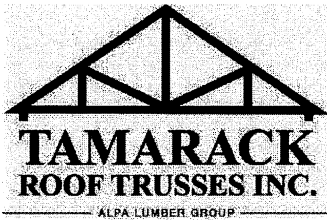




# DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER  
 Builder: ROYAL PINE HOMES  
 Project: CENTREFIELD  
 Location: RICHMOND HILL  
 Model: BLOCK 58  
 Lot #:   
 Elevation: B / UNIT21BLK284

Job Track: 51012  
 PlanLog: 203549  
 Layout ID: 413119  
 Ref #  
 Page: 1 of 2  
 Date: 07-08-2021  
 Designer: Andrew Conway  
 Sales Rep: Mario DiCano

## Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	T10G GABLE	6 / 12	22-08-08	8-01-04	2 x 4	1-05-00	1-02-00 8-01-04	243.75 153.67		
	1 2-ply	T101 Half Hip Girder	6 / 12	19-05-08	4-01-04	2 x 4 2 x 6	1-03-08	1-02-00 4-01-04	182.72 116.00		
	1 2-ply	T101Z Half Hip Girder	6 / 12	19-05-08	4-01-04	2 x 4 2 x 6	1-03-08	1-02-00 4-01-04	182.72 116.00		
	2	T102 Half Hip	6 / 12	19-05-08	5-01-04	2 x 4	1-03-08	1-02-00 5-01-04	159.13 101.33		
	2	T103 Half Hip	6 / 12	19-05-08	6-01-04	2 x 4	1-03-08	1-02-00 6-01-04	172.24 109.67		
	2	T104 Half Hip	6 / 12	19-05-08	7-01-04	2 x 4	1-03-08	1-02-00 7-01-04	173.04 108.00		
	8	T105 Half Hip	6 / 12	19-05-08	8-01-04	2 x 4	1-03-08	1-02-00 8-01-04	689.37 434.67		
	2	T106 Half Hip	6 / 12	19-05-08	9-01-04	2 x 4	1-03-08	1-02-00 9-01-04	181.02 114.00		
	1	T107 Hip Girder	6 / 12	9-06-00	2-04-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	37.9 25.50		
	2	T108 Hip	6 / 12	9-06-00	4-10-12	2 x 4	1-03-08 1-03-08	2-08-00 2-08-00	99.25 67.00		
	1	T109 Hip	6 / 12	9-06-00	3-10-12	2 x 4	1-03-08 1-03-08	2-08-00 2-08-00	45.71 31.67		
	1 2-ply	T110 Monopitch Girder	6 / 12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	58.39 37.67		
	2	PB101 Piggyback	6 / 12	5-07-00	2-00-00	2 x 4		2-00-00	32.19 22.67		
	4	PB102 Piggyback	6 / 12	5-07-00	2-09-08	2 x 4		2-09-08	58.98 40.00		

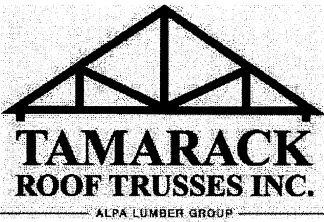
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 BUILDING DIVISION

09/02/2021

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# DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER  
 Builder: ROYAL PINE HOMES  
 Project: CENTREFIELD  
 Location: RICHMOND HILL  
 Model: BLOCK 58  
 Lot #:   
 Elevation: B / UNIT21BLK284

Job Track: 51012  
 PlanLog: 203549  
 Layout ID: 413119  
 Ref #  
 Page: 2 of 2  
 Date: 07-08-2021  
 Designer: Andrew Conway  
 Sales Rep: Mario DiCano

## Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	PB103G GABLE	6 / 12	8-10-00	2-09-08	2 x 4		2-09-08	51.18 35.33		
	11	J01 Jack-Open	6 / 12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	184.74 117.33		
	5	J03 Jack-Open	4 / 12	3-07-00	1-11-03	2 x 4	1-03-08	3-15 1-03-04	50.28 33.33		
	2	J11 Jack-Open	6 / 12	1-10-15	2-01-08	2 x 4	1-03-08 6-09	1-02-00 2-01-08	15.69 10.67		
	2	J12 Jack-Open	6 / 12	2-05-08	2-04-12	2 x 4	1-03-08	1-02-00 2-04-12	17.09 12.00		
	3	J13 Jack-Open	6 / 12	1-10-15	2-01-08	2 x 4	1-03-08 1-01	1-02-00 2-01-08	21.97 14.00		
	1	J14 Jack-Open	6 / 12	2-00-00	3-01-08	2 x 4	1-03-08 1-10-15	1-02-00 2-02-00	9.87 6.00		
	1	J15 Jack-Open	6 / 12	1-10-15	2-01-08	2 x 4	1-03-08 3-11-09	1-02-00 2-01-08	11.74 7.33		
	1	J16 Jack-Open	6 / 12	3-10-15	3-01-08	2 x 4	1-03-08 1-11-09	1-02-00 3-01-08	14.29 8.67		

TOTAL # TRUSS= 62

TOTAL BFT OF ALL TRUSSES= 1722.51

BFT.

TOTAL WEIGHT OF ALL TRSSES 2693.27 LBS

## HARDWARE

QTY	TYPE	MODEL	LENGTH
3	Hardware	LUS24	
1	Hardware	HGUS26-2	

TOTAL NUMBER OF ITEMS= 4

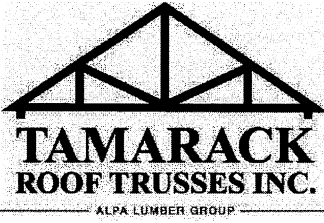
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 BUILDING DIVISION

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# DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER  
 Builder: ROYAL PINE HOMES  
 Project: CENTREFIELD  
 Location: RICHMOND HILL  
 Model: BLOCK 58  
 Lot #:   
 Elevation: B1 / UNIT20BLK284

Job Track: 51012  
 PlanLog: 203549  
 Layout ID: 413120  
 Ref #  
 Page: 1 of 2  
 Date: 07-08-2021  
 Designer: Andrew Conway  
 Sales Rep: Mario DiCano

## Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	5	T11 Piggyback Base	6 /12	45-05-00	8-01-04	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	1279.34 775.00		
	4	T11B Piggyback Base	6 /12	45-00-08	8-01-04	2 x 6	1-03-08	1-02-00 1-02-00	1007.58 617.33		
	1	T11G GABLE	6 /12	45-05-00	8-01-04	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	272.06 169.67		
	1	T11GC GABLE	6 /12	38-11-00	8-01-04	2 x 6	1-03-08	1-02-00 1-02-00	247.87 155.83		
	1	T122 Hip Girder	6 /12	8-07-00	2-04-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	35.14 23.67		
	1 3-ply	T127 Common Girder	6 /12	8-05-08	4-09-12	2 x 4 2 x 6		2-08-00 2-08-12	124.02 84.50		
	1	T141 Common	6 /12	8-07-00	3-03-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	34.58 22.17		
	2	J11 Jack-Open	6 /12	1-10-15	2-01-08	2 x 4	1-03-08 6-09	1-02-00 2-01-08	15.69 10.67		
	2	J12 Jack-Open	6 /12	2-05-08	2-04-12	2 x 4	1-03-08	1-02-00 2-04-12	17.09 12.00		
	2	J13 Jack-Open	6 /12	1-10-15	2-01-08	2 x 4	1-03-08 1-01	1-02-00 2-01-08	14.65 9.33		
	8	PB06 Piggyback	6 /12	17-08-00	4-05-00	2 x 4			410.9 256.00		
	1	PB06G GABLE	6 /12	17-08-00	4-05-00	2 x 4			50.76 31.00		
	1	PB104 Piggyback	6 /12	17-08-00	3-11-04	2 x 4			57.06 37.00		
	1	PB105G Piggyback	6 /12	8-10-00	2-08-00	2 x 4			25.27 17.67		

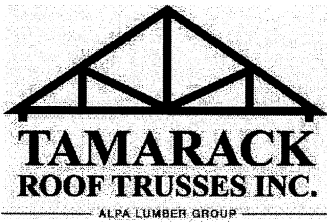
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 BUILDING DIVISION

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 <p><b>TAMARACK</b> ROOF TRUSSES INC. <small>ALFA LUMBER GROUP</small></p>	<b>DELIVERY SHIPLIST</b>			
	Lumber Yard:	TAMARACK LUMBER	Job Track:	51012
	Builder:	ROYAL PINE HOMES	PlanLog:	203549
	Project:	CENTREFIELD	Layout ID:	413120
	Location:	RICHMOND HILL	Ref #	
	Model:	BLOCK 58	Page:	2 of 2
Lot #:		Date:	07-08-2021	
Elevation:	B1 / UNIT20BLK284	Designer:	Andrew Conway	
		Sales Rep:	Mario DiCano	

### Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
TOTAL # TRUSS= 33		TOTAL BFT OF ALL TRUSSES= 2221.84					BFT.	TOTAL WEIGHT OF ALL TRSSES 3592 LBS			

### HARDWARE

QTY	TYPE	MODEL	LENGTH
4	Hardware	LJS26DS	
14	Hardware	H2.5T	
2	Hardware	LGT3	

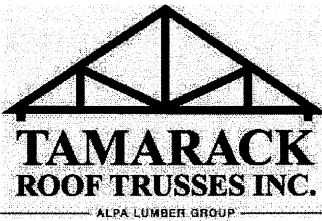
TOTAL NUMBER OF ITEMS= 20

CITY OF RICHMOND HILL  
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 <p><b>TAMARACK</b> ROOF TRUSSES INC. <small>ALPA LUMBER GROUP</small></p>	DELIVERY SHIPLIST	
	Lumber Yard: TAMARACK LUMBER Builder: ROYAL PINE HOMES Project: CENTREFIELD Location: RICHMOND HILL Model: BLOCK 58 Lot #: Elevation: B / UNIT19BLK284	Job Track: 51012 PlanLog: 203549 Layout ID: 413121 Ref # Page: 1 of 2 Date: 07-08-2021 Designer: Sales Rep: Mario DiCano

### Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	5	T11 Piggyback Base	6 /12	45-05-00	8-01-04	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	1279.34 775.00		
	4	T11B Piggyback Base	6 /12	45-00-08	8-01-04	2 x 6	1-03-08	1-02-00 1-02-00	1007.58 617.33		
	1	T11G GABLE	6 /12	45-05-00	8-01-04	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	272.06 169.67		
	1	T11GB GABLE	6 /12	37-03-00	8-01-04	2 x 6	1-03-08	1-02-00 5-03-00	236.81 147.83		
	1	T122 Hip Girder	6 /12	8-07-00	2-04-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	35.14 23.67		
	1	T123 Common	6 /12	8-03-08	3-03-12	2 x 4	1-03-08	1-02-00 1-03-12	32.19 20.83		
	1 3-ply	T127 Monopitch Girder	6 /12	8-05-08	4-09-12	2 x 4 2 x 6		2-08-00 2-08-12	124.02 84.50		
	9	PB06 Piggyback	6 /12	17-08-00	4-05-00	2 x 4			462.27 288.00		
	2	PB06G GABLE	6 /12	17-08-00	4-05-00	2 x 4			101.52 62.00		
	2	J11 Jack-Open	6 /12	1-10-15	2-01-08	2 x 4	1-03-08 6-09	1-02-00 2-01-08	15.69 10.67		
	2	J12 Jack-Open	6 /12	2-05-08	2-04-12	2 x 4	1-03-08	1-02-00 2-04-12	17.09 12.00		
	1	J13 Jack-Open	6 /12	1-10-15	2-01-08	2 x 4	1-03-08 1-01	1-02-00 2-01-08	7.32 4.67		

TOTAL # TRUSS= 32

TOTAL BFT OF ALL TRUSSES= 2216.17 BFT.

TOTAL WEIGHT OF ALL TRUSSES 3591.01 LBS

### HARDWARE

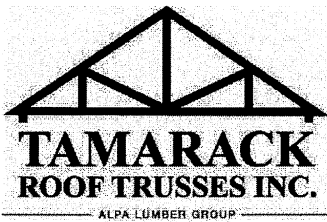
QTY	TYPE	MODEL	LENGTH
4	Hardware	LJS26DS	
14	Hardware	H2.5T	

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 <p><b>TAMARACK</b> ROOF TRUSSES INC. <small>ALPA LUMBER GROUP</small></p>	<b>DELIVERY SHIPLIST</b>	
	Lumber Yard:	TAMARACK LUMBER
	Builder:	ROYAL PINE HOMES
	Project:	CENTREFIELD
	Location:	RICHMOND HILL
	Model:	BLOCK 58
	Lot #:	
Elevation:	B / UNIT19BLK284	
Job Track:	51012	
PlanLog:	203549	
Layout ID:	413121	
Ref #		
Page:	2 of 2	
Date:	07-08-2021	
Designer:		
Sales Rep:	Mario DiCano	

## HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	LGT3	

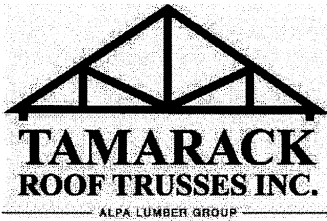
TOTAL NUMBER OF ITEMS= **20**

CITY OF RICHMOND HILL  
BUILDING DIVISION















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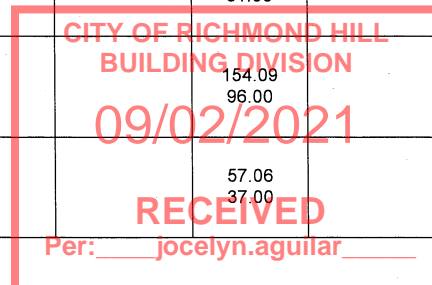
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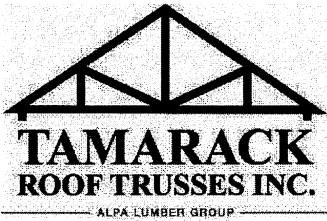
 <p><b>TAMARACK</b> ROOF TRUSSES INC. <small>ALPHA LUMBER GROUP</small></p>	DELIVERY SHIPLIST				Lumber Yard: TAMARACK LUMBER Builder: ROYAL PINE HOMES Project: CENTREFIELD Location: RICHMOND HILL Model: BLOCK 58 Lot #: Elevation: B / UNIT18BLK284		Job Track: 51012 PlanLog: 203549 Layout ID: 413122 Ref # Page: 1 of 2 Date: 07-08-2021 Designer: Andrew Conway Sales Rep: Mario DiCano	
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### Roof Trusses






PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	5	T11 Piggyback Base	6 /12	45-05-00	8-01-04	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	1279.34 775.00		
	4	T11A Piggyback Base	6 /12	38-03-00	8-01-04	2 x 6	1-03-08	1-02-00 1-02-00	886.16 540.00		
	1	T11G GABLE	6 /12	45-05-00	8-01-04	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	272.06 169.67		
	1	T11GB GABLE	6 /12	37-03-00	8-01-04	2 x 6	1-03-08	1-02-00 5-03-00	236.81 147.83		
	1 3-ply	T111 Piggyback Base Girder	6 /12	45-05-00	8-01-04	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	843.15 519.00		
	1	T117 Half Hip Girder	6 /12	8-04-08	2-04-12	2 x 4	1-03-08	1-02-00 2-04-12	34.08 22.83		
	1	T118 Half Hip	6 /12	8-04-08	4-10-12	2 x 4		2-08-00 4-10-12	39.12 26.00		
	1	T119 Half Hip	6 /12	8-04-08	5-10-12	2 x 4		2-08-00 5-10-12	45.3 29.67		
	1	T120 Monopitch	6 /12	8-04-08	6-10-04	2 x 4		2-08-00 6-10-04	41.6 27.33		
	1 3-ply	T121 Jack-Closed Girder	6 /12	8-06-08	6-11-04	2 x 4 2 x 6		2-08-00 6-11-04	141.44 94.00		
	8	PB06 Piggyback	6 /12	17-08-00	4-05-00	2 x 4			410.9 256.00		
	1	PB06G GABLE	6 /12	17-08-00	4-05-00	2 x 4			50.76 31.00		
	1 3-ply	PB06Z Piggyback	6 /12	17-08-00	4-05-00	2 x 4			154.09 96.00		
	1	PB104 Piggyback	6 /12	17-08-00	3-11-04	2 x 4			57.06 37.00		





 <p><b>TAMARACK</b> ROOF TRUSSES INC. <small>ALPHA LUMBER GROUP</small></p>	DELIVERY SHIPLIST					
	Lumber Yard: TAMARACK LUMBER Builder: ROYAL PINE HOMES Project: CENTREFIELD Location: RICHMOND HILL Model: BLOCK 58 Lot #: Elevation: B / UNIT18BLK284				Job Track: 51012 PlanLog: 203549 Layout ID: 413122 Ref # Page: 2 of 2 Date: 07-08-2021 Designer: Andrew Conway Sales Rep: Mario DiCano	

### Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	PB105G Piggyback	6 /12	8-10-00	2-08-00	2 x 4		2-09-08	25.27 17.67		
	5	J03 Jack-Open	4 /12	3-07-00	1-11-03	2 x 4	1-03-08	3-15 1-03-04	50.28 33.33		
	1	J11 Jack-Open	6 /12	1-10-15	2-01-08	2 x 4	1-03-08 6-09	1-02-00 2-01-08	7.85 5.33		
	3	J12 Jack-Open	6 /12	2-05-08	2-04-12	2 x 4	1-03-08	1-02-00 2-04-12	25.63 18.00		
	1	J13 Jack-Open	6 /12	1-10-15	2-01-08	2 x 4	1-03-08 1-01	1-02-00 2-01-08	7.32 4.67		

TOTAL # TRUSS= 45      TOTAL BFT OF ALL TRUSSES= 2850.33      BFT.      TOTAL WEIGHT OF ALL TRSSES 4608.21      LBS

### HARDWARE

QTY	TYPE	MODEL	LENGTH
3	Hardware	LUS24	
4	Hardware	LJS26DS	
1	Hardware	HGUS26-3	
14	Hardware	H2.5T	

TOTAL NUMBER OF ITEMS= 22

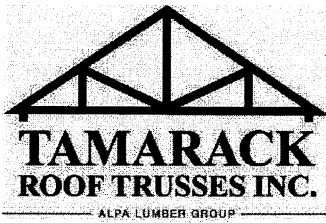
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BUILDING DIVISION

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# DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER  
 Builder: ROYAL PINE HOMES  
 Project: CENTREFIELD  
 Location: RICHMOND HILL  
 Model: BLOCK 58  
 Lot #:   
 Elevation: B / UNIT17BLK284

Job Track: 51012  
 PlanLog: 203549  
 Layout ID: 413123  
 Ref #  
 Page: 1 of 2  
 Date: 07-08-2021  
 Designer: Andrew Conway  
 Sales Rep: Mario DiCano

## Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	T10G GABLE	6 /12	22-08-08	8-01-04	2 x 4	1-05-00	1-02-00 8-01-04	243.75 153.67		
	1 2-ply	T101 Half Hip Girder	6 /12	19-05-08	4-01-04	2 x 4 2 x 6	1-03-08	1-02-00 4-01-04	182.72 116.00		
	1 2-ply	T101Z Half Hip Girder	6 /12	19-05-08	4-01-04	2 x 4 2 x 6	1-03-08	1-02-00 4-01-04	182.72 116.00		
	2	T102 Half Hip	6 /12	19-05-08	5-01-04	2 x 4	1-03-08	1-02-00 5-01-04	159.13 101.33		
	2	T103 Half Hip	6 /12	19-05-08	6-01-04	2 x 4	1-03-08	1-02-00 6-01-04	172.24 109.67		
	2	T104 Half Hip	6 /12	19-05-08	7-01-04	2 x 4	1-03-08	1-02-00 7-01-04	173.04 108.00		
	8	T105 Half Hip	6 /12	19-05-08	8-01-04	2 x 4	1-03-08	1-02-00 8-01-04	689.37 434.67		
	2	T106 Half Hip	6 /12	19-05-08	9-01-04	2 x 4	1-03-08	1-02-00 9-01-04	181.02 114.00		
	1	T107 Hip Girder	6 /12	9-06-00	2-04-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	37.9 25.50		
	2	T108 Hip	6 /12	9-06-00	4-10-12	2 x 4	1-03-08 1-03-08	2-08-00 2-08-00	99.25 67.00		
	1	T109 Hip	6 /12	9-06-00	3-10-12	2 x 4	1-03-08 1-03-08	2-08-00 2-08-00	45.71 31.67		
	1 2-ply	T110 Monopitch Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	58.39 37.67		
	2	PB101 Piggyback	6 /12	5-07-00	2-00-00	2 x 4		2-00-00	32.19 22.67		
	4	PB102 Piggyback	6 /12	5-07-00	2-09-08	2 x 4		2-09-08	58.98 40.00		

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 BUILDING DIVISION










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Per: jocelyn.aguilar

 <p><b>TAMARACK</b> ROOF TRUSSES INC. <small>ALPHA LUMBER GROUP</small></p>	DELIVERY SHIPLIST	
	<p>Lumber Yard: TAMARACK LUMBER          Builder: ROYAL PINE HOMES          Project: CENTREFIELD          Location: RICHMOND HILL          Model: BLOCK 58          Lot #:           Elevation: B / UNIT17BLK284</p>	<p>Job Track: 51012          PlanLog: 203549          Layout ID: 413123          Ref #          Page: 2 of 2          Date: 07-08-2021          Designer: Andrew Conway          Sales Rep: Mario DiCano</p>

## Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	PB103G GABLE	6 / 12	8-10-00	2-09-08	2 x 4		2-09-08	51.18 35.33		
	11	J01 Jack-Open	6 / 12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	184.74 117.33		
	5	J03 Jack-Open	4 / 12	3-07-00	1-11-03	2 x 4	1-03-08	3-15 1-03-04	50.28 33.33		
	2	J11 Jack-Open	6 / 12	1-10-15	2-01-08	2 x 4	1-03-08 6-09	1-02-00 2-01-08	15.69 10.67		
	2	J12 Jack-Open	6 / 12	2-05-08	2-04-12	2 x 4	1-03-08	1-02-00 2-04-12	17.09 12.00		
	3	J13 Jack-Open	6 / 12	1-10-15	2-01-08	2 x 4	1-03-08 1-01	1-02-00 2-01-08	21.97 14.00		
	1	J14 Jack-Open	6 / 12	2-00-00	3-01-08	2 x 4	1-03-08 1-10-15	1-02-00 2-02-00	9.87 6.00		
	1	J15 Jack-Open	6 / 12	1-10-15	2-01-08	2 x 4	1-03-08 3-11-09	1-02-00 2-01-08	11.74 7.33		
	1	J16 Jack-Open	6 / 12	3-10-15	3-01-08	2 x 4	1-03-08 1-11-09	1-02-00 3-01-08	14.29 8.67		

TOTAL # TRUSS= 62      TOTAL BFT OF ALL TRUSSES= 1722.51      BFT.      TOTAL WEIGHT OF ALL TRSSES 2693.27 LBS

## HARDWARE

QTY	TYPE	MODEL	LENGTH
3	Hardware	LUS24	
1	Hardware	HGUS26-2	

TOTAL NUMBER OF ITEMS= 4

CITY OF RICHMOND HILL  
BUILDING DIVISION

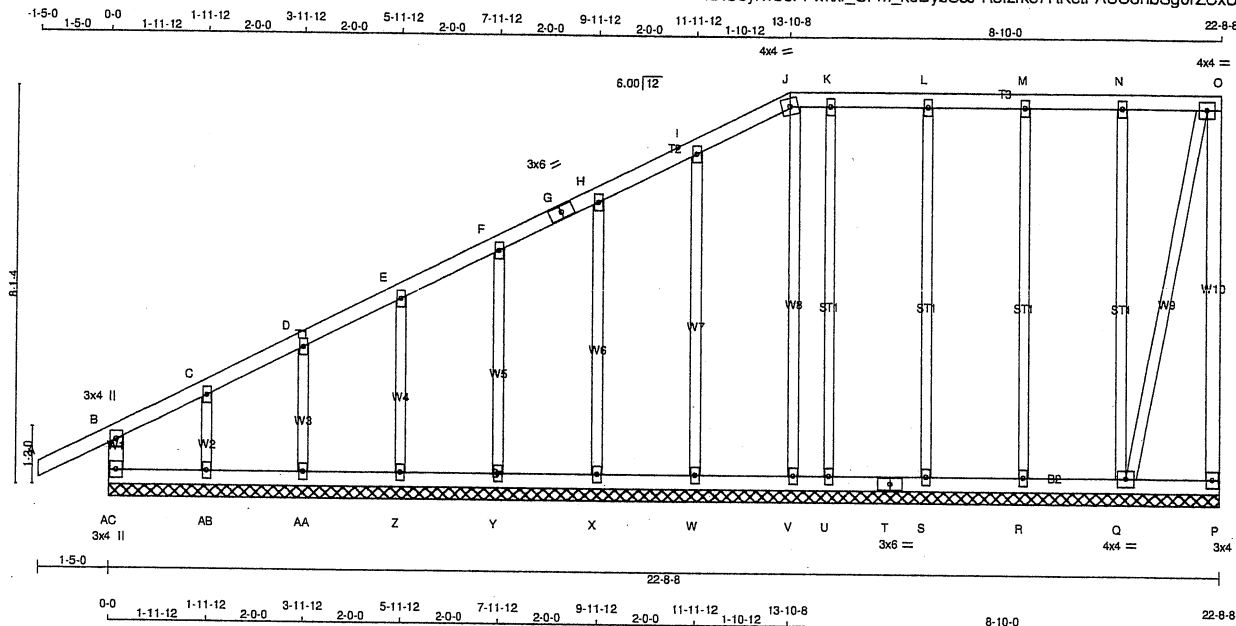
09/02/2021

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Per: jocelyn.aguilar

JOB NAME <b>412868</b>	TRUSS NAME <b>T10G</b>	QUANTITY <b>4</b>	PLY <b>1</b>	JOB DESC. <b>ROYAL PINE HOMES</b>	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 1 10:22:39 2021 Page 1  
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Scale = 1:44.7

TOTAL WEIGHT = 4 X 122 = 488 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
AC - B	2x4	DRY	No.2
A - G	2x4	DRY	No.2
G - J	2x4	DRY	No.2
J - O	2x4	DRY	No.2
P - O	2x4	DRY	No.2
AC - T	2x4	DRY	No.2
T - P	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			
GABLE STUDS SPACED AT	2-0-0	OC.	

PLATES (table is in inches)			
JT TYPE	PLATES	W	LEN Y X
B TMV+p	MT20	3.0	4.0
C, D, E, F, H, I, K, L, M, N			
C TMW+w	MT20	2.0	4.0
G TS-t	MT20	3.0	6.0
J TTW-m	MT20	4.0	4.0
O TMVW-t	MT20	4.0	4.0
P BMV1+p	MT20	3.0	4.0
Q BMW1-t	MT20	4.0	4.0
R, S, U, V, W, X, Y, Z, AA, AB			
R BMW1+w	MT20	2.0	4.0
T BS-t	MT20	3.0	6.0
AC BMV1+p	MT20	3.0	4.0

#### DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

**BEARINGS**  
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.  
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.  
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

**BRACING**  
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.  
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

**LOADING**  
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS (LC)	
FR-TO		FROM TO		FR-TO			
AC-B	-271 / 0	0.0	0.0 0.03 (1)	Q-N	-200 / 0	0.25 (1)	
A-B	0 / 30	-91.8	-91.8 0.14 (1)	R-M	-182 / 0	0.23 (1)	
B-C	-21 / 0	-91.8	-91.8 0.14 (1)	S-L	-190 / 0	0.24 (1)	
C-D	0 / 10	-91.8	-91.8 0.05 (1)	U-K	-149 / 0	0.19 (1)	
D-E	0 / 9	-91.8	-91.8 0.05 (1)	V-J	-95 / 0	0.12 (1)	
E-F	0 / 14	-91.8	-91.8 0.04 (1)	AB-C	-118 / 0	0.02 (1)	
F-G	0 / 16	-91.8	-91.8 0.04 (1)	AA-D	-196 / 0	0.04 (1)	
G-H	0 / 16	-91.8	-91.8 0.04 (1)	Z-E	-179 / 0	0.05 (1)	
H-I	0 / 19	-91.8	-91.8 0.05 (1)	Y-F	-183 / 0	0.07 (1)	
I-J	0 / 13	-91.8	-91.8 0.05 (1)	X-H	-180 / 0	0.11 (1)	
J-K	0 / 20	-91.8	-91.8 0.03 (1)	W-I	-200 / 0	0.18 (1)	
K-L	0 / 20	-91.8	-91.8 0.05 (1)	Q-O	-79 / 0	0.11 (1)	
L-M	0 / 20	-91.8	-91.8 0.05 (1)				
M-N	0 / 20	-91.8	-91.8 0.05 (1)				
N-O	0 / 20	-91.8	-91.8 0.05 (1)				
P-O	0 / 0	0.0	0.0 0.00 (1)				
AC-AB	0 / 0	-18.5	-18.5 0.02 (4)				
AB-AA	-4 / 0	-18.5	-18.5 0.02 (4)				
AA-Z	-9 / 0	-18.5	-18.5 0.01 (4)				
Z-Y	-12 / 0	-18.5	-18.5 0.01 (4)				
Y-X	-14 / 0	-18.5	-18.5 0.01 (4)				
X-W	-16 / 0	-18.5	-18.5 0.02 (4)				
W-V	-18 / 0	-18.5	-18.5 0.02 (4)				
V-U	-20 / 0	-18.5	-18.5 0.01 (4)				
U-T	-20 / 0	-18.5	-18.5 0.02 (4)				
T-S	-20 / 0	-18.5	-18.5 0.02 (4)				
S-R	-20 / 0	-18.5	-18.5 0.02 (4)				
R-Q	-20 / 0	-18.5	-18.5 0.02 (4)				
Q-P	0 / 0	-18.5	-18.5 0.02 (4)				

#### DESIGN CRITERIA

**SPECIFIED LOADS:**  
TOP CH. LL = 25.6 PSF  
DL = 6.0 PSF  
BOT CH. LL = 0.0 PSF  
DL = 7.4 PSF  
TOTAL LOAD = 39.0 PSF

**SPACING = 24.0 IN C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:  
- PART 9 OF CBC 2018, ABC 2019  
- PART 9 OF CBC 2012 (2019 AMENDMENT)  
- CSA 086-14  
- TPIC 2014

**DESIGN ASSUMPTIONS**  
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (A-B:1), BC=0.02/1.00 (Q-R:4), WB=0.25/1.00 (N-Q:1), SS=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10  
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

**NAIL VALUES**  
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)  
MAX MIN MAX MIN MAX MIN  
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

CITY OF P... BUILDING DIVISION  
JSH GRIP = 0.59 (U) (INPUT = 0.90)  
JSH METAL = 0.08 (U) (INPUT = 1.00)

09/02/2021

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Per: jocelyn.aguiar



Structural component only  
DWG# T-2121161





JOB NAME <b>412868</b>	TRUSS NAME <b>T11</b>	QUANTITY <b>15</b>	PLY <b>1</b>	JOB DESC. <b>ROYAL PINE HOMES</b>	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 1 10:22:40 2021 Page 2  
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TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING  
AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.2} PSF AT {31-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}. INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.



Structural component only  
DWG# T-2121162 *ML*

CITY OF RICHMOND HILL  
BUILDING DIVISION

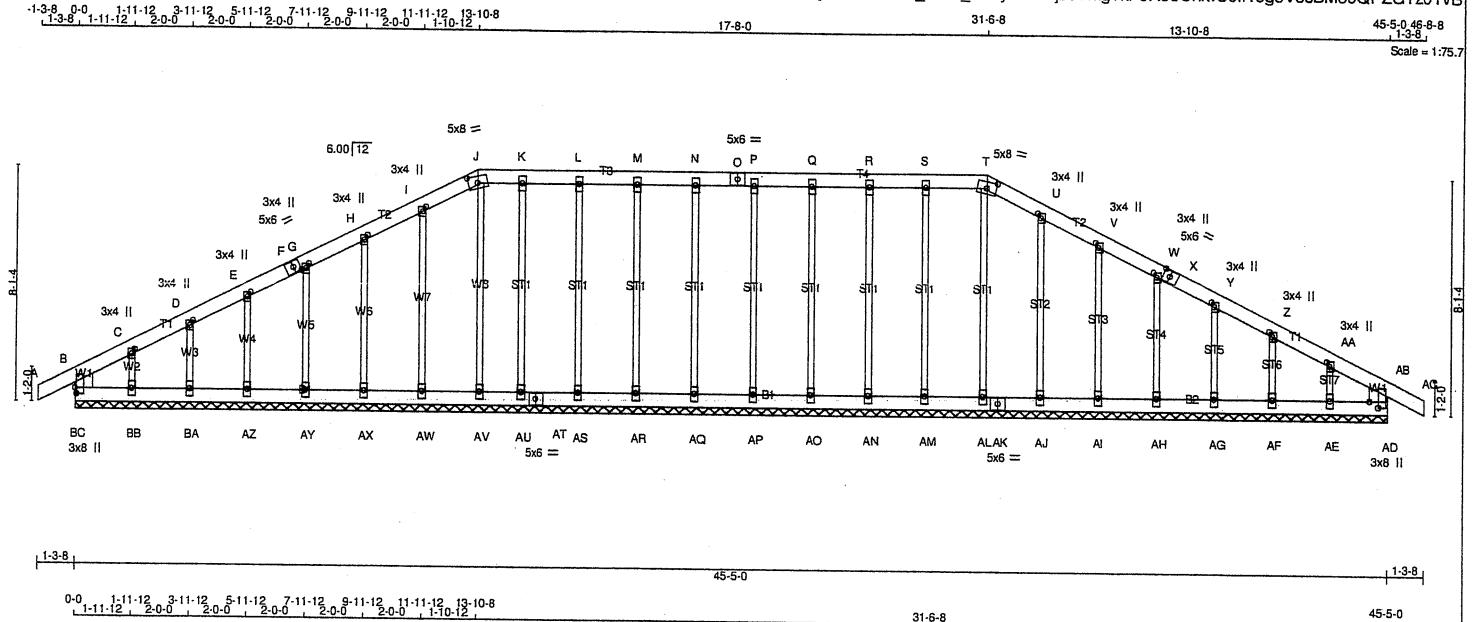
09/02/2021

RECEIVED

Per: jocelyn.aguilar

JOB NAME <b>412868</b>	TRUSS NAME <b>T11G</b>	QUANTITY <b>2</b>	PLY <b>1</b>	JOB DESC. <b>ROYAL PINE HOMES</b>	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Thu Jul 1 10:22:42 2021 Page 1  
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LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
BC- B	2x8	DRY	No.2
A - F	2x6	DRY	No.2
F - J	2x6	DRY	No.2
J - O	2x6	DRY	No.2
O - T	2x6	DRY	No.2
T - X	2x6	DRY	No.2
X - AC	2x6	DRY	No.2
AD- AB	2x8	DRY	No.2
BC- AT	2x6	DRY	No.2
AT- AK	2x6	DRY	No.2
AK- AD	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			
GABLE STUDS SPACED AT 2-0-0 OC.			

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B, AB, AD, BC				
B				
C, D, E, G, H, I, U, V, W, Y, Z, AA				
C TMW+W	MT20	3.0	4.0	2.00 1.25
F TS-I	MT20	5.0	6.0	2.50 2.75
J TTW-m	MT20	5.0	8.0	2.75 4.00
K, L, M, N, P, Q, R, S				
K TMW+W	MT20	3.0	6.0	
O TS-I	MT20	5.0	6.0	
T TTW-m	MT20	5.0	8.0	2.75 4.00
X TS-I	MT20	5.0	6.0	2.50 2.75
AD TMBMV1+p	MT20	3.0	8.0	2.50 3.75
AE, AF, AG, AH, AI, AJ, AL, AM, AN, AO, AP, AQ, AR, AS, AU, AV, AW, AX, AY, AZ, BA, BB				
AE BMW1+W	MT20	3.0	6.0	
AK BS-I	MT20	5.0	6.0	
AT BS-I	MT20	5.0	6.0	
BC TMBMV1+p	MT20	3.0	8.0	2.50 0.50

#### DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

**BEARINGS**  
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.  
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.  
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)  
**BRACING**  
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.  
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.  
**LOADING**  
TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)	MAX. CSI (LC)	MAX. UNBRAC LENGTH
FR-TO				FR-TO			
BC-B	-260 / 0	0.0	0.02 (1)	AL-T	-186 / 0	0.21 (1)	
A-B	0 / 29	-91.8	-91.8 0.06 (1)	AM-S	-190 / 0	0.21 (1)	
B-C	-50 / 0	-91.8	-91.8 0.05 (1)	AN-R	-183 / 0	0.21 (1)	
C-D	-24 / 0	-91.8	-91.8 0.02 (1)	AO-Q	-183 / 0	0.21 (1)	
D-E	-19 / 0	-91.8	-91.8 0.02 (1)	AP-P	-183 / 0	0.21 (1)	
E-F	-13 / 0	-91.8	-91.8 0.02 (1)	AQ-N	-183 / 0	0.21 (1)	
F-G	-13 / 0	-91.8	-91.8 0.02 (1)	AR-M	-184 / 0	0.21 (1)	
G-H	-9 / 0	-91.8	-91.8 0.02 (1)	AS-L	-184 / 0	0.21 (1)	
H-I	-5 / 0	-91.8	-91.8 0.02 (1)	AU-K	-163 / 0	0.18 (1)	
I-J	-3 / 0	-91.8	-91.8 0.02 (1)	AJ-U	-183 / 0	0.14 (1)	
J-K	0 / 0	-91.8	-91.8 0.02 (1)	AI-V	-182 / 0	0.09 (1)	
K-L	0 / 0	-91.8	-91.8 0.02 (1)	AH-W	-182 / 0	0.06 (1)	
L-M	0 / 0	-91.8	-91.8 0.02 (1)	AG-Y	-180 / 0	0.04 (1)	
M-N	0 / 0	-91.8	-91.8 0.02 (1)	AF-Z	-188 / 0	0.03 (1)	
N-O	0 / 0	-91.8	-91.8 0.02 (1)	AE-AA	-137 / 0	0.02 (1)	
O-P	0 / 0	-91.8	-91.8 0.02 (1)	AV-J	-159 / 0	0.18 (1)	
P-Q	0 / 0	-91.8	-91.8 0.02 (1)	BB-C	-137 / 0	0.02 (1)	
Q-R	0 / 0	-91.8	-91.8 0.02 (1)	BA-D	-188 / 0	0.03 (1)	
R-S	0 / 0	-91.8	-91.8 0.02 (1)	AZ-E	-180 / 0	0.04 (1)	
S-T	0 / 0	-91.8	-91.8 0.02 (1)	AY-G	-181 / 0	0.06 (1)	
T-U	-2 / 0	-91.8	-91.8 0.02 (1)	AX-H	-183 / 0	0.10 (1)	
U-V	-5 / 0	-91.8	-91.8 0.02 (1)	AW-I	-184 / 0	0.14 (1)	
V-W	-9 / 0	-91.8	-91.8 0.02 (1)				
W-X	-14 / 0	-91.8	-91.8 0.02 (1)				
X-Y	-14 / 0	-91.8	-91.8 0.02 (1)				
Y-Z	-20 / 0	-91.8	-91.8 0.02 (1)				
Z-AA	-24 / 0	-91.8	-91.8 0.02 (1)				
AA-AB	-51 / 0	-91.8	-91.8 0.05 (1)				
AB-AC	0 / 29	-91.8	-91.8 0.06 (1)				
AD-AB	-260 / 0	0.0	0.02 (1)				
BC-BB	0 / 33	-18.5	-18.5 0.03 (1)				
BB-BA	0 / 24	-18.5	-18.5 0.01 (1)				
BA-AZ	0 / 17	-18.5	-18.5 0.01 (4)				
AZ-AY	0 / 12	-18.5	-18.5 0.01 (4)				
AY-AX	0 / 8	-18.5	-18.5 0.01 (4)				
AX-AW	0 / 5	-18.5	-18.5 0.01 (4)				
AW-AV	0 / 2	-18.5	-18.5 0.01 (4)				
AV-AU	0 / 0	-18.5	-18.5 0.01 (4)				
AU-AT	0 / 0	-18.5	-18.5 0.01 (4)				
AT-AS	0 / 0	-18.5	-18.5 0.01 (4)				
AS-AR	0 / 0	-18.5	-18.5 0.01 (4)				
AR-AQ	0 / 0	-18.5	-18.5 0.01 (4)				
AQ-AP	0 / 0	-18.5	-18.5 0.01 (4)				
AP-AO	0 / 0	-18.5	-18.5 0.01 (4)				
AO-AN	0 / 0	-18.5	-18.5 0.01 (4)				
AN-AM	0 / 0	-18.5	-18.5 0.01 (4)				
AM-AL	0 / 0	-18.5	-18.5 0.01 (4)				
AL-AK	0 / 2	-18.5	-18.5 0.01 (4)				
AK-AJ	0 / 2	-18.5	-18.5 0.01 (4)				
AJ-AI	0 / 5	-18.5	-18.5 0.01 (4)				

#### DESIGN CRITERIA

**SPECIFIED LOADS:**  
TOP CH. LL = 25.6 PSF  
DL = 6.0 PSF  
BOT CH. LL = 0.0 PSF  
DL = 7.4 PSF  
TOTAL LOAD = 39.0 PSF

**SPACING = 24.0 IN/C**

**LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:  
- PART 9 OF CBC 2018, ABC 2019  
- PART 9 OF OBC 2012 (2019 AMENDMENT)  
- CSA 086-14  
- TPIC 2014

**DESIGN ASSUMPTIONS**  
- OVERHANG NOT TO BE ALTERED OR CUT OFF.  
(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.06/1.00 (AB-AC:1), BC=0.03/1.00 (AD-AE:1), WB=0.21/1.00 (S-AM:1), SSI=0.07/1.00 (AB-AC:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

**NAIL VALUES**  
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)  
MAX MIN MAX MIN MAX MIN  
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

MSI GRIP=0.25 (INPUT = 0.90)  
JSI METAL=0.09 (AD) (INPUT = 1.00)

CITY OF RICHMOND HILL

BUILDING DEPARTMENT

09/02/2021

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Per: jocelyn.aguiar



Structural component only  
DWG# T-2121163

JOB NAME <b>412868</b>	TRUSS NAME <b>T11G</b>	QUANTITY <b>2</b>	PLY <b>1</b>	JOB DESC. <b>ROYAL PINE HOMES</b>	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 1 10:22:42 2021 Page 2  
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**LOADING**  
TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX	MEMB.	MAX. FACTORED FORCE (LBS)	MAX	UNBRAC LENGTH	FR-TO
FR-TO		FROM	TO						
AI-AH	0 / 8	-18.5	-18.5	0.01 (4)	10.00				
AI-AG	0 / 12	-18.5	-18.5	0.01 (4)	10.00				
AG-AF	0 / 17	-18.5	-18.5	0.01 (4)	10.00				
AF-AE	0 / 25	-18.5	-18.5	0.01 (1)	10.00				
AE-AD	0 / 33	-18.5	-18.5	0.03 (1)	10.00				



Structural component only  
DWG# T-2121163 *mm*

CITY OF RICHMOND HILL  
BUILDING DIVISION

09/02/2021

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Per: jocelyn.aguilar



DRWG NO.
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Version 3.420 3/24/21 2021 MHEK Industries, Inc. Thu Jul 1 16:03:38 2021 Page 1  
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[M]

CONTINUED ON PAGE 2

JOB NAME <b>413139</b>	TRUSS NAME <b>T11B</b>	QUANTITY <b>8</b>	PLY <b>1</b>	JOB DESC. <b>ROYAL PINE HOMES</b>	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 1 16:03:38 2021 Page 2  
ID:U6yi?rbeFFwkoF UFm koDybSsJ-u6?JkNEKJLFzEH1G4189MPho24xcUhtk?zd?Epz0OvZ

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING  
AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.2} PSF AT {30-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS,  $C_p C_g$ , BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.



Structural component only  
DWG# T-2121238 *mn*

CITY OF RICHMOND HILL  
BUILDING DIVISION

09/02/2021

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Per: jocelyn.aguilar

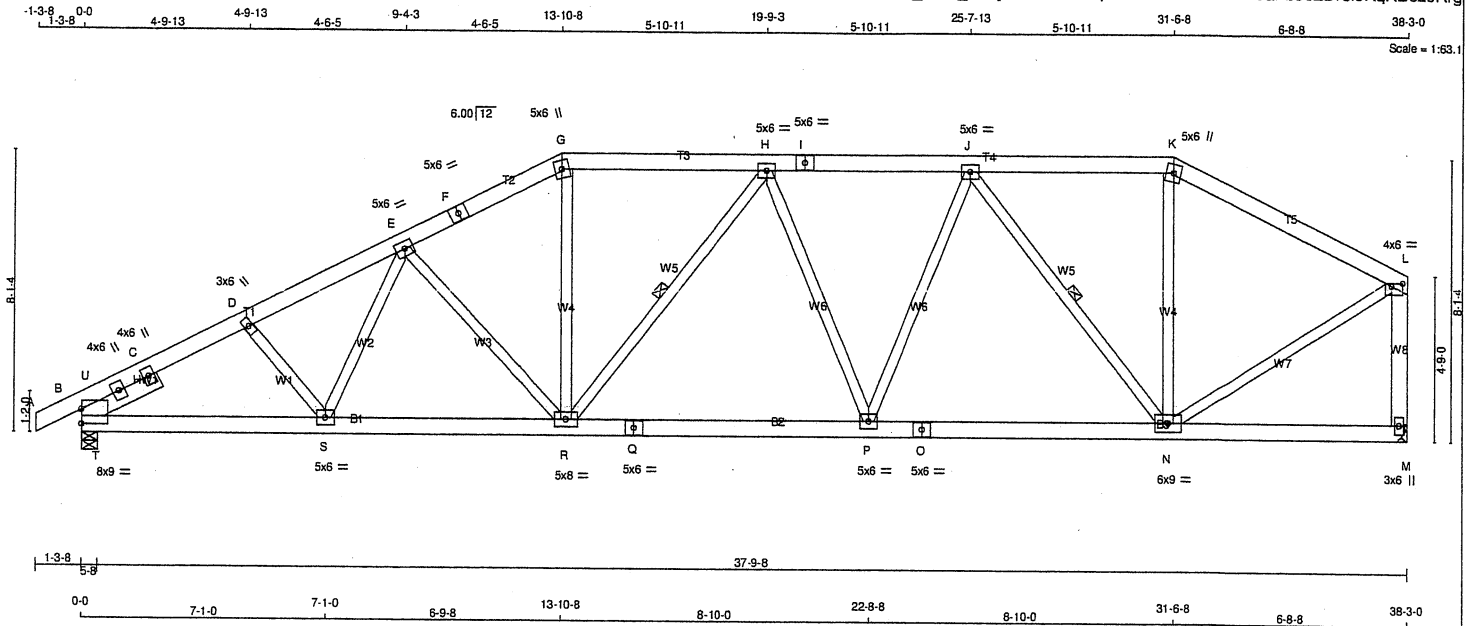
CITY OF RICHMOND HILL  
BUILDING DIVISION

09/02/2021

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Per: jocelyn.aguilar

JOB NAME <b>413139</b>	TRUSS NAME <b>T11A</b>	QUANTITY <b>8</b>	PLY <b>1</b>	JOB DESC. <b>ROYAL PINE HOMES</b>	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.		Version 8.420 S Jan 21 2021 MITek Industries, Inc. Thu Jul 1 12:42:59 2021 Page 1 ID:U6yi?rbeFFwxf_Ufm_koDybSsJ-u?340qXhWNTAP9ATJ7tZxdaPb562Dr5leRqRB6z0Rrg	



TOTAL WEIGHT = 8 X 238 = 1900 lb [M]

LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER		
A - F	2x6	DRY	No.2	SPF	
F - G	2x6	DRY	No.2	SPF	
G - I	2x6	DRY	No.2	SPF	
I - K	2x6	DRY	No.2	SPF	
K - L	2x6	DRY	No.2	SPF	
M - L	2x6	DRY	No.2	SPF	
B - Q	2x6	DRY	No.2	SPF	
Q - O	2x6	DRY	No.2	SPF	
O - M	2x6	DRY	No.2	SPF	

REINFORCING MEMBERS				DESCR.	
	CHORDS	SIZE	LUMBER		
HW1	2x6	DRY	No.2	SPF	

ALL WEBS EXCEPT				DESCR.	
	CHORDS	SIZE	LUMBER		
EXCEPT	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMBMR1-I	MT20	8.0	9.0	5.00
B	RT-t	MT20	4.0	6.0	
B	RT-t	MT20	4.0	6.0	
D	TMW-w	MT20	3.0	6.0	
E, H, J					
E	TMWW-t	MT20	5.0	6.0	
F	TS-t	MT20	5.0	6.0	
G	TTW+m	MT20	5.0	6.0	
I	TS-t	MT20	5.0	6.0	
K	TTW+m	MT20	5.0	6.0	
L	TMWV-p	MT20	4.0	6.0	1.00 3.75
M	BMV1+p	MT20	3.0	6.0	
N	BMVWW-t	MT20	6.0	9.0	
O	BS-t	MT20	5.0	6.0	
P	BMVW-t	MT20	5.0	6.0	
Q	BS-t	MT20	5.0	6.0	
R	BMVWW-t	MT20	5.0	8.0	
S	BMVW-t	MT20	5.0	6.0	

#### DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
B	2237	0	2237	0	5-8
M	2109	0	2109	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT M. MINIMUM BEARING LENGTH AT JOINT M = 3-8.

#### UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	1579	1050 / 0	0 / 0	0 / 0	0 / 0	529 / 0	0 / 0
M	1491	979 / 0	0 / 0	0 / 0	0 / 0	513 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B

#### BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.60 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF H-R, J-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

#### LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSF (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSF (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 0	-91.8	-91.8 0.06 (1)	10.00	D-S	0 / 69	0.02 (4)
B-U	-2908 / 0	-91.8	-91.8 0.05 (1)	4.91	S-E	0 / 65	0.02 (4)
U-C	-2227 / 0	-91.8	-91.8 0.40 (1)	5.00	E-R	-550 / 0	0.36 (1)
C-D	-2227 / 0	-91.8	-91.8 0.40 (1)	5.00	R-G	0 / 854	0.14 (1)
D-E	-3092 / 0	-91.8	-91.8 0.23 (1)	4.60	R-H	-332 / 0	0.15 (1)
E-F	-2766 / 0	-91.8	-91.8 0.17 (1)	4.89	H-P	-338 / 0	0.33 (1)
F-G	-2766 / 0	-91.8	-91.8 0.17 (1)	4.89	P-J	0 / 506	0.08 (1)
G-H	-2465 / 0	-91.8	-91.8 0.20 (1)	5.08	J-N	-1328 / 0	0.60 (1)
H-I	-2543 / 0	-91.8	-91.8 0.19 (1)	5.04	N-K	0 / 252	0.04 (4)
I-J	-2543 / 0	-91.8	-91.8 0.19 (1)	5.04	N-L	0 / 1836	0.30 (1)
J-K	-1563 / 0	-91.8	-91.8 0.18 (1)	6.07	T-U	0 / 2337	0.00 (1)
K-L	-1744 / 0	-91.8	-91.8 0.28 (1)	5.89	T-C	-2249 / 0	0.17 (1)
M-L	-2080 / 0	0.0	0.0 0.44 (1)	7.06			

B-T	0 / 1311	-18.5	-18.5 0.32 (1)	10.00
T-S	0 / 2770	-18.5	-18.5 0.49 (1)	10.00
S-R	0 / 2810	-18.5	-18.5 0.41 (1)	10.00
R-Q	0 / 2664	-18.5	-18.5 0.39 (1)	10.00
Q-P	0 / 2664	-18.5	-18.5 0.39 (1)	10.00
P-O	0 / 2361	-18.5	-18.5 0.35 (1)	10.00
O-N	0 / 2361	-18.5	-18.5 0.35 (1)	10.00
N-M	0 / 0	-18.5	-18.5 0.12 (4)	10.00

#### DESIGN CRITERIA

##### SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF  
DL = 6.0 PSF  
BOT CH. LL = 0.0 PSF  
DL = 7.4 PSF  
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.27")  
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")  
ALLOWABLE DEFL.(TL) = L/360 (1.27")  
CALCULATED VERT. DEFL.(TL) = L/999 (0.21")

CSI: TC=0.44/1.00 (L-M:1), BC=0.49/1.00 (S-T:1), WB=0.60/1.00 (J-N:1), SSI=0.31/1.00 (B-T:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

#### NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION  
(PSI) (PLI) (PLI)  
MAX MIN MAX MIN MAX MIN  
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

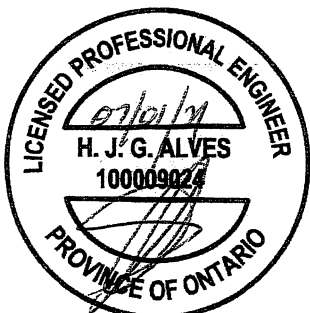
ISI GRIP = 0.82 (L) (INPUT = 0.90)  
ISI METAL = 0.78 (B) (INPUT = 1.00)

CITY OF RICHMOND HILL  
BUILDING DEPARTMENT

09/02/2021

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Per: jocelyn.aguiar

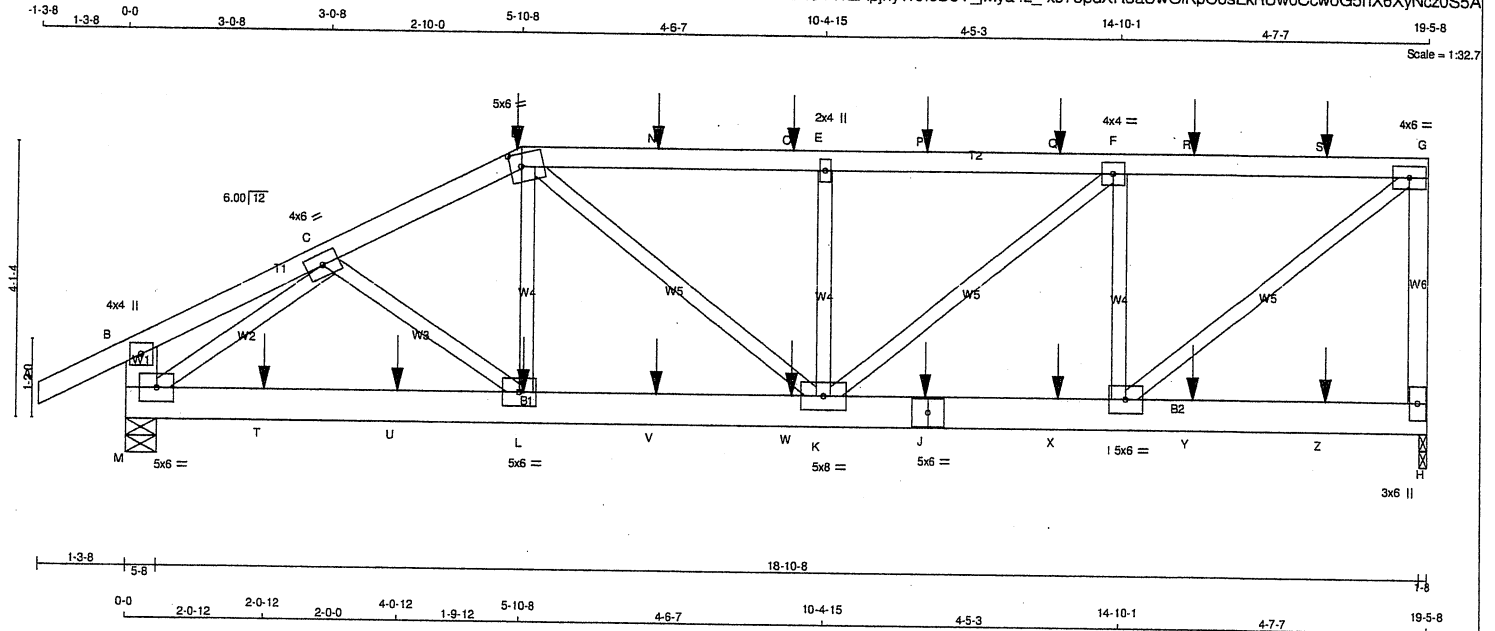


Structural component only  
DWG# T-2121237



JOB NAME <b>412865</b>	TRUSS NAME <b>T101</b>	QUANTITY <b>2</b>	PLY <b>2</b>	JOB DESC. <b>ROYAL PINE HOMES</b>	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.420 S Jan 21 2021 MITek Industries, Inc. Thu Jul 1 12:26:27 2021 Page 1  
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LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
H - G	2x4	DRY	No.2
M - B	2x6	DRY	No.2
M - J	2x6	DRY	No.2
J - H	2x6	DRY	No.2

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - D 1	12	SIDE(61.0)
D - G 1	12	SIDE(61.0)
G - H 1	12	TOP
M - B 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
M - J 2	12	SIDE(183.1)
J - H 2	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

#### DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
GROSS REACTION	VERT	GROSS REACTION	DOWN	BRG	IN-SX	BRG	IN-SX
JT	1774	0	1774	0	1-8	1-8	
M	1855	0	1855	0	5-8	5-8	

#### UNFACTORED REACTIONS

1ST LCASE	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	1255	820 / 0	0 / 0	0 / 0	0 / 0	435 / 0	0 / 0
M	1311	867 / 0	0 / 0	0 / 0	0 / 0	444 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

#### BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.42 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

#### LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		W E B S		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MAX. CSI (LC)	MEMB.	FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-91.8 -91.8	0.07 (1)	C-L	0 / 214	0.03 (1)	
B-C	0 / 9	-91.8 -91.8	0.05 (1)	L-D	0 / 151	0.03 (4)	
C-D	-2488 / 0	-91.8 -91.8	0.07 (1)	M-C	-2546 / 0	0.30 (1)	
D-N	-2488 / 0	-91.8 -91.8	0.23 (1)	I-G	0 / 2332	0.29 (1)	
N-O	-2488 / 0	-91.8 -91.8	0.23 (1)	D-K	0 / 375	0.05 (1)	
O-E	-2488 / 0	-91.8 -91.8	0.23 (1)	I-F	-1302 / 0	0.17 (1)	
E-P	-2488 / 0	-91.8 -91.8	0.26 (1)	K-E	-702 / 0	0.09 (1)	
P-Q	-2488 / 0	-91.8 -91.8	0.26 (1)	K-F	0 / 880	0.11 (1)	
Q-F	-2488 / 0	-91.8 -91.8	0.26 (1)				
F-R	-1814 / 0	-91.8 -91.8	0.25 (1)				
R-S	-1814 / 0	-91.8 -91.8	0.25 (1)				
S-G	-1814 / 0	-91.8 -91.8	0.25 (1)				
H-G	-1712 / 0	0.0 0.0	0.21 (1)				
M-B	-239 / 0	0.0 0.0	0.01 (1)				
M-T	0 / 2025	-18.5 -18.5	0.17 (1)				
T-U	0 / 2025	-18.5 -18.5	0.17 (1)				
U-L	0 / 2025	-18.5 -18.5	0.17 (1)				
L-V	0 / 2198	-18.5 -18.5	0.17 (1)				
V-W	0 / 2198	-18.5 -18.5	0.17 (1)				
W-K	0 / 2198	-18.5 -18.5	0.17 (1)				
K-J	0 / 1814	-18.5 -18.5	0.15 (1)				
J-X	0 / 1814	-18.5 -18.5	0.15 (1)				
X-I	0 / 1814	-18.5 -18.5	0.15 (1)				
I-Y	0 / 0	-18.5 -18.5	0.04 (4)				
Y-Z	0 / 0	-18.5 -18.5	0.04 (4)				
Z-H	0 / 0	-18.5 -18.5	0.04 (4)				

#### SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE
J	5-10-8	-317	-317	---	BACK	VERT
D	11-11-4	-21	-21	---	BACK	VERT
L	5-11-4	-21	-21	---	BACK	VERT
N	7-11-4	-76	-76	---	BACK	VERT
O	9-11-4	-76	-76	---	BACK	VERT
P	11-11-4	-76	-76	---	BACK	VERT
Q	13-11-4	-76	-76	---	BACK	VERT
R	15-11-4	-76	-76	---	BACK	VERT
S	17-11-4	-76	-76	---	BACK	VERT
T	2-0-12	-21	-21	---	BACK	VERT
U	4-0-12	-21	-21	---	BACK	VERT
V	7-11-4	-21	-21	---	BACK	VERT
W	9-11-4	-21	-21	---	BACK	VERT

#### DESIGN CRITERIA

##### SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")  
CALCULATED VERT. DEFL.(LL)= L/999 (0.04")  
ALLOWABLE DEFL.(TL)= L/360 (0.65")  
CALCULATED VERT. DEFL.(TL)= L/999 (0.07")

CSI: TC=0.26/1.00 (E-F:1), BC=0.17/1.00 (L-M:1), WB=0.30/1.00 (C-M:1), SSI=0.18/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00  
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

#### NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.69 (G) (INPUT = 0.90)  
JSI METAL = 0.30 (C) (INPUT = 1.00)

CITY OF RICHMOND HILL  
BUILDING DIVISION

09/02/2021

RECEIVED  
Per Jocelyn.aguilar



Structural component only  
DWG# T-2121206 1/12

JOB NAME <b>412865</b>	TRUSS NAME <b>T101</b>	QUANTITY <b>2</b>	PLY <b>2</b>	JOB DESC. <b>ROYAL PINE HOMES</b>	DRWG NO.
				TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 1 12:26:27 2021 Page 2

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**PLATES (table is in inches)**

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	6.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	5.0	8.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMVW1-t	MT20	5.0	6.0		

**SPECIFIED CONCENTRATED LOADS (LBS)**

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
X	13-11-4	-21	-21	--	BACK	VERT	TOTAL	--	C1
Y	15-11-4	-21	-21	--	BACK	VERT	TOTAL	--	C1
Z	17-11-4	-21	-21	--	BACK	VERT	TOTAL	--	C1

**CONNECTION REQUIREMENTS**

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only  
DWG# T-2121206 *M*

CITY OF RICHMOND HILL  
BUILDING DIVISION

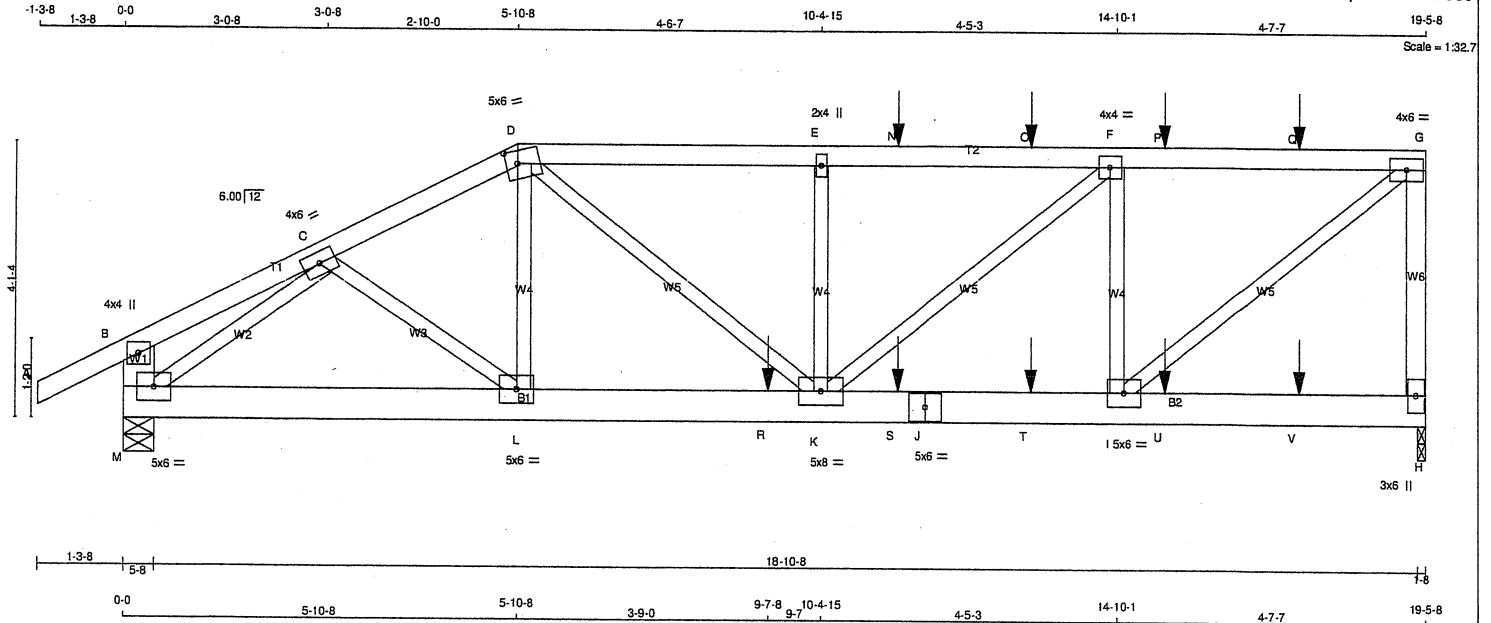
09/02/2021

RECEIVED

Per: jocelyn.aguilar

JOB NAME <b>412865</b>	TRUSS NAME <b>T101Z</b>	QUANTITY <b>1</b>	PLY <b>2</b>	JOB DESC. <b>ROYAL PINE HOMES</b>	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 1 12:26:28 2021 Page 1  
ID:hEKUVWZApjnyW9leD6V\_jMya4z\_-PMhR1zY3vucn0sv?mVNaGe15wcuNXiKqmmGWv2z0S59



LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES	A - D	2x4	DRY	No.2
	D - G	2x4	DRY	No.2
	H - G	2x4	DRY	No.2
	M - B	2x6	DRY	No.2
	M - J	2x6	DRY	No.2
	J - H	2x6	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF  
EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - D 1	12	TOP
D - G 1	12	SIDE(0.0)
G - H 1	12	TOP
M - B 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
M - J 2	12	SIDE(0.0)
J - H 2	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

#### DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
H	2170	0	2170	0
M	2037	0	2037	0

#### UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1531	1022 / 0	0 / 0	0 / 0	0 / 0	509 / 0	0 / 0
M	1434	977 / 0	0 / 0	0 / 0	0 / 0	458 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

#### BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.66 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

#### LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	MAX	MAX	WEBS	MAX. FACTORED	MAX	MAX
MEMB.	FORCE (LBS)	VERT. (PLF)	CS1 (LC)	UNBRAC LENGTH	MEMB.	FORCE (LBS)	CS1 (LC)	UNBRAC LENGTH
FR-TO		FROM TO			FR-TO			
A-B	0 / 28	-91.8 -91.8	0.07 (1)	10.00	C-L	0 / 295	0.04 (1)	
B-C	0 / 9	-91.8 -91.8	0.06 (1)	10.00	L-D	0 / 176	0.02 (4)	
C-D	-2873 / 0	-91.8 -91.8	0.08 (1)	5.35	M-C	-2920 / 0	0.35 (1)	
D-E	-3565 / 0	-91.8 -91.8	0.19 (1)	4.80	I-G	0 / 2975	0.37 (1)	
E-N	-3565 / 0	-91.8 -91.8	0.30 (1)	4.66	D-K	0 / 1296	0.16 (1)	
N-O	-3565 / 0	-91.8 -91.8	0.30 (1)	4.66	I-F	-1812 / 0	0.23 (1)	
O-F	-3565 / 0	-91.8 -91.8	0.30 (1)	4.66	K-E	-529 / 0	0.07 (1)	
F-P	-2315 / 0	-91.8 -91.8	0.27 (1)	5.56	K-F	0 / 1635	0.20 (1)	
P-Q	-2315 / 0	-91.8 -91.8	0.27 (1)	5.56				
Q-G	-2315 / 0	-91.8 -91.8	0.27 (1)	5.56				
H-G	-2095 / 0	0.0 0.0	0.26 (1)	7.65				
M-B	-241 / 0	0.0 0.0	0.01 (1)	7.81				
M-L	0 / 2322	-18.5 -18.5	0.22 (1)	10.00				
L-R	0 / 2564	-18.5 -18.5	0.41 (1)	10.00				
R-K	0 / 2564	-18.5 -18.5	0.41 (1)	10.00				
K-S	0 / 2315	-18.5 -18.5	0.25 (1)	10.00				
S-J	0 / 2315	-18.5 -18.5	0.25 (1)	10.00				
J-T	0 / 2315	-18.5 -18.5	0.25 (1)	10.00				
T-I	0 / 2315	-18.5 -18.5	0.25 (1)	10.00				
I-U	0 / 0	-18.5 -18.5	0.04 (4)	10.00				
U-V	0 / 0	-18.5 -18.5	0.04 (4)	10.00				
V-H	0 / 0	-18.5 -18.5	0.04 (4)	10.00				

#### SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
N	11-6-12	-76	-76	---	FRONT	VERT	TOTAL
O	13-6-12	-76	-76	---	FRONT	VERT	TOTAL
P	15-6-12	-76	-76	---	FRONT	VERT	TOTAL
Q	17-6-12	-76	-76	---	FRONT	VERT	TOTAL
R	9-7-8	-974	-974	---	FRONT	VERT	TOTAL
S	11-6-12	-21	-21	---	FRONT	VERT	TOTAL
T	13-6-12	-21	-21	---	FRONT	VERT	TOTAL
U	15-6-12	-21	-21	---	FRONT	VERT	TOTAL
V	17-6-12	-21	-21	---	FRONT	VERT	TOTAL

#### CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

#### DESIGN CRITERIA

##### SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.65")  
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")  
ALLOWABLE DEFL.(TL) = L/360 (0.65")  
CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CSI: TC=0.30/1.00 (E-F:1), BC=0.41/1.00 (K-L:1), WB=0.37/1.00 (G-I:1), SSI=0.40/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00  
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

#### NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

HEEL CONN. OF ROOF PLATE BUILDING CONNECTION

09/02/2021

RECEIVED  
Per: jocelyn.aguiar



Structural component only  
DWG# T-2121207

JOB NAME <b>412865</b>	TRUSS NAME <b>T101Z</b>	QUANTITY <b>1</b>	PLY <b>2</b>	JOB DESC. <b>ROYAL PINE HOMES</b>	DRWG NO.
				TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MITek Industries, Inc. Thu Jul 1 12:26:28 2021 Page 2

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**PLATES (table is in inches)**

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	6.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	5.0	8.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMVW1-t	MT20	5.0	6.0		



Structural component only  
DWG# T-2121207 *M*

CITY OF RICHMOND HILL  
BUILDING DIVISION

09/02/2021

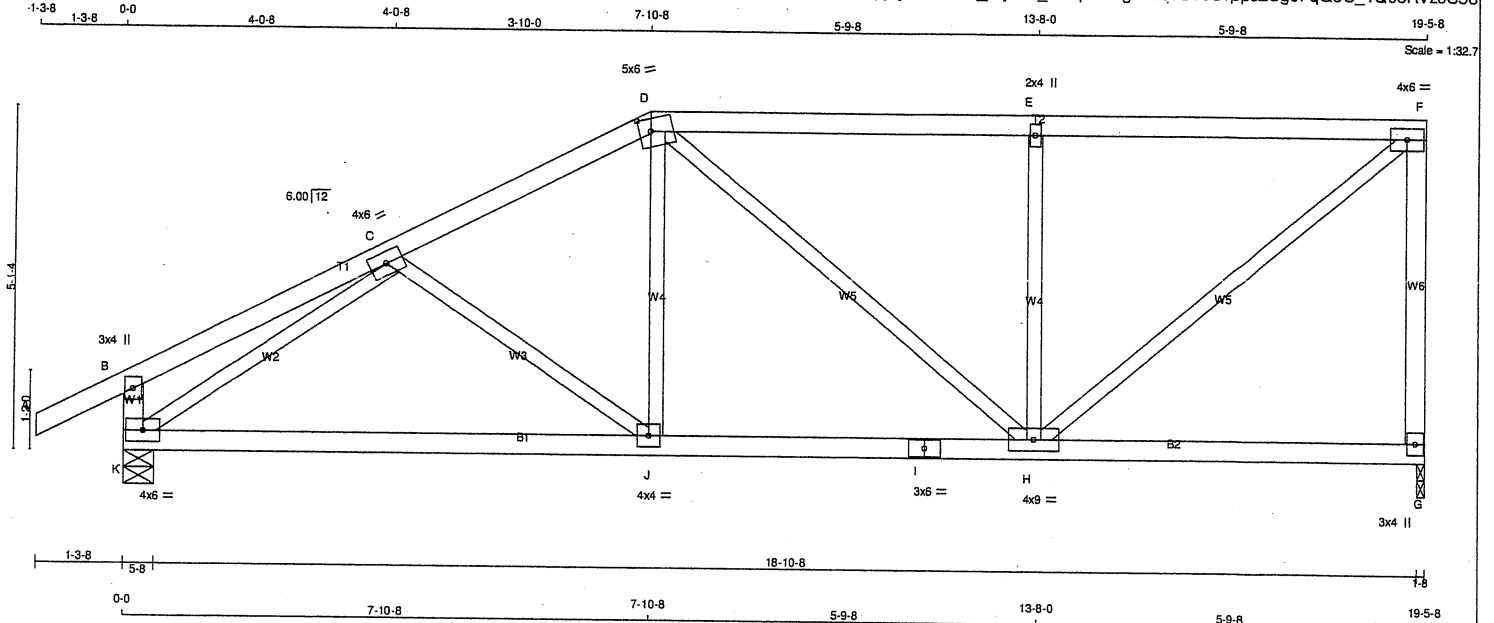
RECEIVED

Per: jocelyn.aguilar

JOB NAME <b>412865</b>	TRUSS NAME <b>T102</b>	QUANTITY <b>3</b>	PLY <b>1</b>	JOB DESC. <b>ROYAL PINE HOMES</b>	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MITek Industries, Inc. Thu Jul 1 12:26:29 2021 Page 1  
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LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2  
EXCEPT  
SPF  
DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMVWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMVW-t	MT20	4.0	4.0		
K	BMVW-t	MT20	4.0	6.0		

#### DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
G	1073	0	1073	0
K	1197	0	1197	0

#### UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS	1ST LCASE	MAX/MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
G	759	498 / 0	0 / 0
K	844	567 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

#### BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.39 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

#### LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	FORCE	FACTORED	VERT. LOAD	LC1	MAX	CS1 (LC)	UNBRAC	LENGTH	FR-TO	MEMB.	FORCE	FACTORED	VERT. LOAD	LC1	MAX	CS1 (LC)
FR-TO	MEMB.	FORCE	FACTORED	VERT. LOAD	LC1	MAX	CS1 (LC)	UNBRAC	LENGTH	FR-TO	MEMB.	FORCE	FACTORED	VERT. LOAD	LC1	MAX	CS1 (LC)
A-B	0 / 28	-91.8	-91.8	0.12	(1)	10.00	C-J	-182 / 2	0.07	(1)							
B-C	0 / 18	-91.8	-91.8	0.21	(1)	10.00	J-D	0 / 253	0.06	(4)							
C-D	-1207 / 0	-91.8	-91.8	0.24	(1)	5.54	D-H	-94 / 0	0.10	(1)							
D-E	-996 / 0	-91.8	-91.8	0.55	(1)	5.39	H-E	-658 / 0	0.25	(1)							
E-F	-996 / 0	-91.8	-91.8	0.56	(1)	5.39	H-F	0 / 1291	0.29	(1)							
G-F	-1026 / 0	0.0	0.0	0.45	(1)	7.74	K-C	-1491 / 0	0.57	(1)							
K-B	-266 / 0	0.0	0.0	0.03	(1)	7.81											
K-J	0 / 1212	-18.5	-18.5	0.34	(4)	10.00											
J-I	0 / 1068	-18.5	-18.5	0.34	(4)	10.00											
I-H	0 / 1068	-18.5	-18.5	0.34	(4)	10.00											
H-G	0 / 0	-18.5	-18.5	0.13	(4)	10.00											

TOTAL WEIGHT = 3 X 80 = 239 lb

#### DESIGN CRITERIA

SPECIFIED LOADS:  
TOP CH. LL = 25.6 PSF  
DL = 6.0 PSF  
BOT CH. LL = 0.0 PSF  
DL = 7.4 PSF  
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:  
- PART 9 OF CBC 2018, ABC 2019  
- PART 9 OF OBC 2012 (2019 AMENDMENT)  
- CSA 086-14  
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.65")  
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")  
ALLOWABLE DEFL.(TL) = L/360 (0.65")  
CALCULATED VERT. DEFL.(TL) = L/999 (0.15")

CSI: TC=0.56/1.00 (E-F:1), BC=0.34/1.00 (H-J:4), WB=0.57/1.00 (C-K:1), SSI=0.26/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10  
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES  
PLATE GRIP(DRY) SHEAR SECTION  
(PSI) (PLI) (PLI)  
MAX MIN MAX MIN MAX MIN  
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.78 (F) (INPUT = 0.90)  
JSI METAL = 0.32 (G) (INPUT = 1.00)

CITY OF RICHMOND HILL  
BUILDING DEPARTMENT

09/02/2021

RECEIVED

Per: jocelyn.aguilar



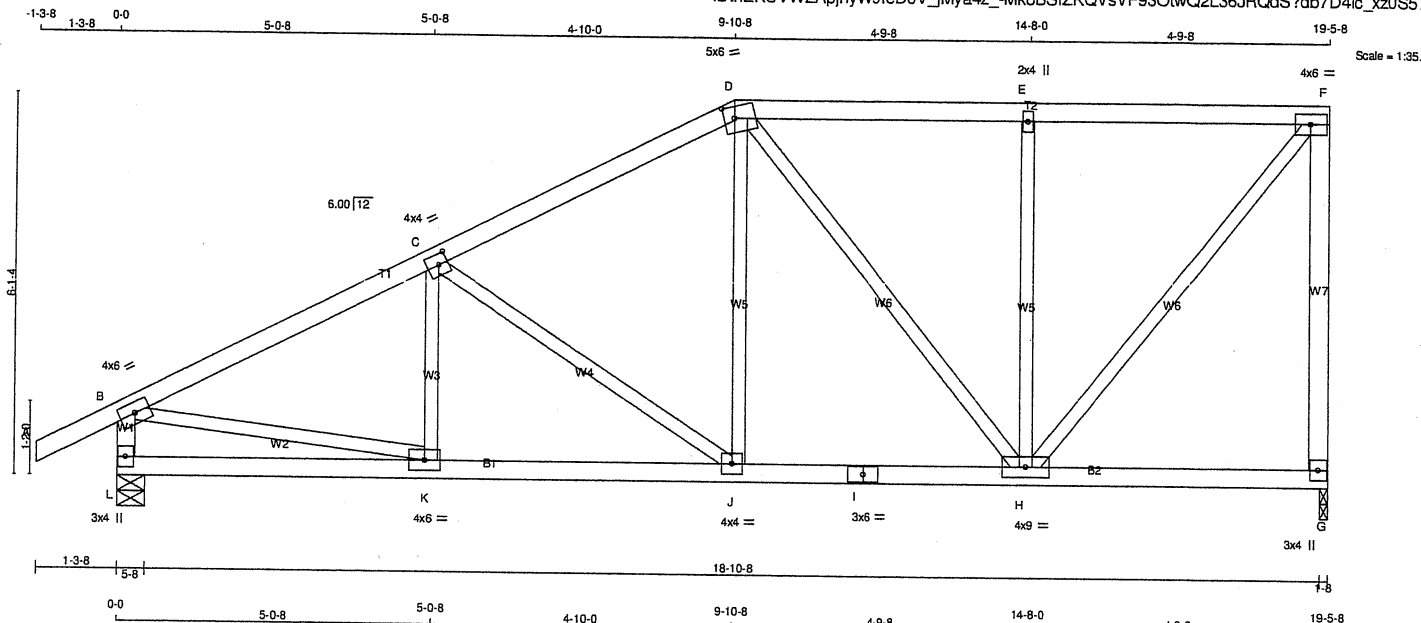
Structural component only  
DWG# T-2121208



JOB NAME <b>412865</b>	TRUSS NAME <b>T103</b>	QUANTITY <b>3</b>	PLY <b>1</b>	JOB DESC. <b>ROYAL PINE HOMES</b>	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MTEK Industries, Inc. Thu Jul 1 12:26:30 2021 Page 1  
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LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
G - F	2x4	DRY	No.2
L - B	2x4	DRY	No.2
L - I	2x4	DRY	No.2
I - G	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF  
EXCEPT  
DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMVW-t	MT20	4.0	6.0
C	TMVW-t	MT20	4.0	4.0 2.00 1.75
D	TTWW-m	MT20	5.0	6.0 2.25 2.00
E	TMVW-w	MT20	2.0	4.0
F	TMVW-t	MT20	4.0	6.0
G	BMV1+p	MT20	3.0	4.0
H	BMVWW-t	MT20	4.0	9.0
I	BS-t	MT20	3.0	6.0
J	BMVW-t	MT20	4.0	4.0
K	BMVW-t	MT20	4.0	6.0
L	BMV1+p	MT20	3.0	4.0

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	DOWN	UP	BRG	BRG
G	1073	0	1073	0	1-8
L	1197	0	1197	0	5-8

UNFACTORED REACTIONS		1ST LCASE	MAX./MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE PERM.LIVE WIND DEAD SOIL
G	759	498 / 0	0 / 0 0 / 0 261 / 0 0 / 0
L	844	567 / 0	0 / 0 0 / 0 277 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

**BRACING**  
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.17 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING		TOTAL LOAD CASES: (4)	
CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO	FROM TO	FR-TO	FROM TO
A-B	0 / 28	K-C	-118 / 43
B-C	-1400 / 0	C-J	-454 / 0
C-D	-1030 / 0	J-D	0 / 343
D-E	-720 / 0	D-H	-292 / 0
E-F	-720 / 0	H-E	-543 / 0
G-F	-1035 / 0	H-F	0 / 1126
L-B	-1157 / 0	B-K	0 / 1291
L-K	0 / 0		
K-J	0 / 1273		
J-I	0 / 905		
I-H	0 / 905		
H-G	0 / 0		

**DESIGN CRITERIA**

SPECIFIED LOADS:  
TOP CH. LL = 25.6 PSF  
DL = 6.0 PSF  
BOT CH. LL = 0.0 PSF  
DL = 7.4 PSF  
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:  
- PART 9 OF CBC 2018, ABC 2019  
- PART 9 OF CBC 2012 (2019 AMENDMENT)  
- CSA 086-14  
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.65")  
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")  
ALLOWABLE DEFL.(TL) = L/360 (0.65")  
CALCULATED VERT. DEFL.(TL) = L/999 (0.07")

CSI: TC=0.75/1.00 (F-G:1), BC=0.25/1.00 (J-K:1), WB=0.32/1.00 (E-H:1), SS=0.21/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

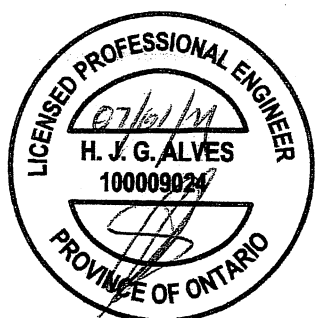
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES		PLATE GRIP(DRY) SHEAR SECTION	
(PSI)	(PLI)	(PSI)	(PLI)
MT20	650	371	1747

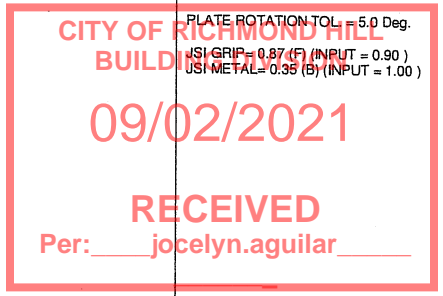
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.87 (F) (INPUT = 0.90)  
JSI METAL = 0.35 (F) (INPUT = 1.00)

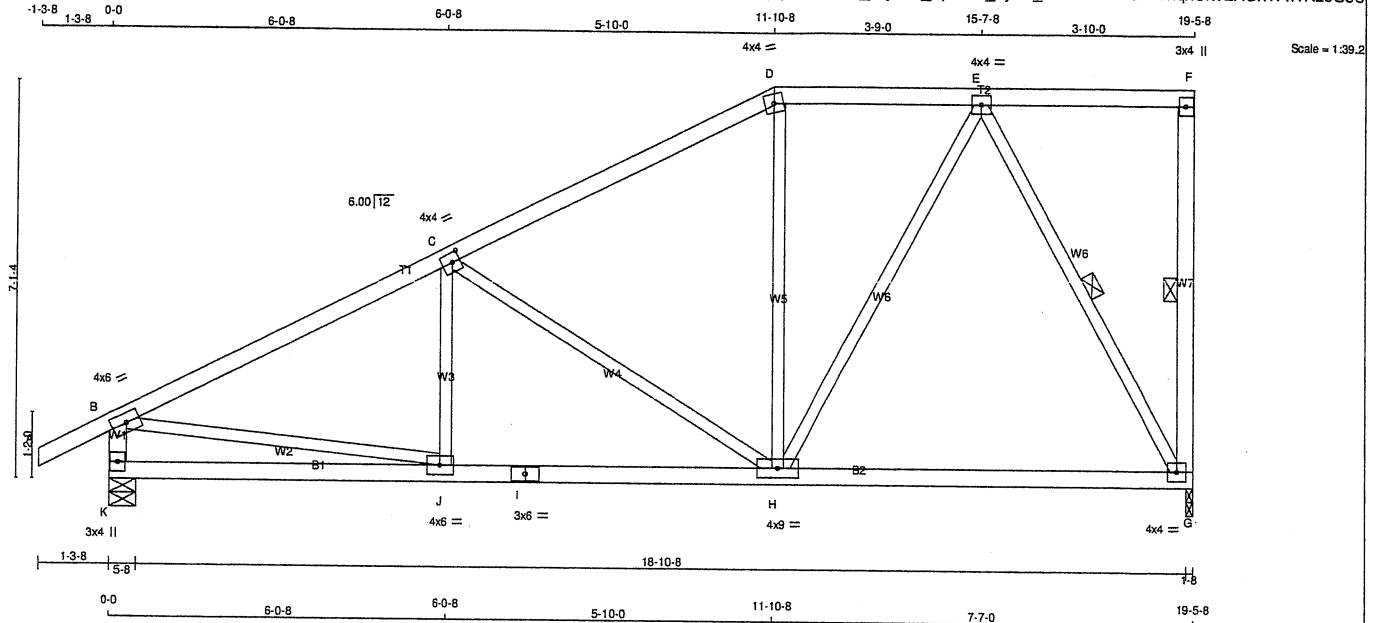


Structural component only  
DWG# T-2121209



JOB NAME <b>412865</b>	TRUSS NAME <b>T104</b>	QUANTITY <b>3</b>	PLY <b>1</b>	JOB DESC. <b>ROYAL PINE HOMES</b>	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 1 12:26:31 2021 Page 1  
ID:hEKUVWZApinyW9leD6V\_jMya4z\_-qwMaf\_ayBo\_MtJeaRdxHuHfYnqxOk?LHskVAWNz0S56



LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER		
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
K - B	2x4	DRY	No.2	SPF	
K - I	2x4	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF  
EXCEPT  
DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-t	MT20	4.0	6.0	
C	TMVW-t	MT20	4.0	4.0	2.00 1.75
D	TTW-m	MT20	4.0	4.0	
E	TMVW-t	MT20	4.0	4.0	
F	TMV-p	MT20	3.0	4.0	
G	BMVW-t	MT20	4.0	4.0	
H	BMVW-t	MT20	4.0	9.0	
I	BS-t	MT20	3.0	6.0	
J	BMVW-t	MT20	4.0	6.0	
K	BMV1+p	MT20	3.0	4.0	

#### DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
GROSS REACTION		GROSS REACTION		BRG		BRG	
JT	VERT	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
G	1073	0	1073	0	1-8	1-8	
K	1197	0	1197	0	5-8	5-8	

#### UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
G	759	498 / 0	0 / 0	0 / 0	0 / 0	261 / 0
K	844	567 / 0	0 / 0	0 / 0	0 / 0	277 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

#### BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.99 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, E-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

#### LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 28	-91.8	-91.8 0.12 (1)	10.00	J-C	-70 / 66	0.02 (4)
B-C	-1384 / 0	-91.8	-91.8 0.45 (1)	4.99	C-H	-629 / 0	0.61 (1)
C-D	-855 / 0	-91.8	-91.8 0.42 (1)	6.02	H-D	0 / 69	0.02 (4)
D-E	-738 / 0	-91.8	-91.8 0.17 (1)	6.25	H-E	0 / 512	0.12 (1)
E-F	0 / 0	-91.8	-91.8 0.22 (1)	10.00	E-G	-1012 / 0	0.41 (1)
G-F	-134 / 0	0.0	0.0 0.03 (1)	6.25	B-J	0 / 1276	0.29 (1)
K-B	-1148 / 0	0.0	0.0 0.12 (1)	7.42			
K-J	0 / 0	-18.5	-18.5 0.14 (4)	10.00			
J-I	0 / 1264	-18.5	-18.5 0.33 (1)	10.00			
I-H	0 / 1264	-18.5	-18.5 0.33 (1)	10.00			
H-G	0 / 496	-18.5	-18.5 0.26 (4)	10.00			

TOTAL WEIGHT = 3 X 87 = 260 lb [M/F]

#### DESIGN CRITERIA

SPECIFIED LOADS:  
TOP CH. LL = 25.6 PSF  
DL = 6.0 PSF  
BOT CH. LL = 0.0 PSF  
DL = 7.4 PSF  
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:  
- PART 9 OF CBC 2018, ABC 2019  
- PART 9 OF OBC 2012 (2019 AMENDMENT)  
- CSA 086-14  
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.65")  
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")  
ALLOWABLE DEFL.(TL) = L/360 (0.65")  
CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CSI: TC=0.45/1.00 (B-C:1), BC=0.33/1.00 (H-J:1), WB=0.61/1.00 (C-H:1), SSI=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10  
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES  
PLATE GRIP(DRY) SHEAR SECTION  
(PSI) (PLI) (PLI)  
MAX MIN MAX MIN MAX MIN  
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

USI GRIP = 0.74 (J) (INPUT = 0.90)  
USI METAL = 0.37 (I) (INPUT = 1.00)

CITY OF RICHMOND HILL  
BUILDING DEPARTMENT

09/02/2021

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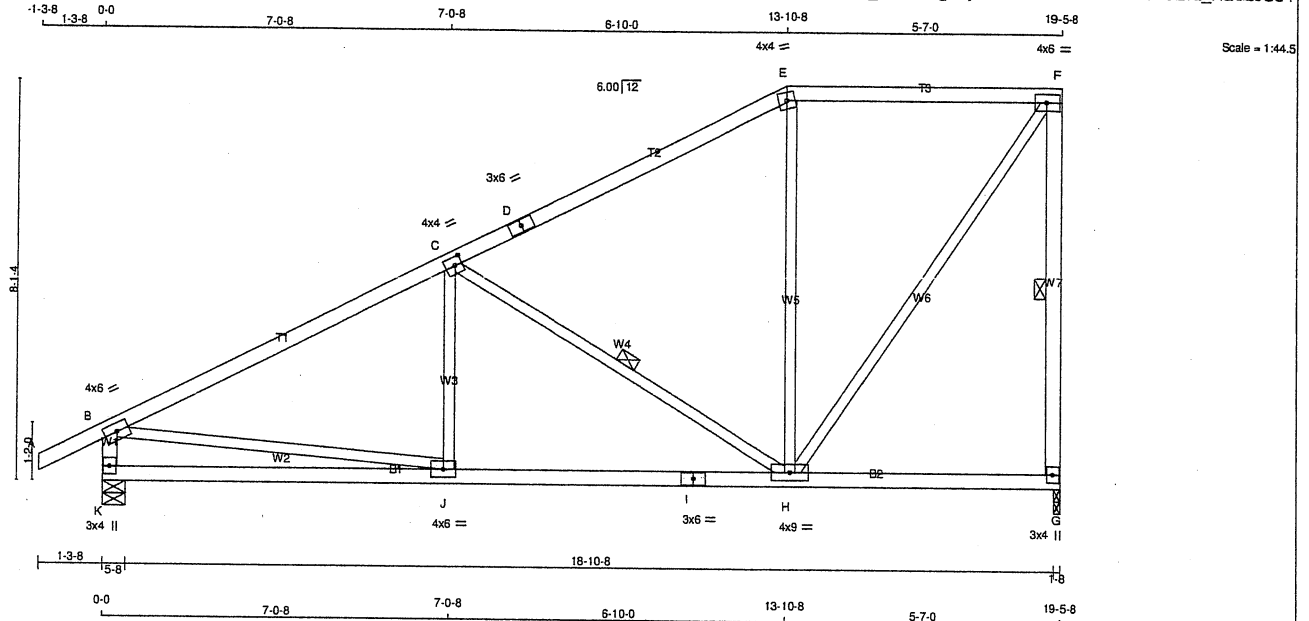
Per: jocelyn.aguilar



Structural component only  
DWG# T-2121210

JOB NAME <b>412865</b>	TRUSS NAME <b>T105</b>	QUANTITY <b>11</b>	PLY <b>1</b>	JOB DESC. <b>ROYAL PINE HOMES</b>	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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TOTAL WEIGHT = 11 X 86 = 948 lb  
(M/F)

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
G - F	2x4	DRY	No.2
K - B	2x4	DRY	No.2
K - I	2x4	DRY	No.2
I - G	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-t	MT20	4.0	6.0	
C	TMVW-t	MT20	4.0	4.0	2.00 1.75
D	TS-t	MT20	3.0	6.0	
E	TTW-m	MT20	4.0	4.0	
F	TMVW-t	MT20	4.0	6.0	
G	BMV1+p	MT20	3.0	4.0	
H	BMVWW-t	MT20	4.0	9.0	
I	BS-t	MT20	3.0	6.0	
J	BMVW-t	MT20	4.0	6.0	
K	BMV1+p	MT20	3.0	4.0	

#### DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	DOWN	GROSS REACTION	DOWN	BRG	IN-SX	BRG	IN-SX
G	1073	0	1073	0	0	1-8	1-8		
K	1197	0	1197	0	0	5-8	5-8		

#### UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
G	759	498 / 0	0 / 0	0 / 0	0 / 0	261 / 0	0 / 0
K	844	567 / 0	0 / 0	0 / 0	0 / 0	277 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

#### BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.69 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, C-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

#### LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-91.8 -91.8	0.12 (1)	10.00	J-C	-8 / 113	0.04 (4)
B-C	-1355 / 0	-91.8 -91.8	0.64 (1)	4.69	C-H	-821 / 0	0.38 (1)
C-D	-649 / 0	-91.8 -91.8	0.58 (1)	6.25	H-E	-216 / 6	0.27 (1)
D-E	-649 / 0	-91.8 -91.8	0.58 (1)	6.25	H-F	0 / 951	0.21 (1)
E-F	-546 / 0	-91.8 -91.8	0.37 (1)	6.25	B-J	0 / 1252	0.28 (1)
G-F	-1034 / 0	0.0 0.0	0.31 (1)	6.18			
K-B	-1144 / 0	0.0 0.0	0.12 (1)	7.43			
K-J	0 / 0	-18.5 -18.5	0.22 (4)	10.00			
J-I	0 / 1243	-18.5 -18.5	0.32 (4)	10.00			
I-H	0 / 1243	-18.5 -18.5	0.32 (4)	10.00			
H-G	0 / 0	-18.5 -18.5	0.15 (4)	10.00			

#### DESIGN CRITERIA

##### SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF  
DL = 6.0 PSF  
BOT CH. LL = 0.0 PSF  
DL = 7.4 PSF  
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) =  $L/360$  (0.65")  
CALCULATED VERT. DEFL. (LL) =  $L/999$  (0.04")  
ALLOWABLE DEFL. (TL) =  $L/360$  (0.65")  
CALCULATED VERT. DEFL. (TL) =  $L/999$  (0.10")

CSI: TC=0.64/1.00 (B-C:1), BC=0.32/1.00 (H-J:4), WB=0.38/1.00 (C-H:1), SSI=0.28/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10  
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

#### NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX MIN
MT20	650 371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.84 (F) (INPUT = 0.90)  
JSI METAL=0.41 (F) (INPUT = 1.00)

CITY OF RICHMOND HILL  
BUILDING DEPARTMENT

09/02/2021

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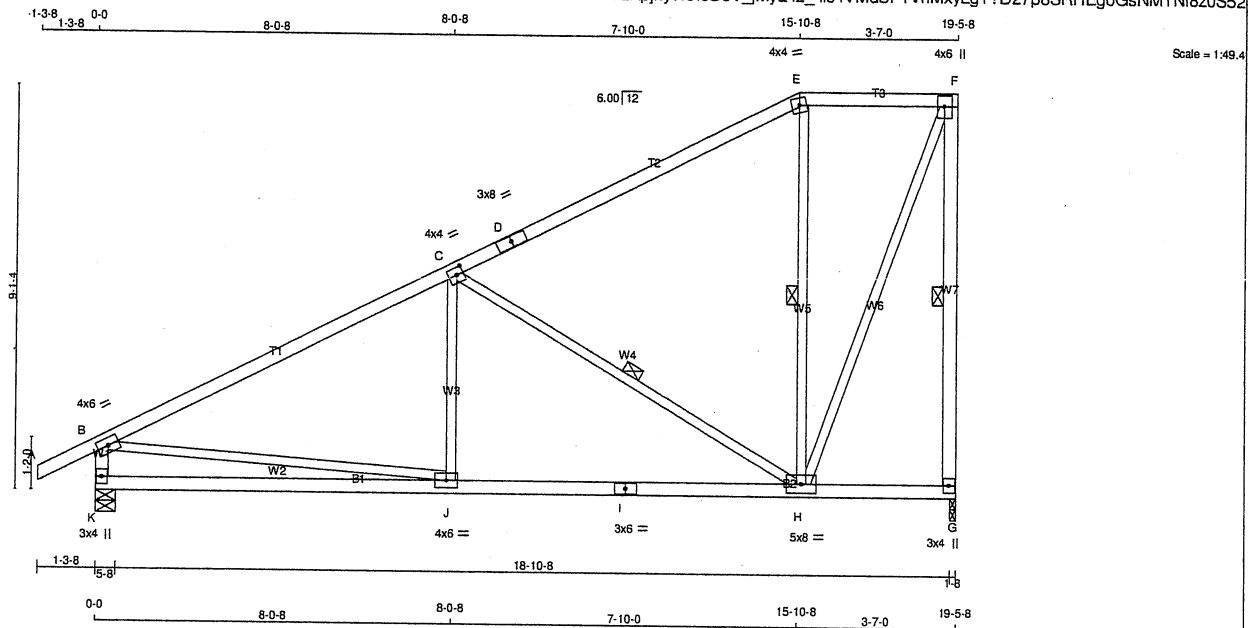
Per: jocelyn.aguilar



Structural component only  
DWG# T-2121212

JOB NAME <b>412865</b>	TRUSS NAME <b>T106</b>	QUANTITY <b>3</b>	PLY <b>1</b>	JOB DESC. <b>ROYAL PINE HOMES</b>	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

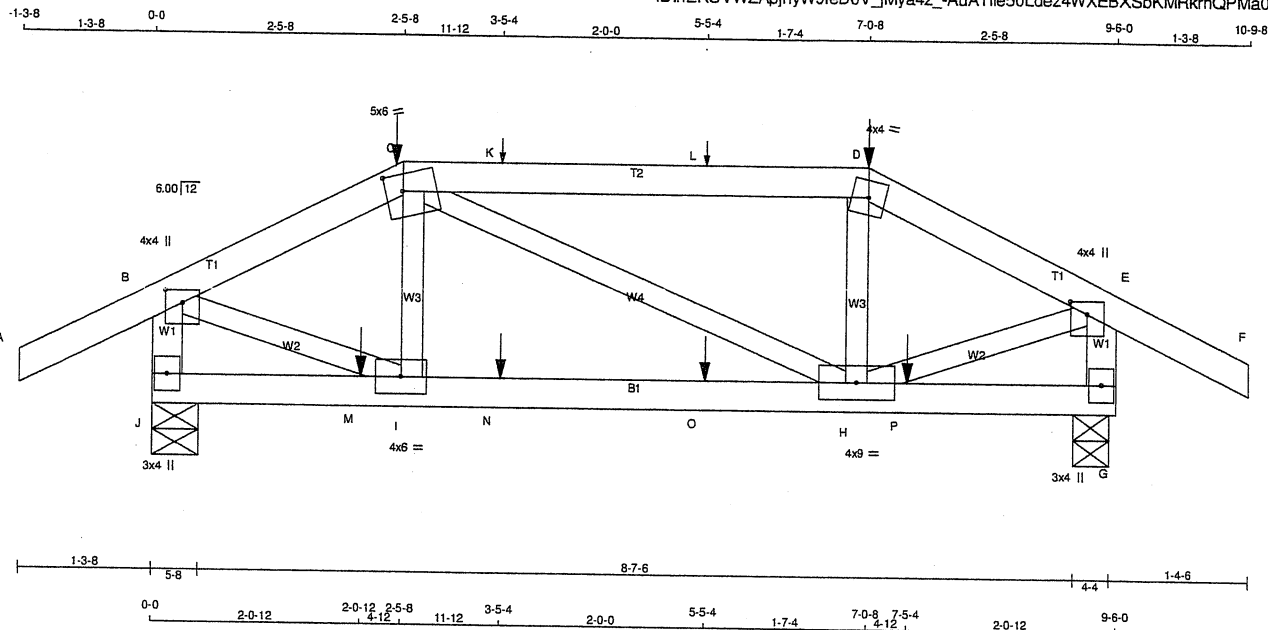
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JOB NAME <b>412865</b>	TRUSS NAME <b>T107</b>	QUANTITY <b>1</b>	PLY <b>1</b>	JOB DESC. <b>ROYAL PINE HOMES</b>	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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TOTAL WEIGHT = 38 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
J - B	2x4	DRY	No.2
G - E	2x4	DRY	No.2
J - G	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2  
EXCEPT

DRY: SEASONED LUMBER.

#### PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.50	2.00
C	TTWW-m	MT20	5.0	6.0	2.00	2.00
D	TTWW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	4.0	1.50	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMVWW-t	MT20	4.0	9.0		
I	BMVWW-t	MT20	4.0	6.0		
J	BMV1+p	MT20	3.0	4.0		

#### DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQRD
JT	GROSS REACTION	GROSS REACTION	DOWN	HORZ	UPLIFT
J	762	0	762	0	0
G	762	0	762	0	0

#### UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
J	535	373 / 0	0 / 0	0 / 0	0 / 0	162 / 0	0 / 0
G	535	373 / 0	0 / 0	0 / 0	0 / 0	162 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

#### BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

#### LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-91.8 -91.8 0.13 (1)	10.00	I-C	-126 / 22	0.02 (1)	
B-C	-678 / 0	-91.8 -91.8 0.11 (1)	6.25	C-H	0 / 0	0.00 (1)	
C-K	-598 / 0	-91.8 -91.8 0.37 (1)	6.25	H-D	-127 / 21	0.02 (1)	
K-L	-598 / 0	-91.8 -91.8 0.37 (1)	6.25	B-I	0 / 638	0.16 (1)	
L-D	-598 / 0	-91.8 -91.8 0.37 (1)	6.25	H-E	0 / 638	0.16 (1)	
D-E	-677 / 0	-91.8 -91.8 0.11 (1)	6.25				
E-F	0 / 28	-91.8 -91.8 0.13 (1)	10.00				
J-B	-745 / 0	0.0 0.0 0.08 (1)	7.81				
G-E	-744 / 0	0.0 0.0 0.08 (1)	7.81				
J-M	0 / 0	-18.5 -18.5 0.06 (4)	10.00				
M-I	0 / 0	-18.5 -18.5 0.06 (4)	10.00				
I-N	0 / 599	-18.5 -18.5 0.15 (1)	10.00				
N-O	0 / 599	-18.5 -18.5 0.15 (1)	10.00				
O-H	0 / 599	-18.5 -18.5 0.15 (1)	10.00				
H-P	0 / 0	-18.5 -18.5 0.06 (4)	10.00				
P-G	0 / 0	-18.5 -18.5 0.06 (4)	10.00				

#### SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-5-8	-74	-74	---	FRONT	VERT	TOTAL	---	C1
D	7-0-8	-74	-74	---	FRONT	VERT	TOTAL	---	C1
K	3-5-4	1	1	---	FRONT	VERT	TOTAL	---	C1
L	5-5-4	1	1	---	FRONT	VERT	TOTAL	---	C1
M	2-0-12	-3	-3	---	FRONT	VERT	TOTAL	---	C1
N	3-5-4	-1	-1	---	FRONT	VERT	TOTAL	---	C1
O	5-5-4	-1	-1	---	FRONT	VERT	TOTAL	---	C1
P	7-5-4	-3	-3	---	FRONT	VERT	TOTAL	---	C1

#### CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

#### DESIGN CRITERIA

##### SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:  
- PART 9 OF BCBC 2018, ABC 2019  
- PART 9 OF OBC 2012 (2019 AMENDMENT)  
- CSA 086-14  
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) =  $L/360$  (0.32")  
CALCULATED VERT. DEFL. (LL) =  $L/999$  (0.01")  
ALLOWABLE DEFL. (TL) =  $L/360$  (0.32")  
CALCULATED VERT. DEFL. (TL) =  $L/999$  (0.02")

CSI: TC=0.37/1.00 (C-D:1), BC=0.15/1.00 (H-I:1), WB=0.16/1.00 (B-I:1), SSI=0.18/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00  
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

#### NAIL VALUES

PLATE GRIP (DRY) SHEAR	SECTION
(PSI)	(PL)
MAX MIN MAX MIN MAX MIN	
MT20	650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.85 (B) (INPUT = 0.90)  
JSI METAL = 0.20 (B) (INPUT = 1.00)



Structural component only  
DWG# T-2121216

09/02/2021

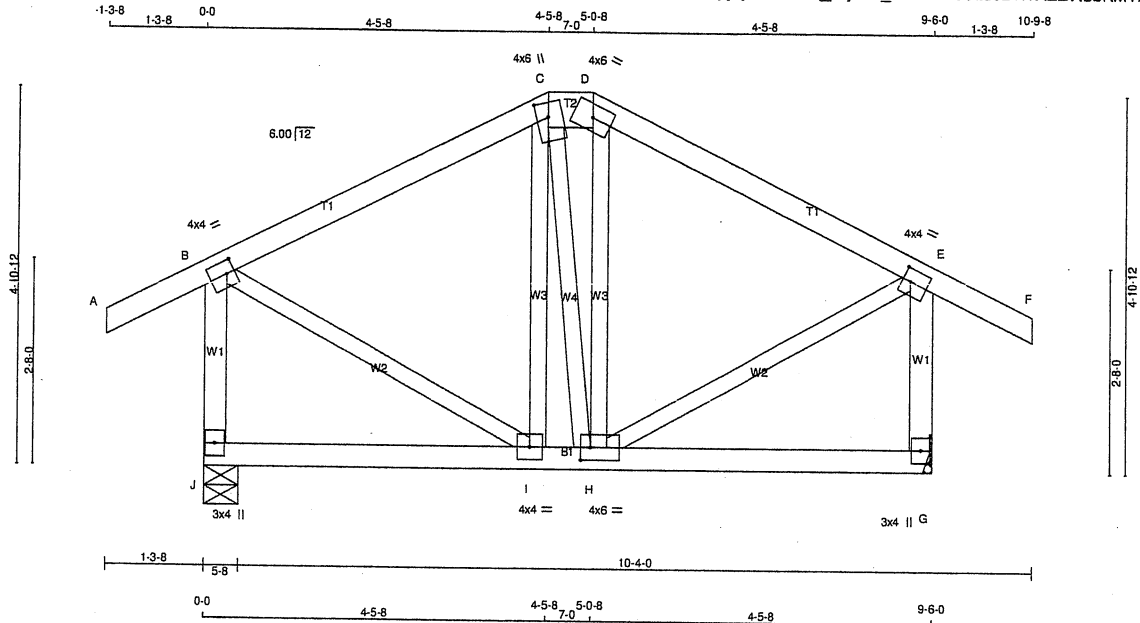
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Per: jocelyn.aguilar



JOB NAME <b>412865</b>	TRUSS NAME <b>T108</b>	QUANTITY <b>2</b>	PLY <b>1</b>	JOB DESC. <b>ROYAL PINE HOMES</b>	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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TOTAL WEIGHT = 2 X 50 = 99 lb  
[M/F]

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - D	2x6	DRY	No.2
D - F	2x4	DRY	No.2
J - B	2x4	DRY	No.2
G - E	2x4	DRY	No.2
J - G	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2  
EXCEPT

DRY: SEASONED LUMBER.

#### PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TTWW+m	MT20	4.0	6.0	2.25	1.75
D	TTW-h	MT20	4.0	6.0		
E	TMVW-t	MT20	4.0	4.0	2.00	1.25
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	6.0	2.00	1.50
I	BMWW-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

#### DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	DOWN	GROSS REACTION	BRG	BRG
J	648	0	648	0	5-8
G	648	0	648	0	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 1-8.

#### UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	456	312/0	0/0	0/0	0/0	144/0	0/0
G	456	312/0	0/0	0/0	0/0	144/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

#### BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

#### LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0/28	-91.8 -91.8	0.12 (1)	10.00	I-C	-88/7	0.03 (1)
B-C	-309/0	-91.8 -91.8	0.23 (1)	6.25	H-D	-96/7	0.03 (1)
C-D	-278/0	-91.8 -91.8	0.00 (1)	6.25	B-I	0/315	0.07 (1)
D-E	-307/0	-91.8 -91.8	0.23 (1)	6.25	H-E	0/314	0.07 (1)
E-F	0/28	-91.8 -91.8	0.12 (1)	10.00	C-H	-7/0	0.00 (1)
J-B	-615/0	0.0 0.0	0.09 (1)	7.81			
G-E	-614/0	0.0 0.0	0.09 (1)	7.81			
J-I	0/0	-18.5 -18.5	0.08 (4)	10.00			
I-H	0/279	-18.5 -18.5	0.10 (4)	10.00			
H-G	0/0	-18.5 -18.5	0.08 (4)	10.00			

#### DESIGN CRITERIA

##### SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
DL = 6.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.4	PSF	
TOTAL LOAD = 39.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) =  $L/360$  (0.32")  
CALCULATED VERT. DEFL. (LL) =  $L/999$  (0.00")  
ALLOWABLE DEFL. (TL) =  $L/360$  (0.32")  
CALCULATED VERT. DEFL. (TL) =  $L/999$  (0.01")

CSI: TC=0.23/1.00 (B-C:1), BC=0.10/1.00 (H-I:4), WB=0.07/1.00 (B-I:1), SSI=0.14/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10  
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

#### NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP=0.45 (B) (INPUT = 0.90)  
JSI METAL=0.18 (B) (INPUT = 1.00)

CITY OF RICHMOND HILL  
BUILDING DEPARTMENT

09/02/2021

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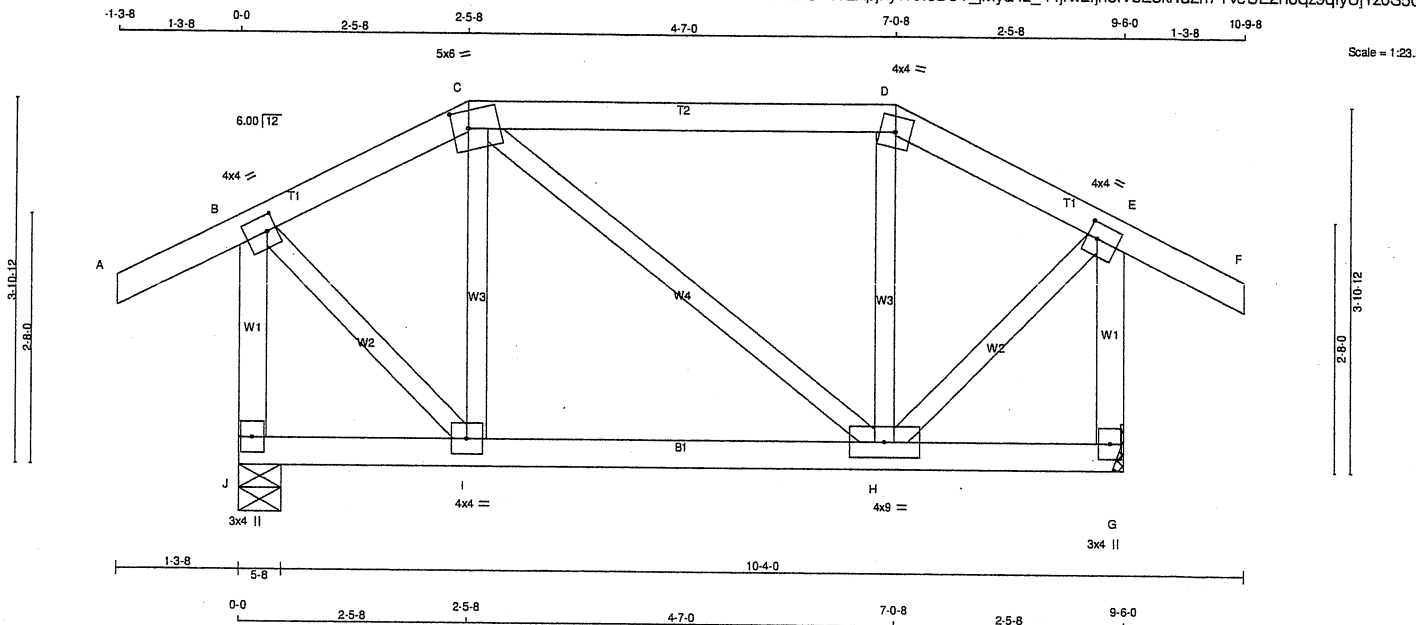
Per: jocelyn.aguilar



Structural component only  
DWG# T-2121217

JOB NAME <b>412865</b>	TRUSS NAME <b>T109</b>	QUANTITY <b>1</b>	PLY <b>1</b>	JOB DESC. <b>ROYAL PINE HOMES</b>	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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LUMBER			
N. L. G. A. RULES	SIZE	DRY	No.2
CHORDS			
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
J - B	2x4	DRY	No.2
G - E	2x4	DRY	No.2
J - G	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2  
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-t	MT20	4.0	4.0	2.00 1.25
C	TTWW-m	MT20	5.0	6.0	2.25 2.00
D	TTW-m	MT20	4.0	4.0	
E	TMVW-t	MT20	4.0	4.0	2.00 1.25
G	BMV1+p	MT20	3.0	4.0	
H	BMVWW-t	MT20	4.0	4.0	9.0
I	BMVW-t	MT20	4.0	4.0	
J	BMV1+p	MT20	3.0	4.0	

#### DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG	
J	VERT	DOWN	HORZ	UPLIFT	IN-SX
J	648	0	648	0	5-8
G	648	0	648	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 1-8.

#### UNFACTORED REACTIONS

JT	1ST CASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
J	456	312 / 0	0 / 0	0 / 0	0 / 0	144 / 0	0 / 0
G	456	312 / 0	0 / 0	0 / 0	0 / 0	144 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

#### BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

#### LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED	FACTORED		MEMB.	MAX. FACTORED	MAX.	
	FORCE	VERT. LOAD	LC1 MAX		FORCE	FORCE	
	(LBS)	(PLF)	CSI (LC)		(LBS)	CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-91.8 -91.8	0.12 (1)	I-C	-183 / 0	0.04 (1)	
B-C	-311 / 0	-91.8 -91.8	0.07 (1)	C-H	0 / 0	0.00 (1)	
C-D	-272 / 0	-91.8 -91.8	0.25 (1)	H-D	-183 / 0	0.04 (1)	
D-E	-311 / 0	-91.8 -91.8	0.07 (1)	B-I	0 / 378	0.09 (1)	
E-F	0 / 28	-91.8 -91.8	0.12 (1)	H-E	0 / 377	0.08 (1)	
J-B	-633 / 0	0.0 0.0	0.09 (1)				
G-E	-632 / 0	0.0 0.0	0.09 (1)				
J-I	0 / 0	-18.5 -18.5	0.06 (4)				
I-H	0 / 273	-18.5 -18.5	0.08 (4)				
H-G	0 / 0	-18.5 -18.5	0.06 (4)				

#### DESIGN CRITERIA

##### SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.32")  
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")  
ALLOWABLE DEFL.(TL)= L/360 (0.32")  
CALCULATED VERT. DEFL.(TL)= L/999 (0.01")

CSI: TC=0.25/1.00 (C-D:1), BC=0.08/1.00 (H-I:4), WB=0.09/1.00 (B-I:1), SS=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10  
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

#### NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CITY OF RICHMOND HILL  
BUILDING DEPARTMENT  
JSL GRIP = 0.52 (B) (INPUT = 0.90)  
JSL METAL = 0.14 (B) (INPUT = 1.00)

09/02/2021

RECEIVED

Per: jocelyn.aguilar

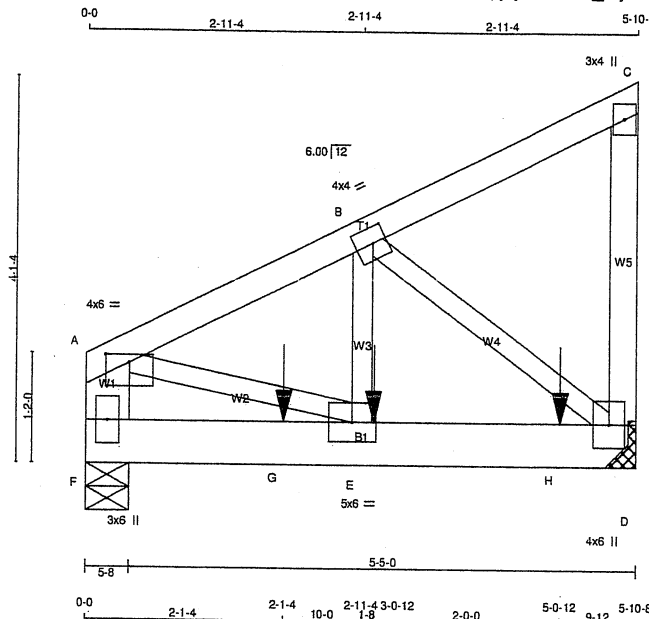


Structural component only  
DWG# T-2121218

JOB NAME <b>412865</b>	TRUSS NAME <b>T110</b>	QUANTITY <b>1</b>	PLY <b>2</b>	JOB DESC. <b>ROYAL PINE HOMES</b>	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 29 = 58 lb

LUMBER				
CHORDS	SIZE	LUMBER	DESCR.	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x6	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF  
EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C 1	12	TOP
C - D 1	12	TOP
F - A 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F - D 2	12	SIDE(183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

#### DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	UPLIFT
D	1408	0	1408	0
F	1130	0	1130	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

#### UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	989	688 / 0	0 / 0	0 / 0	0 / 0	301 / 0	0 / 0
F	794	550 / 0	0 / 0	0 / 0	0 / 0	244 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

#### BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

#### LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LO)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LO)	
FR-TO		FROM TO		FR-TO			
A-B	-1250 / 0	-91.8	-91.8 0.06 (1)	E-B	0 / 1106	0.14 (1)	
B-C	-11 / 0	-91.8	-91.8 0.05 (1)	B-D	-1419 / 0	0.17 (1)	
D-C	-110 / 0	0.0	0.0 0.01 (1)	A-E	0 / 1166	0.14 (1)	
F-A	-971 / 0	0.0	0.0 0.03 (1)				
F-G	0 / 0	-18.5	-18.5 0.11 (1)				
G-E	0 / 0	-18.5	-18.5 0.11 (1)				
E-H	0 / 1128	-18.5	-18.5 0.20 (1)				
H-D	0 / 1128	-18.5	-18.5 0.20 (1)				

#### SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	3-0-12	-441	-441	---	BACK	VERT	TOTAL	---	C1
G	2-1-4	-441	-441	---	BACK	VERT	TOTAL	---	C1
H	5-0-12	-443	-443	---	BACK	VERT	TOTAL	---	C1

#### CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

#### DESIGN CRITERIA

##### SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

##### SPACING = 24.0 IN/C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:  
- PART 9 OF BCBC 2018, ABC 2019  
- PART 9 OF OBC 2012 (2019 AMENDMENT)  
- CSA 086-14  
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")  
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")  
ALLOWABLE DEFL.(TL)= L/360 (0.20")  
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.08/1.00 (A-B:1) , BC=0.20/1.00 (D-E:1) , WB=0.17/1.00 (B-D:1) , SSI=0.17/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00  
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

#### NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.72 (B) (INPUT = 0.90)  
JSI METAL = 0.25 (D) (INPUT = 1.00)

CITY OF RICHMOND HILL  
BUILDING DIVISION

09/02/2021

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Per: jocelyn.aguilar



Structural component only  
DWG# T-2121219

JOB NAME <b>412865</b>	TRUSS NAME <b>T110</b>	QUANTITY <b>1</b>	PLY <b>2</b>	JOB DESC. <b>ROYAL PINE HOMES</b>	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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**PLATES (table is in inches)**

JT	TYPE	PLATES	W	LEN	Y	X
D	BMVW1+p	MT20	4.0	6.0		
E	BMVW-t	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	6.0		



Structural component only  
DWG# T-2121219 *3/12*

CITY OF RICHMOND HILL  
BUILDING DIVISION

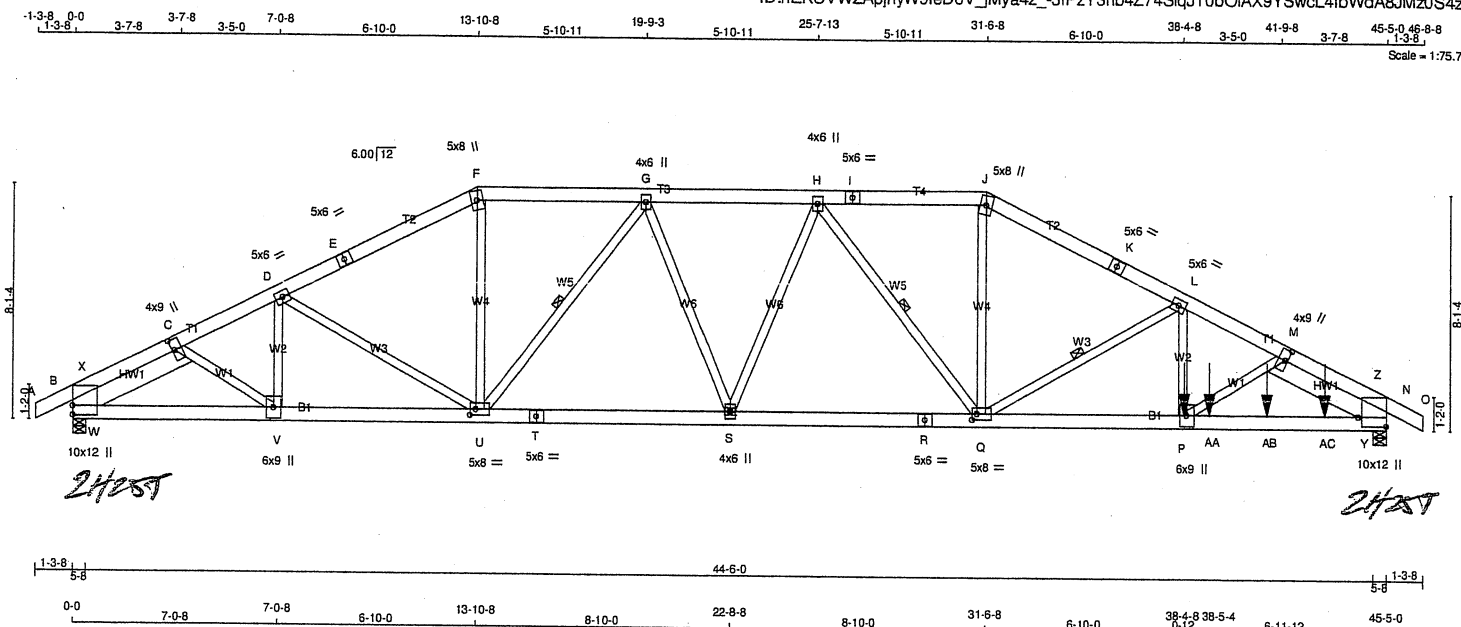
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Per: jocelyn.aguilar



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<b><u>LUMBER</u></b>			
<b>N. L. G. A. RULES</b>			
<b>CHORDS</b>	<b>SIZE</b>	<b>LUMBER</b>	<b>DESCR.</b>
A - E	2x6	DRY No.2	SPF
E - F	2x6	DRY No.2	SPF
F - I	2x6	DRY No.2	SPF
I - J	2x6	DRY No.2	SPF
J - K	2x6	DRY No.2	SPF
K - O	2x6	DRY No.2	SPF
B - T	2x6	DRY No.2	SPF
T - R	2x6	DRY No.2	SPF
R - N	2x6	DRY No.2	SPF
<b>REINFORCING MEMBERS</b>			
HW1	2x8	DRY No.2	SPF
HW2	2x8	DRY No.2	SPF
<b>ALL WEBS 2x4 DRY</b>		<b>No.2</b>	<b>SPF</b>
<b>DRY: SEASONED LUMBER.</b>			

DESIGN CONSISTS OF 3 TRUSSES BUILT  
SEPARATELY THEN FASTENED TOGETHER AS  
FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS			
A-E	2	12	TOP
E-F	2	12	TOP
F-I	2	12	TOP
I-J	2	12	TOP
J-K	2	12	TOP
K-O	2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS			
B-T	2	12	TOP
T-R	2	12	TOP
R-N	2	12	SIDE(451.4)
WEBS : (0.122"x3") SPIRAL NAILS			
L-P	2	4	SIDE(1060.7)
2x4	1	6	
D-V	2	4	
2x8	2	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS



Structural component only  
DWG# T-2121220 *112*

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQ'D BRG
VERT	HORZ	DOWN	HORZ	UPLIFT IN-SX	IN-SX
B	4431 0	4466	193	-1072 5-8	5-8
N	8350 0	8385	0	-1072 5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 1072 LBS FACTORED UPLIFT  
PROVIDE ANCHORAGE AT BEARING JOINT N FOR 1072 LBS FACTORED UPLIFT

PROVIDE FOR 193 LBS FACTORED HORIZONTAL REACTION AT JOINT B

### UNFACTORED REACTIONS

1STLCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	3247	1991 / 0	477 / 0	0 / 0	0 / -1066	802 / 0	0 / 0
B	6011	3847 / 0	477 / 0	0 / 0	59 / -1066	1710 / 0	0 / 0

### HORIZONTAL REACTIONS

B	—	0/0	0/0	0/0	138/-138	0/0	0/0
---	---	-----	-----	-----	----------	-----	-----

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B. N

## BRACING

MAX. UNBRACED TOP CHORD LENGTH = 4.01 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-U, H-Q, L-O

## LOADING

**TOTAL LOAD CASES: (18)**

C H O R D S				W E B S			
MAX. FACTORED		FACTORED		MAX. FACTORED		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX	MAX. UNBRAC	MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO		(PLF)	CSI (LC)	LENGTH	FR-TO		CSI (LC)
A-B	0 / 1	-115.2	-115.2 0.03 (2)	10.00	C-V	-142 / 888	0.05 (6)
B-X	-5083 / 1128	-115.2	-115.2 0.06 (2)	6.03	V-D	-334 / 140	0.02 (8)
X-C	-3427 / 876	-115.2	-115.2 0.05 (2)	6.25	D-U	-793 / 418	0.25 (2)
C-D	-7034 / 1698	-115.2	-115.2 0.14 (2)	5.26	U-F	-408 / 2301	0.12 (1)
D-E	-6551 / 1552	-115.2	-115.2 0.16 (2)	5.40	U-G	-2385 / 487	0.36 (3)
E-F	-6551 / 1552	-115.2	-115.2 0.16 (2)	5.40	G-S	-29 / 1258	0.07 (3)
F-G	-5873 / 1490	-115.2	-115.2 0.10 (1)	5.67	S-H	-968 / 119	0.31 (10)
G-H	-7469 / 1635	-115.2	-115.2 0.11 (1)	5.16	H-Q	-487 / 487	0.07 (8)
H-I	-7589 / 1490	-115.2	-115.2 0.12 (1)	5.13	Q-J	-407 / 3131	0.17 (1)
I-J	-7589 / 1490	-115.2	-115.2 0.12 (1)	5.13	J-K	-5427 / 419	0.56 (3)
J-K	-8446 / 1552	-115.2	-115.2 0.19 (3)	4.86	P-L	0 / 4135	0.22 (2)
K-L	-8446 / 1552	-115.2	-115.2 0.19 (3)	4.86	P-M	-143 / 1628	0.09 (1)
L-M	-13359 / 1699	-115.2	-115.2 0.19 (3)	4.01	W-X	-372 / 2258	0.00 (1)
M-Z	-7226 / 878	-115.2	-115.2 0.11 (3)	5.22	W-Y	-3479 / 777	0.12 (1)
Z-N	-10927 / 1132	-115.2	-115.2 0.12 (3)	4.43	M-Y	-5733 / 777	0.19 (1)
N-O	0 / 1	-115.2	-115.2 0.03 (3)	10.00	Y-Z	-375 / 5268	0.00 (1)
B-W	-844 / 3015	-39.5	-39.5 0.14 (2)	6.25			
W-V	-1434 / 5645	-39.5	-39.5 0.27 (1)	6.25			
V-U	-1531 / 6363	-39.5	-39.5 0.30 (1)	6.25			
U-T	-1368 / 7114	-39.5	-39.5 0.33 (1)	6.25			
T-S	-1368 / 7114	-39.5	-39.5 0.33 (1)	6.25			
S-R	-1314 / 7690	-39.5	-39.5 0.35 (1)	6.25			
R-Q	-1314 / 7690	-39.5	-39.5 0.35 (1)	6.25			
Q-P	-1338 / 12033	-39.5	-39.5 0.54 (1)	6.25			
P-AA	-1243 / 10718	-39.5	-39.5 0.64 (1)	6.25			
AA-AB	-1243 / 10718	-39.5	-39.5 0.64 (1)	6.25			
AB-AC	-1243 / 10718	-39.5	-39.5 0.64 (1)	6.25			
AC-Y	-1243 / 10718	-39.5	-39.5 0.64 (1)	6.25			
Y-N	-653 / 6377	-39.5	-39.5 0.37 (3)	6.25			

## DESIGN CRITERIA

**SPECIFIED LOADS:**

TOP CH.	LL =	33.4	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	57.3	PSF

**SPACING = 24.0 IN. C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE  
OF 2.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL  
OR INDUSTRIAL BUILDING REQUIREMENTS OF  
PART 4. NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 4 OF BCBC 2018 , ABC 2019
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

## DESIGN ASSUMPTIONS

- DESIGN ASSUMPTIONS  
- SLOPE REDUCTION FACTOR NOT USED  
- PERCENTAGE OF GROUND SNOW LOAD IS  
USER-DEFINED.

(80 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 33.4 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL(LL)= L/360 (1.51")  
CALCULATED VERT. DEFL(LL) = L/999 (0.13")  
ALLOWABLE DEFL(TL)= L/180 (3.03")  
CALCULATED VERT. DEFL(TL) = L/999 (0.19")

CSI: TC=0.19/1.00 (J-L:3) , BC=0.64/1.00 (P-Y:1) ,  
WB=0.56/1.00 (L-Q:3) , SSI=0.17/1.00 (N-Y:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10  
COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00  
WIND LOAD IMPORTANCE FACTOR = 1.00  
LIVE LOAD IMPORTANCE FACTOR = 1.00  
COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	GRIP (DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
PLATE	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches  
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (U) (INPUT = 0.90 )  
JSI METAL= 0.92 (M) (INPUT = 1.00 )

Per: jocelyn.aguilar



JOB NAME <b>412865</b>	TRUSS NAME <b>T111</b>	QUANTITY <b>1</b>	PLY <b>3</b>	JOB DESC. <b>ROYAL PINE HOMES</b>	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Thu Jul 1 12:26:40 2021 Page 2  
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**PLATES (table is in inches)**

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMW1+1	MT20	10.0	12.0	3.75	
C	TMWW-t	MT20	4.0	9.0	4.50	1.00
D	TMWW-t	MT20	5.0	6.0		
E, I, K						
E	TS-t	MT20	5.0	6.0		
F	TTW+m	MT20	5.0	8.0		
G	TMWW-t	MT20	4.0	6.0		
H	TMWW-t	MT20	4.0	6.0		
J	TTW+m	MT20	5.0	8.0		
L	TMWW-t	MT20	5.0	6.0		
M	TMWW-t	MT20	4.0	9.0	4.50	1.00
N	TMBMW1+1	MT20	10.0	12.0	3.75	Edge
P	BMWW-t	MT20	6.0	9.0		
Q	BMWW-t	MT20	5.0	8.0	2.50	2.00
R	BS-t	MT20	5.0	6.0		
S	BMWW-t	MT20	4.0	6.0		
T	BS-t	MT20	5.0	6.0		
U	BMWW-t	MT20	5.0	8.0	2.50	2.50
V	BMWW-t	MT20	6.0	9.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

**SPECIFIED CONCENTRATED LOADS (LBS)**

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
P	38-5-4	-2896	-2896	---	BACK	VERT	TOTAL	---	C1
AA	39-3-12	-312	-312	---	BACK	VERT	TOTAL	---	C1
AB	41-3-12	-312	-312	---	BACK	VERT	TOTAL	---	C1
AC	43-3-12	-312	-312	---	BACK	VERT	TOTAL	---	C1

**CONNECTION REQUIREMENTS**

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING  
AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.2) PSF AT (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C<sub>p</sub>C<sub>g</sub>, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM) INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.



Structural component only  
DWG# T-2121220 *ML*

CITY OF RICHMOND HILL  
BUILDING DIVISION

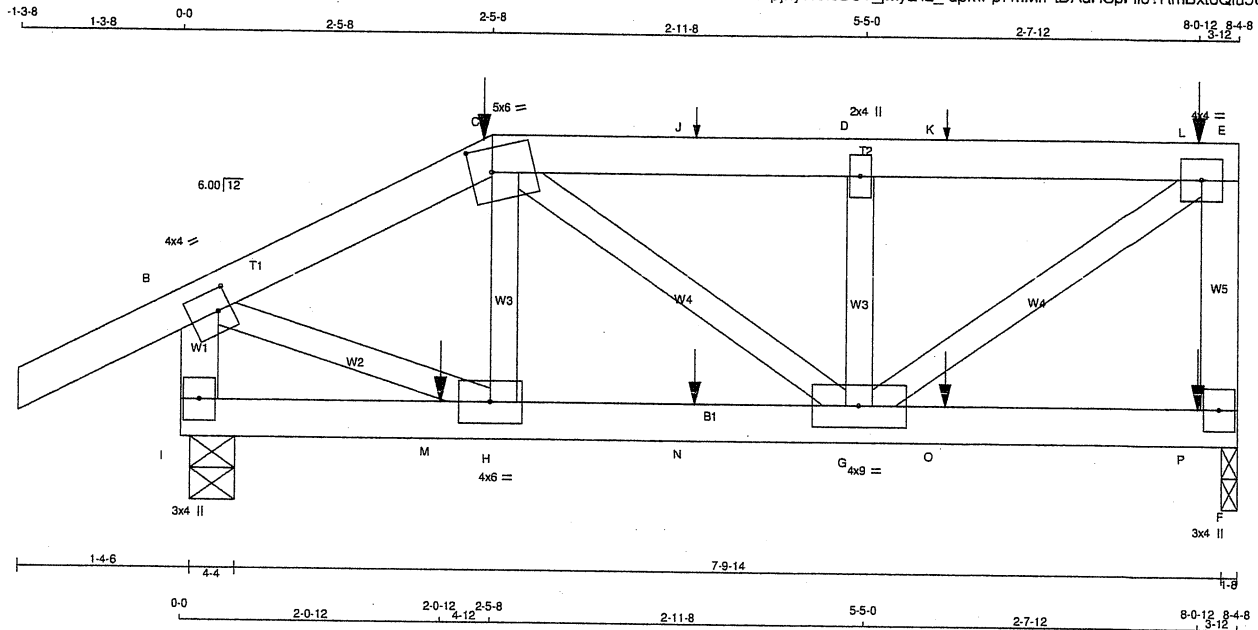
09/02/2021

RECEIVED

Per: jocelyn.aguilar

JOB NAME <b>412865</b>	TRUSS NAME <b>T117</b>	QUANTITY <b>1</b>	PLY <b>1</b>	JOB DESC. <b>ROYAL PINE HOMES</b>	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1:17.3

TOTAL WEIGHT = 34 lb

LUMBER				
N. L. G. A. RULES	SIZE	LUMBER	DESCR.	
CHORDS				
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TTWW-m	MT20	5.0	6.0	2.25	2.00
D	TMVW-w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	4.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMVWW-t	MT20	4.0	9.0		
H	BMVWW-t	MT20	4.0	6.0		
I	BMV1+p	MT20	3.0	4.0		

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
F	527	0	527	0
I	668	0	668	0

**UNFACTORED REACTIONS**

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	373	245 / 0	0 / 0	0 / 0	0 / 0	128 / 0	0 / 0
I	469	327 / 0	0 / 0	0 / 0	0 / 0	142 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, I

**BRACING**

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

**LOADING**

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-91.8 -91.8	0.13 (1)	10.00	H-C	-106 / 11	0.02 (1)
B-C	-546 / 0	-91.8 -91.8	0.11 (1)	6.25	C-G	0 / 44	0.01 (1)
C-J	-518 / 0	-91.8 -91.8	0.14 (1)	6.25	G-D	-332 / 0	0.06 (1)
J-D	-518 / 0	-91.8 -91.8	0.14 (1)	6.25	G-E	0 / 633	0.16 (1)
D-K	-518 / 0	-91.8 -91.8	0.14 (1)	6.25	B-H	0 / 515	0.13 (1)
K-L	-518 / 0	-91.8 -91.8	0.14 (1)	6.25			
L-E	-518 / 0	-91.8 -91.8	0.14 (1)	6.25			
F-E	-494 / 0	0.0 0.0	0.07 (1)	7.81			
I-B	-646 / 0	0.0 0.0	0.07 (1)	7.81			
I-M	0 / 0	-18.5 -18.5	0.03 (4)	10.00			
M-H	0 / 0	-18.5 -18.5	0.03 (4)	10.00			
H-N	0 / 483	-18.5 -18.5	0.11 (1)	10.00			
N-G	0 / 483	-18.5 -18.5	0.11 (1)	10.00			
G-O	0 / 0	-18.5 -18.5	0.04 (4)	10.00			
O-P	0 / 0	-18.5 -18.5	0.04 (4)	10.00			
P-F	0 / 0	-18.5 -18.5	0.04 (4)	10.00			

**SPECIFIED CONCENTRATED LOADS (LBS)**

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-5-8	-74	1	---	FRONT	VERT	TOTAL	---	C1
J	4-0-12	1	1	---	FRONT	VERT	TOTAL	---	C1
K	6-0-12	1	1	---	FRONT	VERT	TOTAL	---	C1
L	8-0-12	-17	-17	---	FRONT	VERT	TOTAL	---	C1
M	2-0-12	-3	-3	---	FRONT	VERT	TOTAL	---	C1
N	4-0-12	-1	-1	---	FRONT	VERT	TOTAL	---	C1
O	6-0-12	-1	-1	---	FRONT	VERT	TOTAL	---	C1
P	8-0-12	-6	-6	---	FRONT	VERT	TOTAL	---	C1

**CONNECTION REQUIREMENTS**

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

**DESIGN CRITERIA**

**SPECIFIED LOADS:**

TOP CH.	LL = 25.6	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

**SPACING = 24.0 IN. C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.28")  
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")  
ALLOWABLE DEFL.(TL) = L/360 (0.28")  
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.14/1.00 (D-E:1), BC=0.11/1.00 (G-H:1), WB=0.16/1.00 (E-G:1), SSI=0.15/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00  
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

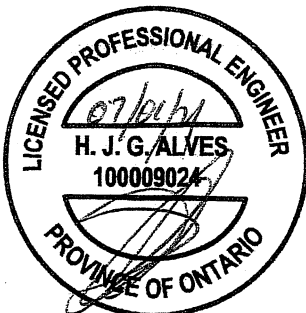
**NAIL VALUES**

PLATE GRIP(DRY)	PLATE GRIP(WET)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.79 (B) (INPUT = 0.90)  
JSI METAL = 0.22 (B) (INPUT = 1.00)



Structural component only  
DWG# T-2121226

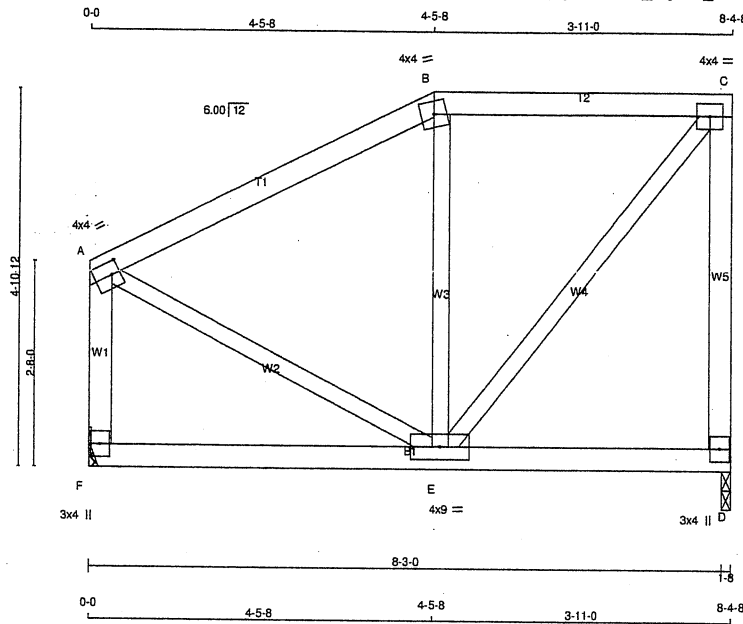
09/02/2021

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Per: jocelyn.aguiar

JOB NAME <b>412865</b>	TRUSS NAME <b>T118</b>	QUANTITY <b>1</b>	PLY <b>1</b>	JOB DESC. <b>ROYAL PINE HOMES</b>	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 1 12:26:47 2021 Page 1  
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Scale = 1/28.5

TOTAL WEIGHT = 39 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF	
B - C	2x4	DRY	No.2	SPF	
D - C	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2  
EXCEPT

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0	2.00	1.25
B	TTW-m	MT20	4.0	4.0		
C	TMVW-t	MT20	4.0	4.0		
D	BMV1+p	MT20	4.0	4.0		
E	BMVWW-t	MT20	4.0	9.0		
F	BMV1+p	MT20	3.0	4.0		

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	UPLIFT	IN-SX
D	462	0	462	0
F	462	0	462	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1'-8".

**UNFACTORED REACTIONS**

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
D	326	214 / 0	0 / 0	0 / 0	0 / 0	0 / 0	112 / 0	0 / 0
F	326	214 / 0	0 / 0	0 / 0	0 / 0	0 / 0	112 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

**BRACING**

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

**LOADING**

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH
FR-TO						FR-TO			
A-B	-245 / 0	-91.8	-91.8	0.23 (1)	6.25	E-B	-275 / 0	0.10 (1)	6.25
B-C	-212 / 0	-91.8	-91.8	0.18 (1)	6.25	E-C	0 / 333	0.07 (1)	7.81
D-C	-436 / 0	0.0	0.0	0.17 (1)	7.81	A-E	0 / 246	0.06 (1)	7.81
F-A	-427 / 0	0.0	0.0	0.06 (1)	7.81				
F-E	0 / 0	-18.5	-18.5	0.09 (4)	10.00				
E-D	0 / 0	-18.5	-18.5	0.09 (4)	10.00				

**DESIGN CRITERIA**

**SPECIFIED LOADS:**

TOP CH.	LL	= 25.6	PSF
	DL	= 6.0	PSF
BOT CH.	LL	= 0.0	PSF
	DL	= 7.4	PSF
TOTAL LOAD		= 39.0	PSF

**SPACING = 24.0 IN/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.28")  
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")  
ALLOWABLE DEFL.(TL)= L/360 (0.28")  
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.23/1.00 (A-B:1), BC=0.09/1.00 (E-F:4), WB=0.10/1.00 (B-E:1), SS=0.14/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10  
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES  
PLATE GRIP(DRY) SHEAR SECTION  
(PSI) (PLI) (PLI)  
MAX MIN MAX MIN MAX MIN  
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP 0.36 (A) (INPUT = 0.90)  
SI METAL= 0.11 (A) (INPUT = 1.00)

CITY OF RICHMOND HILL  
BUILDING DEPARTMENT

09/02/2021

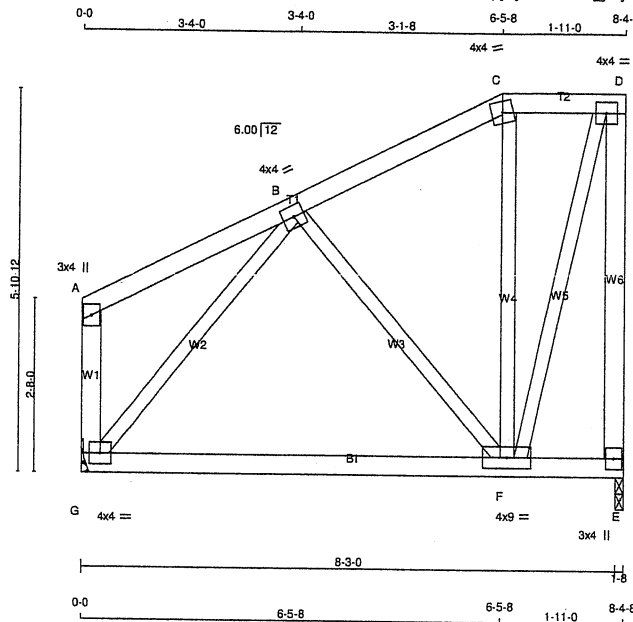
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Per: jocelyn.aguiar



Structural component only  
DWG# T-2121227

JOB NAME <b>412865</b>	TRUSS NAME <b>T119</b>	QUANTITY <b>1</b>	PLY <b>1</b>	JOB DESC. <b>ROYAL PINE HOMES</b>	DRWG NO.
Tamarack Roof Truss, Burlington					



Scale = 1:33.8

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF		
C - D	2x4	DRY	No.2	SPF		
E - D	2x4	DRY	No.2	SPF		
G - A	2x4	DRY	No.2	SPF		
G - E	2x4	DRY	No.2	SPF		

ALL WEBS 2x3 DRY No.2  
EXCEPT

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMVW-t	MT20	4.0	4.0		
C	TTW-m	MT20	4.0	4.0		
D	TMVW-t	MT20	4.0	4.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMVWV-t	MT20	4.0	9.0		
G	BMVW1-t	MT20	4.0	4.0		

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
GROSS REACTION	GROSS REACTION	BRG	BRG	
JT	VERT	HORZ	UPLIFT	IN-SX
E	462	0	462	0
G	462	0	462	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 1-8.

**UNFACTORED REACTIONS**

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	326	214 / 0	0 / 0	0 / 0	0 / 0	112 / 0	0 / 0
G	326	214 / 0	0 / 0	0 / 0	0 / 0	112 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

**BRACING**

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

**LOADING**

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 16	-91.8 -91.8	0.16 (1)	10.00	B-F	-175 / 0	0.08 (1)
B-C	-162 / 0	-91.8 -91.8	0.12 (1)	6.25	F-C	-135 / 0	0.07 (1)
C-D	-128 / 0	-91.8 -91.8	0.04 (1)	6.25	F-D	0 / 416	0.09 (1)
E-D	-483 / 0	0.0 0.0	0.32 (1)	7.81	G-B	-385 / 0	0.16 (1)
G-A	-113 / 0	0.0 0.0	0.02 (1)	7.81			
G-F	0 / 243	-18.5 -18.5	0.19 (4)	10.00			
F-E	0 / 0	-18.5 -18.5	0.18 (4)	10.00			

TOTAL WEIGHT = 45 lb

**DESIGN CRITERIA**

**SPECIFIED LOADS:**

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF	

**SPACING = 24.0 IN C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) =  $L/360$  (0.28")  
CALCULATED VERT. DEFL. (LL) =  $L/999$  (0.01")  
ALLOWABLE DEFL. (TL) =  $L/360$  (0.28")  
CALCULATED VERT. DEFL. (TL) =  $L/999$  (0.05")

CSI: TC=0.32/1.00 (D-E:1), BC=0.19/1.00 (F-G:4), WB=0.16/1.00 (B-G:1), SS=0.13/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10  
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES  
PLATE GRIP (DRY) SHEAR SECTION  
(PSI) (PLI) (PLI)  
MAX MIN MAX MIN MAX MIN  
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.  
SI GRIP = 0.55 (INPUT = 0.90)  
SI METAL = 0.12 (D) (INPUT = 1.00)

CITY OF RICHMOND HILL  
BUILDING DEPARTMENT

09/02/2021

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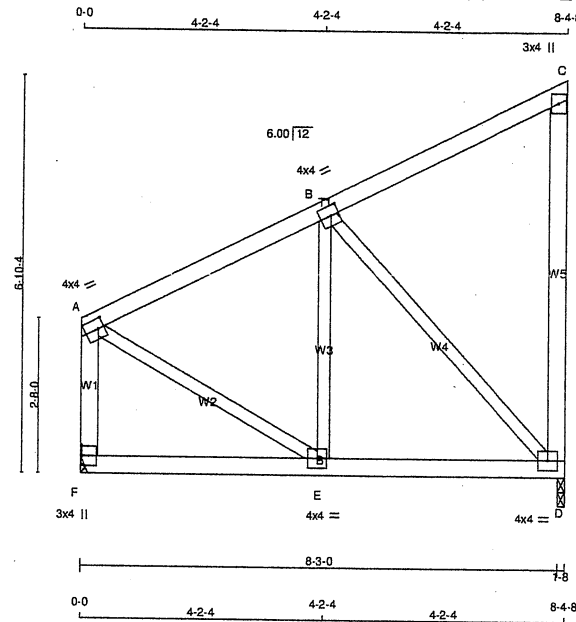
Per: jocelyn.aguiar



Structural component only  
DWG# T-2121228

JOB NAME <b>412865</b>	TRUSS NAME <b>T120</b>	QUANTITY <b>1</b>	PLY <b>1</b>	JOB DESC. <b>ROYAL PINE HOMES</b>	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.420 S Jan 21 2021 MITek Industries, Inc. Thu Jul 1 12:26:48 2021 Page 1  
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Scale = 1:37.9

TOTAL WEIGHT = 42 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4 DRY	No.2	SPF
D - C	2x4 DRY	No.2	SPF
F - A	2x4 DRY	No.2	SPF
F - D	2x4 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF
EXCEPT			

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0	2.00	1.25
B	TMVW-t	MT20	4.0	4.0	2.00	1.75
C	TMV+p	MT20	3.0	4.0		
D	BMVW1-t	MT20	4.0	4.0		
E	BMVW-t	MT20	4.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

FACTORED	MAXIMUM FACTORED	INPUT	REQRD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	UPLIFT
D	462	0	0
F	462	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

**UNFACTORED REACTIONS**

1ST CASE	MAX./MIN. COMPONENT REACTIONS
JT	COMBINED
D	326
F	326

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

**BRACING**

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

**LOADING**

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 CS1 (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 CS1 (LC)
FR-TO		FROM TO			FR-TO		
A-B	-280 / 0	-91.8 -91.8	0.21 (1)	6.25	E-B	-58 / 56	0.02 (4)
B-C	-21 / 0	-91.8 -91.8	0.21 (1)	6.25	B-D	-393 / 0	0.24 (1)
D-C	-145 / 0	0.0 0.0	0.14 (1)	7.81	A-E	0 / 309	0.07 (1)
F-A	-431 / 0	0.0 0.0	0.06 (1)	7.81			
F-E	0 / 0	-18.5 -18.5	0.09 (4)	10.00			
E-D	0 / 269	-18.5 -18.5	0.11 (4)	10.00			

**DESIGN CRITERIA**

**SPECIFIED LOADS:**

TOP CH.	LL = 25.6 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 39.0 PSF

**SPACING = 24.0 IN. C/C**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.28")  
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")  
ALLOWABLE DEFL.(TL)= L/360 (0.28")  
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.21/1.00 (A-B:1), BC=0.11/1.00 (D-E:4), WB=0.24/1.00 (B-D:1), SSI=0.17/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10  
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

**NAIL VALUES**

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

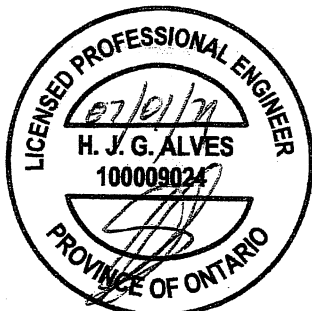
JSI GRIP= 0.44 (A) (INPUT = 0.90)  
JSI METAL= 0.12 (A) (INPUT = 1.00)

CITY OF RICHMOND HILL  
BUILDING DIVISION

09/02/2021

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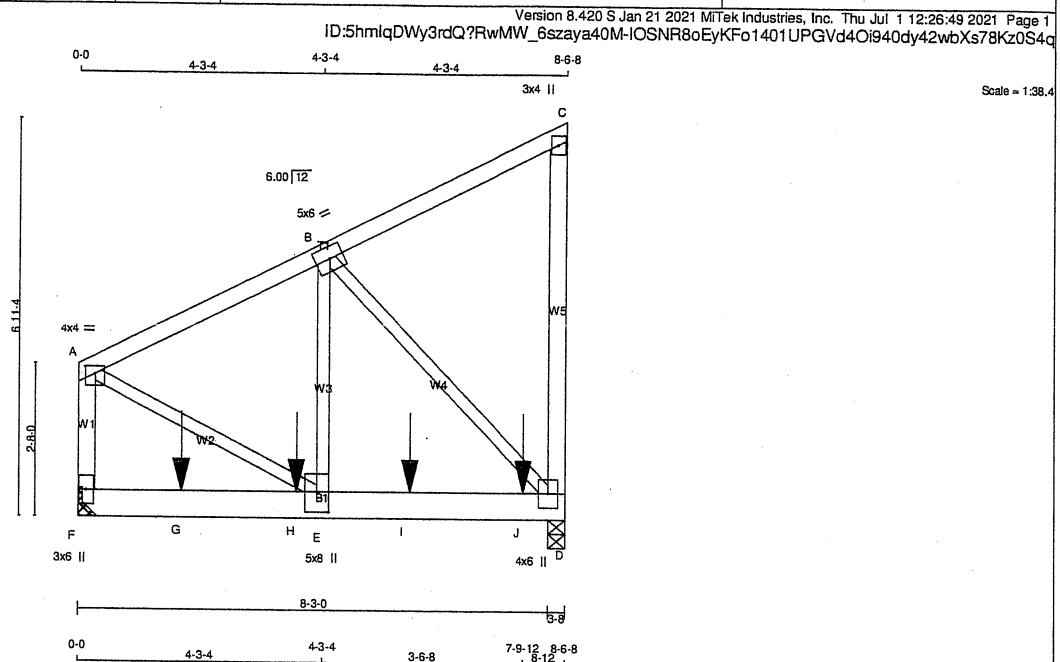
Per: jocelyn.aguilar



Structural component only  
DWG# T-2121229



JOB NAME <b>412865</b>	TRUSS NAME <b>T121</b>	QUANTITY <b>1</b>	PLY <b>3</b>	JOB DESC. <b>ROYAL PINE HOMES</b>	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
F - A	2x4	DRY	No.2
A - C	2x4	DRY	No.2
D - C	2x4	DRY	No.2
F - D	2x6	DRY	No.2
ALL WEBS 2x3 DRY			
DRY: SEASONED LUMBER.			

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
F-A 1	12	TOP
A-C 1	12	TOP
C-D 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F-D 2	6	SIDE(545.9)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

#### PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	4.0	1.50	2.00
B	TMVW-t	MT20	5.0	6.0		
C	TMV-p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		

#### DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQRD
JT	VERT	HORZ	DOWN	UP	BRG
F	4121	0	4121	0	MECHANICAL
D	5184	0	5184	0	3-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 4-0.

#### UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS	2ND LCASE	MAX. MIN. COMPONENT REACTIONS
F	2911	1927 / 0	0 / 0	0 / 0
D	3662	2424 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

#### BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.99 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

#### LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
F-A	-3150 / 0	A-E	0 / 3284
A-B	-3223 / 0	E-B	0 / 3972
B-C	-18 / 0	B-D	-4167 / 0
D-C	-156 / 0		
F-G	0 / 0		
G-H	0 / 0		
H-E	0 / 0		
E-I	0 / 2899		
I-J	0 / 2899		
J-D	0 / 2899		

#### SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
G	1-9-12	-1476	-1476	---	BACK	VERT	---	C1
H	3-9-12	-1476	-1476	---	BACK	VERT	---	C1
I	5-9-12	-1476	-1476	---	BACK	VERT	---	C1
J	7-9-12	-1476	-1476	---	BACK	VERT	---	C1

#### CONNECTION REQUIREMENTS

- C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 3 X 47 = 141 lb

#### DESIGN CRITERIA

##### SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

##### SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

##### THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.28")  
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")  
ALLOWABLE DEFL.(TL) = L/360 (0.28")  
CALCULATED VERT. DEFL.(TL) = L/999 (0.06")

CSI: TC=0.14/1.00 (A-F:1), BC=0.59/1.00 (D-E:1), WB=0.85/1.00 (B-D:1), SSI=0.66/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10  
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

##### NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (A) (INPUT = 0.90)  
JSI METAL= 0.44 (D) (INPUT = 1.00)

CITY OF RICHMOND HILL  
BUILDING DIVISION

09/02/2021

RECEIVED

Per: jocelyn.aguilar



Structural component only  
DWG# T-2121230

JOB NAME <b>412865</b>	TRUSS NAME <b>T121</b>	QUANTITY <b>1</b>	PLY <b>3</b>	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 1 12:26:49 2021 Page 2  
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**PLATES (table is in inches)**

JT	TYPE	PLATES	W	LEN	Y	X
E	BMWw+t	MT20	5.0	8.0	4.25	2.50
F	BMV1+p	MT20	3.0	6.0		



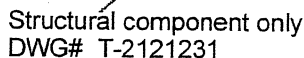
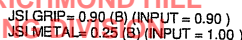
Structural component only  
DWG# T-2121230 *ML*

CITY OF RICHMOND HILL  
BUILDING DIVISION

09/02/2021

RECEIVED

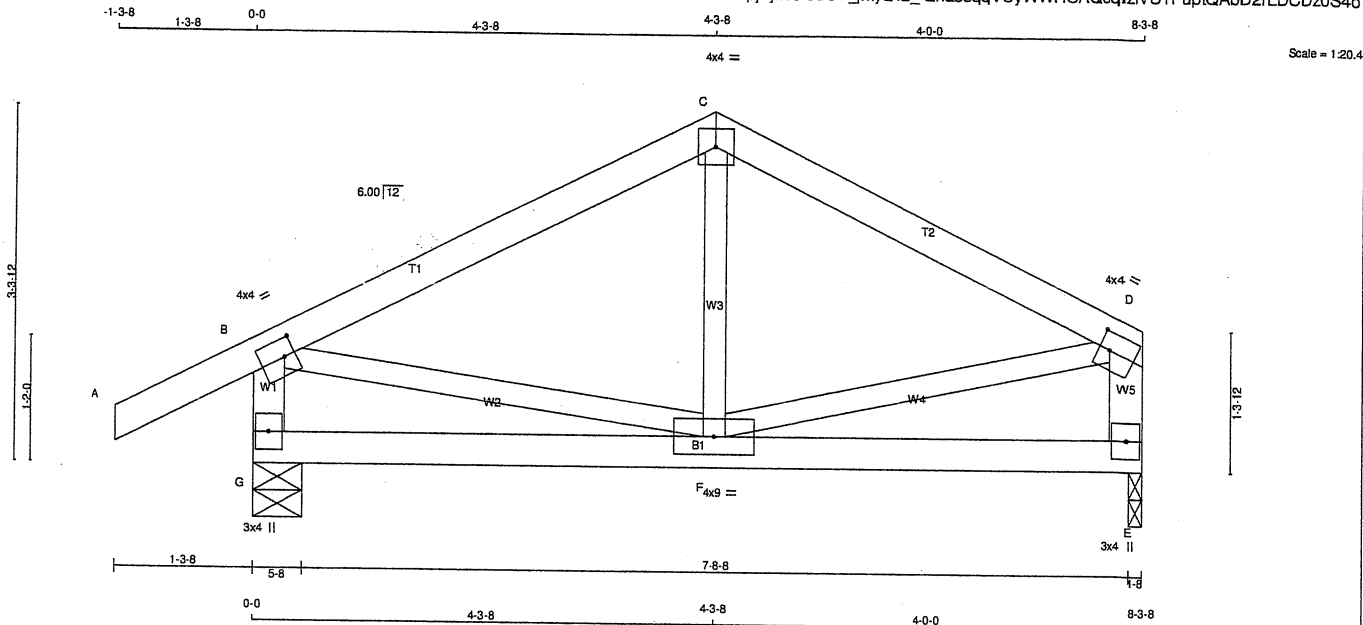
Per: jocelyn.aguilar

Scale = 1:20,000

Per: jocelyn.aguilar

JOB NAME <b>412865</b>	TRUSS NAME <b>T123</b>	QUANTITY <b>1</b>	PLY <b>1</b>	JOB DESC. <b>ROYAL PINE HOMES</b>	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 1 12:26:51 2021 Page 1  
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LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - B	2x4	DRY	No.2
G - B	2x4	DRY	No.2
E - D	2x4	DRY	No.2
G - E	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-t	MT20	4.0	4.0	2.00 1.25
C	TTW-p	MT20	4.0	4.0	
D	TMVW-t	MT20	4.0	4.0	2.00 1.25
E	BMV1+p	MT20	3.0	4.0	
F	BMVWW-t	MT20	4.0	9.0	
G	BMV1+p	MT20	3.0	4.0	

#### DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	GROSS REACTION	BRG	BRG
GT	VERT	HORZ	DOWN	HORZ	UPLIFT
G	582	0	582	0	0
E	457	0	457	0	0

#### UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	PERM.LIVE	WIND	DEAD	SOIL
G	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
G	409	282 / 0	0 / 0	0 / 0	127 / 0	0 / 0
E	323	212 / 0	0 / 0	0 / 0	111 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, E

#### BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

#### LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-91.8 -91.8	0.12 (1)	10.00	F-C	-54 / 52	0.02 (4)
B-C	-364 / 0	-91.8 -91.8	0.22 (1)	6.25	B-F	0 / 332	0.07 (1)
C-D	-364 / 0	-91.8 -91.8	0.19 (1)	6.25	F-D	0 / 336	0.08 (1)
G-B	-550 / 0	0.0 0.0	0.06 (1)	7.81			
E-D	-428 / 0	0.0 0.0	0.04 (1)	7.81			
G-F	0 / 0	-18.5 -18.5	0.09 (4)	10.00			
F-E	0 / 0	-18.5 -18.5	0.09 (4)	10.00			

#### DESIGN CRITERIA

##### SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
DL = 6.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.4	PSF	
TOTAL LOAD = 39.0	PSF	

##### SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.28")  
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")  
ALLOWABLE DEFL.(TL) = L/360 (0.28")  
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.22/1.00 (B-C:1), BC=0.09/1.00 (F-G:4), WB=0.08/1.00 (D-F:1), SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10  
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

#### NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.56 (B) (INPUT = 0.90)  
JSI METAL= 0.17 (B) (INPUT = 1.00)

CITY OF RICHMOND HILL  
BUILDING DIVISION

09/02/2021

RECEIVED

Per: jocelyn.aguilar



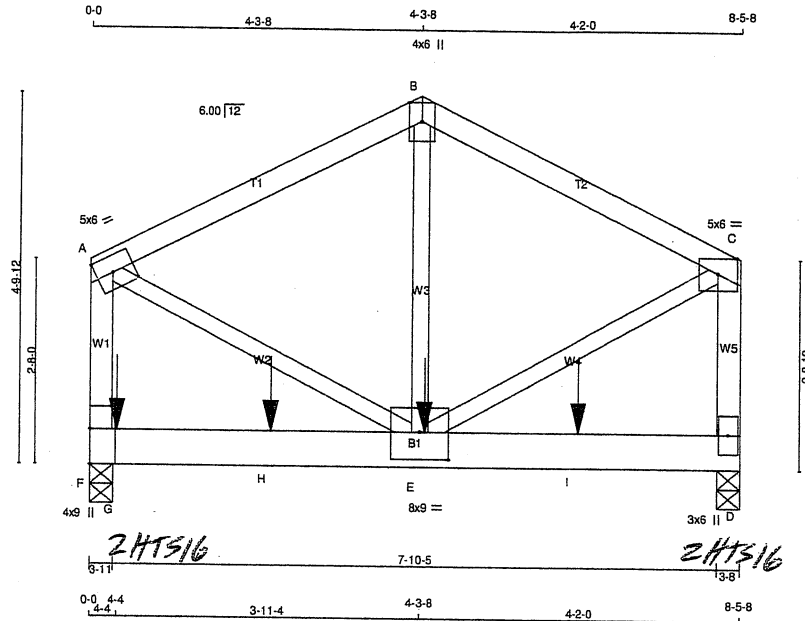
Structural component only  
DWG# T-2121232

JOB NAME <b>412865</b>	TRUSS NAME <b>T127</b>	QUANTITY <b>1</b>	PLY <b>3</b>	JOB DESC. <b>ROYAL PINE HOMES</b>	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 1 15:50:48 2021 Page 1  
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Scale = 1:28.5



2HTS16

2HTS16

TOTAL WEIGHT = 3 X 41 = 124 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4 DRY	No.2	SPF		
B - C	2x4 DRY	No.2	SPF		
F - A	2x4 DRY	No.2	SPF		
D - C	2x4 DRY	No.2	SPF		
F - D	2x6 DRY	No.2	SPF		

ALL WEBS	2x3 DRY	No.2	SPF
EXCEPT			

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - B	12	TOP
B - C	12	TOP
F - A	4	SIDE(419.6)
D - C	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F - D	4	SIDE(1383.3)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

#### DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ
F	9505	0	9505	179
D	6418	0	6418	0

PROVIDE ANCHORAGE AT BEARING JOINT F FOR 2751 LBS FACTORED UPLIFT  
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 1848 LBS FACTORED UPLIFT

PROVIDE FOR 179 LBS FACTORED HORIZONTAL REACTION AT JOINT F

#### UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	6997	4224 / 0	1190 / 0	0 / 0	167 / -2729	1584 / 0	0 / 0
D	4724	2852 / 0	803 / 0	0 / 0	117 / -1836	1069 / 0	0 / 0

#### HORIZONTAL REACTIONS

F	0 / 0	0 / 0	0 / 0	128 / -126	0 / 0	0 / 0
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BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, D

#### BRACING

MAX. UNBRACED TOP CHORD LENGTH = 4.89 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

#### LOADING

TOTAL LOAD CASES: (18)

CHORDS	MAX. FACTORED	FACTORED	MAX. FACTORED	WEBS	MAX. FACTORED	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1 (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO		FR-TO		
A - B	-5261 / 1585	-115.2 -115.2	0.19 (2)	4.89	E - B	-1236 / 4312
B - C	-5261 / 1587	-115.2 -115.2	0.18 (3)	4.90	A - E	-1523 / 5326
F - A	-5094 / 1494	0.0 0.0	0.22 (1)	6.38	E - C	-1544 / 5396
D - C	-5232 / 1532	0.0 0.0	0.23 (1)	6.31		
F - G	-133 / 154	-39.5 -39.5	0.66 (1)	6.25		
G - H	-133 / 154	-39.5 -39.5	0.66 (1)	6.25		
H - E	-133 / 154	-39.5 -39.5	0.66 (1)	6.25		
E - I	-23 / 47	-39.5 -39.5	0.65 (3)	6.25		
I - D	-23 / 47	-39.5 -39.5	0.65 (3)	6.25		

#### SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
E	4-4-4	-2685	-2685	605	FRONT	VERT	TOTAL	C1
G	4-4	-2697	-2697	601	FRONT	VERT	TOTAL	C1
H	2-4-4	-2685	-2685	605	FRONT	VERT	TOTAL	C1
I	6-4-4	-2685	-2685	605	FRONT	VERT	TOTAL	C1

#### CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(b)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.2 } PSF AT (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C<sub>pe</sub>, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.

#### DESIGN CRITERIA

##### SPECIFIED LOADS:

TOP CH.	LL = 33.4 PSF
	DL = 6.0 PSF
BOT CH.	LL = 10.5 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 57.3 PSF

SPACING = 24.0 IN/C/C

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 4 OF BCBC 2018, ABC 2019
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

##### DESIGN ASSUMPTIONS

- SLOPE REDUCTION FACTOR NOT USED
- PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED.

(80 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR = EQUALS 33.4 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.28")  
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")  
ALLOWABLE DEFL.(TL) = L/180 (0.56")  
CALCULATED VERT. DEFL.(TL) = L/999 (0.07")

CSI: TC=0.29/1.00 (C-D:1), BC=0.66/1.00 (E-F:1), WB=0.40/1.00 (C-E:1), SSI=0.89/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10  
COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00  
WIND LOAD IMPORTANCE FACTOR = 1.00  
LIVE LOAD IMPORTANCE FACTOR = 1.00  
COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

##### NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650	371	1747

PLATE PLACEMENT TOL = 0.250 inches

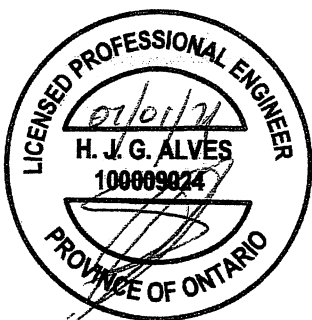
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.87 (A) (INPUT = 0.90)  
JSI METAL= 0.41 (A) (INPUT = 1.00)

CITY OF RICHMOND HILL  
BUILDING DIVISION

09/03/2021

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Per: jocelyn.aguiar



Structural component only  
DWG# T-2121233

CONTINUED ON PAGE 2



JOB NAME <b>412865</b>	TRUSS NAME <b>T127</b>	QUANTITY <b>1</b>	PLY <b>3</b>	JOB DESC. <b>ROYAL PINE HOMES</b>	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 1 15:50:49 2021 Page 2  
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**PLATES (table is in inches)**

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0		Edge
B	TTW+p	MT20	4.0	6.0		Edge
C	TMVW-p	MT20	5.0	6.0		Edge
D	BMV1+p	MT20	3.0	6.0		
E	BMWWW-t	MT20	8.0	9.0	4.25	4.50
F	BMV1+t	MT20	4.0	9.0		5.50

Edge - INDICATES REFERENCE CORNER OF PLATE  
TOUCHES EDGE OF CHORD.



Structural component only  
DWG# T-2121233 *3AL*

CITY OF RICHMOND HILL  
BUILDING DIVISION

09/02/2021

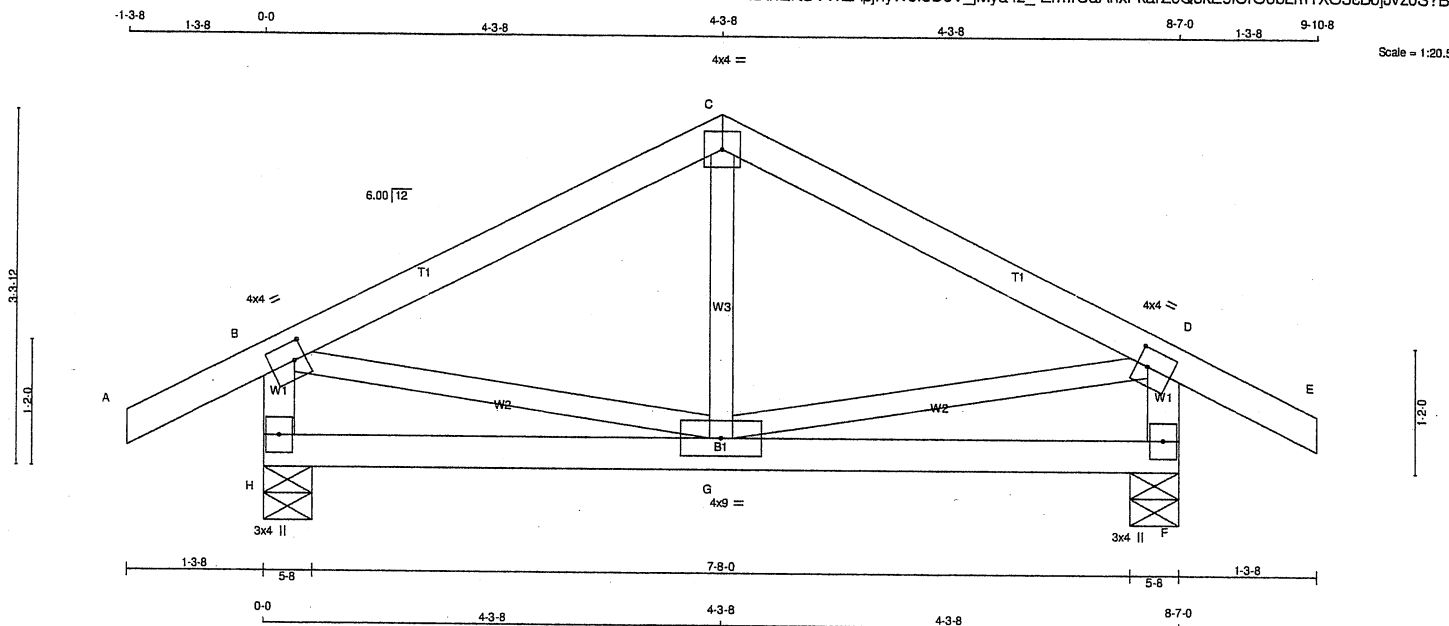
RECEIVED

Per: jocelyn.aguilar

JOB NAME <b>413086</b>	TRUSS NAME <b>T141</b>	QUANTITY <b>1</b>	PLY <b>1</b>	JOB DESC. <b>ROYAL PINE HOMES</b>	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MITek Industries, Inc. Thu Jul 1 12:32:50 2021 Page 1

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TOTAL WEIGHT = 35 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
H - B	2x4	DRY	No.2
F - D	2x4	DRY	No.2
H - F	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TTW-p	MT20	4.0	4.0		
D	TMVW-t	MT20	4.0	4.0	2.00	1.25
F	BMV1+p	MT20	3.0	4.0		
G	BMVWW-t	MT20	4.0	9.0		
H	BMV1+p	MT20	3.0	4.0		

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQD
	VERT	HORZ	DOWN	HORZ
H	598	0	598	0
F	598	0	598	0

**UNFACTORED REACTIONS**

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE			
H	420	289 / 0	0 / 0	0 / 0	0 / 0	0 / 0
F	420	289 / 0	0 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

**BRACING**

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

**LOADING**

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO				FR-TO			
A-B	0 / 28	-91.8 -91.8	0.12 (1)	G-C	-44 / 58	0.02 (4)	
B-C	-391 / 0	-91.8 -91.8	0.22 (1)	B-G	0 / 357	0.08 (1)	
C-D	-391 / 0	-91.8 -91.8	0.22 (1)	G-D	0 / 357	0.08 (1)	
D-E	0 / 28	-91.8 -91.8	0.12 (1)				
H-B	-566 / 0	0.0 0.0	0.06 (1)				
F-D	-566 / 0	0.0 0.0	0.06 (1)				
H-G	0 / 0	-18.5 -18.5	0.09 (4)				
G-F	0 / 0	-18.5 -18.5	0.09 (4)				

**DESIGN CRITERIA**

**SPECIFIED LOADS:**

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

**SPACING = 24.0 IN. C/C**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) =  $L/360$  (0.29")  
CALCULATED VERT. DEFL.(LL) =  $L/999$  (0.00")  
ALLOWABLE DEFL.(TL) =  $L/360$  (0.29")  
CALCULATED VERT. DEFL.(TL) =  $L/999$  (0.01")

CSI: TC=0.22/1.00 (C-D:1), BC=0.09/1.00 (F-G:4), WB=0.08/1.00 (B-G:1), SSI=0.14/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10  
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

**NAIL VALUES**

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.60 (D) (INPUT = 0.90)  
JSI METAL = 0.18 (D) (INPUT = 1.00)

CITY OF RICHMOND HILL  
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09/02/2021

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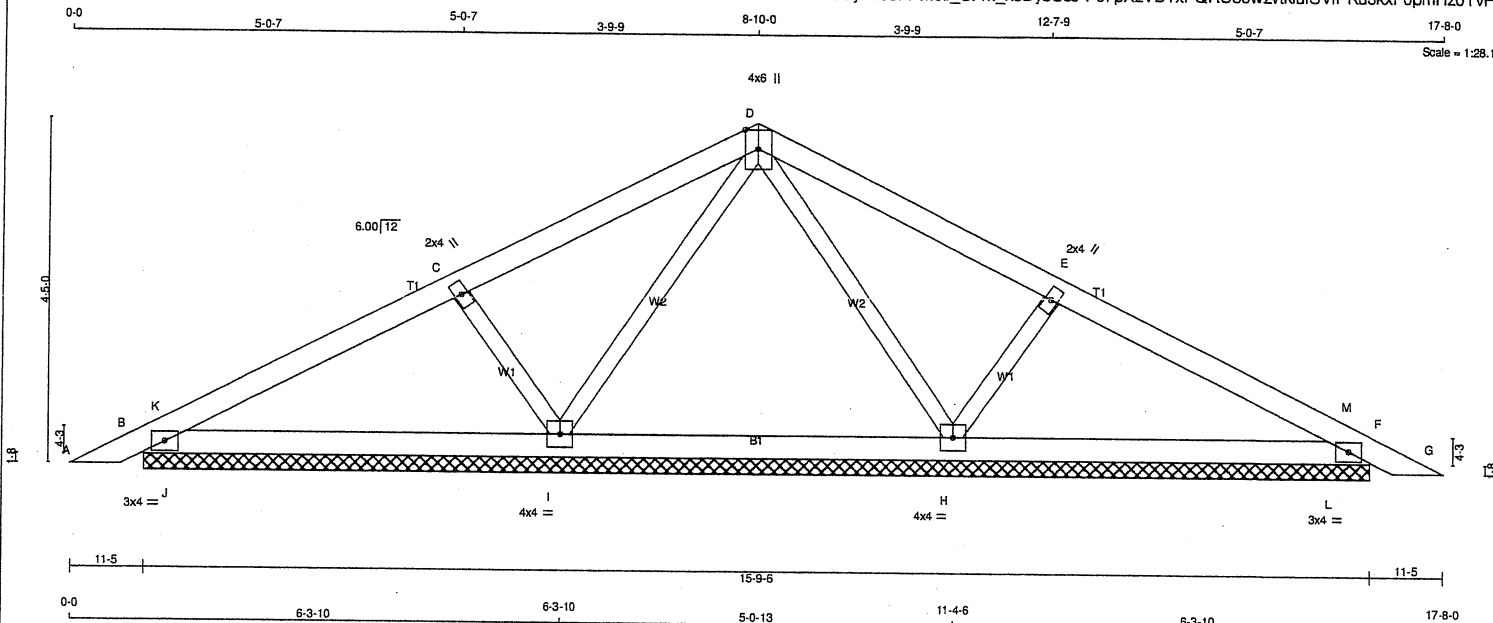
Per: jocelyn.aguilar



Structural component only  
DWG# T-2121235

JOB NAME <b>412868</b>	TRUSS NAME <b>PB06</b>	QUANTITY <b>18</b>	PLY <b>1</b>	JOB DESC. <b>ROYAL PINE HOMES</b>	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Thu Jul 1 10:22:28 2021 Page 1  
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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF		
D - G	2x4	DRY	No.2	SPF		
B - F	2x4	DRY	No.2	SPF		
ALL WEBS	2x3	DRY	No.2	SPF		
DRY: SEASONED LUMBER.						

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0			
C	TMW-w	MT20	2.0	4.0			
D	TTWW+p	MT20	4.0	6.0	Edge		
E	TMW-w	MT20	2.0	4.0			
F	TMB1-I	MT20	3.0	4.0			
H	BMWW1-I	MT20	4.0	4.0			
I	BMWW1-I	MT20	4.0	4.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

#### DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORIZ	DOWN	HORIZ		
B	340	0	340	0	15-9-6	15-9-6
F	340	0	340	0	15-9-6	15-9-6
H	610	0	610	0	15-9-6	15-9-6
I	610	0	610	0	15-9-6	15-9-6

#### UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	239	166 / 0	0 / 0	0 / 0	0 / 0	73 / 0	0 / 0
F	239	166 / 0	0 / 0	0 / 0	0 / 0	73 / 0	0 / 0
H	432	283 / 0	0 / 0	0 / 0	0 / 0	149 / 0	0 / 0
I	432	283 / 0	0 / 0	0 / 0	0 / 0	149 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, H, I

#### BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

#### LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		MAX. UNBRAC LENGTH	MEMB.	WEBS	
	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX. CSI (LC)			MAX. FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO						FR-TO		
A-B	0 / 17	-91.8	-91.8	0.05 (1)	10.00	D-H	-197 / 0	0.07 (1)
B-K	-199 / 0	-91.8	-91.8	0.06 (4)	6.25	H-E	-396 / 0	0.07 (1)
K-C	-143 / 0	-91.8	-91.8	0.23 (1)	6.25	I-D	-197 / 0	0.07 (1)
C-D	0 / 54	-91.8	-91.8	0.24 (1)	10.00	C-I	-396 / 0	0.07 (1)
D-E	0 / 54	-91.8	-91.8	0.24 (1)	10.00	J-K	-69 / 58	0.00 (1)
E-M	-143 / 0	-91.8	-91.8	0.23 (1)	6.25	L-M	-69 / 58	0.00 (1)
M-F	-199 / 0	-91.8	-91.8	0.06 (4)	6.25			
F-G	0 / 17	-91.8	-91.8	0.05 (1)	10.00			
B-J	0 / 149	-18.5	-18.5	0.08 (1)	10.00			
J-I	0 / 149	-18.5	-18.5	0.12 (4)	10.00			
I-H	0 / 40	-18.5	-18.5	0.11 (4)	10.00			
H-L	0 / 149	-18.5	-18.5	0.12 (4)	10.00			
L-F	0 / 149	-18.5	-18.5	0.08 (1)	10.00			

TOTAL WEIGHT = 18 X 51 = 925 lb

#### DESIGN CRITERIA

##### SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF  
DL = 6.0 PSF  
BOT CH. LL = 0.0 PSF  
DL = 7.4 PSF  
TOTAL LOAD = 39.0 PSF

##### SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

##### THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019  
- PART 9 OF OBC 2012 (2019 AMENDMENT)  
- CSA 086-14  
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.24/1.00 (C-D:1), BC=0.12/1.00 (I-J:4), WB=0.07/1.00 (D-H:1), SSI=0.15/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10  
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES  
PLATE GRIP(DRY) SHEAR SECTION  
(PSI) (PLI) (PLI)  
MAX MIN MAX MIN MAX MIN  
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (B) (INPUT = 0.90)

JSI METAL= 0.08 (E) (INPUT = 1.00)



Structural component only  
DWG# T-2121149

CITY OF RICHMOND HILL  
BUILDING DIVISION

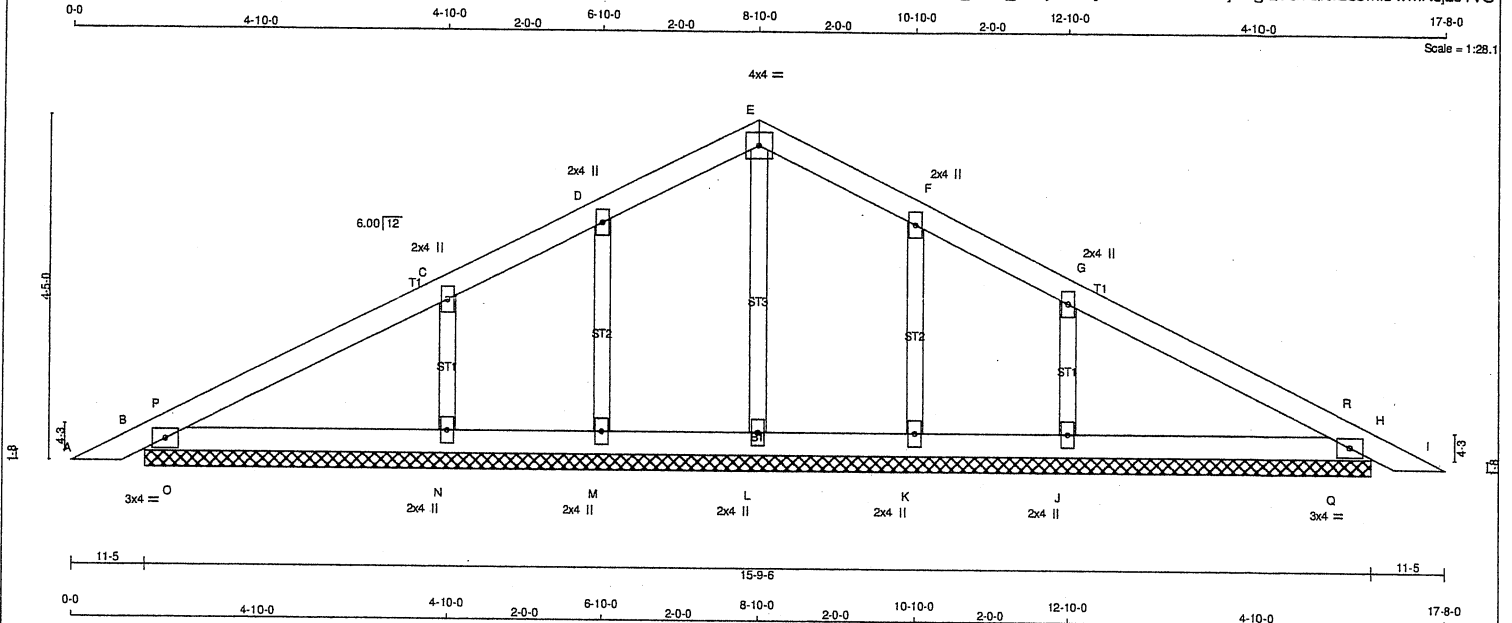
09/02/2021

RECEIVED

Per: jocelyn.aguilar

JOB NAME <b>412868</b>	TRUSS NAME <b>PB06G</b>	QUANTITY <b>4</b>	PLY <b>1</b>	JOB DESC. <b>ROYAL PINE HOMES</b>	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 1 10:22:29 2021 Page 1  
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LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
B - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				
GABLE STUDS SPACED AT 2-0-0 OC.				

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y
B TMB1-I	MT20	3.0	4.0	
C, D, F, G				
C TMW+w	MT20	2.0	4.0	
E TTW-p	MT20	4.0	4.0	
H TMB1-I	MT20	3.0	4.0	
J, K, L, M, N				
J BMW1+w	MT20	2.0	4.0	

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

**BRACING**

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

**LOADING**

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX (LC)		
FR-TO		FROM	TO		FR-TO				
A-B	0 / 17	-91.8	-91.8	0.05 (1)	10.00	L-E	-131 / 0	0.04 (1)	
B-P	-44 / 0	-91.8	-91.8	0.02 (4)	6.25	M-D	-160 / 0	0.03 (1)	
P-C	-44 / 0	-91.8	-91.8	0.13 (1)	6.25	N-C	-306 / 0	0.05 (1)	
C-D	-60 / 0	-91.8	-91.8	0.13 (1)	6.25	K-F	-160 / 0	0.03 (1)	
D-E	-45 / 0	-91.8	-91.8	0.05 (1)	6.25	J-G	-306 / 0	0.05 (1)	
E-F	-45 / 0	-91.8	-91.8	0.05 (1)	6.25	O-P	-171 / 5	0.00 (1)	
F-G	-60 / 0	-91.8	-91.8	0.13 (1)	6.25	Q-R	-171 / 5	0.00 (1)	
G-R	-44 / 0	-91.8	-91.8	0.13 (1)	6.25				
R-H	-44 / 0	-91.8	-91.8	0.02 (4)	6.25				
H-I	0 / 17	-91.8	-91.8	0.05 (1)	10.00				
B-O	0 / 49	-18.5	-18.5	0.10 (1)	10.00				
O-N	0 / 49	-18.5	-18.5	0.10 (1)	10.00				
N-M	0 / 39	-18.5	-18.5	0.07 (1)	10.00				
M-L	0 / 36	-18.5	-18.5	0.02 (4)	10.00				
L-K	0 / 36	-18.5	-18.5	0.02 (4)	10.00				
K-J	0 / 39	-18.5	-18.5	0.07 (1)	10.00				
J-Q	0 / 49	-18.5	-18.5	0.10 (1)	10.00				
Q-H	0 / 49	-18.5	-18.5	0.10 (1)	10.00				

#### DESIGN CRITERIA

**SPECIFIED LOADS:**

TOP CH. LL = 25.6 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

**SPACING = 24.0 IN. C/C**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13/1.00 (G-R:1), BC=0.10/1.00 (H-Q:1), WB=0.05/1.00 (G-J:1), SSI=0.14/1.00 (B-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

**NAIL VALUES**

PLATE GRIP(DRY) SHEAR SECTION

(PSI) (PL) (PL)

MAX MIN MAX MIN MAX MIN

MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (B) (INPUT = 0.90 )

JSI METAL= 0.13 (G) (INPUT = 1.00 )



Structural component only  
DWG# T-2121150

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BUILDING DIVISION

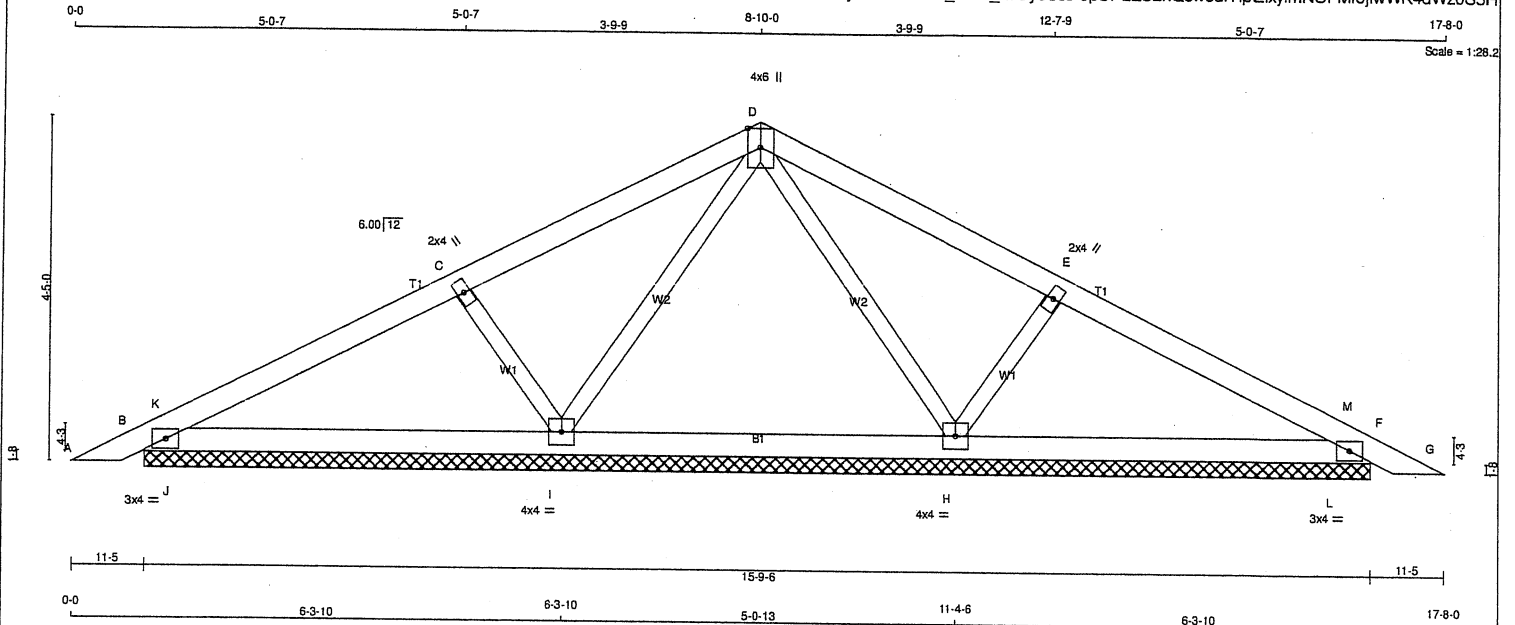
09/02/2021

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Per: jocelyn.aguilar

JOB NAME <b>412865</b>	TRUSS NAME <b>PB06Z</b>	QUANTITY <b>1</b>	PLY <b>3</b>	JOB DESC. <b>ROYAL PINE HOMES</b>	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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CHORDS	ROWS	SURFACE SPACING (IN)	LOAD (PLF)
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
B - F	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY:	SEASONED LUMBER.		

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - D	12	TOP
D - G	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
B - F	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

#### PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TMW+W	MT20	2.0	4.0		
D	TTWW+P	MT20	4.0	6.0	Edge	
E	TMW+W	MT20	2.0	4.0		
F	TMB1-I	MT20	3.0	4.0		
H	BMWW1-I	MT20	4.0	4.0		
I	BMWW1-I	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

#### DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
B	340	0	340	0	15-9-6	15-9-6
F	340	0	340	0	15-9-6	15-9-6
H	610	0	610	0	15-9-6	15-9-6
I	610	0	610	0	15-9-6	15-9-6

#### UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	239	166 / 0	0 / 0	0 / 0	0 / 0	73 / 0	0 / 0
F	239	166 / 0	0 / 0	0 / 0	0 / 0	73 / 0	0 / 0
H	432	283 / 0	0 / 0	0 / 0	0 / 0	149 / 0	0 / 0
I	432	283 / 0	0 / 0	0 / 0	0 / 0	149 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, H, I

#### BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

#### LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM TO			FR-TO			
A-B	0 / 17	-91.8	-91.8	0.02 (1)	10.00	D-H	-197 / 0	0.02 (1)
B-K	-199 / 0	-91.8	-91.8	0.02 (4)	6.25	H-E	-396 / 0	0.02 (1)
K-C	-144 / 0	-91.8	-91.8	0.08 (1)	6.25	I-D	-197 / 0	0.02 (1)
C-D	0 / 54	-91.8	-91.8	0.08 (1)	10.00	C-I	-396 / 0	0.02 (1)
D-E	0 / 54	-91.8	-91.8	0.08 (1)	10.00	J-K	-68 / 57	0.00 (1)
E-M	-144 / 0	-91.8	-91.8	0.08 (1)	6.25	L-M	-68 / 57	0.00 (1)
M-F	-199 / 0	-91.8	-91.8	0.02 (4)	6.25			
F-G	0 / 17	-91.8	-91.8	0.02 (1)	10.00			
B-J	0 / 149	-18.5	-18.5	0.03 (1)	10.00			
J-I	0 / 149	-18.5	-18.5	0.04 (4)	10.00			
I-H	0 / 40	-18.5	-18.5	0.04 (4)	10.00			
H-L	0 / 149	-18.5	-18.5	0.04 (4)	10.00			
L-F	0 / 149	-18.5	-18.5	0.03 (1)	10.00			

TOTAL WEIGHT = 3 X 51 = 154 lb

#### DESIGN CRITERIA

SPECIFIED LOADS:  
TOP CH. LL = 25.6 PSF  
DL = 6.0 PSF  
BOT CH. LL = 0.0 PSF  
DL = 7.4 PSF  
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:  
- PART 9 OF BCBC 2018, ABC 2019  
- PART 9 OF OBC 2012 (2019 AMENDMENT)  
- CSA 086-14  
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.08/1.00 (C-D:1), BC=0.04/1.00 (I-J:4), WB=0.02/1.00 (D-I:1), SSI=0.05/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES  
PLATE GRIP(DRY) SHEAR SECTION (FSI) (PLI) (PLI)  
MAX MIN MAX MIN MAX MIN  
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

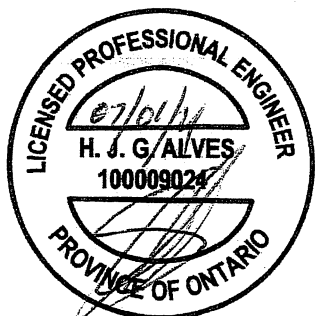
JSI GRIP= 0.08 (F) (INPUT = 0.90)  
JSI METAL= 0.03 (C) (INPUT = 1.00)

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BUILDING DIVISION

09/02/2021

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Per: jocelyn.aguilar

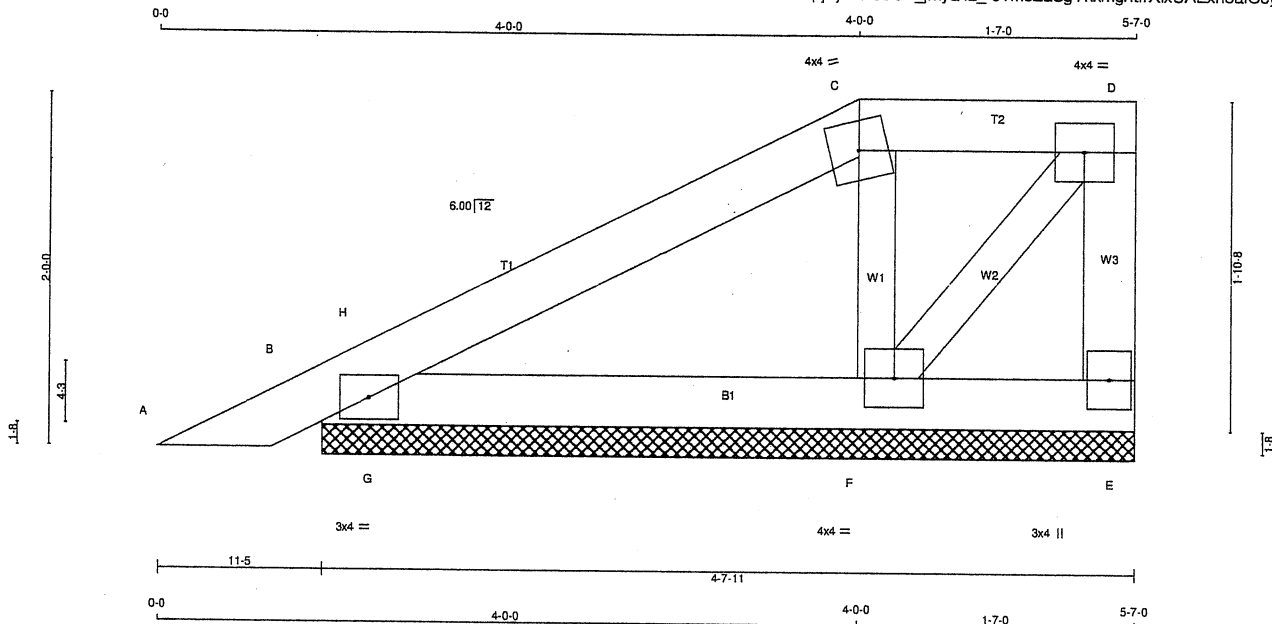


Structural component only  
DWG# T-2121199



JOB NAME <b>412865</b>	TRUSS NAME <b>PB101</b>	QUANTITY <b>4</b>	PLY <b>1</b>	JOB DESC. <b>ROYAL PINE HOMES</b>	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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TOTAL WEIGHT = 4 X 16 = 64 lb [M]

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
E - D	2x4	DRY	No.2
B - E	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-t	MT20	3.0	4.0	
C	TTW-m	MT20	4.0	4.0	
D	TMW-t	MT20	4.0	4.0	
E	BMV1+p	MT20	3.0	4.0	
F	BMW1-t	MT20	4.0	4.0	

#### DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	JT	GROSS REACTION	BRG	IN-SX	BRG	IN-SX
E	76	0	76	0	0	4-7-11	4-7-11
B	252	0	252	0	0	4-7-11	4-7-11
F	264	0	264	0	0	4-7-11	4-7-11

#### UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	1ST LCASE	MAX./MIN. COMPONENT REACTIONS
E	53	39 / 0	0 / 0	0 / 0
B	177	126 / 0	0 / 0	0 / 0
F	188	118 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B, F

#### BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

#### LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 17	-91.8 -91.8	0.05 (1)	10.00	F-C	-176 / 0	0.03 (1)
B-H	-18 / 0	-91.8 -91.8	0.02 (1)	6.25	F-D	0 / 24	0.01 (1)
H-C	-33 / 0	-91.8 -91.8	0.10 (1)	6.25	G-H	-170 / 0	0.00 (1)
C-D	-16 / 0	-91.8 -91.8	0.04 (1)	6.25			
E-D	-90 / 0	0.0 0.0	0.01 (1)	7.81			
B-G	0 / 28	-18.5 -18.5	0.09 (1)	10.00			
G-F	0 / 28	-18.5 -18.5	0.09 (1)	10.00			
F-E	0 / 0	-18.5 -18.5	0.06 (1)	10.00			

#### DESIGN CRITERIA

##### SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
DL = 6.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.4	PSF	
TOTAL LOAD = 39.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.10/1.00 (C-H:1), BC=0.09/1.00 (F-G:1), WB=0.03/1.00 (C-F:1), SSI=0.13/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10  
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

#### NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PL)
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

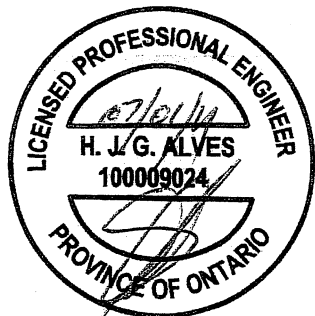
JSI-GRIP= 0.20 (B) (INPUT = 0.90)  
JSI METAL= 0.06 (C) (INPUT = 1.00)

CITY OF RICHMOND HILL  
BUILDING DIVISION

09/02/2021

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Per: jocelyn.aguilar

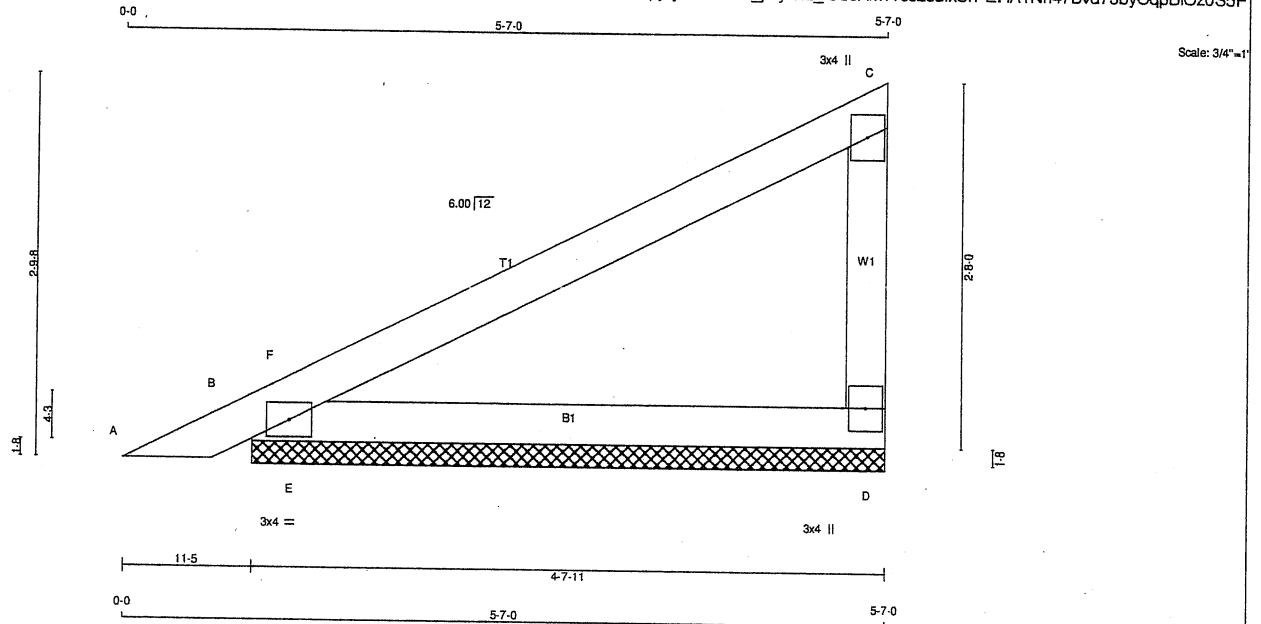


Structural component only  
DWG# T-2121200

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
412865	PB102	8	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
D - C	2x4	DRY	No.2
B - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y
B	TMB1-I	MT20	3.0	4.0	
C	TMV+p	MT20	3.0	4.0	
D	BMV1+p	MT20	3.0	4.0	

#### DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
D	256	0	256	0	4-7-11	4-7-11
B	336	0	336	0	4-7-11	4-7-11

#### UNFACTORED REACTIONS

1ST CASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
D	181	119 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0
B	236	164 / 0	0 / 0	0 / 0	0 / 0	73 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, B

#### BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

#### LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 17	-91.8 -91.8	0.05 (1)	E-F	-310 / 7	0.00 (1)	
B-F	-21 / 45	-91.8 -91.8	0.06 (1)				
F-C	-4 / 2	-91.8 -91.8	0.26 (1)				
D-C	-186 / 0	0.0 0.0	0.02 (1)				
B-E	0 / 0	-18.5 -18.5	0.18 (1)				
E-D	0 / 0	-18.5 -18.5	0.18 (1)				

TOTAL WEIGHT = 8 X 15 = 118 lb

#### DESIGN CRITERIA

##### SPECIFIED LOADS:

TOP CH.	LL	= 25.6	PSF
	DL	= 6.0	PSF
BOT CH.	LL	= 0.0	PSF
	DL	= 7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.26/1.00 (C-F:1), BC=0.18/1.00 (D-E:1), WB=0.00/1.00 (E-F:1), SS=0.25/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

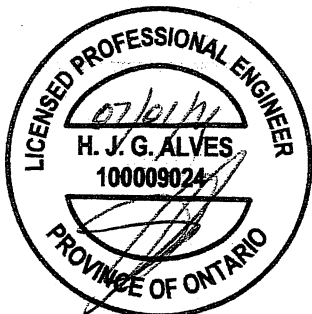
#### NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	650	371	1747
			788
			1987
			1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.26 (B) (INPUT = 0.90)  
JSI METAL= 0.05 (B) (INPUT = 1.00)



Structural component only  
DWG# T-2121201

CITY OF RICHMOND HILL  
BUILDING DIVISION

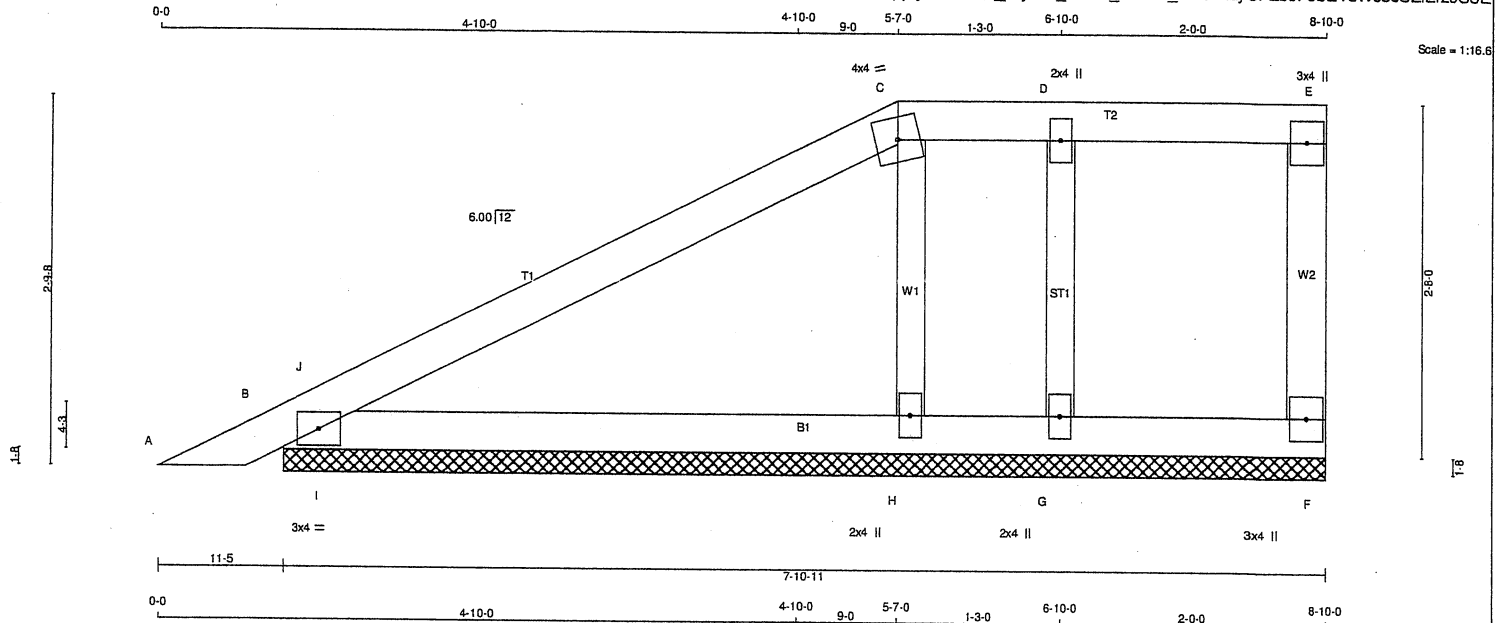
09/02/2021

RECEIVED

Per: jocelyn.aguilar

JOB NAME <b>412865</b>	TRUSS NAME <b>PB103G</b>	QUANTITY <b>4</b>	PLY <b>1</b>	JOB DESC. <b>ROYAL PINE HOMES</b>	DRWG NO.
Tamarack Roof Truss, Burlington					

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LUMBER			
N. L. G. A. RULES	SIZE	DRY	LUMBER
A - C	2x4	DRY	No.2
O - m	2x4	DRY	No.2
T - m	2x4	DRY	No.2
B - T	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			
GABLE STUDS SPACED AT 2-0-0 OC.			

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMB1-I	MT20	3.0	4.0
C	TTW-m	MT20	4.0	4.0
D	TMW-w	MT20	2.0	4.0
E	TMV-p	MT20	3.0	4.0
F	BMV1-p	MT20	3.0	4.0
G	BMW1-w	MT20	2.0	4.0
H	BMW1-w	MT20	2.0	4.0

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**  
**BEARINGS**  
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.  
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.  
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)  
**BRACING**  
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.  
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. (LC)	UNBRAC LENGTH	MEMB.
FR-TO		FROM TO			FR-TO				
A-B	0 / 17	-91.8 -91.8	0.05 (1)	10.00	G-D	-193 / 0	0.03 (1)		
B-J	-9 / 65	-91.8 -91.8	0.09 (1)	10.00	H-C	-206 / 0	0.03 (1)		
J-C	-17 / 0	-91.8 -91.8	0.24 (1)	6.25	I-J	-374 / 0	0.00 (1)		
C-D	0 / 0	-91.8 -91.8	0.05 (1)	10.00					
D-E	0 / 0	-91.8 -91.8	0.05 (1)	10.00					
F-E	-75 / 0	0.0 0.0	0.01 (1)	7.81					
B-I	0 / 9	-18.5 -18.5	0.20 (1)	10.00					
I-H	0 / 9	-18.5 -18.5	0.20 (1)	10.00					
H-G	0 / 0	-18.5 -18.5	0.14 (1)	10.00					
G-F	0 / 0	-18.5 -18.5	0.02 (4)	10.00					

#### DESIGN CRITERIA

**SPECIFIED LOADS:**  
TOP CH. LL = 25.6 PSF  
DL = 6.0 PSF  
BOT CH. LL = 0.0 PSF  
DL = 7.4 PSF  
TOTAL LOAD = 39.0 PSF

**SPACING = 24.0 IN. C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:  
- PART 9 OF BCBC 2018, ABC 2019  
- PART 9 OF OBC 2012 (2019 AMENDMENT)  
- CSA 086-14  
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.24/1.00 (C-J:1), BC=0.20/1.00 (B-I:1), WB=0.03/1.00 (H:1), SS=0.28/1.00 (B-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

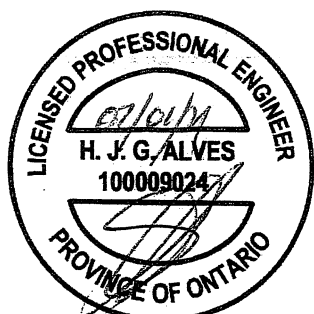
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

**NAIL VALUES**  
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)  
MAX MIN MAX MIN MAX MIN  
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.26 (B) (INPUT = 0.90)  
JSI METAL= 0.11 (C) (INPUT = 1.00)



Structural component only  
DWG# T-2121202

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BUILDING DIVISION

09/02/2021

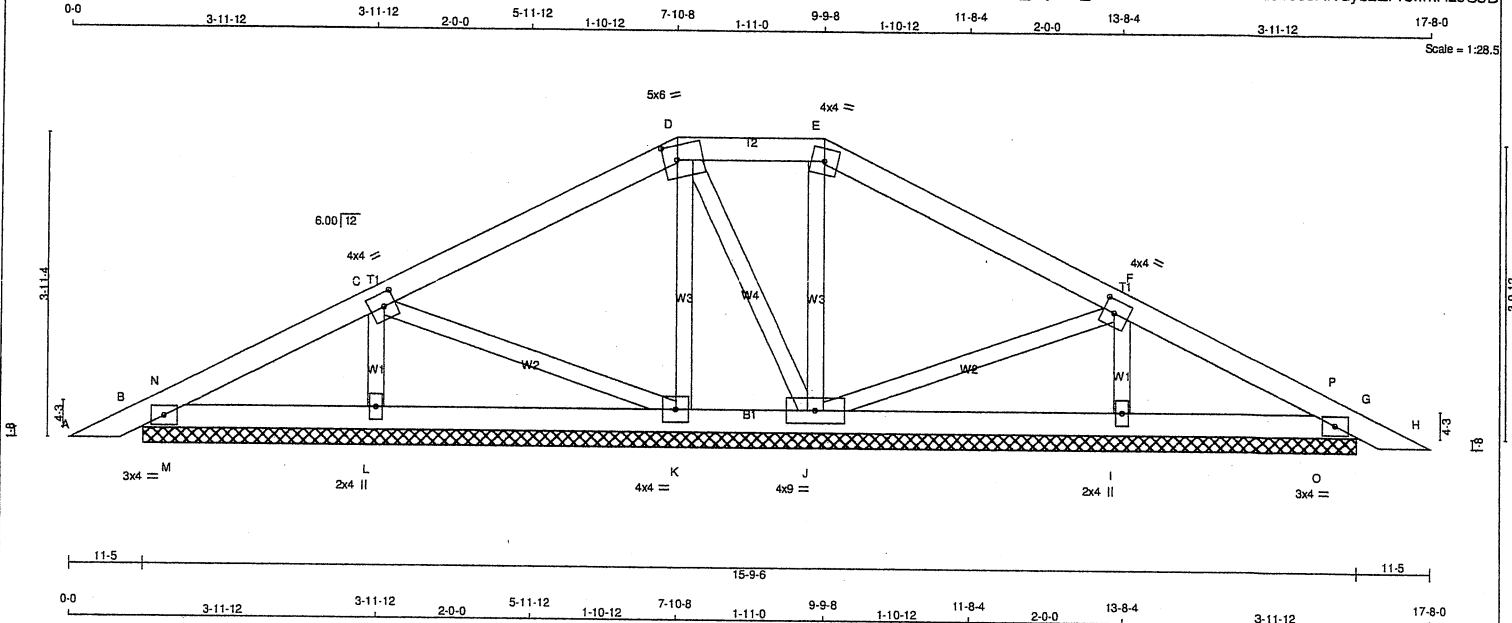
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Per: jocelyn.aguiar

JOB NAME <b>412865</b>	TRUSS NAME <b>PB104</b>	QUANTITY <b>2</b>	PLY <b>1</b>	JOB DESC. <b>ROYAL PINE HOMES</b>	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	SPF	SPF
A - D	2x4	DRY	No.2	SPF	SPF
D - E	2x4	DRY	No.2	SPF	SPF
E - H	2x4	DRY	No.2	SPF	SPF
B - G	2x4	DRY	No.2	SPF	SPF
ALL WEBS 2x3 DRY				No.2	SPF
DRY: SEASONED LUMBER.					

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	
C	TMWW-t	MT20	4.0	4.0	2.00 1.75
D	TTWW-m	MT20	5.0	6.0	2.25 2.00
E	TTWW-m	MT20	4.0	4.0	
F	TMWW-t	MT20	4.0	4.0	2.00 1.75
G	TMB1-I	MT20	3.0	4.0	
I	BMW1-w	MT20	2.0	4.0	
J	BMWVW1-t	MT20	4.0	9.0	
K	BMWV1-t	MT20	4.0	4.0	
L	BMW1-w	MT20	2.0	4.0	

#### DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	BRG	BRG
B	222	0	222	0	0	0	15-9-6	15-9-6	
G	213	0	213	0	0	0	15-9-6	15-9-6	
K	205	0	205	0	0	0	15-9-6	15-9-6	
J	385	0	385	0	0	0	15-9-6	15-9-6	
L	447	0	447	0	0	0	15-9-6	15-9-6	
I	428	0	428	0	0	0	15-9-6	15-9-6	

#### UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	155	112 / 0	0 / 0	0 / 0	0 / 0	0 / 0	43 / 0	0 / 0
G	149	108 / 0	0 / 0	0 / 0	0 / 0	0 / 0	42 / 0	0 / 0
K	147	86 / 0	0 / 0	0 / 0	0 / 0	0 / 0	61 / 0	0 / 0
J	271	187 / 0	0 / 0	0 / 0	0 / 0	0 / 0	84 / 0	0 / 0
L	316	208 / 0	0 / 0	0 / 0	0 / 0	0 / 0	108 / 0	0 / 0
I	303	197 / 0	0 / 0	0 / 0	0 / 0	0 / 0	106 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, G, K, J, L, I

#### BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

#### LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 17	-91.8 -91.8	0.05 (1)	10.00	D-J	-86 / 0	0.02 (1)
B-N	-80 / 0	-91.8 -91.8	0.01 (1)	6.25	C-K	-37 / 0	0.01 (1)
N-C	-39 / 0	-91.8 -91.8	0.19 (1)	6.25	J-F	-59 / 0	0.02 (1)
C-D	-42 / 0	-91.8 -91.8	0.19 (1)	6.25	K-D	-143 / 0	0.03 (1)
D-E	0 / 21	-91.8 -91.8	0.06 (1)	10.00	J-E	-238 / 0	0.05 (1)
E-F	0 / 0	-91.8 -91.8	0.19 (1)	10.00	L-C	-361 / 0	0.05 (1)
F-P	-21 / 0	-91.8 -91.8	0.19 (1)	6.25	I-F	-343 / 0	0.05 (1)
P-G	-63 / 0	-91.8 -91.8	0.02 (1)	6.25	M-N	-50 / 4	0.00 (1)
G-H	0 / 17	-91.8 -91.8	0.05 (1)	10.00	O-P	-50 / 4	0.00 (1)

B-M	0 / 57	-18.5 -18.5	0.04 (1)	10.00
M-L	0 / 57	-18.5 -18.5	0.06 (4)	10.00
L-K	0 / 57	-18.5 -18.5	0.06 (4)	10.00
K-J	0 / 19	-18.5 -18.5	0.04 (4)	10.00
J-I	0 / 42	-18.5 -18.5	0.06 (4)	10.00
I-O	0 / 42	-18.5 -18.5	0.06 (4)	10.00
O-G	0 / 42	-18.5 -18.5	0.04 (1)	10.00

#### DESIGN CRITERIA

##### SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF  
DL = 6.0 PSF  
BOT CH. LL = 0.0 PSF  
DL = 7.4 PSF  
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.19/1.00 (F-P:1), BC=0.06/1.00 (K-L:4), WB=0.05/1.00 (E-J:1), SSI=0.15/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10  
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

##### NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION  
(PSI) (PLI) (PLI)  
MAX MIN MAX MIN MAX MIN  
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.30 (C) (INPUT = 0.90)  
JSI METAL= 0.09 (E) (INPUT = 1.00)

CITY OF RICHMOND HILL  
BUILDING DIVISION

09/02/2021

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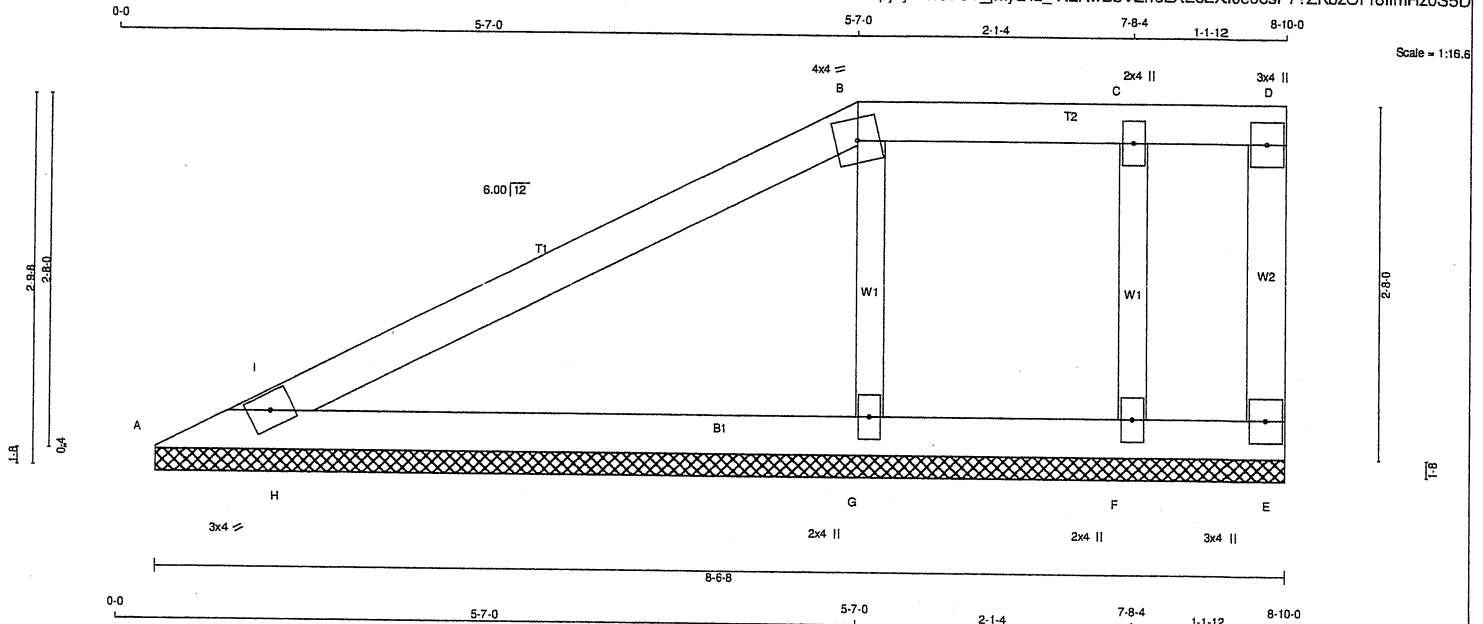
Per: jocelyn.aguilar



Structural component only  
DWG# T-2121203

JOB NAME <b>412865</b>	TRUSS NAME <b>PB105G</b>	QUANTITY <b>2</b>	PLY <b>1</b>	JOB DESC. <b>ROYAL PINE HOMES</b>	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2
B - D	2x4	DRY	No.2
E - D	2x4	DRY	No.2
A - E	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TBM1-h	MT20	3.0	4.0	
B	TTW-m	MT20	4.0	4.0	
C	TMW-w	MT20	2.0	4.0	
D	TMV-p	MT20	3.0	4.0	
E	BMV1-p	MT20	3.0	4.0	
F	BMW1-w	MT20	2.0	4.0	
G	BMW1-w	MT20	2.0	4.0	

#### DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
JT	GROSS REACTION	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	BRG	IN-SX
A	272	0	272	0	0	0	8-6-8	8-6-8	8-6-8
E	65	0	65	0	0	0	8-6-8	8-6-8	8-6-8
G	502	0	502	0	0	0	8-6-8	8-6-8	8-6-8
F	103	0	103	0	0	0	8-6-8	8-6-8	8-6-8

#### UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	
A	192	128 / 0	0 / 0	0 / 0	0 / 0	64 / 0	0 / 0	
E	46	27 / 0	0 / 0	0 / 0	0 / 0	19 / 0	0 / 0	
G	357	224 / 0	0 / 0	0 / 0	0 / 0	132 / 0	0 / 0	
F	71	57 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, E, G, F

#### BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

#### LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	
FR-TO		FROM TO		FR-TO			
A-I	-14 / 0	-91.8 -91.8	0.05 (4)	G-B	-270 / 0	0.04 (1)	
I-B	-13 / 0	-91.8 -91.8	0.29 (1)	F-C	-199 / 0	0.03 (1)	
B-C	0 / 0	-91.8 -91.8	0.05 (1)	H-I	-238 / 0	0.00 (1)	
C-D	0 / 0	-91.8 -91.8	0.05 (1)				
E-D	-20 / 0	0.0 0.0	0.00 (1)				
A-H	-17 / 0	-18.5 -18.5	0.29 (1)				
H-G	0 / 12	-18.5 -18.5	0.29 (1)				
G-F	0 / 0	-18.5 -18.5	0.20 (1)				
F-E	0 / 0	-18.5 -18.5	0.05 (1)				

#### DESIGN CRITERIA

SPECIFIED LOADS:  
TOP CH. LL = 25.6 PSF  
DL = 6.0 PSF  
BOT CH. LL = 0.0 PSF  
DL = 7.4 PSF  
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:  
- PART 9 OF BCBC 2018, ABC 2019  
- PART 9 OF OBC 2012 (2019 AMENDMENT)  
- CSA 086-14  
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.29/1.00 (B-I:1), BC=0.29/1.00 (G-H:1), WB=0.04/1.00 (B-G:1), SS=0.17/1.00 (A-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10  
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES  
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)  
MAX MIN MAX MIN MAX MIN  
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

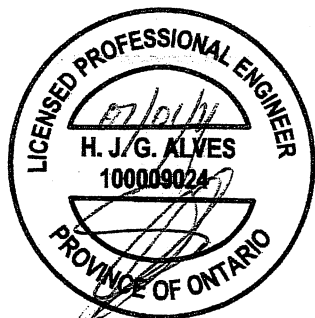
JSI GRIP= 0.22 (B) (INPUT= 0.90)  
JSI METAL= 0.12 (B) (INPUT= 1.00)

CITY OF RICHMOND HILL  
BUILDING DIVISION

09/02/2021

RECEIVED

Per: jocelyn.aguilar

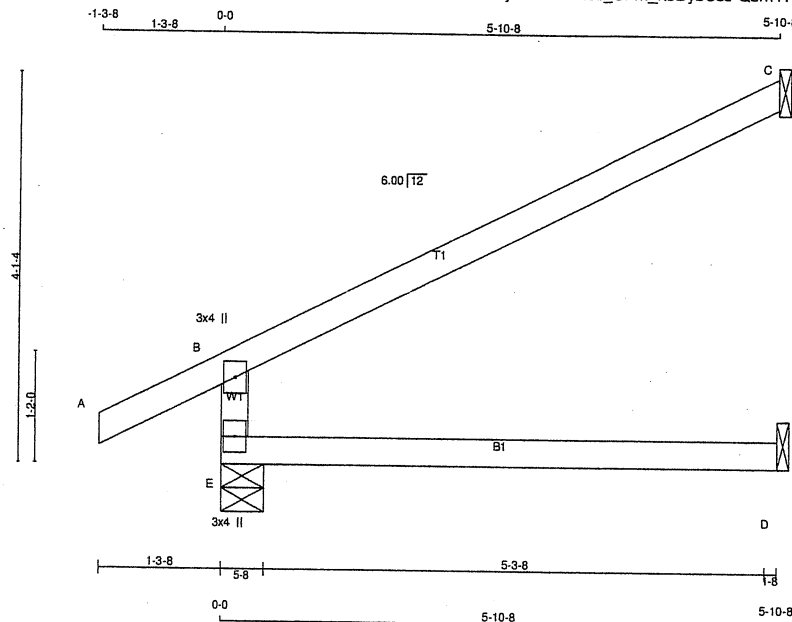


Structural component only  
DWG# T-2121204



JOB NAME <b>412868</b>	TRUSS NAME <b>J01</b>	QUANTITY <b>20</b>	PLY <b>1</b>	JOB DESC. <b>ROYAL PINE HOMES</b>	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1:23.1

TOTAL WEIGHT = 20 X 17 = 336 lb

<b>LUMBER</b>			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4 DRY	No.2	SPF
A - C	2x4 DRY	No.2	SPF
E - D	2x4 DRY	No.2	SPF

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
E	525	0	525	0	0	5-8	5-8	
C	202	0	202	0	0	1-8	1-8	
D	45	0	50	0	0	1-8	1-8	

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

**UNFACTORED REACTIONS**

JT	1ST LOASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	369	257 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0
C	139	113 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

**BRACING**

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

**LOADING**

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO			FR-TO		
E-B	-461 / 0	0.0	0.0	0.13 (4)	7.81			
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00			
B-C	-30 / 0	-91.8	-91.8	0.54 (1)	6.25			
E-D	0 / 0	-18.5	-18.5	0.13 (4)	10.00			

**DESIGN CRITERIA**

**SPECIFIED LOADS:**

TOP CH.	LL = 25.6 PSF
DL = 6.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.4 PSF	
TOTAL LOAD = 39.0 PSF	

**SPACING = 24.0 IN/C**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

**THIS DESIGN COMPLIES WITH:**

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

**DESIGN ASSUMPTIONS**

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")  
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")  
ALLOWABLE DEFL.(TL)= L/360 (0.20")  
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.54/1.00 (B-C:1), BC=0.13/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10  
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

**NAIL VALUES**

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	650	371	1747
		788	1987
			1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (E) (INPUT = 0.90)  
JSI METAL= 0.13 (B) (INPUT = 1.00)

CITY OF RICHMOND HILL  
BUILDING DIVISION

09/02/2021

RECEIVED

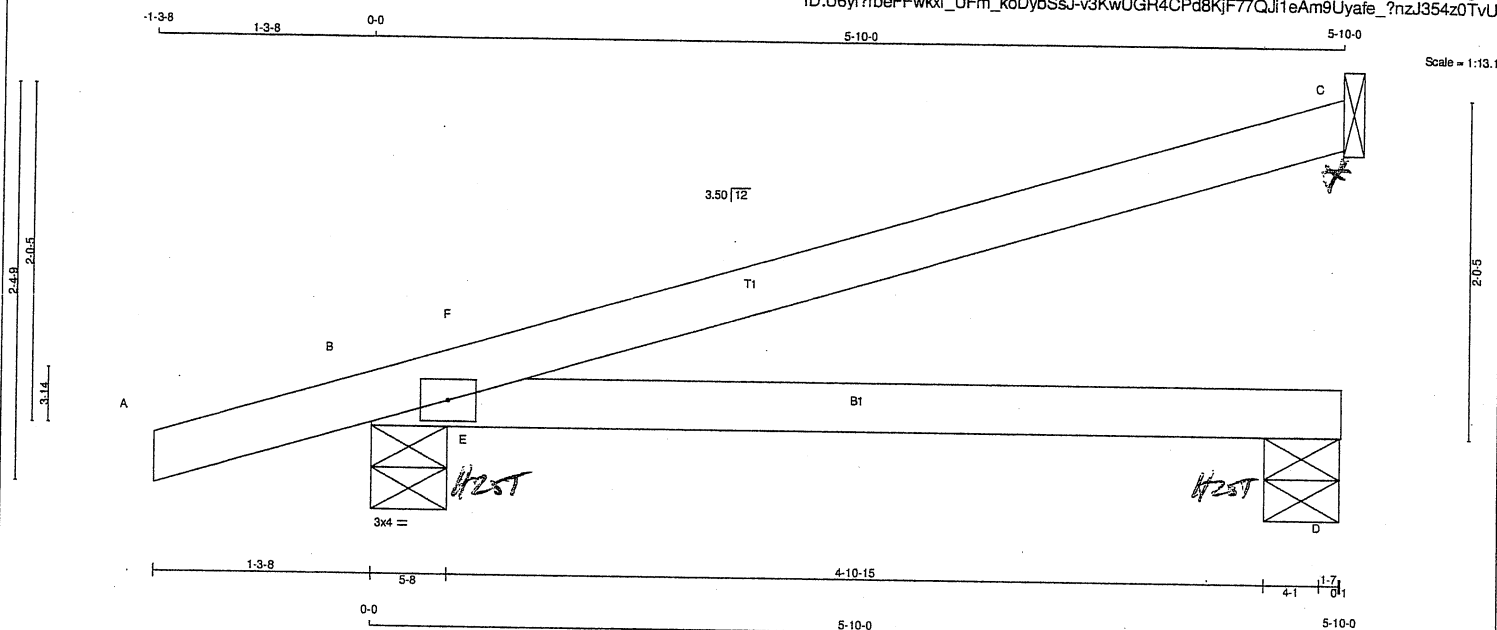
Per: jocelyn.aguilar



Structural component only  
DWG# T-2121142

JOB NAME <b>412868</b>	TRUSS NAME <b>J02</b>	QUANTITY <b>20</b>	PLY <b>1</b>	JOB DESC. <b>ROYAL PINE HOMES</b>	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			

Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Thu Jul 1 10:22:23 2021 Page 1  
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**LUMBER**  
N. L. G. A. RULES  
CHORDS SIZE  
A - C 2x4 DRY No.2  
B - D 2x4 DRY No.2  
DRY: SEASONED LUMBER.

**PLATES** (table is in inches)  
JT TYPE PLATES W LEN Y X  
B TMB1-I MT20 3.0 4.0

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
C	233	0	233	121	-113	1-8
B	444	0	444	0	-308	5-8
D	88	0	88	0	-107	5-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT  
PROVIDE ANCHORAGE AT BEARING JOINT B FOR 308 LBS FACTORED UPLIFT  
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT

PROVIDE FOR 121 LBS FACTORED HORIZONTAL REACTION AT JOINT C

**UNFACTORED REACTIONS**

JT	1ST CASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	WIND			
C	181	125 / 0	0 / 0	0 / 0	0 / -104	36 / 0	0 / 0
B	312	217 / 0	0 / 0	0 / 0	0 / -281	94 / 0	0 / 0
D	66	24 / 0	0 / 0	0 / 0	0 / -103	42 / 0	0 / 0

**HORIZONTAL REACTIONS**

C	0 / 0	0 / 0	0 / 0	86 / 0	0 / 0	0 / 0
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BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

**BRACING**

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

**LOADING**

TOTAL LOAD CASES: (12)

MEMB.	CHORDS		FACTORED		MAX. UNBRACED LENGTH	WEBS		MAX. UNBRACED LENGTH
	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1	LC2		MAX. FACTORED FORCE (LBS)	LC1	
FR-TO		FROM	TO					
A-B	0 / 16	-91.8	-91.8	0.11 (1)	10.00	E-F	-308 / 100	0.00 (1)
B-F	-17 / 44	-91.8	-91.8	0.08 (12)	6.25			
F-C	0 / 77	-91.8	-91.8	0.40 (1)	10.00			
B-E	0 / 0	-18.5	-18.5	0.29 (1)	10.00			
E-D	0 / 0	-18.5	-18.5	0.29 (1)	10.00			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.2) PSF AT (15-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, Cp, G, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 7.4 PSF RESPECTIVELY.

TOTAL WEIGHT = 20 X 15 = 304 lb

**DESIGN CRITERIA**

**SPECIFIED LOADS:**

TOP CH.	LL = 25.6 PSF
DL = 6.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.4 PSF	
TOTAL LOAD = 39.0 PSF	

**SPACING = 24.0 IN./C**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

**THIS DESIGN COMPLIES WITH:**

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")  
CALCULATED VERT. DEFL.(LL) = L/684 (0.10")  
ALLOWABLE DEFL.(TL) = L/360 (0.19")  
CALCULATED VERT. DEFL.(TL) = L/496 (0.14")

CSI: TC=0.40/1.00 (C-F:1), BC=0.29/1.00 (D-E:1), WB=0.00/1.00 (E-F:1), SS=0.24/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10  
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

**NAIL VALUES**

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	788 1987

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.28 (B) (INPUT = 0.90)  
JSI METAL = 0.08 (B) (INPUT = 1.00)



Structural component only  
DWG# T-2121143

09/02/2021

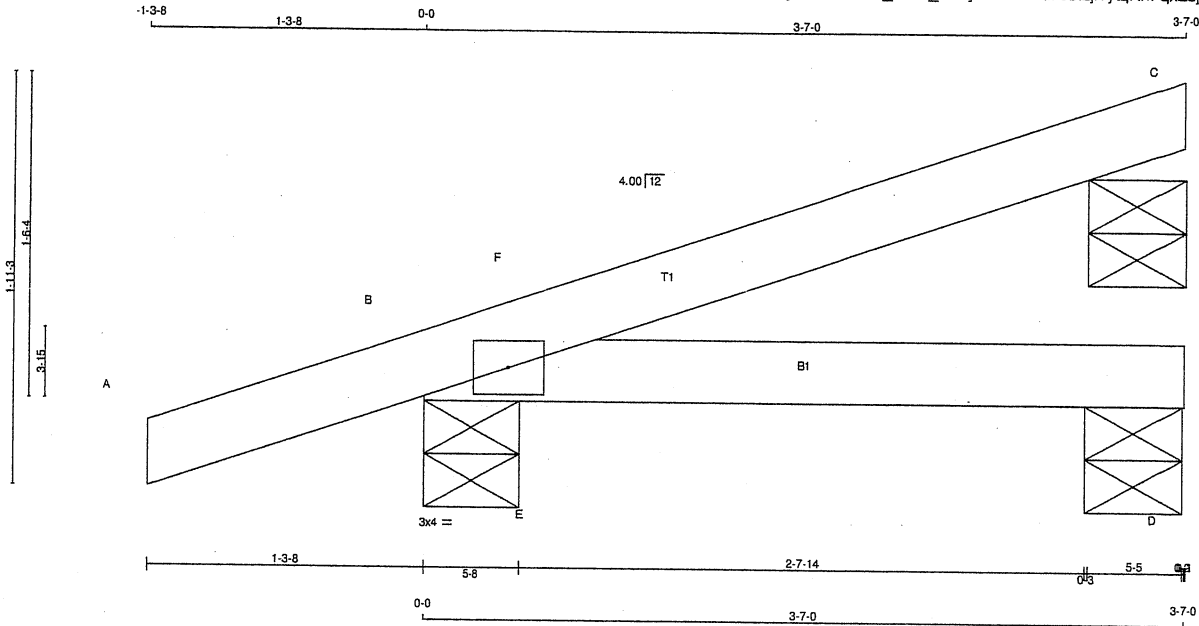
RECEIVED

Per: jocelyn.aguilar

JOB NAME <b>412868</b>	TRUSS NAME <b>J03</b>	QUANTITY <b>15</b>	PLY <b>1</b>	JOB DESC. <b>ROYAL PINE HOMES</b>	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1:10.3



TOTAL WEIGHT = 15 X 10 = 151 lb [M]

**LUMBER**  
N. L. G. A. RULES  
CHORDS SIZE DRY LUMBER No.2 DESCR. SPF  
A - C 2x4 DRY No.2 SPF  
B - D 2x4 DRY No.2 SPF  
DRY: SEASONED LUMBER.

**PLATES (table is in inches)**  
JT TYPE PLATES W LEN Y X  
B TMB1-1 MT20 3.0 4.0

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
C	141	0	141	0	0
B	320	0	320	0	0
D	57	0	57	0	0

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): C

**UNFACTORED REACTIONS**

1ST LCASE	MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
C	98	76 / 0	0 / 0	0 / 0	0 / 0	22 / 0
B	224	160 / 0	0 / 0	0 / 0	0 / 0	64 / 0
D	42	16 / 0	0 / 0	0 / 0	0 / 0	26 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B, D

**BRACING**

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

**LOADING**

TOTAL LOAD CASES: (5)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED UNBRACED LENGTH (FT)
FR-TO			
A-B	0 / 18	-91.8 -91.8 0.13 (5)	10.00
B-F	-10 / 0	-91.8 -91.8 0.04 (4)	6.25
F-C	0 / 2	-91.8 -91.8 0.15 (1)	10.00
B-E	0 / 0	-18.5 -18.5 0.12 (1)	10.00
E-D	0 / 0	-18.5 -18.5 0.12 (1)	10.00

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

**DESIGN CRITERIA**

**SPECIFIED LOADS:**

TOP CH. LL = 25.6 PSF  
DL = 8.0 PSF  
BOT CH. LL = 0.0 PSF  
DL = 7.4 PSF  
TOTAL LOAD = 39.0 PSF

**SPACING = 24.0 IN. C/C**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

**THIS DESIGN COMPLIES WITH:**

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")  
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")  
ALLOWABLE DEFL.(TL) = L/360 (0.19")  
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.15/1.00 (C-F:1), BC=0.12/1.00 (B-E:1),  
WB=0.00/1.00 (E-F:1), SSI=0.11/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10  
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

**NAIL VALUES**

PLATE GRIP(DRY) SHEAR SECTION  
(PSI) (PLI) (PLI)  
MAX MIN MAX MIN MAX MIN  
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (B) (INPUT = 0.90)  
JSI METAL= 0.05 (B) (INPUT = 1.00)



Structural component only  
DWG# T-2121144

CITY OF RICHMOND HILL  
BUILDING DIVISION

09/02/2021

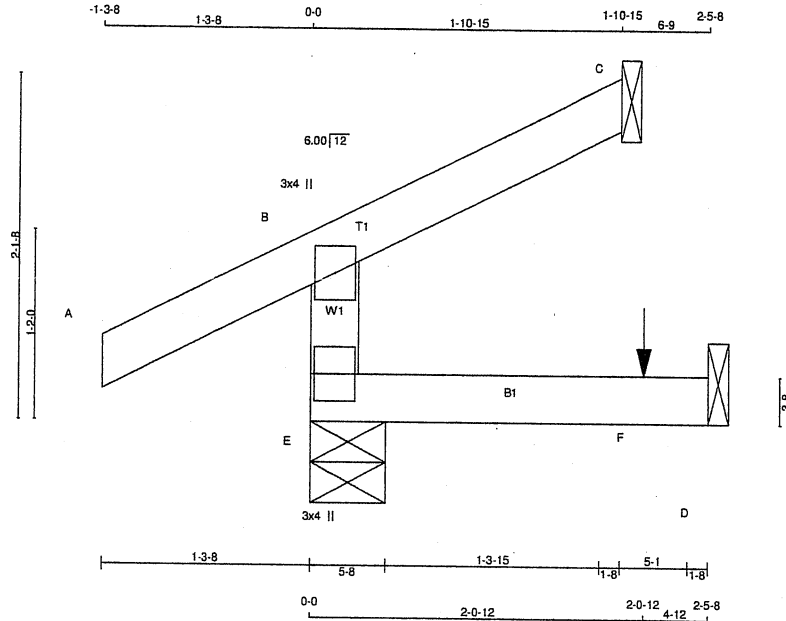
RECEIVED

Per: jocelyn.aguilar

JOB NAME <b>412865</b>	TRUSS NAME <b>J11</b>	QUANTITY <b>5</b>	PLY <b>1</b>	JOB DESC. <b>ROYAL PINE HOMES</b>	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1:13.5

TOTAL WEIGHT = 5 X 8 = 39 lb

LUMBER			
N. L. G. A. RULES	SIZE	DRY	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
E	260	0	260	0	5-8	5-8
C	66	0	66	0	1-8	1-8
D	23	0	26	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

**UNFACTORED REACTIONS**

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
COMBINED		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	182	130 / 0	0 / 0	0 / 0	0 / 0	52 / 0	0 / 0
E	46	37 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0
C	18	0 / 0	0 / 0	0 / 0	0 / 0	18 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

**BRACING**

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**

TOTAL LOAD CASES: (5)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRAC LENGTH	FR-TO	WEBS	
						MAX. FACTORED FORCE (LBS)	CSI (LC)
E-B	-234 / 0	0.0	0.0	0.02 (4)	7.81		
A-B	0 / 28	-91.8	-91.8	0.13 (5)	10.00		
B-C	-10 / 0	-91.8	-91.8	0.06 (1)	10.00		
E-F	0 / 0	-18.5	-18.5	0.03 (4)	10.00		
F-D	0 / 0	-18.5	-18.5	0.03 (4)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	2-0-12	-3	-3	---	BACK	VERT	TOTAL	---	C1

**CONNECTION REQUIREMENTS**

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

**DESIGN CRITERIA**

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
DL = 6.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.4 PSF	
TOTAL LOAD = 39.0 PSF	

**SPACING = 24.0 IN./C**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")  
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")  
ALLOWABLE DEFL.(TL)= L/360 (0.19")  
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.13/1.00 (A-B-5), BC=0.03/1.00 (D-E-4),  
WB=0.00/1.00 (n/a.0), SSI=0.09/1.00 (A-B-5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10  
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.09 (E) (INPUT = 0.90)  
JSI METAL = 0.06 (B) (INPUT = 1.00)

CITY OF RICHMOND HILL  
BUILDING DEPARTMENT

09/02/2021

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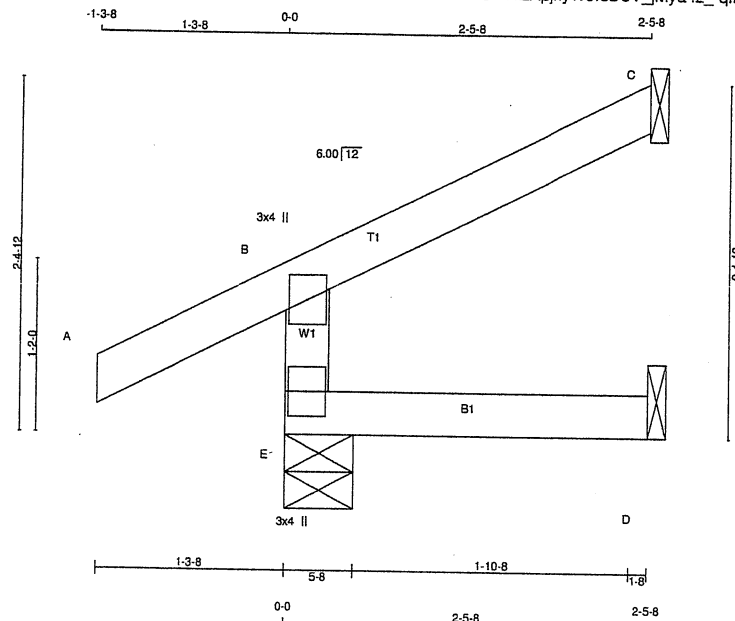
Per: jocelyn.aguilar



Structural component only  
DWG# T-2121191

JOB NAME <b>412865</b>	TRUSS NAME <b>J12</b>	QUANTITY <b>7</b>	PLY <b>1</b>	JOB DESC. <b>ROYAL PINE HOMES</b>	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1:14.9

<b>LUMBER</b>			
N. L. G. A. RULES			
CHORDS	SIZE		LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
E	291	0	291	0
C	85	0	85	0
D	20	0	22	0

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

**UNFACTORED REACTIONS**

JT	1ST LCASE	MAX/MIN	COMPONENT REACTIONS
	COMBINED	SNOW	LIVE
E	203	148 / 0	0 / 0
C	58	47 / 0	0 / 0
D	16	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

**BRACING**

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

**LOADING**

TOTAL LOAD CASES: (5)

FR-TO	CHORDS			WEBS		
	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
E-B		-265 / 0	0.0	0.0	0.02 (4)	7.81
A-B		0 / 28	-91.8	-91.8	0.12 (5)	10.00
B-C		-12 / 0	-91.8	-91.8	0.09 (1)	6.25
E-D		0 / 0	-18.5	-18.5	0.03 (4)	10.00

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 7 X 9 = 60 lb

**DESIGN CRITERIA**

**SPECIFIED LOADS:**

TOP CH.	LL	= 25.6 PSF
	DL	= 6.0 PSF
BOT CH.	LL	= 0.0 PSF
	DL	= 7.4 PSF
TOTAL LOAD	=	39.0 PSF

**SPACING = 24.0 IN/C/C**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

**DESIGN ASSUMPTIONS**

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")  
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")  
ALLOWABLE DEFL.(TL)= L/360 (0.19")  
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.12/1.00 (A-B:5), BC=0.03/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.10/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

**NAIL VALUES**

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.1 (E) (INPUT = 0.90)  
JSI METAL=0.07 (B) (INPUT = 1.00)

CITY OF RICHMOND HILL  
BUILDING DEPARTMENT

09/02/2021

RECEIVED

Per: jocelyn.aguilar



Structural component only  
DWG# T-2121192



Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 1 12:26:15 2021 Page 1  
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PLATE ROTATION TOL = 5.0 Deg.  
JSI GRIP = 0.09 (E) (INPUT = 0.90 )  
JSI METAL = 0.06 (B) (INPUT = 1.00 )

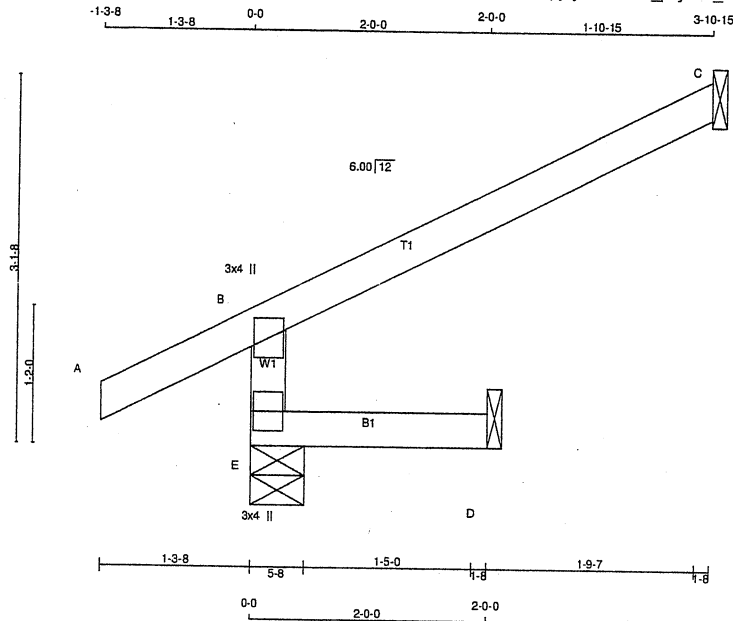
09/02/2021

RECEIVED

Per: jocelyn.aguilar

JOB NAME <b>412865</b>	TRUSS NAME <b>J14</b>	QUANTITY <b>2</b>	PLY <b>1</b>	JOB DESC. <b>ROYAL PINE HOMES</b>	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			

Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Thu Jul 1 12:26:16 2021 Page 1  
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Scale = 1:18.6

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

#### DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX		
E	369	0	369	0	5-8	5-8		
C	135	0	135	0	1-8	1-8		
D	16	0	18	0	1-8	1-8		

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

#### UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	WIND			
E	256	194 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0
C	93	75 / 0	0 / 0	0 / 0	0 / 0	18 / 0	0 / 0
D	13	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

#### BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

#### LOADING

TOTAL LOAD CASES: (5)

FR-TO	CHORDS		MEMB.		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FORCE (LBS)	MAX. UNBRACED LENGTH	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH
E-B	-349 / 0	0.0	0.0	0.01 (4)	7.81	
A-B	0 / 28	-91.8	-91.8	0.13 (5)	10.00	
B-C	-20 / 0	-91.8	-91.8	0.24 (1)	6.25	
E-D	0 / 0	-18.5	-18.5	0.02 (4)	10.00	

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 2 X 10 = 20 lb [M]

#### DESIGN CRITERIA

##### SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
DL = 6.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.4 PSF	
TOTAL LOAD = 39.0 PSF	

##### SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

##### THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

##### DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) =  $L/360$  (0.19")  
CALCULATED VERT. DEFL.(LL) =  $L/999$  (0.00")  
ALLOWABLE DEFL.(TL) =  $L/360$  (0.19")  
CALCULATED VERT. DEFL.(TL) =  $L/999$  (0.00")

CSI: TC=0.24/1.00 (B-C:1), BC=0.02/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

##### NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.14 (E) (INPUT = 0.90)  
JSI METAL = 0.10 (E) (INPUT = 1.00)

CITY OF RICHMOND HILL  
BUILDING DEPARTMENT

09/02/2021

RECEIVED

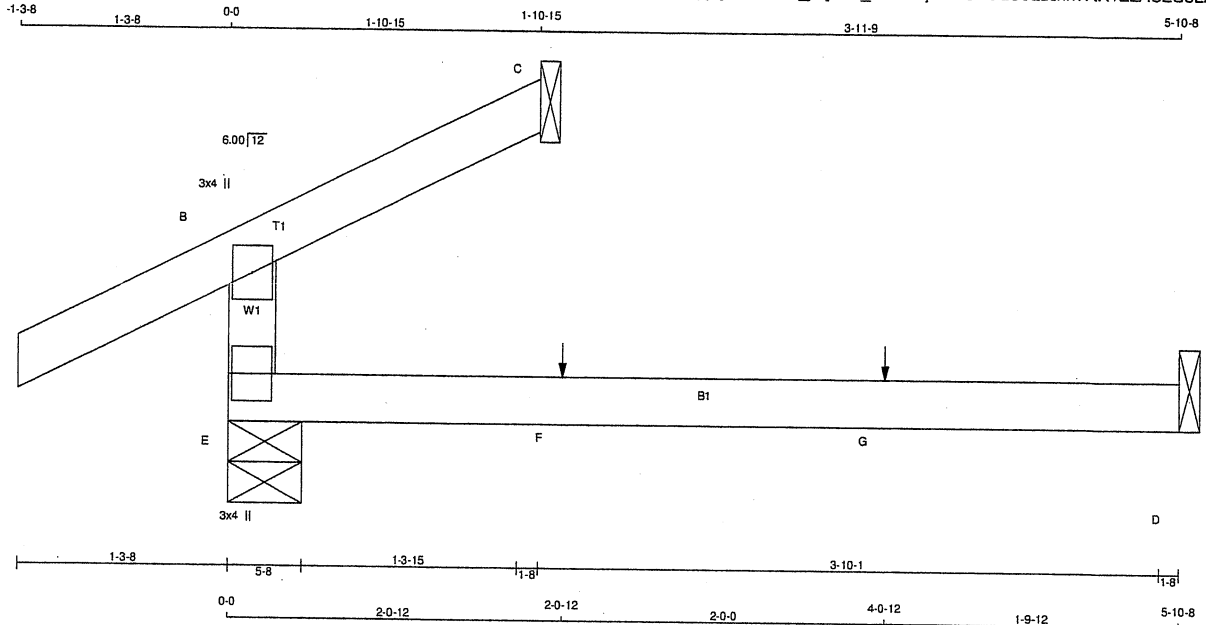
Per: jocelyn.aguilar



Structural component only  
DWG# T-2121194

JOB NAME <b>412865</b>	TRUSS NAME <b>J15</b>	QUANTITY <b>2</b>	PLY <b>1</b>	JOB DESC. <b>ROYAL PINE HOMES</b>	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1:13.5

<b>LUMBER</b>			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	
E - B	2x4 DRY	No.2	
A - C	2x4 DRY	No.2	
E - D	2x4 DRY	No.2	

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
E	297	0	297	0	5-8	5-8
C	66	0	66	0	1-8	1-8
D	45	0	50	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

**UNFACTORED REACTIONS**

JT	1ST LC CASE		MAX / MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
E	212	130 / 0	0 / 0	0 / 0	0 / 0	82 / 0	0 / 0
C	46	37 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

**BRACING**

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

**LOADING**

TOTAL LOAD CASES: (4)

FR-TO	CHORDS		FACTORED		MAX.	MEMB.	WEBS		MAX.
	MAX. FACTORED	FORCE	VERT. LOAD	LC1	MAX		MAX. FACTORED	FORCE	
	(LBS)	(LBS)	(PLF)	CSI (LC)	UNBRAC LENGTH		(LBS)	CSI (LC)	
E-B	-234 / 0	0.0	0.0	0.13 (4)	7.81				
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00				
B-C	-10 / 0	-91.8	-91.8	0.06 (1)	10.00				
E-F	0 / 0	-18.5	-18.5	0.13 (4)	10.00				
F-G	0 / 0	-18.5	-18.5	0.13 (4)	10.00				
G-D	0 / 0	-18.5	-18.5	0.13 (4)	10.00				

**SPECIFIED CONCENTRATED LOADS (LBS)**

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	2-0-12	1	1	---	FRONT	VERT	TOTAL	---	C1
G	4-0-12	1	1	---	FRONT	VERT	TOTAL	---	C1

**CONNECTION REQUIREMENTS**

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

**DESIGN CRITERIA**

**SPECIFIED LOADS:**

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

**SPACING = 24.0 IN. C/C**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

**DESIGN ASSUMPTIONS**

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) =  $L/360$  (0.20")  
CALCULATED VERT. DEFL. (LL) =  $L/999$  (0.00")  
ALLOWABLE DEFL. (TL) =  $L/360$  (0.20")  
CALCULATED VERT. DEFL. (TL) =  $L/999$  (0.03")

CSI: TC=0.13/1.00 (B-E:4), BC=0.13/1.00 (D-E:4),  
WB=0.00/1.00 (n/a:0), SSI=0.09/1.00 (A-B:1)

DOL LUMBER=0.99 NAIL=0.99 LS BEND=1.10  
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

**NAIL VALUES**

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.09 (E) (INPUT = 0.90)  
JSI METAL = 0.07 (B) (INPUT = 1.00)

CITY OF RICHMOND HILL  
BUILDING DEPARTMENT

09/02/2021

RECEIVED

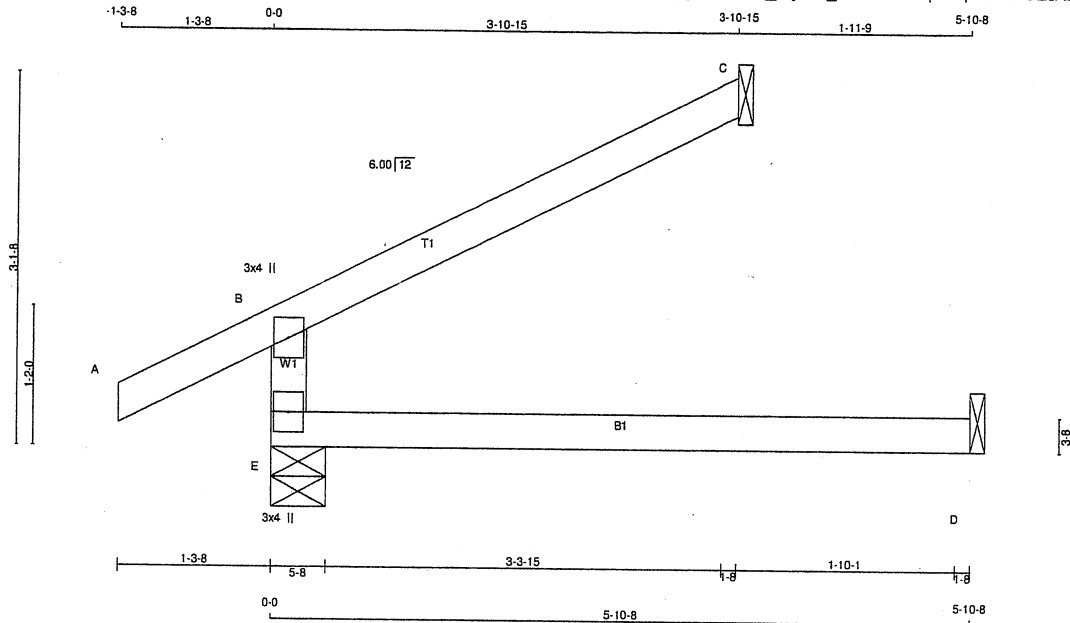
Per: jocelyn.aguiar



Structural component only  
DWG# T-2121195

JOB NAME <b>412865</b>	TRUSS NAME <b>J16</b>	QUANTITY <b>2</b>	PLY <b>1</b>	JOB DESC. <b>ROYAL PINE HOMES</b>	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 1 12:26:18 2021 Page 1  
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TOTAL WEIGHT = 2 X 14 = 29 lb [M]

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4 DRY	No.2	SPF
A - C	2x4 DRY	No.2	SPF
E - D	2x4 DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

#### DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX		
E	412	0	412	0	5-8	5-8		
C	135	0	135	0	1-8	1-8		
D	45	0	50	0	1-8	1-8		

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

#### UNFACTORED REACTIONS

JT	1ST CASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
E	291	194 / 0	0 / 0	0 / 0	0 / 0	97 / 0	0 / 0
C	93	75 / 0	0 / 0	0 / 0	0 / 0	18 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

#### BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

#### LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		WEBS		MAX. FACTORED	
	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX (LC)	MEMB. FORCE (LBS)	MAX (LC)	MAX. FORCE (LBS)	MAX (LC)
FR-TO		FROM	TO		FR-TO			
E-B	-349 / 0	0.0	0.0	0.13 (4)	7.81			
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00			
B-C	-20 / 0	-91.8	-91.8	0.24 (1)	6.25			
E-D	0 / 0	-18.5	-18.5	0.13 (4)	10.00			

#### DESIGN CRITERIA

##### SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

##### SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

##### DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")  
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")  
ALLOWABLE DEFL.(TL)= L/360 (0.20")  
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.24/1.00 (B-C:1), BC=0.13/1.00 (D-E:4),  
WB=0.00/1.00 (n/a:0), SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10  
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

##### NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.14 (E) (INPUT = 0.90)  
JSI METAL=0.10 (E) (INPUT = 1.00)

CITY OF RICHMOND HILL  
BUILDING DEPARTMENT

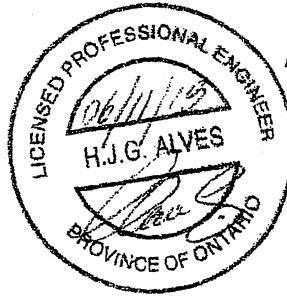
09/02/2021

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Per: jocelyn.aguilar



Structural component only  
DWG# T-2121196



## Alves Engineering Services Inc.

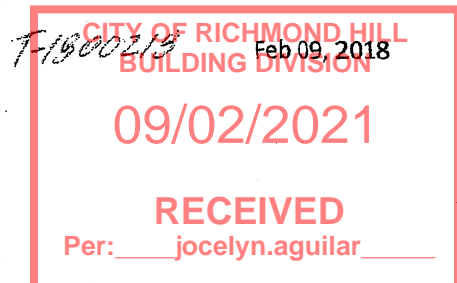
5208 Easton road  
Burlington, Ontario L7L 6N6  
(289) 259 5455

### RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

### SPECIFICATIONS

- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.





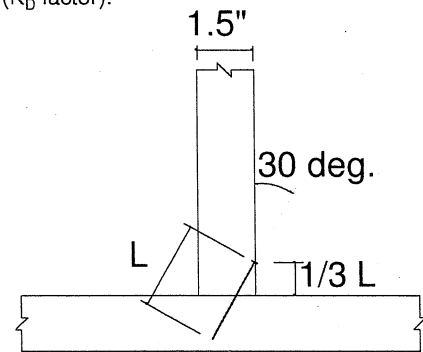
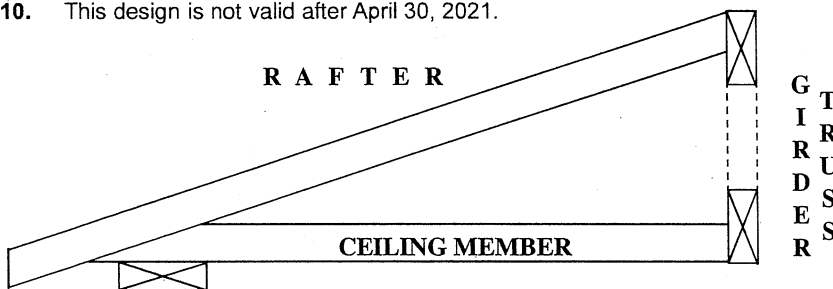
# BEARING ANCHORAGE BY TOE-NAILS FOR LATERAL CAPACITY

B97791H1

NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL LATERAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	132	147
	3.25	0.144	132	147
	3.50	0.160	159	177
COMMON SPIRAL	3.00	0.122	97	108
	3.25	0.122	97	108
	3.50	0.152	145	162

## NOTES:

1. Rafter and ceiling members may be anchored to top and bottom chords of girder truss by toe-nailing rafter and ceiling members to girder chords provided the reaction does not exceed the lateral capacities in the table. Hangers (specified by others) are required for reactions higher than the maximum toe-nail capacity. Reactions are based on factored loads.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor  $J_A$  in CSA O86-14, section 12.9.4.1.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in tables below.
5. Nail values in table are based on the following relative lumber densities: G = 0.42 (SPF), G = 0.49 (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See next page for nailing on bearing plate).
7. For loads due to **wind** the nail lateral capacity in this table may be multiplied by 1.15 ( $K_D$  factor).
8. Lumber must be dry ( < 19% moisture content ) at the time of nail installation.
9. Nail values in this table comply with CSA O86-14, section 12.9.4
10. This design is not valid after April 30, 2021.

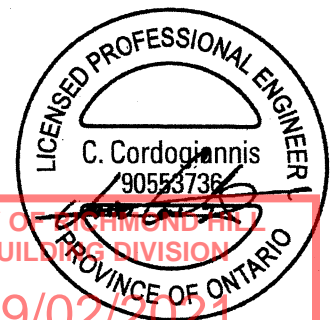


TOE-NAIL INSTALLATION

Nail type	Common wire	Common spiral	Common wire	Common spiral
Nail dia. (in)	0.160	0.152	0.144	0.122
	( 3.5" nail )		( 3" and 3.25" nail )	
LUMBER SIZE	MAXIMUM NUMBER OF TOE-NAILS			
2X4 SPF	2	2	3	3
2X4 D. Fir	2	2	2	2
2X6 SPF	4	4	4	5
2X6 D. Fir	3	3	3	4

**MiTek**® MiTek Canada Inc  
100 Industrial Rd.  
Bradford, Ontario L3Z 3G7

PEO  
Certificate No. 10889485



April 2, 2020

**RECEIVED**  
Per: jocelyn.aguilar

# BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING

B97791H2

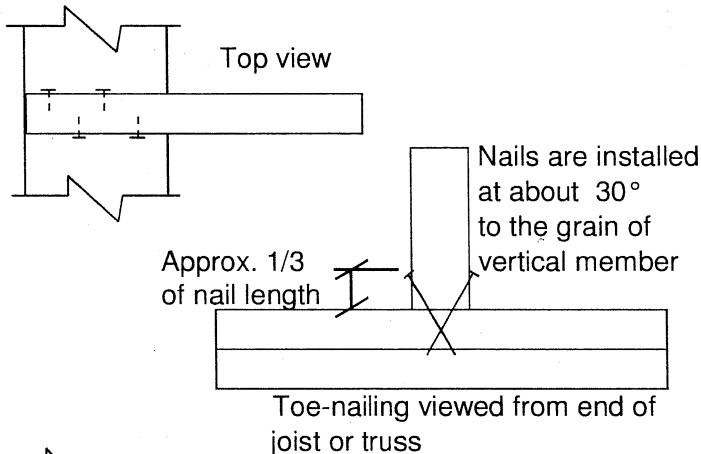
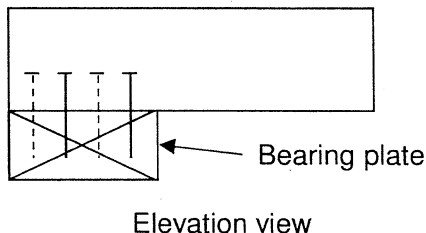
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	30	42
	3.25	0.144	32	45
	3.50	0.160	38	52
COMMON SPIRAL	3.00	0.122	26	36
	3.25	0.122	28	40
	3.50	0.152	36	50

**Note:** If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

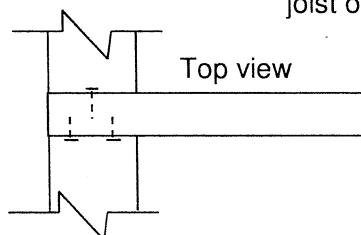
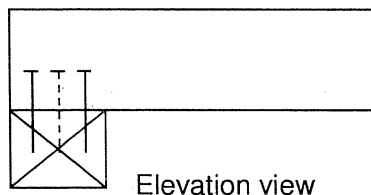
## NOTES:

1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to **wind** or **earthquake** load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor  $J_A$  in CSA O86-14, section 12.9.5.2.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities:  $G = 0.42$ (SPF),  $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of  $30^\circ$  to the grain of the member (See drawing on detail B37579H1).
7. Lumber must be dry ( < 19% moisture content ) at the time of nail installation.
8. Nail values in this table comply with CSA O86-14, section 12.9.5
9. This design is not valid after April 30, 2021.

### Toe-nailing on 2x6 Bearing Plate



### Toe-nailing on 2x4 Bearing Plate

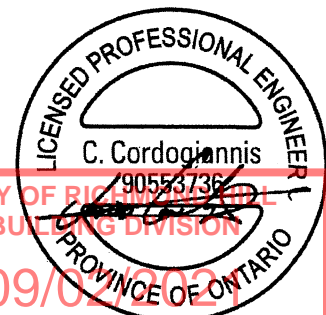


**MiTek**®

MiTek Canada Inc  
100 Industrial Rd.  
Bradford, Ontario L3Z 3G7

April 2, 2020

PEO  
Certificate No. 10889485



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# HUS/LJS – Double Shear Joist Hangers

**SIMPSON**  
**Strong-Tie**

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

**Material:** See table

**Finish:** G90 galvanized

**Design:**

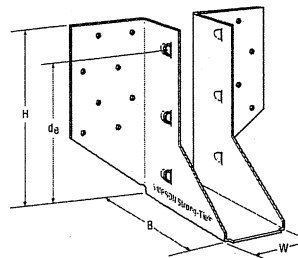
- Factored resistances are in accordance with CSA O86 -14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

**Installation:**

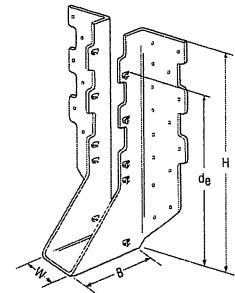
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

**Options:**

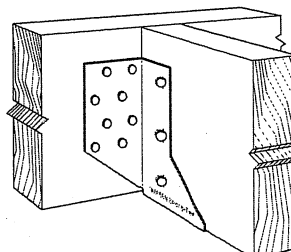
- See current catalogue for options



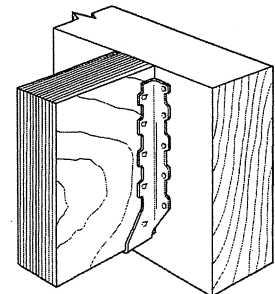
LJS26DS



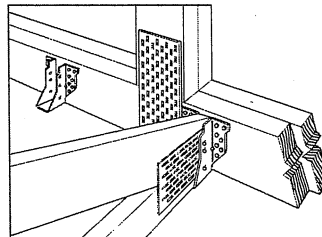
HUS210  
(HUS26, HUS28, similar)



Typical LJS26DS  
Installation



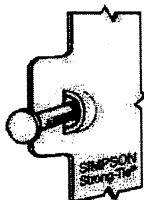
Typical HUS  
Installation



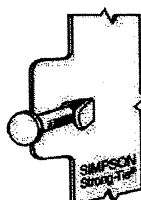
Typical HUS Installation  
(Truss Designer to provide fastener quantity for connecting multiple members together)

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d <sub>g</sub> <sup>1</sup>	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K <sub>p</sub> =1.15) lb.	Normal (K <sub>p</sub> =1.00) lb.	Uplift (K <sub>p</sub> =1.15) lb.	Normal (K <sub>p</sub> =1.00) lb.
LJS26DS	18	1½	5	3½	4⅞	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1⅝	5⅞	3	3⅝	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1⅝	7⅝	3	6⅝	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1⅝	9⅝	3	7⅝	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1⅝	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

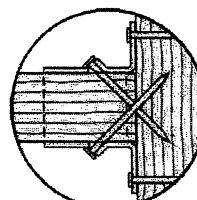
1. d<sub>g</sub> is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).  
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing  
Top View.



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# HGUS – Double Shear Joist Hangers

**SIMPSON**  
**Strong-Tie**

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

**Material:** 12 gauge

**Finish:** G90 galvanized

**Design:**

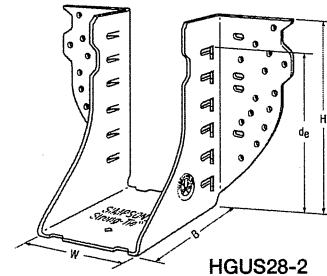
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%.  
No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

**Installation:**

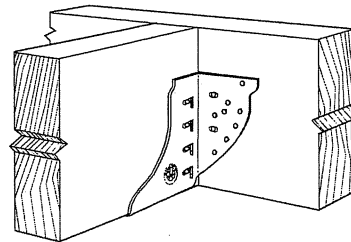
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

**Options:**

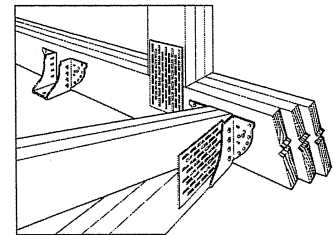
- See current catalogue for options



HGUS28-2



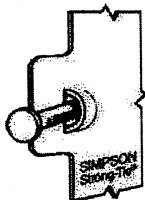
Typical HGUS Installation



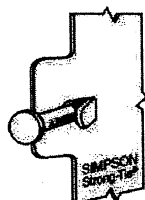
Typical HGUS Installation  
(Truss Designer to provide fastener quantity for connecting multiple members together)

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d <sub>e</sub> <sup>1</sup>	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K <sub>p</sub> =1.15)	Normal (K <sub>p</sub> =1.00)	Uplift (K <sub>p</sub> =1.15)	Normal (K <sub>p</sub> =1.00)
HGUS26	12	1⅝	5⅝	5	4⅞	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3⅝	5⅝	4	4⅞	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4⅝	5⅝	4	4⅞	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6⅝	5⅝	4	4⅞	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1⅝	7⅝	5	6⅞	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3⅝	7⅝	4	6⅞	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4⅝	7⅝	4	6⅞	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6⅝	7⅝	4	6⅞	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210	12	1⅝	9⅝	5	7⅞	(46) 16d	(16) 16d	3535	11070	2510	8090
HGUS210-2	12	3⅝	9⅝	4	8⅞	(46) 16d	(16) 16d	6840	14015	4855	10270
HGUS210-3	12	4⅝	9⅝	4	8⅞	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6⅝	9⅝	4	8⅞	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6⅝	10⅝	4	10⅞	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6⅝	12⅝	4	11⅞	(66) 16d	(22) 16d	10130	16400	7195	11645

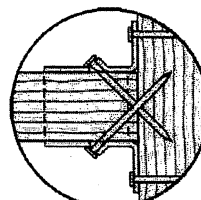
1. d<sub>e</sub> is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).  
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



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# H – Seismic and Hurricane Ties

**SIMPSON**  
**Strong-Tie**

The H connector series provides wind and seismic ties for trusses and rafters.

**Material:** 18 gauge **Finish:** G90 galvanized

**Design:** • Factored resistances are in accordance with CSA O86-14

• Factored resistances have been increased 15%. No further increase is permitted.

**Installation:** • Use all specified fasteners

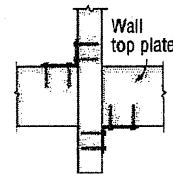
• Nails: 8d = 0.131" dia. x 2½" long common wire, 8d x 1½" = 0.131" x 1½" long, 10d x 1½" = 0.146" x 1½" long

• H1 can be installed with flanges facing outwards

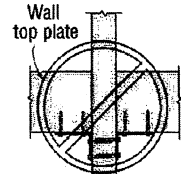
• Hurricane ties do not replace solid blocking

Factored resistances for more than one direction for a single connection cannot be added together. A factored load which can be divided into components in the directions given must be evaluated as follows: Factored Shear/Resisting Shear + Factored Tension/Resisting Tension ≤ 1.0.

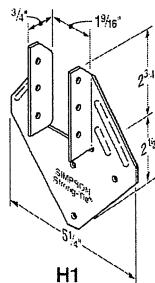
## Hurricane Tie Installations to Achieve Twice the Load (Top View)



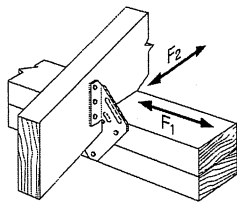
Install diagonally across from each other for minimum 2x truss.



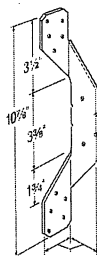
Nailing into both sides of a single ply 2x truss may cause the wood to split.



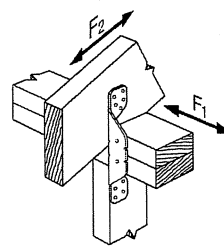
H1



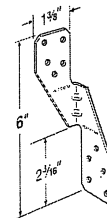
H1 Installation



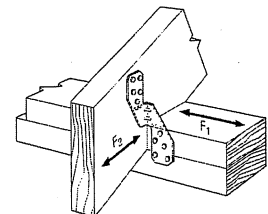
H2A



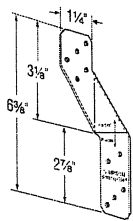
H2A Installation



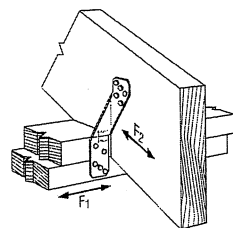
H2.5A



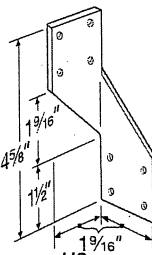
H2.5A Installation



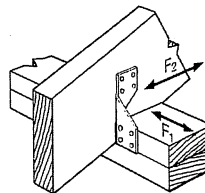
H2.5T



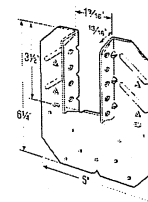
H2.5T Installation  
(Nails into both top plates)



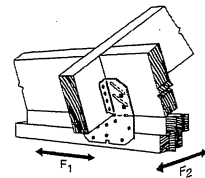
H3



H3 Installation



H10A



H10A Installation

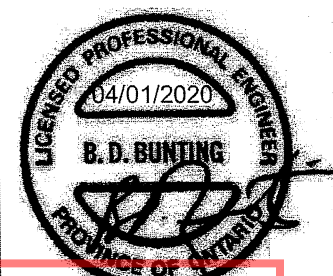
Model No.	Ga.	Fasteners			Factored Resistance (lb.)					
					D.Fir-L			S-P-F		
		To Rafter	To Plates	To Studs	Uplift	Normal		Uplift	Normal	
						F <sub>1</sub>	F <sub>2</sub>		F <sub>1</sub>	F <sub>2</sub>
						(K <sub>b</sub> =1.15)			(K <sub>b</sub> =1.15)	
H1	18	(6) 8d x 1½"	(4) 8d	—	740	685	300	680	485	215
H2A	18	(5) 8d x 1½"	(2) 8d x 1½"	(5) 8d x 1½"	830	220	75	590	155	55
H2.5A	18	(5) 8d	(5) 8d	—	805	160	160	755	160	160
H2.5T	18	(5) 8d	(5) 8d	—	835	175	240	740	160	210
H3	18	(4) 8d	(4) 8d	—	740	180	265	615	125	190
H10A	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1735	795	410	1505	565	290

1. Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.

2. Factored resistances are for one anchor. A minimum rafter thickness of 2½" must be used when framing anchors are installed on each side of the joist and on the same side of the plate.

3. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.

4. Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a Continuous Load Path, connections must be on same side of the wall.



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# LUS – Double Shear Joist Hangers

**SIMPSON**  
**Strong-Tie**

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

**Material:** 18 gauge

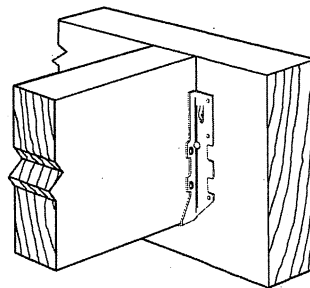
**Finish:** G90 galvanized

## Design:

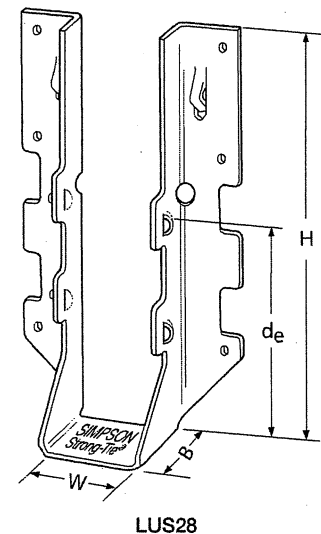
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

## Installation:

- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire,  
10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.



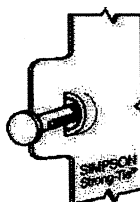
Typical LUS  
Installation



LUS28

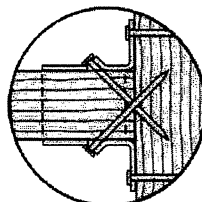
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d <sub>e</sub> <sup>1</sup>	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K <sub>o</sub> =1.15)	Normal (K <sub>o</sub> =1.00)	Uplift (K <sub>o</sub> =1.15)	Normal (K <sub>o</sub> =1.00)
LUS24	18	1⅞	3⅞	1¾	1 15/16	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3⅞	3⅞	2	1 13/16	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1⅞	4¾	1¾	3⅞	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3⅞	4¾	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4⅞	4¾	2	3¼	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1⅞	6⅞	1¾	3¾	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3⅞	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4⅞	6¼	2	3¼	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1⅞	7 13/16	1¾	3⅞	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3⅞	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4⅞	8 3/16	2	5¼	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d<sub>e</sub> is the distance from the seat of the hanger to the highest joist nail.



Dome Double  
Shear Nailing  
prevents tabs  
breaking off  
(available on  
some models).

U.S. Patent  
5,603,580



Double  
Shear  
Nailing  
Top View.



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FABRICATORS ASSOCIATION

## TECH-NOTES

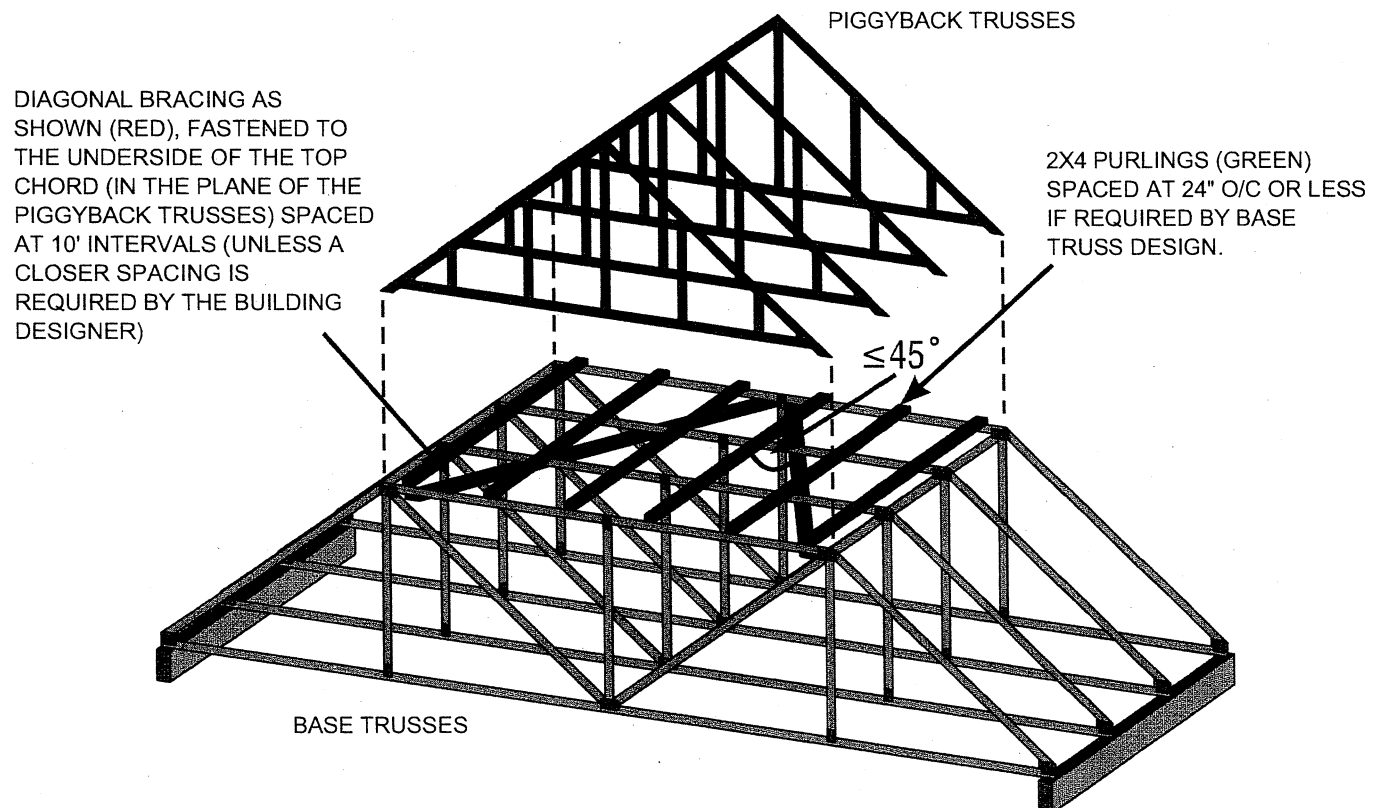
### TN 15-001 Piggyback Bracing

#### Overview:

Where piggybacks are connected ovetop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

#### Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

#### Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

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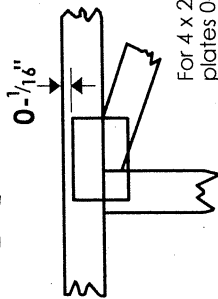
09/02/2021

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## Symbols

### PLATE LOCATION AND ORIENTATION

Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- $\frac{1}{16}$ " from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.



\* Plate location details available in MiTek software or upon request.

### PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

### LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.



### BEARING

Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

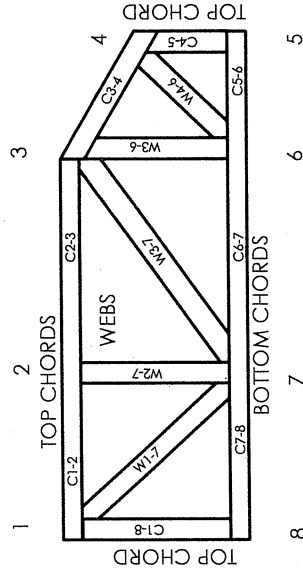


### Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses  
DS8-89: Design Standard for Bracing.  
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

## Numbering System

6-4-8 dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

### PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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MiTek Engineering Reference Sheet: MI-7473C rev. 10-'08

## General Safety Notes

### Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSi.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and waste at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.