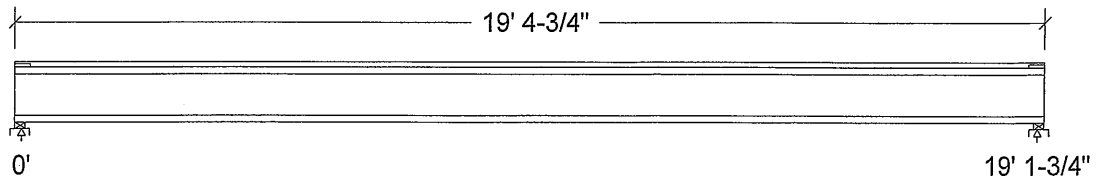
## Design Check Calculation Sheet

Nordic Sizer – Canada 7.2

### Loads:

| Load  | Type | Distribution | Pat-<br>tern | Location [ft]<br>Start End | Magnitude<br>Start End | Unit |
|-------|------|--------------|--------------|----------------------------|------------------------|------|
| Load1 | Dead | Full Area    |              |                            | 20.00                  | psf  |
| Load2 | Live | Full Area    |              |                            | 40.00                  | psf  |

### Maximum Reactions (lbs) and Support Bearing (in):



|             |       |  |       |
|-------------|-------|--|-------|
| Unfactored: |       |  |       |
| Dead        | 191   |  | 191   |
| Live        | 383   |  | 383   |
| Factored:   |       |  |       |
| Total       | 814   |  | 814   |
| Bearing:    |       |  |       |
| Capacity    |       |  |       |
| Joist       | 2188  |  | 2188  |
| Support     | 5573  |  | 5573  |
| Des ratio   |       |  |       |
| Joist       | 0.37  |  | 0.37  |
| Support     | 0.15  |  | 0.15  |
| Load case   | #2    |  | #2    |
| Length      | 2-3/8 |  | 2-3/8 |
| Min req'd   | 1-3/4 |  | 1-3/4 |
| Stiffener   | No    |  | No    |
| KD          | 1.00  |  | 1.00  |
| KB support  | 1.00  |  | 1.00  |
| fcp sup     | 769   |  | 769   |
| Kzcp sup    | 1.09  |  | 1.09  |

### Nordic 11-7/8" NI-80 Floor joist @ 12" o.c.

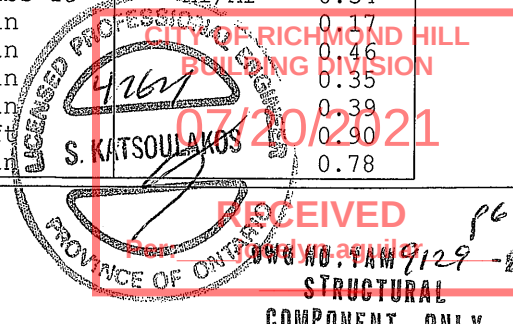
Supports: All - Lumber Sill plate, No.1/No.2

Total length: 19' 4-3/4"; Clear span: 19'; 3/4" nailed and glued OSB sheathing

**This section PASSES the design code check.**

### Limit States Design using CSA O86-14 and Vibration Criterion:

| Criterion    | Analysis Value | Design Value | Unit   | Analysis/Design |
|--------------|----------------|--------------|--------|-----------------|
| Shear        | Vf = 814       | Vr = 2336    | lbs    | Vf/Vr = 0.35    |
| Moment (+)   | Mf = 3894      | Mr = 11609   | lbs-ft | Mf/Mr = 0.34    |
| Perm. Defl'n | 0.11 = < L/999 | 0.64 = L/360 | in     | 0.17            |
| Live Defl'n  | 0.22 = < L/999 | 0.48 = L/480 | in     | 0.46            |
| Total Defl'n | 0.33 = L/690   | 0.96 = L/240 | in     | 0.35            |
| Bare Defl'n  | 0.25 = L/920   | 0.64 = L/360 | in     | 0.39            |
| Vibration    | Lmax = 19'-1.8 | Lv = 21'-2.7 | ft     | 0.90            |
| Defl'n       | = 0.026        | = 0.033      | in     | 0.78            |



**Additional Data:**

| FACTORS: | f/E           | KD   | KH   | KZ | KL    | KT | KS | KN | LC# |
|----------|---------------|------|------|----|-------|----|----|----|-----|
| Vr       | 2336          | 1.00 | 1.00 | -  | -     | -  | -  | -  | #2  |
| Mr+      | 11609         | 1.00 | 1.00 | -  | 1.000 | -  | -  | -  | #2  |
| EI       | 547.1 million | -    | -    | -  | -     | -  | -  | -  | #2  |

**CRITICAL LOAD COMBINATIONS:**

Shear : LC #2 = 1.25D + 1.5L

Moment(+) : LC #2 = 1.25D + 1.5L

Deflection: LC #1 = 1.0D (permanent)

LC #2 = 1.0D + 1.0L (live)

LC #2 = 1.0D + 1.0L (total)

LC #2 = 1.0D + 1.0L (bare joist)

Bearing : Support 1 - LC #2 = 1.25D + 1.5L

Support 2 - LC #2 = 1.25D + 1.5L

Load Types: D=dead W=wind S=snow H=earth,groundwater E=earthquake

L=live(use,occupancy) Ls=live(storage,equipment) f=fire

Load Patterns: s=S/2 L=L+Ls \_=no pattern load in this span

All Load Combinations (LCs) are listed in the Analysis output

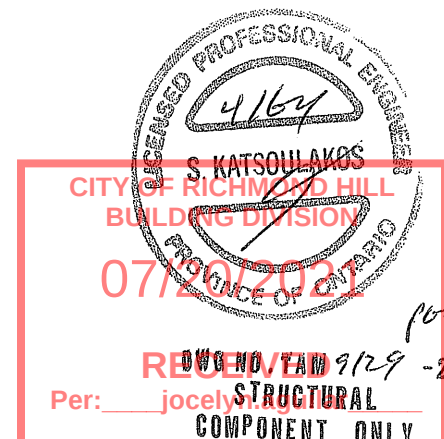
**CALCULATIONS:**E<sub>I</sub>eff = 625.37 lb-in<sup>2</sup> K= 6.18e06 lbs

"Live" deflection is due to all non-dead loads (live, wind, snow...) CONFORMS TO CBC 2012

**Design Notes:**

AMENDED 2020

1. WoodWorks analysis and design are in accordance with the 2015 National Building Code of Canada (NBC), Division B, Part 4, and the CSA O86-14 Engineering Design in Wood standard, Update No. 2 (June 2017).
2. Please verify that the default deflection limits are appropriate for your application.
3. Refer to Nordic Structures technical documentation for installation guidelines and construction details.
4. Nordic I-joists are listed in CCMC evaluation report 13032-R.
5. Joists shall be laterally supported at supports and continuously along the compression edge.
6. The design assumptions and specifications have been provided by the client. Any damages resulting from faulty or incorrect information, specifications, and/or designs furnished, and the correctness or accuracy of this information is their responsibility. This analysis does not constitute a record of the structural integrity of the building nor suitability of the design assumptions made. Nordic Structures is responsible only for the structural adequacy of this component based on the design criteria and loadings shown.



# NORDIC STRUCTURES

COMPANY  
July 22, 2020 08:26

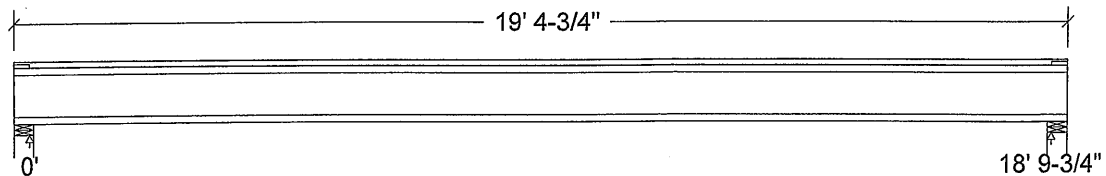
PROJECT  
J5 2ND FLOOR.wwb

## Design Check Calculation Sheet Nordic Sizer – Canada 7.2

### Loads:

| Load  | Type | Distribution | Pat-<br>tern | Location [ft]<br>Start End | Magnitude<br>Start End | Unit |
|-------|------|--------------|--------------|----------------------------|------------------------|------|
| Load1 | Dead | Full Area    |              |                            | 20.00                  | psf  |
| Load2 | Live | Full Area    |              |                            | 40.00                  | psf  |

### Maximum Reactions (lbs) and Support Bearing (in):



|             |       |  |       |
|-------------|-------|--|-------|
| Unfactored: |       |  |       |
| Dead        | 188   |  | 188   |
| Live        | 376   |  | 376   |
| Factored:   |       |  |       |
| Total       | 800   |  | 800   |
| Bearing:    |       |  |       |
| Capacity    |       |  |       |
| Joist       | 2336  |  | 2336  |
| Support     | 10841 |  | 10841 |
| Des ratio   |       |  |       |
| Joist       | 0.34  |  | 0.34  |
| Support     | 0.07  |  | 0.07  |
| Load case   | #2    |  | #2    |
| Length      | 4-3/8 |  | 4-3/8 |
| Min req'd   | 1-3/4 |  | 1-3/4 |
| Stiffener   | No    |  | No    |
| KD          | 1.00  |  | 1.00  |
| KB support  | -     |  | -     |
| fcp sup     | 769   |  | 769   |
| Kzcp sup    | -     |  | -     |

Bearing for wall supports is perpendicular-to-grain bearing on top plate. No stud design included.

### Nordic Joist 11-7/8" NI-80 Floor joist @ 12" o.c.

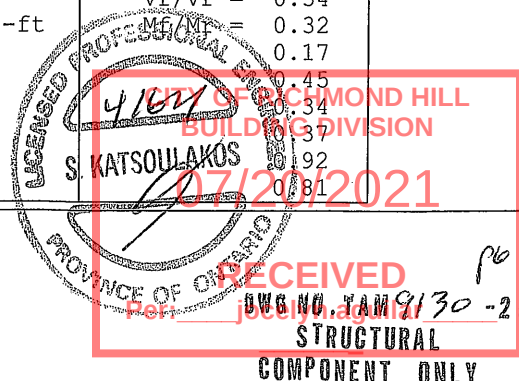
Supports: All - Lumber Wall, No.1/No.2

Total length: 19' 4-3/4"; Clear span: 18' 8"; 5/8" nailed and glued OSB sheathing with 1/2" gypsum ceiling

This section PASSES the design code check.

### Limit States Design using CSA O86-14 and Vibration Criterion:

| Criterion    | Analysis Value | Design Value | Unit   | Analysis/Design |
|--------------|----------------|--------------|--------|-----------------|
| Shear        | Vf = 800       | Vr = 2336    | lbs    | Vf/Vr = 0.34    |
| Moment(+)    | Mf = 3760      | Mr = 11609   | lbs-ft | Mf/Mr = 0.32    |
| Perm. Defl'n | 0.11 = < L/999 | 0.63 = L/360 | in     | 0.17            |
| Live Defl'n  | 0.21 = < L/999 | 0.47 = L/480 | in     | 0.45            |
| Total Defl'n | 0.32 = L/712   | 0.94 = L/240 | in     | 0.34            |
| Bare Defl'n  | 0.23 = L/966   | 0.63 = L/360 | in     | 0.37            |
| Vibration    | Lmax = 18'-9.8 | Lv = 20'-5.8 | ft     | 0.92            |
| Defl'n       | = 0.027        | = 0.033      | in     | 0.81            |



**Additional Data:**

| FACTORS: | f/E           | KD   | KH   | KZ | KL    | KT | KS | KN | LC# |
|----------|---------------|------|------|----|-------|----|----|----|-----|
| Vr       | 2336          | 1.00 | 1.00 | -  | -     | -  | -  | -  | #2  |
| Mr+      | 11609         | 1.00 | 1.00 | -  | 1.000 | -  | -  | -  | #2  |
| EI       | 547.1 million | -    | -    | -  | -     | -  | -  | -  | #2  |

**CRITICAL LOAD COMBINATIONS:**

Shear : LC #2 = 1.25D + 1.5L

Moment(+) : LC #2 = 1.25D + 1.5L

Deflection: LC #1 = 1.0D (permanent)

LC #2 = 1.0D + 1.0L (live)

LC #2 = 1.0D + 1.0L (total)

LC #2 = 1.0D + 1.0L (bare joist)

Bearing : Support 1 - LC #2 = 1.25D + 1.5L

Support 2 - LC #2 = 1.25D + 1.5L

Load Types: D=dead W=wind S=snow H=earth,groundwater E=earthquake  
L=live(use,occupancy) Ls=live(storage,equipment) f=fire

Load Patterns: s=S/2 L=L+Ls \_=no pattern load in this span

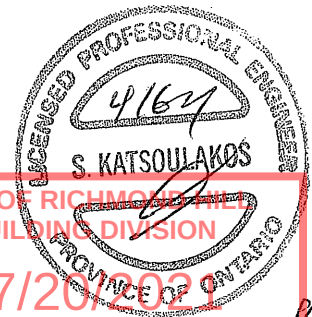
All Load Combinations (LCs) are listed in the Analysis output

**CALCULATIONS:**EI<sub>eff</sub> = 613.27 lb-in<sup>2</sup> K= 6.18e06 lbs

"Live" deflection is due to all non-dead loads (live, wind, snow...)

**CONFORMS TO OBC 2012****Design Notes:****AMENDED 2020**

1. WoodWorks analysis and design are in accordance with the 2015 National Building Code of Canada (NBC), Division B, Part 4, and the CSA O86-14 Engineering Design in Wood standard, Update No. 2 (June 2017).
2. Please verify that the default deflection limits are appropriate for your application.
3. Refer to Nordic Structures technical documentation for installation guidelines and construction details.
4. Nordic I-joists are listed in CCMC evaluation report 13032-R.
5. Joists shall be laterally supported at supports and continuously along the compression edge.
6. The design assumptions and specifications have been provided by the client. Any damages resulting from faulty or incorrect information, specifications, and/or designs furnished, and the correctness or accuracy of this information is their responsibility. This analysis does not constitute a record of the structural integrity of the building nor suitability of the design assumptions made. Nordic Structures is responsible only for the structural adequacy of this component based on the design criteria and loadings shown.

CITY OF RICHMOND HILL  
BUILDING DIVISION

07/20/2021

DWG NO. TAM 9/30-21

**RECEIVED**

Per: jockey COMPONENT ONLY



# Single 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

**PASSED**

## 1ST FLR FRAMING\Flush Beams\B1(i333) (Flush Beam)

Dry | 1 span | No cant.

July 22, 2020 16:20:19

BC CALC® Member Report

Build 7493

Job name:

Address:

City, Province, Postal Code:

Customer:

Code reports:

CCMC 12472-R

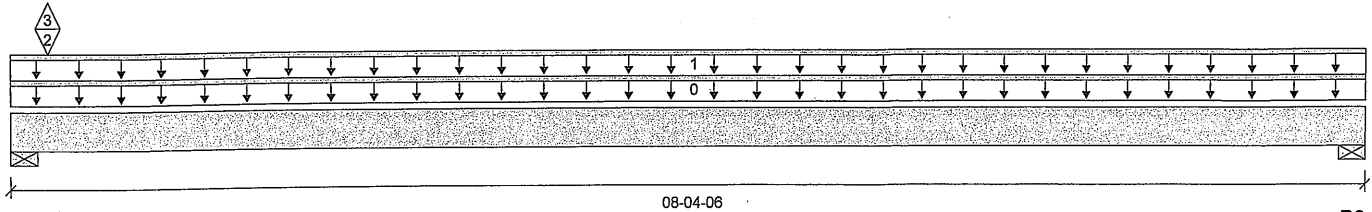
File name: UNIT 2009 EL A,B.mmdl

Description: 1ST FLR FRAMING\Flush Beams\B1(i333)

Specifier:

Designer:

Company:



Total Horizontal Product Length = 08-04-06

### Reaction Summary (Down / Uplift) (lbs)

| Bearing    | Live       | Dead     | Snow | Wind |
|------------|------------|----------|------|------|
| B1, 3-1/2" | 1392 / 240 | 1276 / 0 |      |      |
| B2, 4-3/8" | 112 / 0    | 82 / 0   |      |      |

### Load Summary

| Tag | Description        | Load Type         | Ref. | Start    | End      | Loc. | Live | Dead | Snow | Wind | Tributary |
|-----|--------------------|-------------------|------|----------|----------|------|------|------|------|------|-----------|
| 0   | Self-Weight        | Unf. Lin. (lb/ft) | L    | 00-00-00 | 08-04-06 | Top  | 1.00 | 0.65 | 1.00 | 1.15 | 00-00-00  |
| 1   | FC1 Floor Material | Unf. Lin. (lb/ft) | L    | 00-00-00 | 08-04-06 | Top  | 27   | 13   |      |      | n/a       |
| 2   | E1(i185)           | Conc. Pt. (lbs)   | L    | 00-02-12 | 00-02-12 | Top  | 1281 | 1196 |      |      | n/a       |
| 3   | E1(i185)           | Conc. Pt. (lbs)   | L    | 00-02-12 | 00-02-12 | Top  | -240 |      |      |      | n/a       |

### Controls Summary

|                       | Factored Demand | Factored Resistance | Demand/Resistance | Case | Location |
|-----------------------|-----------------|---------------------|-------------------|------|----------|
| Pos. Moment           | 492 ft-lbs      | 17696 ft-lbs        | 2.8%              | 1    | 04-01-12 |
| End Shear             | 184 lbs         | 7232 lbs            | 2.5%              | 1    | 01-03-06 |
| Total Load Deflection | L/999 (0.008")  | n/a                 | n/a               | 6    | 04-01-12 |
| Live Load Deflection  | L/999 (0.005")  | n/a                 | n/a               | 8    | 04-01-12 |
| Max Defl.             | 0.008"          | n/a                 | n/a               | 6    | 04-01-12 |
| Span / Depth          | 7.9             |                     |                   |      |          |

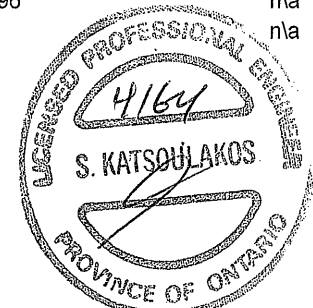
| Bearing Supports | Dim. (LxW)                 | Demand   | Demand/Resistance Support | Demand/Resistance Member | Material        |
|------------------|----------------------------|----------|---------------------------|--------------------------|-----------------|
| B1               | Wall/Plate 3-1/2" x 1-3/4" | 3683 lbs | 97.7%                     | 49.3%                    | Spruce-Pine-Fir |
| B2               | Wall/Plate 4-3/8" x 1-3/4" | 271 lbs  | 5.7%                      | 2.9%                     | Spruce-Pine-Fir |

### Notes

Design meets Code minimum (L/240) Total load deflection criteria.  
 Design meets Code minimum (L/360) Live load deflection criteria.  
 Calculations assume member is fully braced.  
 Resistance Factor phi has been applied to all presented results per CSA O86.  
 BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.  
 Design based on Dry Service Condition.  
 Importance Factor : Normal Part code : Part 9

CONFORMS TO CBC 2012

AMENDED 2020



OWB NO. TAM 9131 -21  
 STRUCTURAL  
 COMPONENT ONLY

### Disclosure

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BC CALC®, BC FRAMER®, AJSTM®, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,

Per: jocelyn.aguilar

CITY OF VANCOUVER  
 BUILDING DEPARTMENT  
 07/20/2021

BC CALC® Member Report

Build 7493

Job name:

File name: UNIT 2009 EL A,B.mmdl

Address:

Description: 1ST FLR FRAMING\Flush Beams\B2(i348)

City, Province, Postal Code:

Specifier:

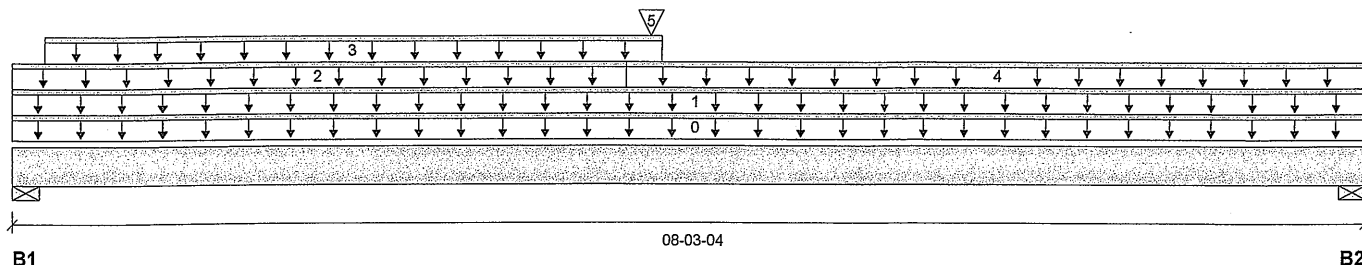
Customer:

Designer:

Code reports:

CCMC 12472-R

Company:



### Reaction Summary (Down / Uplift) (lbs)

| Bearing    | Live    | Dead    | Snow | Wind |
|------------|---------|---------|------|------|
| B1, 2-3/8" | 700 / 0 | 431 / 0 |      |      |
| B2, 4-3/8" | 506 / 0 | 333 / 0 |      |      |

### Load Summary

| Tag | Description        | Load Type         | Ref. | Start    | End      | Loc. | Live<br>1.00 | Dead<br>0.65 | Snow<br>1.00 | Wind<br>1.15 | Tributary |
|-----|--------------------|-------------------|------|----------|----------|------|--------------|--------------|--------------|--------------|-----------|
| 0   | Self-Weight        | Unf. Lin. (lb/ft) | L    | 00-00-00 | 08-03-04 | Top  |              | 12           |              |              | 00-00-00  |
| 1   | FC1 Floor Material | Unf. Lin. (lb/ft) | L    | 00-00-00 | 08-03-04 | Top  | 27           | 13           |              |              | n/a       |
| 2   | FC1 Floor Material | Unf. Lin. (lb/ft) | L    | 00-00-00 | 03-08-10 | Top  | 6            | 3            |              |              | n/a       |
| 3   | STAIR              | Unf. Lin. (lb/ft) | L    | 00-02-06 | 03-11-03 | Top  | 120          | 60           |              |              | n/a       |
| 4   | FC1 Floor Material | Unf. Lin. (lb/ft) | L    | 03-08-10 | 08-03-04 | Top  | 27           | 13           |              |              | n/a       |
| 5   | B4(i304)           | Conc. Pt. (lbs)   | L    | 03-10-06 | 03-10-06 | Top  | 395          | 258          |              |              | n/a       |

### Controls Summary

|                       | Factored Demand | Factored Resistance | Demand/Resistance | Case | Location |
|-----------------------|-----------------|---------------------|-------------------|------|----------|
| Pos. Moment           | 3583 ft-lbs     | 35392 ft-lbs        | 10.1%             | 1    | 03-10-06 |
| End Shear             | 1236 lbs        | 14464 lbs           | 8.5%              | 1    | 01-02-04 |
| Total Load Deflection | L/999 (0.026")  | n/a                 | n/a               | 4    | 03-11-03 |
| Live Load Deflection  | L/999 (0.016")  | n/a                 | n/a               | 5    | 03-11-03 |
| Max Defl.             | 0.026"          | n/a                 | n/a               | 4    | 03-11-03 |
| Span / Depth          | 7.9             |                     |                   |      |          |

### Bearing Supports

|    | Dim. (LxW)                 | Demand   | Demand/Resistance Support | Demand/Resistance Member | Material        |
|----|----------------------------|----------|---------------------------|--------------------------|-----------------|
| B1 | Wall/Plate 2-3/8" x 3-1/2" | 1589 lbs | 31.1%                     | 15.7%                    | Spruce-Pine-Fir |
| B2 | Wall/Plate 4-3/8" x 3-1/2" | 1176 lbs | 12.5%                     | 6.3%                     | Spruce-Pine-Fir |

### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor : Normal Part code : Part 9

CONFORMS TO OBC 2012

AMENDED 2020





# Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

PASSED

## 1ST FLR FRAMING\Flush Beams\B2(i348) (Flush Beam)

Dry | 1 span | No cant.

July 22, 2020 16:20:19

BC CALC® Member Report

Build 7493

Job name:

File name: UNIT 2009 EL A,B.mmdl

Address:

Description: 1ST FLR FRAMING\Flush Beams\B2(i348)

City, Province, Postal Code:

Specifier:

Customer:

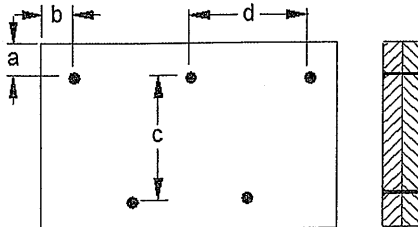
Designer:

Code reports:

CCMC 12472-R

Company:

### Connection Diagram: Full Length of Member



a minimum = 2"

c = 7-7/8"

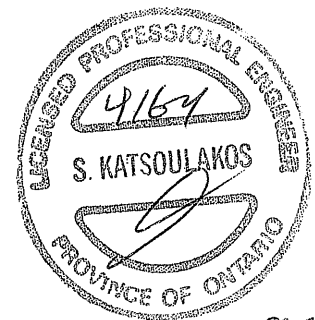
b minimum = 3"

d = 10 6/11"

Calculated Side Load = 457.5 lb/ft

Connectors are: 1 Nails

3 1/2" ARDOX SPIRAL



OWG NO. YAM 2132-21  
STRUCTURAL  
COMPONENT ONLY

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BC CALC®, BC FRAMER®, AJSTM®, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,

Per: jocelyn.aguilar

BC CALC® Member Report

Build 7493

Job name:

File name: UNIT 2009 EL A,B.mmdl

Address:

Description: 1ST FLR FRAMING\Flush Beams\B3(i332)

City, Province, Postal Code:

Specifier:

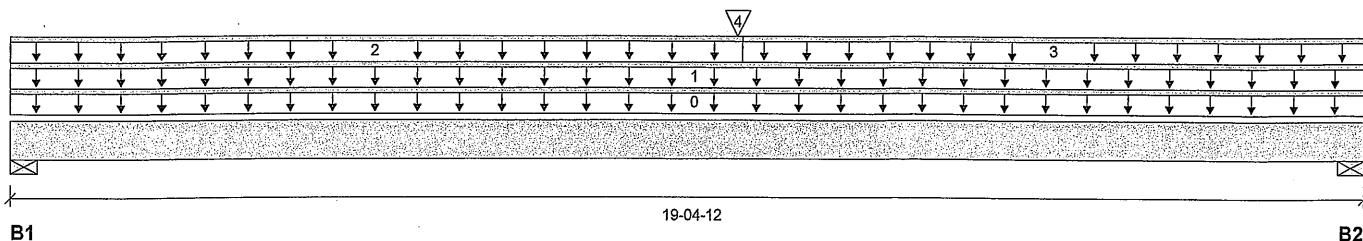
Customer:

Designer:

Code reports:

CCMC 12472-R

Company:


**Reaction Summary (Down / Uplift) (lbs)**

| Bearing    | Live    | Dead    | Snow | Wind |
|------------|---------|---------|------|------|
| B1, 2-3/8" | 289 / 0 | 294 / 0 |      |      |
| B2, 2-3/8" | 256 / 0 | 282 / 0 |      |      |

**Load Summary**

| Tag | Description        | Load Type         | Ref. | Start    | End      | Loc. | Live<br>1.00 | Dead<br>0.65 | Snow<br>1.00 | Wind<br>1.15 | Tributary |
|-----|--------------------|-------------------|------|----------|----------|------|--------------|--------------|--------------|--------------|-----------|
| 0   | Self-Weight        | Unf. Lin. (lb/ft) | L    | 00-00-00 | 19-04-12 | Top  |              | 12           |              |              | 00-00-00  |
| 1   | FC1 Floor Material | Unf. Lin. (lb/ft) | L    | 00-00-00 | 19-04-12 | Top  | 10           | 5            |              |              | n/a       |
| 2   | FC1 Floor Material | Unf. Lin. (lb/ft) | L    | 00-00-00 | 10-04-06 | Top  | 15           | 7            |              |              | n/a       |
| 3   | FC1 Floor Material | Unf. Lin. (lb/ft) | L    | 10-04-06 | 19-04-12 | Top  | 6            | 3            |              |              | n/a       |
| 4   | B7(i331)           | Conc. Pt. (lbs)   | L    | 10-03-08 | 10-03-08 | Top  | 155          | 147          |              |              | n/a       |

**Controls Summary**

|                       | Factored Demand | Factored Resistance | Demand/<br>Resistance | Case | Location |
|-----------------------|-----------------|---------------------|-----------------------|------|----------|
| Pos. Moment           | 4616 ft-lbs     | 35392 ft-lbs        | 13.0%                 | 1    | 10-03-08 |
| End Shear             | 722 lbs         | 14464 lbs           | 5.0%                  | 1    | 01-02-04 |
| Total Load Deflection | L/1102 (0.208") | n/a                 | 21.8%                 | 4    | 09-08-10 |
| Live Load Deflection  | L/999 (0.102")  | n/a                 | n/a                   | 5    | 09-08-10 |
| Max Defl.             | 0.208"          | n/a                 | n/a                   | 4    | 09-08-10 |
| Span / Depth          | 19.3            |                     |                       |      |          |

**Bearing Supports**

|    | Dim. (LxW)                 | Demand  | Demand/<br>Resistance Support | Demand/<br>Resistance Member | Material        |
|----|----------------------------|---------|-------------------------------|------------------------------|-----------------|
| B1 | Wall/Plate 2-3/8" x 3-1/2" | 801 lbs | 15.7%                         | 7.9%                         | Spruce-Pine-Fir |
| B2 | Wall/Plate 2-3/8" x 3-1/2" | 736 lbs | 14.4%                         | 7.3%                         | Spruce-Pine-Fir |

**Notes**

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

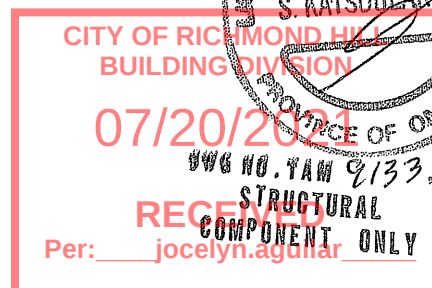
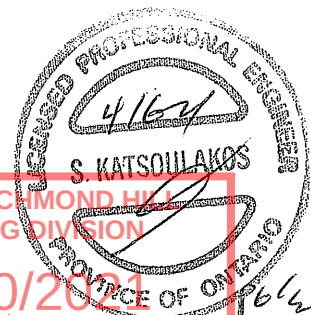
BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor : Normal Part code : Part 9

CONFORMS TO OBC 2012

AMENDED 2020





# Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

PASSED

## 1ST FLR FRAMING\Flush Beams\B3(i332) (Flush Beam)

Dry | 1 span | No cant.

July 22, 2020 16:20:19

BC CALC® Member Report  
Build 7493

Job name:

Address:

City, Province, Postal Code:

Customer:

Code reports: CCMC 12472-R

File name: UNIT 2009 EL A,B.mmdl

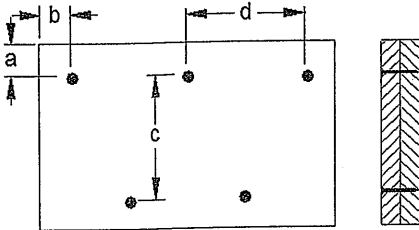
Description: 1ST FLR FRAMING\Flush Beams\B3(i332)

Specifier:

Designer:

Company:

### Connection Diagram: Full Length of Member



a minimum = 2"

c = 7-7/8"

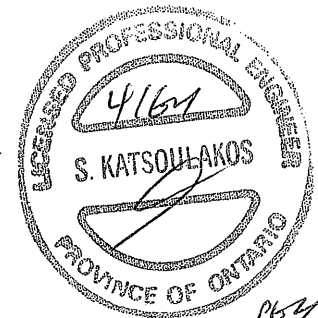
b minimum = 3"

d = 2'-0"

Calculated Side Load = 208.1 lb/ft

Connectors are: 1 Nails

3/4" ARDOX SPIRAL



OWB NO. TAM 9133-21  
STRUCTURAL  
COMPONENT ONLY

### Disclosure

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Per: jocelyn.aguilar

BC CALC® Member Report

Build 7493

Job name:

File name: UNIT 2009 EL A,B.mmdl

Address:

Description: 1ST FLR FRAMING\Flush Beams\B4(i304)

City, Province, Postal Code:

Specifier:

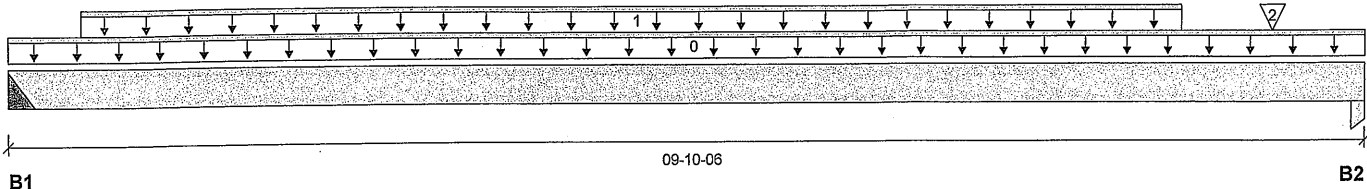
Customer:

Designer:

Code reports:

CCMC 12472-R

Company:



Total Horizontal Product Length = 09-10-06

### Reaction Summary (Down / Uplift) (lbs)

| Bearing    | Live    | Dead    | Snow | Wind |
|------------|---------|---------|------|------|
| B1, 4"     | 407 / 0 | 266 / 0 |      |      |
| B2, 1-3/4" | 416 / 0 | 268 / 0 |      |      |

### Load Summary

| Tag | Description   | Load Type         | Ref. | Start    | End      | Loc. | Live | Dead | Snow | Wind | Tributary |
|-----|---------------|-------------------|------|----------|----------|------|------|------|------|------|-----------|
| 0   | Self-Weight   | Unf. Lin. (lb/ft) | L    | 00-00-00 | 09-10-06 | Top  | 1.00 | 0.65 | 1.00 | 1.15 | 00-00-00  |
| 1   | Smoothed Load | Unf. Lin. (lb/ft) | L    | 00-06-04 | 08-06-04 | Top  | 91   | 46   |      |      | n/a       |
| 2   | J4(i314)      | Conc. Pt. (lbs)   | L    | 09-02-04 | 09-02-04 | Top  | 90   | 45   |      |      | n/a       |

### Controls Summary

|                       | Factored Demand | Factored Resistance | Demand/Resistance | Case | Location |
|-----------------------|-----------------|---------------------|-------------------|------|----------|
| Pos. Moment           | 2374 ft-lbs     | 35392 ft-lbs        | 6.7%              | 1    | 05-02-04 |
| End Shear             | 888 lbs         | 14464 lbs           | 6.1%              | 1    | 01-03-14 |
| Total Load Deflection | L/999 (0.028")  | n/a                 | n/a               | 4    | 05-00-04 |
| Live Load Deflection  | L/999 (0.017")  | n/a                 | n/a               | 5    | 05-00-04 |
| Max Defl.             | 0.028"          | n/a                 | n/a               | 4    | 05-00-04 |
| Span / Depth          | 9.6             |                     |                   |      |          |

| Bearing Supports | Dim. (LxW)      | Demand  | Demand/Resistance Support | Demand/Resistance Member | Material    |
|------------------|-----------------|---------|---------------------------|--------------------------|-------------|
| B1 Hanger        | 4" x 3-1/2"     | 943 lbs | n/a                       | 5.5%                     | HGUS410     |
| B2 Column        | 1-3/4" x 3-1/2" | 959 lbs | 19.3%                     | 12.8%                    | Unspecified |

### Cautions

Header for the hanger HGUS410 is a Double 1-3/4" x 11-7/8" LVL Beam.

Hanger model HGUS410 and seat length were input by the user. Hanger has not been analyzed for adequate capacity.

### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA O86.

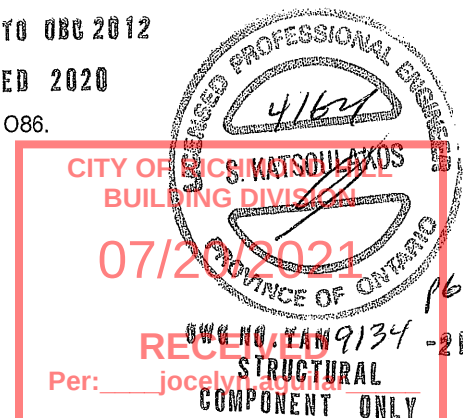
BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor : Normal Part code : Part 9

CONFORMS TO OBC 2012

AMENDED 2020





# Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

PASSED

## 1ST FLR FRAMING\Flush Beams\B4(i304) (Flush Beam)

Dry | 1 span | No cant.

July 22, 2020 16:20:19

BC CALC® Member Report

Build 7493

Job name:

Address:

City, Province, Postal Code:

Customer:

Code reports:

CCMC 12472-R

File name: UNIT 2009 EL A,B.mmdl

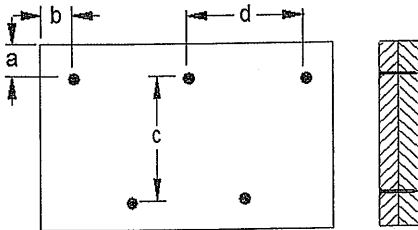
Description: 1ST FLR FRAMING\Flush Beams\B4(i304)

Specifier:

Designer:

Company:

### Connection Diagram: Full Length of Member



a minimum = 2"

c = 7-7/8" "

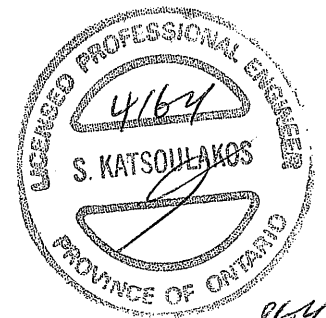
b minimum = 3"

d = 2 1/4" "

Calculated Side Load = 257.8 lb/ft

Connectors are: 3

3 1/2" ARDOX SPIRAL Nails



DWG NO. YAM 9134-21

STRUCTURAL

COMPONENT ONLY

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BC CALC®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,

Per: jocelyn.aguilar

BC CALC® Member Report  
Build 7493

Job name:

File name: UNIT 2009 EL A,B.mmdl

Address:

Description: 1ST FLR FRAMING\Flush Beams\B5(i315)

City, Province, Postal Code:

Specifier:

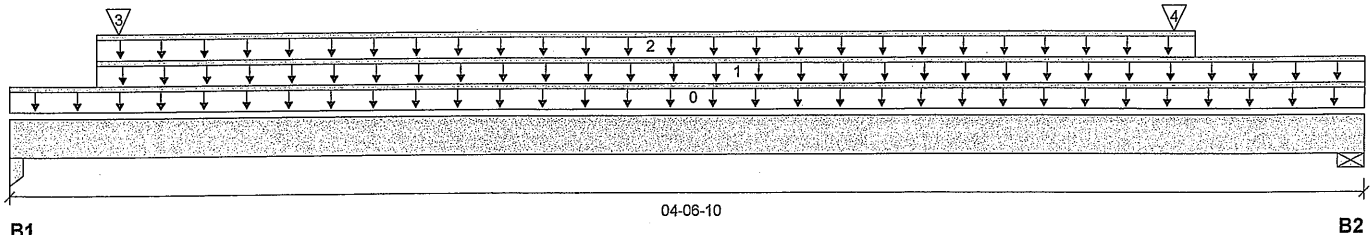
Customer:

Designer:

Code reports:

CCMC 12472-R

Company:



Total Horizontal Product Length = 04-06-10

### Reaction Summary (Down / Uplift) (lbs)

| Bearing    | Live    | Dead    | Snow | Wind |
|------------|---------|---------|------|------|
| B1, 3-1/2" | 444 / 0 | 249 / 0 |      |      |
| B2, 4-3/8" | 179 / 0 | 115 / 0 |      |      |

### Load Summary

| Tag | Description        | Load Type         | Ref. | Start    | End      | Loc. | Live<br>1.00 | Dead<br>0.65 | Snow<br>1.00 | Wind<br>1.15 | Tributary |
|-----|--------------------|-------------------|------|----------|----------|------|--------------|--------------|--------------|--------------|-----------|
| 0   | Self-Weight        | Unf. Lin. (lb/ft) | L    | 00-00-00 | 04-06-10 | Top  |              | 6            |              |              | 00-00-00  |
| 1   | FC1 Floor Material | Unf. Lin. (lb/ft) | L    | 00-03-08 | 04-06-10 | Top  | 15           | 7            |              |              | n/a       |
| 2   | FC1 Floor Material | Unf. Lin. (lb/ft) | L    | 00-03-08 | 03-11-12 | Top  | 12           | 6            |              |              | n/a       |
| 3   | B9(i317)           | Conc. Pt. (lbs)   | L    | 00-04-06 | 00-04-06 | Top  | 381          | 203          |              |              | n/a       |
| 4   | B8(i165)           | Conc. Pt. (lbs)   | L    | 03-10-14 | 03-10-14 | Top  | 136          | 80           |              |              | n/a       |

### Controls Summary

|                       | Factored Demand | Factored Resistance | Demand/<br>Resistance | Case | Location |
|-----------------------|-----------------|---------------------|-----------------------|------|----------|
| Pos. Moment           | 186 ft-lbs      | 17696 ft-lbs        | 1.1%                  | 1    | 02-07-14 |
| End Shear             | 142 lbs         | 7232 lbs            | 2.0%                  | 1    | 03-02-06 |
| Total Load Deflection | L/999 (0.001")  | n/a                 | n/a                   | 4    | 02-04-05 |
| Live Load Deflection  | L/999 (0")      | n/a                 | n/a                   | 5    | 02-04-05 |
| Max Defl.             | 0.001"          | n/a                 | n/a                   | 4    | 02-04-05 |
| Span / Depth          | 4.1             |                     |                       |      |          |

### Bearing Supports

|         |            |                 |         | Demand/<br>Resistance<br>Support | Demand/<br>Resistance<br>Member |                 |
|---------|------------|-----------------|---------|----------------------------------|---------------------------------|-----------------|
| Bearing | Supports   | Dim. (LxW)      | Demand  |                                  |                                 | Material        |
| B1      | Column     | 3-1/2" x 1-3/4" | 978 lbs | 19.7%                            | 13.1%                           | Unspecified     |
| B2      | Wall/Plate | 4-3/8" x 1-3/4" | 412 lbs | 8.8%                             | 4.4%                            | Spruce-Pine-Fir |

### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor : Normal Part code : Part 9

CONFORMS TO OBC 2012

AMENDED 2020



DWG NO. YAW 8135-21

STRUCTURAL

COMPONENT ONLY

### Disclosure

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BC CALC®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,

Per: jocelyn.aguilar

CITY OF RICHMOND  
BUILDING DEPARTMENT

07/20/2021

BC CALC® Member Report

Build 7493

Job name:

Address:

City, Province, Postal Code:

Customer:

Code reports: CCMC 12472-R

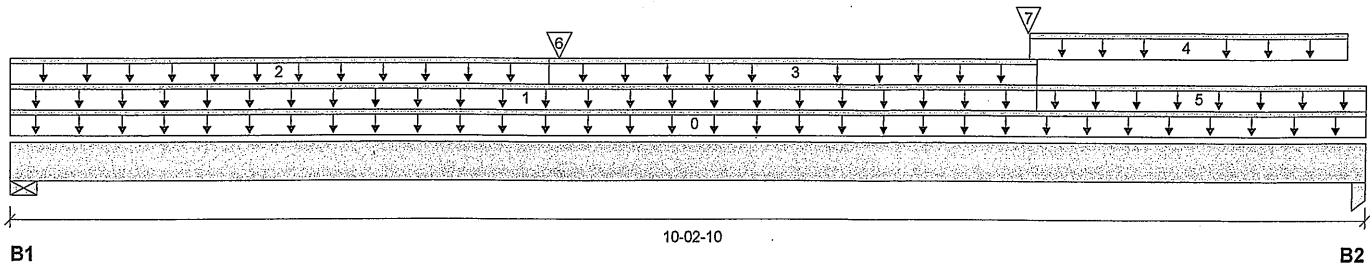
File name: UNIT 2009 EL A,B.mmdl

Description: 1ST FLR FRAMING\Flush Beams\B6(i340)

Specifier:

Designer:

Company:



### Reaction Summary (Down / Uplift) (lbs)

| Bearing    | Live    | Dead    | Snow | Wind |
|------------|---------|---------|------|------|
| B1, 2-3/8" | 334 / 0 | 258 / 0 |      |      |
| B2, 1-3/4" | 319 / 0 | 362 / 0 |      |      |

### Load Summary

| Tag | Description        | Load Type         | Ref. | Start    | End      | Loc. | Live<br>1.00 | Dead<br>0.65 | Snow<br>1.00 | Wind<br>1.15 | Tributary |
|-----|--------------------|-------------------|------|----------|----------|------|--------------|--------------|--------------|--------------|-----------|
| 0   | Self-Weight        | Unf. Lin. (lb/ft) | L    | 00-00-00 | 10-02-10 | Top  |              | 12           |              |              | 00-00-00  |
| 1   | FC1 Floor Material | Unf. Lin. (lb/ft) | L    | 00-00-00 | 07-08-06 | Top  | 5            | 3            |              |              | n/a       |
| 2   | FC1 Floor Material | Unf. Lin. (lb/ft) | L    | 00-00-00 | 04-00-02 | Top  | 6            | 3            |              |              | n/a       |
| 3   | FC1 Floor Material | Unf. Lin. (lb/ft) | L    | 04-00-02 | 07-08-06 | Top  | 21           | 11           |              |              | n/a       |
| 4   | WALL               | Unf. Lin. (lb/ft) | L    | 07-07-10 | 10-00-14 | Top  |              | 60           |              |              | n/a       |
| 5   | FC1 Floor Material | Unf. Lin. (lb/ft) | L    | 07-08-06 | 10-02-10 | Top  | 8            | 4            |              |              | n/a       |
| 6   | B9(i317)           | Conc. Pt. (lbs)   | L    | 04-01-00 | 04-01-00 | Top  | 367          | 196          |              |              | n/a       |
| 7   | B8(i165)           | Conc. Pt. (lbs)   | L    | 07-07-08 | 07-07-08 | Top  | 121          | 73           |              |              | n/a       |

### Controls Summary

|                       | Factored Demand | Factored Resistance | Demand/Resistance | Case | Location |
|-----------------------|-----------------|---------------------|-------------------|------|----------|
| Pos. Moment           | 2925 ft-lbs     | 35392 ft-lbs        | 8.3%              | 1    | 04-01-00 |
| End Shear             | 820 lbs         | 14464 lbs           | 5.7%              | 1    | 09-01-00 |
| Total Load Deflection | L/999 (0.036")  | n/a                 | n/a               | 4    | 05-00-15 |
| Live Load Deflection  | L/999 (0.02")   | n/a                 | n/a               | 5    | 05-00-15 |
| Max Defl.             | 0.036"          | n/a                 | n/a               | 4    | 05-00-15 |
| Span / Depth          | 10.1            |                     |                   |      |          |

### Bearing Supports

|    | Dim. (LxW)                 | Demand  | Demand/Resistance Support | Demand/Resistance Member | Material        |
|----|----------------------------|---------|---------------------------|--------------------------|-----------------|
| B1 | Wall/Plate 2-3/8" x 3-1/2" | 823 lbs | 16.1%                     | 8.1%                     | Spruce-Pine-Fir |
| B2 | Column 1-3/4" x 3-1/2"     | 931 lbs | 18.7%                     | 12.5%                    | Unspecified     |

### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume unbraced length of Top: 00-00-00, Bottom: 00-00-00.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor : Normal Part code : Part 9

CONFORMS TO OBC 2012

AMENDED 2020

CITY OF RICHMOND HILL  
BUILDING DIVISION

07/20/2021

RECEIVED

Per: jocelyn.aguilar



4162  
NO. TAM 9136-21  
STRUCTURAL  
COMPONENT ONLY



# Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

PASSED

## 1ST FLR FRAMING\Flush Beams\B6(i340) (Flush Beam)

Dry | 1 span | No cant.

July 22, 2020 16:20:19

BC CALC® Member Report

Build 7493

Job name:

Address:

City, Province, Postal Code:

Customer:

Code reports:

CCMC 12472-R

File name: UNIT 2009 EL A,B.mmdl

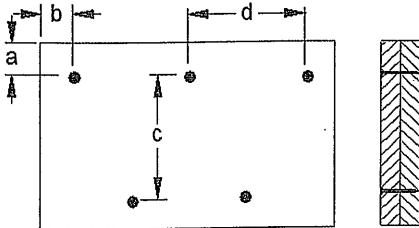
Description: 1ST FLR FRAMING\Flush Beams\B6(i340)

Specifier:

Designer:

Company:

### Connection Diagram: Full Length of Member



a minimum = 2"

c = 7-7/8"

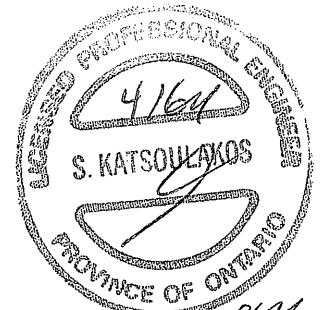
b minimum = 3"

d = 2'-3"

Calculated Side Load = 397.8 lb/ft

Connectors are: 16d Nails

3 1/2" ARDOX SPIRAL



OWG NO. YAM 9136 -21

STRUCTURAL  
COMPONENT ONLY

### Disclosure

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

CITY OF BOISE  
BUILDING DEPARTMENT

07/20/2021

BC CALC®, BC FRAMER®, AJST™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,

Per: jocelyn.aguilar

BC CALC® Member Report  
Build 7493

Dry | 1 span | No cant.

July 22, 2020 16:20:19

Job name:

File name: UNIT 2009 EL A,B.mmdl

Address:

Description: 1ST FLR FRAMING\Flush Beams\B7(i331)

City, Province, Postal Code:

Specifier:

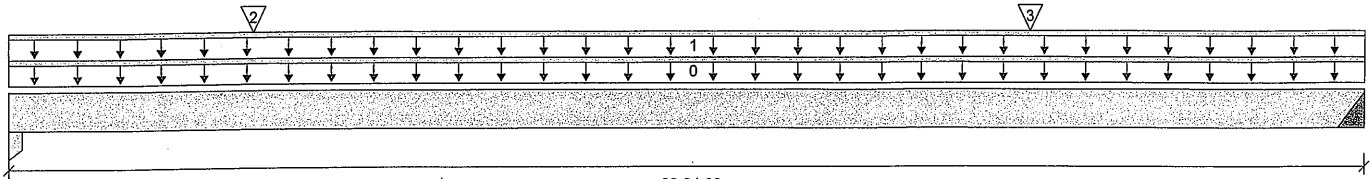
Customer:

Designer:

Code reports:

CCMC 12472-R

Company:



B1 02-04-00 B2

Total Horizontal Product Length = 02-04-00

### Reaction Summary (Down / Uplift) (lbs)

| Bearing    | Live    | Dead    | Snow | Wind |
|------------|---------|---------|------|------|
| B1, 3-1/2" | 209 / 0 | 186 / 0 |      |      |
| B2, 2"     | 179 / 0 | 162 / 0 |      |      |

### Load Summary

| Tag | Description | Load Type         | Ref. | Start    | End      | Loc. | Live | Dead | Snow | Wind | Tributary |
|-----|-------------|-------------------|------|----------|----------|------|------|------|------|------|-----------|
| 0   | Self-Weight | Unf. Lin. (lb/ft) | L    | 00-00-00 | 02-04-00 | Top  | 1.00 | 0.65 | 1.00 | 1.15 | 00-00-00  |
| 1   | WALL        | Unf. Lin. (lb/ft) | L    | 00-00-00 | 02-04-00 | Top  |      | 60   |      |      | n/a       |
| 2   | J1(i325)    | Conc. Pt. (lbs)   | L    | 00-05-00 | 00-05-00 | Top  | 174  | 87   |      |      | n/a       |
| 3   | J1(i328)    | Conc. Pt. (lbs)   | L    | 01-09-00 | 01-09-00 | Top  | 214  | 107  |      |      | n/a       |

### Controls Summary

|                       | Factored Demand | Factored Resistance | Demand/Resistance | Case | Location |
|-----------------------|-----------------|---------------------|-------------------|------|----------|
| Pos. Moment           | 212 ft-lbs      | 17696 ft-lbs        | 1.2%              | 1    | 01-09-00 |
| End Shear             | 116 lbs         | 7232 lbs            | 1.6%              | 1    | 01-03-06 |
| Total Load Deflection | L/999 (0")      | n/a                 | n/a               | 4    | 01-03-06 |
| Live Load Deflection  | L/999 (0")      | n/a                 | n/a               | 5    | 01-03-06 |
| Max Defl.             | 0"              | n/a                 | n/a               | 4    | 01-03-06 |
| Span / Depth          | 2.0             |                     |                   |      |          |

### Bearing Supports

|                  |        |                 |         | Demand/<br>Resistance<br>Support | Demand/<br>Resistance<br>Member |             |
|------------------|--------|-----------------|---------|----------------------------------|---------------------------------|-------------|
| Bearing Supports |        | Dim. (LxW)      | Demand  |                                  |                                 | Material    |
| B1               | Column | 3-1/2" x 1-3/4" | 545 lbs | 11.0%                            | 7.3%                            | Unspecified |
| B2               | Hanger | 2" x 1-3/4"     | 472 lbs | n/a                              | 11.0%                           | HUS1.81/10  |

### Cautions

Header for the hanger HUS1.81/10 is a Double 1-3/4" x 11-7/8" LVL Beam.  
Hanger model HUS1.81/10 and seat length were input by the user. Hanger has not been analyzed for adequate capacity.

### Notes

Design meets Code minimum (L/240) Total load deflection criteria.  
Design meets Code minimum (L/360) Live load deflection criteria.  
Calculations assume member is fully braced.  
Hanger Manufacturer: Unassigned  
Resistance Factor phi has been applied to all presented results per CSA O86.  
BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.  
Design based on Dry Service Condition.  
Importance Factor : Normal Part code : Part 9

CONFORMS TO CBC 2012

AMENDED 2020

### Disclosure

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BC CALC®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,

Per: jocelyn.aguilar



DWG NO. TAM 9137 -21  
STRUCTURAL  
COMPONENT ONLY

CITY OF BRIMLEY  
BUILDING DIVISION  
07/20/2021



# Single 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

PASSED

## 1ST FLR FRAMING\Flush Beams\B8(i165) (Flush Beam)

BC CALC® Member Report  
Build 7493

Dry | 1 span | No cant.

July 22, 2020 16:20:19

Job name:

File name: UNIT 2009 EL A,B.mmdl

Address:

Description: 1ST FLR FRAMING\Flush Beams\B8(i165)

City, Province, Postal Code:

Specifier:

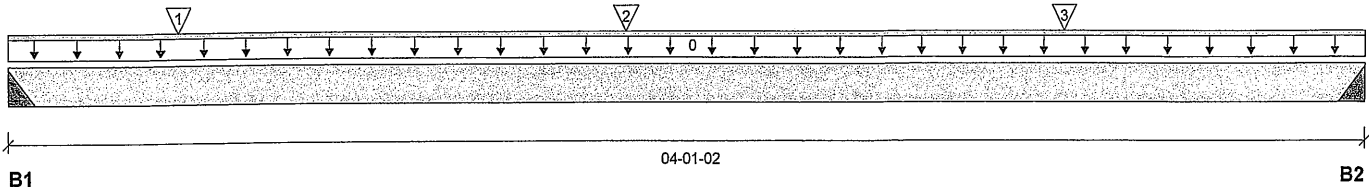
Customer:

Designer:

Code reports:

CCMC 12472-R

Company:



Total Horizontal Product Length = 04-01-02

### Reaction Summary (Down / Uplift) (lbs)

| Bearing | Live    | Dead   | Snow | Wind |
|---------|---------|--------|------|------|
| B1, 2"  | 136 / 0 | 80 / 0 |      |      |
| B2, 2"  | 121 / 0 | 73 / 0 |      |      |

### Load Summary

| Tag | Description | Load Type         | Ref. | Start    | End      | Loc. | Live | Dead | Snow | Wind | Tributary |
|-----|-------------|-------------------|------|----------|----------|------|------|------|------|------|-----------|
| 0   | Self-Weight | Unf. Lin. (lb/ft) | L    | 00-00-00 | 04-01-02 | Top  | 1.00 | 0.65 | 1.00 | 1.15 | 00-00-00  |
| 1   | J5(i174)    | Conc. Pt. (lbs)   | L    | 00-06-02 | 00-06-02 | Top  | 71   | 35   |      |      | n/a       |
| 2   | J5(i150)    | Conc. Pt. (lbs)   | L    | 01-10-02 | 01-10-02 | Top  | 98   | 49   |      |      | n/a       |
| 3   | J5(i126)    | Conc. Pt. (lbs)   | L    | 03-02-02 | 03-02-02 | Top  | 88   | 44   |      |      | n/a       |

### Controls Summary

|                       | Factored Demand | Factored Resistance | Demand/Resistance | Case | Location |
|-----------------------|-----------------|---------------------|-------------------|------|----------|
| Pos. Moment           | 316 ft-lbs      | 17696 ft-lbs        | 1.8%              | 1    | 01-10-02 |
| End Shear             | 218 lbs         | 7232 lbs            | 3.0%              | 1    | 02-11-04 |
| Total Load Deflection | L/999 (0.001")  | n/a                 | n/a               | 4    | 02-00-09 |
| Live Load Deflection  | L/999 (0.001")  | n/a                 | n/a               | 5    | 02-00-09 |
| Max Defl.             | 0.001"          | n/a                 | n/a               | 4    | 02-00-09 |
| Span / Depth          | 3.9             |                     |                   |      |          |

### Bearing Supports

|    | Dim. (LxW)         | Demand  | Demand/Resistance Support | Demand/Resistance Member | Material   |
|----|--------------------|---------|---------------------------|--------------------------|------------|
| B1 | Hanger 2" x 1-3/4" | 304 lbs | n/a                       | 7.1%                     | HUS1.81/10 |
| B2 | Hanger 2" x 1-3/4" | 272 lbs | n/a                       | 6.4%                     | HUS1.81/10 |

### Cautions

Header for the hanger HUS1.81/10 is a Single 1-3/4" x 11-7/8" LVL Beam.

Hanger model HUS1.81/10 and seat length were input by the user. Hanger has not been analyzed for adequate capacity.

Header for the hanger HUS1.81/10 is a Double 1-3/4" x 11-7/8" LVL Beam.

### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor : Normal Part code : Part 9

CONFORMS TO CBC 2012

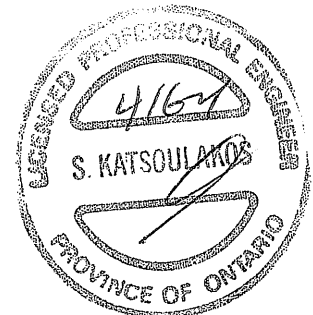
AMENDED 2020

### Disclosure

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BC CALC®, BC FRAMER®, AJST™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,

Per: jocelyn.aguiar



UWG NO. TAM 9138 -21

STRUCTURAL

COMPONENT ONLY

07/20/2021

BC CALC® Member Report

Build 7493

Job name:

File name: UNIT 2009 EL A,B.mmdl

Address:

Description: 1ST FLR FRAMING\Flush Beams\B9(i317)

City, Province, Postal Code:

Specifier:

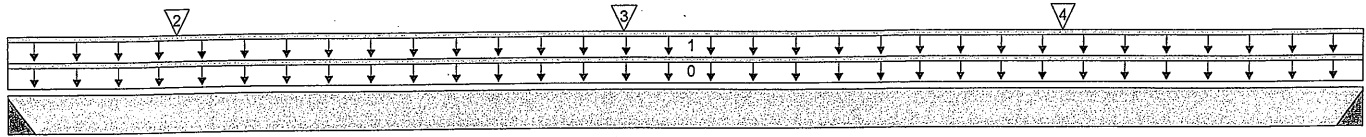
Customer:

Designer:

Code reports:

CCMC 12472-R

Company:



B1

Total Horizontal Product Length = 04-01-02

B2

### Reaction Summary (Down / Uplift) (lbs)

| Bearing | Live    | Dead    | Snow | Wind |
|---------|---------|---------|------|------|
| B1, 2"  | 382 / 0 | 203 / 0 |      |      |
| B2, 2"  | 367 / 0 | 196 / 0 |      |      |

### Load Summary

| Tag | Description | Load Type         | Ref. | Start    | End      | Loc. | Live<br>1.00 | Dead<br>0.65 | Snow<br>1.00 | Wind<br>1.15 | Tributary |
|-----|-------------|-------------------|------|----------|----------|------|--------------|--------------|--------------|--------------|-----------|
| 0   | Self-Weight | Unf. Lin. (lb/ft) | L    | 00-00-00 | 04-01-02 | Top  |              | 6            |              |              | 00-00-00  |
| 1   | STAIR       | Unf. Lin. (lb/ft) | L    | 00-00-00 | 04-01-02 | Top  | 120          | 60           |              |              | n/a       |
| 2   | J5(i174)    | Conc. Pt. (lbs)   | L    | 00-06-02 | 00-06-02 | Top  | 71           | 35           |              |              | n/a       |
| 3   | J5(i150)    | Conc. Pt. (lbs)   | L    | 01-10-02 | 01-10-02 | Top  | 98           | 49           |              |              | n/a       |
| 4   | J5(i126)    | Conc. Pt. (lbs)   | L    | 03-02-02 | 03-02-02 | Top  | 88           | 44           |              |              | n/a       |

### Controls Summary

|                       | Factored Demand | Factored Resistance | Demand/Resistance | Case | Location |
|-----------------------|-----------------|---------------------|-------------------|------|----------|
| Pos. Moment           | 792 ft-lbs      | 17696 ft-lbs        | 4.5%              | 1    | 01-10-02 |
| End Shear             | 445 lbs         | 7232 lbs            | 6.2%              | 1    | 02-11-04 |
| Total Load Deflection | L/999 (0.003")  | n/a                 | n/a               | 4    | 02-00-09 |
| Live Load Deflection  | L/999 (0.002")  | n/a                 | n/a               | 5    | 02-00-09 |
| Max Defl.             | 0.003"          | n/a                 | n/a               | 4    | 02-00-09 |
| Span / Depth          | 3.9             |                     |                   |      |          |

| Bearing Supports | Dim. (LxW)  | Demand  | Demand/Resistance Support | Demand/Resistance Member | Material   |
|------------------|-------------|---------|---------------------------|--------------------------|------------|
| B1 Hanger        | 2" x 1-3/4" | 826 lbs | n/a                       | 19.3%                    | HUS1.81/10 |
| B2 Hanger        | 2" x 1-3/4" | 794 lbs | n/a                       | 18.6%                    | HUS1.81/10 |

### Cautions

Header for the hanger HUS1.81/10 is a Single 1-3/4" x 11-7/8" LVL Beam.  
Hanger model HUS1.81/10 and seat length were input by the user. Hanger has not been analyzed for adequate capacity.  
Header for the hanger HUS1.81/10 is a Double 1-3/4" x 11-7/8" LVL Beam.

### Notes

Design meets Code minimum (L/240) Total load deflection criteria.  
Design meets Code minimum (L/360) Live load deflection criteria.  
Calculations assume unbraced length of Top: 00-00-00, Bottom: 00-00-00.  
Hanger Manufacturer: Unassigned  
Resistance Factor phi has been applied to all presented results per CSA O86.  
BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.  
Design based on Dry Service Condition.  
Importance Factor : Normal Part code : Part 9



OWG NO. TAM 2139 -21  
STRUCTURAL  
COMPONENT ONLY

### Disclosure

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BC CALC®, BC FRAMER®, AJST™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,

Per: jocelyn.aguilar

CONFORMS TO OBC 2012

AMENDED 2020

CITY OF BOISE BUILDING DEPARTMENT

07/20/2021

BC CALC® Member Report  
Build 7493

Job name:

File name: UNIT 2009 EL A,B.mmdl

Address:

Description: 2ND FLR FRAMING\Flush Beams\B10(i326)

City, Province, Postal Code:

Specifier:

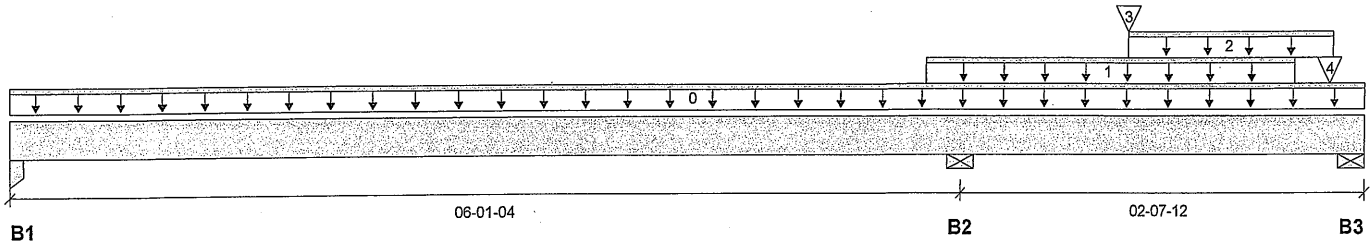
Customer:

Designer:

Code reports:

CCMC 12472-R

Company:



### Reaction Summary (Down / Uplift) (lbs)

| Bearing    | Live    | Dead    | Snow    | Wind |
|------------|---------|---------|---------|------|
| B1, 3-1/2" | 0 / 6   | 25 / 0  | 0 / 4   |      |
| B2, 5-1/2" | 216 / 0 | 333 / 0 | 191 / 0 |      |
| B3, 5-1/2" | 186 / 0 | 229 / 0 | 188 / 0 |      |

### Load Summary

| Tag | Description        | Load Type         | Ref. | Start    | End      | Loc. | Live<br>1.00 | Dead<br>0.65 | Snow<br>1.00 | Wind<br>1.15 | Tributary |
|-----|--------------------|-------------------|------|----------|----------|------|--------------|--------------|--------------|--------------|-----------|
| 0   | Self-Weight        | Unf. Lin. (lb/ft) | L    | 00-00-00 | 08-09-00 | Top  |              | 12           |              |              | 00-00-00  |
| 1   | E10(i260)          | Unf. Lin. (lb/ft) | L    | 05-10-08 | 08-03-08 | Top  | 55           | 131          | 130          |              | n/a       |
| 2   | FC2 Floor Material | Unf. Lin. (lb/ft) | L    | 07-02-08 | 08-06-08 | Top  | 6            |              |              |              | n/a       |
| 3   | J4(i344)           | Conc. Pt. (lbs)   | L    | 07-02-08 | 07-02-08 | Top  | 229          | 115          |              |              | n/a       |
| 4   | E9(i258)           | Conc. Pt. (lbs)   | L    | 08-06-04 | 08-06-04 | Top  | 25           | 47           | 60           |              | n/a       |

### Controls Summary

|                       | Factored Demand | Factored Resistance | Demand/Resistance | Case | Location |
|-----------------------|-----------------|---------------------|-------------------|------|----------|
| Pos. Moment           | 434 ft-lbs      | 29611 ft-lbs        | 1.5%              | 18   | 07-02-08 |
| Neg. Moment           | -180 ft-lbs     | -29611 ft-lbs       | 0.6%              | 1    | 06-01-04 |
| End Shear             | 187 lbs         | 14464 lbs           | 1.3%              | 43   | 07-03-10 |
| Cont. Shear           | 239 lbs         | 14464 lbs           | 1.7%              | 43   | 07-03-14 |
| Total Load Deflection | L/999 (0")      | n/a                 | n/a               | 126  | 07-03-01 |
| Live Load Deflection  | L/999 (-0")     | n/a                 | n/a               | 178  | 03-07-12 |
| Total Neg. Defl.      | L/999 (-0")     | n/a                 | n/a               | 126  | 04-03-03 |
| Max Defl.             | 0"              | n/a                 | n/a               | 126  | 07-03-01 |
| Span / Depth          | 5.9             |                     |                   |      |          |

| Bearing Supports | Dim. (LxW)      | Demand  | Demand/Resistance Support | Demand/Resistance Member | Material        |
|------------------|-----------------|---------|---------------------------|--------------------------|-----------------|
| B1 Column        | 3-1/2" x 3-1/2" | 35 lbs  | 0.5%                      | 0.4%                     | Unspecified     |
| B2 Wall/Plate    | 5-1/2" x 3-1/2" | 931 lbs | 7.9%                      | 4.0%                     | Spruce-Pine-Fir |
| B3 Wall/Plate    | 5-1/2" x 3-1/2" | 754 lbs | 6.4%                      | 3.2%                     | Spruce-Pine-Fir |

### Notes

Design meets Code minimum (L/240) Total load deflection criteria.  
 Design meets Code minimum (L/360) Live load deflection criteria.  
 Calculations assume unbraced length of Top: 05-10-08, Bottom: 05-10-08.  
 Resistance Factor phi has been applied to all presented results per CSA O86.  
 BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.  
 Unbalanced snow loads determined from building geometry were used in selected product's verification.  
 Design based on Dry Service Condition.  
 Importance Factor : Normal Part code : Part 9

CONFORMS TO OBC 2012

AMENDED

2020 OF RICHMOND HILL  
BUILDING DIVISION

07/20/2021

RECEIVED

Per: joco COMPONENT ONLY



OWN NO. TAM 9140 -21



# Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

PASSED

## 2ND FLR FRAMING\Flush Beams\B10(i326) (Flush Beam)

Dry | 2 spans | No cant.

July 22, 2020 16:20:19

BC CALC® Member Report

Build 7493

Job name:

Address:

City, Province, Postal Code:

Customer:

Code reports:

CCMC 12472-R

File name: UNIT 2009 EL A,B.mmdl

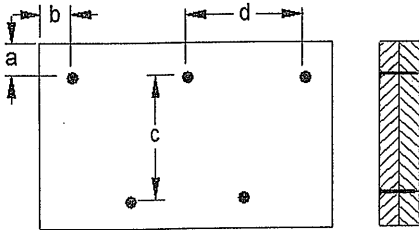
Description: 2ND FLR FRAMING\Flush Beams\B10(i326)

Specifier:

Designer:

Company:

### Connection Diagram: Full Length of Member



a minimum = 2"

c = 7-7/8"

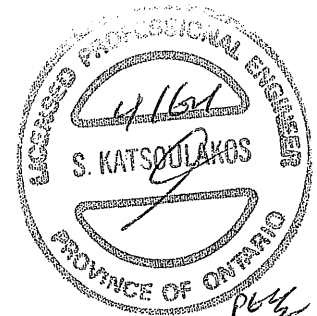
b minimum = 3"

d = 2'-0" 8"

Calculated Side Load = 243.6 lb/ft

Connectors are: 3/4" ARDOX SPIRAL Nails

3/4" ARDOX SPIRAL



ENG NO. YAM 9140 -21

STRUCTURAL  
COMPONENT ONLY

### Disclosure

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BC CALC®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,

Per: jocelyn.aguilar

BC CALC® Member Report

Dry | 1 span | No cant.

July 22, 2020 16:20:19

Build 7493

Job name:

File name: UNIT 2009 EL A,B.mmdl

Address:

Description: 2ND FLR FRAMING\Flush Beams\B14(i289) (Flush Beam)

City, Province, Postal Code:

Specifier:

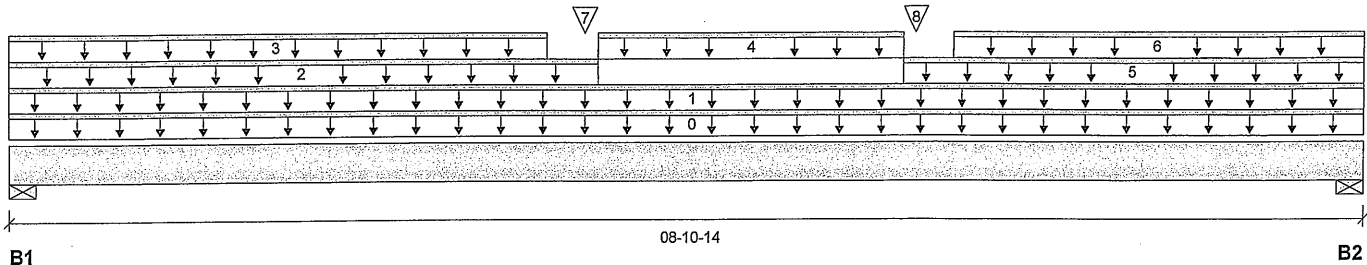
Customer:

Designer:

Code reports:

CCMC 12472-R

Company:



Total Horizontal Product Length = 08-10-14

### Reaction Summary (Down / Uplift) (lbs)

| Bearing    | Live     | Dead     | Snow     | Wind |
|------------|----------|----------|----------|------|
| B1, 5-1/2" | 1281 / 0 | 1546 / 0 | 2814 / 0 |      |
| B2, 4-3/8" | 1249 / 0 | 1509 / 0 | 2743 / 0 |      |

### Load Summary

| Tag | Description        | Load Type         | Ref. | Start    | End      | Loc. | Live<br>1.00 | Dead<br>0.65 | Snow<br>1.00 | Wind<br>1.15 | Tributary |
|-----|--------------------|-------------------|------|----------|----------|------|--------------|--------------|--------------|--------------|-----------|
| 0   | Self-Weight        | Unf. Lin. (lb/ft) | L    | 00-00-00 | 08-10-14 | Top  |              | 12           |              |              | 00-00-00  |
| 1   | FC2 Floor Material | Unf. Lin. (lb/ft) | L    | 00-00-00 | 08-10-14 | Top  | 20           | 10           |              |              | n/a       |
| 2   | E12(i261)          | Unf. Lin. (lb/ft) | L    | 00-00-00 | 03-10-00 | Top  |              | 81           |              |              | n/a       |
| 3   | E12(i261)          | Unf. Lin. (lb/ft) | L    | 00-00-00 | 03-06-00 | Top  | 264          | 240          | 624          |              | n/a       |
| 4   | E13(i262)          | Unf. Lin. (lb/ft) | L    | 03-10-00 | 05-10-00 | Top  |              | 61           |              |              | n/a       |
| 5   | E14(i263)          | Unf. Lin. (lb/ft) | L    | 05-10-00 | 08-10-14 | Top  |              | 81           |              |              | n/a       |
| 6   | E14(i263)          | Unf. Lin. (lb/ft) | L    | 06-02-00 | 08-10-14 | Top  | 264          | 240          | 624          |              | n/a       |
| 7   | E12(i261)          | Conc. Pt. (lbs)   | L    | 03-09-00 | 03-09-00 | Top  | 362          | 349          | 856          |              | n/a       |
| 8   | E14(i263)          | Conc. Pt. (lbs)   | L    | 05-11-00 | 05-11-00 | Top  | 342          | 331          | 808          |              | n/a       |

### Controls Summary

|                       | Factored Demand | Factored Resistance | Demand/Resistance | Case | Location |
|-----------------------|-----------------|---------------------|-------------------|------|----------|
| Pos. Moment           | 13516 ft-lbs    | 35392 ft-lbs        | 38.2%             | 13   | 03-09-00 |
| End Shear             | 5047 lbs        | 14464 lbs           | 34.9%             | 13   | 01-05-06 |
| Total Load Deflection | L/769 (0.128")  | n/a                 | 31.2%             | 35   | 04-05-08 |
| Live Load Deflection  | L/999 (0.093")  | n/a                 | n/a               | 51   | 04-05-08 |
| Max Defl.             | 0.128"          | n/a                 | n/a               | 35   | 04-05-08 |
| Span / Depth          | 8.3             |                     |                   |      |          |

| Bearing Supports | Dim. (LxW)                 | Demand   | Demand/Resistance Support | Demand/Resistance Member | Material        |
|------------------|----------------------------|----------|---------------------------|--------------------------|-----------------|
| B1               | Wall/Plate 5-1/2" x 3-1/2" | 7434 lbs | 62.8%                     | 31.7%                    | Spruce-Pine-Fir |
| B2               | Wall/Plate 4-3/8" x 3-1/2" | 7249 lbs | 77.0%                     | 38.8%                    | Spruce-Pine-Fir |

### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

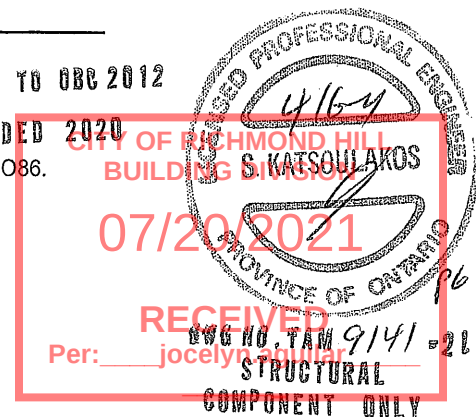
Unbalanced snow loads determined from building geometry were used in selected product's verification.

Design based on Dry Service Condition.

Importance Factor : Normal Part code : Part 9

CONFORMS TO OBC 2012

AMENDED 2020





# Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

PASSED

## 2ND FLR FRAMING\Flush Beams\B14(i289) (Flush Beam)

Dry | 1 span | No cant.

July 22, 2020 16:20:19

BC CALC® Member Report  
Build 7493

Job name:

File name: UNIT 2009 EL A,B.mmdl

Address:

Description: 2ND FLR FRAMING\Flush Beams\B14(i289)

City, Province, Postal Code:

Specifier:

Customer:

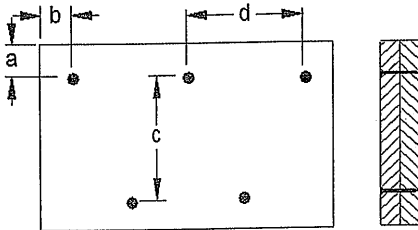
Designer:

Code reports:

CCMC 12472-R

Company:

### Connection Diagram: Full Length of Member



a minimum = 2"

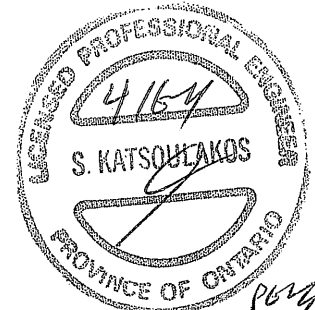
c = 7-7/8"

b minimum = 3"

d = 24 6"

Connectors are:

3 1/2" ARDOX SPIRAL Nails



ENG NO. TAM 9141 -2C  
STRUCTURAL  
COMPONENT ONLY

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BC CALC®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,

Per: jocelyn.aguilar

BC CALC® Member Report  
Build 0

Dry | 1 span | No cant.

July 22, 2020 16:20:19

Job name:

File name: UNIT 2009 EL A,B.mmdl

Address:

Description: 2ND FLR FRAMING\Flush Beams\B15(i313)

City, Province, Postal Code:

Specifier:

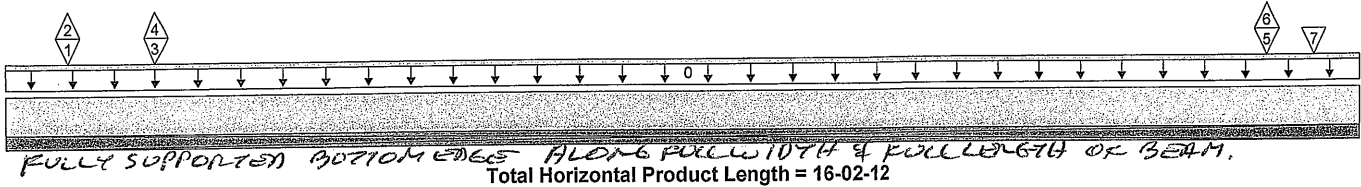
Customer:

Designer:

Code reports:

CCMC 12472-R

Company:



### Load Summary

| Tag | Description | Load Type         | Ref. | Start    | End      | Loc. | Live<br>1.00 | Dead<br>0.65 | Snow<br>1.00 | Wind<br>1.15 | Tributary |
|-----|-------------|-------------------|------|----------|----------|------|--------------|--------------|--------------|--------------|-----------|
| 0   | Self-Weight | Unf. Lin. (lb/ft) | L    | 00-00-00 | 16-02-12 | Top  |              |              |              |              | 00-00-00  |
| 1   | J1(i227)    | Conc. Pt. (lbs)   | L    | 00-09-00 | 00-09-00 | Top  | 183          | 62           |              |              | n/a       |
| 2   | J1(i227)    | Conc. Pt. (lbs)   | L    | 00-09-00 | 00-09-00 | Top  | -60          |              |              |              | n/a       |
| 3   | B12(i306)   | Conc. Pt. (lbs)   | L    | 01-09-12 | 01-09-12 | Top  | 354          | 238          |              |              | n/a       |
| 4   | B12(i306)   | Conc. Pt. (lbs)   | L    | 01-09-12 | 01-09-12 | Top  | -429         |              |              |              | n/a       |
| 5   | B11(i310)   | Conc. Pt. (lbs)   | L    | 15-01-04 | 15-01-04 | Top  | 1240         | 820          |              |              | n/a       |
| 6   | B11(i310)   | Conc. Pt. (lbs)   | L    | 15-01-04 | 15-01-04 | Top  | -436         |              |              |              | n/a       |
| 7   | J5(i211)    | Conc. Pt. (lbs)   | L    | 15-08-00 | 15-08-00 | Top  | 309          | 155          |              |              | n/a       |

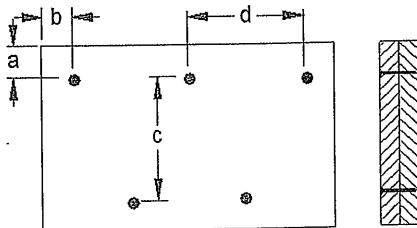
### Controls Summary

|            | Factored Demand | Factored Resistance | Demand/Resistance | Case | Location |
|------------|-----------------|---------------------|-------------------|------|----------|
| Dist. Load | 13.28 lb/ft     | 57645.00 lb/ft      | n/a               |      |          |
| Conc. Load | 2885 lbs        | 16813 lbs           | 17.2%             |      |          |

### Cautions

Concentrated side load(s) 7 are closer than 18" from end of member. Please consult a technical representative or Professional of Record.

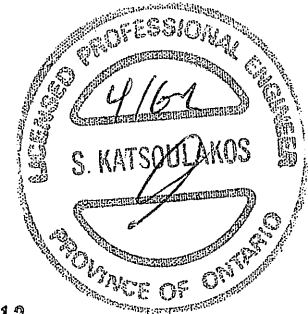
### Connection Diagram: Full Length of Member



a minimum = 2"      c = 7-7/8"  
b minimum = 3"      d = 12"

Calculated Side Load = 328.6 lb/ft  
Connectors are: 16d      1 Nails

3/2" ARDOX SPIRAL



CONFORMS TO OBC 2012  
AMENDED 2020  
STRUCTURAL COMPONENT ONLY

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BC CALC®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,

Per: jocelyn.aguilar



Boise Cascade

**Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP****2ND FLR FRAMING\Flush Beams\B10(i599) (Flush Beam)****PASSED**

BC CALC® Member Report

Dry | 1 span | No cant.

July 27, 2020 08:46:28

Build 7493

Job name:

File name: UNIT 2009 EL A,B.mmdl

Address:

Description: 2ND FLR FRAMING\Flush Beams\B10(i599)

City, Province, Postal Code: RICHMOND HILL

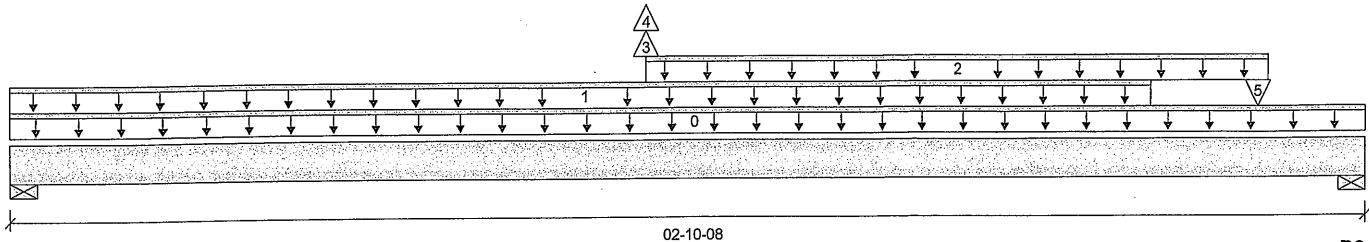
Specifier:

Customer:

Designer: AJ

Code reports: CCMC 12472-R

Company:



B1

Total Horizontal Product Length = 02-10-08

B2

**Reaction Summary (Down / Uplift) (lbs)**

| Bearing    | Live      | Dead    | Snow    | Wind |
|------------|-----------|---------|---------|------|
| B1, 5-1/2" | 100 / 181 | 125 / 0 | 187 / 0 |      |
| B2, 5-1/2" | 101 / 148 | 129 / 0 | 187 / 0 |      |

**Load Summary**

| Tag | Description        | Load Type         | Ref. | Start    | End      | Loc. | Live<br>1.00 | Dead<br>0.65 | Snow<br>1.00 | Wind<br>1.15 | Tributary |
|-----|--------------------|-------------------|------|----------|----------|------|--------------|--------------|--------------|--------------|-----------|
| 0   | Self-Weight        | Unf. Lin. (lb/ft) | L    | 00-00-00 | 02-10-08 | Top  |              | 12           |              |              | 00-00-00  |
| 1   | E10(i260)          | Unf. Lin. (lb/ft) | L    | 00-00-00 | 02-05-00 | Top  | 55           | 131          | 130          |              | n/a       |
| 2   | FC2 Floor Material | Unf. Lin. (lb/ft) | L    | 01-04-00 | 02-08-00 | Top  | 6            | 3            |              |              | n/a       |
| 3   | J4(i344)           | Conc. Pt. (lbs)   | L    | 01-04-00 | 01-04-00 | Top  | 35           | -147         |              |              | n/a       |
| 4   | J4(i344)           | Conc. Pt. (lbs)   | L    | 01-04-00 | 01-04-00 | Top  | -329         |              |              |              | n/a       |
| 5   | E9(i258)           | Conc. Pt. (lbs)   | L    | 02-07-12 | 02-07-12 | Top  | 25           | 47           | 60           |              | n/a       |

**Controls Summary**

|                       | Factored Demand | Factored Resistance | Demand/<br>Resistance | Case | Location |
|-----------------------|-----------------|---------------------|-----------------------|------|----------|
| Pos. Moment           | 171 ft-lbs      | 35392 ft-lbs        | 0.5%                  | 25   | 01-07-03 |
| Neg. Moment           | -253 ft-lbs     | -35392 ft-lbs       | 0.7%                  | 24   | 01-04-00 |
| End Shear             | 309 lbs         | 14464 lbs           | 2.1%                  | 2    | 01-05-02 |
| Total Load Deflection | L/999 (0")      | n/a                 | n/a                   | 58   | 01-05-06 |
| Live Load Deflection  | L/999 (-0")     | n/a                 | n/a                   | 84   | 01-04-13 |
| Total Neg. Defl.      | L/999 (-0")     | n/a                 | n/a                   | 57   | 01-04-09 |
| Max Defl.             | 0"              | n/a                 | n/a                   | 58   | 01-05-06 |
| Span / Depth          | 2.1             |                     |                       |      |          |

**Bearing Supports**

|    | Dim. (LxW)                 | Demand  | Demand/<br>Resistance<br>Support | Demand/<br>Resistance<br>Member | Material        |
|----|----------------------------|---------|----------------------------------|---------------------------------|-----------------|
| B1 | Wall/Plate 5-1/2" x 3-1/2" | 537 lbs | 4.5%                             | 2.3%                            | Spruce-Pine-Fir |
| B1 | Uplift                     | 158 lbs |                                  |                                 |                 |
| B2 | Wall/Plate 5-1/2" x 3-1/2" | 543 lbs | 4.6%                             | 2.3%                            | Spruce-Pine-Fir |
| B2 | Uplift                     | 106 lbs |                                  |                                 |                 |

**Cautions**

Uplift of 158 lbs found at bearing B1. (SIMPSON 2-H2-5A @ B1).

CITY OF RICHMOND HILL  
BUILDING DIVISION

07/20/2021

OWG NO. YAM 9143 -21

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STRUCTURAL  
COMPONENT ONLY

Per: jocelyn.aguiar



**Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP**  
**2ND FLR FRAMING\Flush Beams\B10(i599) (Flush Beam)**

**PASSED**

BC CALC® Member Report

Dry | 1 span | No cant.

July 27, 2020 08:46:28

Build 7493

Job name:

File name: UNIT 2009 EL A,B.mmdl

Address:

Description: 2ND FLR FRAMING\Flush Beams\B10(i599)

City, Province, Postal Code: RICHMOND HILL

Specifier:

Customer:

Designer: AJ

Code reports:

CCMC 12472-R

Company:

**Notes**

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Unbalanced snow loads determined from building geometry were used in selected product's verification.

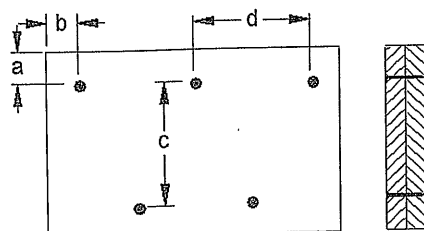
Design based on Dry Service Condition.

Importance Factor : Normal Part code : Part 9

CONFORMS TO OBC 2012

AMENDED 2020

**Connection Diagram: Full Length of Member**



a minimum = 2"  
b minimum = 3"

c = 7-7/8"  
d = 6"

Calculated Side Load = 338.6 lb/ft

Connectors are: 16d Nails

**3 1/2" ARDOX SPIRAL**



ENG. NO. 9143 -21  
**STRUCTURAL  
COMPONENT ONLY**

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BC CALC®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,

Per: jocelyn.aguiar



# Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

## 2ND FLR FRAMING\Flush Beams\B11(i597) (Flush Beam)

**PASSED**

BC CALC® Member Report

Dry | 1 span | No cant.

July 27, 2020 08:46:28

Build 7493

Job name:

File name: UNIT 2009 EL A,B.mmdl

Address:

Description: 2ND FLR FRAMING\Flush Beams\B11(i597)

City, Province, Postal Code: RICHMOND HILL

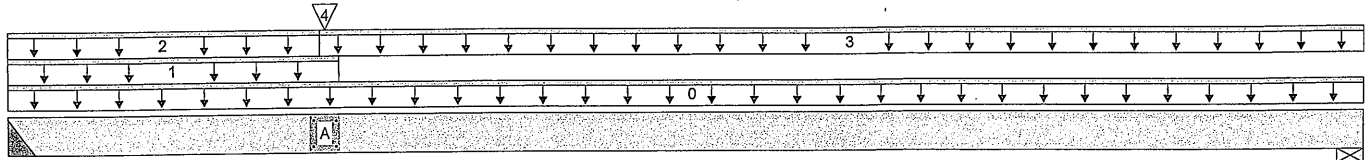
Specifier:

Customer:

Designer: AJ

Code reports: CCMC 12472-R

Company:



B1

Total Horizontal Product Length = 19-00-06

B2

### Reaction Summary (Down / Uplift) (lbs)

| Bearing    | Live     | Dead     | Snow | Wind |
|------------|----------|----------|------|------|
| B1, 4"     | 1254 / 0 | 1020 / 0 |      |      |
| B2, 2-3/8" | 631 / 0  | 469 / 0  |      |      |

### Load Summary

| Tag | Description        | Load Type         | Ref. | Start    | End      | Loc. | Live<br>1.00 | Dead<br>0.65 | Snow<br>1.00 | Wind<br>1.15 | Tributary |
|-----|--------------------|-------------------|------|----------|----------|------|--------------|--------------|--------------|--------------|-----------|
| 0   | Self-Weight        | Unf. Lin. (lb/ft) | L    | 00-00-00 | 19-00-06 | Top  |              | 12           |              |              | 00-00-00  |
| 1   | WALL               | Unf. Lin. (lb/ft) | L    | 00-00-00 | 04-07-04 | Top  |              | 60           |              |              | n/a       |
| 2   | FC2 Floor Material | Unf. Lin. (lb/ft) | L    | 00-00-00 | 04-04-00 | Top  | 14           | 7            |              |              | n/a       |
| 3   | FC2 Floor Material | Unf. Lin. (lb/ft) | L    | 04-04-00 | 19-00-06 | Top  | 38           | 19           |              |              | n/a       |
| 4   | B13(i590)          | Conc. Pt. (lbs)   | L    | 04-04-14 | 04-04-14 | Top  | 1261         | 671          |              |              | n/a       |

### Controls Summary

|                       | Factored Demand | Factored Resistance | Demand/<br>Resistance | Case | Location |
|-----------------------|-----------------|---------------------|-----------------------|------|----------|
| Pos. Moment           | 11889 ft-lbs    | 35392 ft-lbs        | 33.6%                 | 1    | 04-04-14 |
| End Shear             | 2998 lbs        | 14464 lbs           | 20.7%                 | 1    | 01-03-14 |
| Total Load Deflection | L/454 (0.492")  | n/a                 | 52.8%                 | 4    | 08-11-06 |
| Live Load Deflection  | L/777 (0.288")  | n/a                 | 46.3%                 | 5    | 08-11-06 |
| Max Defl.             | 0.492"          | n/a                 | n/a                   | 4    | 08-11-06 |
| Span / Depth          | 18.8            |                     |                       |      |          |

| Bearing Supports | Dim. (LxW)      | Demand   | Demand/<br>Resistance<br>Support | Demand/<br>Resistance<br>Member | Material        |
|------------------|-----------------|----------|----------------------------------|---------------------------------|-----------------|
| B1 Hanger        | 4" x 3-1/2"     | 3157 lbs | n/a                              | 18.5%                           | HGUS410         |
| B2 Wall/Plate    | 2-3/8" x 3-1/2" | 1533 lbs | 30.0%                            | 15.1%                           | Spruce-Pine-Fir |

### Cautions

Header for the hanger HGUS410 is a Double 1-3/4" x 11-7/8" LVL Beam.

Hanger model HGUS410 and seat length were input by the user. Hanger has not been analyzed for adequate capacity.

### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor : Normal Part code : Part 9

CONFORMS TO OBC 2012

AMENDED 2020

CITY OF RICHMOND HILL  
BUILDING DIVISION

07/20/2021

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COMPONENT ONLY

Per: jocelyn.aguiar



07/20/2021

07/20/2021

07/20/2021

BC CALC® Member Report  
Build 7493

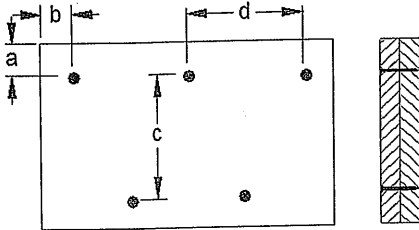
Dry | 1 span | No cant.

July 27, 2020 08:46:28

Job name:  
Address:  
City, Province, Postal Code: RICHMOND HILL  
Customer:  
Code reports: CCMC 12472-R

File name: UNIT 2009 EL A,B.mmdl  
Description: 2ND FLR FRAMING\Flush Beams\B11(i597)  
Specifier:  
Designer: AJ  
Company:

### Connection Diagram: Full Length of Member



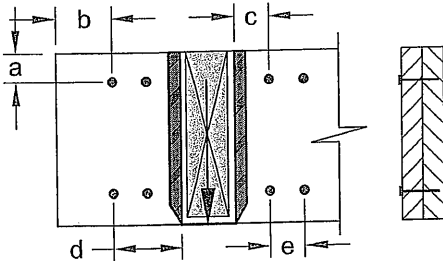
a minimum = 2"  
b minimum = 3"  
c = 7-7/8"  
d = 24"

Connectors are: 1 Nails

3 1/2" ARDOX SPIRAL

### Connection Diagrams: Concentrated Side Loads

Connection Tag: A Applies to load tag(s): 4



a minimum = 2"  
b minimum = 4"  
c minimum = 4"  
d maximum = 12"  
e minimum = 4"

Connectors are: 16d Nails

3 1/2" ARDOX SPIRAL



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BC CALC®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,

Per: jocelyn.aguilar

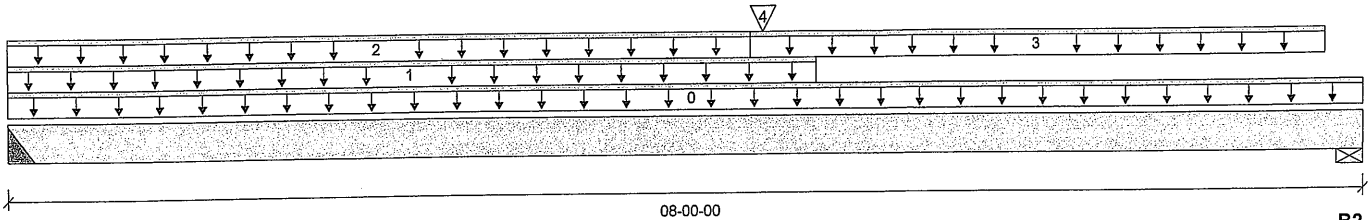
BC CALC® Member Report  
Build 7493

Dry | 1 span | No cant.

July 27, 2020 08:46:28

Job name:  
Address:  
City, Province, Postal Code: RICHMOND HILL  
Customer:  
Code reports: CCMC 12472-R

File name: UNIT 2009 EL A,B.mmdl  
Description: 2ND FLR FRAMING\Flush Beams\B12(i595)  
Specifier:  
Designer: AJ  
Company:



Total Horizontal Product Length = 08-00-00

### Reaction Summary (Down / Uplift) (lbs)

| Bearing    | Live    | Dead    | Snow | Wind |
|------------|---------|---------|------|------|
| B1, 2"     | 342 / 0 | 409 / 0 |      |      |
| B2, 5-1/2" | 469 / 0 | 367 / 0 |      |      |

### Load Summary

| Tag | Description        | Load Type         | Ref. | Start    | End      | Loc. | Live<br>1.00 | Dead<br>0.65 | Snow<br>1.00 | Wind<br>1.15 | Tributary |
|-----|--------------------|-------------------|------|----------|----------|------|--------------|--------------|--------------|--------------|-----------|
| 0   | Self-Weight        | Unf. Lin. (lb/ft) | L    | 00-00-00 | 08-00-00 | Top  |              | 6            |              |              | 00-00-00  |
| 1   | WALL               | Unf. Lin. (lb/ft) | L    | 00-00-00 | 04-08-09 | Top  |              | 60           |              |              | n/a       |
| 2   | FC2 Floor Material | Unf. Lin. (lb/ft) | L    | 00-00-00 | 04-04-00 | Top  | 24           | 12           |              |              | n/a       |
| 3   | FC2 Floor Material | Unf. Lin. (lb/ft) | L    | 04-04-00 | 07-09-04 | Top  | 47           | 23           |              |              | n/a       |
| 4   | B13(i590)          | Conc. Pt. (lbs)   | L    | 04-04-14 | 04-04-14 | Top  | 545          | 313          |              |              | n/a       |

### Controls Summary

|                       | Factored Demand | Factored Resistance | Demand/<br>Resistance | Case | Location |
|-----------------------|-----------------|---------------------|-----------------------|------|----------|
| Pos. Moment           | 3105 ft-lbs     | 17696 ft-lbs        | 17.5%                 | 1    | 04-04-14 |
| End Shear             | 1030 lbs        | 7232 lbs            | 14.2%                 | 1    | 06-06-10 |
| Total Load Deflection | L/999 (0.04")   | n/a                 | n/a                   | 4    | 03-11-04 |
| Live Load Deflection  | L/999 (0.021")  | n/a                 | n/a                   | 5    | 04-00-07 |
| Max Defl.             | 0.04"           | n/a                 | n/a                   | 4    | 03-11-04 |
| Span / Depth          | 7.6             |                     |                       |      |          |

### Bearing Supports

|    | Dim. (LxW)                 | Demand   | Demand/<br>Resistance Support | Demand/<br>Resistance Member | Material        |
|----|----------------------------|----------|-------------------------------|------------------------------|-----------------|
| B1 | Hanger 2" x 1-3/4"         | 1024 lbs | n/a                           | 24.0%                        | HUS1.81/10      |
| B2 | Wall/Plate 5-1/2" x 1-3/4" | 1162 lbs | 19.6%                         | 9.9%                         | Spruce-Pine-Fir |

### Cautions

Header for the hanger HUS1.81/10 is a Double 1-3/4" x 11-7/8" LVL Beam.  
Hanger model HUS1.81/10 and seat length were input by the user. Hanger has not been analyzed for adequate capacity.

### Notes

Design meets Code minimum (L/240) Total load deflection criteria.  
Design meets Code minimum (L/360) Live load deflection criteria.  
Calculations assume member is fully braced.  
Hanger Manufacturer: Unassigned  
Resistance Factor phi has been applied to all presented results per CSA O86.  
BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.  
Design based on Dry Service Condition.  
Importance Factor : Normal Part code : Part 9

CONFORMS TO OBC 2012

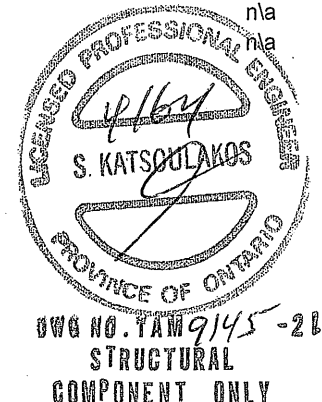
AMENDED 2020

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BC CALC®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,

Per: jocelyn.aguilar





**Single 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP**  
**2ND FLR FRAMING\Flush Beams\B13(i590) (Flush Beam)**

**PASSED**

BC CALC® Member Report

Dry | 1 span | No cant.

July 27, 2020 08:46:28

Build 7493

Job name:

File name: UNIT 2009 EL A,B.mmdl

Address:

Description: 2ND FLR FRAMING\Flush Beams\B13(i590)

City, Province, Postal Code: RICHMOND HILL

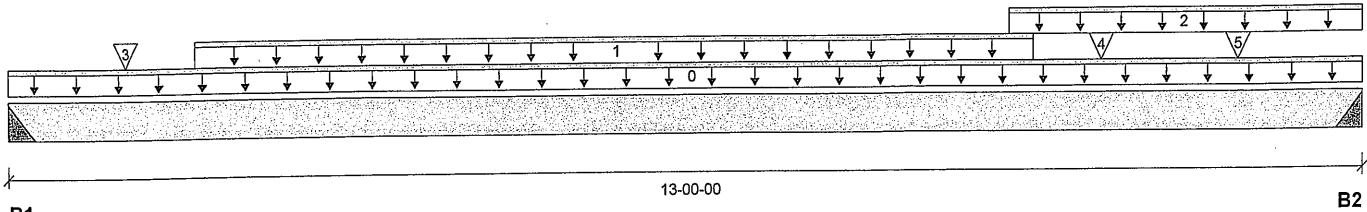
Specifier:

Customer:

Designer: AJ

Code reports: CCMC 12472-R

Company:



Total Horizontal Product Length = 13-00-00

**Reaction Summary (Down / Uplift) (lbs)**

| Bearing | Live     | Dead    | Snow | Wind |
|---------|----------|---------|------|------|
| B1, 2"  | 539 / 0  | 310 / 0 |      |      |
| B2, 2"  | 1267 / 0 | 674 / 0 |      |      |

**Load Summary**

| Tag | Description   | Load Type         | Ref. | Start    | End      | Loc. | Live<br>1.00 | Dead<br>0.65 | Snow<br>1.00 | Wind<br>1.15 | Tributary |
|-----|---------------|-------------------|------|----------|----------|------|--------------|--------------|--------------|--------------|-----------|
| 0   | Self-Weight   | Unf. Lin. (lb/ft) | L    | 00-00-00 | 13-00-00 | Top  |              | 6            |              |              | 00-00-00  |
| 1   | Smoothed Load | Unf. Lin. (lb/ft) | L    | 01-09-08 | 09-09-08 | Top  | 68           | 34           |              |              | n/a       |
| 2   | STAIR         | Unf. Lin. (lb/ft) | L    | 09-06-11 | 13-00-00 | Top  | 120          | 60           |              |              | n/a       |
| 3   | J5(i523)      | Conc. Pt. (lbs)   | L    | 01-01-08 | 01-01-08 | Top  | 87           | 43           |              |              | n/a       |
| 4   | J2(i465)      | Conc. Pt. (lbs)   | L    | 10-05-08 | 10-05-08 | Top  | 364          | 182          |              |              | n/a       |
| 5   | J2(i404)      | Conc. Pt. (lbs)   | L    | 11-09-08 | 11-09-08 | Top  | 396          | 198          |              |              | n/a       |

**Controls Summary**

|                       | Factored Demand | Factored Resistance | Demand/Resistance | Case | Location |
|-----------------------|-----------------|---------------------|-------------------|------|----------|
| Pos. Moment           | 5152 ft-lbs     | 17696 ft-lbs        | 29.1%             | 1    | 07-09-08 |
| End Shear             | 2440 lbs        | 7232 lbs            | 33.7%             | 1    | 11-10-02 |
| Total Load Deflection | L/693 (0.222")  | n/a                 | 34.7%             | 4    | 06-09-08 |
| Live Load Deflection  | L/1077 (0.143") | n/a                 | 33.4%             | 5    | 06-09-08 |
| Max Defl.             | 0.222"          | n/a                 | n/a               | 4    | 06-09-08 |
| Span / Depth          | 12.9            |                     |                   |      |          |

**Bearing Supports**

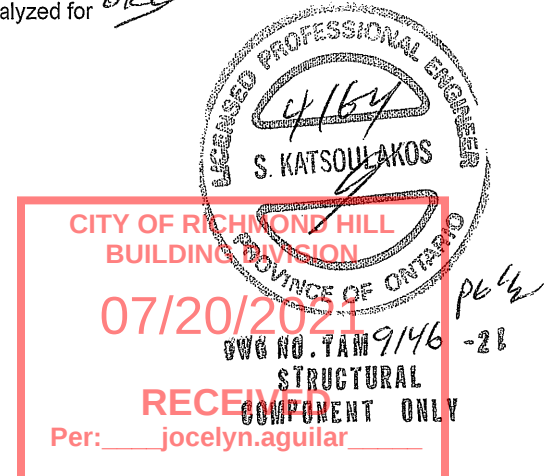
|           | Dim. (LxW)  | Demand   | Demand/Resistance Support | Demand/Resistance Member | Material   |
|-----------|-------------|----------|---------------------------|--------------------------|------------|
| B1 Hanger | 2" x 1-3/4" | 1196 lbs | n/a                       | 28.0%                    | HUS1.81/10 |
| B2 Hanger | 2" x 1-3/4" | 2743 lbs | n/a                       | 64.2%                    | HUS1.81/10 |

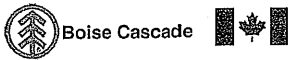
**Cautions**

Header for the hanger HUS1.81/10 is a Single 1-3/4" x 11-7/8" LVL Beam.

Hanger model HUS1.81/10 and seat length were input by the user. Hanger has not been analyzed for adequate capacity.

Header for the hanger HUS1.81/10 is a Double 1-3/4" x 11-7/8" LVL Beam.





**Single 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP**  
**2ND FLR FRAMING\Flush Beams\B13(i590) (Flush Beam)**

**PASSED**

BC CALC® Member Report  
Build 7493

Dry | 1 span | No cant.

July 27, 2020 08:46:28

Job name:  
Address:  
City, Province, Postal Code: RICHMOND HILL  
Customer:  
Code reports: CCMC 12472-R

File name: UNIT 2009 EL A,B.mmdl  
Description: 2ND FLR FRAMING\Flush Beams\B13(i590)  
Specifier:  
Designer: AJ  
Company:

**Notes**

Design meets Code minimum (L/240) Total load deflection criteria.  
Design meets Code minimum (L/360) Live load deflection criteria.  
Calculations assume member is fully braced.  
Hanger Manufacturer: Unassigned  
Resistance Factor phi has been applied to all presented results per CSA O86.  
BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.  
Design based on Dry Service Condition.  
Importance Factor : Normal Part code : Part 9

CONFORMS TO OBC 2012

AMENDED 2020



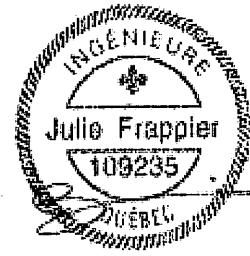
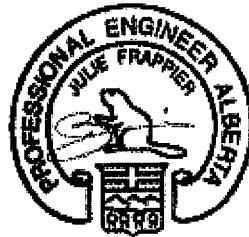
OWN NO. TAM 9/46 -21  
STRUCTURAL  
COMPONENT ONLY

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Per: jocelyn.aguilar



## Maximum Floor Spans

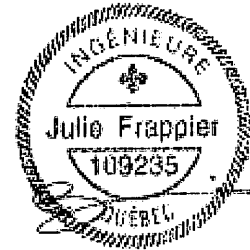
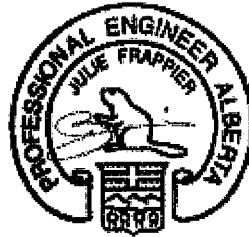
Live Load = 40 psf, Dead Load = 15 psf  
Simple Spans, L/480 Deflection Limit  
5/8" OSB G&N Sheathing

| Depth   | Series | Bare              |         |         |     | 1/2" Gypsum Ceiling |         |         |     |
|---------|--------|-------------------|---------|---------|-----|---------------------|---------|---------|-----|
|         |        | On Centre Spacing |         |         |     | On Centre Spacing   |         |         |     |
|         |        | 12"               | 16"     | 19.2"   | 24" | 12"                 | 16"     | 19.2"   | 24" |
| 9-1/2"  | NI-20  | 15'-1"            | 14'-2"  | 13'-9"  | N/A | 15'-7"              | 14'-8"  | 14'-2"  | N/A |
|         | NI-40x | 16'-1"            | 15'-2"  | 14'-8"  | N/A | 16'-7"              | 15'-7"  | 15'-1"  | N/A |
|         | NI-60  | 16'-3"            | 15'-4"  | 14'-10" | N/A | 16'-8"              | 15'-9"  | 15'-3"  | N/A |
|         | NI-70  | 17'-1"            | 16'-1"  | 15'-6"  | N/A | 17'-5"              | 16'-5"  | 15'-10" | N/A |
|         | NI-80  | 17'-3"            | 16'-3"  | 15'-8"  | N/A | 17'-8"              | 16'-7"  | 16'-0"  | N/A |
| 11-7/8" | NI-20  | 16'-11"           | 16'-0"  | 15'-5"  | N/A | 17'-6"              | 16'-6"  | 16'-0"  | N/A |
|         | NI-40x | 18'-1"            | 17'-0"  | 16'-5"  | N/A | 18'-9"              | 17'-6"  | 16'-11" | N/A |
|         | NI-60  | 18'-4"            | 17'-3"  | 16'-7"  | N/A | 19'-0"              | 17'-8"  | 17'-1"  | N/A |
|         | NI-70  | 19'-6"            | 18'-0"  | 17'-4"  | N/A | 20'-1"              | 18'-7"  | 17'-9"  | N/A |
|         | NI-80  | 19'-9"            | 18'-3"  | 17'-6"  | N/A | 20'-4"              | 18'-10" | 17'-11" | N/A |
|         | NI-90x | 20'-4"            | 18'-9"  | 17'-11" | N/A | 20'-10"             | 19'-3"  | 18'-5"  | N/A |
| 14"     | NI-40x | 20'-1"            | 18'-7"  | 17'-10" | N/A | 20'-10"             | 19'-4"  | 18'-6"  | N/A |
|         | NI-60  | 20'-5"            | 18'-11" | 18'-1"  | N/A | 21'-2"              | 19'-7"  | 18'-9"  | N/A |
|         | NI-70  | 21'-7"            | 20'-0"  | 19'-1"  | N/A | 22'-3"              | 20'-7"  | 19'-8"  | N/A |
|         | NI-80  | 21'-11"           | 20'-3"  | 19'-4"  | N/A | 22'-7"              | 20'-11" | 20'-0"  | N/A |
|         | NI-90x | 22'-7"            | 20'-11" | 19'-11" | N/A | 23'-3"              | 21'-6"  | 20'-6"  | N/A |
| 16"     | NI-60  | 22'-3"            | 20'-8"  | 19'-9"  | N/A | 23'-1"              | 21'-5"  | 20'-6"  | N/A |
|         | NI-70  | 23'-6"            | 21'-9"  | 20'-9"  | N/A | 24'-3"              | 22'-5"  | 21'-5"  | N/A |
|         | NI-80  | 23'-11"           | 22'-1"  | 21'-1"  | N/A | 24'-8"              | 22'-10" | 21'-9"  | N/A |
|         | NI-90x | 24'-8"            | 22'-9"  | 21'-9"  | N/A | 25'-4"              | 23'-5"  | 22'-4"  | N/A |

| Depth   | Series | Mid-Span Blocking |         |         |     | Mid-Span Blocking and 1/2" Gypsum Ceiling |         |         |     |
|---------|--------|-------------------|---------|---------|-----|-------------------------------------------|---------|---------|-----|
|         |        | On Centre Spacing |         |         |     | On Centre Spacing                         |         |         |     |
|         |        | 12"               | 16"     | 19.2"   | 24" | 12"                                       | 16"     | 19.2"   | 24" |
| 9-1/2"  | NI-20  | 16'-8"            | 15'-3"  | 14'-5"  | N/A | 16'-8"                                    | 15'-3"  | 14'-5"  | N/A |
|         | NI-40x | 17'-11"           | 16'-11" | 16'-1"  | N/A | 18'-5"                                    | 17'-1"  | 16'-1"  | N/A |
|         | NI-60  | 18'-2"            | 17'-1"  | 16'-4"  | N/A | 18'-7"                                    | 17'-4"  | 16'-4"  | N/A |
|         | NI-70  | 19'-2"            | 17'-10" | 17'-2"  | N/A | 19'-7"                                    | 18'-3"  | 17'-7"  | N/A |
|         | NI-80  | 19'-5"            | 18'-0"  | 17'-4"  | N/A | 19'-10"                                   | 18'-5"  | 17'-8"  | N/A |
| 11-7/8" | NI-20  | 19'-6"            | 18'-1"  | 17'-3"  | N/A | 19'-11"                                   | 18'-3"  | 17'-3"  | N/A |
|         | NI-40x | 21'-0"            | 19'-6"  | 18'-8"  | N/A | 21'-7"                                    | 20'-2"  | 19'-2"  | N/A |
|         | NI-60  | 21'-4"            | 19'-9"  | 18'-11" | N/A | 21'-11"                                   | 20'-4"  | 19'-6"  | N/A |
|         | NI-70  | 22'-6"            | 20'-10" | 19'-11" | N/A | 23'-0"                                    | 21'-5"  | 20'-5"  | N/A |
|         | NI-80  | 22'-9"            | 21'-1"  | 20'-1"  | N/A | 23'-3"                                    | 21'-7"  | 20'-8"  | N/A |
|         | NI-90x | 23'-4"            | 21'-8"  | 20'-8"  | N/A | 23'-10"                                   | 22'-2"  | 21'-2"  | N/A |
| 14"     | NI-40x | 23'-7"            | 21'-11" | 20'-11" | N/A | 24'-3"                                    | 22'-7"  | 21'-7"  | N/A |
|         | NI-60  | 24'-0"            | 22'-3"  | 21'-3"  | N/A | 24'-8"                                    | 22'-11" | 21'-11" | N/A |
|         | NI-70  | 25'-3"            | 23'-4"  | 22'-3"  | N/A | 25'-10"                                   | 24'-0"  | 22'-11" | N/A |
|         | NI-80  | 25'-7"            | 23'-8"  | 22'-7"  | N/A | 26'-2"                                    | 24'-4"  | 23'-2"  | N/A |
|         | NI-90x | 26'-4"            | 24'-4"  | 23'-3"  | N/A | 26'-10"                                   | 24'-11" | 23'-9"  | N/A |
| 16"     | NI-60  | 26'-5"            | 24'-6"  | 23'-4"  | N/A | 27'-2"                                    | 25'-3"  | 24'-2"  | N/A |
|         | NI-70  | 27'-9"            | 25'-8"  | 24'-6"  | N/A | 28'-5"                                    | 26'-5"  | 25'-2"  | N/A |
|         | NI-80  | 28'-2"            | 26'-1"  | 24'-10" | N/A | 28'-10"                                   | 26'-9"  | 25'-6"  | N/A |
|         | NI-90x | 29'-0"            | 26'-10" | 25'-7"  | N/A | 29'-7"                                    | 27'-5"  | 26'-2"  | N/A |

- Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 15 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/480 and a total load deflection limit of L/240.
- Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 5/8 inch for a joist spacing of 19.2 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.
- Minimum bearing length shall be 1-3/4 inches for the end bearings.
- Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.
- This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA O86-09, NBC 2010, and OBC 2012.
- Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-1274C.

RECEIVED  
2014-01-18 / Page 1 of 1  
Per: jocelyn.aguiar



## Maximum Floor Spans

Live Load = 40 psf, Dead Load = 15 psf  
Simple Spans, L/480 Deflection Limit  
3/4" OSB G&N Sheathing

| Depth   | Series | Bare              |         |         |         | 1/2" Gypsum Ceiling |         |         |         |
|---------|--------|-------------------|---------|---------|---------|---------------------|---------|---------|---------|
|         |        | On Centre Spacing |         |         |         | On Centre Spacing   |         |         |         |
|         |        | 12"               | 16"     | 19.2"   | 24"     | 12"                 | 16"     | 19.2"   | 24"     |
| 9-1/2"  | NI-20  | 15'-10"           | 15'-0"  | 14'-5"  | 13'-5"  | 16'-4"              | 15'-5"  | 14'-6"  | 13'-5"  |
|         | NI-40x | 17'-0"            | 16'-0"  | 15'-5"  | 14'-9"  | 17'-5"              | 16'-5"  | 15'-10" | 15'-2"  |
|         | NI-60  | 17'-2"            | 16'-2"  | 15'-7"  | 14'-11" | 17'-6"              | 16'-7"  | 15'-11" | 15'-3"  |
|         | NI-70  | 18'-0"            | 16'-11" | 16'-3"  | 15'-7"  | 18'-5"              | 17'-3"  | 16'-7"  | 15'-11" |
|         | NI-80  | 18'-3"            | 17'-1"  | 16'-5"  | 15'-9"  | 18'-8"              | 17'-5"  | 16'-9"  | 16'-1"  |
| 11-7/8" | NI-20  | 17'-10"           | 16'-10" | 16'-2"  | 15'-6"  | 18'-6"              | 17'-4"  | 16'-9"  | 16'-1"  |
|         | NI-40x | 19'-4"            | 17'-11" | 17'-3"  | 16'-6"  | 19'-11"             | 18'-6"  | 17'-9"  | 17'-0"  |
|         | NI-60  | 19'-7"            | 18'-2"  | 17'-5"  | 16'-9"  | 20'-2"              | 18'-9"  | 17'-11" | 17'-2"  |
|         | NI-70  | 20'-9"            | 19'-2"  | 18'-3"  | 17'-5"  | 21'-4"              | 19'-9"  | 18'-10" | 17'-10" |
|         | NI-80  | 21'-1"            | 19'-5"  | 18'-6"  | 17'-7"  | 21'-7"              | 20'-0"  | 19'-0"  | 18'-0"  |
| 14"     | NI-90x | 21'-8"            | 20'-0"  | 19'-1"  | 18'-0"  | 22'-2"              | 20'-6"  | 19'-6"  | 18'-6"  |
|         | NI-40x | 21'-5"            | 19'-10" | 18'-11" | 17'-11" | 22'-1"              | 20'-6"  | 19'-7"  | 18'-7"  |
|         | NI-60  | 21'-10"           | 20'-2"  | 19'-3"  | 18'-2"  | 22'-5"              | 20'-10" | 19'-11" | 18'-10" |
|         | NI-70  | 23'-0"            | 21'-3"  | 20'-3"  | 19'-2"  | 23'-8"              | 21'-11" | 20'-10" | 19'-9"  |
|         | NI-80  | 23'-5"            | 21'-7"  | 20'-7"  | 19'-5"  | 24'-0"              | 22'-3"  | 21'-2"  | 20'-0"  |
| 16"     | NI-90x | 24'-1"            | 22'-3"  | 21'-2"  | 20'-0"  | 24'-8"              | 22'-10" | 21'-9"  | 20'-7"  |
|         | NI-60  | 23'-9"            | 22'-0"  | 20'-11" | 19'-10" | 24'-6"              | 22'-9"  | 21'-8"  | 20'-6"  |
|         | NI-70  | 25'-1"            | 23'-2"  | 22'-0"  | 20'-10" | 25'-9"              | 23'-10" | 22'-9"  | 21'-6"  |
|         | NI-80  | 25'-6"            | 23'-6"  | 22'-4"  | 21'-2"  | 26'-1"              | 24'-2"  | 23'-1"  | 21'-10" |
|         | NI-90x | 26'-4"            | 24'-3"  | 23'-1"  | 21'-10" | 26'-11"             | 24'-11" | 23'-8"  | 22'-5"  |

| Depth   | Series | Mid-Span Blocking |         |         |         | Mid-Span Blocking and 1/2" Gypsum Ceiling |         |         |         |
|---------|--------|-------------------|---------|---------|---------|-------------------------------------------|---------|---------|---------|
|         |        | On Centre Spacing |         |         |         | On Centre Spacing                         |         |         |         |
|         |        | 12"               | 16"     | 19.2"   | 24"     | 12"                                       | 16"     | 19.2"   | 24"     |
| 9-1/2"  | NI-20  | 16'-10"           | 15'-5"  | 14'-6"  | 13'-5"  | 16'-10"                                   | 15'-5"  | 14'-6"  | 13'-5"  |
|         | NI-40x | 18'-8"            | 17'-2"  | 16'-3"  | 15'-2"  | 18'-10"                                   | 17'-2"  | 16'-3"  | 15'-2"  |
|         | NI-60  | 18'-11"           | 17'-6"  | 16'-6"  | 15'-5"  | 19'-2"                                    | 17'-6"  | 16'-6"  | 15'-5"  |
|         | NI-70  | 20'-0"            | 18'-7"  | 17'-9"  | 16'-7"  | 20'-5"                                    | 18'-11" | 17'-10" | 16'-7"  |
|         | NI-80  | 20'-3"            | 18'-10" | 17'-11" | 16'-10" | 20'-8"                                    | 19'-3"  | 18'-2"  | 16'-10" |
| 11-7/8" | NI-20  | 20'-1"            | 18'-5"  | 17'-5"  | 16'-2"  | 20'-1"                                    | 18'-5"  | 17'-5"  | 16'-2"  |
|         | NI-40x | 21'-10"           | 20'-4"  | 19'-4"  | 17'-8"  | 22'-5"                                    | 20'-6"  | 19'-4"  | 17'-8"  |
|         | NI-60  | 22'-1"            | 20'-7"  | 19'-7"  | 18'-4"  | 22'-8"                                    | 20'-10" | 19'-8"  | 18'-4"  |
|         | NI-70  | 23'-4"            | 21'-8"  | 20'-8"  | 19'-7"  | 23'-10"                                   | 22'-3"  | 21'-2"  | 19'-9"  |
|         | NI-80  | 23'-7"            | 21'-11" | 20'-11" | 19'-9"  | 24'-1"                                    | 22'-6"  | 21'-5"  | 20'-0"  |
| 14"     | NI-90x | 24'-3"            | 22'-6"  | 21'-6"  | 20'-4"  | 24'-8"                                    | 23'-0"  | 22'-0"  | 20'-9"  |
|         | NI-40x | 24'-5"            | 22'-9"  | 21'-8"  | 19'-5"  | 25'-1"                                    | 23'-2"  | 21'-9"  | 19'-5"  |
|         | NI-60  | 24'-10"           | 23'-1"  | 22'-0"  | 20'-10" | 25'-6"                                    | 23'-8"  | 22'-4"  | 20'-10" |
|         | NI-70  | 26'-1"            | 24'-3"  | 23'-2"  | 21'-10" | 26'-8"                                    | 24'-11" | 23'-9"  | 22'-4"  |
|         | NI-80  | 26'-6"            | 24'-7"  | 23'-5"  | 22'-2"  | 27'-1"                                    | 25'-3"  | 24'-1"  | 22'-9"  |
| 16"     | NI-90x | 27'-3"            | 25'-4"  | 24'-1"  | 22'-9"  | 27'-9"                                    | 25'-11" | 24'-8"  | 23'-4"  |
|         | NI-60  | 27'-3"            | 25'-5"  | 24'-2"  | 22'-10" | 28'-0"                                    | 26'-2"  | 24'-9"  | 23'-1"  |
|         | NI-70  | 28'-8"            | 26'-8"  | 25'-4"  | 23'-11" | 29'-3"                                    | 27'-4"  | 26'-1"  | 24'-8"  |
|         | NI-80  | 29'-1"            | 27'-0"  | 25'-9"  | 24'-4"  | 29'-8"                                    | 27'-9"  | 26'-5"  | 25'-0"  |
|         | NI-90x | 29'-11"           | 27'-10" | 26'-6"  | 25'-0"  | 30'-6"                                    | 28'-5"  | 27'-2"  | 25'-8"  |

1. Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 15 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/480 and a total load deflection limit of L/240.

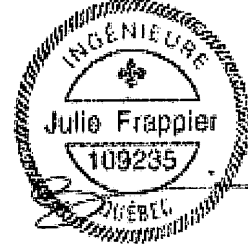
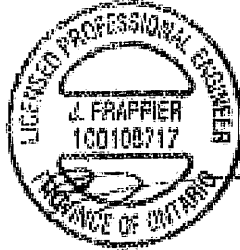
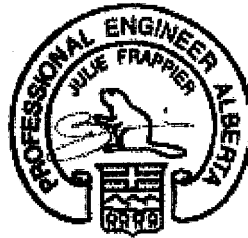
2. Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 3/4 inch for a joist spacing of 24 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.

3. Minimum bearing length shall be 1-3/4 inches for the end bearings.

4. Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.

5. This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA O86-09, NBC 2010, and OBC 2012.

6. Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.



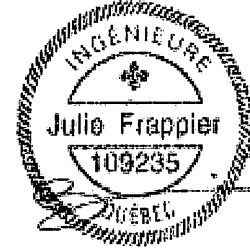
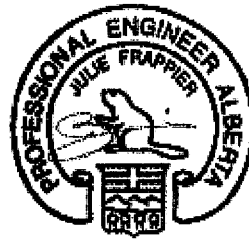
## Maximum Floor Spans

Live Load = 40 psf, Dead Load = 30 psf  
Simple Spans, L/480 Deflection Limit  
5/8" OSB G&N Sheathing

| Depth   | Series | Bare              |         |         |     | 1/2" Gypsum Ceiling |         |         |     |
|---------|--------|-------------------|---------|---------|-----|---------------------|---------|---------|-----|
|         |        | On Centre Spacing |         |         |     | On Centre Spacing   |         |         |     |
|         |        | 12"               | 16"     | 19.2"   | 24" | 12"                 | 16"     | 19.2"   | 24" |
| 9-1/2"  | NI-20  | 15'-1"            | 14'-1"  | 13'-3"  | N/A | 15'-7"              | 14'-1"  | 13'-3"  | N/A |
|         | NI-40x | 16'-1"            | 15'-2"  | 14'-8"  | N/A | 16'-7"              | 15'-7"  | 15'-1"  | N/A |
|         | NI-60  | 16'-3"            | 15'-4"  | 14'-10" | N/A | 16'-8"              | 15'-9"  | 15'-3"  | N/A |
|         | NI-70  | 17'-1"            | 16'-1"  | 15'-6"  | N/A | 17'-5"              | 16'-5"  | 15'-10" | N/A |
|         | NI-80  | 17'-3"            | 16'-3"  | 15'-8"  | N/A | 17'-8"              | 16'-7"  | 16'-0"  | N/A |
| 11-7/8" | NI-20  | 16'-11"           | 16'-0"  | 15'-5"  | N/A | 17'-6"              | 16'-6"  | 16'-0"  | N/A |
|         | NI-40x | 18'-1"            | 17'-0"  | 16'-5"  | N/A | 18'-9"              | 17'-6"  | 16'-11" | N/A |
|         | NI-60  | 18'-4"            | 17'-3"  | 16'-7"  | N/A | 19'-0"              | 17'-8"  | 17'-1"  | N/A |
|         | NI-70  | 19'-6"            | 18'-0"  | 17'-4"  | N/A | 20'-1"              | 18'-7"  | 17'-9"  | N/A |
|         | NI-80  | 19'-9"            | 18'-3"  | 17'-6"  | N/A | 20'-4"              | 18'-10" | 17'-11" | N/A |
| 14"     | NI-90x | 20'-4"            | 18'-9"  | 17'-11" | N/A | 20'-10"             | 19'-3"  | 18'-5"  | N/A |
|         | NI-40x | 20'-1"            | 18'-7"  | 17'-10" | N/A | 20'-10"             | 19'-4"  | 18'-6"  | N/A |
|         | NI-60  | 20'-5"            | 18'-11" | 18'-1"  | N/A | 21'-2"              | 19'-7"  | 18'-9"  | N/A |
|         | NI-70  | 21'-7"            | 20'-0"  | 19'-1"  | N/A | 22'-3"              | 20'-7"  | 19'-8"  | N/A |
|         | NI-80  | 21'-11"           | 20'-3"  | 19'-4"  | N/A | 22'-7"              | 20'-11" | 20'-0"  | N/A |
| 16"     | NI-90x | 22'-7"            | 20'-11" | 19'-11" | N/A | 23'-3"              | 21'-6"  | 20'-6"  | N/A |
|         | NI-60  | 22'-3"            | 20'-8"  | 19'-9"  | N/A | 23'-1"              | 21'-5"  | 20'-6"  | N/A |
|         | NI-70  | 23'-6"            | 21'-9"  | 20'-9"  | N/A | 24'-3"              | 22'-5"  | 21'-5"  | N/A |
|         | NI-80  | 23'-11"           | 22'-1"  | 21'-1"  | N/A | 24'-8"              | 22'-10" | 21'-9"  | N/A |
|         | NI-90x | 24'-8"            | 22'-9"  | 21'-9"  | N/A | 25'-4"              | 23'-5"  | 22'-4"  | N/A |

| Depth   | Series | Mid-Span Blocking |         |         |     | Mid-Span Blocking and 1/2" Gypsum Ceiling |         |        |     |
|---------|--------|-------------------|---------|---------|-----|-------------------------------------------|---------|--------|-----|
|         |        | On Centre Spacing |         |         |     | On Centre Spacing                         |         |        |     |
|         |        | 12"               | 16"     | 19.2"   | 24" | 12"                                       | 16"     | 19.2"  | 24" |
| 9-1/2"  | NI-20  | 15'-7"            | 14'-1"  | 13'-3"  | N/A | 15'-7"                                    | 14'-1"  | 13'-3" | N/A |
|         | NI-40x | 17'-9"            | 16'-1"  | 15'-1"  | N/A | 17'-9"                                    | 16'-1"  | 15'-1" | N/A |
|         | NI-60  | 18'-1"            | 16'-4"  | 15'-4"  | N/A | 18'-1"                                    | 16'-4"  | 15'-4" | N/A |
|         | NI-70  | 19'-2"            | 17'-10" | 16'-9"  | N/A | 19'-7"                                    | 17'-10" | 16'-9" | N/A |
|         | NI-80  | 19'-5"            | 18'-0"  | 17'-1"  | N/A | 19'-10"                                   | 18'-3"  | 17'-1" | N/A |
| 11-7/8" | NI-20  | 18'-9"            | 17'-0"  | 16'-0"  | N/A | 18'-9"                                    | 17'-0"  | 16'-0" | N/A |
|         | NI-40x | 21'-0"            | 19'-3"  | 17'-9"  | N/A | 21'-3"                                    | 19'-3"  | 17'-9" | N/A |
|         | NI-60  | 21'-4"            | 19'-8"  | 18'-5"  | N/A | 21'-8"                                    | 19'-8"  | 18'-5" | N/A |
|         | NI-70  | 22'-6"            | 20'-10" | 19'-11" | N/A | 23'-0"                                    | 21'-4"  | 20'-0" | N/A |
|         | NI-80  | 22'-9"            | 21'-1"  | 20'-1"  | N/A | 23'-3"                                    | 21'-7"  | 20'-5" | N/A |
| 14"     | NI-90x | 23'-4"            | 21'-8"  | 20'-8"  | N/A | 23'-10"                                   | 22'-2"  | 21'-2" | N/A |
|         | NI-40x | 23'-7"            | 21'-5"  | 19'-6"  | N/A | 24'-1"                                    | 21'-5"  | 19'-6" | N/A |
|         | NI-60  | 24'-0"            | 22'-3"  | 21'-0"  | N/A | 24'-8"                                    | 22'-5"  | 21'-0" | N/A |
|         | NI-70  | 25'-3"            | 23'-4"  | 22'-3"  | N/A | 25'-10"                                   | 24'-0"  | 22'-9" | N/A |
|         | NI-80  | 25'-7"            | 23'-8"  | 22'-7"  | N/A | 26'-2"                                    | 24'-4"  | 23'-2" | N/A |
| 16"     | NI-90x | 26'-4"            | 24'-4"  | 23'-3"  | N/A | 26'-10"                                   | 24'-11" | 23'-9" | N/A |
|         | NI-60  | 26'-5"            | 24'-6"  | 23'-4"  | N/A | 27'-2"                                    | 24'-10" | 23'-4" | N/A |
|         | NI-70  | 27'-9"            | 25'-8"  | 24'-6"  | N/A | 28'-5"                                    | 26'-5"  | 25'-2" | N/A |
|         | NI-80  | 28'-2"            | 26'-1"  | 24'-10" | N/A | 28'-10"                                   | 26'-9"  | 25'-6" | N/A |
|         | NI-90x | 29'-0"            | 26'-10" | 25'-7"  | N/A | 29'-7"                                    | 27'-5"  | 26'-2" | N/A |

- Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 30 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/480 and a total load deflection limit of L/240.
- Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 5/8 inch for a joist spacing of 19.2 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.
- Minimum bearing length shall be 1-3/4 inches for the end bearings.
- Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.
- This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA O86-09, NBC 2010, and OBC 2012.
- Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.



## Maximum Floor Spans

Live Load = 40 psf, Dead Load = 30 psf  
Simple Spans, L/480 Deflection Limit  
3/4" OSB G&N Sheathing

| Depth   | Series | Bare              |         |         |         | 1/2" Gypsum Ceiling |         |         |         |
|---------|--------|-------------------|---------|---------|---------|---------------------|---------|---------|---------|
|         |        | On Centre Spacing |         |         |         | On Centre Spacing   |         |         |         |
|         |        | 12"               | 16"     | 19.2"   | 24"     | 12"                 | 16"     | 19.2"   | 24"     |
| 9-1/2"  | NI-20  | 15'-7"            | 14'-2"  | 13'-4"  | 12'-4"  | 15'-7"              | 14'-2"  | 13'-4"  | 12'-4"  |
|         | NI-40x | 17'-0"            | 16'-0"  | 15'-1"  | 13'-11" | 17'-5"              | 16'-1"  | 15'-1"  | 13'-11" |
|         | NI-60  | 17'-2"            | 16'-2"  | 15'-5"  | 14'-3"  | 17'-6"              | 16'-5"  | 15'-5"  | 14'-3"  |
|         | NI-70  | 18'-0"            | 16'-11" | 16'-3"  | 15'-6"  | 18'-5"              | 17'-3"  | 16'-7"  | 15'-6"  |
|         | NI-80  | 18'-3"            | 17'-1"  | 16'-5"  | 15'-9"  | 18'-8"              | 17'-5"  | 16'-9"  | 15'-10" |
| 11-7/8" | NI-20  | 17'-10"           | 16'-10" | 16'-0"  | 14'-10" | 18'-6"              | 17'-1"  | 16'-0"  | 14'-10" |
|         | NI-40x | 19'-4"            | 17'-11" | 17'-3"  | 15'-10" | 19'-11"             | 18'-6"  | 17'-9"  | 15'-10" |
|         | NI-60  | 19'-7"            | 18'-2"  | 17'-5"  | 16'-9"  | 20'-2"              | 18'-9"  | 17'-11" | 17'-1"  |
|         | NI-70  | 20'-9"            | 19'-2"  | 18'-3"  | 17'-5"  | 21'-4"              | 19'-9"  | 18'-10" | 17'-10" |
|         | NI-80  | 21'-1"            | 19'-5"  | 18'-6"  | 17'-7"  | 21'-7"              | 20'-0"  | 19'-0"  | 18'-0"  |
| 14"     | NI-90x | 21'-8"            | 20'-0"  | 19'-1"  | 18'-0"  | 22'-2"              | 20'-6"  | 19'-6"  | 18'-6"  |
|         | NI-40x | 21'-5"            | 19'-10" | 18'-11" | 17'-5"  | 22'-1"              | 20'-6"  | 19'-6"  | 17'-5"  |
|         | NI-60  | 21'-10"           | 20'-2"  | 19'-3"  | 18'-2"  | 22'-5"              | 20'-10" | 19'-11" | 18'-10" |
|         | NI-70  | 23'-0"            | 21'-3"  | 20'-3"  | 19'-2"  | 23'-8"              | 21'-11" | 20'-10" | 19'-9"  |
|         | NI-80  | 23'-5"            | 21'-7"  | 20'-7"  | 19'-5"  | 24'-0"              | 22'-3"  | 21'-2"  | 20'-0"  |
| 16"     | NI-90x | 24'-1"            | 22'-3"  | 21'-2"  | 20'-0"  | 24'-8"              | 22'-10" | 21'-9"  | 20'-7"  |
|         | NI-60  | 23'-9"            | 22'-0"  | 20'-11" | 19'-10" | 24'-6"              | 22'-9"  | 21'-8"  | 20'-6"  |
|         | NI-70  | 25'-1"            | 23'-2"  | 22'-0"  | 20'-10" | 25'-9"              | 23'-10" | 22'-9"  | 21'-6"  |
|         | NI-80  | 25'-6"            | 23'-6"  | 22'-4"  | 21'-2"  | 26'-1"              | 24'-2"  | 23'-1"  | 21'-10" |
|         | NI-90x | 26'-4"            | 24'-3"  | 23'-1"  | 21'-10" | 26'-11"             | 24'-11" | 23'-8"  | 22'-5"  |

| Depth   | Series | Mid-Span Blocking |         |        |         | Mid-Span Blocking and 1/2" Gypsum Ceiling |         |         |         |
|---------|--------|-------------------|---------|--------|---------|-------------------------------------------|---------|---------|---------|
|         |        | On Centre Spacing |         |        |         | On Centre Spacing                         |         |         |         |
|         |        | 12"               | 16"     | 19.2"  | 24"     | 12"                                       | 16"     | 19.2"   | 24"     |
| 9-1/2"  | NI-20  | 15'-7"            | 14'-2"  | 13'-4" | 12'-4"  | 15'-7"                                    | 14'-2"  | 13'-4"  | 12'-4"  |
|         | NI-40x | 17'-9"            | 16'-1"  | 15'-1" | 13'-11" | 17'-9"                                    | 16'-1"  | 15'-1"  | 13'-11" |
|         | NI-60  | 18'-1"            | 16'-5"  | 15'-5" | 14'-3"  | 18'-1"                                    | 16'-5"  | 15'-5"  | 14'-3"  |
|         | NI-70  | 19'-10"           | 17'-11" | 16'-9" | 15'-6"  | 19'-10"                                   | 17'-11" | 16'-9"  | 15'-6"  |
|         | NI-80  | 20'-2"            | 18'-3"  | 17'-1" | 15'-10" | 20'-2"                                    | 18'-3"  | 17'-1"  | 15'-10" |
| 11-7/8" | NI-20  | 18'-10"           | 17'-1"  | 16'-0" | 14'-10" | 18'-10"                                   | 17'-1"  | 16'-0"  | 14'-10" |
|         | NI-40x | 21'-3"            | 19'-3"  | 17'-9" | 15'-10" | 21'-3"                                    | 19'-3"  | 17'-9"  | 15'-10" |
|         | NI-60  | 21'-9"            | 19'-8"  | 18'-5" | 17'-1"  | 21'-9"                                    | 19'-8"  | 18'-5"  | 17'-1"  |
|         | NI-70  | 23'-4"            | 21'-5"  | 20'-1" | 18'-6"  | 23'-8"                                    | 21'-5"  | 20'-1"  | 18'-6"  |
|         | NI-80  | 23'-7"            | 21'-10" | 20'-5" | 18'-11" | 24'-1"                                    | 21'-10" | 20'-5"  | 18'-11" |
| 14"     | NI-90x | 24'-3"            | 22'-6"  | 21'-3" | 19'-7"  | 24'-8"                                    | 22'-7"  | 21'-3"  | 19'-7"  |
|         | NI-40x | 24'-2"            | 21'-5"  | 19'-6" | 17'-5"  | 24'-2"                                    | 21'-5"  | 19'-6"  | 17'-5"  |
|         | NI-60  | 24'-9"            | 22'-5"  | 21'-0" | 19'-6"  | 24'-9"                                    | 22'-5"  | 21'-0"  | 19'-6"  |
|         | NI-70  | 26'-1"            | 24'-3"  | 22'-9" | 21'-0"  | 26'-8"                                    | 24'-3"  | 22'-9"  | 21'-0"  |
|         | NI-80  | 26'-6"            | 24'-7"  | 23'-3" | 21'-6"  | 27'-1"                                    | 24'-10" | 23'-3"  | 21'-6"  |
| 16"     | NI-90x | 27'-3"            | 25'-4"  | 24'-1" | 22'-4"  | 27'-9"                                    | 25'-10" | 24'-3"  | 22'-4"  |
|         | NI-60  | 27'-3"            | 24'-11" | 23'-5" | 21'-7"  | 27'-6"                                    | 24'-11" | 23'-5"  | 21'-7"  |
|         | NI-70  | 28'-8"            | 26'-8"  | 25'-3" | 23'-4"  | 29'-3"                                    | 26'-11" | 25'-3"  | 23'-4"  |
|         | NI-80  | 29'-1"            | 27'-0"  | 25'-9" | 23'-10" | 29'-8"                                    | 27'-6"  | 25'-10" | 23'-10" |
|         | NI-90x | 29'-11"           | 27'-10" | 26'-6" | 24'-10" | 30'-6"                                    | 28'-5"  | 26'-11" | 24'-10" |

- Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 30 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/480 and a total load deflection limit of L/240.
- Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 3/4 inch for a joist spacing of 24 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.
- Minimum bearing length shall be 1-3/4 inches for the end bearings.
- Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.
- This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA O86-09, NBC 2010, and OBC 2012.
- Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.



## Construction Detail

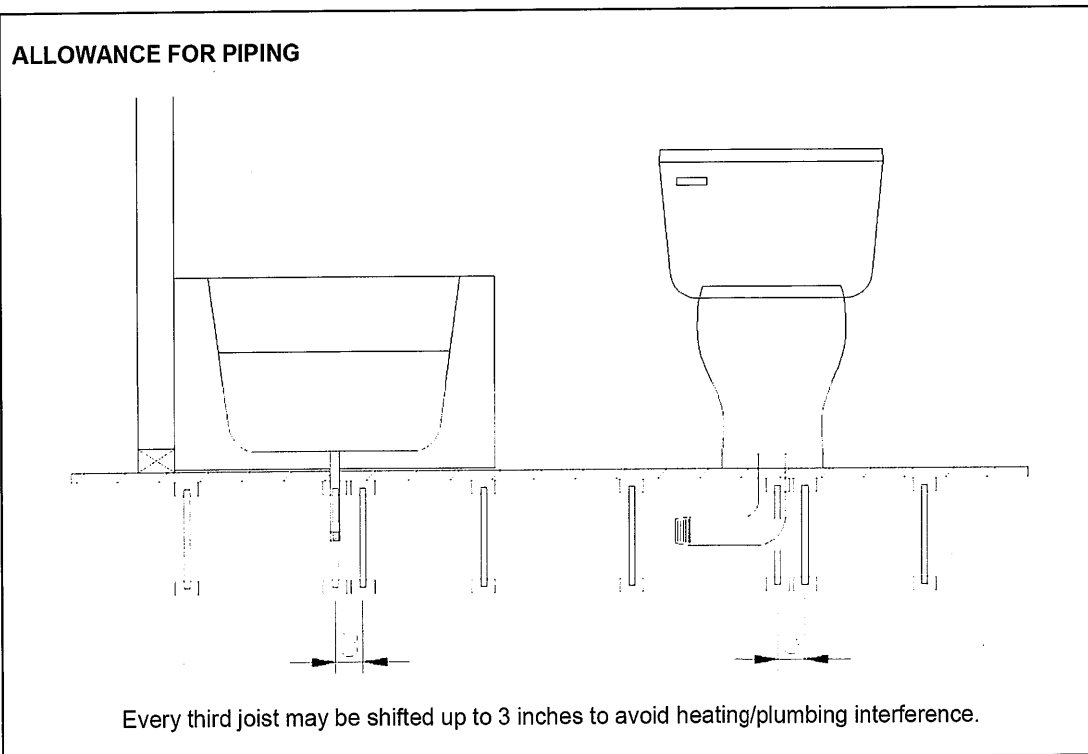
Limit States Design

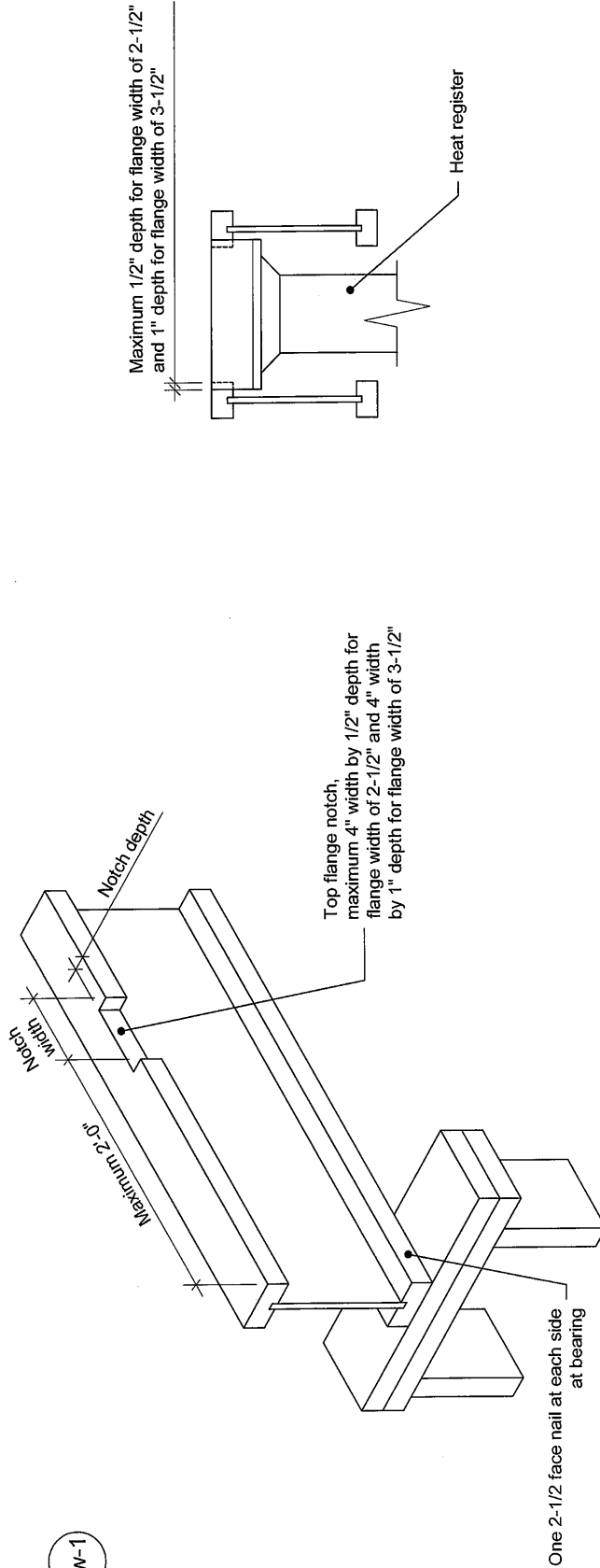
### Allowance for Piping (Installation Notes)

The floor layouts have usually not been checked for heating and/or plumbing interference. On-site adjustment of joists of up to 3 inches is permitted to avoid interferences. When moving a joist, the subfloor thickness shall be checked with code requirements when the joist spacing exceeds 19.2 inches. Except for cutting to length, I-joist flanges should never be cut, drilled, or notched.

Installation of Nordic I-joists shall be as per *Nordic Joist Installation Guide for Residential Floors*. Refer to Tables 1 and 2 for maximum web hole and duct chase openings, respectively. These tables are based on the I-joists being used at their maximum spans. The minimum distance given may be reduced for shorter spans; contact your distributor for additional information.

The detail below shows the 3-inch allowance for piping. Every third joist may be shifted up to 3 inches to avoid heating/plumbing interference. For other applications, please contact your distributor.





**Notes:**

1. Blocking required at bearing for lateral support, not shown for clarity.
2. The maximum dimensions for a notch on the side of the top flange are 4-inch width by 1/2-inch depth for flange width of 2-1/2 inches, and 4-inch width by 1-inch depth for flange width of 3-1/2 inches.
3. This detail applies to simple-span joists and multiple-span joists where the notch is located at the end half-span.
4. For other applications, contact Nordic Structures.

This document supersedes all previous versions. If the document has been in effect for more than one year, consult [nordic.ca](http://nordic.ca) or contact Nordic Structures.

All nails shown in the details are assumed to be common nails unless otherwise noted. Nails shall have a diameter not less than 0.128 inch for 2-1/2-inch nails, or 0.144 inch for 3-inch nails. Individual components not shown to scale for clarity.

|                          |  |                                                                              |                                                          |            |                |
|--------------------------|--|------------------------------------------------------------------------------|----------------------------------------------------------|------------|----------------|
| <b>NORDIC STRUCTURES</b> |  | T 514-871-8526<br>1 866 817-3418<br><a href="http://nordic.ca">nordic.ca</a> | Notch in I-joist for Heat Register                       | -          | DOCUMENT       |
|                          |  |                                                                              | 1-joist - Typical Floor Framing and Construction Details | 2018-04-10 | NUMBER<br>1W-1 |

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