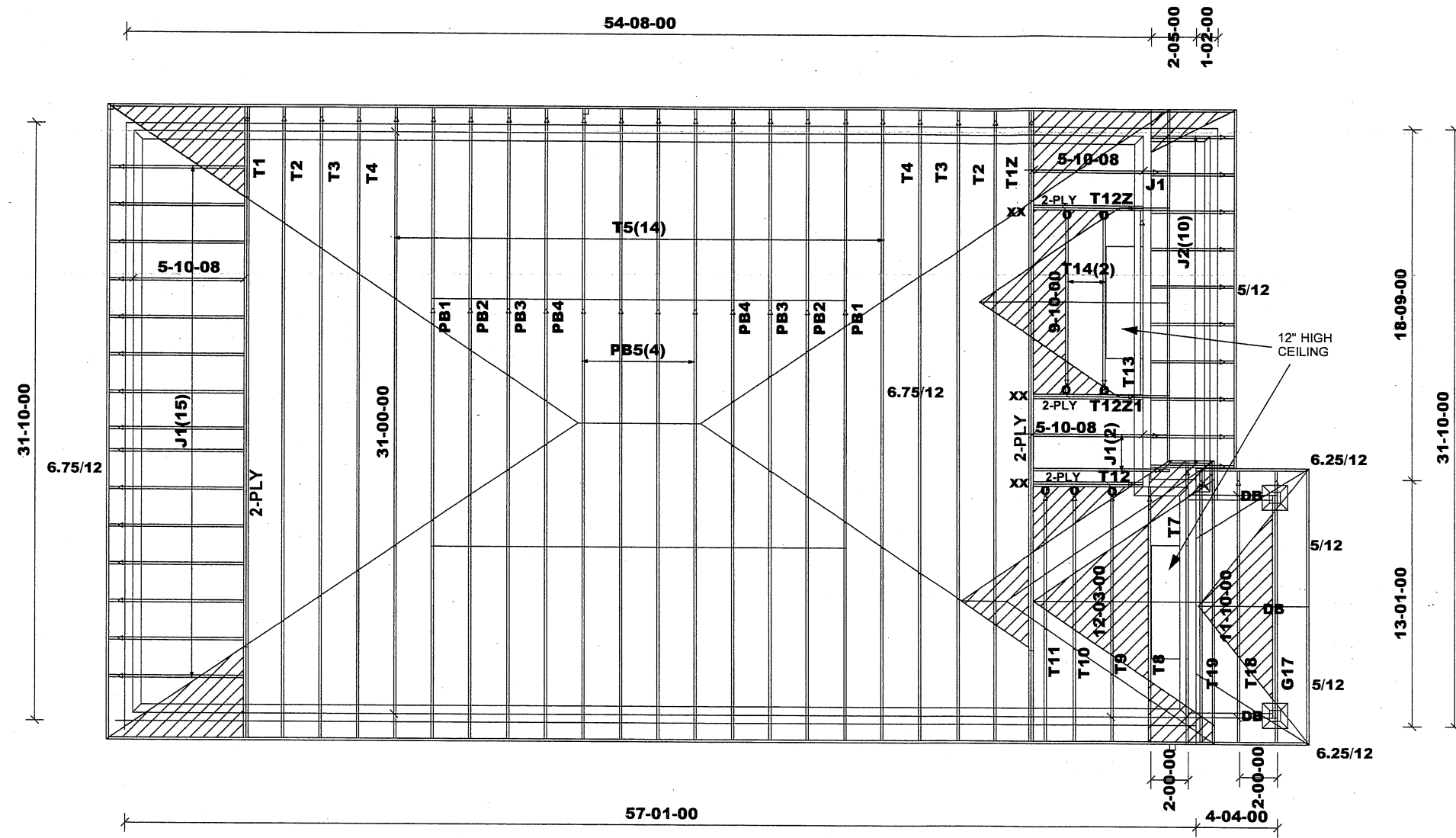


10/12 ROOF PITCHES UNLESS OTHERWISE NOTED



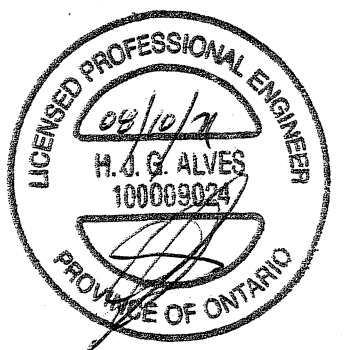
ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

 **DENOTES:**
CONVENTIONAL
FRAMING

DESIGN CONFORMS WITH OBC 2012
2019 (AMENDMENT)
OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 27.2 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 23.3 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)



DWG NO. TAM *T-2127198*
STRUCTURAL
COMPONENT ONLY

All conventional framing to conform with Part 9 of O.B.C. 2012. Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DB - DROPPED BEAMS BY OTHERS

DWG# T-2125197 to T-2125222, T1800218

TRUSS PLACEMENT PLAN.

This is a truss placement plan only, NOT a final roof framing plan. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual truss drawings for each component identified on this placement plan.

Please verify that all dimensions match the dimensions found on the job. The Building designer is responsible for the temporary/permanent bracing of the roof and floor system and its integration into the bracing of the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer unless otherwise noted in this plan. Building designer to review and approve this plan to ascertain conformity to his overall structural plan.



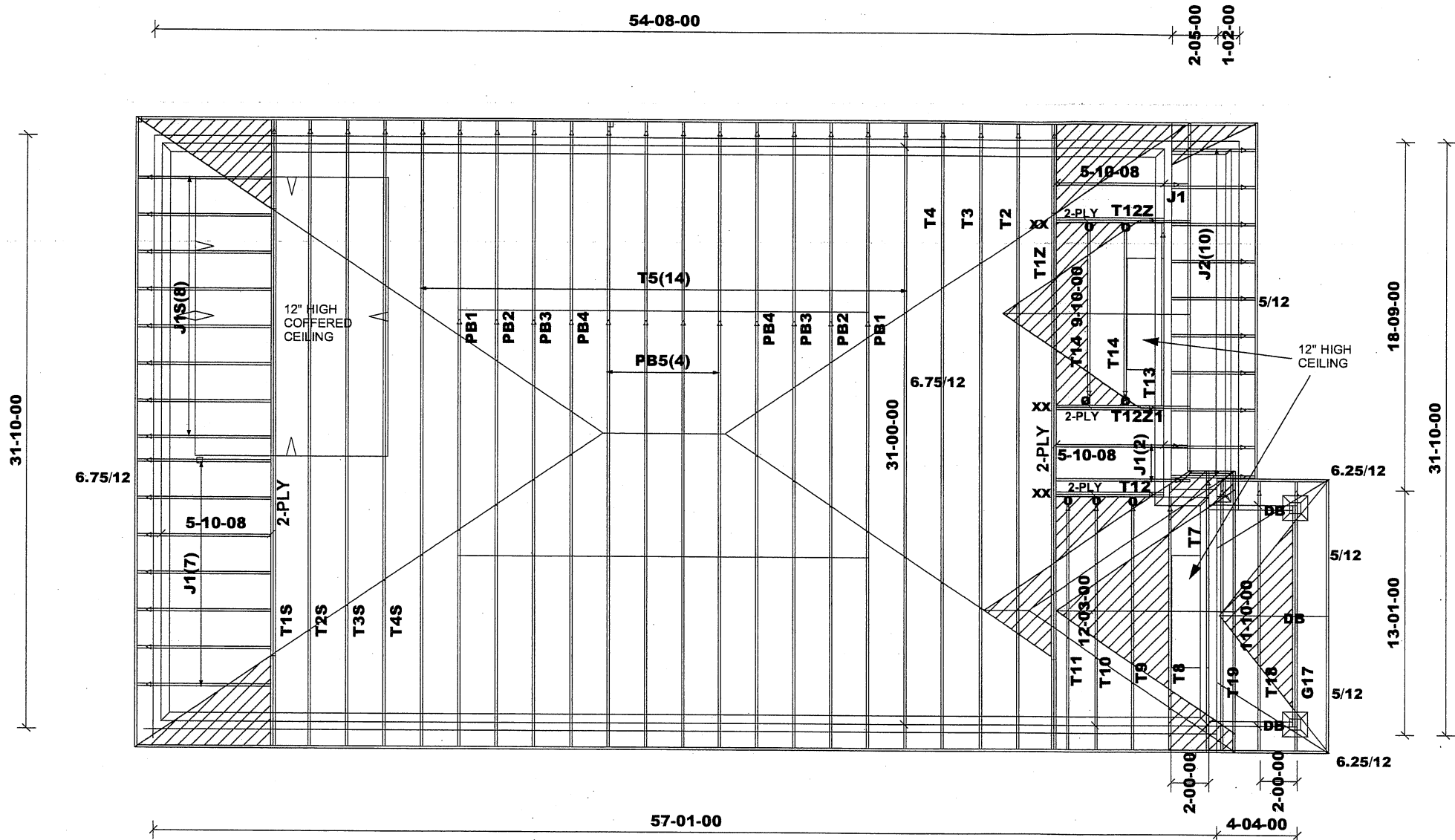
Job Track: **52430**
Layout ID: **420295**
Plan Log: **205018**

Builder / Location: **ROYAL PINE HOMES / BRAMPTON**
Project: **VALES OF CASTLEMORE NORTH**
Date: 2021-08-10 Designer: JC

Model / Elevation: **MODEL 40-5 / A STD. 4 & 5 BEDROOM**
Mitek ver 8.4.2.286
THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

M14215

10/12 ROOF PITCHES UNLESS OTHERWISE NOTED



ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

 **DENOTES:**
CONVENTIONAL
FRAMING

DESIGN CONFORMS WITH OBC 2012
2019 (AMENDMENT)
OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 27.2 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 23.3 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)



DWG NO. TAM T-2127199
STRUCTURAL
COMPONENT ONLY

All conventional framing to conform with Part 9 of O.B.C. 2012. Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

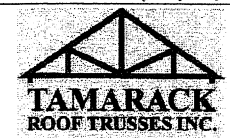
DB - DROPPED BEAMS BY OTHERS

DWG# T-2125197 to T-2125204, T-2125206 to
T-2125227, T1800218

TRUSS PLACEMENT PLAN.

This is a truss placement plan only, NOT a final roof framing plan. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual truss drawings for each component identified on this placement plan.

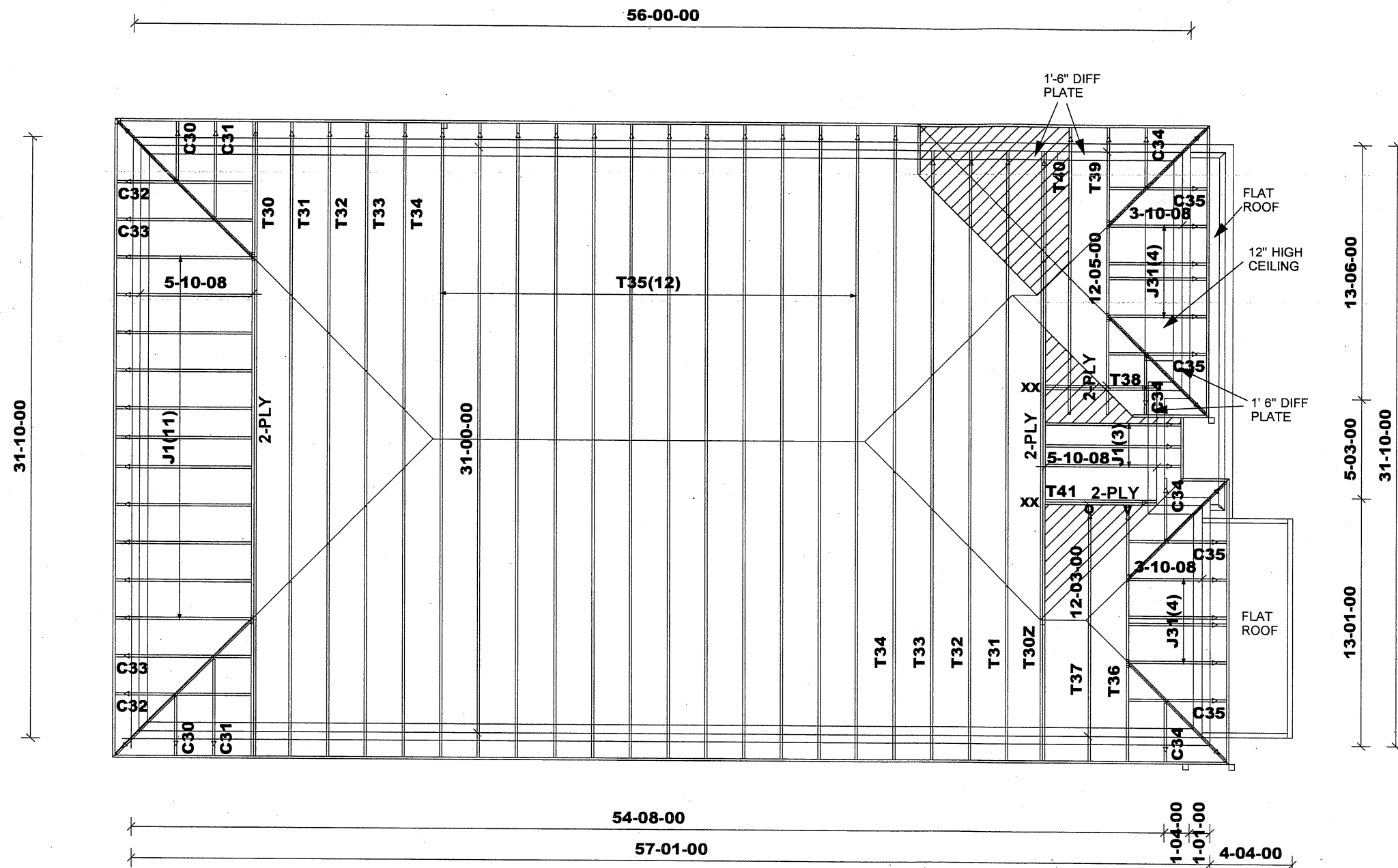
Please verify that all dimensions match the dimensions found on the job. The Building designer is responsible for the temporary/permanent bracing of the roof and floor system and its integration into the bracing of the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer unless otherwise noted in this plan. Building designer to review and approve this plan to ascertain conformity to his overall structural plan.



Job Track:	52430	Builder / Location:	ROYAL PINE HOMES / BRAMPTON
Layout ID:	420297	Project:	VALES OF CASTLEMORE NORTH
Plan Log:	205018	Date: 2021-08-10	Designer: JC

Model / Elevation:	MODEL 40-5 / A OPT. 4 & 5 BEDROOM
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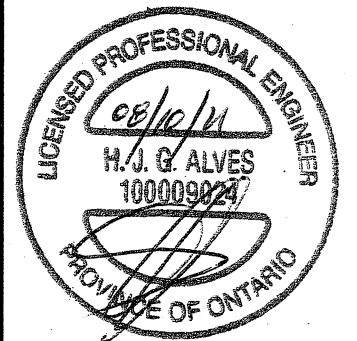
ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

 **DENOTES:**
CONVENTIONAL
FRAMING

DESIGN CONFORMS WITH OBC 2012
2019 (AMENDMENT)
OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 27.2 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 23.3 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)



DWG NO. TAM F-2127200
STRUCTURAL
COMPONENT ONLY

DWG# T-2125198, T-2125228 to T-2125233, T-2125235 to T-2125247, T-2126376, T1800218

TRUSS PLACEMENT PLAN.

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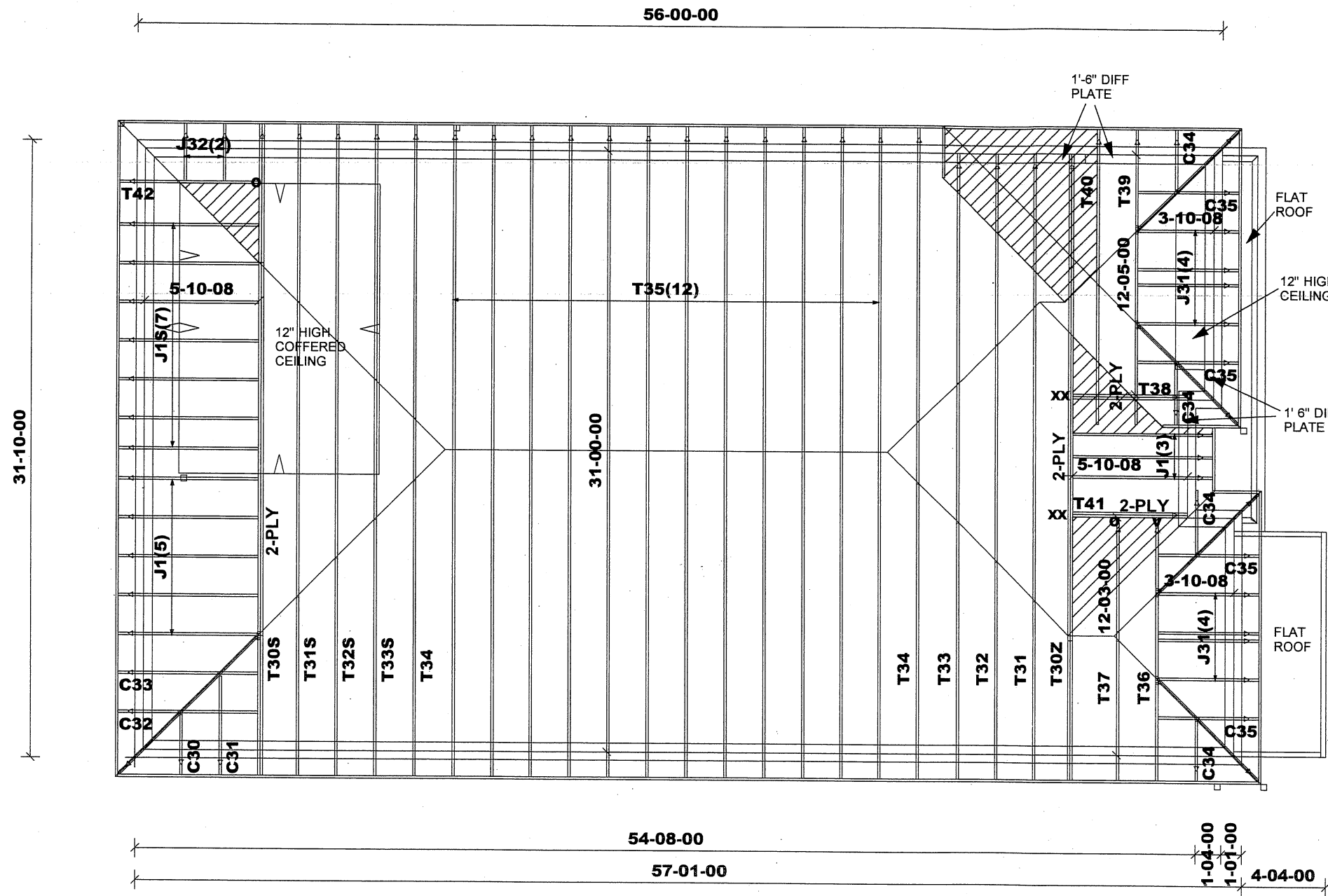


Job Track:	52430
Layout ID:	420298
Plan Log:	205018

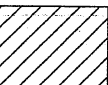
Builder / Location:		
ROYAL PINE HOMES / BRAMPTON		
Project:		
VALES OF CASTLEMORE NORTH		
Date: 2021-08-05	Designer: JC	

Model / Elevation:	MODEL 40-5 / B STD. 4 & 5 BEDROOM	Mitek ver 8.4.2.286
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6.75/12 ROOF PITCHES UNLESS OTHERWISE NOTED



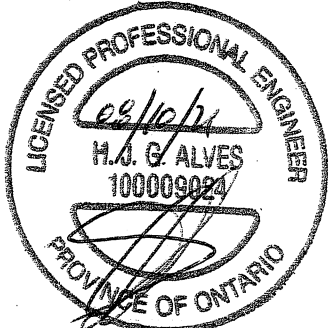
ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

 **DENOTES:**
CONVENTIONAL
FRAMING

DESIGN CONFORMS WITH OBC 2012
2019 (AMENDMENT)
OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 27.2 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 23.3 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)



DWG# T-2125198, T-2125223, T-2125228 to T-2125233, T-2125235, T-2125237 to T-2125247, T-2125249 to T-2125252, T-2126375 to T-2126377, T1800218

DWG NO. TAM *T-2127201*
STRUCTURAL
COMPONENT ONLY

All conventional framing to conform with Part 9 of O.B.C. 2012. Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

TRUSS PLACEMENT PLAN.

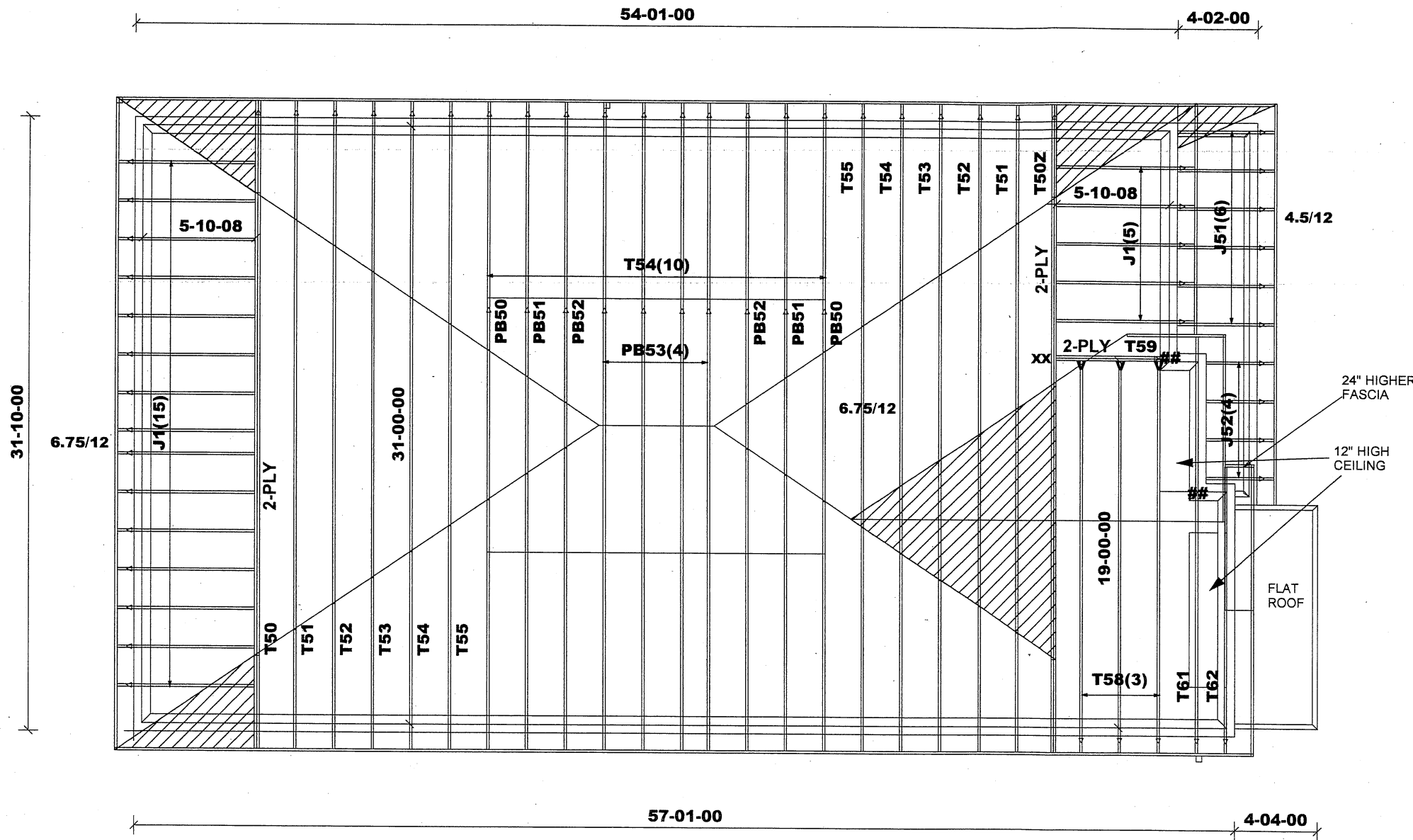
This is a truss placement plan only, NOT a final roof framing plan. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual truss drawings for each component identified on this placement plan.

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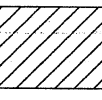
114215

	Job Track: 52430	Builder / Location: ROYAL PINE HOMES / BRAMPTON		Model / Elevation: MODEL 40-5 / B OPT. 4 & 5 BEDROOM	
	Layout ID: 420299	Project: VALES OF CASTLEMORE NORTH		Mitek ver 8.4.2.286	
	Plan Log: 205018	Date: 2021-08-05 Designer: JC		THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.	

10/12 ROOF PITCHES UNLESS OTHERWISE NOTED



ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

 **DENOTES:**
CONVENTIONAL
FRAMING

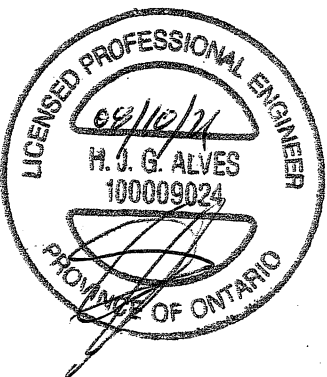
DESIGN CONFORMS WITH OBC 2012
2019 (AMENDMENT)
OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 27.2 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 23.3 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)

- 12" PLATE
DIFF.

DWG# T-2125198, T-2125254 to T-2125270,
T1800218



DWG NO. TAM T-2127202
STRUCTURAL
COMPONENT ONLY

All conventional framing to conform with Part 9 of O.B.C. 2012. Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

TRUSS PLACEMENT PLAN.

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Job Track: **52430**
Layout ID: **420300**
Plan Log: **205018**

Builder / Location: **ROYAL PINE HOMES / BRAMPTON**
Project: **VALES OF CASTLEMORE NORTH**
Date: 2021-08-05 Designer: **JC**

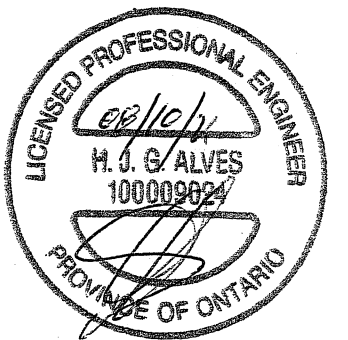
Model / Elevation: **MODEL 40-5 / C STD. 4 & 5 BEDROOM**
Mitek ver 8.4.2.286
THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.



DESIGN LOADS:
TCSL = 23.3 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

- 12" PLATE
DIFF.

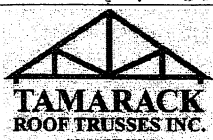


DWG NO. TAME T-2127203
STRUCTURAL
COMPONENT ONLY

DWG# T-2125198, T-2125223, T-2125254 to T-2125259, T-2125261 to T-2125274, T1800218

TRUSS PLACEMENT PLAN.

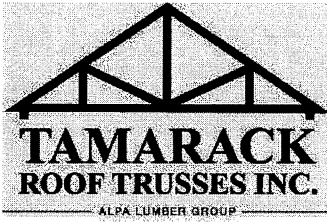
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Job Track:	52430	Builder / Location:		
Layout ID:	420301	ROYAL PINE HOMES / BRAMPTON		
Plan Log:	205018	Project: VALES OF CASTLEMORE NORTH		
		Date: 2021-08-05	Designer: JC	

Model / Elevation:	MODEL 40-5 / C OPT. 4 & 5 BEDROOM	Mitek ver 8.4.2.286
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DELIVERY SHIPLIST

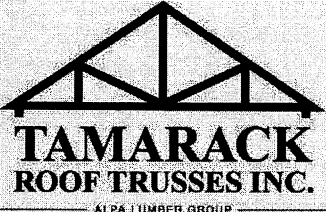


Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: VALES OF CASTLEMORE NORTH
 Location: BRAMPTON
 Model: MODEL 40-5
 Lot #:
 Elevation: A STD. 4 & 5 BEDROOM

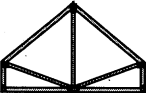
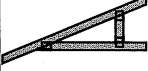
Job Track: 52430
 PlanLog: 205018
 Layout ID: 420295
 Ref #
 Page: 1 of 3
 Date: 08-04-2021
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	10 /12	31-00-00	4-06-12	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	344.45 212.00		
	1 2-ply	T1Z Hip Girder	10 /12	31-00-00	4-06-12	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	344.45 212.00		
	2	T2 Hip	10 /12	31-00-00	5-08-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	266.14 173.33		
	2	T3 Hip	10 /12	31-00-00	6-09-12	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	294.81 187.00		
	2	T4 Hip	10 /12	31-00-00	7-11-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	292.85 184.33		
	14	T5 Hip	10 /12	31-00-00	9-00-12	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	2073.21 1304.33		
	1	T7 Roof Special	10 /12	12-03-00	6-06-12	2 x 4	1-03-08 1-03-08	9-11 9-11	63.56 43.83		
	1	T8 Common	10 /12	12-03-00	6-06-12	2 x 4	1-03-08 1-03-08	9-11 9-11	56.27 35.00		
	1	T9 Hip	10 /12	12-03-00	3-06-01	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	52.78 34.33		
	1	T10 Hip	10 /12	12-03-00	4-07-09	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	54.9 37.17		
	1	T11 Hip	10 /12	12-03-00	5-06-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	58.23 37.00		
	1 2-ply	T12 Monopitch Girder	6.75 /1 2	5-10-08	4-06-12	2 x 4 2 x 6		1-03-01 4-06-12	60.07 38.00		
	1 2-ply	T12Z Monopitch Girder	6.75 /1 2	5-10-08	4-06-12	2 x 4 2 x 6		1-03-01 4-06-12	60.07 38.00		
	1 2-ply	T12Z1 Monopitch Girder	6.75 /1 2	5-10-08	4-06-12	2 x 4 2 x 6		1-03-01 4-06-12	60.07 38.00		

 TAMARACK ROOF TRUSSES INC. ALPHA LUMBER GROUP	DELIVERY SHIPLIST	
	Lumber Yard: TAMARACK LUMBER Builder: ROYAL PINE HOMES Project: VALES OF CASTLEMORE NORTH Location: BRAMPTON Model: MODEL 40-5 Lot #: Elevation: A STD. 4 & 5 BEDROOM	Job Track: 52430 PlanLog: 205018 Layout ID: 420295 Ref # Page: 2 of 3 Date: 08-04-2021 Designer: Sales Rep: Rick DiCiano

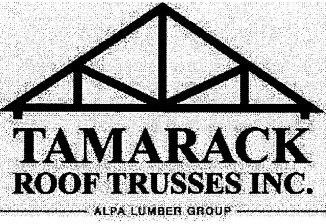
Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T13 Roof Special	10 /12	9-10-00	6-02-04	2 x 4	0-06	2-01-01 2-01-01	52.76 35.17		
	2	T14 Common	10 /12	9-10-00	6-02-04	2 x 4		2-01-01 2-01-01	84.99 53.33		
	1	G17 GABLE	5 /12	11-10-00	3-03-13	2 x 4	1-03-08 1-03-08	4-01 4-01	35.66 22.17		
	1	T18 Hip	10 /12	11-10-00	2-02-04	2 x 4	1-03-08 1-03-08	11-03 11-03	44.91 31.00		
	1	T19 Hip	10 /12	11-10-00	3-02-12	2 x 4	1-03-08 1-03-08	11-03 11-03	48.9 32.83		
	2	PB1 Piggyback	10 /12	13-02-05	1-01-08	2 x 4			77.23 49.00		
	2	PB2 Piggyback	10 /12	13-02-05	2-03-00	2 x 4			81.18 51.33		
	2	PB3 Piggyback	10 /12	13-02-05	3-04-08	2 x 4			80.25 52.00		
	2	PB4 Piggyback	10 /12	13-02-05	4-06-00	2 x 4			83.97 54.33		
	4	PB5 Piggyback	10 /12	13-02-05	5-06-00	2 x 4			171.55 106.00		
	18	J1 Jack-Open	6.75 /1 2	5-10-08	4-06-12	2 x 4	1-03-08	1-03-01 4-06-11	309 192.00		
	10	J2 Jack-Open	5 /12	3-02-00	2-06-04	2 x 4	1-03-08	4-01 1-07-14	124.32 80.00		

TOTAL # TRUSS= 81 TOTAL BFT OF ALL TRUSSES= 3333.48 BFT. TOTAL WEIGHT OF ALL TRSSES 5276.58 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
7	Hardware	LUS24	
3	Hardware	HGUS26-2	

DELIVERY SHIPLIST			
 TAMARACK ROOF TRUSSES INC. ALFA LUMBER GROUP	Lumber Yard:	TAMARACK LUMBER	Job Track: 52430
	Builder:	ROYAL PINE HOMES	PlanLog: 205018
	Project:	VALES OF CASTLEMORE NORTH	Layout ID: 420295
	Location:	BRAMPTON	Ref #
	Model:	MODEL 40-5	Page: 3 of 3
	Lot #:		Date: 08-04-2021
	Elevation:	A STD. 4 & 5 BEDROOM	Designer:
			Sales Rep: Rick DiCiano

HARDWARE

QTY	TYPE	MODEL	LENGTH
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TOTAL NUMBER OF ITEMS= 10

DELIVERY SHIPLIST



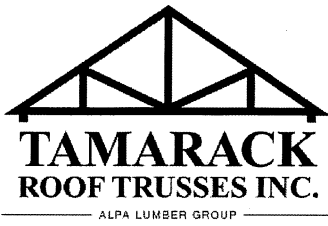
Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: VALES OF CASTLEMORE NORTH
 Location: BRAMPTON
 Model: MODEL 40-5
 Lot #:
 Elevation: A OPT. 4 & 5 BEDROOM

Job Track: 52430
 PlanLog: 205018
 Layout ID: 420297
 Ref #
 Page: 1 of 3
 Date: 08-10-2021
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1S Hip Girder	10 / 12	31-00-00	4-06-12	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	351.52 222.00		
	1 2-ply	T1Z Hip Girder	10 / 12	31-00-00	4-06-12	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	344.46 212.00		
	1	T2 Hip	10 / 12	31-00-00	5-08-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	133.07 86.67		
	1	T2S Hip	10 / 12	31-00-00	5-08-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	141.06 92.33		
	1	T3 Hip	10 / 12	31-00-00	6-09-12	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	147.41 93.50		
	1	T3S Hip	10 / 12	31-00-00	6-09-12	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	152.05 97.67		
	1	T4 Hip	10 / 12	31-00-00	7-11-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	146.43 92.17		
	1	T4S Hip	10 / 12	31-00-00	7-11-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	154.02 97.50		
	14	T5 Hip	10 / 12	31-00-00	9-00-12	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	2073.21 1304.33		
	1	T7 Roof Special	10 / 12	12-03-00	6-06-12	2 x 4	1-03-08 1-03-08	9-11 9-11	63.56 43.83		
	1	T8 Common	10 / 12	12-03-00	6-06-12	2 x 4	1-03-08 1-03-08	9-11 9-11	54.44 33.00		
	1	T9 Hip	10 / 12	12-03-00	3-06-01	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	52.78 34.33		
	1	T10 Hip	10 / 12	12-03-00	4-07-09	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	54.9 37.17		
	1	T11 Hip	10 / 12	12-03-00	5-06-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	58.23 37.00		

DELIVERY SHIPLIST

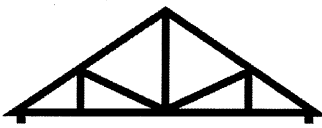


Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: VALES OF CASTLEMORE NORTH
 Location: BRAMPTON
 Model: MODEL 40-5
 Lot #:
 Elevation: A OPT. 4 & 5 BEDROOM

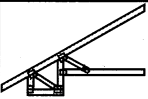
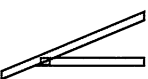
Job Track: 52430
 PlanLog: 205018
 Layout ID: 420297
 Ref #
 Page: 2 of 3
 Date: 08-10-2021
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T12 Monopitch Girder	6.75 /1 2	5-10-08	4-06-12	2 x 4 2 x 6		1-03-01 4-06-12	60.07 38.00		
	1 2-ply	T12Z Monopitch Girder	6.75 /1 2	5-10-08	4-06-12	2 x 4 2 x 6		1-03-01 4-06-12	60.07 38.00		
	1 2-ply	T12Z1 Monopitch Girder	6.75 /1 2	5-10-08	4-06-12	2 x 4 2 x 6		1-03-01 4-06-12	60.07 38.00		
	1	T13 Roof Special	10 /12	9-10-00	6-02-04	2 x 4	0-06	2-01-01 2-01-01	52.76 35.17		
	2	T14 Common	10 /12	9-08-08	6-02-04	2 x 4		2-01-01 2-02-05	84.44 53.33		
	1	T18 Hip	10 /12	11-10-00	2-02-04	2 x 4	1-03-08 1-03-08	11-03 11-03	44.91 31.00		
	1	T19 Hip	10 /12	11-10-00	3-02-12	2 x 4	1-03-08 1-03-08	11-03 11-03	47.32 31.33		
	1	G17 GABLE	5 /12	11-10-00	3-03-13	2 x 4	1-03-08 1-03-08	4-01 4-01	35.66 22.17		
	2	PB1 Piggyback	10 /12	13-02-05	1-01-08	2 x 4			77.23 49.00		
	2	PB2 Piggyback	10 /12	13-02-05	2-03-00	2 x 4			81.18 51.33		
	2	PB3 Piggyback	10 /12	13-02-05	3-04-08	2 x 4			80.25 52.00		
	2	PB4 Piggyback	10 /12	13-02-05	4-06-00	2 x 4			83.97 54.33		
	4	PB5 Piggyback	10 /12	13-02-05	5-06-00	2 x 4			171.55 106.00		
	10	J1 Jack-Open	6.75 /1 2	5-10-08	4-06-12	2 x 4	1-03-08	1-03-01 4-06-11	171.67 106.67		

DELIVERY SHIPLIST				
 TAMARACK ROOF TRUSSES INC. ALPA LUMBER GROUP	Lumber Yard:	TAMARACK LUMBER	Job Track:	52430
	Builder:	ROYAL PINE HOMES	PlanLog:	205018
	Project:	VALES OF CASTLEMORE NORTH	Layout ID:	420297
	Location:	BRAMPTON	Ref #	
	Model:	MODEL 40-5	Page:	3 of 3
	Lot #:		Date:	08-10-2021
	Elevation:	A OPT. 4 & 5 BEDROOM	Designer:	
			Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	8	J1S Jack-Open	6.75 /1 2	5-10-08	4-06-12	2 x 4	1-03-08	1-03-01 3-06-11	171.45 117.33		
	10	J2 Jack-Open	5 /12	3-02-00	2-06-04	2 x 4	1-03-08	4-01 1-07-14	112.37 66.67		

TOTAL # TRUSS= 81

TOTAL BFT OF ALL TRUSSES= 3373.83

BFT.

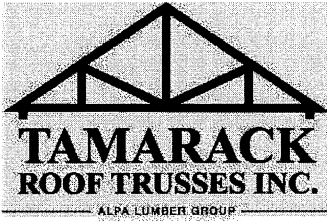
TOTAL WEIGHT OF ALL TRSSES 5322.09 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
7	Hardware	LUS24	
3	Hardware	HGUS26-2	

TOTAL NUMBER OF ITEMS= 10

DELIVERY SHIPLIST

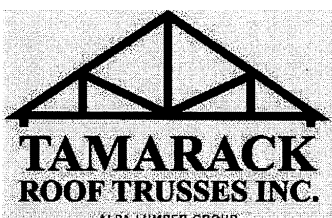


Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: VALES OF CASTLEMORE NORTH
 Location: BRAMPTON
 Model: MODEL 40-5
 Lot #:
 Elevation: B STD. 4 & 5 BEDROOM

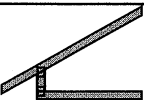
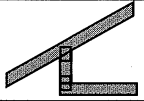
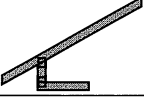
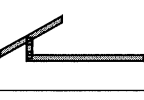
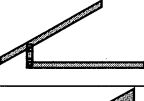
Job Track: 52430
 PlanLog: 205018
 Layout ID: 420298
 Ref #
 Page: 1 of 2
 Date: 08-04-2021
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T30 Hip Girder	6.75 /1 2	31-00-00	4-06-12	2 x 4 2 x 6	1-03-08 1-03-08	1-03-01 1-03-01	320.2 193.33		
	1 2-ply	T30Z Hip Girder	6.75 /1 2	31-00-00	4-06-12	2 x 4 2 x 6	1-03-08 1-03-08	1-03-01 1-03-01	320.2 193.33		
	2	T31 Hip	6.75 /1 2	31-00-00	5-08-04	2 x 4	1-03-08 1-03-08	1-03-01 1-03-01	242.88 152.00		
	2	T32 Hip	6.75 /1 2	31-00-00	6-09-12	2 x 4	1-03-08 1-03-08	1-03-01 1-03-01	262.3 165.00		
	2	T33 Hip	6.75 /1 2	31-00-00	7-11-04	2 x 4	1-03-08 1-03-08	1-03-01 1-03-01	277.51 178.67		
	2	T34 Hip	6.75 /1 2	31-00-00	9-00-12	2 x 4	1-03-08 1-03-08	1-03-01 1-03-01	277.97 175.67		
	12	T35 Common	6.75 /1 2	31-00-00	9-11-11	2 x 4	1-03-08 1-03-08	1-03-01 1-03-01	1618.42 1000.00		
	1	T36 Hip Girder	6.75 /1 2	12-03-00	3-05-04	2 x 4 2 x 6	1-03-08 1-03-08	1-03-01 1-03-01	56.68 37.50		
	1	T37 Hip	6.75 /1 2	12-03-00	4-06-12	2 x 4	1-03-08 1-03-08	1-03-01 1-03-01	48.14 30.50		
	1 2-ply	T38 Flat Girder	0 /12	5-10-08	1-06-00	2 x 4 2 x 6		1-06-00 1-06-00	47.73 29.67		
	1	T39 Hip Girder	6.75 /1 2	12-08-00	3-05-04	2 x 4 2 x 6	1-03-08 1-03-08	1-03-01 1-03-01	58.19 38.00		
	1	T40 Hip	6.75 /1 2	12-08-00	4-06-12	2 x 4	1-03-08 1-03-08	1-03-01 1-03-01	55.35 34.67		
	1 2-ply	T41 Monopitch Girder	6.75 /1 2	5-10-08	4-06-12	2 x 4 2 x 6		1-03-01 4-06-11	60.07 38.00		
	14	J1 Jack-Open	6.75 /1 2	5-10-08	4-06-12	2 x 4	1-03-08	1-03-01 4-06-11	240.34 149.33		

DELIVERY SHIPLIST				
 TAMARACK ROOF TRUSSES INC. ALFA LUMBER GROUP	Lumber Yard:	TAMARACK LUMBER	Job Track:	52430
	Builder:	ROYAL PINE HOMES	PlanLog:	205018
	Project:	VALES OF CASTLEMORE NORTH	Layout ID:	420298
	Location:	BRAMPTON	Ref #	
	Model:	MODEL 40-5	Page:	2 of 2
	Lot #:		Date:	08-04-2021
	Elevation:	B STD. 4 & 5 BEDROOM	Designer:	
		Sales Rep:	Rick DiCiano	

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	8	J31 Jack-Open	6.75 /1 2	3-10-08	3-05-04	2 x 4	1-03-08	1-03-01 3-05-04	98.16 64.00		
	2	C30 Jack-Open	6.75 /1 2	1-09-07	2-03-02	2 x 4	1-03-08 1-01	1-03-01 2-03-02	14.51 9.33		
	2	C31 Jack-Open	6.75 /1 2	1-10-08	3-04-10	2 x 4	1-03-08 1-10-15	1-03-01 2-03-12	19.75 12.00		
	2	C32 Jack-Open	6.75 /1 2	1-09-07	2-03-02	2 x 4	1-03-08 4-01-01	1-03-01 2-03-02	23.64 14.67		
	2	C33 Jack-Open	6.75 /1 2	3-09-07	3-04-10	2 x 4	1-03-08 2-01-01	1-03-01 3-04-10	28.87 17.33		
	4	C34 Jack-Open	6.75 /1 2	1-09-07	2-03-02	2 x 4	1-03-08 1-01	1-03-01 2-03-02	29.03 18.67		
	4	C35 Jack-Open	6.75 /1 2	1-09-07	2-03-02	2 x 4	1-03-08 2-01-01	1-03-01 2-03-02	38.15 24.00		

TOTAL # TRUSS= 70

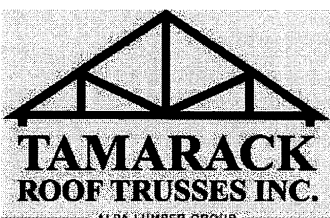
TOTAL BFT OF ALL TRUSSES= 2575.67 BFT.

TOTAL WEIGHT OF ALL TRSSES 4138.07 LBS

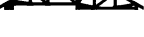
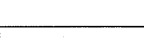
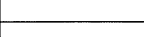
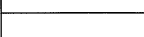
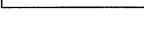
HARDWARE

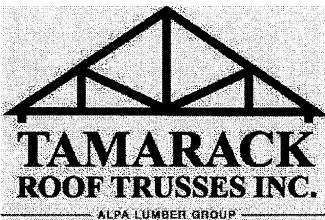
QTY	TYPE	MODEL	LENGTH
1	Hardware	LUS24	
1	Hardware	LJS26DS	
2	Hardware	HGUS26-2	

TOTAL NUMBER OF ITEMS= 4

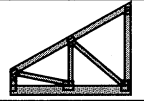
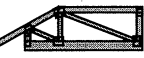
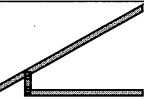
DELIVERY SHIPLIST				
 TAMARACK ROOF TRUSSES INC. ALPHA LUMBER GROUP	Lumber Yard:	TAMARACK LUMBER	Job Track:	52430
	Builder:	ROYAL PINE HOMES	PlanLog:	205018
	Project:	VALES OF CASTLEMORE NORTH	Layout ID:	420299
	Location:	BRAMPTON	Ref #	
	Model:	MODEL 40-5	Page:	1 of 3
	Lot #:		Date:	08-04-2021
	Elevation:	B OPT. 4 & 5 BEDROOM	Designer:	
		Sales Rep:	Rick DiCiano	

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T30S Hip Girder	6.75 /1 2	31-00-00	4-06-12	2 x 4 2 x 6	1-03-08 1-03-08	1-03-01 1-03-01	331.92 208.00		
	1 2-ply	T30Z Hip Girder	6.75 /1 2	31-00-00	4-06-12	2 x 4 2 x 6	1-03-08 1-03-08	1-03-01 1-03-01	320.2 193.33		
	1	T31 Hip	6.75 /1 2	31-00-00	5-08-04	2 x 4	1-03-08 1-03-08	1-03-01 1-03-01	121.44 76.00		
	1	T31S Hip	6.75 /1 2	31-00-00	5-08-04	2 x 4	1-03-08 1-03-08	1-03-01 1-03-01	135.9 86.33		
	1	T32 Hip	6.75 /1 2	31-00-00	6-09-12	2 x 4	1-03-08 1-03-08	1-03-01 1-03-01	131.15 82.50		
	1	T32S Hip	6.75 /1 2	31-00-00	6-09-12	2 x 4	1-03-08 1-03-08	1-03-01 1-03-01	138.1 86.67		
	1	T33 Hip	6.75 /1 2	31-00-00	7-11-04	2 x 4	1-03-08 1-03-08	1-03-01 1-03-01	138.76 89.33		
	1	T33S Hip	6.75 /1 2	31-00-00	7-11-04	2 x 4	1-03-08 1-03-08	1-03-01 1-03-01	143.72 92.50		
	2	T34 Hip	6.75 /1 2	31-00-00	9-00-12	2 x 4	1-03-08 1-03-08	1-03-01 1-03-01	277.97 175.67		
	12	T35 Common	6.75 /1 2	31-00-00	9-11-11	2 x 4	1-03-08 1-03-08	1-03-01 1-03-01	1613 1000.00		
	1	T36 Hip Girder	6.75 /1 2	12-03-00	3-05-04	2 x 4 2 x 6	1-03-08	1-03-01 1-03-01	54.99 36.17		
	1	T37 Hip	6.75 /1 2	12-03-00	4-06-12	2 x 4	1-03-08	1-03-01 1-03-01	49.45 31.33		
	1 2-ply	T38 Flat Girder	0 /12	5-10-08	1-06-00	2 x 4 2 x 6		1-06-00 1-06-00	47.73 29.67		
	1	T39 Hip Girder	6.75 /1 2	12-08-00	3-05-04	2 x 4 2 x 6	1-03-08 1-03-08	1-03-01 1-03-01	58.19 38.00		

DELIVERY SHIPLIST				
 TAMARACK ROOF TRUSSES INC. ALPHA LUMBER GROUP	Lumber Yard:	TAMARACK LUMBER	Job Track:	52430
	Builder:	ROYAL PINE HOMES	PlanLog:	205018
	Project:	VALES OF CASTLEMORE NORTH	Layout ID:	420299
	Location:	BRAMPTON	Ref #	
	Model:	MODEL 40-5	Page:	2 of 3
	Lot #:		Date:	08-04-2021
	Elevation:	B OPT. 4 & 5 BEDROOM	Designer:	
			Sales Rep:	Rick DiCiano

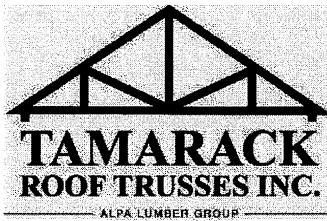
Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T40 Hip	6.75 /1 2	12-08-00	4-06-12	2 x 4	1-03-08 1-03-08	1-03-01 1-03-01	55.35 34.67		
	1 2-ply	T41 Monopitch Girder	6.75 /1 2	5-10-08	4-06-12	2 x 4 2 x 6		1-03-01 4-06-11	60.07 38.00		
	1	T42 Half Hip Girder	6.75 /1 2	5-10-08	2-02-05	2 x 4 2 x 6	1-03-08	1-03-01 2-02-05	27.39 18.00		
	8	J1 Jack-Open	6.75 /1 2	5-10-08	4-06-12	2 x 4	1-03-08	1-03-01 4-06-11	137.33 85.33		
	7	J1S Jack-Open	6.75 /1 2	5-10-08	4-06-12	2 x 4	1-03-08	1-03-01 3-06-12	156.86 107.33		
	8	J31 Jack-Open	6.75 /1 2	3-10-08	3-05-04	2 x 4	1-03-08	1-03-01 3-05-04	98.16 64.00		
	2	J32 Jack-Open	6.75 /1 2	1-08-00	2-02-05	2 x 4	1-03-08	1-03-01 2-02-05	13.72 9.33		
	1	C30 Jack-Open	6.75 /1 2	1-09-07	2-03-02	2 x 4	1-03-08 1-01	1-03-01 2-03-02	7.26 4.67		
	1	C31 Jack-Open	6.75 /1 2	1-10-08	3-04-10	2 x 4	1-03-08 1-10-15	1-03-01 2-03-12	9.87 6.00		
	1	C32 Jack-Open	6.75 /1 2	1-09-07	2-03-02	2 x 4	1-03-08 4-01-01	1-03-01 2-03-02	11.82 7.33		
	1	C33 Jack-Open	6.75 /1 2	3-09-07	3-04-10	2 x 4	1-03-08 2-01-01	1-03-01 3-04-10	14.43 8.67		
	4	C34 Jack-Open	6.75 /1 2	1-09-07	2-03-02	2 x 4	1-03-08 1-01	1-03-01 2-03-02	29.03 18.67		
	4	C35 Jack-Open	6.75 /1 2	1-09-07	2-03-02	2 x 4	1-03-08 2-01-01	1-03-01 2-03-02	38.15 24.00		

TOTAL # TRUSS= 70

TOTAL BFT OF ALL TRUSSES= 2651.5

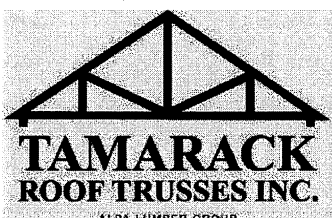
BFT. TOTAL WEIGHT OF ALL TRSSES 4221.95 LBS

DELIVERY SHIPLIST		
	Lumber Yard:	TAMARACK LUMBER
	Builder:	ROYAL PINE HOMES
	Project:	VALES OF CASTLEMORE NORTH
	Location:	BRAMPTON
	Model:	MODEL 40-5
	Lot #:	
	Elevation:	B OPT. 4 & 5 BEDROOM
	Job Track:	52430
	PlanLog:	205018
	Layout ID:	420299
	Ref #	
	Page:	3 of 3
	Date:	08-04-2021
	Designer:	
	Sales Rep:	Rick DiCiano

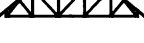
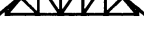
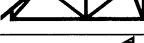
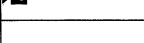
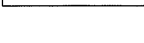
HARDWARE

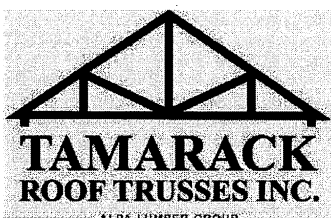
QTY	TYPE	MODEL	LENGTH
2	Hardware	LUS24	
1	Hardware	LJS26DS	
2	Hardware	HGUS26-2	

TOTAL NUMBER OF ITEMS= 5

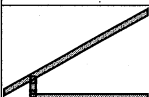
DELIVERY SHIPLIST				
 TAMARACK ROOF TRUSSES INC. ALPHA LUMBER GROUP	Lumber Yard:	TAMARACK LUMBER	Job Track:	52430
	Builder:	ROYAL PINE HOMES	PlanLog:	205018
	Project:	VALES OF CASTLEMORE NORTH	Layout ID:	420300
	Location:	BRAMPTON	Ref #	
	Model:	MODEL 40-5	Page:	1 of 2
	Lot #:		Date:	08-04-2021
	Elevation:	C STD. 4 & 5 BEDROOM	Designer:	
		Sales Rep:	Rick DiCiano	

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T50 Hip Girder	10 /12	31-00-00	4-06-12	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	324.26 198.67		
	1 2-ply	T50Z Hip Girder	10 /12	31-00-00	4-06-12	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	324.26 198.67		
	2	T51 Hip	10 /12	31-00-00	5-08-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	266.14 173.33		
	2	T52 Hip	10 /12	31-00-00	6-09-12	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	277.02 175.00		
	2	T53 Hip	10 /12	31-00-00	7-11-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	291.92 182.33		
	12	T54 Hip	10 /12	31-00-00	9-00-12	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	1980.48 1224.00		
	2	T55 Hip	10 /12	31-00-00	10-02-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	345.72 212.00		
	3	T58 Common	10 /12	19-00-00	10-06-11	2 x 4	1-03-08	1-07-11 3-07-11	261.45 165.00		
	1 2-ply	T59 Monopitch Girder	6.75 /1 2	5-10-08	4-06-12	2 x 4 2 x 6		1-03-01 4-06-12	60.07 38.00		
	1	T61 Roof Special	10 /12	19-00-00	10-06-11	2 x 4	1-03-08 1-03-08	1-07-11 2-07-11	98.88 62.83		
	1	T62 Roof Special	10 /12	19-00-00	10-06-11	2 x 4	1-03-08 1-03-08	1-07-11 2-07-11	98.88 62.83		
	2	PB50 Piggyback	10 /12	13-02-05	2-03-00	2 x 4			81.18 51.33		
	2	PB51 Piggyback	10 /12	13-02-05	3-04-08	2 x 4			80.25 52.00		
	2	PB52 Piggyback	10 /12	13-02-05	4-06-00	2 x 4			83.97 54.33		

DELIVERY SHIPLIST				
 TAMARACK ROOF TRUSSES INC. ALPHA LUMBER GROUP	Lumber Yard:	TAMARACK LUMBER	Job Track:	52430
	Builder:	ROYAL PINE HOMES	PlanLog:	205018
	Project:	VALES OF CASTLEMORE NORTH	Layout ID:	420300
	Location:	BRAMPTON	Ref #	
	Model:	MODEL 40-5	Page:	2 of 2
	Lot #:		Date:	08-04-2021
	Elevation:	C STD. 4 & 5 BEDROOM	Designer:	
		Sales Rep:	Rick DiCiano	

Roof Trusses

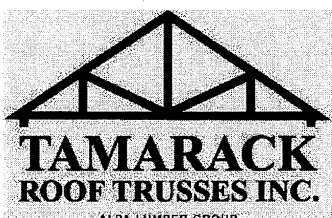
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	4	PB53 Piggyback	10 /12	13-02-05	5-06-00	2 x 4			171.55 106.00		
	20	J1 Jack-Open	6.75 /1 2	5-10-08	4-06-12	2 x 4	1-03-08	1-03-01 4-06-12	343.34 213.33		
	6	J51 Jack-Open	4.5 /12	3-09-00	2-06-03	2 x 4	1-03-08	4-00 1-08-14	75.04 48.00		
	4	J52 Jack-Open	4.5 /12	2-03-00	1-11-07	2 x 4	1-03-08	4-00 1-02-02	35.87 24.00		

TOTAL # TRUSS= 71 TOTAL BFT OF ALL TRUSSES= 3241.65 BFT. TOTAL WEIGHT OF ALL TRSSES 5200.27 LBS

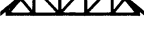
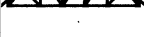
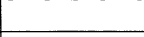
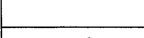
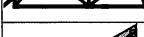
HARDWARE

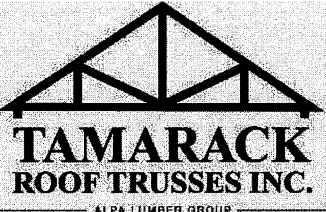
QTY	TYPE	MODEL	LENGTH
3	Hardware	LJS26DS	
1	Hardware	HGUS26-2	

TOTAL NUMBER OF ITEMS= 4

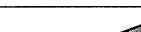
DELIVERY SHIPLIST				
 TAMARACK ROOF TRUSSES INC. ALPHA LUMBER GROUP	Lumber Yard:	TAMARACK LUMBER	Job Track:	52430
	Builder:	ROYAL PINE HOMES	PlanLog:	205018
	Project:	VALES OF CASTLEMORE NORTH	Layout ID:	420301
	Location:	BRAMPTON	Ref #	
	Model:	MODEL 40-5	Page:	1 of 2
	Lot #:		Date:	08-04-2021
	Elevation:	C OPT. 4 & 5 BEDROOM	Designer:	
		Sales Rep:	Rick DiCiano	

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T50S Hip Girder	10 /12	31-00-00	4-06-12	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	351.55 222.00		
	1 2-ply	T50Z Hip Girder	10 /12	31-00-00	4-06-12	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	324.27 198.67		
	1	T51 Hip	10 /12	31-00-00	5-08-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	133.07 86.67		
	1	T51S Hip	10 /12	31-00-00	5-08-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	140.5 92.00		
	1	T52 Hip	10 /12	31-00-00	6-09-12	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	138.51 87.50		
	1	T52S Hip	10 /12	31-00-00	6-09-12	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	151.49 97.33		
	1	T53 Hip	10 /12	31-00-00	7-11-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	145.96 91.17		
	1	T53S Hip	10 /12	31-00-00	7-11-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	153.18 96.67		
	12	T54 Hip	10 /12	31-00-00	9-00-12	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	1980.48 1224.00		
	2	T55 Hip	10 /12	31-00-00	10-02-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	345.72 212.00		
	3	T58 Common	10 /12	19-00-00	10-06-11	2 x 4	1-03-08	1-07-11 3-07-11	261.45 165.00		
	1 2-ply	T59 Monopitch Girder	6.75 /1 2	5-10-08	4-06-12	2 x 4 2 x 6		1-03-01 4-06-12	60.07 38.00		
	1	T61 Roof Special	10 /12	19-00-00	10-06-11	2 x 4	1-03-08 1-03-08	1-07-11 2-07-11	99.23 62.83		
	1	T62 Roof Special	10 /12	12-03-00	6-08-15	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	63.4 42.00		

 TAMARACK ROOF TRUSSES INC. ALPHA LUMBER GROUP	DELIVERY SHIPLIST	
	Lumber Yard: TAMARACK LUMBER Builder: ROYAL PINE HOMES Project: VALES OF CASTLEMORE NORTH Location: BRAMPTON Model: MODEL 40-5 Lot #: Elevation: C OPT. 4 & 5 BEDROOM	Job Track: 52430 PlanLog: 205018 Layout ID: 420301 Ref # Page: 2 of 2 Date: 08-04-2021 Designer: Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	PB50 Piggyback	10 /12	13-02-05	2-03-00	2 x 4			81.18 51.33		
	2	PB51 Piggyback	10 /12	13-02-05	3-04-08	2 x 4			80.25 52.00		
	2	PB52 Piggyback	10 /12	13-02-05	4-06-00	2 x 4			83.97 54.33		
	4	PB53 Piggyback	10 /12	13-02-05	5-06-00	2 x 4			171.55 106.00		
	12	J1 Jack-Open	6.75 /1 2	5-10-08	4-06-12	2 x 4	1-03-08	1-03-01 4-06-12	206 128.00		
	8	J1S Jack-Open	6.75 /1 2	5-10-08	4-06-12	2 x 4	1-03-08	1-03-01 3-06-12	179.27 122.67		
	6	J51 Jack-Open	4.5 /12	3-09-00	2-06-03	2 x 4	1-03-08	4-00 1-08-14	88.91 60.00		
	4	J52 Jack-Open	4.5 /12	2-03-00	1-11-07	2 x 4	1-03-08	4-00 1-02-02	42.56 29.33		

TOTAL # TRUSS= 71 TOTAL BFT OF ALL TRUSSES= 3319.5 BFT. TOTAL WEIGHT OF ALL TRSSES 5282.58 LBS

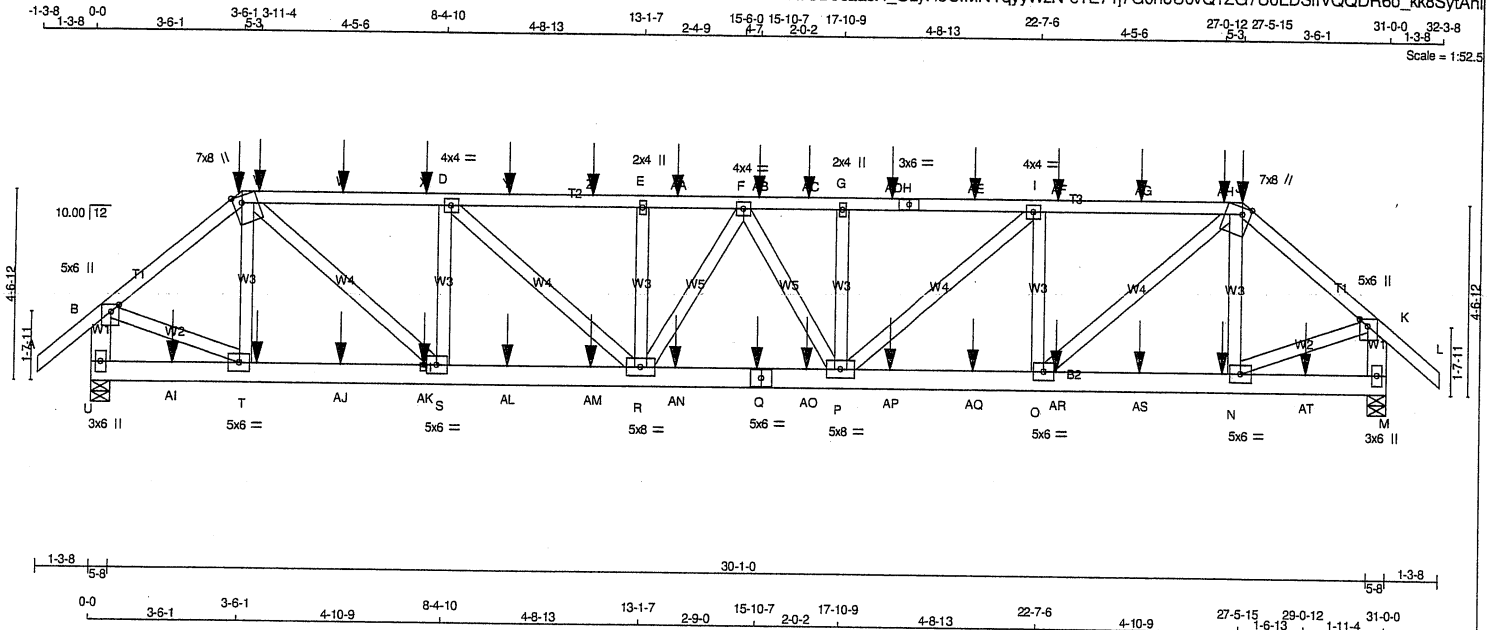
HARDWARE

QTY	TYPE	MODEL	LENGTH
3	Hardware	LJS26DS	
1	Hardware	HGUS26-2	

TOTAL NUMBER OF ITEMS= 4

JOB NAME 420295	TRUSS NAME T1	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.		Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 29 15:35:42 2021 Page 1	

ID: X73D9caach_OLyHeCIMNYqyyWzN-e1E71j7G0hoU0vQ1ZG7U0LDSfVQQDR6o_kk8SytAhI



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - H	2x4	DRY	No.2
H - J	2x4	DRY	No.2
J - L	2x4	DRY	No.2
U - B	2x6	DRY	No.2
M - K	2x6	DRY	No.2
U - Q	2x6	DRY	No.2
Q - M	2x6	DRY	No.2

ALL WEBS	2x4	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12	TOP
C - H	12	SIDE(61.0)
H - J	12	SIDE(61.0)
J - L	12	TOP
U - B	12	TOP
M - K	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
U - Q	12	SIDE(183.1)
Q - M	12	SIDE(183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
2x4	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REORD BRG
JT VERT	2849	0	0	5-8
U	2849	0	0	5-8
M	2853	0	0	5-8

UNFACTORED REACTIONS

	1ST LCASE COMBINED	MAX. / MIN. SNOW	MIN. LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	2020	1296 / 0	0 / 0	0 / 0	0 / 0	724 / 0	0 / 0
U	2020	1296 / 0	0 / 0	0 / 0	0 / 0	725 / 0	0 / 0
M	2023	1298 / 0	0 / 0	0 / 0	0 / 0	725 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.97 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED UNBRACED LENGTH	MAX. FACTORED CSI (LC)
FR-TO				FR-TO			
A-B	0 / 38	-84.9	-84.9 0.06 (1)	10.00	T-C	-568 / 0	0.06 (1)
B-C	-2836 / 0	-84.9	-84.9 0.13 (1)	5.32	C-S	0 / 2589	0.23 (1)
C-U	-4106 / 0	-84.9	-84.9 0.36 (1)	4.35	S-D	-1526 / 0	0.16 (1)
U-V	-4106 / 0	-84.9	-84.9 0.36 (1)	4.35	D-R	0 / 1172	0.10 (1)
V-X	-4106 / 0	-84.9	-84.9 0.36 (1)	4.35	R-E	-485 / 0	0.05 (1)
X-D	-4106 / 0	-84.9	-84.9 0.36 (1)	4.35	P-G	-505 / 0	0.05 (1)
D-Y	-4985 / 0	-84.9	-84.9 0.39 (1)	3.97	P-I	0 / 1180	0.10 (1)
Y-Z	-4985 / 0	-84.9	-84.9 0.39 (1)	3.97	O-I	-1529 / 0	0.16 (1)
Z-E	-4985 / 0	-84.9	-84.9 0.39 (1)	3.97	O-J	0 / 2573	0.23 (1)
E-AA	-4985 / 0	-84.9	-84.9 0.20 (1)	4.17	N-J	-568 / 0	0.06 (1)
AA-F	-4985 / 0	-84.9	-84.9 0.20 (1)	4.17	B-T	0 / 2290	0.20 (1)
F-AB	-4997 / 0	-84.9	-84.9 0.20 (1)	4.16	N-K	0 / 2293	0.20 (1)
AB-AC	-4997 / 0	-84.9	-84.9 0.20 (1)	4.16	R-F	-162 / 0	0.02 (1)
AC-G	-4997 / 0	-84.9	-84.9 0.20 (1)	4.16	F-P	-137 / 0	0.02 (1)
G-AD	-4997 / 0	-84.9	-84.9 0.39 (1)	3.97			
AD-H	-4997 / 0	-84.9	-84.9 0.39 (1)	3.97			
H-AE	-4997 / 0	-84.9	-84.9 0.39 (1)	3.97			
AE-I	-4997 / 0	-84.9	-84.9 0.39 (1)	3.97			
I-AF	-4112 / 0	-84.9	-84.9 0.35 (1)	4.35			
AF-AG	-4112 / 0	-84.9	-84.9 0.35 (1)	4.35			
AG-AH	-4112 / 0	-84.9	-84.9 0.35 (1)	4.35			
AH-J	-4112 / 0	-84.9	-84.9 0.35 (1)	4.35			
J-K	-2839 / 0	-84.9	-84.9 0.13 (1)	5.32			
K-L	0 / 38	-84.9	-84.9 0.06 (1)	10.00			
U-B	-2817 / 0	0.0	0.0 0.10 (1)	7.81			
M-K	-2820 / 0	0.0	0.0 0.10 (1)	7.81			

U-AI	0 / 0	-18.5	-18.5 0.03 (4)	10.00
AI-T	0 / 0	-18.5	-18.5 0.03 (4)	10.00
T-AJ	0 / 2152	-18.5	-18.5 0.16 (1)	10.00
AJ-AK	0 / 2152	-18.5	-18.5 0.16 (1)	10.00
AK-S	0 / 2152	-18.5	-18.5 0.16 (1)	10.00
S-AL	0 / 4106	-18.5	-18.5 0.30 (1)	10.00
AL-AM	0 / 4106	-18.5	-18.5 0.30 (1)	10.00
AM-R	0 / 4106	-18.5	-18.5 0.30 (1)	10.00
R-AN	0 / 5065	-18.5	-18.5 0.37 (1)	10.00
AN-Q	0 / 5065	-18.5	-18.5 0.37 (1)	10.00
Q-AO	0 / 5065	-18.5	-18.5 0.37 (1)	10.00
AO-P	0 / 5065	-18.5	-18.5 0.37 (1)	10.00
P-AP	0 / 4113	-18.5	-18.5 0.30 (1)	10.00
AP-AQ	0 / 4113	-18.5	-18.5 0.30 (1)	10.00
AQ-O	0 / 4113	-18.5	-18.5 0.30 (1)	10.00
O-AR	0 / 2155	-18.5	-18.5 0.16 (1)	10.00

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	36.7	PSF	

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL) = L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/999 (0.20")

CSI: TC=0.39/1.00 (D-E:1), BC=0.37/1.00 (P-R:1), WB=0.23/1.00 (J-O:1), SS=0.19/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PS)	SECTION (PL)
MT20	650	371
	778	1987
	1873	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.62 (T) (INPUT = 0.90)
JSI METAL = 0.48 (Q) (INPUT = 1.00)



Structural component only
DWG# T-2125205

CONTINUED ON PAGE 2

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTWW+m	MT20	7.0	8.0	Edge	2.25
D, F, I						
D	TMVW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
G	TMW+w	MT20	2.0	4.0		
H	TS-t	MT20	3.0	6.0		
J	TTWW+m	MT20	7.0	8.0	Edge	2.25
K	TMVW+p	MT20	5.0	6.0	2.00	2.25
M	BMV1+p	MT20	3.0	6.0		
N, O, S, T						
N	BMVW-t	MT20	5.0	6.0		
P	BMVWW-t	MT20	5.0	8.0		
Q	BS-t	MT20	5.0	6.0		
R	BMVWW-t	MT20	5.0	8.0		
U	BMV1+p	MT20	3.0	6.0		
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.						
NOTES- (1)						
1) Lateral braces to be a minimum of 2X4 SPF #2.						

LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MEMB.	MAX. FACTORED (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX	MAX. FACTORED (LBS)	MEMB.	MAX. FACTORED (LBS)	CS1 (LC)	MAX
FR-TO		FROM	TO			FR-TO			
AR-AS	0 / 2155	-18.5	-18.5	0.16 (1)	10.00				
AS-N	0 / 2155	-18.5	-18.5	0.16 (1)	10.00				
N-AT	0 / 0	-18.5	-18.5	0.03 (4)	10.00				
AT-M	0 / 0	-18.5	-18.5	0.03 (4)	10.00				
SPECIFIED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-6-1	-35	-35	---	TOP	VERT	DEAD	---	C1
C	3-6-1	-133	-133	---	TOP	VERT	SNOW	---	C1
J	27-5-15	-35	-35	---	TOP	VERT	DEAD	---	C1
J	27-5-15	-133	-133	---	TOP	VERT	SNOW	---	C1
N	27-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
Q	15-10-7	-21	-21	---	FRONT	VERT	TOTAL	---	C1
T	3-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
V	3-11-4	-87	-87	---	FRONT	VERT	TOTAL	---	C1
W	5-11-4	-71	-71	---	FRONT	VERT	TOTAL	---	C1
X	7-11-4	-71	-71	---	FRONT	VERT	TOTAL	---	C1
Y	9-11-4	-71	-71	---	FRONT	VERT	TOTAL	---	C1
Z	11-11-4	-71	-71	---	FRONT	VERT	TOTAL	---	C1
AA	13-11-4	-71	-71	---	FRONT	VERT	TOTAL	---	C1
AB	15-10-7	-71	-71	---	FRONT	VERT	TOTAL	---	C1
AC	17-0-12	-71	-71	---	FRONT	VERT	TOTAL	---	C1
AD	19-0-12	-71	-71	---	FRONT	VERT	TOTAL	---	C1
AE	21-0-12	-71	-71	---	FRONT	VERT	TOTAL	---	C1
AF	23-0-12	-71	-71	---	FRONT	VERT	TOTAL	---	C1
AG	25-0-12	-71	-71	---	FRONT	VERT	TOTAL	---	C1
AH	27-0-12	-87	-87	---	FRONT	VERT	TOTAL	---	C1
AI	1-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AJ	5-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AK	7-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AL	9-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AM	11-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AN	13-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AO	17-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AP	19-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AQ	21-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AR	23-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AS	25-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AT	29-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

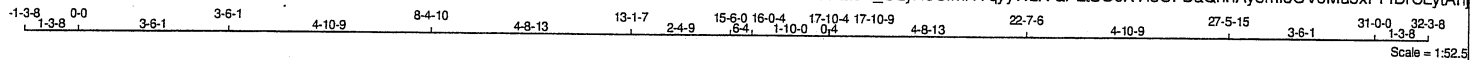
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



JOB NAME 420295	TRUSS NAME T1Z	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.420 S Jan 21 2021 MITek Industries, Inc. Thu Jul 29 15:35:44 2021 Page 1

ID:X73D9caach_OLyHeCIMNYqyWzN-aPLtSO9XYI3CFDaQhhAy5mloOV6Mu5xPFIDrCLytAh



N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
U - Q	2x6	DRY	No.2	SPF
Q - M	2x6	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C	12	SIDE (61.0)
C-H	12	SIDE (61.0)
H-J	12	TOP
J-L	12	TOP
U-B	12	TOP
M-K	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
U-Q	12	SIDE (183.1)
Q-M	12	SIDE (183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
T-C	6	SIDE (24.5)
2x4	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	UPLIFT	IN-SX
U	3855	0	0	5-8
M	3302	0	0	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
U	2725	1798 / 0	0 / 0	0 / 0	0 / 0	927 / 0
M	2335	1535 / 0	0 / 0	0 / 0	0 / 0	800 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.26 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MAX. FACTORED		FACTORED		MAX. FACTORED		FACTORED		
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM	TO		FR-TO			
A-B	0 / 38	-84.9	-84.9	0.06 (1)	10.00	T-C	-103 / 37	0.01 (1)
B-C	-3945 / 0	-84.9	-84.9	0.17 (1)	4.64	C-S	0 / 3171	0.28 (1)
C-D	-5430 / 0	-84.9	-84.9	0.31 (1)	3.94	S-D	-2006 / 0	0.22 (1)
D-E	-7185 / 0	-84.9	-84.9	0.41 (1)	3.39	D-R	0 / 2342	0.21 (1)
E-F	-7185 / 0	-84.9	-84.9	0.23 (1)	3.53	R-E	-284 / 0	0.03 (1)
F-V	-7690 / 0	-84.9	-84.9	0.25 (1)	3.40	P-G	-406 / 0	0.04 (1)
V-G	-7690 / 0	-84.9	-84.9	0.25 (1)	3.40	P-I	0 / 2800	0.25 (1)
G-H	-7690 / 0	-84.9	-84.9	0.44 (1)	3.26	O-I	-2308 / 0	0.25 (1)
H-I	-7690 / 0	-84.9	-84.9	0.44 (1)	3.26	O-J	0 / 4024	0.36 (1)
I-J	-5592 / 0	-84.9	-84.9	0.31 (1)	3.89	N-J	-804 / 0	0.09 (1)
J-K	-3340 / 0	-84.9	-84.9	0.15 (1)	4.98	B-T	0 / 3186	0.28 (1)
K-L	0 / 38	-84.9	-84.9	0.06 (1)	10.00	N-K	0 / 2697	0.24 (1)
U-B	-3814 / 0	0.0	0.0	0.14 (1)	7.23	R-F	-659 / 0	0.09 (1)
M-K	-3270 / 0	0.0	0.0	0.12 (1)	7.68	F-P	0 / 362	0.03 (1)
U-W	0 / 0	-18.5	-18.5	0.02 (4)	10.00			
W-T	0 / 0	-18.5	-18.5	0.02 (4)	10.00			
T-S	0 / 3017	-18.5	-18.5	0.22 (1)	10.00			
S-R	0 / 5430	-18.5	-18.5	0.38 (1)	10.00			
R-X	0 / 7511	-18.5	-18.5	0.66 (1)	10.00			
X-Q	0 / 7511	-18.5	-18.5	0.66 (1)	10.00			
Q-P	0 / 7511	-18.5	-18.5	0.66 (1)	10.00			
P-Y	0 / 5592	-18.5	-18.5	0.60 (1)	10.00			
Y-O	0 / 5592	-18.5	-18.5	0.60 (1)	10.00			
O-N	0 / 2530	-18.5	-18.5	0.21 (1)	10.00			
N-M	0 / 0	-18.5	-18.5	0.02 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-6-1	-35	-35	---	FRONT	VERT	DEAD	C1
C	3-6-1	-133	-133	---	FRONT	VERT	SNOW	C1
G	17-10-4	-71	-71	---	BACK	VERT	TOTAL	C1
P	17-10-4	-21	-21	---	BACK	VERT	TOTAL	C1
Q	16-0-4	-21	-21	---	BACK	VERT	TOTAL	C1
T	3-10-8	-561	-561	---	BACK	VERT	TOTAL	C1
V	16-0-4	-71	-71	---	BACK	VERT	TOTAL	C1
W	1-11-4	-21	-21	---	BACK	VERT	TOTAL	C1
X	13-11-8	-561	-561	---	BACK	VERT	TOTAL	C1
Y	18-7-8	-1127	-1127	---	BACK	VERT	TOTAL	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	= 23.3	PSF
	DL	= 6.0	PSF
BOT CH.	LL	= 0.0	PSF
	DL	= 7.4	PSF
TOTAL LOAD	=	36.7	PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/999 (0.16")
ALLOWABLE DEFL.(TL) = L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/999 (0.30")

CSI: TC=0.44/1.00 (G-I), BC=0.66/1.00 (P-R), WB=0.36/1.00 (J-O), SS=0.48/1.00 (O-P:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

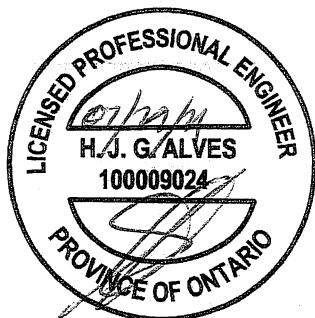
NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.87 (T) (INPUT = 0.90)
JSI METAL = 0.76 (Q) (INPUT = 1.00)



Structural component only
DWG# T-2125206

CONTINUED ON PAGE 2

JOB NAME 420295	TRUSS NAME T1Z	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 29 15:35:44 2021 Page 2			

ID:X73D9caacH OLyHeCIMNYgyyWzN-aPLtSO9XYI3CFDaQhhAy5mloOV6Mu5xPFIDrCLytAh

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTWW+m	MT20	7.0	8.0	Edge	2.25
D	TMWW-t	MT20	4.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMW+w	MT20	2.0	4.0		
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	4.0	6.0		
J	TTWW+m	MT20	7.0	8.0	Edge	2.25
K	TMVW+p	MT20	5.0	6.0	2.00	2.25
M	BMV1+p	MT20	3.0	6.0		
N, O, S, T						
N	BMWW-t	MT20	5.0	6.0		
P	BMWWW-t	MT20	5.0	8.0		
Q	BS-t	MT20	5.0	6.0		
R	BMWWW-t	MT20	5.0	8.0		
U	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

A circular professional seal for a Licensed Professional Engineer in the Province of Ontario. The seal contains the name "H. J. G. ALVES" and the license number "100009024". There is a handwritten signature across the seal.

Structural component only
DWG# T-2125206



Structural component only
DWG# T-2125206 *mm*

Tamarack Roof Truss, Burlington

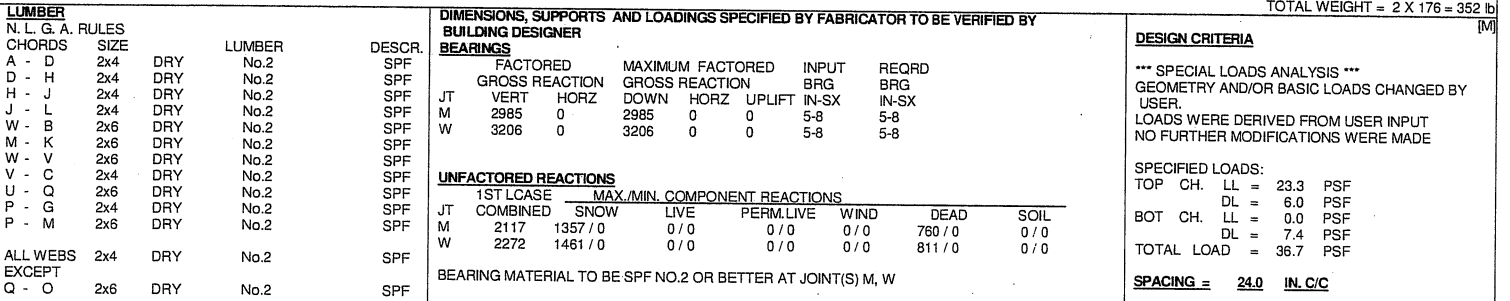
Version 8.4.20 Jan 21 2021 MiTek Industries, Inc. Thu Jul 29 15:42:19 2021 Page 1

ID:X73D9caach_OlyHeCIMNYqyyWzN-WPjCLSxIwFq98wGLK8lbc39FokyyymiY5FOFaxyAbY

-1-3-8 0-0 1-9-8 1-9-8 3-6-1 3-10-4 4-2-11 8-0-15 12-6-2 15-9-7 16-9-8 22-1-12 27-0-12 27-5-15 31-0-0 32-3-8

1-3-8 1-9-8 1-9-8 4-3 4-5-2 3-3-5 1-0-1 5-4-4 4-11-1 5-3 3-6-1 1-3-8

Scale = 1:53.2



DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS SURFACE SPACING (IN)

LOAD(PLF)

TOP CHORDS : (0.122"x3") SPIRAL NAILS

A-D 1 12 TOP

D-H 1 12 SIDE(61.0)

H-J 1 12 SIDE(61.0)

J-L 1 12 TOP

W-B 2 12 TOP

M-K 2 12 TOP

BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS

W-V 2 12 SIDE(122.0)

U-Q 2 12 SIDE(183.1)

P-M 2 12 SIDE(183.1)

V-C 1 12 TOP

G-P 1 12 TOP

WEBS : (0.122"x3") SPIRAL NAILS

2x4 1 6

2x6 2 6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.15 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MAX. FACTORED

FACTORED

VERT. LOAD

LC1

MAX

MAX.

MEMB.

MAX.

FACTORED

FORCE

MAX

CSI (LC)

FR-TO

FROM

TO

A-B 0 / 38

-84.9 -84.9 0.06 (1)

10.00

C-T 0 / 142

0.01 (1)

B-C -3953 / 0

-84.9 -84.9 0.11 (1)

4.68

T-D 0 / 154

0.02 (4)

C-D -4132 / 0

-84.9 -84.9 0.10 (1)

4.61

Q-O 0 / 4464

0.29 (1)

D-X -5845 / 0

-84.9 -84.9 0.35 (1)

3.77

Q-I 0 / 3113

0.28 (1)

X-Y -5845 / 0

-84.9 -84.9 0.35 (1)

3.77

Q-I -2441 / 0

0.26 (1)

Y-E -5845 / 0

-84.9 -84.9 0.35 (1)

3.77

Q-J 0 / 2853

0.25 (1)

E-Z -7161 / 0

-84.9 -84.9 0.42 (1)

3.36

N-J -605 / 0

0.06 (1)

Z-AA -7161 / 0

-84.9 -84.9 0.42 (1)

3.36

N-K 0 / 2413

0.21 (1)

AA-F -7161 / 0

-84.9 -84.9 0.42 (1)

3.36

W-U -105 / 0

0.01 (1)

F-AB -7282 / 0

-84.9 -84.9 0.39 (1)

3.39

B-U 0 / 3022

0.27 (1)

AB-AC -7282 / 0

-84.9 -84.9 0.39 (1)

3.39

F-Q 0 / 148

0.01 (1)

AC-G -7282 / 0

-84.9 -84.9 0.39 (1)

3.39

D-S 0 / 3246

0.29 (1)

G-AD -7225 / 0

-84.9 -84.9 0.63 (1)

3.15

R-F -665 / 0

0.05 (1)

AD-H -7225 / 0

-84.9 -84.9 0.63 (1)

3.15

S-E -1592 / 0

0.12 (1)

H-AE -7225 / 0

-84.9 -84.9 0.63 (1)

3.15

E-R 0 / 1621

0.14 (1)

AE-I -7225 / 0

-84.9 -84.9 0.63 (1)

3.15

I-AF -4500 / 0

-84.9 -84.9 0.45 (1)

4.08

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***

ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019

- PART 9 OF OBC 2012 (2019 AMENDMENT)

- CSA 086-14

- TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")

CALCULATED VERT. DEFL.(LL) = L/ 999 (0.18")

ALLOWABLE DEFL.(TL)= L/360 (1.03")

CALCULATED VERT. DEFL.(TL) = L/ 999 (0.34")

CSI: TC=0.63/1.00 (G-I=1) , BC=0.53/1.00 (Q-R=1)



Structural component only
DWG# T-2125224 *1/2*

AF-AG	-4500 / 0	-84.9	-84.9	0.45 (1)	4.08
AG-AH	-4500 / 0	-84.9	-84.9	0.45 (1)	4.08
AH-J	-4500 / 0	-84.9	-84.9	0.45 (1)	4.08
J-K	-2988 / 0	-84.9	-84.9	0.14 (1)	5.21
K-L	0 / 38	-84.9	-84.9	0.06 (1)	10.00
W-B	-3135 / 0	0.0	0.0	0.12 (1)	7.80
M-K	-2953 / 0	0.0	0.0	0.11 (1)	7.81

W-V	0 / 89	-18.5	-18.5	0.01 (1)	10.00
V-U	0 / 113	0.0	0.0	0.08 (1)	10.00
U-C	-314 / 0	0.0	0.0	0.07 (1)	7.81
U-T	0 / 3057	-18.5	-18.5	0.23 (1)	10.00
T-AI	0 / 3175	-18.5	-18.5	0.25 (1)	10.00
AI-S	0 / 3175	-18.5	-18.5	0.25 (1)	10.00
S-AJ	0 / 5845	-18.5	-18.5	0.45 (1)	10.00
AJ-AK	0 / 5845	-18.5	-18.5	0.45 (1)	10.00
AK-R	0 / 5845	-18.5	-18.5	0.45 (1)	10.00
R-AL	0 / 7161	-18.5	-18.5	0.53 (1)	10.00
AL-AM	0 / 7161	-18.5	-18.5	0.53 (1)	10.00
AM-Q	0 / 7161	-18.5	-18.5	0.53 (1)	10.00
P-Q	0 / 67	0.0	0.0	0.14 (1)	10.00
Q-G	-646 / 0	0.0	0.0	0.14 (1)	7.81
P-AN	0 / 182	-18.5	-18.5	0.06 (4)	10.00
AN-AO	0 / 182	-18.5	-18.5	0.06 (4)	10.00
AO-O	0 / 182	-18.5	-18.5	0.06 (4)	10.00
O-AP	0 / 2267	-18.5	-18.5	0.17 (1)	10.00

WB=0.29/1.00 (O-Q:1), SSL=0.19/1.00 (H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN THE
TRUSS MANUFACTURING PLANT.

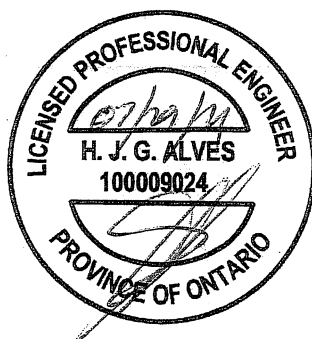
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2



PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	TTWW+m	MT20	7.0	8.0	Edge 2.25	
E	TMVW-t	MT20	4.0	4.0		
F	TMVW-t	MT20	4.0	4.0		
G	TMV+p	MT20	3.0	4.0		
H	TS-t	MT20	3.0	6.0		
I	TMVW-t	MT20	4.0	6.0		
J	TTWW+m	MT20	7.0	8.0	Edge 2.25	
K	TMVW+p	MT20	5.0	6.0	2.00	2.25
M	BMV1+p	MT20	3.0	6.0		
N, R, S, T						
N	BMVW-t	MT20	5.0	6.0		
O	BMVW-t	MT20	8.0	12.0		
P	BMV+p	MT20	3.0	6.0		
Q	BMVW-t	MT20	8.0	12.0	4.50	4.50
U	BMVW-t	MT20	6.0	12.0	3.00	7.50
V	BMV+p	MT20	3.0	6.0		
W	BMVW-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX
FR-TO		FROM	TO			FR-TO		FROM	TO	
AP-AQ	0 / 2267	-18.5	-18.5	0.17 (1)	10.00					
AQ-N	0 / 2267	-18.5	-18.5	0.17 (1)	10.00					
N-AR	0 / 0	-18.5	-18.5	0.03 (4)	10.00					
AR-M	0 / 0	-18.5	-18.5	0.03 (4)	10.00					

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	3-6-1	-35	-35		TOP	VERT	DEAD		C1
D	3-6-1	-133	-133		TOP	VERT	SNOW		C1
E	7-10-4	-67	-67		FRONT	VERT	TOTAL		C1
G	17-0-12	-71	-71		FRONT	VERT	TOTAL		C1
J	27-5-15	-35	-35		TOP	VERT	DEAD		C1
J	27-5-15	-133	-133		TOP	VERT	SNOW		C1
N	27-0-12	-21	-21		FRONT	VERT	TOTAL		C1
Q	16-11-4	-28	-28		FRONT	VERT	TOTAL		C1
S	7-10-4	-65	-65		FRONT	VERT	TOTAL		C1
T	3-10-4	-65	-65		FRONT	VERT	TOTAL		C1
V	1-7-12	-71	-71		FRONT	VERT	TOTAL		C1
X	3-10-4	-87	-87		FRONT	VERT	TOTAL		C1
Y	5-10-4	-67	-67		FRONT	VERT	TOTAL		C1
Z	9-10-4	-67	-67		FRONT	VERT	TOTAL		C1
AA	11-10-4	-67	-67		FRONT	VERT	TOTAL		C1
AB	13-10-4	-67	-67		FRONT	VERT	TOTAL		C1
AC	15-9-7	-67	-67		FRONT	VERT	TOTAL		C1
AD	19-0-12	-71	-71		FRONT	VERT	TOTAL		C1
AE	21-0-12	-71	-71		FRONT	VERT	TOTAL		C1
AF	23-0-12	-71	-71		FRONT	VERT	TOTAL		C1
AG	25-0-12	-71	-71		FRONT	VERT	TOTAL		C1
AH	27-0-12	-87	-87		FRONT	VERT	TOTAL		C1
AI	5-10-4	-65	-65		FRONT	VERT	TOTAL		C1
AJ	9-10-4	-65	-65		FRONT	VERT	TOTAL		C1
AK	11-10-4	-65	-65		FRONT	VERT	TOTAL		C1
AL	13-10-4	-65	-65		FRONT	VERT	TOTAL		C1
AM	15-9-7	-65	-65		FRONT	VERT	TOTAL		C1
AN	19-0-12	-21	-21		FRONT	VERT	TOTAL		C1
AO	21-0-12	-21	-21		FRONT	VERT	TOTAL		C1
AP	23-0-12	-21	-21		FRONT	VERT	TOTAL		C1
AQ	25-0-12	-21	-21		FRONT	VERT	TOTAL		C1
AR	29-0-12	-21	-21		FRONT	VERT	TOTAL		C1

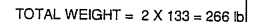
CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

JSI GRIP= 0.90 (D) (INPUT = 0.90)
JSI METAL= 0.42 (Q) (INPUT = 1.00)

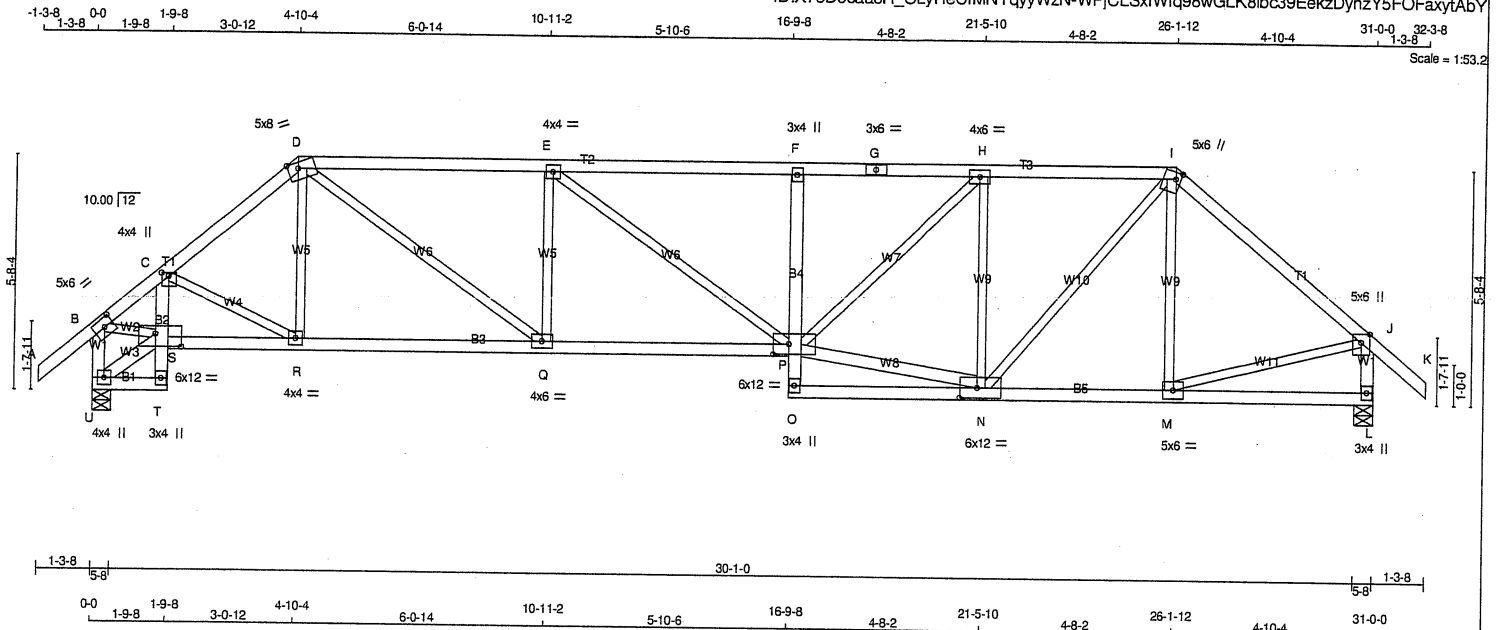


Structural component only
DWG# T-2125224 *mn*



JOB NAME 420297	TRUSS NAME T2S	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 29 15:42:19 2021 Page 1
ID: X73D9caacH_OlyHeCIMNYqyyWzN-WPjCLsXiWfq98wGLK8lbc39EekzDyhzY5FOFaxyAbY



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - K	2x4	DRY	No.2
U - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
U - T	2x4	DRY	No.2
T - C	2x4	DRY	No.2
S - P	2x4	DRY	No.2
O - F	2x4	DRY	No.2
O - L	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
P - N	2x4	DRY	No.2
U - S	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	2.75
C	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	TTWW-m	MT20	5.0	8.0	Edge	3.00
E	TMVW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMVW-t	MT20	4.0	6.0		
I	TTWW+m	MT20	5.0	6.0	2.00	1.50
J	TMVW+p	MT20	5.0	6.0	Edge	
L	BMV1+p	MT20	3.0	4.0		
M	BMVW-t	MT20	5.0	6.0		
N	BMVW-t	MT20	6.0	12.0	3.00	5.00
O	BMV+p	MT20	3.0	4.0		
P	BVMWVW-I	MT20	6.0	12.0	3.00	4.50
Q	BMVW-t	MT20	4.0	6.0		
R	BMVW-t	MT20	4.0	4.0		
S	BVMWVW-I	MT20	6.0	12.0	3.75	7.25
T	BMV+p	MT20	3.0	4.0		
U	BMVW1+p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
L	1717	0	1717	0
U	1722	0	1722	0

UNFACTORED REACTIONS

	1ST LOASE	MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1217	785 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0
U	1220	788 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, U

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.48 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 38	-84.9 -84.9	0.12 (1)	10.00	C-R	-119 / 0	0.03 (1)
B-C	-2053 / 0	-84.9 -84.9	0.16 (1)	4.59	R-D	0 / 95	0.03 (4)
C-D	-1958 / 0	-84.9 -84.9	0.18 (1)	4.66	D-Q	0 / 1428	0.32 (1)
D-E	-2555 / 0	-84.9 -84.9	0.68 (1)	3.58	Q-E	-713 / 0	0.23 (1)
E-F	-2830 / 0	-84.9 -84.9	0.70 (1)	3.46	E-P	0 / 216	0.05 (1)
F-G	-2814 / 0	-84.9 -84.9	0.38 (1)	3.82	P-N	0 / 1975	0.32 (1)
G-H	-2814 / 0	-84.9 -84.9	0.38 (1)	3.82	P-H	0 / 1136	0.26 (1)
H-I	-1983 / 0	-84.9 -84.9	0.31 (1)	4.49	N-H	-1212 / 0	0.60 (1)
I-J	-1607 / 0	-84.9 -84.9	0.43 (1)	4.74	N-I	0 / 1161	0.26 (1)
J-K	0 / 38	-84.9 -84.9	0.12 (1)	10.00	M-I	-226 / 0	0.11 (1)
U-B	-1672 / 0	0.0 0.0	0.18 (1)	6.42	M-J	0 / 1273	0.29 (1)
L-J	-1679 / 0	0.0 0.0	0.18 (1)	6.41	U-S	-61 / 0	0.01 (1)
U-T	0 / 50	-18.5 -18.5	0.02 (4)	10.00	B-S	0 / 1570	0.35 (1)
T-S	0 / 17	0.0 0.0	0.08 (1)	10.00			
S-C	-87 / 0	0.0 0.0	0.08 (1)	7.81			
S-R	0 / 1601	-18.5 -18.5	0.32 (1)	10.00			
R-Q	0 / 1496	-18.5 -18.5	0.31 (1)	10.00			
Q-P	0 / 2655	-18.5 -18.5	0.51 (1)	10.00			
O-P	0 / 39	0.0 0.0	0.10 (1)	10.00			
P-F	-416 / 0	0.0 0.0	0.12 (1)	7.81			
O-N	0 / 68	-18.5 -18.5	0.09 (4)	10.00			
N-M	0 / 1227	-18.5 -18.5	0.25 (1)	10.00			
M-L	0 / 0	-18.5 -18.5	0.10 (4)	10.00			

TOTAL WEIGHT = 141 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	23.3	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.4	PSF
TOTAL LOAD	=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/999 (0.14")
ALLOWABLE DEFL.(TL) = L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/999 (0.28")

CSI: TC=0.70/1.00 (E-F:1), BC=0.51/1.00 (P-Q:1), WB=0.60/1.00 (H-N:1), SI=0.24/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

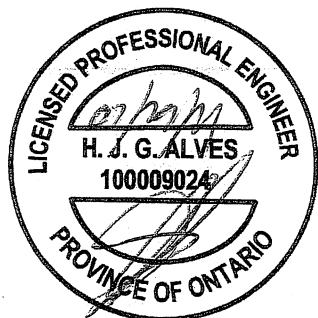
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371 1747 788 1987 1873	

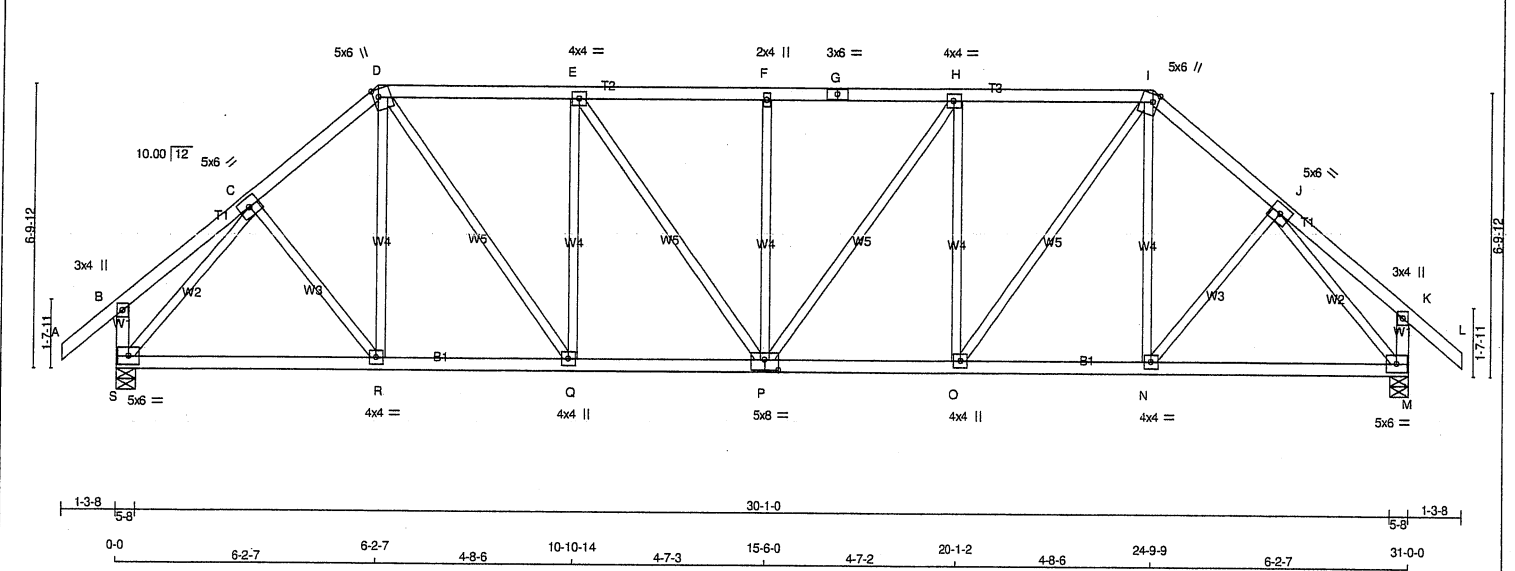
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (D) (INPUT = 0.90)
JSI METAL= 0.60 (J) (INPUT = 1.00)



Structural component only
DWG# T-2125225



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - L	2x4	DRY	No.2
S - B	2x4	DRY	No.2
M - K	2x4	DRY	No.2
S - P	2x4	DRY	No.2
P - M	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-i	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMVW-i	MT20	4.0	4.0		
F	TMV+w	MT20	2.0	4.0		
G	TS-i	MT20	3.0	6.0		
H	TMVW-i	MT20	4.0	4.0		
I	TTWW+m	MT20	5.0	6.0	2.25	1.50
J	TMVW-i	MT20	5.0	6.0		
K	TMV+p	MT20	3.0	4.0		
M	BMVW1-i	MT20	5.0	6.0		
N	BMVW-i	MT20	4.0	4.0		
O	BMVW-i	MT20	4.0	4.0		
P	BSWW-i	MT20	5.0	8.0	3.00	4.00
Q	BMVW-i	MT20	4.0	4.0		
R	BMVW-i	MT20	4.0	4.0		
S	BMVW1-i	MT20	5.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
S	1720	0	1720	0
M	1720	0	1720	0

UNFACTORED REACTIONS

1ST LCASE	MAX /MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
S	1218	786 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0
M	1218	786 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.52 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX. (LC)	MEMB.	FORCE (LBS)	FACTORED MAX. (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 38	-84.9	-84.9 0.12 (1)	10.00	C-R	0 / 127	0.03 (1)
B-C	0 / 17	-84.9	-84.9 0.11 (1)	10.00	R-D	0 / 70	0.02 (4)
C-D	-1628 / 0	-84.9	-84.9 0.15 (1)	5.05	N-I	0 / 70	0.02 (4)
D-E	-1770 / 0	-84.9	-84.9 0.32 (1)	4.68	N-J	0 / 127	0.03 (1)
E-F	-1923 / 0	-84.9	-84.9 0.33 (1)	4.52	S-C	-1852 / 0	0.72 (1)
F-G	-1923 / 0	-84.9	-84.9 0.33 (1)	4.52	J-M	-1852 / 0	0.72 (1)
G-H	-1923 / 0	-84.9	-84.9 0.33 (1)	4.52	O-I	0 / 915	0.21 (1)
H-I	-1770 / 0	-84.9	-84.9 0.32 (1)	4.68	D-Q	0 / 915	0.21 (1)
I-J	-1628 / 0	-84.9	-84.9 0.15 (1)	5.05	O-H	-658 / 0	0.51 (1)
J-K	0 / 17	-84.9	-84.9 0.11 (1)	10.00	Q-E	-658 / 0	0.51 (1)
K-L	0 / 38	-84.9	-84.9 0.12 (1)	10.00	P-H	0 / 264	0.06 (1)
S-B	-222 / 0	0.0	0.0 0.02 (1)	7.81	E-P	0 / 264	0.06 (1)
M-K	-222 / 0	0.0	0.0 0.02 (1)	7.81	P-F	-358 / 0	0.28 (1)
S-R	0 / 1152	-18.5	-18.5 0.27 (1)	10.00			
R-Q	0 / 1234	-18.5	-18.5 0.28 (1)	10.00			
Q-P	0 / 1770	-18.5	-18.5 0.33 (1)	10.00			
P-O	0 / 1770	-18.5	-18.5 0.33 (1)	10.00			
O-N	0 / 1234	-18.5	-18.5 0.28 (1)	10.00			
N-M	0 / 1152	-18.5	-18.5 0.27 (1)	10.00			

TOTAL WEIGHT = 2 X 147 = 295 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	36.7	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/999 (0.16")

CSI: TC=0.33/1.00 (E-F:1), BC=0.33/1.00 (P-Q:1), WB=0.72/1.00 (J-M:1), SSI=0.19/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

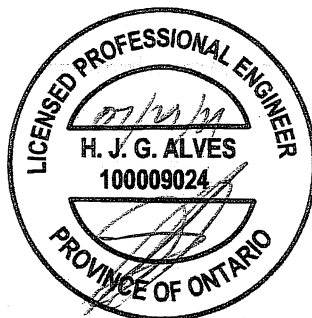
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
	MAX	MIN	MAX
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (O) (INPUT = 0.90)
JSI METAL= 0.41 (C) (INPUT = 1.00)



Structural component only
DWG# T-2125208

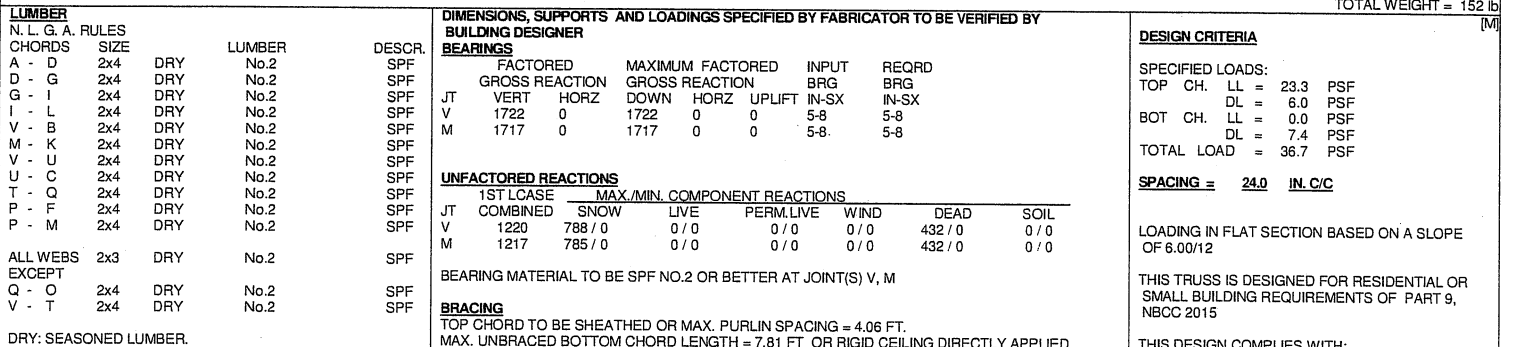
Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 29 15:42:20 2021 Page 1

ID:X73D9caacH_OLyHeCIMNyqyWzN-_bHbZoywHzY0l3rXtsGq8HiSp8K_h4vUv8o6OytAbX

Structural drawing of a roof truss. The drawing shows a series of vertical and diagonal members. Dimensions are indicated by horizontal lines with arrows: 1-3-8, 0-0, 1-9-8, 1-9-8, 4-4-15, 6-2-7, 5-4-13, 11-7-4, 16-9-8, 4-0-1, 20-9-9, 4-0-1, 24-9-9, 2-11-15, 27-9-9, 3-2-7, 31-0-0, 32-3-8. A scale bar at the bottom right indicates a scale of 1:53.2.

Scale = 1:53.2

[illegible]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	6.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN
LOAD) EQUALS 23.9 P.S.F. SPECIFIED ROOF
LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.48/1.00 (E-F:1) , BC=0.41/1.00 (Q-R:1) ,
WB=0.87/1.00 (H-O:1) , SS=0.22/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN THE
TRUSS MANUFACTURING PLANT .

NAIL VALUES					
PLATE	GRIP(DRY)		SHEAR		SECTION
	(PSI)		(PLI)		(PLI)
	MAX	MIN	MAX	MIN	MAX
MT20	650	371	1747	788	1987 1873

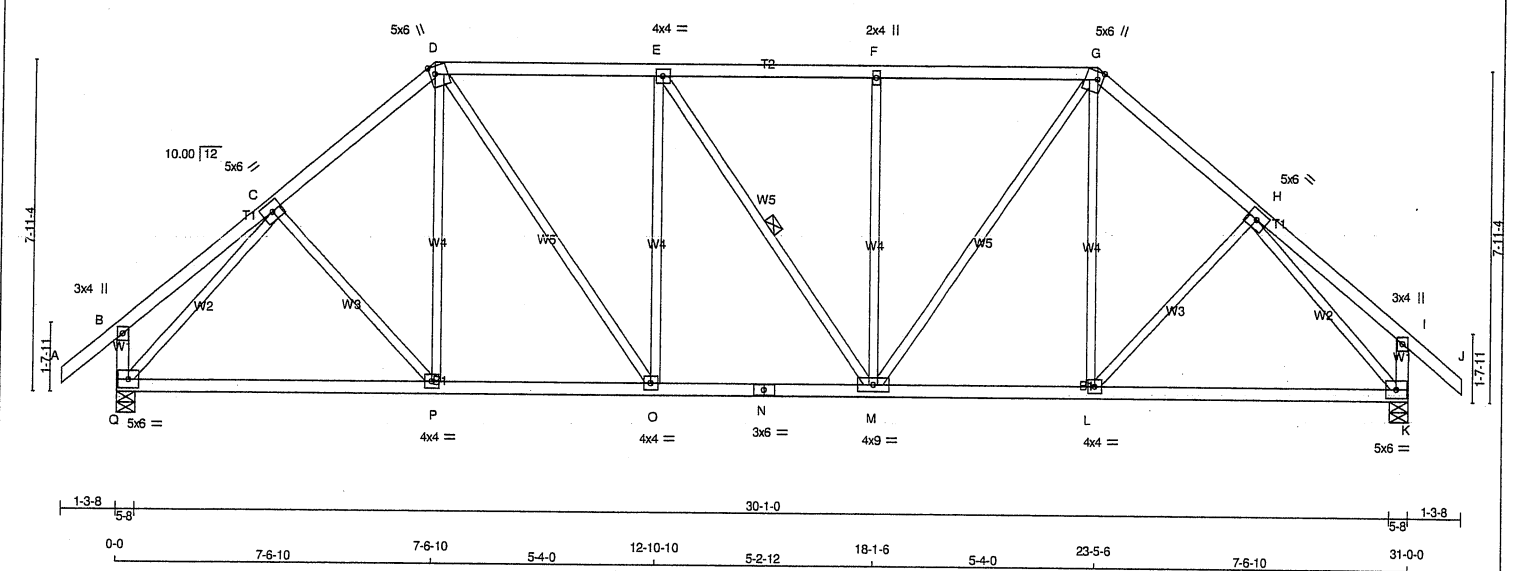
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.85 (J) (INPUT = 0.90)
 JSI METAL= 0.50 (B) (INPUT = 1.00)



Structural component only
DWG# T-2125226



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
G - J	2x4	DRY	No.2
Q - B	2x4	DRY	No.2
K - I	2x4	DRY	No.2
Q - N	2x4	DRY	No.2
N - K	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
C	TMWW-t	MT20	5.0	6.0	
D	TTWW+m	MT20	5.0	6.0	2.25 1.50
E	TMWW-t	MT20	4.0	4.0	
F	TMWW+w	MT20	2.0	4.0	
G	TTWW+m	MT20	5.0	6.0	2.25 1.50
H	TMWW-t	MT20	5.0	6.0	
I	TMV+p	MT20	3.0	4.0	
K	BMVW1-t	MT20	5.0	6.0	
L, O, P					
L	BMVW-t	MT20	4.0	4.0	
M	BMVWW-t	MT20	4.0	9.0	
N	BS-t	MT20	3.0	6.0	
Q	BMVW1-t	MT20	5.0	6.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT.	VERT	HORZ	DOWN	HORZ	UPLIFT
Q	1720	0	1720	0	0
K	1720	0	1720	0	0

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1218	786 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0
K	1218	786 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRAC CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRAC CSI (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 38	-84.9 -84.9	0.12 (1)	10.00	C-P	0 / 54	
B-C	0 / 23	-84.9 -84.9	0.18 (1)	10.00	P-D	0 / 125	
C-D	-1604 / 0	-84.9 -84.9	0.23 (1)	4.99	D-O	0 / 694	
D-E	-1612 / 0	-84.9 -84.9	0.38 (1)	4.77	O-E	-484 / 0	
E-F	-1611 / 0	-84.9 -84.9	0.38 (1)	4.78	E-M	-3 / 0	
F-G	-1611 / 0	-84.9 -84.9	0.38 (1)	4.79	M-F	-483 / 0	
G-H	-1604 / 0	-84.9 -84.9	0.23 (1)	4.99	M-G	0 / 692	
H-I	0 / 23	-84.9 -84.9	0.18 (1)	10.00	L-G	0 / 127	
I-J	0 / 38	-84.9 -84.9	0.12 (1)	10.00	L-H	0 / 54	
Q-B	-234 / 0	0.0	0.0 0.02 (1)	7.81	Q-C	-1861 / 0	
K-I	-234 / 0	0.0	0.0 0.02 (1)	7.81	H-K	-1861 / 0	
Q-P	0 / 1195	-18.5 -18.5	0.33 (4)	10.00			
P-O	0 / 1214	-18.5 -18.5	0.34 (4)	10.00			
O-N	0 / 1612	-18.5 -18.5	0.31 (1)	10.00			
N-M	0 / 1612	-18.5 -18.5	0.31 (1)	10.00			
M-L	0 / 1214	-18.5 -18.5	0.34 (4)	10.00			
L-K	0 / 1195	-18.5 -18.5	0.33 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/999 (0.14")

CSI: TC=0.38/1.00 (D-E:1), BC=0.34/1.00 (L-M:4), WB=0.98/1.00 (H-K:1), SSI=0.21/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

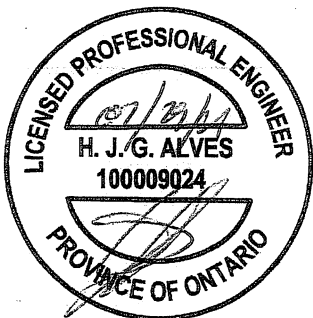
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

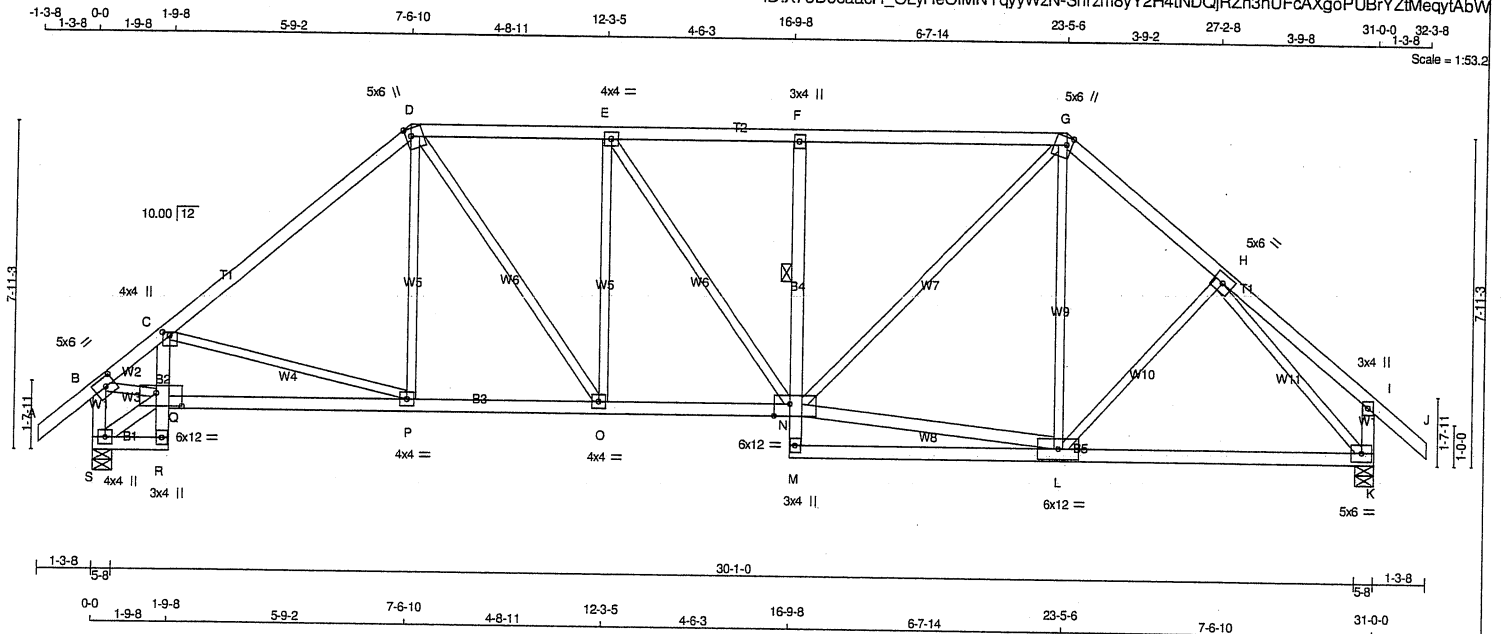
JSI GRIP= 0.80 (H) (INPUT = 0.90)
JSI METAL= 0.55 (N) (INPUT = 1.00)



Structural component only
DWG# T-2125209

JOB NAME 420297	TRUSS NAME T4S	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 29 15:42:21 2021 Page 1
ID:X73D9caacH_OLyHeCIMNYqyWzN-Snrzm8yY2H4tNDQjRZn3hUFcAXgoPUBrYzIMeqyAbW



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
G - J	2x4	DRY	No.2
S - B	2x4	DRY	No.2
K - I	2x4	DRY	No.2
S - R	2x4	DRY	No.2
R - C	2x4	DRY	No.2
Q - N	2x4	DRY	No.2
M - F	2x4	DRY	No.2
M - K	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
N - L	2x4	DRY	No.2
S - Q	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	2.75
C	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMVW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	TTWW+m	MT20	5.0	6.0	2.25	1.50
H	TMVW-t	MT20	5.0	6.0		
I	TMV+p	MT20	3.0	4.0		
K	BMVW1-t	MT20	5.0	6.0		
L	BMVW1-t	MT20	6.0	12.0		
M	BMV+p	MT20	3.0	4.0		
N	BVMW1-t	MT20	6.0	12.0	Edge	4.50
O	BMVW-t	MT20	4.0	4.0		
P	BMVW-t	MT20	4.0	4.0		
Q	BVMW1-t	MT20	6.0	12.0	3.75	7.25
R	BMV+p	MT20	3.0	4.0		
S	BMVW1+p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	UPLIFT
JT	1717	0	1717	0
K	1717	0	1717	0
S	1722	0	1722	0

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN.	COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
K	1217	785 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0
S	1220	788 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, S

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.16 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 38	-84.9 -84.9	0.12 (1)	C-P	-377 / 0	0.26 (1)	
B-C	-2151 / 0	-84.9 -84.9	0.40 (1)	P-D	0 / 175	0.05 (4)	
C-D	-1827 / 0	-84.9 -84.9	0.53 (1)	D-O	0 / 730	0.16 (1)	
D-E	-1807 / 0	-84.9 -84.9	0.29 (1)	O-E	-509 / 0	0.42 (1)	
E-F	-1895 / 0	-84.9 -84.9	0.51 (1)	E-N	0 / 152	0.03 (1)	
F-G	-1889 / 0	-84.9 -84.9	0.57 (1)	N-L	0 / 1192	0.19 (1)	
G-H	-1605 / 0	-84.9 -84.9	0.23 (1)	N-G	0 / 966	0.22 (1)	
H-I	0 / 23	-84.9 -84.9	0.19 (1)	L-G	-37 / 74	0.04 (1)	
I-J	0 / 38	-84.9 -84.9	0.12 (1)	L-H	0 / 54	0.02 (4)	
S-B	-1671 / 0	0.0 0.0	0.18 (1)	H-K	-1862 / 0	1.00 (1)	
K-I	-236 / 0	0.0 0.0	0.03 (1)	S-Q	-64 / 0	0.01 (1)	
				B-Q	0 / 1712	0.39 (1)	
S-R	0 / 52	-18.5 -18.5	0.02 (4)				
R-Q	0 / 17	0.0 0.0	0.08 (1)				
Q-C	-102 / 11	0.0 0.0	0.08 (1)				
Q-P	0 / 1743	-18.5 -18.5	0.36 (1)				
P-O	0 / 1383	-18.5 -18.5	0.30 (1)				
O-N	0 / 1807	-18.5 -18.5	0.34 (1)				
M-N	0 / 47	0.0 0.0	0.05 (1)				
N-F	-552 / 0	0.0 0.0	0.07 (1)				
M-L	0 / 34	-18.5 -18.5	0.27 (4)				
L-K	0 / 1198	-18.5 -18.5	0.37 (4)				

TOTAL WEIGHT = 154 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 36.7	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/999 (0.16")

CSI: TC=0.57/1.00 (F-G:1), BC=0.37/1.00 (K-L:4), WB=1.00/1.00 (H-K:1), SI=0.25/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

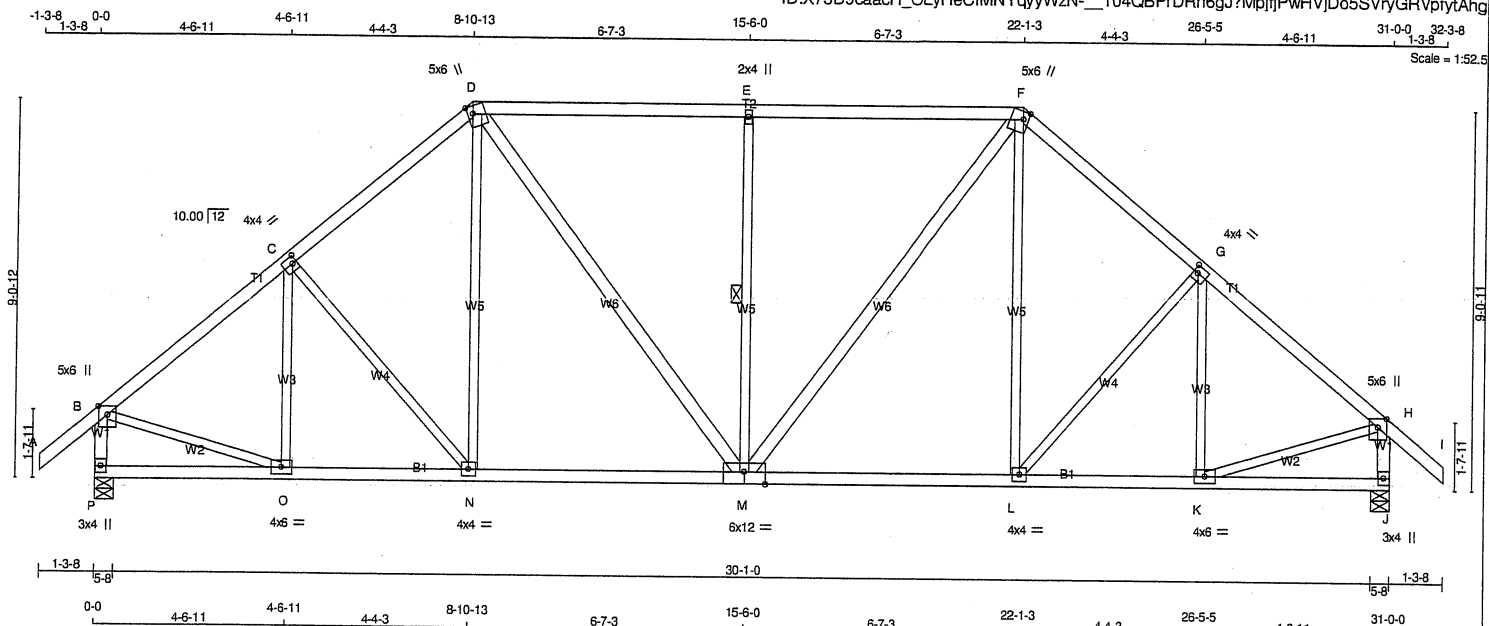
JSI GRIP= 0.86 (G) (INPUT = 0.90)
JSI METAL= 0.52 (B) (INPUT = 1.00)



Structural component only
DWG# T-2125227

JOB NAME 420295	TRUSS NAME T5	QUANTITY 14	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.420 S Jan 21 2021 MTEK Industries, Inc. Thu Jul 29 15:35:47 2021 Page 1
ID:X73D9caacH_OLyHeCIMNYqyyWzN-104QBPrDRn6gJ?MpjiPwHVjD05SVryGRVpftAng



LUMBER				N. L. G. A. RULES	
CHORDS	SIZE	LUMBER	DESCR.	SPF	
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - I	2x4	DRY	No.2	SPF	
P - B	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
P - M	2x4	DRY	No.2	SPF	
M - J	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
D - M	2x4	DRY	No.2	SPF	
M - F	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	5.0	6.0	Edge
C	TMVW-t	MT20	4.0	4.0	2.00 1.25
D	TTVW+m	MT20	5.0	6.0	2.25 1.50
E	TMVW-w	MT20	2.0	4.0	
F	TTVW+m	MT20	5.0	6.0	2.25 1.50
G	TMVW-t	MT20	4.0	4.0	2.00 1.25
H	TMVW+p	MT20	5.0	6.0	Edge
J	BMV1+p	MT20	3.0	4.0	
K	BMVW-t	MT20	4.0	6.0	
L	BMVW-t	MT20	4.0	4.0	
M	BSVWV-t	MT20	6.0	12.0	Edge 6.00
N	BMVW-t	MT20	4.0	4.0	
O	BMVW-t	MT20	4.0	6.0	
P	BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
P	1720	0	1720	0	0
J	1720	0	1720	0	0

UNFACTORED REACTIONS		1ST CASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	
P	1218	786 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0	
J	1218	786 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.76 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED	FACTORED	W E B S	MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRACED LENGTH	MEMB.	FORCE (LBS)
FR-TO					FR-TO	
A-B	0 / 38	-84.9	-84.9	0.12 (1)	O-C	-286 / 0
B-C	-1616 / 0	-84.9	-84.9	0.25 (1)	C-N	-143 / 0
C-D	-1554 / 0	-84.9	-84.9	0.25 (1)	N-D	0 / 232
D-E	-1467 / 0	-84.9	-84.9	0.52 (1)	D-M	0 / 492
E-F	-1467 / 0	-84.9	-84.9	0.52 (1)	M-E	-687 / 0
F-G	-1554 / 0	-84.9	-84.9	0.25 (1)	M-F	0 / 492
G-H	-1616 / 0	-84.9	-84.9	0.25 (1)	L-F	0 / 232
H-I	0 / 38	-84.9	-84.9	0.12 (1)	L-G	-143 / 0
P-B	-1681 / 0	0.0	0.0	0.18 (1)	K-G	-286 / 0
J-H	-1681 / 0	0.0	0.0	0.18 (1)	B-O	0 / 1314
					K-H	0 / 1314
P-O	0 / 0	-18.5	-18.5	0.08 (4)		
O-N	0 / 1263	-18.5	-18.5	0.30 (1)		
N-M	0 / 1171	-18.5	-18.5	0.28 (1)		
M-L	0 / 1171	-18.5	-18.5	0.28 (1)		
L-K	0 / 1263	-18.5	-18.5	0.30 (1)		
K-J	0 / 0	-18.5	-18.5	0.08 (4)		

TOTAL WEIGHT = 14 X 154 = 2156 lb
(M/F)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL)= L/999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL)= L/999 (0.14")

CSI: TC=0.52/1.00 (D-E:1), BC=0.30/1.00 (K-L:1), WB=0.37/1.00 (E-M:1), SS=0.27/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (H) (INPUT = 0.90)
JSI METAL= 0.60 (B) (INPUT = 1.00)



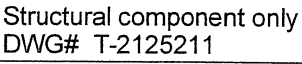
Structural component only
DWG# T-2125210



THIS DESIGN COMPLIES WITH:

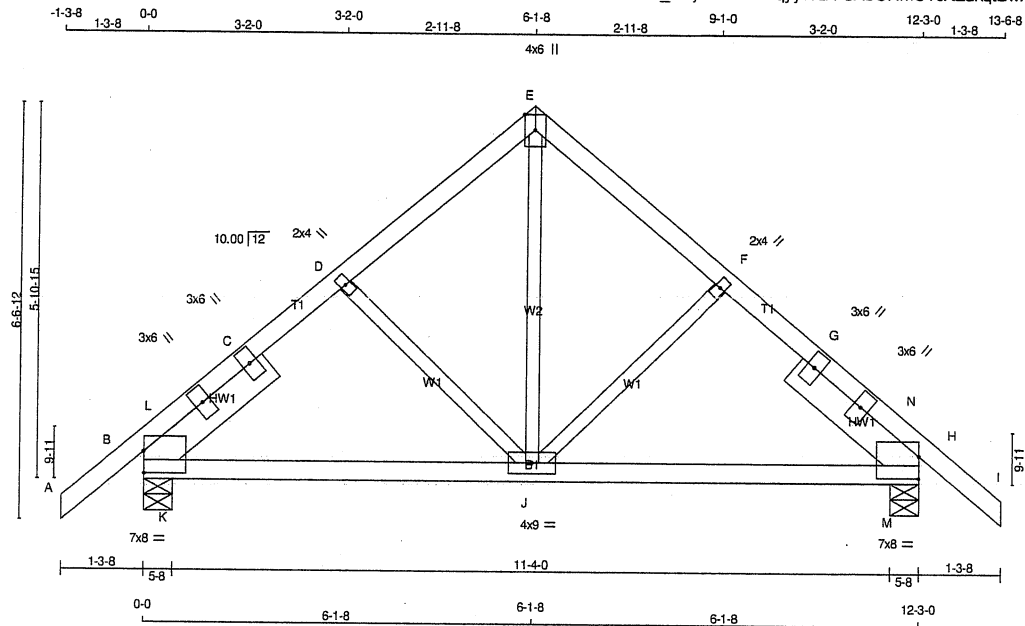
E- F	-548 / 0	-84.9	-84.9	0.10 (1)	6.25	J- G	-445 / 0	0.07 (1)
						D- M	-420 / 0	0.10 (1)

JSI GRIP= 0.51 (H) (INPUT = 0.90)
JSI METAL= 0.14 (G) (INPUT = 1.00)



JOB NAME 420295	TRUSS NAME T8	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 29 15:35:48 2021 Page 1
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TOTAL WEIGHT = 56 lb
[M/F]

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - E	2x4	DRY	No.2
E - I	2x4	DRY	No.2
B - H	2x4	DRY	No.2
REINFORCING MEMBERS			
HW1	2x6	DRY	No.2
HW2	2x6	DRY	No.2
ALL WEBS			
2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMBMR1-I	MT20	7.0	8.0	4.25
B, B, H, H					
B	RT-t	MT20	3.0	6.0	
D	TTW-w	MT20	2.0	4.0	
E	TTW-p	MT20	4.0	6.0	Edge
F	TTW-w	MT20	2.0	4.0	
H	TMBMR1-I	MT20	7.0	8.0	4.25
J	BMWWV-t	MT20	4.0	9.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQRD
GROSS REACTION		GROSS REACTION	DOWN	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
B	751	0	751	0	0
H	751	0	751	0	0

UNFACTORED REACTIONS

1ST LCASE	MAX / MIN	COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
B	531	350 / 0	0 / 0	0 / 0	0 / 0	181 / 0	0 / 0
H	531	350 / 0	0 / 0	0 / 0	0 / 0	181 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE	FACTORED		MEMB.	FORCE	FACTORED	
	(LBS)	VERT. LOAD	LC1		(LBS)	VERT. LOAD	LC1
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 26	-84.9	-84.9	J-E	0 / 374	-186 / 0	0.06 (1)
B-L	-780 / 0	-84.9	-84.9	D-J	-186 / 0	0.06 (1)	
L-C	-532 / 0	-84.9	-84.9	K-L	0 / 341	0.02 (1)	
C-D	-532 / 0	-84.9	-84.9	M-N	0 / 341	0.02 (1)	
D-E	-477 / 0	-84.9	-84.9	M-G	-244 / 0	0.02 (1)	
E-F	-477 / 0	-84.9	-84.9				
F-G	-532 / 0	-84.9	-84.9				
G-N	-532 / 0	-84.9	-84.9				
N-H	-780 / 0	-84.9	-84.9				
H-I	0 / 26	-84.9	-84.9				
B-K	0 / 353	-18.5	-18.5				
K-J	0 / 489	-18.5	-18.5				
J-M	0 / 489	-18.5	-18.5				
M-H	0 / 353	-18.5	-18.5				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.11/1.00 (A-B:1), BC=0.17/1.00 (J-K:4),
WB=0.08/1.00 (E-J:1), SS=0.13/1.00 (H-M:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

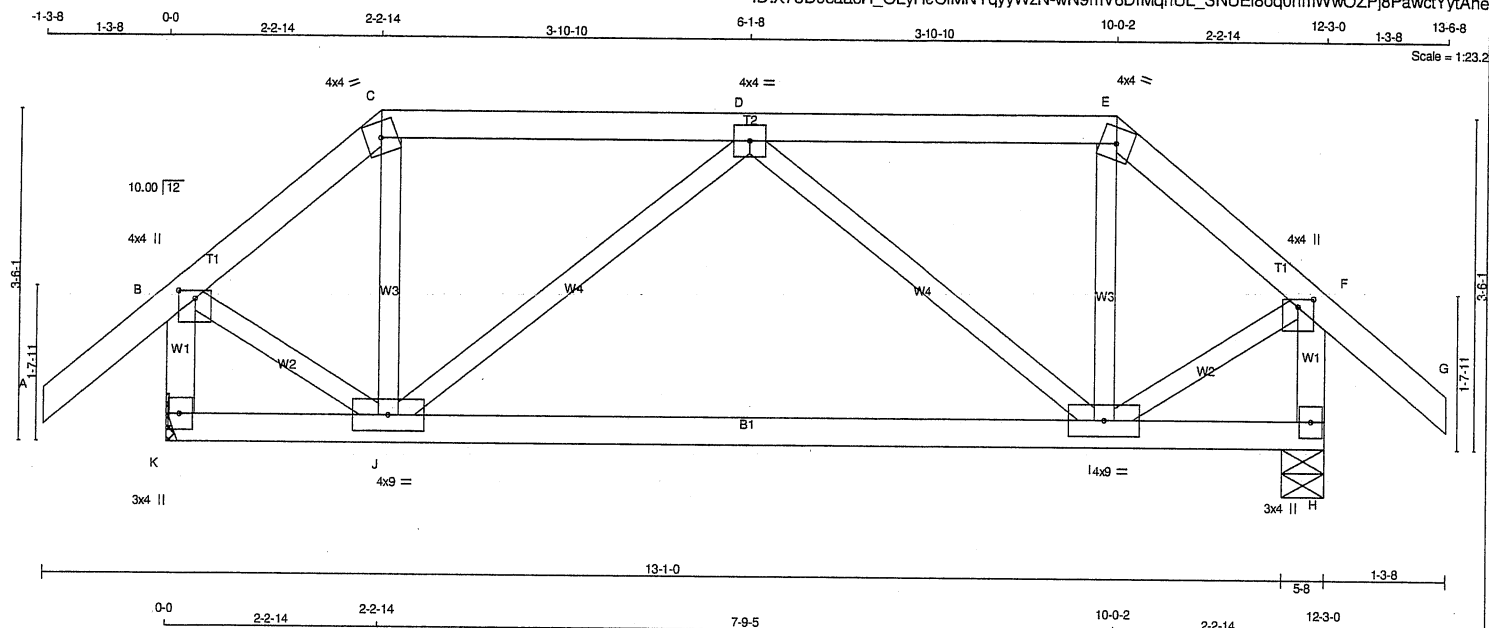
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.32 (J) (INPUT = 0.90)
JSI METAL= 0.24 (H) (INPUT = 1.00)



Structural component only
DWG# T-2125212

JOB NAME 420295	TRUSS NAME T9	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.		Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 29 15:35:49 2021 Page 1 ID:X73D9caach_OLyHeCIMNYqyyWzN-wN9mV6DfMqhUL_SNUEI8oq0hmWwOZPJ8PawctYytAhe	



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
K - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
K - H	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMVW+p	MT20	4.0	4.0 1.00 2.00
C	TTW-m	MT20	4.0	4.0
D	TMVW-t	MT20	4.0	4.0
E	TTW-m	MT20	4.0	4.0
F	TMVW+p	MT20	4.0	4.0 1.00 2.00
H	BMV1+p	MT20	3.0	4.0
I	BMVWW-t	MT20	4.0	9.0
J	BMVWW-t	MT20	4.0	9.0
K	BMV1+p	MT20	3.0	4.0

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	SNOW	DOWN	BRG	BRG
K	VERT	0	751	0	0
H	HORIZ	0	751	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K. MINIMUM BEARING LENGTH AT JOINT K = 1-8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
K	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
K	531	350 / 0	0 / 0	0 / 0	181 / 0	0 / 0
H	531	350 / 0	0 / 0	0 / 0	181 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 38	J-C	0 / 106
B-C	-511 / 0	J-D	-311 / 0
C-D	-394 / 0	D-I	-311 / 0
D-E	-394 / 0	E-F	0 / 106
E-F	-511 / 0	F-G	0 / 447
F-G	0 / 38	G-H	0 / 447
K-B	-759 / 0	H-F	0 / 447
H-F	-759 / 0		
K-J	0 / 0		
J-I	0 / 631		
I-H	0 / 0		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = U/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = U/999 (0.06")

CSI: TC=0.21/1.00 (D-E:1), BC=0.22/1.00 (I-J:4), WB=0.13/1.00 (D-J:1), SS=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

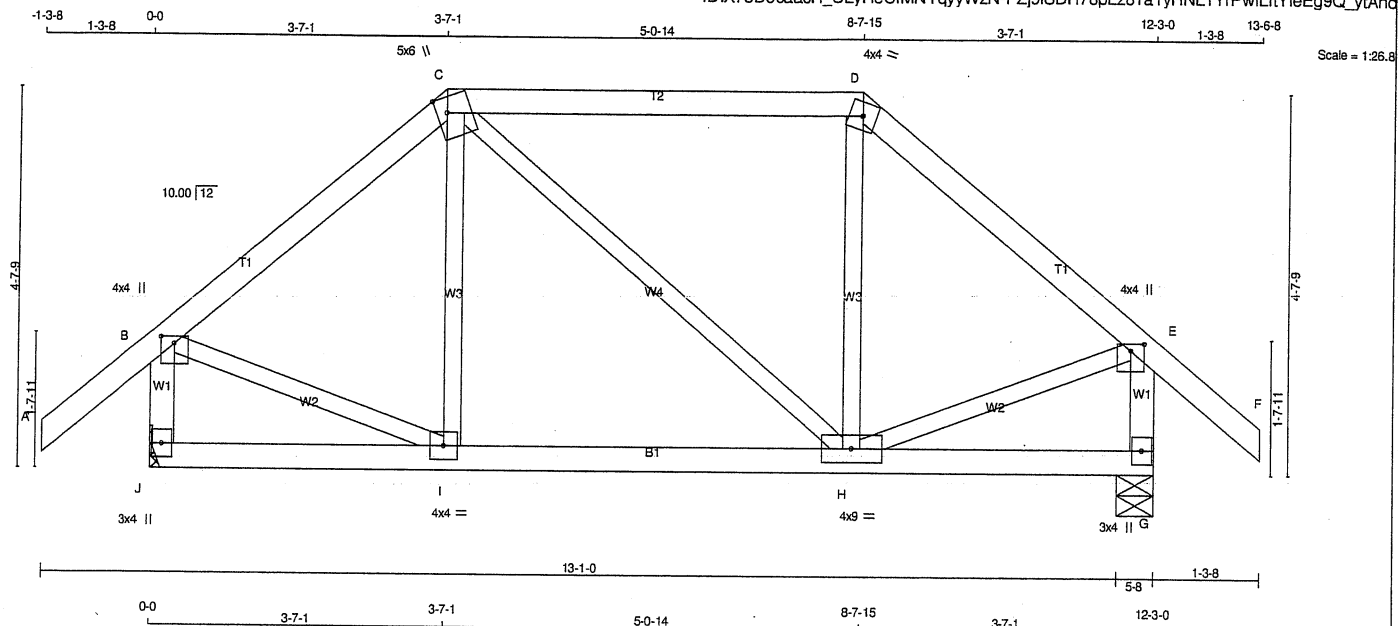
JSI GRIP= 0.58 (B) (INPUT = 0.90)
JSI METAL= 0.16 (B) (INPUT = 1.00)



Structural component only
DWG# T-2125213

JOB NAME 420295	TRUSS NAME T10	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 29 15:35:50 2021 Page 1
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Scale = 1:26.8

TOTAL WEIGHT = 55 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4 DRY	No.2	SPF
C - D	2x4 DRY	No.2	SPF
D - F	2x4 DRY	No.2	SPF
J - B	2x4 DRY	No.2	SPF
G - E	2x4 DRY	No.2	SPF
J - G	2x4 DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	4.0	1.00	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMVWW-t	MT20	4.0	9.0		
I	BMVWW-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

NOTES-

(1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
J	751	0	0	0
G	751	0	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 1-8.

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX./MIN. SNOW	MAX./MIN. LIVE	PERM. LIVE	WIND	DEAD	SOIL
J	531	350 / 0	0 / 0	0 / 0	0 / 0	181 / 0	0 / 0
G	531	350 / 0	0 / 0	0 / 0	0 / 0	181 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	
FR-TO				FR-TO			
A-B	0 / 38	-84.9	0.12 (1)	I-C	-47 / 48	0.02 (4)	
B-C	-498 / 0	-84.9	0.14 (1)	C-H	0 / 0	0.00 (1)	
C-D	-380 / 0	-84.9	0.28 (1)	H-D	-47 / 48	0.02 (4)	
D-E	-498 / 0	-84.9	0.14 (1)	B-I	0 / 405	0.09 (1)	
E-F	0 / 38	-84.9	0.12 (1)	H-E	0 / 405	0.09 (1)	
J-B	-725 / 0	0.0	0.08 (1)				
G-E	-725 / 0	0.0	0.08 (1)				
J-I	0 / 0	-18.5	0.08 (4)				
I-H	0 / 380	-18.5	0.11 (4)				
H-G	0 / 0	-18.5	0.08 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.28/1.00 (C-D:1), BC=0.11/1.00 (H-I:4),
WB=0.09/1.00 (B-I:1), SS=0.17/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

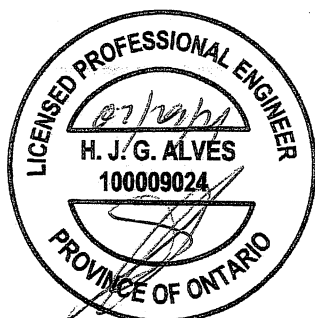
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

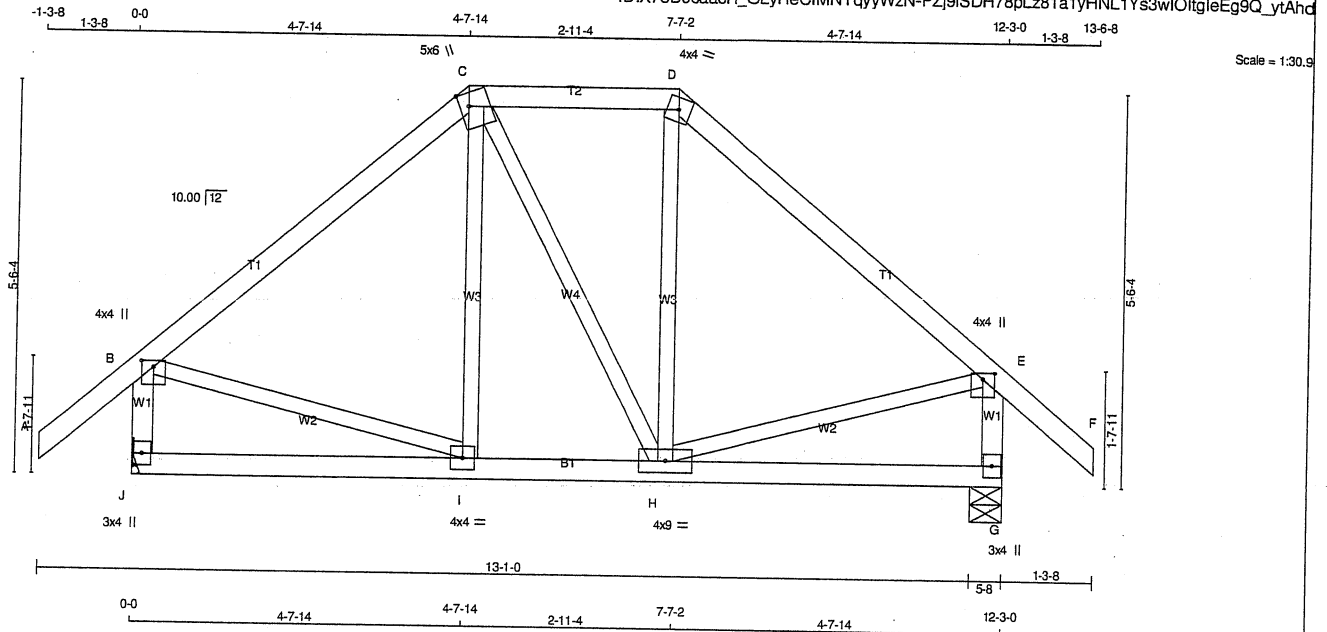
JSI GRIP= 0.61 (B) (INPUT = 0.90)
JSI METAL= 0.17 (B) (INPUT = 1.00)



Structural component only
DWG# T-2125214

JOB NAME 420295	TRUSS NAME T11	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 29 15:35:50 2021 Page 1
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LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	DESCR.
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
J - B	2x4	DRY	No.2
G - E	2x4	DRY	No.2
J - G	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	4.0	1.00	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	9.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	DOWN	HORZ	UPLIFT
J	751	0	751	0	0
G	751	0	751	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
J	531	350 / 0	0 / 0	0 / 0	0 / 0	0 / 0	181 / 0	0 / 0
G	531	350 / 0	0 / 0	0 / 0	0 / 0	0 / 0	181 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 38	I-C	-23 / 47
B-C	-467 / 0	C-H	0 / 2
C-D	-358 / 0	H-D	-22 / 49
D-E	-467 / 0	B-I	0 / 371
E-F	0 / 38	H-E	0 / 371
J-B	-713 / 0		
G-E	-712 / 0		
J-I	0 / 0		
I-H	0 / 357		
H-G	0 / 0		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.24/1.00 (D-E:1), BC=0.11/1.00 (H-I:4), WB=0.08/1.00 (E-H:1), SSI=0.12/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.61 (E) (INPUT = 0.90)
JSI METAL= 0.17 (E) (INPUT = 1.00)

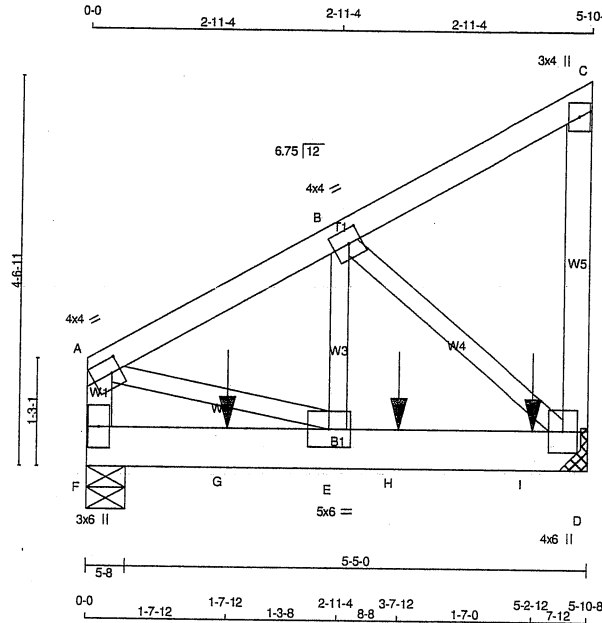


Structural component only
DWG# T-2125215

JOB NAME 420295	TRUSS NAME T12	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 29 15:35:51 2021 Page 1
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Scale = 1:25.7

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C 1	12	TOP
C-D 1	12	TOP
F-A 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F-D 2	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0	2.00	1.25
B	TMVW-t	MT20	4.0	4.0	2.00	1.75
C	TMV+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ
D	1618	0	1618	0
F	1189	0	1189	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LOASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
	COMBINED						
D	1141	763 / 0	0 / 0	0 / 0	0 / 0	378 / 0	0 / 0
F	839	560 / 0	0 / 0	0 / 0	0 / 0	280 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

C H O R D S				W E B S			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRAC LENGTH
FR-TO		FROM TO			FR-TO		
A-B	-1146 / 0	-84.9 -84.9	0.06 (1)	6.25	E-B	0 / 1134	0.14 (1)
B-C	-12 / 0	-84.9 -84.9	0.05 (1)	6.25	B-D	-1328 / 0	0.17 (1)
D-C	-101 / 0	0.0 0.0	0.02 (1)	7.81	A-E	0 / 1052	0.13 (1)
F-A	-964 / 0	0.0 0.0	0.05 (1)	7.81			
F-G	0 / 0	-18.5 -18.5	0.13 (1)	10.00			
G-E	0 / 0	-18.5 -18.5	0.13 (1)	10.00			
E-H	0 / 1009	-18.5 -18.5	0.22 (1)	10.00			
H-I	0 / 1009	-18.5 -18.5	0.22 (1)	10.00			
I-D	0 / 1009	-18.5 -18.5	0.22 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-7-12	-516	-516	---	BACK	VERT	TOTAL	---	C1
H	3-7-12	-516	-516	---	BACK	VERT	TOTAL	---	C1
I	5-2-12	-518	-518	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 30 = 60 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	23.3	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.4	PSF
TOTAL LOAD	=	36.7	PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.06/1.00 (A-B:1), BC=0.22/1.00 (D-E:1),
WB=0.17/1.00 (B-D:1), SSI=0.29/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
			788
			1987
			1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (A) (INPUT = 0.90)

JSI METAL= 0.23 (D) (INPUT = 1.00)



Structural component only
DWG# T-2125216

CONTINUED ON PAGE 2

JOB NAME 420295	TRUSS NAME T12	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.																												
Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 29 15:35:51 2021 Page 2 ID:X73D9caach OLyHeCIMNYqyyWzn-IGXwoEwuSxCblcmbfoctF54eKcx1JcRsuPiyRytAhc																													
PLATES (table is in inches) <table border="1"><thead><tr><th>JT</th><th>TYPE</th><th>PLATES</th><th>W</th><th>LEN</th><th>Y</th><th>X</th></tr></thead><tbody><tr><td>D</td><td>BMVW1+p</td><td>MT20</td><td>4.0</td><td>6.0</td><td></td><td></td></tr><tr><td>E</td><td>BMWW-t</td><td>MT20</td><td>5.0</td><td>6.0</td><td></td><td></td></tr><tr><td>F</td><td>BMV1+p</td><td>MT20</td><td>3.0</td><td>6.0</td><td></td><td></td></tr></tbody></table> NOTES: (1) 1) Lateral braces to be a minimum of 2X4 SPF #2.						JT	TYPE	PLATES	W	LEN	Y	X	D	BMVW1+p	MT20	4.0	6.0			E	BMWW-t	MT20	5.0	6.0			F	BMV1+p	MT20	3.0	6.0		
JT	TYPE	PLATES	W	LEN	Y	X																											
D	BMVW1+p	MT20	4.0	6.0																													
E	BMWW-t	MT20	5.0	6.0																													
F	BMV1+p	MT20	3.0	6.0																													
 Structural component only DWG# T-2125216 <i>mc</i>																																	

JOB NAME 420295	TRUSS NAME T12Z	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 29 15:35:52 2021 Page 2
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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
D	BMVW1+p	MT20	4.0	6.0		
E	BMWW-t	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	6.0		

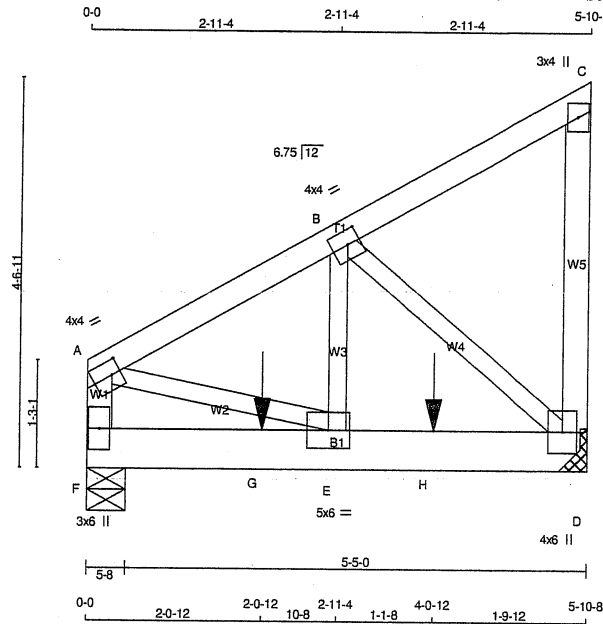
NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only
DWG# T-2125217 *en*

JOB NAME 420295	TRUSS NAME T1221	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 29 15:35:53 2021 Page 1 ID:X73D9caach_OLyHeCIMNYqyyWzN-p8OHLTGAQ3Bwqbm9j4q4ygAQ57InVE_kKCup1JytAha			



Scale = 1:25.7

TOTAL WEIGHT = 2 X 30 = 60 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12	TOP
C - D	12	TOP
F - A	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F - D	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	1	6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0	2.00	1.25
B	TMVW-t	MT20	4.0	4.0	2.00	1.75
C	TMVW-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
D	814	0	814	0
F	772	0	772	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. SNOW	MIN. LIVE	PERM. LIVE	WIND	DEAD	SOIL
D	576	376 / 0	0 / 0	0 / 0	0 / 0	201 / 0	0 / 0
F	547	356 / 0	0 / 0	0 / 0	0 / 0	191 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)
FR-TO		FROM TO			FR-TO		
A-B	-746 / 0	-84.9	-84.9 0.06 (1)	6.25	E-B	0 / 636	0.08 (1)
B-C	-13 / 0	-84.9	-84.9 0.06 (1)	6.25	B-D	-871 / 0	0.11 (1)
D-C	-98 / 0	0.0	0.0 0.02 (1)	7.81	A-E	0 / 690	0.09 (1)
F-A	-664 / 0	0.0	0.0 0.04 (1)	7.81			
F-G	0 / 0	-18.5	-18.5 0.07 (1)	10.00			
G-E	0 / 0	-18.5	-18.5 0.07 (1)	10.00			
E-H	0 / 662	-18.5	-18.5 0.13 (1)	10.00			
H-D	0 / 662	-18.5	-18.5 0.13 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	2-0-12	-346	-346	---	FRONT	VERT	TOTAL	---	C1
H	4-0-12	-346	-346	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.06/1.00 (A-B:1), BC=0.13/1.00 (D-E:1),
WB=0.11/1.00 (B-D:1), SS=0.14/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.53 (A) (INPUT = 0.90)
JSI METAL= 0.15 (D) (INPUT = 1.00)



Structural component only
DWG# T-2125218

CONTINUED ON PAGE 2

JOB NAME 420295	TRUSS NAME T12Z1	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 29 15:35:53 2021 Page 2
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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
D	BMVW1+p	MT20	4.0	6.0		
E	BMWW-1	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	6.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

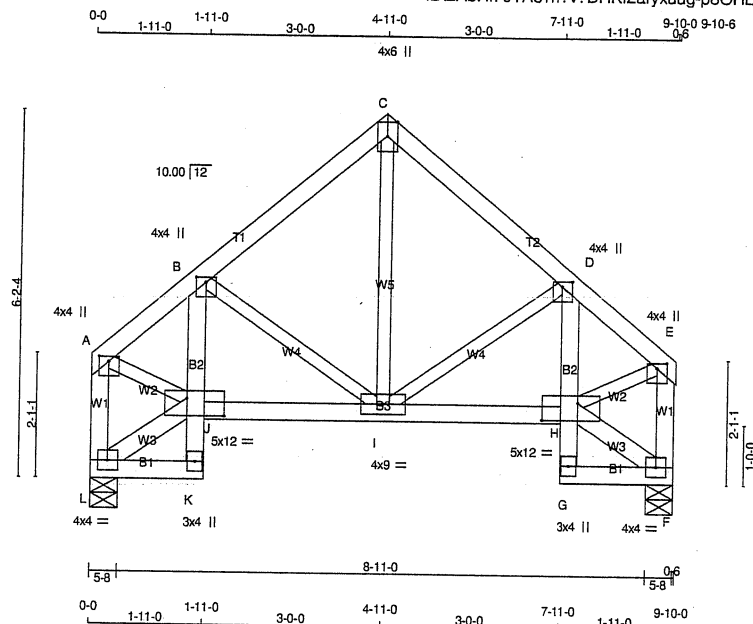


Structural component only
DWG# T-2125218 *m*

JOB NAME 420295	TRUSS NAME T13	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 29 15:35:53 2021 Page 1
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Scale = 1:37.0



TOTAL WEIGHT = 53 lb
[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER
A - C	2x4	DRY No.2
C - E	2x4	DRY No.2
L - A	2x4	DRY No.2
F - E	2x4	DRY No.2
L - K	2x4	DRY No.2
K - B	2x4	DRY No.2
J - H	2x4	DRY No.2
G - D	2x4	DRY No.2
G - F	2x4	DRY No.2

ALL WEBS	2x3	DRY No.2
EXCEPT		
L - J	2x4	DRY No.2
H - F	2x4	DRY No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A, B, D, E						
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW+p	MT20	4.0	6.0	Edge	
F	BMVW1-t	MT20	4.0	4.0		
G	BMV+p	MT20	3.0	4.0		
H	BMVWV-t	MT20	5.0	12.0	3.50	7.50
I	BMVWV-t	MT20	4.0	9.0		
J	BMVWV-t	MT20	5.0	12.0	3.50	7.50
K	BMV+p	MT20	3.0	4.0		
L	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES-

(1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
L	508	0	508	0
F	508	0	508	0

UNFACTORED REACTIONS

	1ST LCASE	MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
L	361	229 / 0	0 / 0	0 / 0	0 / 0	132 / 0	0 / 0
F	361	229 / 0	0 / 0	0 / 0	0 / 0	132 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	FR-TO
A-B	-468 / 0	-84.9	-84.9 0.08 (1)	I-C	0 / 214	6.25	0.05 (1)
B-C	-355 / 0	-84.9	-84.9 0.09 (1)	I-D	-150 / 0	6.25	0.04 (1)
C-D	-355 / 0	-84.9	-84.9 0.09 (1)	B-I	-150 / 0	6.25	0.04 (1)
D-E	-468 / 0	-84.9	-84.9 0.08 (1)	L-J	-11 / 0	6.25	0.00 (1)
L-A	-486 / 0	0.0	0.0 0.06 (1)	H-F	-11 / 0	7.81	0.00 (1)
F-E	-486 / 0	0.0	0.0 0.06 (1)	H-E	0 / 402	7.81	0.09 (1)
L-K	0 / 9	-18.5	-18.5 0.02 (4)	A-J	0 / 402	7.81	0.09 (1)
K-J	0 / 18	0.0	0.0 0.02 (1)				
J-B	-86 / 0	0.0	0.0 0.01 (1)				
J-I	0 / 384	-18.5	-18.5 0.09 (1)				
I-H	0 / 384	-18.5	-18.5 0.09 (1)				
G-H	0 / 18	0.0	0.0 0.02 (1)				
H-D	-86 / 0	0.0	0.0 0.01 (1)				
G-F	0 / 9	-18.5	-18.5 0.02 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.33")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.33")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.09/1.00 (B-C:1), BC=0.09/1.00 (I-J:1), WB=0.09/1.00 (A-J:1), SS=0.09/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

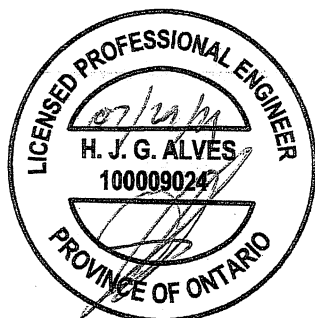
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

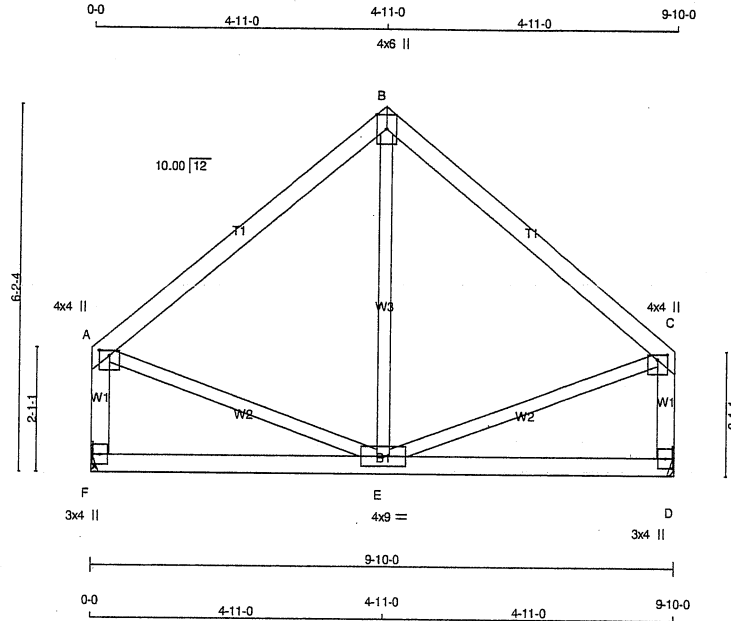
JSI GRIP= 0.46 (E) (INPUT = 0.90)
JSI METAL= 0.13 (E) (INPUT = 1.00)



Structural component only
DWG# T-2125219

JOB NAME 420295	TRUSS NAME T14	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 29 15:35:54 2021 Page 1
ID:2AbHI70TAclh?v?DHKf2aryxuug-HKyfYpHoBNJnSILLGnLJVtjXfX4Ei7tZseNZIytAhZ



Scale = 1:37.0

TOTAL WEIGHT = 2 X 42 = 85 lb
[M][F]

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW+p	MT20	4.0	4.0	1.00 2.00
B	TTW+p	MT20	4.0	6.0	Edge
C	TMVW+p	MT20	4.0	4.0	1.00 2.00
D	BMV1+p	MT20	3.0	4.0	
E	BMVWW-t	MT20	4.0	9.0	
F	BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT
F	508	0	508	0	0
D	508	0	508	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F, D. MINIMUM BEARING LENGTH AT JOINT F = 1-8, JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	361	229 / 0	0 / 0	0 / 0	0 / 0	132 / 0	0 / 0
D	361	229 / 0	0 / 0	0 / 0	0 / 0	132 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX.	MEMB.	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. (PLF)	CSI (LC)	UNBRAC	LENGTH	FR-TO	MEMB.	FORCE (LBS)	MAX
A-B	-290 / 0	-84.9	-84.9	0.26 (1)	6.25	E-B	-47 / 64	0.03 (1)	
B-C	-290 / 0	-84.9	-84.9	0.26 (1)	6.25	A-E	0 / 236	0.05 (1)	
F-A	-473 / 0	0.0	0.0	0.06 (1)	7.81	E-C	0 / 236	0.05 (1)	
D-C	-473 / 0	0.0	0.0	0.06 (1)	7.81				
F-E	0 / 0	-18.5	-18.5	0.13 (4)	10.00				
E-D	0 / 0	-18.5	-18.5	0.13 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.33")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.33")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.26/1.00 (B-C:1), BC=0.13/1.00 (E-F:4), WB=0.05/1.00 (A-E:1), SSI=0.13/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

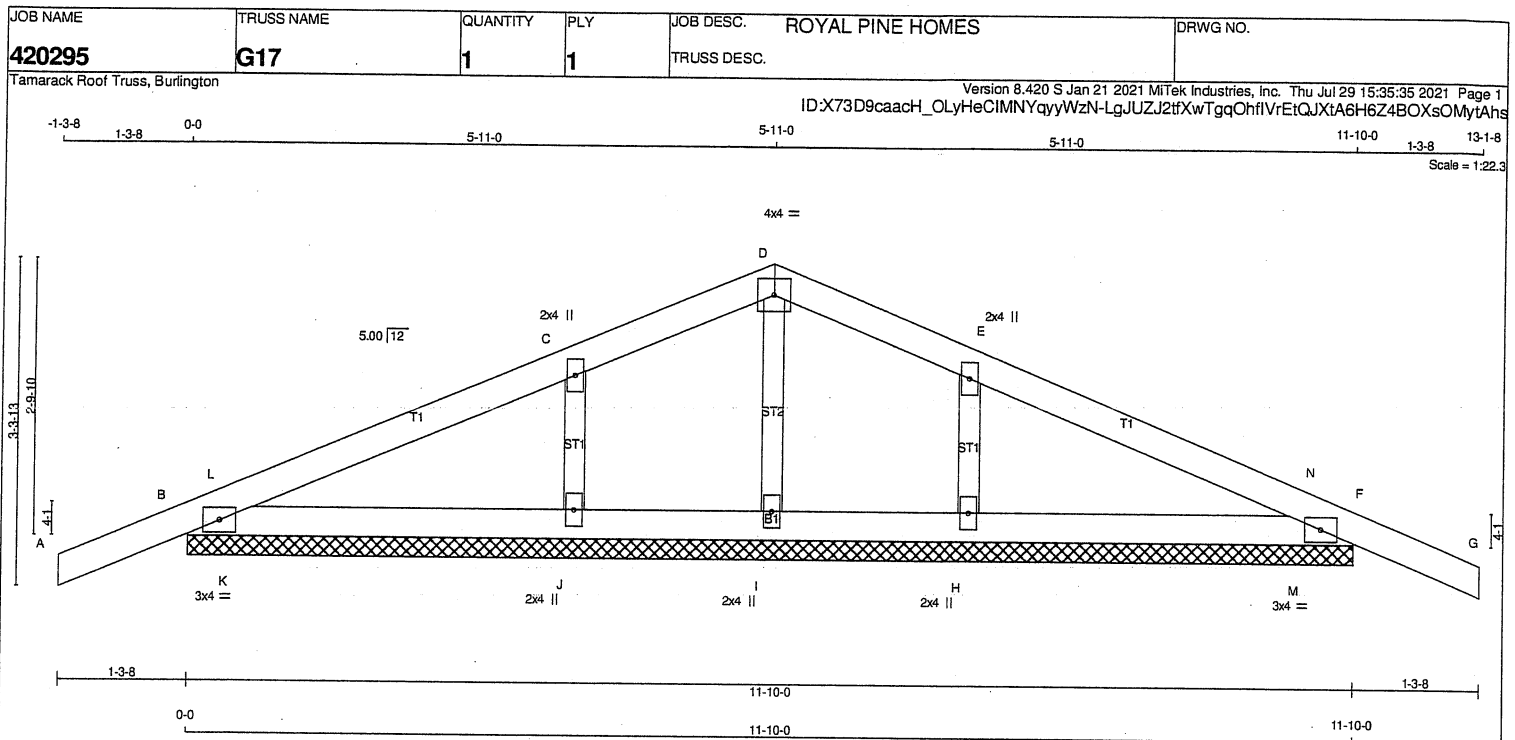
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.39 (A) (INPUT = 0.90)
JSI METAL= 0.11 (C) (INPUT = 1.00)



Structural component only
DWG# T-2125220



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
B - F	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2

ALL GABLE WEBS 2x3 DRY No.2

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2'-0" O.C.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TMW+w	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TMB1-I	MT20	3.0	4.0		
H, I, J						
H	BMW1+w	MT20	2.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 21	-84.9 -84.9	0.11 (1)	10.00	I-D	-59 / 0	0.01 (1)
B-L	-41 / 0	-84.9 -84.9	0.03 (4)	6.25	J-C	-301 / 0	0.04 (1)
L-C	-29 / 0	-84.9 -84.9	0.13 (1)	6.25	H-E	-301 / 0	0.04 (1)
C-D	-47 / 0	-84.9 -84.9	0.13 (1)	6.25	K-L	-134 / 5	0.00 (1)
D-E	-47 / 0	-84.9 -84.9	0.13 (1)	6.25	M-N	-134 / 5	0.00 (1)
E-N	-29 / 0	-84.9 -84.9	0.13 (1)	6.25			
N-F	-41 / 0	-84.9 -84.9	0.03 (4)	6.25			
F-G	0 / 21	-84.9 -84.9	0.11 (1)	10.00			
B-K	0 / 37	-18.5 -18.5	0.10 (1)	10.00			
K-J	0 / 37	-18.5 -18.5	0.10 (1)	10.00			
J-I	0 / 27	-18.5 -18.5	0.07 (1)	10.00			
I-H	0 / 27	-18.5 -18.5	0.07 (1)	10.00			
H-M	0 / 37	-18.5 -18.5	0.10 (1)	10.00			
M-F	0 / 37	-18.5 -18.5	0.10 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13/1.00 (C-L:1), BC=0.10/1.00 (B-K:1), WB=0.04/1.00 (C-J:1), SSI=0.12/1.00 (C-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

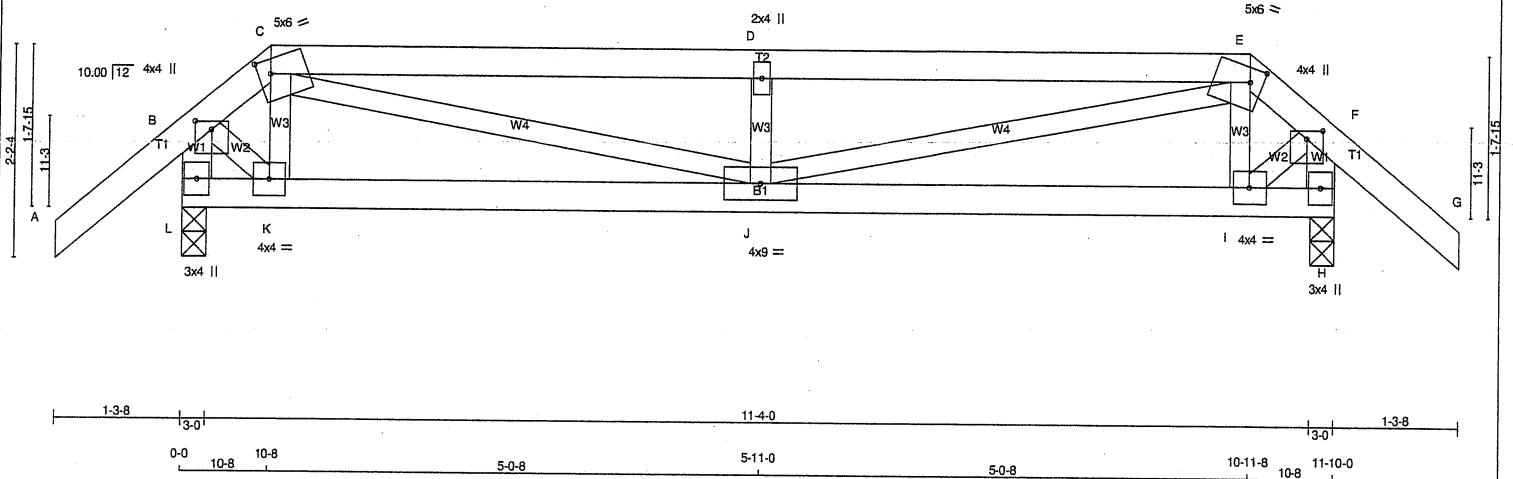
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.21 (B) (INPUT = 0.90)
JSI METAL = 0.11 (C) (INPUT = 1.00)



Structural component only
DWG# T-2125197



LUMBER
 N. L. G. A. RULES
 CHORDS SIZE LUMBER DESCR.
 A - C 2x4 DRY No.2 SPF
 C - E 2x4 DRY No.2 SPF
 E - G 2x4 DRY No.2 SPF
 L - B 2x4 DRY No.2 SPF
 H - F 2x4 DRY No.2 SPF
 L - H 2x4 DRY No.2 SPF
 ALL WEBS 2x3 DRY No.2 SPF
 EXCEPT
 DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTWW-m	MT20	5.0	6.0	1.75	1.50
D	TMVW-w	MT20	2.0	4.0		
E	TTWW-m	MT20	5.0	6.0	1.75	1.50
F	TMVW+p	MT20	4.0	4.0	1.00	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMVW-t	MT20	4.0	4.0		
J	BMVWW-t	MT20	4.0	9.0		
K	BMVW-t	MT20	4.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

NOTES: (1)
 1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
L	729	0	729	0	3-0	3-0
H	729	0	729	0	3-0	3-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM.	LIVE			
L	515	340 / 0	0 / 0	0 / 0	0 / 0	175 / 0	0 / 0
H	515	340 / 0	0 / 0	0 / 0	0 / 0	175 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.08 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
 ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)		FACTORED VERT. LOAD (PLF)		MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	WEBS MAX. FACTORED FORCE (LBS)		MAX. CSI (LC)
	FR-TO	FROM TO	FR-TO	FROM TO				FR-TO	FROM TO	
A-B	0 / 38	-84.9	-84.9	0.12 (1)	10.00	K-C	-80 / 17	0.01 (1)		
B-C	-549 / 0	-84.9	-84.9	0.11 (1)	6.25	C-J	0 / 1075	0.24 (1)		
C-D	-1403 / 0	-84.9	-84.9	0.35 (1)	5.08	J-D	-504 / 0	0.07 (1)		
D-E	-1403 / 0	-84.9	-84.9	0.35 (1)	5.08	E-E	0 / 1075	0.24 (1)		
E-F	-549 / 0	-84.9	-84.9	0.11 (1)	6.25	I-E	-80 / 17	0.01 (1)		
F-G	0 / 38	-84.9	-84.9	0.12 (1)	10.00	B-K	0 / 434	0.10 (1)		
L-B	-792 / 0	0.0	0.0	0.08 (1)	7.81	I-F	0 / 434	0.10 (1)		
H-F	-792 / 0	0.0	0.0	0.08 (1)	7.81					
L-K	0 / 0	-18.5	-18.5	0.10 (4)	10.00					
K-J	0 / 365	-18.5	-18.5	0.15 (1)	10.00					
J-I	0 / 365	-18.5	-18.5	0.15 (1)	10.00					
I-H	0 / 0	-18.5	-18.5	0.10 (4)	10.00					

TOTAL WEIGHT = 45 lb

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 23.3 PSF
 DL = 6.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF CBC 2015, ABC 2019
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 086-14
 - TPIC 2014

DESIGN ASSUMPTIONS
 - OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.39")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
 ALLOWABLE DEFL.(TL) = L/360 (0.39")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.09")

CSI: TC=0.35/1.00 (D-E:1), BC=0.15/1.00 (I-J:1), WB=0.24/1.00 (C-J:1), SSI=0.20/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (E) (INPUT = 0.90)
 JSI METAL= 0.23 (J) (INPUT = 1.00)



Structural component only
 DWG# T-2125221

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 29 15:35:56 2021 Page 1
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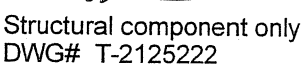
Scale = 1"=22'



NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM TO			FR-TO			
A-B	0 / 38	-84.9	-84.9	0.12 (1)	10.00	K-C	-57 / 23	0.01 (1)
B-C	-599 / 0	-84.9	-84.9	0.07 (1)	6.25	C-J	0 / 419	0.09 (1)
C-D	-810 / 0	-84.9	-84.9	0.20 (1)	6.25	J-D	-384 / 0	0.06 (1)
D-E	-810 / 0	-84.9	-84.9	0.20 (1)	6.25	J-E	0 / 419	0.09 (1)
E-F	-599 / 0	-84.9	-84.9	0.07 (1)	6.25	I-E	-57 / 23	0.01 (1)
F-G	0 / 38	-84.9	-84.9	0.12 (1)	10.00	B-K	0 / 474	0.11 (1)
L-B	-717 / 0	0.0	0.0	0.07 (1)	7.81	I-F	0 / 474	0.11 (1)
H-F	-717 / 0	0.0	0.0	0.07 (1)	7.81			
L-K	0 / 0	-18.5	-18.5	0.04 (4)	10.00			
K-J	0 / 455	-18.5	-18.5	0.10 (1)	10.00			
J-I	0 / 455	-18.5	-18.5	0.10 (1)	10.00			
I-H	0 / 0	-18.5	-18.5	0.04 (4)	10.00			

TRUSS PLATE MANUFACTURER IS NOT



amarcar Roof Truss, Burlington

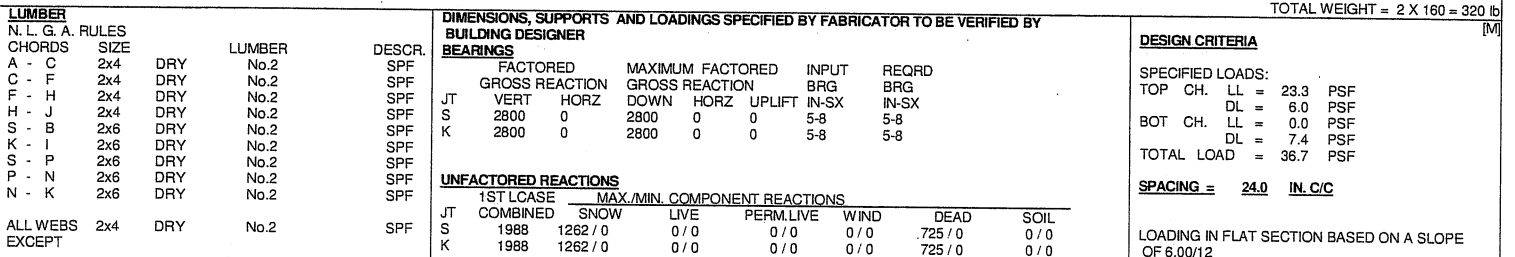
Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 29 15:59:32 2021 Page 1

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The diagram shows a roof truss with various dimensions and member lengths. The dimensions are as follows:

- 1-3-8
- 0-0
- 3-0-8
- 3-0-8
- 2-10-0
- 5-10-8
- 2-0-12
- 7-11-4
- 2-9-14
- 10-9-2
- 15-6-0
- 4-8-14
- 20-2-14
- 4-10-10
- 25-1-8
- 2-10-0
- 27-11-8
- 3-0-8
- 31-0-0
- 1-3-8

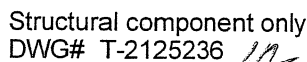
Scale = 1:52 S



DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.



BEARINGS							
FACTORED GROSS REACTION			MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQ'D BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
S	2800	0	2800	0	0	5-8	5-8
K	2800	0	2800	0	0	5-8	5-8

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1988	1262 / 0	0 / 0	0 / 0	0 / 0	725 / 0	0 / 0
K	1988	1262 / 0	0 / 0	0 / 0	0 / 0	725 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FORCE (LBS)	FACTORED VERT. LOAD (LBS)	LC1 MAX (PLF)	LC2 MAX (LBS)	MAX. UNBRAC LENGTH	MEMB.	MAX. FORCE (LBS)	FACTORED MAX CSI (LC)
FR-TO		FROM	TO			FR-TO		
A-B	0 / 28	-84.9	-84.9	0.06 (1)	10.00	R-C	-289 / 46	0.03 (1)
B-C	-3832 / 0	-84.9	-84.9	0.42 (1)	4.46	L-H	-289 / 46	0.03 (1)
C-T	-4814 / 0	-84.9	-84.9	0.37 (1)	4.05	B-R	0 / 3371	0.30 (1)
T-U	-4814 / 0	-84.9	-84.9	0.37 (1)	4.05	L-I	0 / 3371	0.30 (1)
U-D	-4814 / 0	-84.9	-84.9	0.37 (1)	4.05	M-H	0 / 1956	0.17 (1)
D-V	-5256 / 0	-84.9	-84.9	0.39 (1)	3.88	C-Q	0 / 1956	0.17 (1)
V-W	-5256 / 0	-84.9	-84.9	0.39 (1)	3.88	M-G	-1121 / 0	0.12 (1)
W-E	-5256 / 0	-84.9	-84.9	0.39 (1)	3.88	Q-D	-1121 / 0	0.12 (1)
E-X	-5256 / 0	-84.9	-84.9	0.39 (1)	3.88	C-G	0 / 589	0.05 (1)
X-F	-5256 / 0	-84.9	-84.9	0.39 (1)	3.88	D-O	0 / 589	0.05 (1)
F-Y	-5256 / 0	-84.9	-84.9	0.39 (1)	3.88	C-E	-635 / 0	0.07 (1)
Y-G	-5256 / 0	-84.9	-84.9	0.39 (1)	3.88			
G-Z	-4814 / 0	-84.9	-84.9	0.37 (1)	4.05			
Z-AA	-4814 / 0	-84.9	-84.9	0.37 (1)	4.05			
AA-H	-4814 / 0	-84.9	-84.9	0.37 (1)	4.05			
H-I	-3832 / 0	-84.9	-84.9	0.42 (1)	4.46			
I-J	0 / 28	-84.9	-84.9	0.06 (1)	10.00			
S-B	-2724 / 0	0.0	0.0	0.10 (1)	7.81			
K-I	-2724 / 0	0.0	0.0	0.10 (1)	7.81			
S-AB	0 / 0	-18.5	-18.5	0.07 (4)	10.00			
AB-AC	0 / 0	-18.5	-18.5	0.07 (4)	10.00			
AC-R	0 / 0	-18.5	-18.5	0.07 (4)	10.00			
R-AD	0 / 3327	-18.5	-18.5	0.24 (1)	10.00			
AD-AE	0 / 3327	-18.5	-18.5	0.24 (1)	10.00			
AE-Q	0 / 3327	-18.5	-18.5	0.24 (1)	10.00			
Q-P	0 / 4814	-18.5	-18.5	0.36 (1)	10.00			
P-AF	0 / 4814	-18.5	-18.5	0.36 (1)	10.00			
AF-O	0 / 4814	-18.5	-18.5	0.36 (1)	10.00			
O-AG	0 / 4814	-18.5	-18.5	0.36 (1)	10.00			
AG-N	0 / 4814	-18.5	-18.5	0.36 (1)	10.00			
N-M	0 / 4814	-18.5	-18.5	0.36 (1)	10.00			
M-AH	0 / 3327	-18.5	-18.5	0.24 (1)	10.00			
AH-AI	0 / 3327	-18.5	-18.5	0.24 (1)	10.00			
AI-L	0 / 3327	-18.5	-18.5	0.24 (1)	10.00			
L-AJ	0 / 0	-18.5	-18.5	0.07 (4)	10.00			
AJ-AK	0 / 0	-18.5	-18.5	0.07 (4)	10.00			
AK-K	0 / 0	-18.5	-18.5	0.07 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-8	-294	-294	---	FRONT	VERT	TOTAL	---	C1
E	15-6-0	-71	-71	---	FRONT	VERT	TOTAL	---	C1

SPECIFIED LOADS:			
TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD		= 36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR
SMALL BUILDING REQUIREMENTS OF PART 9,
NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL(LL)= L/360 (1.03")
CALCULATED VERT. DEFL(LL) = L/ 999 (0.11")
ALLOWABLE DEFL(TL)= L/360 (1.03")
CALCULATED VERT. DEFL(TL) = L/ 999 (0.22")

CSI: TC=0.42/1.00 (B-C:1), BC=0.36/1.00 (O-Q:1),
WB=0.30/1.00 (B-R:1), SSI=0.17/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (B) (INPUT = 0.90)
JSI METAL= 0.46 (B) (INPUT = 1.00)

JOB NAME 420298	TRUSS NAME T30	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 29 15:59:32 2021 Page 2
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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.25	2.00
C	TTWW-m	MT20	5.0	8.0	2.00	2.50
D	TMVW-t	MT20	4.0	4.0		
E	TMVW-w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMVW-t	MT20	4.0	4.0		
H	TTWW-m	MT20	5.0	8.0	2.00	2.50
I	TMVW-t	MT20	5.0	6.0	2.25	2.00
K	BMV1+p	MT20	3.0	6.0		
L, M, Q, R						
L	BMVW-t	MT20	5.0	6.0		
N	BS-t	MT20	5.0	6.0		
O	BMVWVW-t	MT20	5.0	8.0		
P	BS-t	MT20	5.0	6.0		
S	BMV1+p	MT20	3.0	6.0		

NOTES: (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
H	25-1-8	-294	-294	---	FRONT	VERT	TOTAL	---	C1
L	25-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
N	19-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
O	15-6-0	-21	-21	---	FRONT	VERT	TOTAL	---	C1
P	11-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
R	5-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
T	7-11-4	-71	-71	---	FRONT	VERT	TOTAL	---	C1
U	9-11-4	-71	-71	---	FRONT	VERT	TOTAL	---	C1
V	11-11-4	-71	-71	---	FRONT	VERT	TOTAL	---	C1
W	13-11-4	-71	-71	---	FRONT	VERT	TOTAL	---	C1
X	17-0-12	-71	-71	---	FRONT	VERT	TOTAL	---	C1
Y	19-0-12	-71	-71	---	FRONT	VERT	TOTAL	---	C1
Z	21-0-12	-71	-71	---	FRONT	VERT	TOTAL	---	C1
AA	23-0-12	-71	-71	---	FRONT	VERT	TOTAL	---	C1
AB	1-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AC	3-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AD	7-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AE	9-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AF	13-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AG	17-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AH	21-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AI	23-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AJ	27-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AK	29-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1

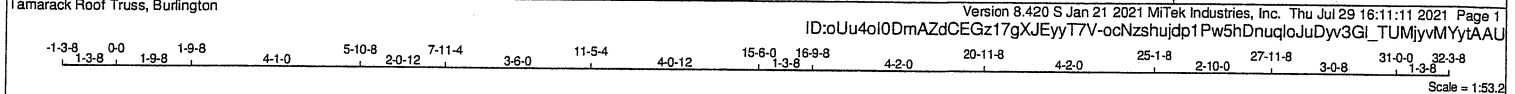
CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2125236 *M*

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420299	T30S	1	2	ROYAL PINE HOMES	
TRUSS DESC.					



JOB NAME 420299	TRUSS NAME T30S	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 29 16:11:11 2021 Page 2
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GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0		
C	TMVW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW-m	MT20	5.0	8.0	2.00	3.00
E	TMVW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	TMVW-t	MT20	5.0	6.0		
H	TS-t	MT20	3.0	6.0		
I	TTWW-m	MT20	5.0	6.0	2.00	1.75
J	TMVW-t	MT20	5.0	6.0	2.00	2.00
L	BMV1+p	MT20	3.0	6.0		
M, Q, R						
M	BMVW-t	MT20	5.0	6.0		
N	BMVW-t	MT20	6.0	9.0		
O	BMV+p	MT20	3.0	6.0		
P	BVMW-t	MT20	8.0	12.0	4.50	4.50
S	BVMW-t	MT20	6.0	12.0	3.25	7.00
T	BMV+p	MT20	3.0	6.0		
U	BMVW-t	MT20	5.0	6.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

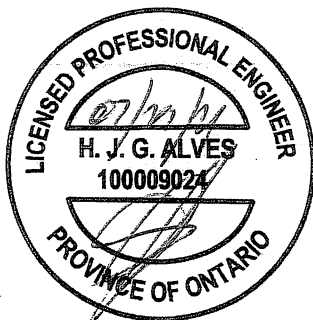
SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	5-10-8	-58	-58	---	FRONT	VERT	DEAD	---	C1
D	5-10-8	-73	-73	---	FRONT	VERT	TOTAL	---	C1
D	5-10-8	-233	-233	---	FRONT	VERT	SNOW	---	C1
F	17-0-12	-71	-71	---	FRONT	VERT	TOTAL	---	C1
G	21-0-12	-71	-71	---	FRONT	VERT	TOTAL	---	C1
I	25-1-8	-294	-294	---	FRONT	VERT	TOTAL	---	C1
M	25-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
N	21-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
P	16-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
R	5-11-4	-65	-65	---	FRONT	VERT	TOTAL	---	C1
T	1-7-12	-197	-197	---	FRONT	VERT	TOTAL	---	C1
V	7-11-4	-67	-67	---	FRONT	VERT	TOTAL	---	C1
W	9-11-4	-67	-67	---	FRONT	VERT	TOTAL	---	C1
X	11-11-4	-67	-67	---	FRONT	VERT	TOTAL	---	C1
Y	13-11-4	-67	-67	---	FRONT	VERT	TOTAL	---	C1
Z	15-6-0	-67	-67	---	FRONT	VERT	TOTAL	---	C1
AA	19-0-12	-71	-71	---	FRONT	VERT	TOTAL	---	C1
AB	23-0-12	-71	-71	---	FRONT	VERT	TOTAL	---	C1
AC	3-11-4	-65	-65	---	FRONT	VERT	TOTAL	---	C1
AD	7-11-4	-65	-65	---	FRONT	VERT	TOTAL	---	C1
AE	9-11-4	-65	-65	---	FRONT	VERT	TOTAL	---	C1
AF	11-11-4	-65	-65	---	FRONT	VERT	TOTAL	---	C1
AG	13-11-4	-65	-65	---	FRONT	VERT	TOTAL	---	C1
AH	15-6-0	-65	-65	---	FRONT	VERT	TOTAL	---	C1
AI	19-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AJ	23-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AK	27-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AL	29-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1

JSI GRIP= 0.90 (J) (INPUT = 0.90)
JSI METAL= 0.62 (B) (INPUT = 1.00)

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



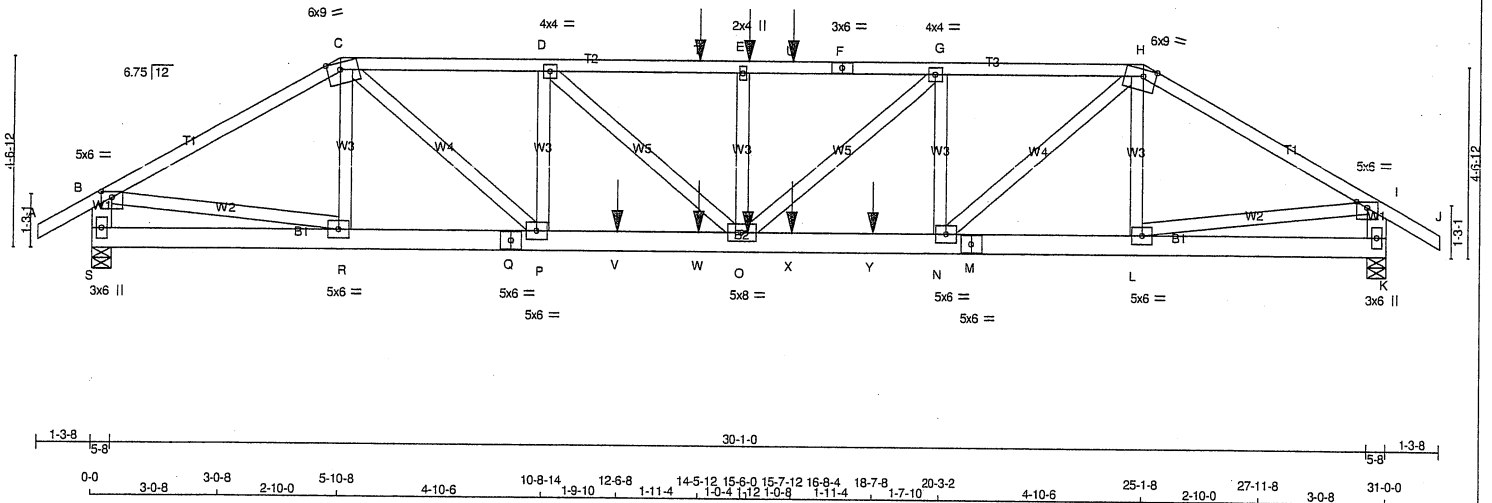
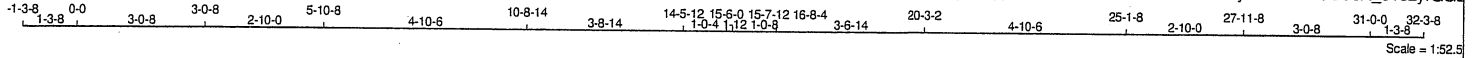
Structural component only
DWG# T-2125249 *2/2*

JOB NAME 420299	TRUSS NAME T30Z	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed Aug 4 10:52:48 2021 Page 1

ID:Uu4ol0DmAZdCEGz17gXJEyyT7V-29LrMfOCUrBzbKL6ih2IUymbAZMHOU5eX_8?J2yrGGz



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - J	2x4	DRY	No.2
S - B	2x6	DRY	No.2
K - I	2x6	DRY	No.2
S - Q	2x6	DRY	No.2
Q - M	2x6	DRY	No.2
M - K	2x6	DRY	No.2

ALL WEBS	2x4	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12	TOP
C - F	12	SIDE(61.0)
F - H	12	TOP
H - J	12	TOP
S - B	12	TOP
K - I	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
S - Q	12	TOP
Q - M	12	SIDE(183.1)
M - K	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x4	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
S	3028	0	DOWN	5-8	5-8
K	2965	0	UP	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
S	2145	1389 / 0	0 / 0	0 / 0	756 / 0
K	2100	1361 / 0	0 / 0	0 / 0	739 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.44 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	FORCE	MEMB.	FORCE
FR-TO	MAX. FACTORED	FR-TO	MAX. FACTORED
A-B	0 / 28	B-R	0 / 3707
B-C	-4214 / 0	R-C	-538 / 0
C-D	-6190 / 0	C-P	0 / 3345
D-T	-7052 / 0	P-D	-1219 / 0
T-E	-7052 / 0	D-O	0 / 1148
E-U	-7052 / 0	O-E	-652 / 0
U-F	-7052 / 0	F-G	0 / 1421
F-G	-7052 / 0	G-H	-1405 / 0
G-H	-5985 / 0	H-I	0 / 3179
H-I	-4121 / 0	I-J	-473 / 0
I-J	0 / 28	J-K	0 / 3626
S-B	-2959 / 0		
K-I	-2902 / 0		

LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
JT	15-7-12	-71	-71	---	BACK	VERT	---	C1
E	15-7-12	-21	-21	---	BACK	VERT	---	C1
O	15-7-12	-71	-71	---	BACK	VERT	---	C1
T	16-8-4	-71	-71	---	BACK	VERT	---	C1
U	12-6-8	-906	-906	---	BACK	VERT	---	C1
W	14-5-12	-21	-21	---	BACK	VERT	---	C1
X	16-8-4	-21	-21	---	BACK	VERT	---	C1
Y	18-7-8	-629	-629	---	BACK	VERT	---	C1

SPECIFIED CONCENTRATED LOADS (LBS)

LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
JT	15-7-12	-71	-71	---	BACK	VERT	---	C1
E	15-7-12	-21	-21	---	BACK	VERT	---	C1
O	15-7-12	-71	-71	---	BACK	VERT	---	C1
T	16-8-4	-71	-71	---	BACK	VERT	---	C1
U	12-6-8	-906	-906	---	BACK	VERT	---	C1
W	14-5-12	-21	-21	---	BACK	VERT	---	C1
X	16-8-4	-21	-21	---	BACK	VERT	---	C1
Y	18-7-8	-629	-629	---	BACK	VERT	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 160 = 320 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 23.3	PSF
DL	= 6.0	PSF
BOT CH. LL	= 0.0	PSF
DL	= 7.4	PSF
TOTAL LOAD	= 36.7	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/999 (0.16")
ALLOWABLE DEFL.(TL) = L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/999 (0.30")

CSI: TC=0.44/1.00 (B-C:1), BC=0.79/1.00 (O-P:1), WB=0.33/1.00 (B-R:1), SSI=0.27/1.00 (O-P:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
650	371	1747
788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.88 (C) (INPUT = 0.90)

JSI METAL = 0.44 (Q) (INPUT = 1.00)



Structural component only
DWG# T-2126376

JOB NAME 420299	TRUSS NAME T30Z	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed Aug 4 10:52:48 2021 Page 2		ID: oUu4oI0DmAZdCEGz17gXJEvyT7V-29LrMfOCUrBzbKL6ih2lUymbAZMHOU5eX 8?J2yrGGz	

PLATES (table is in inches)

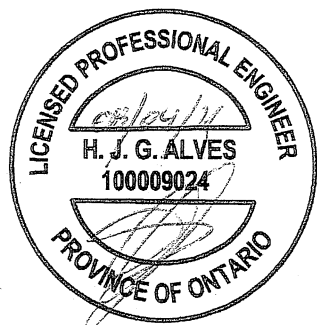
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.75	3.00
C	TTWW-m	MT20	6.0	9.0	2.00	3.75
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW-m	MT20	6.0	9.0	2.00	3.75
I	TMVW-p	MT20	5.0	6.0	1.75	3.00
K	BMV1+p	MT20	3.0	6.0		
L, N, P, R						
L	BMWW-t	MT20	5.0	6.0		
M	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	8.0		
Q	BS-t	MT20	5.0	6.0		
S	BMV1+p	MT20	3.0	6.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

CONNECTION REQUIREMENTS

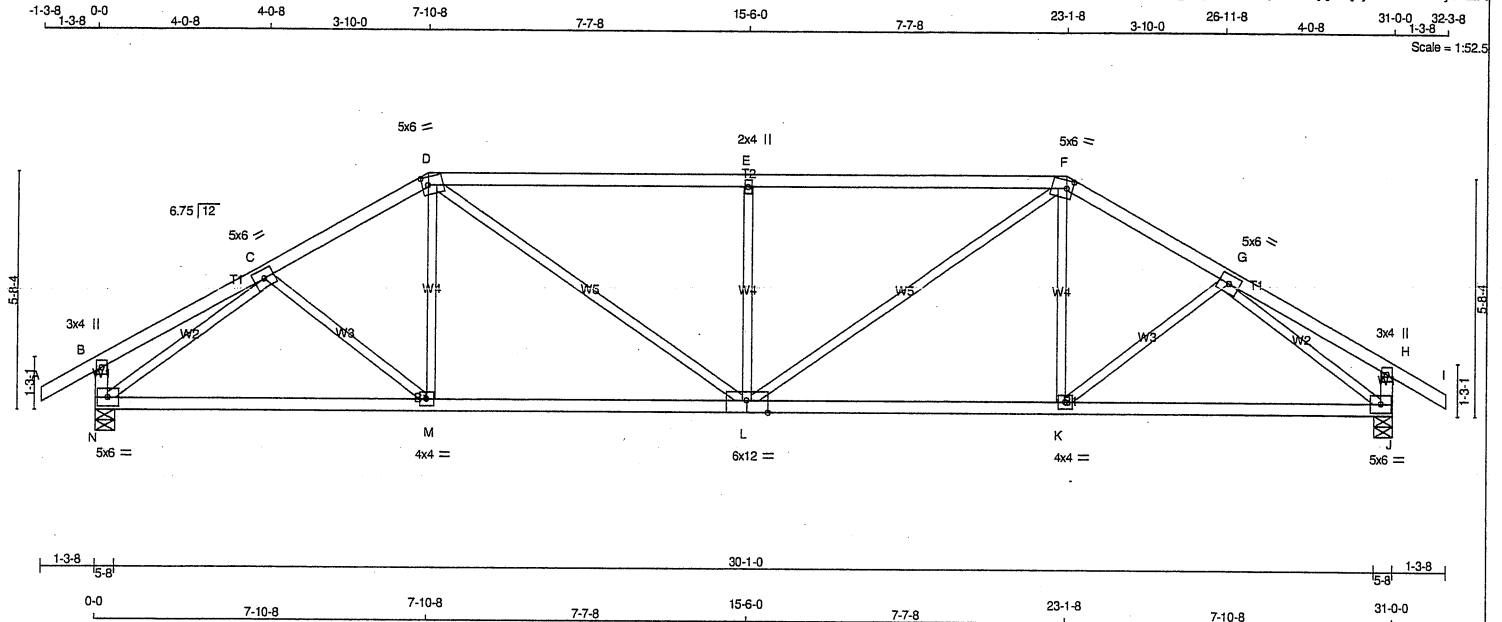
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2126376

JOB NAME 420298	TRUSS NAME T31	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 29 15:59:34 2021 Page 1
ID:0Uu4o10DmAZdCEGz17gXJEyyT7V-iWUaMSWngDlpzuFS0ql2KX4yyClyyu4ncWPSytALN



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
N - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
N - L	2x4	DRY	No.2
L - J	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
C	TMVW-t	MT20	5.0	6.0	
D	TTWW-m	MT20	5.0	6.0	2.25 1.75
E	TMVW-w	MT20	2.0	4.0	
F	TTWW-m	MT20	5.0	6.0	2.25 1.75
G	TMVW-t	MT20	5.0	6.0	
H	TMV+p	MT20	3.0	4.0	
J	BMVW-1-t	MT20	5.0	6.0	
K	BMVW-1-t	MT20	4.0	4.0	
L	BSWWW-1	MT20	6.0	12.0	Edge 6.00
M	BMVW-1-t	MT20	4.0	4.0	
N	BMVW-1-t	MT20	5.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
N	1718	0	1718	0	0
J	1718	0	1718	0	0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. SNOW	MAX/MIN. LIVE	MAX/MIN. PERM. LIVE	WIND	DEAD	SOIL
N	1217	785 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0
J	1217	785 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.45 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-84.9 -84.9	0.11 (1)	10.00	C-M	0 / 63	0.02 (4)
B-C	0 / 17	-84.9 -84.9	0.19 (1)	10.00	M-D	0 / 169	0.06 (4)
C-D	-2051 / 0	-84.9 -84.9	0.20 (1)	4.57	D-L	0 / 782	0.18 (1)
D-E	-2413 / 0	-84.9 -84.9	0.80 (1)	3.45	L-E	-797 / 0	0.39 (1)
E-F	-2413 / 0	-84.9 -84.9	0.80 (1)	3.45	L-F	0 / 782	0.18 (1)
F-G	-2051 / 0	-84.9 -84.9	0.20 (1)	4.57	K-F	0 / 169	0.06 (4)
G-H	0 / 17	-84.9 -84.9	0.19 (1)	10.00	K-G	0 / 63	0.02 (4)
H-I	0 / 28	-84.9 -84.9	0.11 (1)	10.00	N-C	-2257 / 0	0.93 (1)
N-B	-249 / 0	0.0 0.0	0.03 (1)	7.81	G-J	-2257 / 0	0.93 (1)
J-H	-249 / 0	0.0 0.0	0.03 (1)	7.81			
N-M	0 / 1761	-18.5 -18.5	0.47 (4)	10.00			
M-L	0 / 1774	-18.5 -18.5	0.48 (4)	10.00			
L-K	0 / 1774	-18.5 -18.5	0.48 (4)	10.00			
K-J	0 / 1761	-18.5 -18.5	0.47 (4)	10.00			

TOTAL WEIGHT = 2 X 121 = 243 lb [M/F]

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
BOT CH. LL = 6.0 PSF
DL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/999 (0.25")

CSI: TC=0.80/1.00 (D-E:1) , BC=0.48/1.00 (L-M:4) , WB=0.93/1.00 (C-N:1) , SSI=0.31/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

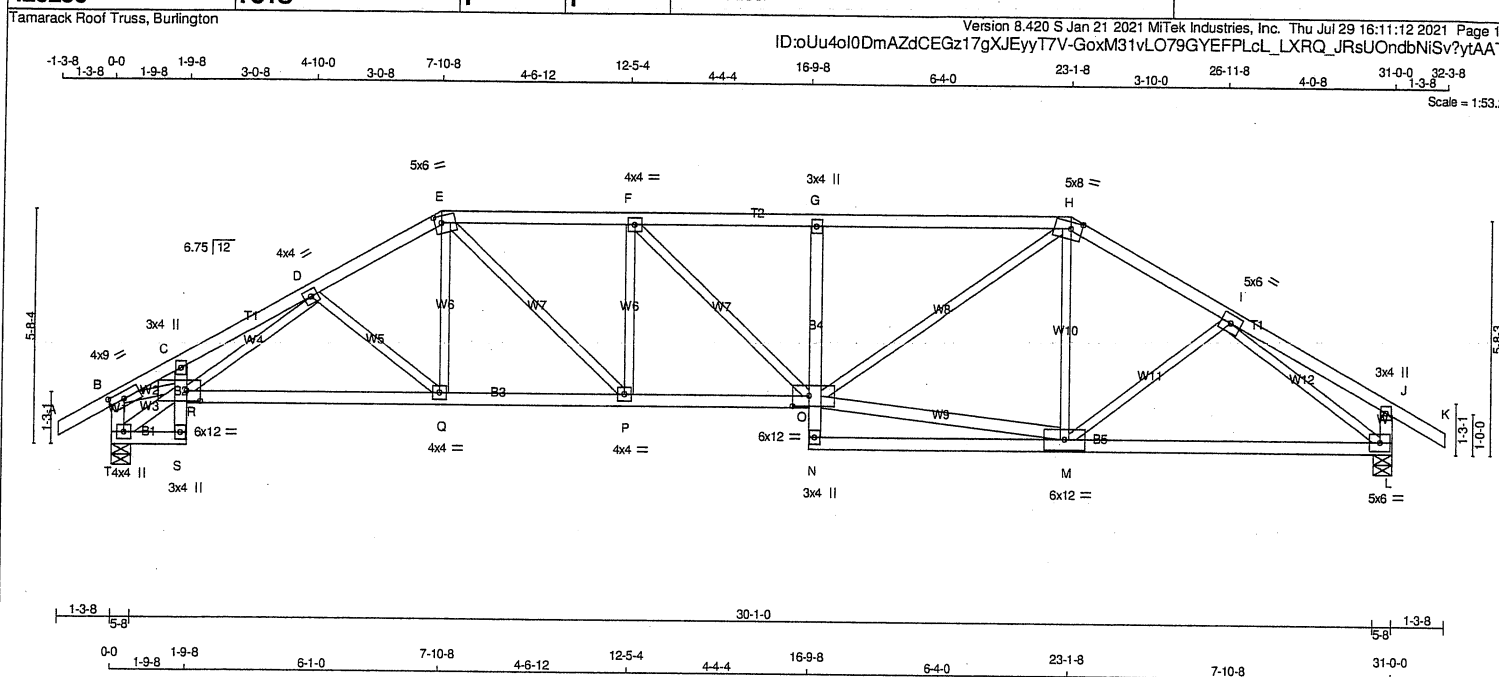
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (J) (INPUT = 0.90)
JSI METAL= 0.51 (C) (INPUT = 1.00)



Structural component only
DWG# T-2125237



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - E	2x4	DRY	No.2
E - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
T - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
T - S	2x4	DRY	No.2
S - C	2x4	DRY	No.2
R - O	2x4	DRY	No.2
N - G	2x4	DRY	No.2
N - L	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
O - M	2x4	DRY	No.2
T - R	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	9.0	2.00	4.00
C, G, J						
D	TMVW-t	MT20	3.0	4.0		
E	TTWW-m	MT20	4.0	4.0		
F	TMVW-t	MT20	5.0	6.0	2.00	2.00
H	TTWW-m	MT20	4.0	4.0		
I	TMVW-t	MT20	5.0	8.0	2.00	3.25
L	BMVW-t	MT20	5.0	6.0		
M	BMVW-t	MT20	5.0	6.0		
N	BMVW-t	MT20	6.0	12.0		
O	BMVW-t	MT20	3.0	4.0		
P	BMVW-t	MT20	6.0	12.0	3.00	4.75
Q	BMVW-t	MT20	4.0	4.0		
R	BMVW-t	MT20	4.0	4.0		
S	BMVW-t	MT20	6.0	12.0	3.00	3.75
T	BMVW-t	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	IN-SX	IN-SX
L	1715	0	5-8	5-8
T	1720	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
L	1215	784 / 0	0 / 0	0 / 0	0 / 0	431 / 0	0 / 0
T	1219	787 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, T

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.64 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO				FR-TO			
A-B	0 / 28	-84.9	-84.9 0.11 (1)	10.00	Q-E	0 / 374	0.08 (1)
B-C	-2919 / 0	-84.9	-84.9 0.18 (1)	3.95	E-P	0 / 818	0.18 (1)
C-D	-3044 / 0	-84.9	-84.9 0.26 (1)	3.81	P-F	-490 / 0	0.16 (1)
D-E	-2476 / 0	-84.9	-84.9 0.18 (1)	4.24	F-O	0 / 164	0.04 (1)
E-F	-2739 / 0	-84.9	-84.9 0.31 (1)	3.94	O-M	0 / 1714	0.28 (1)
F-G	-2856 / 0	-84.9	-84.9 0.39 (1)	3.77	M-H	0 / 1317	0.30 (1)
G-H	-2839 / 0	-84.9	-84.9 0.53 (1)	3.64	H-I	-119 / 42	0.06 (1)
H-I	-2045 / 0	-84.9	-84.9 0.20 (1)	4.58	I-L	0 / 62	0.02 (4)
I-J	0 / 17	-84.9	-84.9 0.19 (1)	10.00	L-K	-2251 / 0	0.93 (1)
J-K	0 / 28	-84.9	-84.9 0.11 (1)	10.00	K-R	0 / 291	0.07 (1)
T-B	-1608 / 0	0.0	0.0 0.16 (1)	6.52	D-Q	-408 / 0	0.12 (1)
L-J	-248 / 0	0.0	0.0 0.03 (1)	7.81	T-R	-188 / 0	0.02 (1)
					B-R	0 / 2500	0.56 (1)
T-S	0 / 160	-18.5	-18.5 0.03 (1)	10.00			
S-R	0 / 17	0.0	0.0 0.22 (1)	10.00			
R-C	-116 / 0	0.0	0.0 0.17 (1)	7.81			
R-Q	0 / 2458	-18.5	-18.5 0.48 (1)	10.00			
Q-P	0 / 2149	-18.5	-18.5 0.43 (1)	10.00			
P-O	0 / 2739	-18.5	-18.5 0.49 (1)	10.00			
N-O	0 / 43	0.0	0.0 0.11 (1)	10.00			
O-G	-524 / 0	0.0	0.0 0.13 (1)	7.81			
N-M	0 / 73	-18.5	-18.5 0.28 (4)	10.00			
M-L	0 / 1757	-18.5	-18.5 0.43 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.30")

CSI: TC=0.53/1.00 (G-H:1), BC=0.49/1.00 (O-P:1), WB=0.93/1.00 (L-L:1), SSI=0.24/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (E) (INPUT = 0.90)
JSI METAL= 0.51 (I) (INPUT = 1.00)

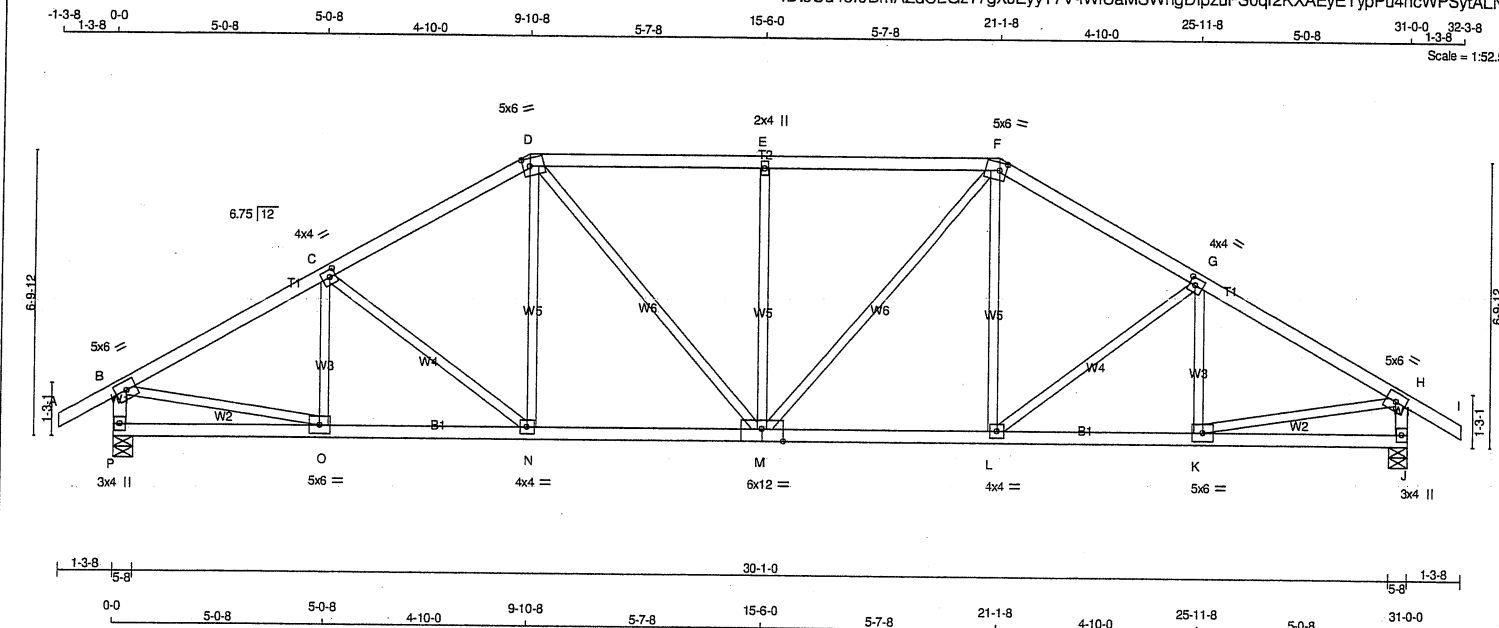


Structural component only
DWG# T-2125250

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
420298	T32	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 29 15:59:34 2021 Page 1
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TOTAL WEIGHT = 2 X 131 = 262 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - I	2x4	DRY	No.2	SPF	
P - B	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
P - M	2x4	DRY	No.2	SPF	
M - J	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0		
C	TMVW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMVW-w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMVW-t	MT20	4.0	4.0	2.00	1.75
H	TMVW-t	MT20	5.0	6.0		
J	BMV1-p	MT20	3.0	4.0		
K	BMVW-t	MT20	5.0	6.0		
L	BMVW-t	MT20	4.0	4.0		
M	BSWWW-t	MT20	6.0	12.0	Edge	6.00
N	BMVW-t	MT20	4.0	4.0		
O	BMVW-t	MT20	5.0	6.0		
P	BMV1-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	UP/LIFT	IN-SX
P	1718	0	1718	0
J	1718	0	1718	0

UNFACTORED REACTIONS

1ST CASE	MAX./MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND
P	1217	785 / 0	0 / 0	0 / 0	432 / 0
J	1217	785 / 0	0 / 0	0 / 0	432 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.39 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)
FR-TO					FR-TO			
A-B	0 / 28	-84.9	-84.9	0.11 (1)	10.00	O-C	-257 / 0	0.07 (1)
B-C	-2110 / 0	-84.9	-84.9	0.32 (1)	4.39	C-N	-249 / 0	0.17 (1)
C-D	-1933 / 0	-84.9	-84.9	0.31 (1)	4.56	N-D	0 / 264	0.06 (1)
D-E	-1954 / 0	-84.9	-84.9	0.40 (1)	4.42	D-M	0 / 441	0.10 (1)
E-F	-1954 / 0	-84.9	-84.9	0.40 (1)	4.42	M-E	-584 / 0	0.45 (1)
F-G	-1933 / 0	-84.9	-84.9	0.31 (1)	4.56	M-F	0 / 441	0.10 (1)
G-H	-2110 / 0	-84.9	-84.9	0.32 (1)	4.39	L-F	0 / 264	0.06 (1)
H-I	0 / 28	-84.9	-84.9	0.11 (1)	10.00	L-G	-249 / 0	0.17 (1)
P-B	-1676 / 0	0.0	0.0	0.17 (1)	6.41	K-G	-257 / 0	0.07 (1)
J-H	-1676 / 0	0.0	0.0	0.17 (1)	6.41	B-O	0 / 1891	0.43 (1)
P-O	0 / 0	-18.5	-18.5	0.10 (4)	10.00	K-H	0 / 1891	0.43 (1)
O-N	0 / 1859	-18.5	-18.5	0.37 (1)	10.00			
N-M	0 / 1666	-18.5	-18.5	0.34 (1)	10.00			
M-L	0 / 1666	-18.5	-18.5	0.34 (1)	10.00			
L-K	0 / 1859	-18.5	-18.5	0.37 (1)	10.00			
K-J	0 / 0	-18.5	-18.5	0.10 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/999 (0.09")
ALLOWABLE DEFL.(TL) = L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/999 (0.18")

CSI: TC=0.40/1.00 (D-E:1), BC=0.37/1.00 (N-O:1), WB=0.45/1.00 (E-M:1), SS=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

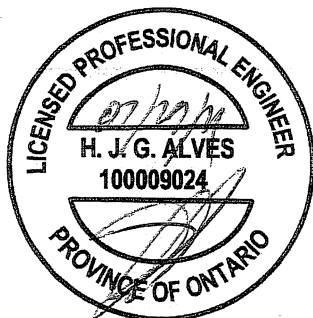
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (O) (INPUT = 0.90)
JSI METAL= 0.52 (B) (INPUT = 1.00)

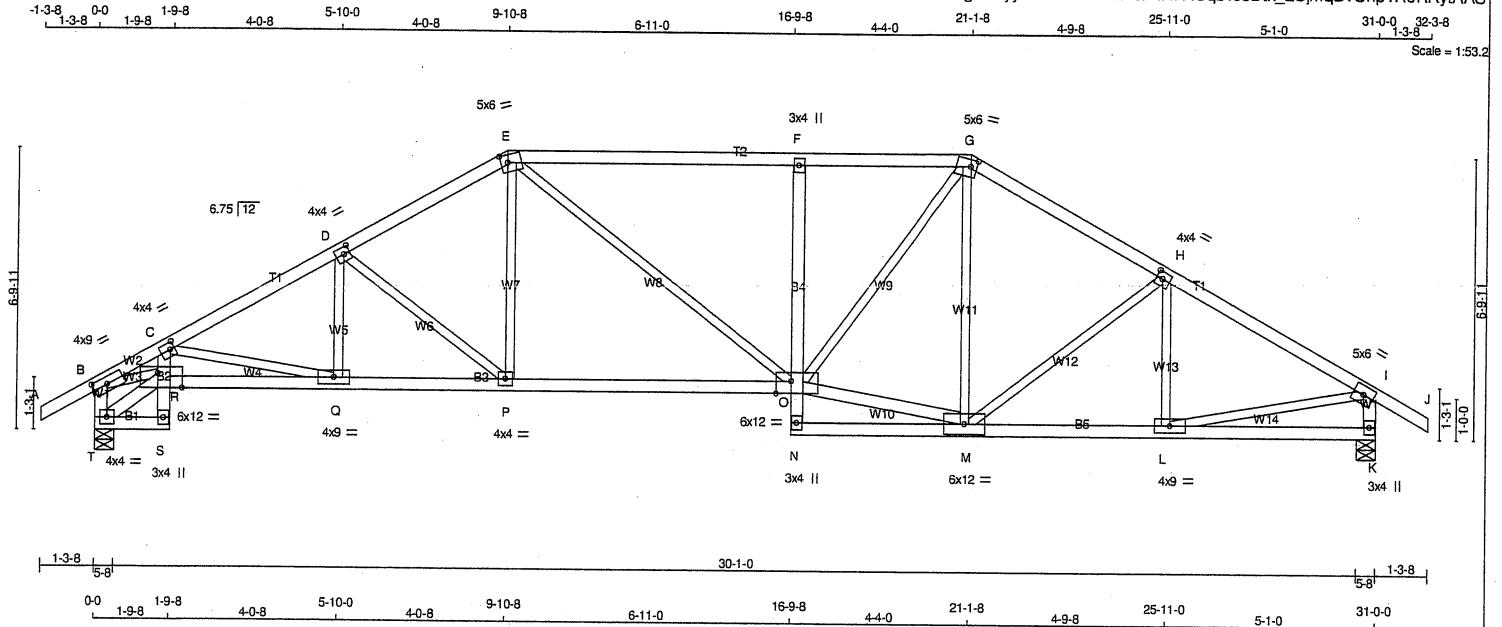


Structural component only
DWG# T-2125238

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
420299	T32S	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 29 16:11:13 2021 Page 1
ID:0Uu40l0DmAZdCEGz17gXJeyT7V-k?UkHnWz9RH7AOqbVJsDtk_aUjmqDvCnp1R0RRyAAS



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
G - J	2x4	DRY	No.2	SPF	
T - B	2x4	DRY	No.2	SPF	
K - I	2x4	DRY	No.2	SPF	
T - S	2x4	DRY	No.2	SPF	
S - C	2x4	DRY	No.2	SPF	
R - O	2x4	DRY	No.2	SPF	
N - F	2x4	DRY	No.2	SPF	
N - K	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	
O - M	2x4	DRY	No.2	SPF	
T - R	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	9.0	2.00	4.00
C	TMVW-t	MT20	4.0	4.0	2.00	1.25
D	TMVW-t	MT20	4.0	4.0	2.00	1.75
E	TTWW-m	MT20	5.0	6.0	2.25	2.00
F	TMV+p	MT20	3.0	4.0		
G	TTWW-m	MT20	5.0	6.0	2.00	1.75
H	TMVW-t	MT20	4.0	4.0	2.00	1.75
I	TMVW-t	MT20	5.0	6.0		
K	BMV1+p	MT20	3.0	4.0		
L	BMVW-t	MT20	4.0	9.0		
M	BMVWW-t	MT20	6.0	12.0		
N	BMV+p	MT20	3.0	4.0		
O	BMVWW-t	MT20	6.0	12.0	Edge	4.25
P	BMVW-t	MT20	4.0	4.0		
Q	BMVW-t	MT20	4.0	9.0		
R	BMVWW-t	MT20	6.0	12.0	4.25	7.00
S	BMV+p	MT20	3.0	4.0		
T	BMVW-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES (1)
1)



Structural component only
DWG# T-2125251

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	IN-SX	IN-SX
K	1715	0	5-8	5-8
T	1720	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
K	COMBINED	SNOW	LIVE	PERM. LIVE	
T	1215	784 / 0	0 / 0	0 / 0	431 / 0
	1219	787 / 0	0 / 0	0 / 0	432 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, T

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.67 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. (LC)	MAX. UNBRACED LENGTH	WEBS	MAX. FACTORED FORCE (LBS)	MAX. (LC)
FR-TO					FR-TO		
A-B	0 / 28	-84.9	-84.9 0.11 (1)	10.00	P-E	0 / 472	0.11 (1)
B-C	-3395 / 0	-84.9	-84.9 0.20 (1)	3.67	E-O	0 / 418	0.09 (1)
C-D	-2712 / 0	-84.9	-84.9 0.31 (1)	3.96	O-M	0 / 1664	0.27 (1)
D-E	-2267 / 0	-84.9	-84.9 0.26 (1)	4.33	O-G	0 / 1038	0.23 (1)
E-F	-2293 / 0	-84.9	-84.9 0.61 (1)	3.87	M-G	-135 / 12	0.11 (1)
F-G	-2286 / 0	-84.9	-84.9 0.46 (1)	4.03	T-R	-242 / 0	0.03 (1)
G-H	-1929 / 0	-84.9	-84.9 0.31 (1)	4.56	B-R	0 / 2940	0.66 (1)
H-I	-2108 / 0	-84.9	-84.9 0.33 (1)	4.39	Q-D	0 / 123	0.03 (4)
I-J	0 / 28	-84.9	-84.9 0.11 (1)	10.00	D-P	-552 / 0	0.25 (1)
T-B	-1567 / 0	0.0	0.0 0.16 (1)	6.58	C-Q	-859 / 0	0.26 (1)
K-I	-1673 / 0	0.0	0.0 0.17 (1)	6.42	L-H	-251 / 0	0.06 (1)
					M-H	-253 / 0	0.17 (1)
T-S	0 / 199	-18.5	-18.5 0.04 (1)	10.00	L-I	0 / 1889	0.43 (1)
S-R	0 / 17	0.0	0.0 0.31 (1)	10.00			
R-C	0 / 381	0.0	0.0 0.37 (1)	10.00			
R-Q	0 / 3233	-18.5	-18.5 0.57 (1)	10.00			
Q-P	0 / 2390	-18.5	-18.5 0.49 (1)	10.00			
P-O	0 / 1963	-18.5	-18.5 0.42 (1)	10.00			
N-O	0 / 33	0.0	0.0 0.06 (1)	10.00			
O-F	-613 / 0	0.0	0.0 0.24 (1)	7.81			
N-M	0 / 40	-18.5	-18.5 0.09 (4)	10.00			
M-L	0 / 1857	-18.5	-18.5 0.35 (1)	10.00			
L-K	0 / 0	-18.5	-18.5 0.10 (4)	10.00			

TOTAL WEIGHT = 139 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	23.3	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.4	PSF
TOTAL LOAD	=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.30")

CSI: TC=0.61/1.00 (E-F:1), BC=0.57/1.00 (Q-R:1), WB=0.66/1.00 (B-R:1), SS=0.28/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

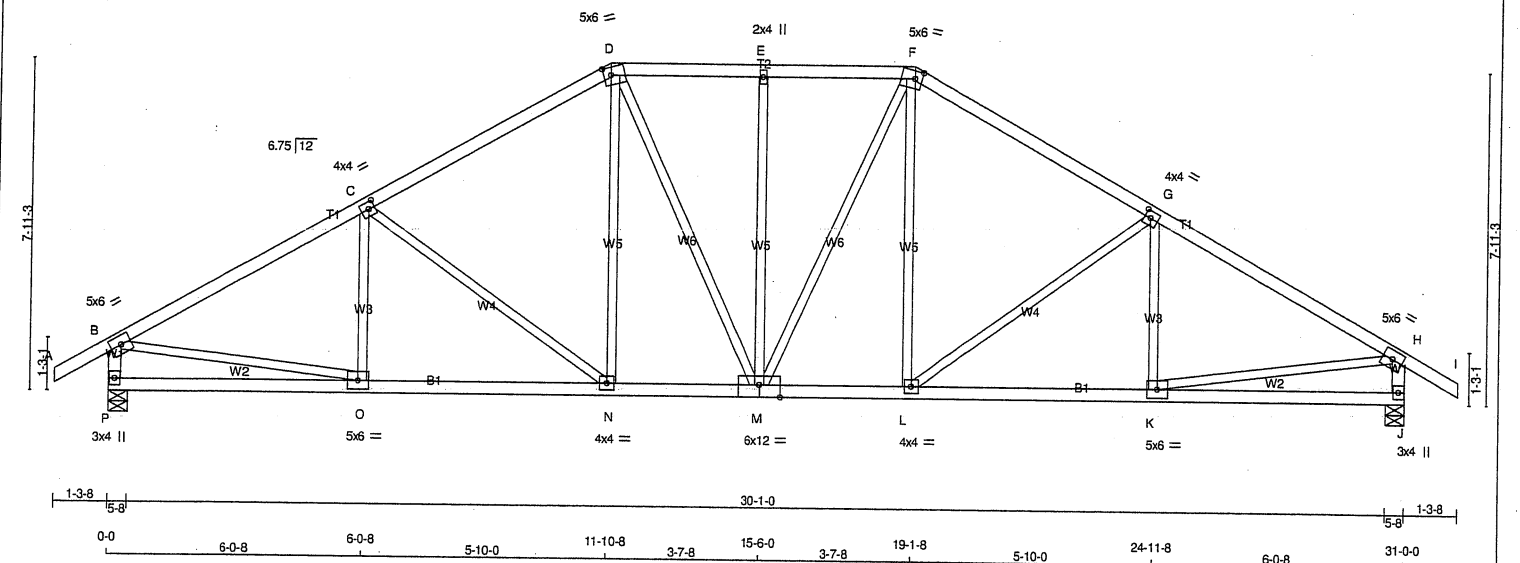
PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (T) (INPUT = 0.90)

JSI METAL= 0.55 (B) (INPUT = 1.00)



LUMBER
 N. L. G. A. RULES
 CHORDS SIZE LUMBER DESCR.

A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-t	MT20	5.0	6.0		
C	TMW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMW-w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMW-t	MT20	4.0	4.0	2.00	1.75
H	TMW-t	MT20	5.0	6.0		
J	BMV1-p	MT20	3.0	4.0		
K	BMW-t	MT20	5.0	6.0		
L	BMW-t	MT20	4.0	4.0		
M	BSWW-t	MT20	6.0	12.0	Edge	6.00
N	BMW-t	MT20	4.0	4.0		
O	BMW-t	MT20	5.0	6.0		
P	BMV1-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
 1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
P	1718	0	1718	0
J	1718	0	1718	0

UNFACTORED REACTIONS

	1ST LCASE	MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1217	785 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0
J	1217	785 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.19 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-84.9 -84.9	0.11 (1)	10.00	O-C	-173 / 35	0.06 (1)
B-C	-2139 / 0	-84.9 -84.9	0.48 (1)	4.19	C-N	-434 / 0	0.46 (1)
C-D	-1797 / 0	-84.9 -84.9	0.44 (1)	4.53	N-D	0 / 353	0.08 (1)
D-E	-1641 / 0	-84.9 -84.9	0.16 (1)	5.04	D-M	0 / 226	0.05 (1)
E-F	-1641 / 0	-84.9 -84.9	0.16 (1)	5.04	M-E	-366 / 0	0.44 (1)
F-G	-1797 / 0	-84.9 -84.9	0.44 (1)	4.53	L-F	0 / 353	0.08 (1)
G-H	-2139 / 0	-84.9 -84.9	0.48 (1)	10.00	L-G	-434 / 0	0.46 (1)
H-I	0 / 28	-84.9 -84.9	0.11 (1)	6.42	K-G	-173 / 35	0.06 (1)
P-B	-1671 / 0	0.0 0.0	0.17 (1)	6.42	B-O	0 / 1912	0.43 (1)
J-H	-1671 / 0	0.0 0.0	0.17 (1)	6.42	K-H	0 / 1912	0.43 (1)
P-O	0 / 0	-18.5 -18.5	0.16 (4)	10.00			
O-N	0 / 1889	-18.5 -18.5	0.38 (1)	10.00			
N-M	0 / 1544	-18.5 -18.5	0.30 (1)	10.00			
M-L	0 / 1544	-18.5 -18.5	0.30 (1)	10.00			
L-K	0 / 1889	-18.5 -18.5	0.38 (1)	10.00			
K-J	0 / 0	-18.5 -18.5	0.16 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 23.3 PSF
 DL = 6.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF CBC 2018, ABC 2019
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 086-14
 - TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
 CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
 ALLOWABLE DEFL.(TL)= L/360 (1.03")
 CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.48/1.00 (B-C:1) , BC=0.38/1.00 (N-O:1) , WB=0.46/1.00 (C-N:1) , SSI=0.21/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

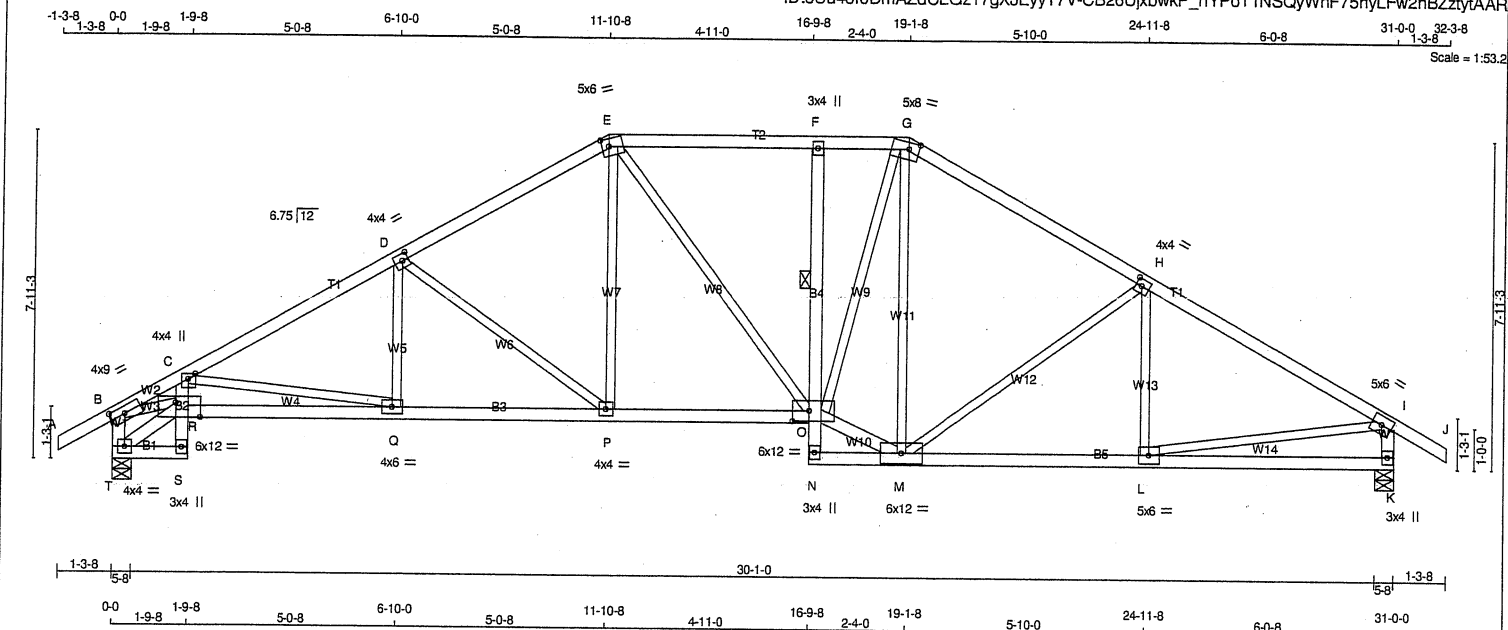
JSI GRIP= 0.82 (K) (INPUT = 0.90)
 JSI METAL= 0.53 (B) (INPUT = 1.00)



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
420299	T33S	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 29 16:11:14 2021 Page 1
ID:0Uu4o10DmAZdCEGz17gXJEyyT7V-CB26UjxbwkP_nYPot1NSQyWnF75nyLFw2hBZtytAAR



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
G - J	2x4	DRY	No.2	SPF	
T - B	2x4	DRY	No.2	SPF	
K - I	2x4	DRY	No.2	SPF	
T - S	2x4	DRY	No.2	SPF	
S - C	2x4	DRY	No.2	SPF	
R - O	2x4	DRY	No.2	SPF	
N - F	2x4	DRY	No.2	SPF	
N - K	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	
O - M	2x4	DRY	No.2	SPF	
T - R	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	9.0	2.00	4.00
C	TMVW+p	MT20	4.0	4.0	1.50	2.00
D	TMVW-t	MT20	4.0	4.0	2.00	1.75
E	TTWW-m	MT20	5.0	6.0	2.25	2.00
F	TMV+p	MT20	3.0	4.0		
G	TTWW-m	MT20	5.0	8.0	2.00	3.00
H	TMVW-t	MT20	4.0	4.0	2.00	1.75
I	TMVW-t	MT20	5.0	6.0		
K	BMV1+p	MT20	3.0	4.0		
L	BMVW-t	MT20	5.0	6.0		
M	BMVW-t	MT20	6.0	12.0		
N	BMV+p	MT20	3.0	4.0		
O	BMVW-t	MT20	6.0	12.0	3.00	4.75
P	BMVW-t	MT20	4.0	4.0		
Q	BMVW-t	MT20	4.0	6.0		
R	BMVW-t	MT20	6.0	12.0	4.25	7.00
S	BMV+p	MT20	3.0	4.0		
T	BMVW-t	MT20	4.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
JT	1715	0	1715	0	5-8	5-8
K	1715	0	1715	0	5-8	5-8
T	1720	0	1720	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. SNOW	MAX/MIN. LIVE	MAX/MIN. PERM. LIVE	MAX/MIN. WIND	MAX/MIN. DEAD	MAX/MIN. SOIL
JT	1215	784/0	0/0	0/0	431/0	0/0
K	1215	784/0	0/0	0/0	431/0	0/0
T	1219	787/0	0/0	0/0	432/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, T

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.64 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FORCE (LBS)	FACTORED MAX CSI (LC)		
FR-TO		FROM	TO		FR-TO				
A-B	0/28	-84.9	-84.9	0.11 (1)	10.00	C-Q	-988/0 0.46 (1)		
B-C	-3447/0	-84.9	-84.9	0.20 (1)	3.64	Q-D	0/173 0.05 (4)		
C-D	-2631/0	-84.9	-84.9	0.41 (1)	3.92	D-P	-691/0 0.51 (1)		
D-E	-2050/0	-84.9	-84.9	0.35 (1)	4.40	P-E	0/514 0.12 (1)		
E-F	-1874/0	-84.9	-84.9	0.30 (1)	4.62	E-O	0/172 0.04 (1)		
F-G	-1869/0	-84.9	-84.9	0.22 (1)	4.71	O-M	0/1663 0.27 (1)		
G-H	-1795/0	-84.9	-84.9	0.44 (1)	4.53	O-G	0/1076 0.24 (1)		
H-I	-2134/0	-84.9	-84.9	0.48 (1)	4.19	M-G	-362/0 0.43 (1)		
I-J	0/28	-84.9	-84.9	0.11 (1)	10.00	M-H	-431/0 0.46 (1)		
T-B	-1565/0	0.0	0.0	0.16 (1)	6.59	L-H	-177/34 0.06 (1)		
K-I	-1667/0	0.0	0.0	0.17 (1)	6.42	L-I	0/1908 0.43 (1)		
						T-R	-246/0 0.03 (1)		
						B-R	0/2997 0.67 (1)		
T-S	0/202	-18.5	-18.5	0.04 (1)	10.00				
S-R	0/17	0.0	0.0	0.32 (1)	10.00				
R-C	0/370	0.0	0.0	0.37 (1)	10.00				
R-Q	0/3294	-18.5	-18.5	0.59 (1)	10.00				
Q-P	0/2319	-18.5	-18.5	0.43 (1)	10.00				
P-O	0/1770	-18.5	-18.5	0.34 (1)	10.00				
N-O	-8/0	0.0	0.0	0.03 (1)	10.00				
O-F	-411/0	0.0	0.0	0.05 (1)	6.25				
N-M	0/33	-18.5	-18.5	0.10 (4)	10.00				
M-L	0/1885	-18.5	-18.5	0.38 (1)	10.00				
L-K	0/0	-18.5	-18.5	0.15 (4)	10.00				

TOTAL WEIGHT = 144 lb

[M][F]

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/999 (0.12")
ALLOWABLE DEFL.(TL) = L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/999 (0.24")

CSI: TC=0.48/1.00 (H-I:1), BC=0.59/1.00 (Q-R:1), WB=0.67/1.00 (B-R:1), SS=0.25/1.00 (C-R:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

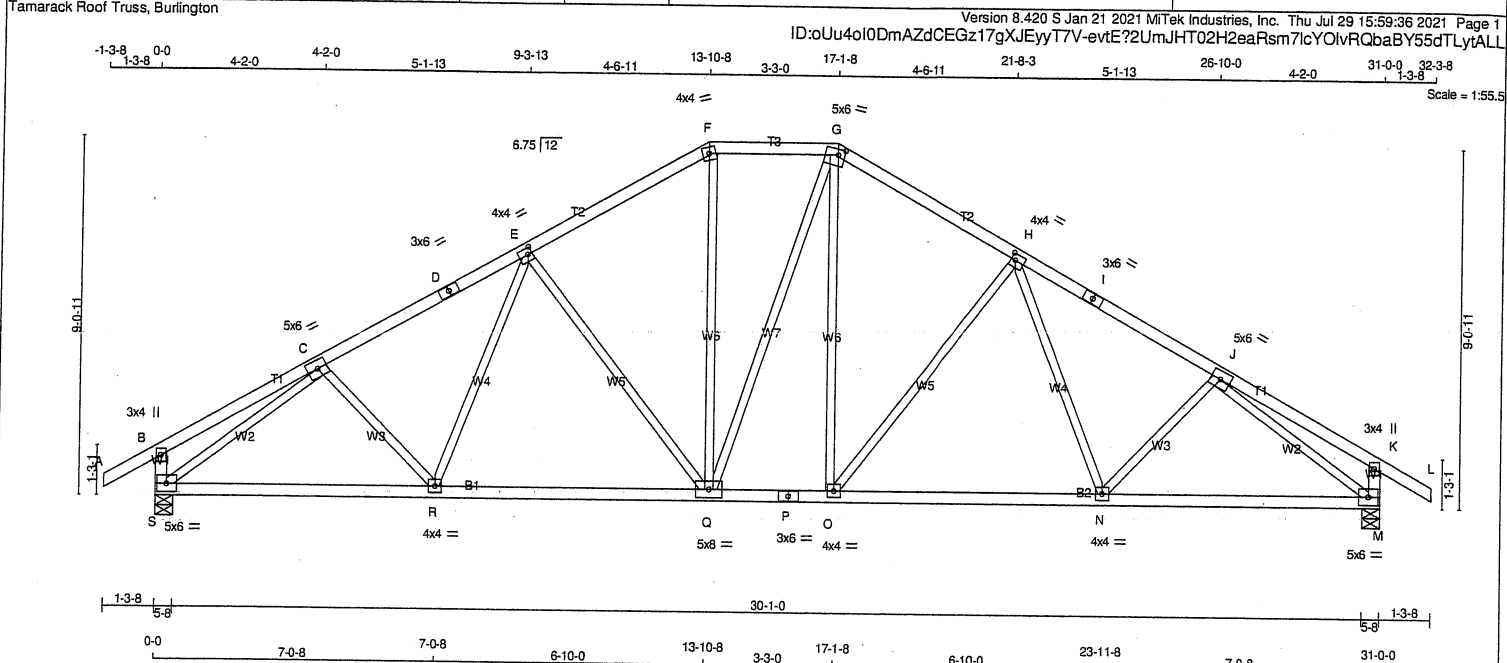
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (T) (INPUT = 0.90)
JSI METAL = 0.56 (B) (INPUT = 1.00)



Structural component only
DWG# T-2125252

JOB NAME 420298	TRUSS NAME T34	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - L	2x4	DRY	No.2
S - B	2x4	DRY	No.2
M - K	2x4	DRY	No.2
S - P	2x4	DRY	No.2
P - M	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DESCR. SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
C	TMWW-t	MT20	5.0	6.0	
D	TS-t	MT20	3.0	6.0	
E	TMWW-t	MT20	4.0	4.0	2.00 1.25
F	TTW-m	MT20	4.0	4.0	
G	TTWW-m	MT20	5.0	6.0	1.75 2.00
H	TMWW-t	MT20	4.0	4.0	2.00 1.25
I	TS-t	MT20	3.0	6.0	
J	TMWW-t	MT20	5.0	6.0	
K	TMV+p	MT20	3.0	4.0	
M	BMWW-t	MT20	5.0	6.0	
N, O, R					
N	BMWW-t	MT20	4.0	4.0	
P	BS-t	MT20	3.0	6.0	
Q	BMWW-t	MT20	5.0	8.0	
S	BMWW-t	MT20	5.0	6.0	

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	HORZ	BRG	IN-SX	BRG	IN-SX
S	1718	0	1718	0	0	5-8	5-8	5-8	5-8
M	1718	0	1718	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS		1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL		
S	1217	785 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0		
M	1217	785 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.50 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

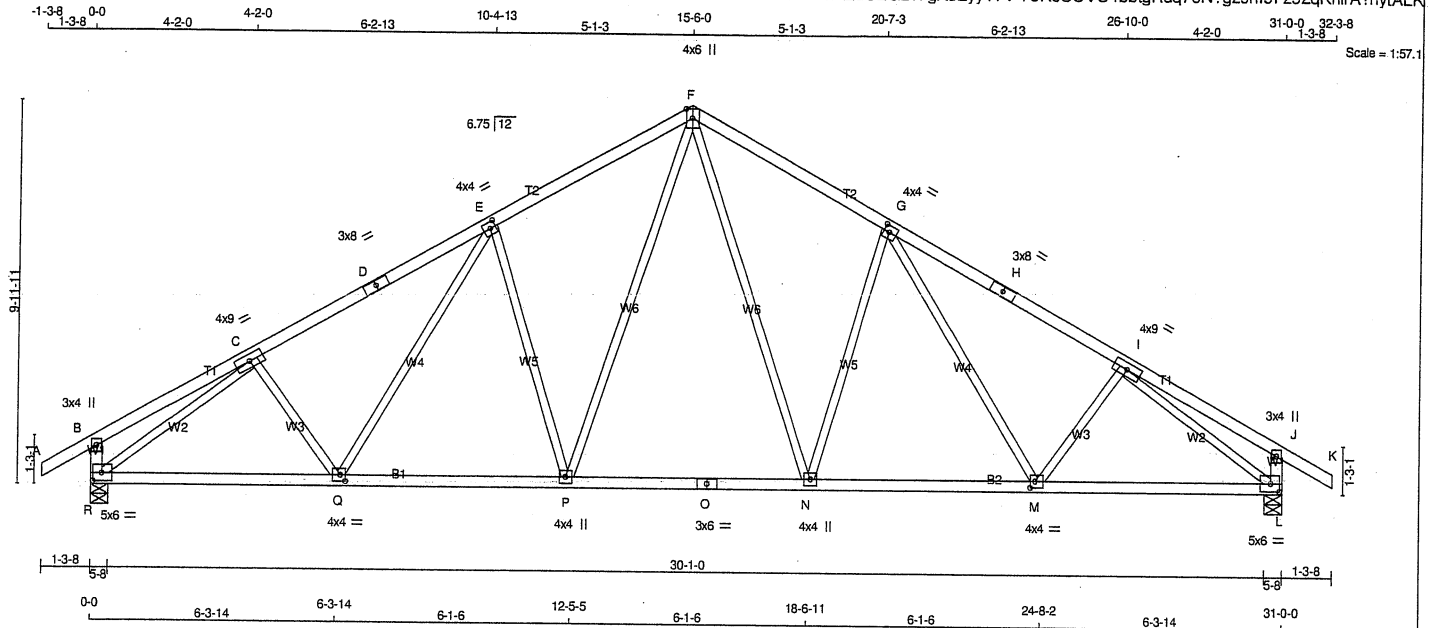
LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	FORCE (LBS)	FACTORED MAX CSI (LC)
FR-TO		FROM	TO		FR-TO		
A-B	0 / 28	-84.9	-84.9 0.11 (1)	10.00	C-R	-11 / 56	0.02 (4)
B-C	0 / 19	-84.9	-84.9 0.22 (1)	10.00	R-E	0 / 164	0.05 (4)
C-D	-2031 / 0	-84.9	-84.9 0.29 (1)	4.50	E-Q	-524 / 0	0.64 (1)
D-E	-2031 / 0	-84.9	-84.9 0.29 (1)	4.50	Q-F	0 / 506	0.11 (1)
E-F	-1633 / 0	-84.9	-84.9 0.26 (1)	4.94	Q-G	0 / 4	0.00 (1)
F-G	-1408 / 0	-84.9	-84.9 0.14 (1)	5.36	O-G	0 / 502	0.11 (1)
G-H	-1631 / 0	-84.9	-84.9 0.26 (1)	4.94	O-H	-526 / 0	0.64 (1)
H-I	-2032 / 0	-84.9	-84.9 0.29 (1)	4.50	H-N	0 / 166	0.05 (4)
I-J	-2032 / 0	-84.9	-84.9 0.29 (1)	4.50	N-J	-11 / 56	0.02 (4)
J-K	0 / 19	-84.9	-84.9 0.22 (1)	10.00	S-C	-2275 / 0	0.99 (1)
K-L	0 / 28	-84.9	-84.9 0.11 (1)	10.00	J-M	-2276 / 0	0.99 (1)
S-B	-249 / 0	0.0	0.0 0.03 (1)	7.81			
M-K	-249 / 0	0.0	0.0 0.03 (1)	7.81			
S-R	0 / 1781	-18.5	-18.5 0.40 (1)	10.00			
R-Q	0 / 1718	-18.5	-18.5 0.39 (1)	10.00			
Q-P	0 / 1407	-18.5	-18.5 0.29 (1)	10.00			
P-O	0 / 1407	-18.5	-18.5 0.29 (1)	10.00			
O-N	0 / 1717	-18.5	-18.5 0.39 (1)	10.00			
N-M	0 / 1782	-18.5	-18.5 0.40 (1)	10.00			

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
420298	T35	12	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MITek Industries, Inc. Thu Jul 29 15:59:37 2021 Page 1
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LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
R - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - L	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
SPF			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
C	TMWW-t	MT20	4.0	9.0	
D	TS-t	MT20	3.0	8.0	
E	TMWW-t	MT20	4.0	4.0	2.00 1.75
F	TTWW+p	MT20	4.0	6.0	Edge
G	TMWW-t	MT20	4.0	4.0	2.00 1.75
H	TS-t	MT20	3.0	8.0	
I	TMWW-t	MT20	4.0	9.0	
J	TMV+p	MT20	3.0	4.0	
L	BMVW1-t	MT20	5.0	6.0	2.50 2.75
M	BMWW-t	MT20	4.0	4.0	2.00 1.50
N	BMWW-t	MT20	4.0	4.0	
O	BS-t	MT20	3.0	6.0	
P	BMWW-t	MT20	4.0	4.0	
Q	BMWW-t	MT20	4.0	4.0	2.00 1.50
R	BMVW1-t	MT20	5.0	6.0	2.50 2.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	BRG	IN-SX
R	1718	0	1718	0	0	5-8	5-8		
L	1718	0	1718	0	0	5-8	5-8		

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1217	785 / 0	0 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0
L	1217	785 / 0	0 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.40 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		W E B S	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)
FR-TO		FROM TO		FR-TO	
A-B	0 / 28	-84.9	-84.9 0.11 (1)	F-N	0 / 717
B-C	0 / 26	-84.9	-84.9 0.29 (1)	N-G	-595 / 0
C-D	-2024 / 0	-84.9	-84.9 0.41 (1)	G-M	0 / 193
D-E	-2024 / 0	-84.9	-84.9 0.41 (1)	M-I	-54 / 47
E-F	-1749 / 0	-84.9	-84.9 0.36 (1)	P-F	0 / 717
F-G	-1749 / 0	-84.9	-84.9 0.41 (1)	E-P	-595 / 0
G-H	-2024 / 0	-84.9	-84.9 0.41 (1)	Q-E	0 / 193
H-I	-2024 / 0	-84.9	-84.9 0.41 (1)	C-Q	-54 / 47
I-J	0 / 26	-84.9	-84.9 0.29 (1)	R-C	-2300 / 0
J-K	0 / 28	-84.9	-84.9 0.11 (1)	I-L	-2300 / 0
R-B	-236 / 0	0.0	0.0 0.02 (1)		
L-J	-236 / 0	0.0	0.0 0.02 (1)		
R-Q	0 / 1801	-18.5	-18.5 0.38 (1)		
Q-P	0 / 1672	-18.5	-18.5 0.34 (1)		
P-O	0 / 1282	-18.5	-18.5 0.28 (1)		
O-N	0 / 1282	-18.5	-18.5 0.28 (1)		
N-M	0 / 1672	-18.5	-18.5 0.34 (1)		
M-L	0 / 1801	-18.5	-18.5 0.38 (1)		

TOTAL WEIGHT = 12 X 135 = 1617 lb [M/F]

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/999 (0.18")

CSI: TC=0.41/1.00 (G-I:1), BC=0.38/1.00 (Q-R:1),
WB=0.99/1.00 (C-R:1), SSI=0.19/1.00 (G-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

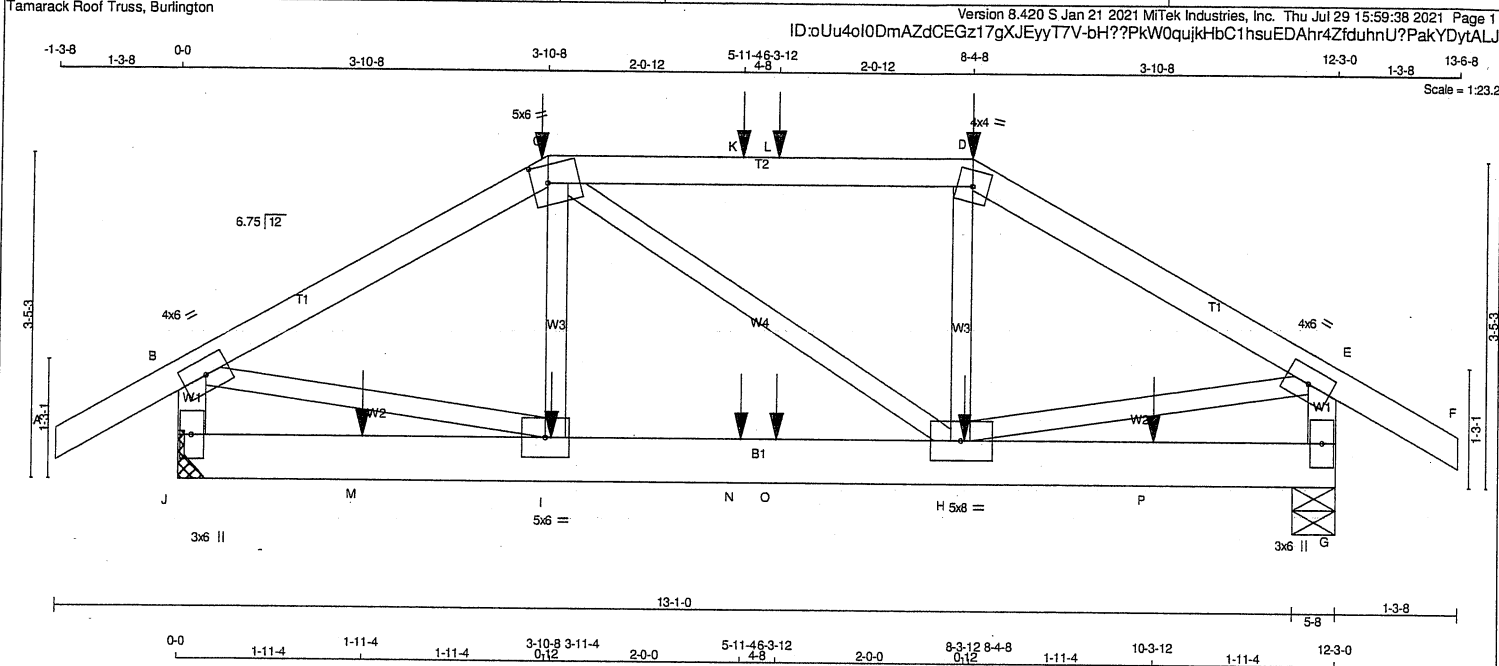
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (N) (INPUT = 0.90)
JSI METAL= 0.45 (O) (INPUT = 1.00)



Structural component only
DWG# T-2125241

JOB NAME 420298	TRUSS NAME T36	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2 SPF
C - D	2x4	DRY	No.2 SPF
D - F	2x4	DRY	No.2 SPF
J - B	2x4	DRY	No.2 SPF
G - E	2x4	DRY	No.2 SPF
J - G	2x6	DRY	No.2 SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0		
C	TTWW-m	MT20	5.0	6.0	2.25	2.00
D	TTW-m	MT20	4.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	6.0		
H	BMVWW-t	MT20	5.0	8.0		
I	BMVW-t	MT20	5.0	6.0		
J	BMV1+p	MT20	3.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REORD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
J	1051	0	1051	0	0
G	1051	0	1051	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 3-8.

UNFACTORED REACTIONS

JT	COMBINED	1ST CASE	MAX/MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
J	743	490 / 0	0 / 0	0 / 0	0 / 0	253 / 0	0 / 0
G	743	490 / 0	0 / 0	0 / 0	0 / 0	253 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.65 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	VERT. LOAD	MAX. FACTORED	MEMB.	FORCE (LBS)	MAX. FACTORED	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-84.9 -84.9 0.12 (1)	10.00	I-C	-95 / 55	0.02 (4)	
B-C	-1075 / 0	-84.9 -84.9 0.27 (1)	5.72	C-H	0 / 3	0.00 (4)	
C-K	-932 / 0	-84.9 -84.9 0.47 (1)	5.65	H-D	-94 / 57	0.02 (4)	
K-L	-932 / 0	-84.9 -84.9 0.47 (1)	5.65	B-I	0 / 958	0.24 (1)	
L-D	-932 / 0	-84.9 -84.9 0.47 (1)	5.65	H-E	0 / 959	0.24 (1)	
D-E	-1078 / 0	-84.9 -84.9 0.27 (1)	5.71				
E-F	0 / 28	-84.9 -84.9 0.12 (1)	10.00				
J-B	-1011 / 0	0.0 0.0 0.11 (1)	7.75				
G-E	-1008 / 0	0.0 0.0 0.11 (1)	7.77				
J-M	0 / 0	-18.5 -18.5 0.04 (4)	10.00				
M-I	0 / 0	-18.5 -18.5 0.04 (4)	10.00				
I-N	0 / 933	-18.5 -18.5 0.17 (1)	10.00				
N-O	0 / 933	-18.5 -18.5 0.17 (1)	10.00				
O-H	0 / 933	-18.5 -18.5 0.17 (1)	10.00				
H-P	0 / 0	-18.5 -18.5 0.05 (4)	10.00				
P-G	0 / 0	-18.5 -18.5 0.05 (4)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-10-8	-160	-160	---	BACK	VERT	TOTAL	---	C1
D	8-4-8	-160	-160	---	BACK	VERT	TOTAL	---	C1
H	8-3-12	-10	-10	---	BACK	VERT	TOTAL	---	C1
I	3-11-4	-10	-10	---	BACK	VERT	TOTAL	---	C1
K	5-11-4	-27	-27	---	BACK	VERT	TOTAL	---	C1
L	6-3-12	-27	-27	---	BACK	VERT	TOTAL	---	C1
M	1-11-4	-8	-8	---	BACK	VERT	TOTAL	---	C1
N	5-11-4	-10	-10	---	BACK	VERT	TOTAL	---	C1
O	6-3-12	-10	-10	---	BACK	VERT	TOTAL	---	C1
P	10-3-12	-8	-8	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.47/1.00 (C-D:1), BC=0.17/1.00 (H-I:1),
WB=0.24/1.00 (E-H:1), SSI=0.20/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

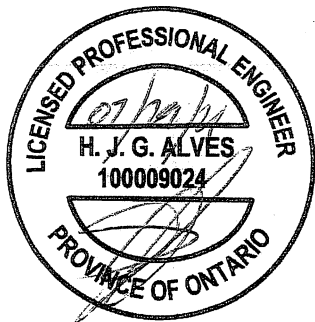
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.60 (D) (INPUT = 0.90)
JSI METAL= 0.28 (E) (INPUT = 1.00)



Structural component only
DWG# T-2125242



TOTAL WEIGHT = 48 lb

JSI GRIP= 0.76 (B) (INPUT = 0.90)
JSI METAL= 0.24 (D) (INPUT = 1.00)

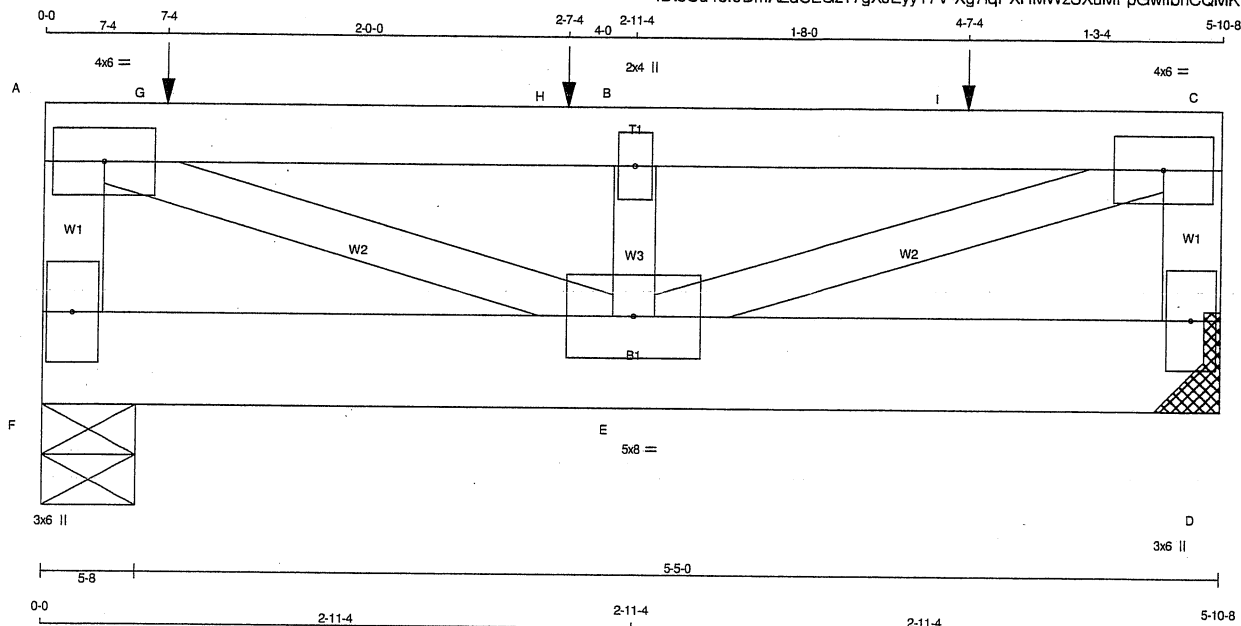
Structural component only
DWG# T-2125243

JOB NAME 420298	TRUSS NAME T38	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MTek Industries, Inc. Thu Jul 29 15:59:40 2021 Page 1

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Scale = 1:11.0

TOTAL WEIGHT = 2 X 24 = 48 lb

LUMBER					
N. L. G. A. RULES					
CHORDS		SIZE	LUMBER		DESCR.
F - A	2x4	DRY	No.2		SPF
A - C	2x4	DRY	No.2		SPF
D - C	2x4	DRY	No.2		SPF
F - D	2x6	DRY	No.2		SPF
ALL WEBS		2x3	DRY	No.2	SPF

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
F - A	12	TOP
A - C	5	SIDE(244.1)
C - D	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F - D	12	SIDE(14.0)
WEBS : (0.122"x3") SPIRAL NAILS		
B - E	5	SIDE(13.5)
2x3	1	6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0		
B	TMVW-w	MT20	2.0	4.0		
C	TMVW-t	MT20	4.0	6.0		
D	BMV1-p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
F	1253	0	1253	0
D	1372	0	1372	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	894	546 / 0	0 / 0	0 / 0	0 / 0	348 / 0	0 / 0
D	979	592 / 0	0 / 0	0 / 0	0 / 0	387 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.68 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH
FR-TO						FR-TO			
F-A	-1063 / 0	0.0	0.0	0.06 (1)	7.81	A-E	0 / 2178	0.27 (1)	
A-G	-2034 / 0	-84.9	-84.9	0.26 (1)	5.83	E-B	-1683 / 0	0.13 (1)	
G-H	-2034 / 0	-84.9	-84.9	0.26 (1)	5.83	B-I	0 / 2178	0.27 (1)	
H-B	-2034 / 0	-84.9	-84.9	0.26 (1)	5.83				
B-I	-2034 / 0	-84.9	-84.9	0.35 (1)	5.68				
I-C	-2034 / 0	-84.9	-84.9	0.35 (1)	5.68				
D-C	-1182 / 0	0.0	0.0	0.07 (1)	7.81				
F-E	0 / 0	-43.5	-43.5	0.14 (1)	10.00				
E-D	0 / 0	-43.5	-43.5	0.14 (1)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	7-4	-139	-139	---	TOP	VERT	TOTAL	---	C1
H	2-7-4	-699	-699	---	FRONT	VERT	TOTAL	---	C1
I	4-7-4	-486	-486	---	TOP	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.35/1.00 (B-C:1), BC=0.14/1.00 (E-F:1), WB=0.27/1.00 (A-E:1), SSI=0.49/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (E) (INPUT = 0.90)
JSI METAL= 0.28 (B) (INPUT = 1.00)

CONTINUED ON PAGE 2



Structural component only
DWG# T-2125244

JOB NAME 420298	TRUSS NAME T38	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

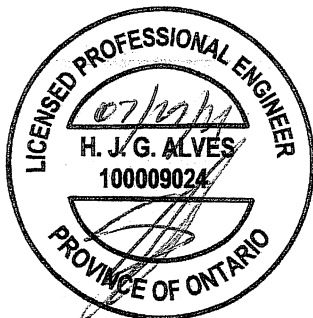
Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 29 15:59:40 2021 Page 2
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PLATES (table is in inches)

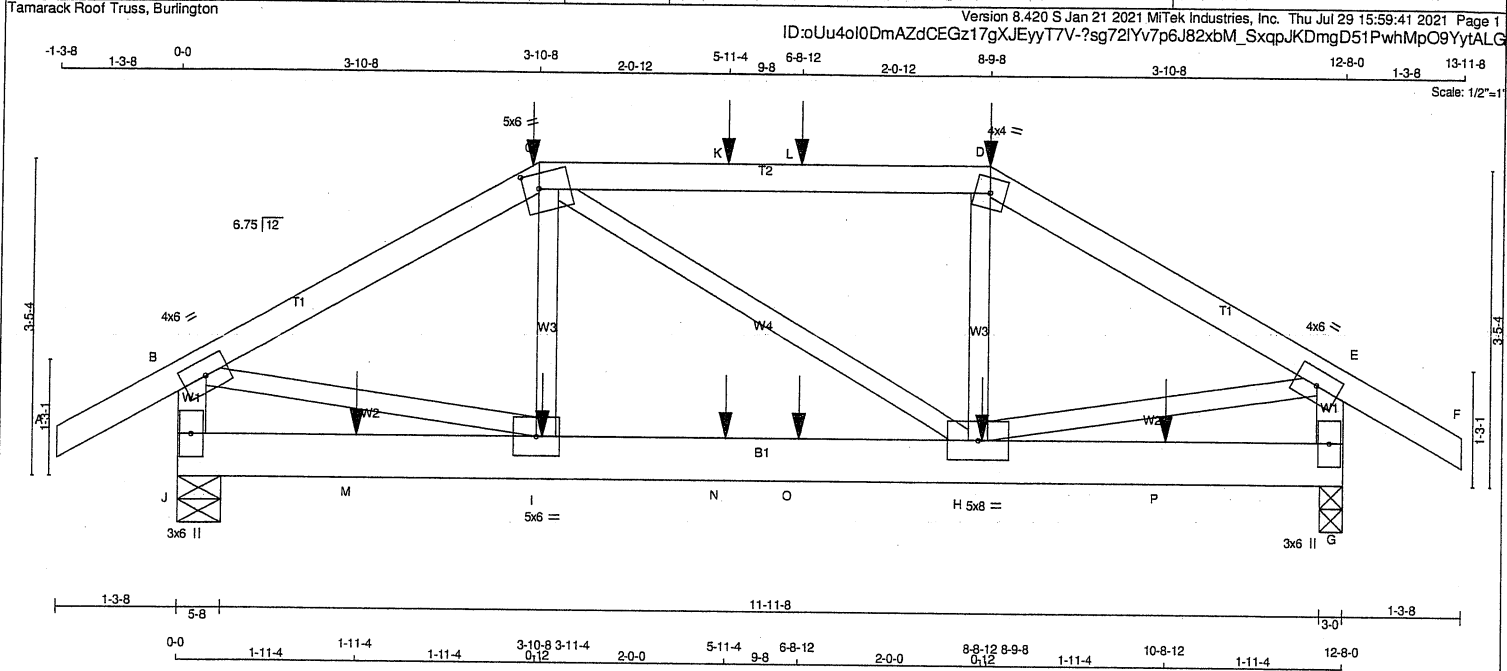
JT	TYPE	PLATES	W	LEN	Y	X
E	BMWVW-t	MT20	5.0	8.0		
F	BMV1+p	MT20	3.0	6.0		

NOTES-

- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only
DWG# T-2125244 *en*



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
J - B	2x4	DRY	No.2
G - E	2x4	DRY	No.2
J - G	2x6	DRY	No.2

ALL WEBS

EXCEPT

2x3

DRY

No.2

SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	UP/LIFT	IN-SX
J	1073	0	0	5-8
G	1072	0	0	3-0

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW
J	758	500 / 0
G	758	500 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0		
C	TTWW-m	MT20	5.0	6.0	2.00	2.00
D	TTW-m	MT20	4.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	6.0		
H	BMVWW-t	MT20	5.0	8.0		
I	BMVWW-t	MT20	5.0	6.0		
J	BMV1+p	MT20	3.0	6.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

CHORDS

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CS1 (LC)	MAX. UNBRAC LENGTH	FR-TO	W E B S	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
A-B	0 / 28	-84.9	-84.9	0.12 (1)	10.00	I-C	-96 / 58	0.02 (4)
B-C	-1109 / 0	-84.9	-84.9	0.27 (1)	5.65	C-H	0 / 3	0.00 (4)
C-K	-981 / 0	-84.9	-84.9	0.54 (1)	5.44	H-D	-95 / 60	0.02 (4)
K-L	-981 / 0	-84.9	-84.9	0.54 (1)	5.44	B-I	0 / 987	0.24 (1)
L-D	-981 / 0	-84.9	-84.9	0.54 (1)	5.44	H-E	0 / 989	0.24 (1)
D-E	-1112 / 0	-84.9	-84.9	0.27 (1)	5.65			
E-F	0 / 28	-84.9	-84.9	0.12 (1)	10.00			
J-B	-1034 / 0	0.0	0.0	0.11 (1)	7.69			
G-E	-1031 / 0	0.0	0.0	0.11 (1)	7.70			

J-M

0 / 0

-18.5

-18.5

0.04 (4)

10.00

M-I

0 / 0

-18.5

-18.5

0.04 (4)

10.00

I-N

0 / 962

-18.5

-18.5

0.17 (1)

10.00

N-O

0 / 962

-18.5

-18.5

0.17 (1)

10.00

O-H

0 / 962

-18.5

-18.5

0.17 (1)

10.00

H-P

0 / 0

-18.5

-18.5

0.04 (4)

10.00

P-G

0 / 0

-18.5

-18.5

0.04 (4)

10.00

LOADING

TOTAL LOAD CASES: (4)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF

BOT CH. LL = 6.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019

- PART 9 OF OBC 2012 (2019 AMENDMENT)

- CSA 086-14

- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.42")

CALCULATED VERT. DEFL.(LL)= L/ 999 (0.02")

ALLOWABLE DEFL.(TL)= L/360 (0.42")

CALCULATED VERT. DEFL.(TL)= L/ 999 (0.04")

CSI: TC=0.54/1.00 (C-D:1) , BC=0.17/1.00 (H-I:1) , WB=0.24/1.00 (E-H:1) , SS=0.21/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00

COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-10-8	-160	-160	---	BACK	VERT	TOTAL	---	C1
D	8-9-8	-160	-160	---	BACK	VERT	TOTAL	---	C1
H	8-8-12	-10	-10	---	BACK	VERT	TOTAL	---	C1
K	3-11-4	-10	-10	---	BACK	VERT	TOTAL	---	C1
I	5-11-4	-27	-27	---	BACK	VERT	TOTAL	---	C1
L	6-8-12	-27	-27	---	BACK	VERT	TOTAL	---	C1
M	1-11-4	-8	-8	---	BACK	VERT	TOTAL	---	C1
N	5-11-4	-10	-10	---	BACK	VERT	TOTAL	---	C1
O	6-8-12	-10	-10	---	BACK	VERT	TOTAL	---	C1
P	10-8-12	-8	-8	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

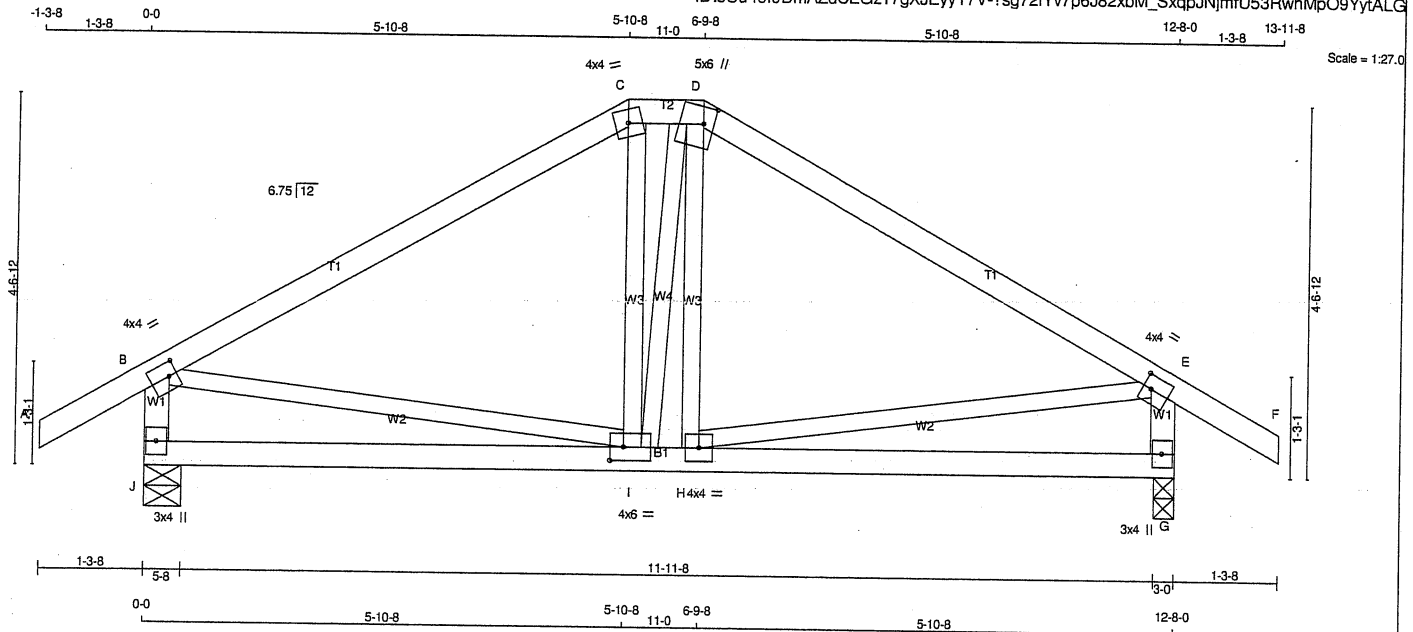
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

Structural component only

DWG# T-2125245

JOB NAME 420298	TRUSS NAME T40	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 29 15:59:41 2021 Page 1
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LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
J - B	2x4	DRY	No.2
G - E	2x4	DRY	No.2
J - G	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TTW-m	MT20	4.0	4.0		
D	TTWW+m	MT20	5.0	6.0	2.50	1.50
E	TMVW-t	MT20	4.0	4.0	2.00	1.25
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	6.0	2.00	2.00
J	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
J	770	0	770	0	0
G	770	0	770	0	0

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	545	358 / 0	0 / 0	0 / 0	0 / 0	186 / 0	0 / 0
G	545	358 / 0	0 / 0	0 / 0	0 / 0	186 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 28	I-C	-6 / 52
B-C	-575 / 0	I-D	0 / 10
C-D	-501 / 0	H-D	-10 / 42
D-E	-574 / 0	B-I	0 / 507
E-F	0 / 28	H-E	0 / 506
J-B	-725 / 0		
G-E	-726 / 0		
J-I	0 / 0		
I-H	0 / 500		
H-G	0 / 0		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.42")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.42")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.38/1.00 (B-C:1), BC=0.22/1.00 (H-I:4), WB=0.11/1.00 (B-I:1), SSI=0.17/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

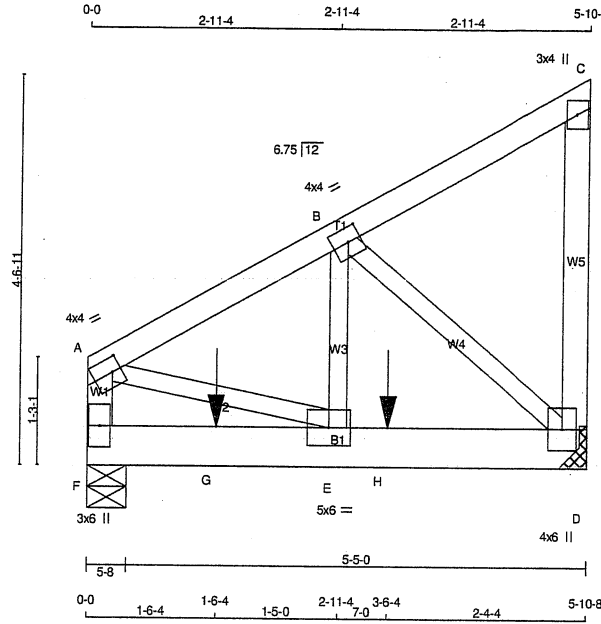
JSI GRIP= 0.82 (B) (INPUT = 0.90)
JSI METAL= 0.26 (B) (INPUT = 1.00)



Structural component only
DWG# T-2125246

JOB NAME 420298	TRUSS NAME T41	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 29 15:59:42 2021 Page 1
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Scale = 1:25.7

TOTAL WEIGHT = 2 X 30 = 60 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
D - C	2x4	DRY	No.2	SPF	
F - A	2x4	DRY	No.2	SPF	
F - D	2x6	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12	TOP
C - D	12	TOP
F - A	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F - D	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0	2.00	1.25
B	TMWW-t	MT20	4.0	4.0	2.00	1.75
C	TMV+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
D	1009	0	1009	0
F	1361	0	1361	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
D	712	473 / 0	0 / 0	0 / 0	0 / 0	240 / 0
F	961	640 / 0	0 / 0	0 / 0	0 / 0	322 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MAX. UNBRAC LENGTH	WEBS	MEMB. MAX. FORCE (LBS)	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO					FR-TO			
A-B	-1119 / 0	-84.9	-84.9	0.06 (1)	6.25	E-B	0 / 1100	0.14 (1)
B-C	-12 / 0	-84.9	-84.9	0.05 (1)	6.25	B-D	-1297 / 0	0.16 (1)
D-C	-101 / 0	0.0	0.0	0.02 (1)	7.81	A-E	0 / 1027	0.13 (1)
F-A	-943 / 0	0.0	0.0	0.05 (1)	7.81			
F-G	0 / 0	-18.5	-18.5	0.23 (1)	10.00			
G-E	0 / 0	-18.5	-18.5	0.23 (1)	10.00			
E-H	0 / 986	-18.5	-18.5	0.18 (1)	10.00			
H-D	0 / 986	-18.5	-18.5	0.18 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1 MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-6-4	-728	-728	---	BACK	VERT	---	C1
H	3-6-4	-514	-514	---	BACK	VERT	---	C1

CONNECTION REQUIREMENTS

- C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	36.7	PSF	

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.06/1.00 (A-B:1), BC=0.23/1.00 (E-F:1),
WB=0.16/1.00 (B-D:1), SSI=0.24/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (A) (INPUT = 0.90)

JSI METAL= 0.22 (D) (INPUT = 1.00)



Structural component only
DWG# T-2125247

CONTINUED ON PAGE 2

JOB NAME 420298	TRUSS NAME T41	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 29 15:59:42 2021 Page 2
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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
D	BMVW1+p	MT20	4.0	6.0		
E	BMWW-t	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	6.0		

NOTES- (1)

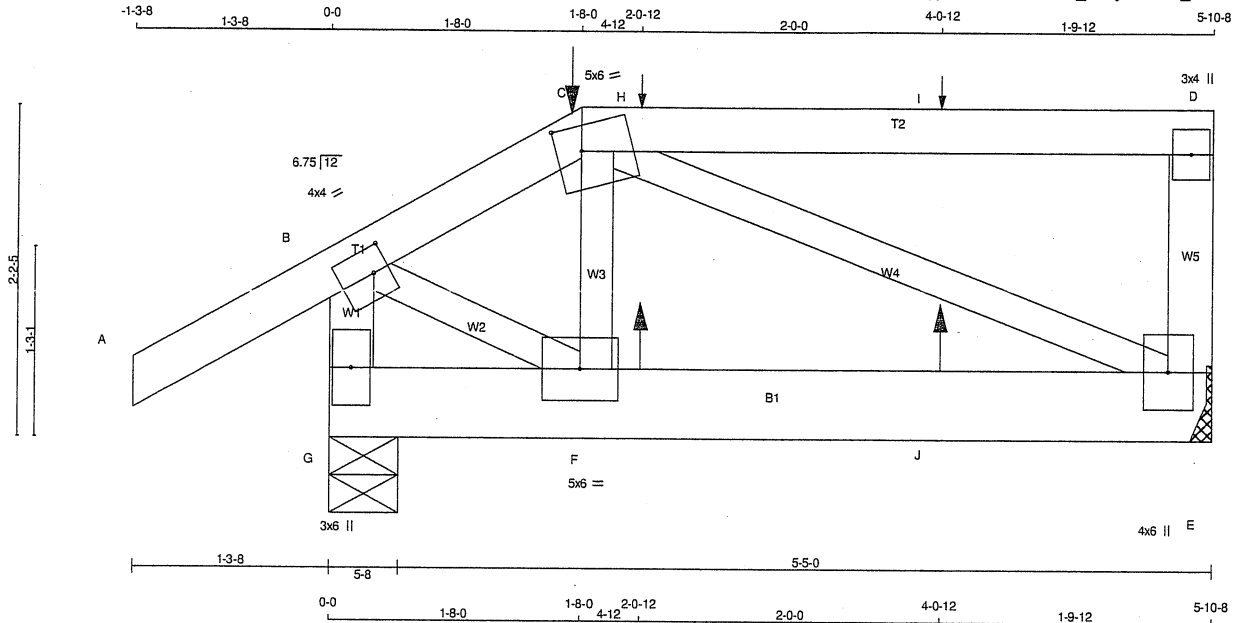
1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only
DWG# T-2125247 *m*

JOB NAME 420299	TRUSS NAME T42	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed Aug 4 11:07:24 2021 Page 1			
		ID:Uu4ol0DmAZdCEGzT7gXJEyyT7V-PhG?CB?hB_ZhhyJT1fm_A_h2GEhW6eipazGLM4GyrG3H			

Scale = 1:14.6



TOTAL WEIGHT = 27 b

LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	SPF	
A - C	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	
G - B	2x4	DRY	No.2	SPF	
G - E	2x6	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
DRY: SEASONED LUMBER.					

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-t	MT20	4.0	4.0	2.00 1.25
C	TTWW-m	MT20	5.0	6.0	2.00 2.00
D	TMV+p	MT20	3.0	4.0	
E	BMVW1+p	MT20	4.0	6.0	
F	BMVW-t	MT20	5.0	6.0	
G	BMV1+p	MT20	3.0	6.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQRD
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
E	574	0	574	0	0
G	715	0	715	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-12.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
E	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	403	284 / 0	0 / 0	0 / 0	0 / 0	119 / 0	0 / 0
G	499	361 / 0	0 / 0	0 / 0	0 / 0	139 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS		MAX. FACTORED	FACTORED	MAX.	WEBS	MAX. FACTORED	MAX.
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	FORCE (LBS)	MAX. UNBRACED LENGTH (LC)
FR-TO					FR-TO		
A-B	0 / 28	-84.9	-84.9	0.12 (1)	10.00	F-C	-186 / 9
B-C	-513 / 0	-84.9	-84.9	0.12 (1)	6.25	C-E	-451 / 0
C-H	0 / 0	-168.9	-168.9	0.56 (1)	10.00	B-F	0 / 473
H-I	0 / 0	-168.9	-168.9	0.56 (1)	10.00		
I-D	0 / 0	-168.9	-168.9	0.56 (1)	10.00		
E-D	-355 / 0	0.0	0.0	0.04 (1)	7.81		
G-B	-682 / 0	0.0	0.0	0.08 (1)	7.81		
G-F	0 / 0	-18.5	-18.5	0.02 (1)	10.00		
F-J	0 / 415	-18.5	-18.5	0.09 (1)	10.00		
J-E	0 / 415	-18.5	-18.5	0.09 (1)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	1-8-0	-3	-3	---	TOP	VERT	DEAD	---	C1
C	1-8-0	-30	-30	---	FRONT	VERT	DEAD	---	C1
C	1-8-0	-9	-9	---	TOP	VERT	SNOW	---	C1
C	1-8-0	-115	-115	---	FRONT	VERT	SNOW	---	C1
F	2-0-12	5	1	8	BACK	VERT	TOTAL	---	C1
H	2-0-12	1	1	51	BACK	VERT	TOTAL	---	C1
I	4-0-12	1	1	65	BACK	VERT	TOTAL	---	C1
J	4-0-12	5	1	8	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.56/1.00 (C-D:1), BC=0.09/1.00 (E-F:1), WB=0.14/1.00 (C-E:1), SSI=0.30/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

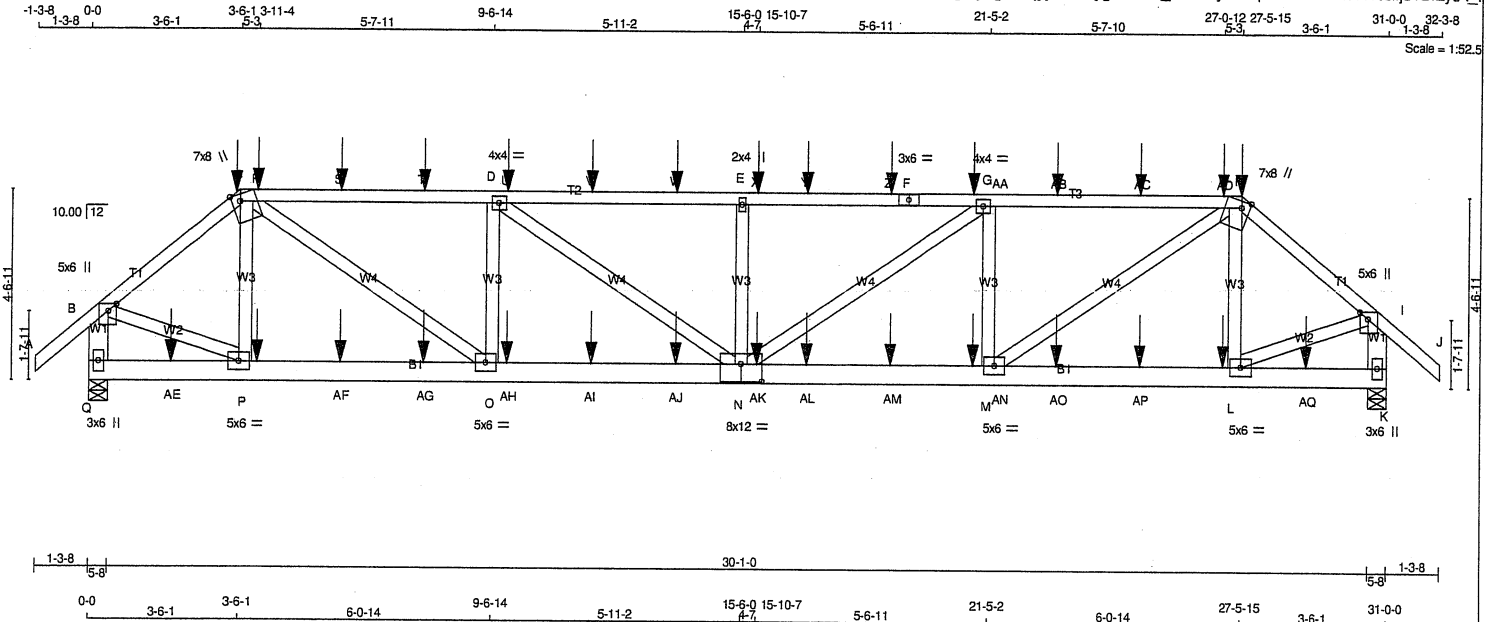
JSI GRIP= 0.67 (B) (INPUT = 0.90)
JSI METAL= 0.21 (B) (INPUT = 1.00)



Structural component only
DWG# T-2126377

JOB NAME 420300	TRUSS NAME T50	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 29 16:24:11 2021 Page 1
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LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - J	2x4	DRY	No.2
Q - B	2x6	DRY	No.2
K - I	2x6	DRY	No.2
Q - N	2x6	DRY	No.2
N - K	2x6	DRY	No.2
ALL WEBS	2x4	DRY	No.2
EXCEPT			SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C	12	TOP
C-F	12	SIDE(61.0)
F-H	12	SIDE(61.0)
H-J	12	TOP
Q-B	2	TOP
K-I	2	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
Q-N	2	SIDE(183.1)
N-K	2	SIDE(183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
2x4	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION		GROSS REACTION		BRG	BRG
JT VERT	0	2849	0	5-8	5-8
Q	0	2849	0	5-8	5-8
K	0	2852	0	5-8	5-8

UNFACTORED REACTIONS		1ST LOASE	MAX/MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
Q	2020	1296 / 0	0 / 0	0 / 0	0 / 0	724 / 0	0 / 0		
K	2022	1298 / 0	0 / 0	0 / 0	0 / 0	725 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.68 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 38	P-C	-545 / 0
B-C	-2845 / 0	C-O	0 / 2803
C-R	-4472 / 0	O-D	-1405 / 0
R-S	-4472 / 0	D-N	0 / 847
S-T	-4472 / 0	N-E	-779 / 0
T-D	-4472 / 0	N-G	0 / 835
D-U	-5164 / 0	M-G	-1408 / 0
U-V	-5164 / 0	M-H	0 / 2812
V-W	-5164 / 0	L-H	-546 / 0
W-E	-5164 / 0	B-P	0 / 2298
E-X	-5164 / 0	L-I	0 / 2300
X-Y	-5164 / 0		
Y-Z	-5164 / 0		
Z-F	-5164 / 0		
F-AA	-5164 / 0		
AA-G	-5164 / 0		
G-AB	-4481 / 0		
AB-AC	-4481 / 0		
AC-AD	-4481 / 0		
AD-H	-4481 / 0		
H-I	-2848 / 0		
I-J	0 / 38		
Q-B	-2825 / 0		
K-I	-2828 / 0		

Q-AE	0 / 0	-18.5	-18.5	0.04 (4)	10.00
AE-P	0 / 0	-18.5	-18.5	0.04 (4)	10.00
P-AF	0 / 2160	-18.5	-18.5	0.17 (1)	10.00
AF-AG	0 / 2160	-18.5	-18.5	0.17 (1)	10.00
AG-O	0 / 2160	-18.5	-18.5	0.17 (1)	10.00
O-AH	0 / 4472	-18.5	-18.5	0.34 (1)	10.00
AH-AI	0 / 4472	-18.5	-18.5	0.34 (1)	10.00
AI-AJ	0 / 4472	-18.5	-18.5	0.34 (1)	10.00
AJ-N	0 / 4472	-18.5	-18.5	0.34 (1)	10.00
N-AK	0 / 4481	-18.5	-18.5	0.34 (1)	10.00
AK-AL	0 / 4481	-18.5	-18.5	0.34 (1)	10.00
AL-AM	0 / 4481	-18.5	-18.5	0.34 (1)	10.00
AM-AN	0 / 4481	-18.5	-18.5	0.34 (1)	10.00
AN-M	0 / 4481	-18.5	-18.5	0.34 (1)	10.00
M-AO	0 / 2162	-18.5	-18.5	0.17 (1)	10.00
AO-AP	0 / 2162	-18.5	-18.5	0.17 (1)	10.00
AP-L	0 / 2162	-18.5	-18.5	0.17 (1)	10.00
L-AQ	0 / 0	-18.5	-18.5	0.04 (4)	10.00

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL(LL)= L/360 (1.03")
CALCULATED VERT. DEFL(LL)= L/999 (0.11")
ALLOWABLE DEFL(TL)= L/360 (1.03")
CALCULATED VERT. DEFL(TL)= L/999 (0.21")

CSI: TC=0.61/1.00 (E-G:1), BC=0.34/1.00 (M-N:1),
WB=0.25/1.00 (H-M:1), SSI=0.20/1.00 (E-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (C) (INPUT = 0.90)
JSI METAL= 0.31 (N) (INPUT = 1.00)

CONTINUED ON PAGE 2



Structural component only
DWG# T-2125260

JOB NAME 420300	TRUSS NAME T50	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 29 16:24:11 2021 Page 2
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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTWW+m	MT20	7.0	8.0	Edge	2.25
D	TMVW-t	MT20	4.0	4.0		
E	TMVW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMVW-t	MT20	4.0	4.0		
H	TTWW+m	MT20	7.0	8.0	Edge	2.25
I	TMVW+p	MT20	5.0	6.0	2.00	2.25
K	BMV1+p	MT20	3.0	6.0		
L, M, O, P						
L	BMVW-t	MT20	5.0	6.0		
N	BSVWW-t	MT20	8.0	12.0	5.00	6.00
Q	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE	VERT. LOAD	LC1	MAX.	MEMB.	FORCE	MAX.
	(LBS)	(PLF)	CSI (LC)	UNBRAC		(LBS)	CSI (LC)
FR-TO		FROM	TO	LENGTH	FR-TO		
AQ-K	0 / 0	-18.5	-18.5	0.04 (4)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-6-1	-35	-35	---	TOP	VERT	DEAD	---	C1
C	3-6-1	-133	-133	---	TOP	VERT	SNOW	---	C1
H	27-5-15	-35	-35	---	TOP	VERT	DEAD	---	C1
H	27-5-15	-133	-133	---	TOP	VERT	SNOW	---	C1
L	27-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
P	3-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
R	3-11-4	-87	-87	---	FRONT	VERT	TOTAL	---	C1
S	5-11-4	-71	-71	---	FRONT	VERT	TOTAL	---	C1
T	7-11-4	-71	-71	---	FRONT	VERT	TOTAL	---	C1
U	9-11-4	-71	-71	---	FRONT	VERT	TOTAL	---	C1
V	11-11-4	-71	-71	---	FRONT	VERT	TOTAL	---	C1
W	13-11-4	-71	-71	---	FRONT	VERT	TOTAL	---	C1
X	15-10-7	-71	-71	---	FRONT	VERT	TOTAL	---	C1
Y	17-0-12	-71	-71	---	FRONT	VERT	TOTAL	---	C1
Z	19-0-12	-71	-71	---	FRONT	VERT	TOTAL	---	C1
AA	21-0-12	-71	-71	---	FRONT	VERT	TOTAL	---	C1
AB	23-0-12	-71	-71	---	FRONT	VERT	TOTAL	---	C1
AC	25-0-12	-71	-71	---	FRONT	VERT	TOTAL	---	C1
AD	27-0-12	-87	-87	---	FRONT	VERT	TOTAL	---	C1
AE	1-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AF	5-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AG	7-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AH	9-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AI	11-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AJ	13-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AK	15-10-7	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AL	17-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AM	19-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AN	21-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AO	23-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AP	25-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AQ	29-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1

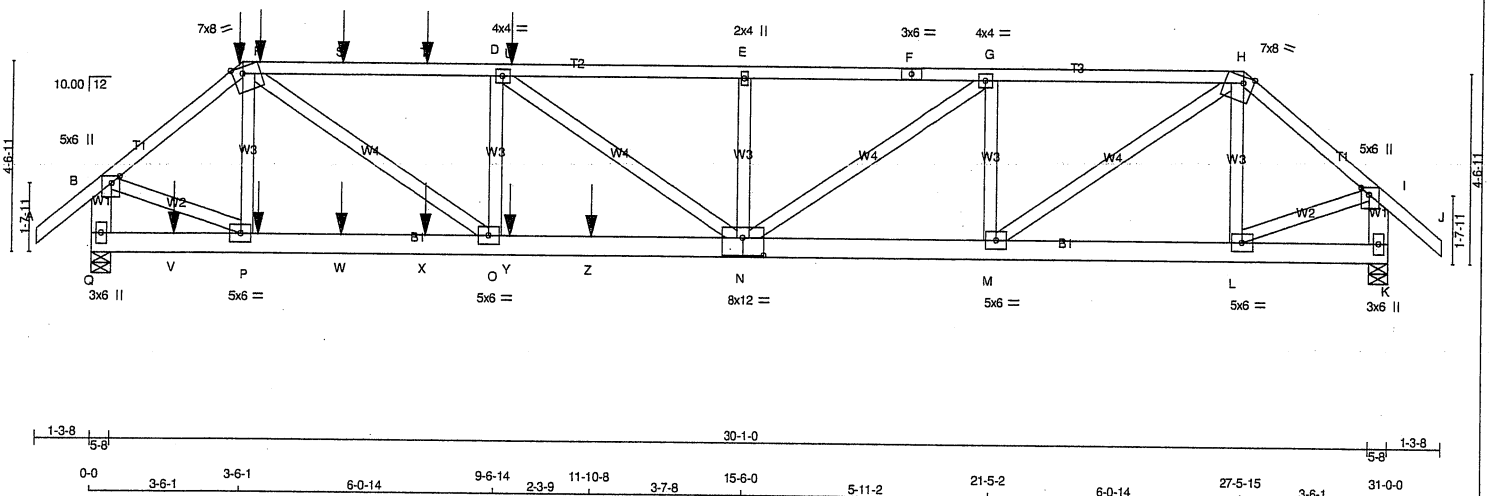
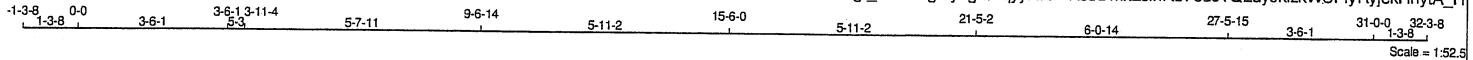
CONNECTION REQUIREMENTS
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2125260

JOB NAME 420300	TRUSS NAME T50Z	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 29 16:24:12 2021 Page 1
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LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	DESCR.
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - J	2x4	DRY	No.2
Q - B	2x6	DRY	No.2
K - I	2x6	DRY	No.2
Q - N	2x6	DRY	No.2
N - K	2x6	DRY	No.2
ALL WEBS	2x4	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	1	12
C - F	1	12
F - H	1	12
H - J	1	12
Q - B	2	12
K - I	2	12
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
Q - N	2	12
N - K	2	12
WEBS : (0.122"x3") SPIRAL NAILS		
2x4	1	6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQ'D BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
Q	3323	0	3323	0	5-8
K	2451	0	2451	0	5-8

UNFACTORED REACTIONS		1ST LCASE	MAX./MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
Q	2353	1526 / 0	0 / 0
K	1736	1124 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.56 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	FORCE (LBS)	MEMB.	FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 38	A-B	-84.9
B-C	-3296 / 0	B-C	-84.9
C-D	-5742 / 0	C-D	-84.9
D-E	-5742 / 0	D-E	-84.9
E-F	-5742 / 0	E-F	-84.9
F-G	-5742 / 0	F-G	-84.9
G-H	-4279 / 0	G-H	-84.9
H-I	-2412 / 0	H-I	-84.9
I-J	0 / 38	I-J	-84.9
J-K	-3231 / 0	J-K	-84.9
K-L	-2436 / 0	K-L	-84.9
Q-V	0 / 0	Q-V	-18.5
V-P	0 / 0	V-P	-18.5
P-W	0 / 2492	P-W	-18.5
W-X	0 / 2492	W-X	-18.5
X-O	0 / 2492	X-O	-18.5
O-Y	0 / 5742	O-Y	-18.5
Y-Z	0 / 5742	Y-Z	-18.5
Z-N	0 / 5742	Z-N	-18.5
N-M	0 / 4279	N-M	-18.5
M-L	0 / 1830	M-L	-18.5
L-K	0 / 0	L-K	-18.5

SPECIFIED CONCENTRATED LOADS (LBS)		LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
JT	3-6-1	-35	-35	---	---	TOP	VERT	DEAD	---	C1
C	3-6-1	-133	-133	---	---	TOP	VERT	SNOW	---	C1
P	3-11-4	-21	-21	---	---	BACK	VERT	TOTAL	---	C1
R	3-11-4	-87	-87	---	---	BACK	VERT	TOTAL	---	C1
S	5-11-4	-71	-71	---	---	BACK	VERT	TOTAL	---	C1
T	7-11-4	-71	-71	---	---	BACK	VERT	TOTAL	---	C1
U	9-11-4	-71	-71	---	---	BACK	VERT	TOTAL	---	C1
V	1-11-4	-21	-21	---	---	BACK	VERT	TOTAL	---	C1
W	5-11-4	-21	-21	---	---	BACK	VERT	TOTAL	---	C1
X	7-11-4	-21	-21	---	---	BACK	VERT	TOTAL	---	C1
Y	9-11-4	-21	-21	---	---	BACK	VERT	TOTAL	---	C1
Z	11-10-8	-1079	-1079	---	---	BACK	VERT	TOTAL	---	C1

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL	=	23.3	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.4	PSF
TOTAL LOAD	=	36.7	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL)= L/999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL)= L/999 (0.34")

CSI: TC=0.61/1.00 (C-D:1), BC=1.00/1.00 (N-O:1), WB=0.35/1.00 (C-O:1), SS=0.38/1.00 (N-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

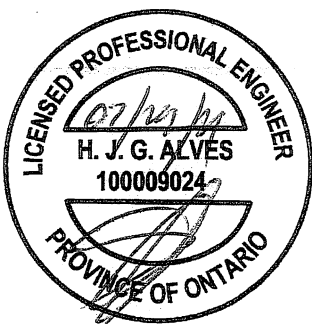
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (G) (INPUT = 0.90)
JSI METAL= 0.40 (N) (INPUT = 1.00)

CONTINUED ON PAGE 2



Structural component only
DWG# T-2125261

JOB NAME 420300	TRUSS NAME T50Z	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 29 16:24:12 2021 Page 2
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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTWW-m	MT20	7.0	8.0	Edge	2.75
D	TMVW-t	MT20	4.0	4.0		
F	TMVW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMVW-t	MT20	4.0	4.0		
I	TTWW-m	MT20	7.0	8.0	Edge	2.75
J	TMVW+p	MT20	5.0	6.0	2.00	2.25
K	BMV1+p	MT20	3.0	6.0		
L, M, O, P						
L	BMVW-t	MT20	5.0	6.0		
N	BSWWVW-l	MT20	8.0	12.0	5.00	6.00
Q	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

CONNECTION REQUIREMENTS

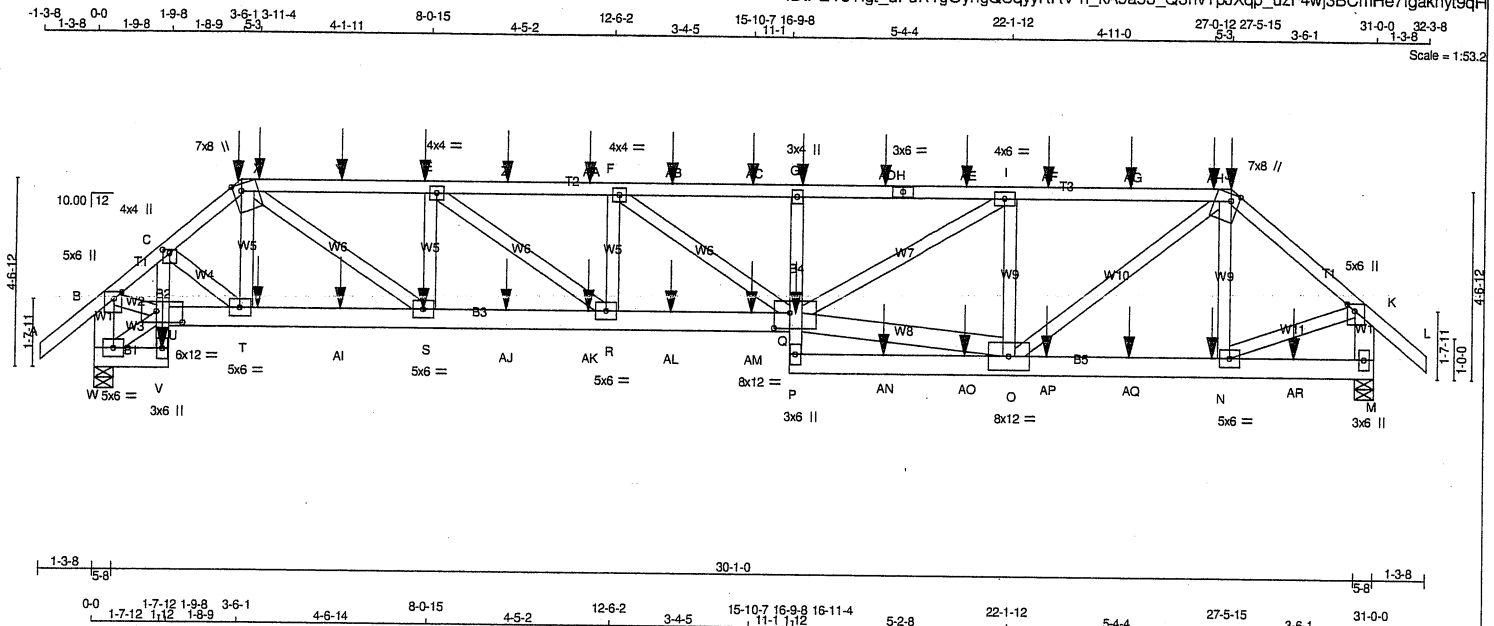
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2125261 *mm*

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 29 16:34:52 2021 Page 1 ID:PEY5Tlgt_uPuRTgOyngQsqyyRRV-h_KA5a53_Q3nvTpJXqp_uzF4wj3BCmHe7lgakhyt9qH



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
W - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
W - V	2x6	DRY	No.2	SPF
V - C	2x4	DRY	No.2	SPF
U - Q	2x6	DRY	No.2	SPF
P - G	2x4	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF
EXCEPT				
Q - O	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-D 1	12	TOP
D-H 1	12	SIDE(61.0)
H-J 1	12	SIDE(61.0)
J-L 1	12	TOP
W-B 2	12	TOP
M-K 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
W-V 2	12	SIDE(122.0)
U-Q 2	12	SIDE(183.1)
P-M 2	12	SIDE(183.1)
V-C 1	12	TOP
G-P 1	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x4 1	6	
2x6 2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQ'D BRG
JT	VERT	HORZ	DOWN	HORZ
M	2996	0	2996	0
W	3287	0	3287	0

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	2124	1363 / 0	0 / 0	0 / 0	0 / 0	761 / 0	0 / 0
W	2329	1502 / 0	0 / 0	0 / 0	0 / 0	827 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, W

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.13 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED UNBRACED LENGTH (FT)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED UNBRACED LENGTH (FT)
MEMB.					MEMB.		
FR-TO					FR-TO		
A-B	0 / 38	-84.9	-84.9	0.06 (1)	10.00	C-T	0 / 21
B-C	-4136 / 0	-84.9	-84.9	0.11 (1)	4.60	T-D	0 / 304
C-D	-4195 / 0	-84.9	-84.9	0.11 (1)	4.58	Q-O	0 / 4489
D-X	-5886 / 0	-84.9	-84.9	0.30 (1)	3.81	Q-I	0 / 3128
X-Y	-5886 / 0	-84.9	-84.9	0.30 (1)	3.81	O-I	-2459 / 0
Y-E	-5886 / 0	-84.9	-84.9	0.30 (1)	3.81	O-J	0 / 2873
E-Z	-7215 / 0	-84.9	-84.9	0.37 (1)	3.42	N-J	-608 / 0
Z-AA	-7215 / 0	-84.9	-84.9	0.37 (1)	3.42	W-U	-110 / 0
AA-F	-7215 / 0	-84.9	-84.9	0.37 (1)	3.42	N-K	0 / 2423
F-AB	-7320 / 0	-84.9	-84.9	0.35 (1)	3.42	B-U	0 / 3155
AB-AC	-7320 / 0	-84.9	-84.9	0.35 (1)	3.42	F-Q	0 / 129
AC-G	-7320 / 0	-84.9	-84.9	0.35 (1)	3.42	D-S	0 / 3226
G-AD	-7263 / 0	-84.9	-84.9	0.64 (1)	3.13	R-F	-469 / 0
AD-H	-7263 / 0	-84.9	-84.9	0.64 (1)	3.13	S-E	-1406 / 0
H-AE	-7263 / 0	-84.9	-84.9	0.64 (1)	3.13	E-R	0 / 1638
AE-I	-7263 / 0	-84.9	-84.9	0.64 (1)	3.13		
I-AF	-4525 / 0	-84.9	-84.9	0.46 (1)	4.06		
AF-AG	-4525 / 0	-84.9	-84.9	0.46 (1)	4.06		
AG-AH	-4525 / 0	-84.9	-84.9	0.46 (1)	4.06		
AH-J	-4525 / 0	-84.9	-84.9	0.46 (1)	4.06		
J-K	-3000 / 0	-84.9	-84.9	0.14 (1)	5.21		
K-L	0 / 38	-84.9	-84.9	0.06 (1)	10.00		
W-B	-3212 / 0	0.0	0.0	0.12 (1)	7.72		
M-K	-2964 / 0	0.0	0.0	0.11 (1)	7.81		
W-V	0 / 93	-18.5	-18.5	0.01 (1)	10.00		
V-U	0 / 193	0.0	0.0	0.09 (1)	10.00		
U-C	-159 / 0	0.0	0.0	0.07 (1)	7.81		
U-T	0 / 3201	-18.5	-18.5	0.25 (1)	10.00		
T-AI	0 / 3232	-18.5	-18.5	0.27 (1)	10.00		
AI-S	0 / 3232	-18.5	-18.5	0.27 (1)	10.00		
S-AJ	0 / 5886	-18.5	-18.5	0.47 (1)	10.00		
AJ-AK	0 / 5886	-18.5	-18.5	0.47 (1)	10.00		
AK-R	0 / 5886	-18.5	-18.5	0.47 (1)	10.00		
R-AL	0 / 7215	-18.5	-18.5	0.55 (1)	10.00		
AL-AM	0 / 7215	-18.5	-18.5	0.55 (1)	10.00		
AM-Q	0 / 7215	-18.5	-18.5	0.55 (1)	10.00		
P-Q	0 / 68	0.0	0.0	0.14 (1)	10.00		
Q-G	-567 / 0	0.0	0.0	0.14 (1)	7.81		
P-AN	0 / 183	-18.5	-18.5	0.06 (4)	10.00		
AN-AO	0 / 183	-18.5	-18.5	0.06 (4)	10.00		
AO-O	0 / 183	-18.5	-18.5	0.06 (4)	10.00		
O-AP	0 / 2276	-18.5	-18.5	0.17 (1)	10.00		

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.
(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/999 (0.18")
ALLOWABLE DEFL.(TL) = L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/999 (0.35")

CSI: TC=0.64/1.00 (G-I:1), BC=0.55/1.00 (Q-R:1), WB=0.29/1.00 (Q-Q:1), SS=0.19/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

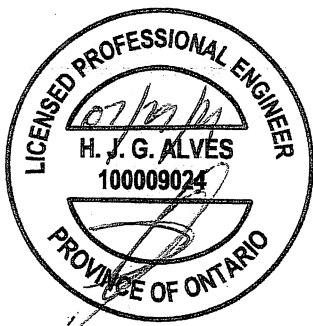
AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.



Structural component only
DWG# T-2125271

JOB NAME 420301	TRUSS NAME T50S	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 29 16:34:52 2021 Page 2			ID:PEY5Tlgt uPuRTgOvngQSqyRRV-h kA5a53 Q3nvTpJXqp uzF4wj3BCmHe7lgakhyt9qH

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 29 16:34:52 2021 Page 2
ID:PEY5Tlgt uPuRTgOvngQSqyRRV-h kA5a53 Q3nvTpJXqp uzF4wj3BCmHe7lgakhyt9qH

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	TTWW+m	MT20	7.0	8.0	Edge	2.25
E	TMVW-t	MT20	4.0	4.0		
F	TMVW-t	MT20	4.0	4.0		
G	TMV+p	MT20	3.0	4.0		
H	TS-t	MT20	3.0	6.0		
I	TMVW-t	MT20	4.0	6.0		
J	TTWW+m	MT20	7.0	8.0	Edge	2.25
K	TMVW+p	MT20	5.0	6.0	2.00	2.25
M	BMV1+p	MT20	3.0	6.0		
N, R, S, T						
N	BMVW-t	MT20	5.0	6.0		
O	BMVW-t	MT20	8.0	12.0		
P	BMV+p	MT20	3.0	6.0		
Q	BMVW-t	MT20	8.0	12.0	4.50	4.50
U	BMVW-t	MT20	8.0	12.0	3.25	7.50
V	BMV+p	MT20	3.0	6.0		
W	BMVW-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES:

- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	FACTORED	MEMB.	FORCE (LBS)	FACTORED	MAX. FACTORED
FR-TO		FROM TO	CS1 (LC)	UNBRAC			
AP-AQ	0 / 2276	-18.5	-18.5 0.17 (1)	10.00			
AQ-N	0 / 2276	-18.5	-18.5 0.17 (1)	10.00			
N-AR	0 / 0	-18.5	-18.5 0.03 (4)	10.00			
AR-M	0 / 0	-18.5	-18.5 0.03 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	3-6-1	-35	-35	---	TOP	VERT	DEAD	---	C1
E	7-11-4	-12	-12	---	FRONT	VERT	TOTAL	---	C1
G	17-0-12	-71	-71	---	FRONT	VERT	TOTAL	---	C1
J	27-5-15	-35	-35	---	TOP	VERT	DEAD	---	C1
N	27-5-15	-133	-133	---	TOP	VERT	SNOW	---	C1
Q	16-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
S	7-11-4	-122	-122	---	FRONT	VERT	TOTAL	---	C1
T	3-11-4	-122	-122	---	FRONT	VERT	TOTAL	---	C1
V	1-7-12	-128	-128	---	FRONT	VERT	TOTAL	---	C1
X	3-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
Y	5-11-4	-12	-12	---	FRONT	VERT	TOTAL	---	C1
Z	9-11-4	-12	-12	---	FRONT	VERT	TOTAL	---	C1
AA	11-11-4	-12	-12	---	FRONT	VERT	TOTAL	---	C1
AB	13-11-4	-12	-12	---	FRONT	VERT	TOTAL	---	C1
AC	15-10-7	-12	-12	---	FRONT	VERT	TOTAL	---	C1
AD	19-0-12	-71	-71	---	FRONT	VERT	TOTAL	---	C1
AE	21-0-12	-71	-71	---	FRONT	VERT	TOTAL	---	C1
AF	23-0-12	-71	-71	---	FRONT	VERT	TOTAL	---	C1
AG	25-0-12	-71	-71	---	FRONT	VERT	TOTAL	---	C1
AH	27-0-12	-87	-87	---	FRONT	VERT	TOTAL	---	C1
AI	5-11-4	-122	-122	---	FRONT	VERT	TOTAL	---	C1
AJ	9-11-4	-122	-122	---	FRONT	VERT	TOTAL	---	C1
AK	11-11-4	-122	-122	---	FRONT	VERT	TOTAL	---	C1
AL	13-11-4	-122	-122	---	FRONT	VERT	TOTAL	---	C1
AM	15-10-7	-122	-122	---	FRONT	VERT	TOTAL	---	C1
AN	19-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AO	21-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AP	23-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AQ	25-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AR	29-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1

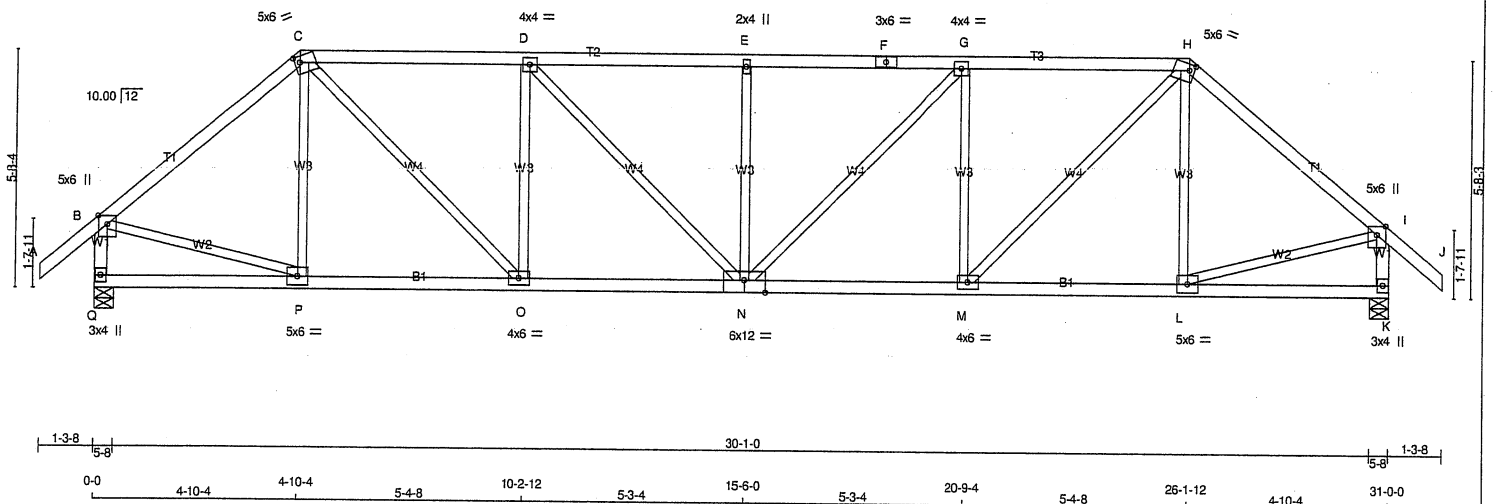
CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

JSI GRIP= 0.89 (D) (INPUT = 0.90)
JSI METAL= 0.42 (Q) (INPUT = 1.00)



Structural component only
DWG# T-2125271



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2 SPF
C - F	2x4	DRY	No.2 SPF
F - H	2x4	DRY	No.2 SPF
H - J	2x4	DRY	No.2 SPF
Q - B	2x4	DRY	No.2 SPF
K - I	2x4	DRY	No.2 SPF
Q - N	2x4	DRY	No.2 SPF
N - K	2x4	DRY	No.2 SPF
ALL WEBS EXCEPT	2x3	DRY	No.2 SPF

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMWW+p	MT20	5.0	6.0 Edge
C	TTWW-m	MT20	5.0	6.0 1.75 1.50
D	TMWW-t	MT20	4.0	4.0
E	TMWW-w	MT20	2.0	4.0
F	TS-t	MT20	3.0	6.0
G	TMWW-t	MT20	4.0	4.0
H	TTWW-m	MT20	5.0	6.0 1.75 1.50
I	TMWW-p	MT20	5.0	6.0 Edge
K	BMV1+p	MT20	3.0	4.0
L	BMWW-t	MT20	5.0	6.0
M	BMWW-t	MT20	4.0	6.0
N	BSWWW-t	MT20	6.0	12.0 Edge 6.00
O	BMWW-t	MT20	4.0	6.0
P	BMWW-t	MT20	5.0	6.0
Q	BMV1+p	MT20	3.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
 1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	HORZ	DOWN	HORZ	BRG	IN-SX	BRG	IN-SX
Q	1720	0	1720	0	0	5-8	5-8	5-8	5-8
K	1720	0	1720	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS		1ST LOASE		MAX./MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND
Q	1218	786 / 0	0 / 0	0 / 0	0 / 0
K	1218	786 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.02 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 38	P-C	-220 / 0
B-C	-1611 / 0	C-O	0 / 1214
C-D	-2089 / 0	O-D	-754 / 0
D-E	-2331 / 0	D-N	0 / 345
E-F	-2331 / 0	N-E	-412 / 0
F-G	-2331 / 0	N-G	0 / 345
G-H	-2089 / 0	M-G	-754 / 0
H-I	-1611 / 0	M-H	0 / 1214
I-J	0 / 38	L-H	-220 / 0
Q-B	-1683 / 0	B-P	0 / 1277
K-I	-1683 / 0	L-I	0 / 1277
Q-P	0 / 0		
P-O	0 / 1231		
O-N	0 / 2090		
N-M	0 / 2090		
M-L	0 / 1231		
L-K	0 / 0		

TOTAL WEIGHT = 2 X 133 = 266 lb

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 23.3 PSF
 DL = 6.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBC 2018, ABC 2019
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 086-14
 - TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.03")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.10")
 ALLOWABLE DEFL.(TL) = L/360 (1.03")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.20")

CSI: TC=0.47/1.00 (E-G:1), BC=0.40/1.00 (M-N:1), WB=0.37/1.00 (G-M:1), SSI=0.21/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

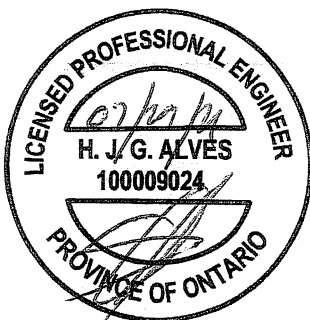
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION
 (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.90 (H) (INPUT = 0.90)
 JSI METAL = 0.60 (B) (INPUT = 1.00)



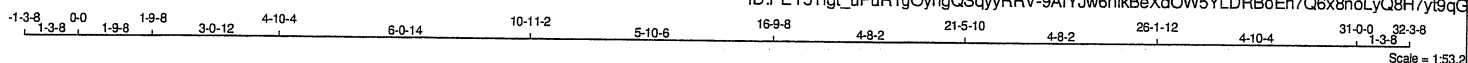
Structural component only
 DWG# T-2125262

JOB NAME 420301	TRUSS NAME T51S	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Thu Jul 29 16:34:53 2021 Page 1

ID:PEY5Tlgt_uPuRTgOyngQSqyRRV-9AIYJw6hkBxOW5YLDReh7Q6x8nLyQ8H7y19qG



Scale = 1:53.2

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - K	2x4	DRY	No.2
U - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
U - T	2x4	DRY	No.2
T - C	2x4	DRY	No.2
S - P	2x4	DRY	No.2
O - F	2x4	DRY	No.2
O - L	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
P - N	2x4	DRY	No.2
U - S	2x4	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-t	MT20	5.0	6.0	2.50 2.75
C	TMVW+p	MT20	4.0	4.0	1.00 2.00
D	TTWW-m	MT20	5.0	8.0	Edge 3.00
E	TMVW-t	MT20	4.0	4.0	
F	TMV+p	MT20	3.0	4.0	
G	TS-t	MT20	3.0	6.0	
H	TMVW-t	MT20	4.0	6.0	
I	TTWW+m	MT20	5.0	6.0	2.00 1.50
J	TMVW+p	MT20	5.0	6.0	Edge
L	BMV1+p	MT20	3.0	4.0	
M	BMVW-t	MT20	5.0	6.0	
N	BMVW-t	MT20	6.0	12.0	3.00 4.50
O	BMV+p	MT20	3.0	4.0	
P	BMVW-t	MT20	6.0	12.0	3.00 4.50
Q	BMVW-t	MT20	4.0	6.0	
R	BMVW-t	MT20	4.0	4.0	
S	BMVW-t	MT20	6.0	12.0	Edge 7.50
T	BMV+p	MT20	3.0	4.0	
U	BMVW1+p	MT20	4.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
		GROSS REACTION		GROSS REACTION		BRG		BRG	
JT		VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
L	1217	0	1717	0	0	5-8	5-8		
U	1722	0	1722	0	0	5-8	5-8		

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
L	1217	785 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0		
U	1220	788 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, U

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.46 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED				W E B S			
MEMB.	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM	TO			FR-TO			
A-B	0 / 38	-84.9	-84.9	0.12 (1)	10.00	C-R	-80 / 0	0.01 (1)	
B-C	-1988 / 0	-84.9	-84.9	0.15 (1)	4.65	R-D	0 / 95	0.03 (4)	
C-D	-1959 / 0	-84.9	-84.9	0.18 (1)	4.66	D-Q	0 / 1428	0.32 (1)	
D-E	-2655 / 0	-84.9	-84.9	0.68 (1)	3.58	Q-E	-713 / 0	0.23 (1)	
E-F	-2830 / 0	-84.9	-84.9	0.70 (1)	3.46	E-P	0 / 216	0.05 (1)	
F-G	-2814 / 0	-84.9	-84.9	0.38 (1)	3.82	P-N	0 / 1975	0.32 (1)	
G-H	-2814 / 0	-84.9	-84.9	0.38 (1)	3.82	N-H	0 / 1136	0.26 (1)	
H-I	-1983 / 0	-84.9	-84.9	0.31 (1)	4.49	N-H	-1212 / 0	0.60 (1)	
I-J	-1607 / 0	-84.9	-84.9	0.43 (1)	4.74	N-I	0 / 1161	0.26 (1)	
J-K	0 / 38	-84.9	-84.9	0.12 (1)	10.00	M-I	-226 / 0	0.11 (1)	
U-B	-1674 / 0	0.0	0.0	0.18 (1)	6.42	U-S	-59 / 0	0.01 (1)	
L-J	-1679 / 0	0.0	0.0	0.18 (1)	6.41	M-J	0 / 1273	0.29 (1)	
						B-S	0 / 1527	0.34 (1)	
U-T	0 / 49	-18.5	-18.5	0.02 (4)	10.00				
T-S	0 / 17	0.0	0.0	0.08 (1)	10.00				
S-C	-157 / 0	0.0	0.0	0.07 (1)	7.81				
S-R	0 / 1548	-18.5	-18.5	0.31 (1)	10.00				
R-Q	0 / 1496	-18.5	-18.5	0.31 (1)	10.00				
Q-P	0 / 2655	-18.5	-18.5	0.51 (1)	10.00				
O-P	0 / 39	0.0	0.0	0.10 (1)	10.00				
P-F	-416 / 0	0.0	0.0	0.12 (1)	7.81				
O-N	0 / 68	-18.5	-18.5	0.09 (4)	10.00				
N-M	0 / 1227	-18.5	-18.5	0.25 (1)	10.00				
M-L	0 / 0	-18.5	-18.5	0.10 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 240 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL) = L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.28")

CSI: TC=0.70/1.00 (E-F:1), BC=0.51/1.00 (P-Q:1), WB=0.60/1.00 (H-N:1), SS=0.24/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
650	371	1747	788
1987	1873		

PLATE PLACEMENT TOL. = 0.250 inches

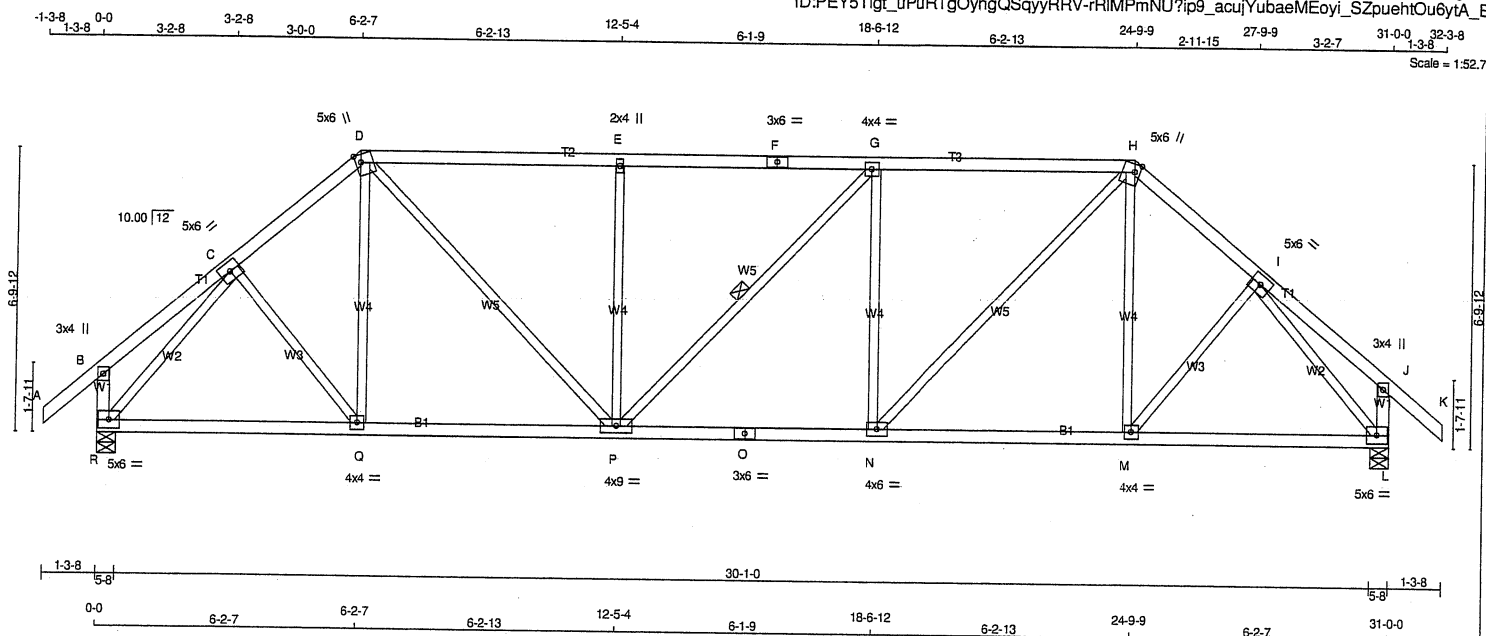
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (D) (INPUT = 0.90)
JSI METAL= 0.60 (J) (INPUT = 1.00)



Structural component only.
DWG# T-2125272

CONTINUED ON PAGE 2



LUMBER		N. L. G. A. RULES		LUMBER		DESCR.	
CHORDS		SIZE					
A -	D	2x4	DRY	No.2		SPF	
D -	F	2x4	DRY	No.2		SPF	
F -	H	2x4	DRY	No.2		SPF	
H -	K	2x4	DRY	No.2		SPF	
R -	B	2x4	DRY	No.2		SPF	
L -	J	2x4	DRY	No.2		SPF	
R -	O	2x4	DRY	No.2		SPF	
O -	L	2x4	DRY	No.2		SPF	

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+P	MT20	3.0	4.0	
C	CTMMW+T	MT20	5.0	6.0	
D	TTVVW+m	MT20	5.0	6.0	2.25 1.50
E	TMW+W	MT20	2.0	4.0	
F	TS-t	MT20	3.0	6.0	
G	CTMMW+T	MT20	4.0	6.0	
H	TTVVW+m	MT20	5.0	6.0	2.25 1.50
I	JTMMW+T	MT20	5.0	6.0	
J	TMV+P	MT20	3.0	4.0	
L	BMVW1-t	MT20	5.0	6.0	
M	BMVW+T	MT20	4.0	4.0	
N	BMVW+T	MT20	4.0	6.0	
O	BS-t	MT20	3.0	6.0	
P	BMVWVW+T	MT20	4.0	9.0	
Q	BMVW+T	MT20	4.0	4.0	
R	BMVW1-t	MT20	5.0	6.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER BEARINGS

BEARINGS		FACTORED		MAXIMUM FACTORED			INPUT	REQD
		GROSS REACTION		GROSS REACTION			BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
R	1720	0	1720	0	0	5-8	5-8	
L	1720	0	1720	0	0	5-8	5-8	

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
	1218	786 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0
L	1218	786 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R. L.

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.26 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

C H O R D S					W E B S			
MAX. FACTORED		FACTORED			MAX. FACTORED			
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM	TO		FR-TO			
A-B	0 / 38	-84.9	-84.9	0.12 (1)	10.00	C-Q	0 / 128 0.03 (4)	
B-C	0 / 17	-84.9	-84.9	0.11 (1)	10.00	Q-Q	0 / 83 0.03 (4)	
C-D	-1630 / 0	-84.9	-84.9	0.15 (1)	5.05	D-P	0 / 938 0.21 (1)	
D-E	-1885 / 0	-84.9	-84.9	0.56 (1)	4.27	P-E	-567 / 0 0.44 (1)	
E-F	-1885 / 0	-84.9	-84.9	0.56 (1)	4.26	G-G	-2 / 0 0.00 (1)	
F-G	-1885 / 0	-84.9	-84.9	0.56 (1)	4.26	N-G	-568 / 0 0.44 (1)	
G-H	-1886 / 0	-84.9	-84.9	0.57 (1)	4.26	H-H	0 / 940 0.21 (1)	
H-I	-1630 / 0	-84.9	-84.9	0.15 (1)	5.05	M-H	0 / 82 0.03 (4)	
I-J	0 / 17	-84.9	-84.9	0.11 (1)	10.00	M-I	0 / 128 0.03 (4)	
J-K	0 / 38	-84.9	-84.9	0.12 (1)	10.00	R-C	-1854 / 0 0.72 (1)	
R-B	-222 / 0	0.0	0.0	0.02 (1)	7.81	I-L	-1853 / 0 0.72 (1)	
L-J	-222 / 0	0.0	0.0	0.02 (1)	7.81			
R-Q	0 / 1153	-18.5	-18.5	0.28 (1)	10.00			
Q-P	0 / 1236	-18.5	-18.5	0.30 (1)	10.00			
P-O	0 / 1886	-18.5	-18.5	0.37 (1)	10.00			
O-N	0 / 1886	-18.5	-18.5	0.37 (1)	10.00			
N-M	0 / 1236	-18.5	-18.5	0.30 (1)	10.00			
M-L	0 / 1153	-18.5	-18.5	0.28 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR
SMALL BUILDING REQUIREMENTS OF PART 9,
NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL(LL)= L/360 (1.03")
CALCULATED VERT. DEFL(LL) = L/ 999 (0.08")
ALLOWABLE DEFL(TL)= L/360 (1.03")
CALCULATED VERT. DEFL(TL) = L/ 999 (0.16")

CSI: TC=0.57/1.00 (G-H:1), BC=0.37/1.00 (N-P:1),
WB=0.72/1.00 (C-R:1), SSI=0.25/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES						
PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (C) (INPUT = 0.90)
JSI METAL= 0.63 (O) (INPUT = 1.00)



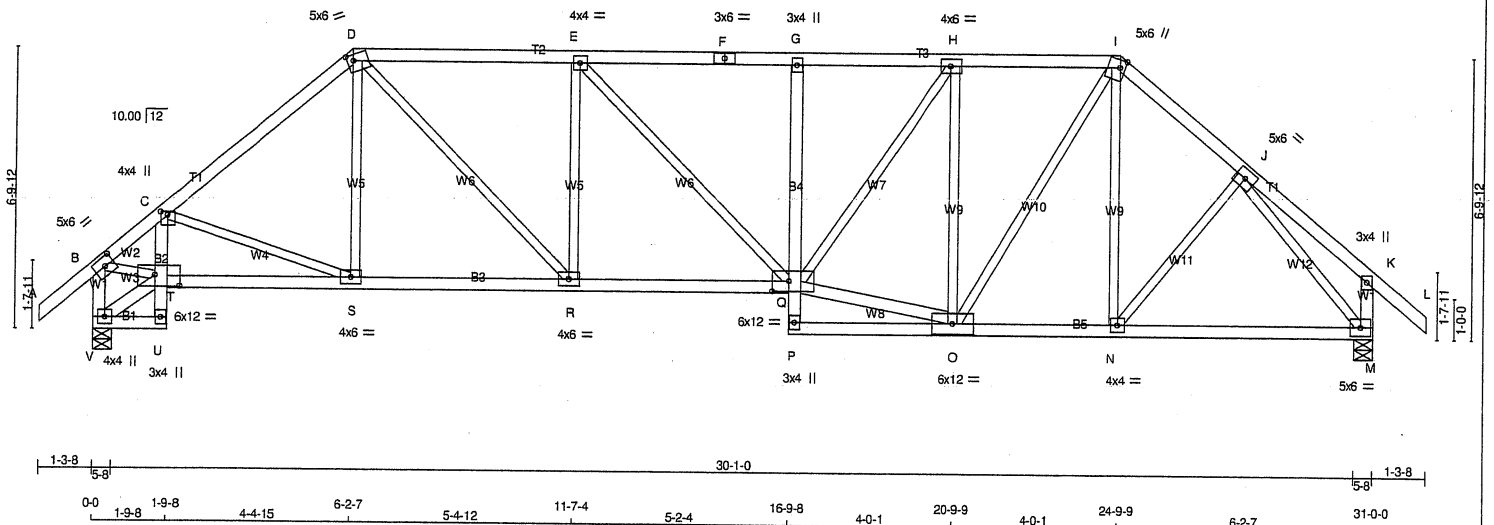
Structural component only
DWG# T-2125263

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 29 16:34:54 2021 Page 1
ID: PEY5Tgt_uPuRTgOyngQSqyyRRV-dNswWG6JW1JV8mzieFsZOKSsXntgXjxpc9hpayt9qf

Structural drawing of a roof truss showing dimensions and member lengths. The drawing includes a top chord with a total length of 32'-3" and a bottom chord with a total length of 32'-3". The truss is supported by a 1'-3" wide base. The top chord is divided into segments of 1'-3", 0'-0", 1'-9", 1'-9", 4'-4", 15'-6", 2'-7", 5'-4", 12'-1", 11'-7", 5'-2", 16'-9", 8'-0", 4'-0", 1", 20'-9", 9'-0", 4'-0", 1", 24'-9", 9'-0", 3'-0", 27'-9", 9'-0", 3'-2", 31'-0", and 32'-3". The bottom chord is divided into segments of 1'-3", 1'-9", 1'-9", 4'-4", 15'-6", 2'-7", 5'-4", 12'-1", 11'-7", 5'-2", 16'-9", 8'-0", 4'-0", 1", 20'-9", 9'-0", 4'-0", 1", 24'-9", 9'-0", 3'-0", 27'-9", 9'-0", 3'-2", 31'-0", and 32'-3". The truss is supported by a 1'-3" wide base. The drawing is labeled with dimensions and member lengths.

Scale = 1:53.2



LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
V - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
V - U	2x4	DRY	No.2	SPF
U - C	2x4	DRY	No.2	SPF
T - Q	2x4	DRY	No.2	SPF
P - G	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
V - T	2x4	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-t	MT20	5.0	6.0	2.50 2.75
C	TMVW+p	MT20	4.0	4.0	1.00 2.00
D	TTVW-m	MT20	5.0	6.0	1.75 1.75
E	TMVW-t	MT20	4.0	4.0	
F	TS-t	MT20	3.0	6.0	
G	TMV+p	MT20	3.0	4.0	
H	TTVW-t	MT20	4.0	6.0	
I	TTTTW+m	MT20	5.0	6.0	2.25 1.50
J	TMVW-t	MT20	5.0	6.0	
K	TMV+p	MT20	3.0	4.0	
M	BMVW-1-t	MT20	5.0	6.0	
N	BMVW-t	MT20	4.0	4.0	
O	BMVWVW-t	MT20	6.0	12.0	
P	BMV+p	MT20	3.0	4.0	
Q	BVMVWVW-1	MT20	6.0	12.0	3.00 4.75
R	BMVW-t	MT20	4.0	6.0	
S	BMVW-t	MT20	4.0	6.0	
T	BVMVW-1	MT20	6.0	12.0	3.25 7.00
U	BMV+p	MT20	3.0	4.0	
V	BMVW-1+p	MT20	4.0	4.0	

~~NOTES~~ (1)
1)



Structural component only
DWG# T-2125273

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS								
FACTORED			MAXIMUM FACTORED			INPUT	REQD	
GROSS REACTION			GROSS REACTION			BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
V	1722	0	1722	0	0	5-8	5-8	
M	1717	0	1717	0	0	5-8	5-8	

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
V	1220	788 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0
M	1217	785 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.06 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED			
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC	MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO	LENGTH	FR-TO		
A-B	0 / 38	-84.9	-84.9 0.12 (1)	10.00	C-S	-175 / 0	0.07 (1)
B-C	-2026 / 0	-84.9	-84.9 0.22 (1)	4.53	S-D	0 / 130	0.04 (4)
C-D	-1898 / 0	-84.9	-84.9 0.31 (1)	4.60	D-R	0 / 1014	0.23 (1)
D-E	-2153 / 0	-84.9	-84.9 0.48 (1)	4.14	R-E	-617 / 0	0.32 (1)
E-F	-2246 / 0	-84.9	-84.9 0.48 (1)	4.06	E-Q	0 / 133	0.03 (1)
F-G	-2246 / 0	-84.9	-84.9 0.48 (1)	4.06	Q-O	0 / 1711	0.28 (1)
G-H	-2238 / 0	-84.9	-84.9 0.24 (1)	4.35	Q-H	0 / 933	0.21 (1)
H-I	-1693 / 0	-84.9	-84.9 0.21 (1)	4.90	O-H	-1119 / 0	0.87 (1)
I-J	-1625 / 0	-84.9	-84.9 0.15 (1)	5.06	O-I	0 / 890	0.20 (1)
J-K	0 / 17	-84.9	-84.9 0.11 (1)	10.00	N-I	0 / 67	0.02 (4)
K-L	0 / 38	-84.9	-84.9 0.12 (1)	10.00	N-J	0 / 126	0.03 (4)
V-B	-1674 / 0	0.0	0.0 0.18 (1)	6.42	V-T	-60 / 0	0.01 (1)
M-K	-222 / 0	0.0	0.0 0.02 (1)	7.81	J-M	-1848 / 0	0.72 (1)
					B-T	0 / 1584	0.36 (1)
V-U	0 / 50	-18.5	-18.5 0.02 (4)	10.00			
U-T	0 / 17	0.0	0.0 0.08 (1)	10.00			
T-C	-160 / 0	0.0	0.0 0.07 (1)	7.81			
T-S	0 / 1605	-18.5	-18.5 0.32 (1)	10.00			
S-R	0 / 1443	-18.5	-18.5 0.29 (1)	10.00			
R-Q	0 / 2154	-18.5	-18.5 0.41 (1)	10.00			
P-Q	0 / 35	0.0	0.0 0.07 (1)	10.00			
Q-G	-365 / 0	0.0	0.0 0.13 (1)	7.81			
P-O	0 / 45	-18.5	-18.5 0.07 (4)	10.00			
O-N	0 / 1231	-18.5	-18.5 0.28 (1)	10.00			
N-M	0 / 1150	-18.5	-18.5 0.27 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:				
TOP	CH.	LL =	23.3	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR
SMALL BUILDING REQUIREMENTS OF PART 9,
NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.48/1.00 (E-G:1) , BC=0.41/1.00 (Q-R:1) ,
WB=0.87/1.00 (H-O:1) , SSI=0.22/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

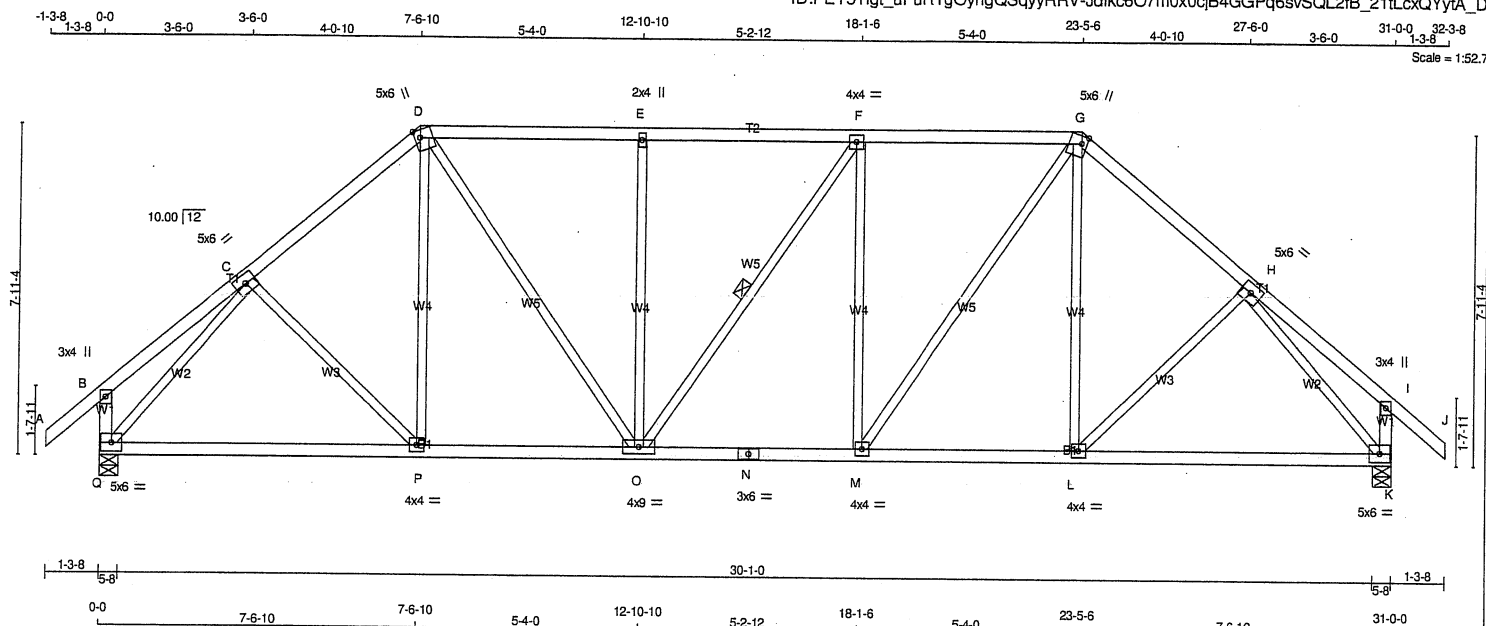
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (J) (INPUT = 0.90)
JSI METAL= 0.49 (B) (INPUT = 1.00)

JOB NAME 420300	TRUSS NAME T53	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 29 16:24:16 2021 Page 1
ID:PEY5Tlgt_uPuRTgOyngQsqyyRRV-JdIk6O7m0X0cB4GGPq6svSQL2tB_21tLcxQYytA_D



TOTAL WEIGHT = 2 X 146 = 292 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
G - J	2x4	DRY	No.2
Q - B	2x4	DRY	No.2
K - I	2x4	DRY	No.2
Q - N	2x4	DRY	No.2
N - K	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTWW+m	MT20	5.0	6.0	2.25	1.50
H	TMWW-t	MT20	5.0	6.0		
I	TMV+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0		
L, M, P						
L	BMWW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	9.0		
Q	BMWW-t	MT20	5.0	6.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	UPLIFT	IN-SX
Q	1720	0	1720	0	5-8
K	1720	0	1720	0	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1218	786 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0
K	1218	786 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 38	C-P	0 / 61
B-C	0 / 26	P-D	0 / 121
C-D	-1604 / 0	D-O	0 / 692
D-E	-1611 / 0	O-E	-483 / 0
E-F	-1611 / 0	F-G	-3 / 0
F-G	-1612 / 0	G-H	-484 / 0
G-H	-1604 / 0	H-I	0 / 694
H-I	0 / 26	I-J	0 / 120
I-J	0 / 38	J-K	0 / 61
Q-B	-220 / 0	K-L	-1863 / 0
K-I	-220 / 0		
Q-P	0 / 1181		
P-O	0 / 1214		
O-N	0 / 1612		
N-M	0 / 1612		
M-L	0 / 1214		
L-K	0 / 1181		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/999 (0.14")

CSI: TC=0.38/1.00 (F-G:1), BC=0.34/1.00 (O-P:4), WB=0.85/1.00 (C-Q:1), SSI=0.21/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (C) (INPUT = 0.90)
JSI METAL= 0.55 (N) (INPUT = 1.00)

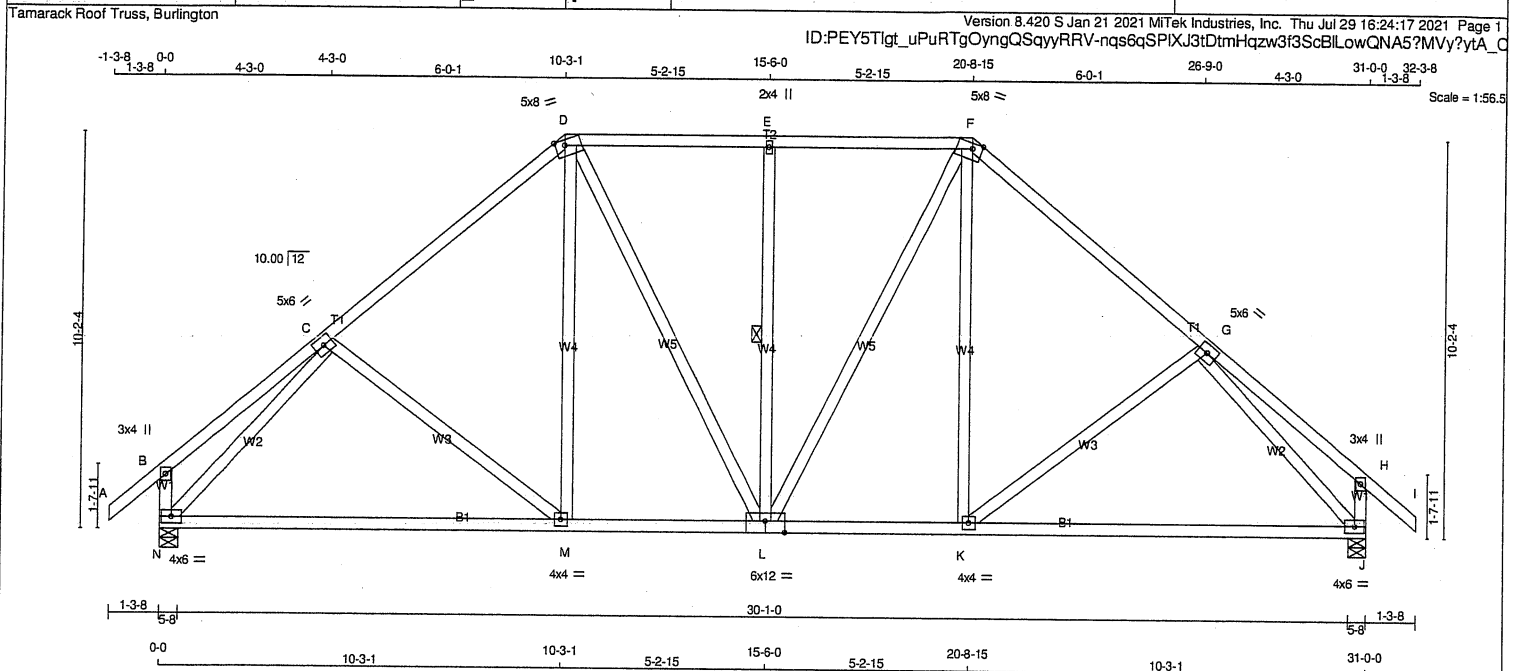


Structural component only
DWG# T-2125264



Structural component only
DWG# T-2125265

JOB NAME 420300	TRUSS NAME T55	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
N - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
N - L	2x4	DRY	No.2
L - J	2x4	DRY	No.2
ALL WEBS	2x4	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
C	TMVW-t	MT20	5.0	6.0	
D	TTWW-m	MT20	5.0	8.0	Edge 3.00
E	TMV+w	MT20	2.0	4.0	
F	TTWW-m	MT20	5.0	8.0	Edge 3.00
G	TMVW-t	MT20	5.0	6.0	
H	TMV+p	MT20	3.0	4.0	
J	BMVW1-t	MT20	4.0	6.0	
K	BMVW-t	MT20	4.0	4.0	
L	BSWWW-t	MT20	6.0	12.0	Edge 6.00
M	BMVW-t	MT20	4.0	4.0	
N	BMVW1-t	MT20	4.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES - (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	HORZ	UPLIFT	IN-SX
N	1720	0	1720	0	5-8
J	1720	0	1720	0	5-8

UNFACTORED REACTIONS

JT	1ST LOASE	MAX/MIN. SNOW	MAX/MIN. LIVE	MAX/MIN. PERM. LIVE	MAX/MIN. WIND	MAX/MIN. DEAD	MAX/MIN. SOIL
N	COMBINED	786 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0
J	COMBINED	786 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-L

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 38	C-M	-133 / 15
B-C	0 / 46	M-D	0 / 276
C-D	-1504 / 0	D-L	0 / 320
D-E	-1283 / 0	L-E	-542 / 0
E-F	-1283 / 0	F-F	0 / 320
F-G	-1504 / 0	K-F	0 / 276
G-H	0 / 46	K-G	-133 / 15
H-I	0 / 38	N-C	-1883 / 0
N-B	-220 / 0	J-G	-1883 / 0
J-H	-220 / 0		
N-M	0 / 1235		
M-L	0 / 1133		
L-K	0 / 1133		
K-J	0 / 1235		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL)= L/999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL)= L/999 (0.34")

CSI: TC=0.45/1.00 (C-D:1), BC=0.54/1.00 (L-M:4), WB=0.91/1.00 (G-J:1), SSI=0.22/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

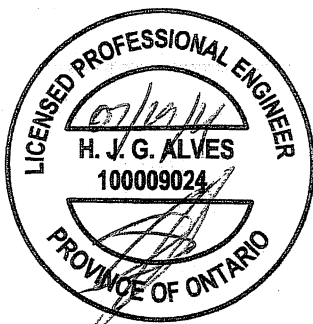
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

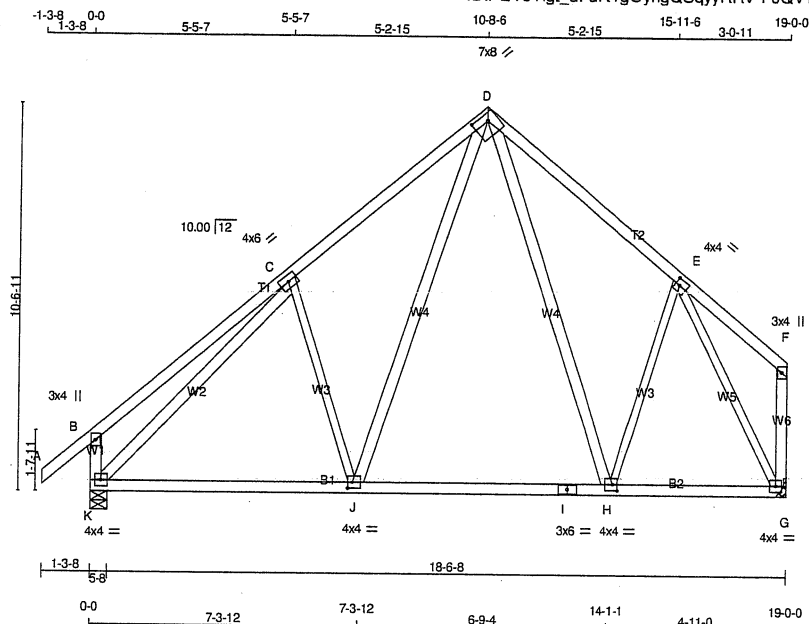
JSI GRIP= 0.87 (J) (INPUT = 0.90)
JSI METAL= 0.42 (C) (INPUT = 1.00)



Structural component only
DWG# T-2125266

JOB NAME 420300	TRUSS NAME T58	QUANTITY 3	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 29 16:24:18 2021 Page 1
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TOTAL WEIGHT = 3 X 104 = 313 lb

LUMBER				DESCR.	
N. L. G. A. RULES					
CHORDS	SIZE	LUMBER			
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
K - B	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
K - I	2x4	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF	
C - J	2x3	DRY	No.2	SPF	
H - E	2x3	DRY	No.2	SPF	
E - G	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
		GROSS REACTION		GROSS REACTION		BRG		BRG	
JT		VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
K		1100	0	1100	0	0	5-8	5-8	
G		982	0	982	0	0	MECHANICAL		

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

1ST LOASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
K	778	507 / 0	0 / 0	0 / 0	0 / 0	271 / 0	0 / 0
G	697	443 / 0	0 / 0	0 / 0	0 / 0	255 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	FORCE (LBS)	MEMB.	FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 38	C-J	-320 / 0
B-C	0 / 36	J-D	0 / 478
C-D	-813 / 0	D-H	0 / 84
D-E	-841 / 0	H-E	0 / 69
E-F	0 / 46	K-C	-1035 / 0
K-B	-286 / 0	E-G	-1004 / 0
G-F	-56 / 0		
K-J	0 / 703		
J-I	0 / 446		
I-H	0 / 446		
H-G	0 / 462		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/999 (0.08")

CSI: TC=0.41/1.00 (B-C:1), BC=0.29/1.00 (J-K:4), WB=0.91/1.00 (C-K:1), SSI=0.17/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (K) (INPUT = 0.90)
JSI METAL= 0.31 (E) (INPUT = 1.00)

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	4.0	6.0		
D	TTWW-h	MT20	7.0	8.0	2.25	5.00
E	TMVW-t	MT20	4.0	4.0	2.00	1.50
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BMVW-t	MT20	4.0	4.0	2.00	1.50
I	BS-t	MT20	3.0	6.0		
J	BMVW-t	MT20	4.0	4.0	2.00	1.50
K	BMVW1-t	MT20	4.0	4.0		

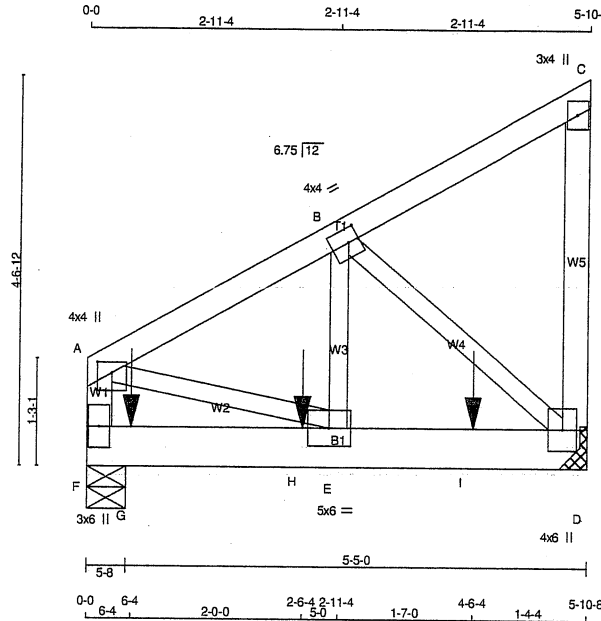
NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only
DWG# T-2125267

JOB NAME 420300	TRUSS NAME T59	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 29 16:24:19 2021 Page 1
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Scale = 1:25.7

TOTAL WEIGHT = 2 X 30 = 60 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C	12	TOP
C-D	12	TOP
F-A	3	SIDE(8.4)
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F-D	8	SIDE(244.1)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.50	2.00
B	TMVW-t	MT20	4.0	4.0	2.00	1.50
C	TMV+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX	MECHANICAL	
D	1544	0	1544	0	0	5-8	5-8	
F	1958	0	1958	0	0	5-8	5-8	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE				
D	1094	708 / 0	0 / 0	0 / 0	0 / 0	388 / 0	0 / 0
F	1387	895 / 0	0 / 0	0 / 0	0 / 0	493 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	-1364 / 0	-84.9	-84.9 0.06 (1)	6.25	E-B	0 / 1405	0.17 (1)
B-C	-11 / 0	-84.9	-84.9 0.05 (1)	6.25	B-D	-1577 / 0	0.20 (1)
D-C	-102 / 0	0.0	0.0 0.02 (1)	7.81	A-E	0 / 1249	0.15 (1)
F-A	-1127 / 0	0.0	0.0 0.06 (1)	7.81			
F-G	0 / 0	-18.5	-18.5 0.16 (1)	10.00			
G-H	0 / 0	-18.5	-18.5 0.16 (1)	10.00			
H-E	0 / 0	-18.5	-18.5 0.16 (1)	10.00			
E-I	0 / 1199	-18.5	-18.5 0.28 (1)	10.00			
I-D	0 / 1199	-18.5	-18.5 0.28 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	6-4	-686	-686	---	BACK	VERT	TOTAL	---	C1
H	2-6-4	-682	-682	---	BACK	VERT	TOTAL	---	C1
I	4-6-4	-682	-682	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.06/1.00 (A-F:1), BC=0.28/1.00 (D-E:1),
WB=0.20/1.00 (B-D:1), SSI=0.38/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (B) (INPUT = 0.90)
JSI METAL= 0.27 (D) (INPUT = 1.00)



Structural component only
DWG# T-2125268

CONTINUED ON PAGE 2

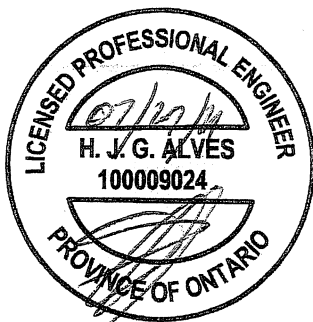
JOB NAME 420300	TRUSS NAME T59	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 29 16:24:19 2021 Page 2
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PLATES (table is in inches)

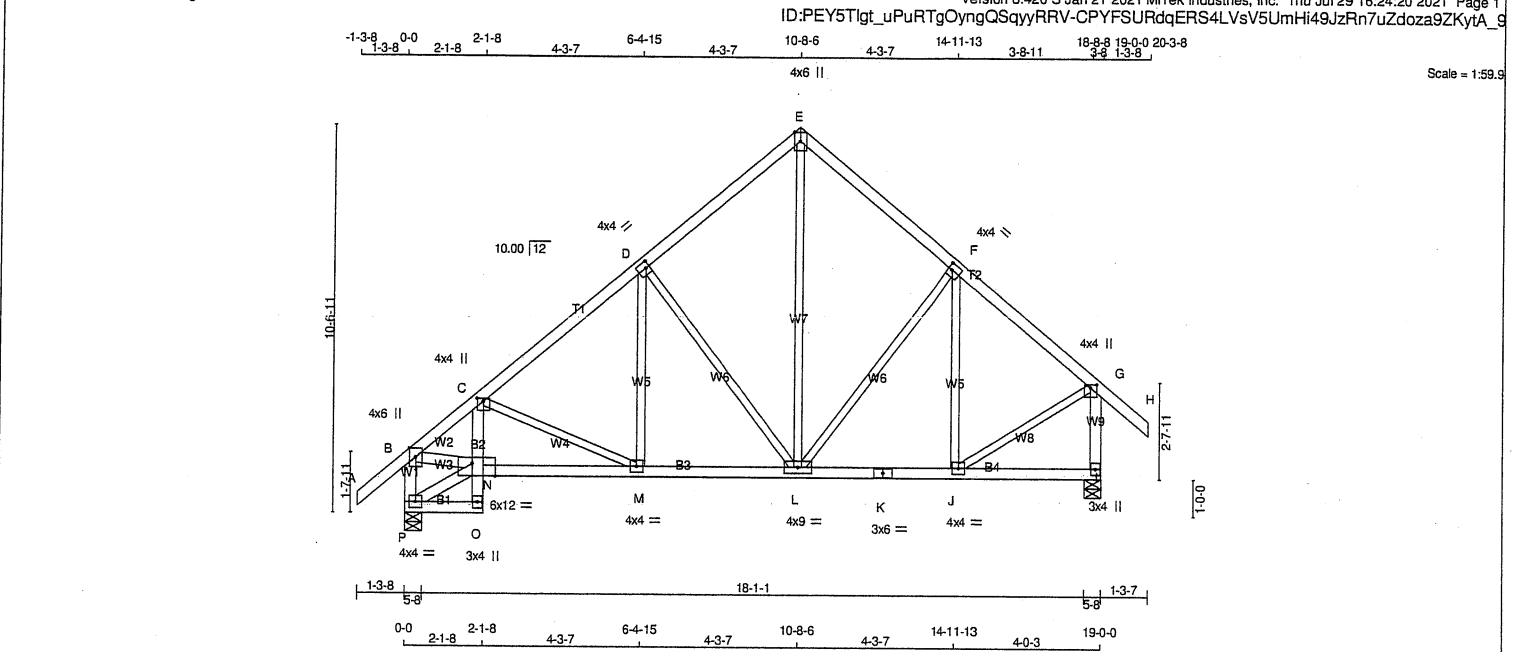
JT	TYPE	PLATES	W	LEN	Y	X
D	BMVW1+p	MT20	4.0	6.0		
E	BMVW1-t	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only
DWG# T-2125268 *2/2*

JOB NAME 420300	TRUSS NAME T61	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER				N. L. G. A. RULES				CHORDS				SIZE				LUMBER				DESCR.			
A - E				2x4				DRY				No.2				SPF							
E - H				2x4				DRY				No.2				SPF							
P - B				2x4				DRY				No.2				SPF							
I - G				2x4				DRY				No.2				SPF							
P - O				2x4				DRY				No.2				SPF							
O - C				2x4				DRY				No.2				SPF							
N - K				2x4				DRY				No.2				SPF							
K - I				2x4				DRY				No.2				SPF							
ALL WEBS EXCEPT				2x3				DRY				No.2				SPF							
P - N				2x4				DRY				No.2				SPF							
B - N				2x4				DRY				No.2				SPF							
DRY: SEASONED LUMBER.																							
PLATES (table is in inches)																							
JT		TYPE		PLATES		W		LEN		Y		X											
B		TMVW+p		MT20		4.0		6.0		Edge													
C		TMVW+p		MT20		4.0		4.0		1.00		2.00											
D		TMVW-t		MT20		4.0		4.0		2.00		1.25											
E		TTW+p		MT20		4.0		6.0		Edge													
F		TMVW-t		MT20		4.0		4.0		2.00		1.25											
G		TMVW+p		MT20		4.0		4.0		1.00		2.00											
I		BMV1+p		MT20		3.0		4.0															
J		BMVW-t		MT20		4.0		4.0															
K		BS-t		MT20		3.0		6.0															
L		BMVWW-t		MT20		4.0		9.0															
M		BMVW-t		MT20		4.0		4.0															
N		BVMW-t		MT20		6.0		12.0		Edge		7.50											
O		BMV+p		MT20		3.0		4.0															
P		BMVW1-t		MT20		4.0		4.0															
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.																							
NOTES- (1)																							
1) Lateral braces to be a minimum of 2X4 SPF #2.																							

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER																															
BEARINGS																															
				FACTORED				MAXIMUM FACTORED				INPUT				REQRD															
				GROSS REACTION				GROSS REACTION				BRG				BRG															
JT				VERT				HORZ				DOWN				HORZ				UPLIFT				IN-SX				IN-SX			
I				1099				0				1099				0				0				5-8 (5-7)				5-8			
P				1099				0				1099				0				0				5-8				5-8			
VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH																															
UNFACTORED REACTIONS																															
				1ST LCASE				MAX./MIN. COMPONENT REACTIONS																							
JT				COMBINED				SNOW				LIVE				PERM.LIVE				WIND				DEAD				SOIL			
I				778				507 / 0				0 / 0				0 / 0				0 / 0				271 / 0				0 / 0			
P				778				507 / 0				0 / 0				0 / 0				0 / 0				271 / 0				0 / 0			
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, P																															
BRACING																															
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.68 FT.																															
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.																															
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.																															
LOADING																															
TOTAL LOAD CASES: (4)																															
CHORDS								WEBS																							
				MAX. FACTORED				FACTORED								MAX. FACTORED															
MEMB.				FORCE				VERT. LOAD				LC1 MAX				MEMB.				FORCE				MAX							
				(LBS)				(PLF)				CSI (LC)				UNBRAC				(LBS)				CSI (LC)							
FR-TO								FROM TO				LENGTH				FR-TO															
A-B				0 / 38				-84.9 -84.9				0.12 (1)				10.00				C-M				-230 / 0				0.09 (1)			
B-C				-1236 / 0				-84.9 -84.9				0.10 (1)				5.68				M-D				0 / 158				0.04 (4)			
C-D				-995 / 0				-84.9 -84.9				0.20 (1)				6.04				D-L				-453 / 0				0.44 (1)			
D-E				-677 / 0				-84.9 -84.9				0.19 (1)				6.25				L-E				0 / 551				0.12 (1)			
E-F				-677 / 0				-84.9 -84.9				0.19 (1)				6.25				J-F				-252 / 0				0.14 (1)			
F-G				-735 / 0				-84.9 -84.9				0.19 (1)				6.25				L-F				-142 / 0				0.14 (1)			
G-H				0 / 38				-84.9 -84.9				0.12 (1)				10.00				J-G				0 / 676				0.15 (1)			
P-B				-1067 / 0				0.0 0.0				0.11 (1)				7.63				P-N				-29 / 0				0.00 (1)			
I-G				-1069 / 0				0.0 0.0				0.15 (1)				7.63				B-N				0 / 975				0.16 (1)			
P-O				0 / 25				-18.5 -18.5				0.02 (4)				10.00															
O-N				0 / 21				0.0 0.0				0.04 (1)				10.00															
N-C				-36 / 26				0.0 0.0				0.03 (1)				7.81															
N-M				0 / 985				-18.5 -18.5				0.20 (1)				10.00															
M-L				0 / 774				-18.5 -18.5				0.16 (1)				10.00															
L-K				0 / 586				-18.5 -18.5				0.13 (1)				10.00															
K-J				0 / 586				-18.5 -18.5				0.13 (1)				10.00															
J-I				0 / 0				-18.5 -18.5				0.07 (4)				10.00															

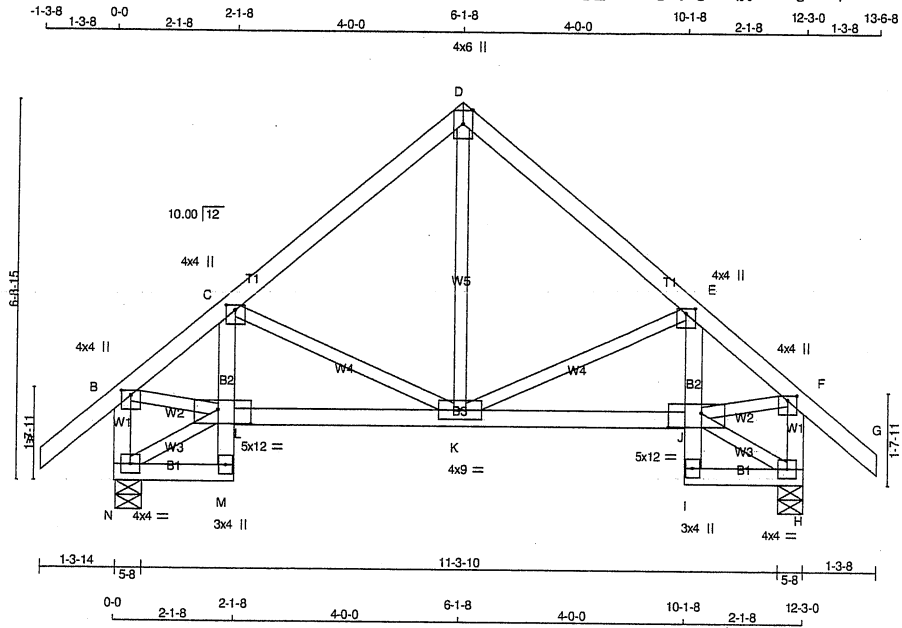
DESIGN CRITERIA																											
SPECIFIED LOADS:																											
TOP CH.				LL				=				23.3 PSF															
				DL				=				6.0 PSF															
BOT CH.				LL				=				0.0 PSF															
				DL				=				7.4 PSF															
TOTAL LOAD				=				36.7 PSF																			
SPACING = 24.0 IN. C/C																											
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015																											
THIS DESIGN COMPLIES WITH:																											
- PART 9 OF BCBC 2018 , ABC 2019																											
- PART 9 OF OBC 2012 (2019 AMENDMENT)																											
- CSA 086-14																											
- TPIC 2014																											
(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD																											
ALLOWABLE DEFL.(LL)= L/360 (0.63")																											
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")																											
ALLOWABLE DEFL.(TL)= L/360 (0.63")																											
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")																											
CSI: TC=0.20/1.00 (C-D:1) , BC=0.20/1.00 (M-N:1) , WB=0.44/1.00 (D-L:1) , SSI=0.13/1.00 (E-F:1)																											
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10																											
COMP=1.10 SHEAR=1.10 TENS=1.10																											
COMPANION LIVE LOAD FACTOR = 1.00																											
AUTOSOLVE HEELS OFF																											
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .																											
NAIL VALUES																											
PLATE GRIP(DRY) SHEAR SECTION																											
				(PSI)				(PLI)				(PLI)															
				MAX				MIN				MAX				MIN											
MT20				650				371				1747				788				1987				1873			
PLATE PLACEMENT TOL. = 0.250 inches																											

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2125269



TOTAL WEIGHT = 62 lb [M/F]

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
N - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
N - M	2x4	DRY	No.2
M - C	2x4	DRY	No.2
L - J	2x4	DRY	No.2
I - E	2x4	DRY	No.2
I - H	2x4	DRY	No.2
ALL WEBS 2x3 DRY: SEASONED LUMBER.			No.2

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B, C, E, F					
B	TMVW+p	MT20	4.0	4.0	1.00 2.00
D	TTW+p	MT20	4.0	6.0	Edge
H	BMVW-1-t	MT20	4.0	4.0	
I	BMV+p	MT20	3.0	4.0	
J	BMVWW-1	MT20	5.0	12.0	3.00 7.00
K	BMVWW-1	MT20	4.0	9.0	
L	BMVWW-1	MT20	5.0	12.0	3.00 7.00
M	BMV+p	MT20	3.0	4.0	
N	BMVW-1-t	MT20	4.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	IN-SX	BRG
N	751	0	0	751	0	0	0	5-8	5-8
H	751	0	0	751	0	0	0	5-8	5-8

UNFACTORED REACTIONS		1ST LCASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
N	531	350 / 0	0 / 0	0 / 0	0 / 0	181 / 0	0 / 0		
H	531	350 / 0	0 / 0	0 / 0	0 / 0	181 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING		TOTAL LOAD CASES: (4)		CHORDS		WEBS	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX. (LC)	MAX. UNBRACED LENGTH	MEMB.	FORCE (LBS)	MAX. FACTORED (LC)
FR-TO		FROM	TO		FR-TO		
A-B	0 / 38	-84.9	-84.9	0.12 (1)	10.00	N-L	-20 / 0
B-C	-762 / 0	-84.9	-84.9	0.13 (1)	6.25	B-L	0 / 620
C-D	-494 / 0	-84.9	-84.9	0.17 (1)	6.25	J-H	-20 / 0
D-E	-494 / 0	-84.9	-84.9	0.17 (1)	6.25	J-F	0 / 620
E-F	-762 / 0	-84.9	-84.9	0.13 (1)	6.25	K-D	0 / 316
F-G	0 / 38	-84.9	-84.9	0.12 (1)	10.00	C-K	-289 / 0
N-B	-723 / 0	0.0	0.0	0.08 (1)	7.81	K-E	-289 / 0
H-F	-723 / 0	0.0	0.0	0.08 (1)	7.81		
N-M	0 / 18	-18.5	-18.5	0.02 (4)	10.00		
M-L	0 / 21	0.0	0.0	0.03 (1)	10.00		
L-C	-19 / 30	0.0	0.0	0.02 (4)	7.81		
L-K	0 / 626	-18.5	-18.5	0.15 (1)	10.00		
K-J	0 / 626	-18.5	-18.5	0.15 (1)	10.00		
I-J	0 / 21	0.0	0.0	0.03 (1)	10.00		
J-E	-19 / 30	0.0	0.0	0.02 (4)	7.81		
I-H	0 / 18	-18.5	-18.5	0.02 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.17/1.00 (C-D:1), BC=0.15/1.00 (J-K:1), WB=0.14/1.00 (F-J:1), SS=0.12/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

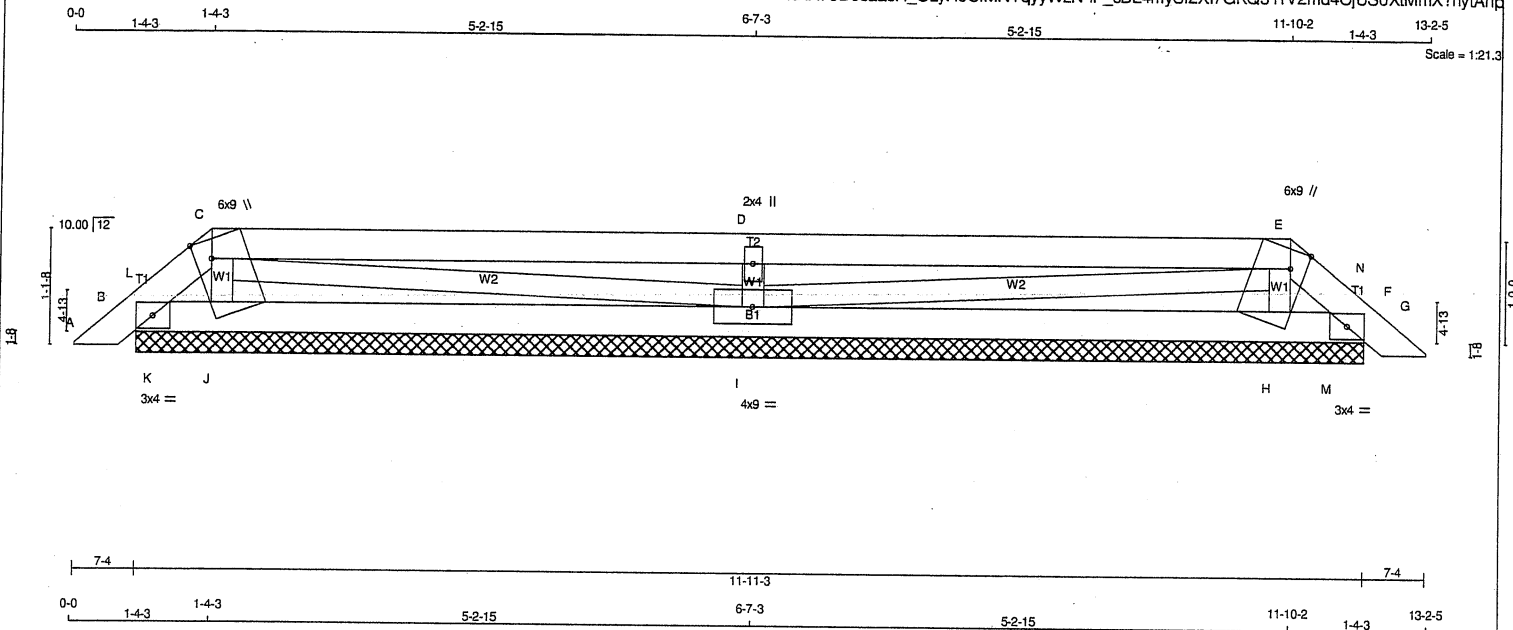
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (B) (INPUT = 0.90)
JSI METAL= 0.23 (B) (INPUT = 1.00)



JOB NAME 420295	TRUSS NAME PB1	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 29 15:35:38 2021 Page 1
ID:X73D9caach_OlyHeCIMNYqyyWzN-IF_cBL4mySi2X17GKQ3YrV2md4CjUSoXtMmX?hytAhp



TOTAL WEIGHT = 2 X 39 = 77 lb [M]

LUMBER				DESCR.	SPF
N. L. G. A. RULES	CHORDS	SIZE	LUMBER		
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
B - F	2x4	DRY	No.2	SPF	
ALL WEBS 2x3 DRY				SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	
C, E, H, J					
C	TTBW1*-m	MT20	6.0	9.0	Edge 1.75
D	TMW+w	MT20	2.0	4.0	
E	TTBW1*-m	MT20	6.0	9.0	Edge 1.75
F	TMB1-I	MT20	3.0	4.0	
I	BMWW1-t	MT20	4.0	9.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX	IN-SX	IN-SX
B	82	0	82	0	-17	11-11-3	11-11-3	
F	82	0	82	0	-17	11-11-3	11-11-3	
J	251	0	251	0	0	11-11-3	11-11-3	
I	656	0	656	0	0	11-11-3	11-11-3	
H	251	0	251	0	0	11-11-3	11-11-3	

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT F FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		SNOW MAX/MIN. COMPONENT REACTIONS		LIVE PERM. LIVE		WIND DEAD		SOIL	
	SNOW	LIVE	SNOW	LIVE	SNOW	LIVE	SNOW	LIVE	SNOW	LIVE
B	53	65/0	0/0	0/0	0/0	0/0	0/-12	0/0	0/0	0/0
F	53	65/0	0/0	0/0	0/0	0/0	0/-12	0/0	0/0	0/0
J	184	84/0	0/0	0/0	0/0	0/0	99/0	0/0	0/0	0/0
I	463	306/0	0/0	0/0	0/0	0/0	158/0	0/0	0/0	0/0
H	184	84/0	0/0	0/0	0/0	0/0	99/0	0/0	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, J, I, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0/13	-84.9	-84.9 0.02 (1)	10.00	J-C	-139/0	0.02 (1)
B-L	-33/20	-84.9	-84.9 0.03 (4)	6.25	C-I	-3/0	0.00 (1)
L-C	-51/0	-84.9	-84.9 0.02 (4)	6.25	I-D	-556/0	0.08 (1)
C-D	0/0	-84.9	-84.9 0.40 (1)	10.00	I-E	-3/0	0.00 (1)
D-E	0/0	-84.9	-84.9 0.40 (1)	10.00	H-E	-139/0	0.02 (1)
E-N	-51/0	-84.9	-84.9 0.02 (4)	6.25	K-L	-58/0	0.00 (1)
N-F	-33/20	-84.9	-84.9 0.03 (4)	6.25	M-N	-58/0	0.00 (1)
F-G	0/13	-84.9	-84.9 0.02 (1)	10.00			
B-K	0/25	-18.5	-18.5 0.01 (1)	10.00			
K-J	0/25	-18.5	-18.5 0.08 (4)	10.00			
J-I	0/4	-18.5	-18.5 0.10 (4)	10.00			
I-H	0/4	-18.5	-18.5 0.10 (4)	10.00			
H-M	0/25	-18.5	-18.5 0.08 (4)	10.00			
M-F	0/25	-18.5	-18.5 0.01 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.40/1.00 (D-E:1), BC=0.10/1.00 (I-J:4), WB=0.08/1.00 (D-I:1), SS=0.22/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

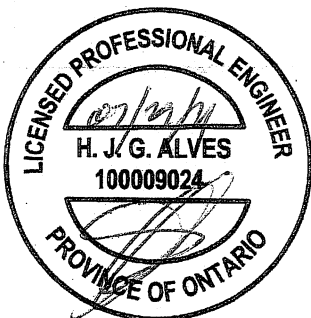
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.28 (D) (INPUT = 0.90)
JSI METAL= 0.12 (D) (INPUT = 1.00)

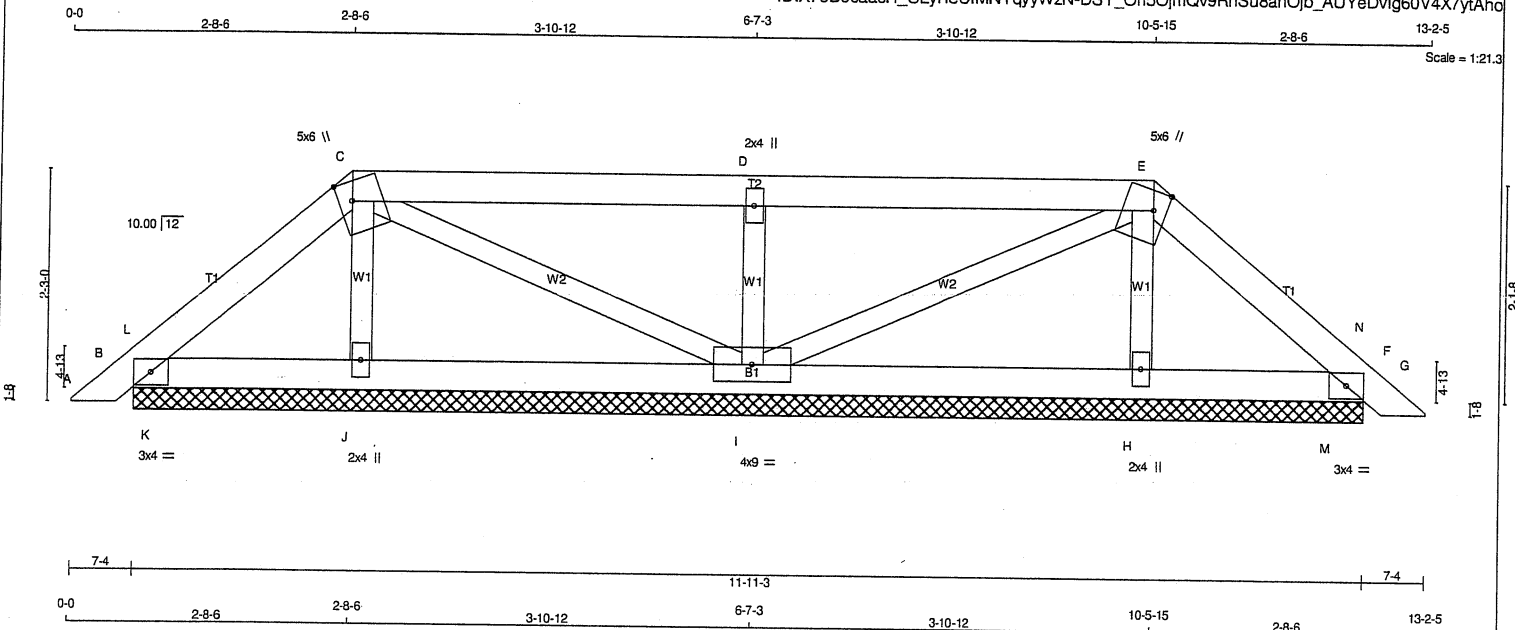


Structural component only
DWG# T-2125200

JOB NAME 420295	TRUSS NAME PB2	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 29 15:35:39 2021 Page 1
ID:X73D9caacH_OLyHeCIMNYqyyWzN-DSY_Oh5OjmQv9RhSu8anOjb_AUyEvlG60V4X7ytAho



LUMBER				TOTAL WEIGHT = 2 X 41 = 81 lb			
N. L. G. A. RULES							
CHORDS	SIZE	LUMBER	DESCR.				
A - C	2x4	DRY	No.2				
C - E	2x4	DRY	No.2				
E - G	2x4	DRY	No.2				
B - F	2x4	DRY	No.2				
ALL WEBS 2x3 DRY No.2							
DRY: SEASONED LUMBER.							

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	
C	TTWW+m	MT20	5.0	6.0	2.25 1.50
D	TWW+w	MT20	2.0	4.0	
E	TTWW+m	MT20	5.0	6.0	2.25 1.50
F	TMB1-I	MT20	3.0	4.0	
H	BMW1+w	MT20	2.0	4.0	
I	BMW1+w	MT20	4.0	9.0	
J	BMW1+w	MT20	2.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	BRG	IN-SX	BRG	IN-SX
B	180	0	180	0	0	11-11-3	11-11-3		
F	180	0	180	0	0	11-11-3	11-11-3		
J	228	0	228	0	0	11-11-3	11-11-3		
I	507	0	507	0	0	11-11-3	11-11-3		
H	228	0	228	0	0	11-11-3	11-11-3		

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	126	92 / 0	0 / 0	0 / 0	0 / 0	34 / 0	0 / 0
F	126	92 / 0	0 / 0	0 / 0	0 / 0	34 / 0	0 / 0
J	164	92 / 0	0 / 0	0 / 0	0 / 0	72 / 0	0 / 0
I	358	237 / 0	0 / 0	0 / 0	0 / 0	121 / 0	0 / 0
H	164	92 / 0	0 / 0	0 / 0	0 / 0	72 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, J, I, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED UNBRACED LENGTH	LC1 MAX
FR-TO		FROM	TO	FR-TO			
A-B	0 / 13	-84.9	-84.9 0.02 (1)	10.00	J-C	-147 / 0	0.02 (1)
B-L	-33 / 0	-84.9	-84.9 0.01 (1)	6.25	C-I	-30 / 0	0.01 (1)
L-C	-61 / 0	-84.9	-84.9 0.04 (1)	6.25	I-D	-413 / 0	0.06 (1)
C-D	-8 / 0	-84.9	-84.9 0.22 (1)	10.00	D-E	-30 / 0	0.01 (1)
D-E	-8 / 0	-84.9	-84.9 0.22 (1)	10.00	E-H	-147 / 0	0.02 (1)
E-N	-61 / 0	-84.9	-84.9 0.04 (1)	6.25	H-K	-125 / 0	0.00 (1)
N-F	-33 / 0	-84.9	-84.9 0.01 (1)	6.25	M-N	-125 / 0	0.00 (1)
F-G	0 / 13	-84.9	-84.9 0.02 (1)	10.00			
B-K	0 / 44	-18.5	-18.5 0.05 (1)	10.00			
K-J	0 / 44	-18.5	-18.5 0.05 (1)	10.00			
J-I	0 / 35	-18.5	-18.5 0.06 (4)	10.00			
I-H	0 / 35	-18.5	-18.5 0.06 (4)	10.00			
H-M	0 / 44	-18.5	-18.5 0.05 (1)	10.00			
M-F	0 / 44	-18.5	-18.5 0.05 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.22/1.00 (D-E:1), BC=0.06/1.00 (H-I:4), WB=0.06/1.00 (D-I:1), SSI=0.16/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

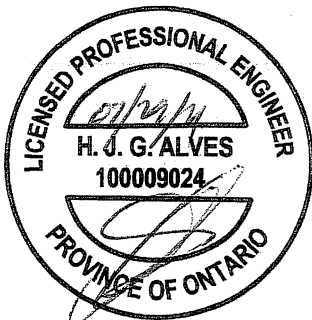
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

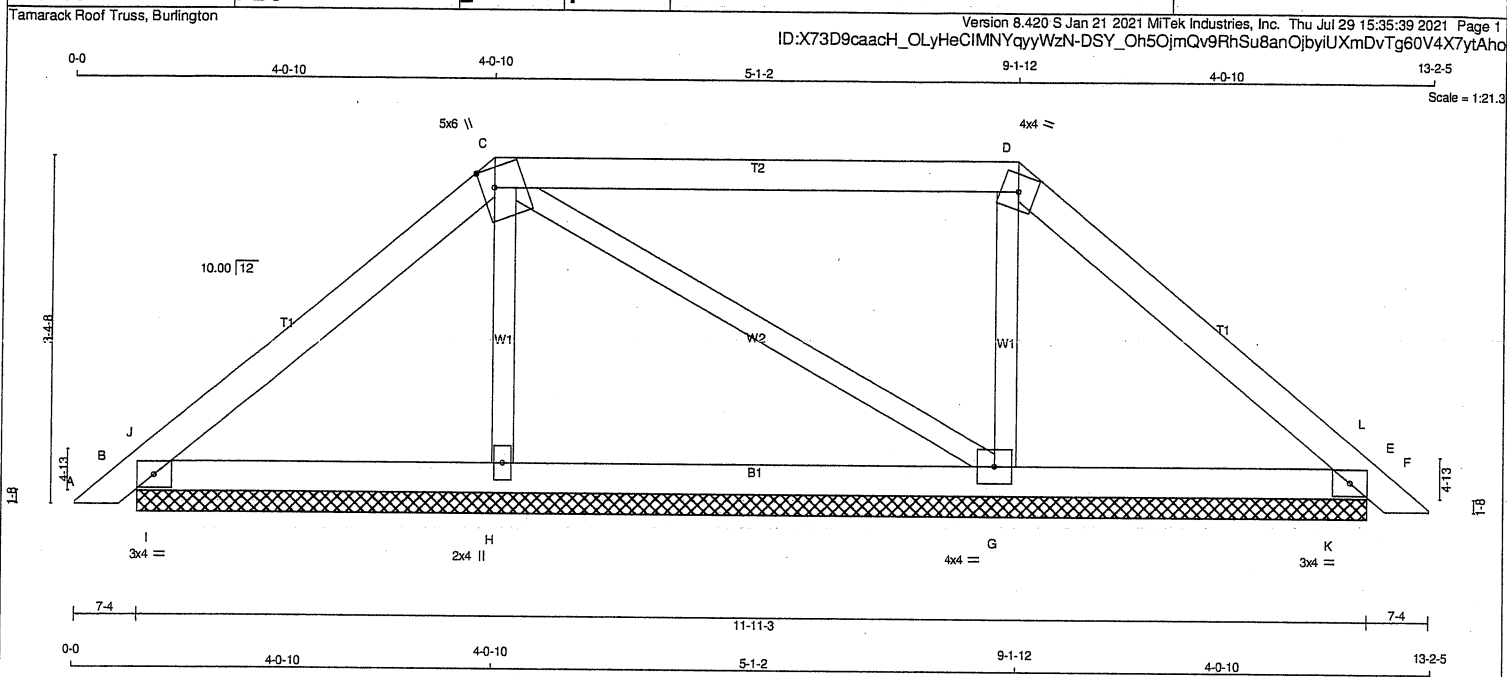
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.21 (D) (INPUT = 0.90)

JSI METAL = 0.09 (D) (INPUT = 1.00)



Structural component only
DWG# T-2125201



TOTAL WEIGHT = 2 X 40 = 80 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4 DRY	No.2	SPF
C - D	2x4 DRY	No.2	SPF
D - F	2x4 DRY	No.2	SPF
B - E	2x4 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-l	MT20	3.0	4.0		
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMB1-l	MT20	3.0	4.0		
G	BMWW1-t	MT20	4.0	4.0		
H	BMWW1-w	MT20	2.0	4.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
B	306	0	306	0	11-11-3	11-11-3
E	285	0	285	0	11-11-3	11-11-3
H	339	0	339	0	11-11-3	11-11-3
G	392	0	392	0	11-11-3	11-11-3

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	214	154/0	0/0	0/0	0/0	61/0	0/0
E	199	142/0	0/0	0/0	0/0	58/0	0/0
H	243	140/0	0/0	0/0	0/0	103/0	0/0
G	280	169/0	0/0	0/0	0/0	111/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO		FROM	TO		FR-TO		
A-B	0/13	-84.9	-84.9 0.02 (1)	10.00	H-C	-228/0	0.04 (1)
B-J	-44/0	-84.9	-84.9 0.06 (1)	6.25	C-G	-31/0	0.02 (1)
J-C	-155/0	-84.9	-84.9 0.12 (1)	6.25	G-D	-265/0	0.05 (1)
C-D	-78/0	-84.9	-84.9 0.38 (1)	6.25	I-J	-295/0	0.00 (1)
D-L	-123/0	-84.9	-84.9 0.12 (1)	6.25	K-L	-299/0	0.00 (1)
L-E	-22/0	-84.9	-84.9 0.05 (1)	6.25			
E-F	0/13	-84.9	-84.9 0.02 (1)	10.00			
B-I	0/113	-18.5	-18.5 0.11 (1)	10.00			
I-H	0/113	-18.5	-18.5 0.11 (1)	10.00			
H-G	0/105	-18.5	-18.5 0.09 (4)	10.00			
G-K	0/88	-18.5	-18.5 0.11 (1)	10.00			
K-E	0/88	-18.5	-18.5 0.11 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.38/1.00 (C-D:1), BC=0.11/1.00 (B-I:1), WB=0.05/1.00 (D-G:1), SSI=0.23/1.00 (E-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

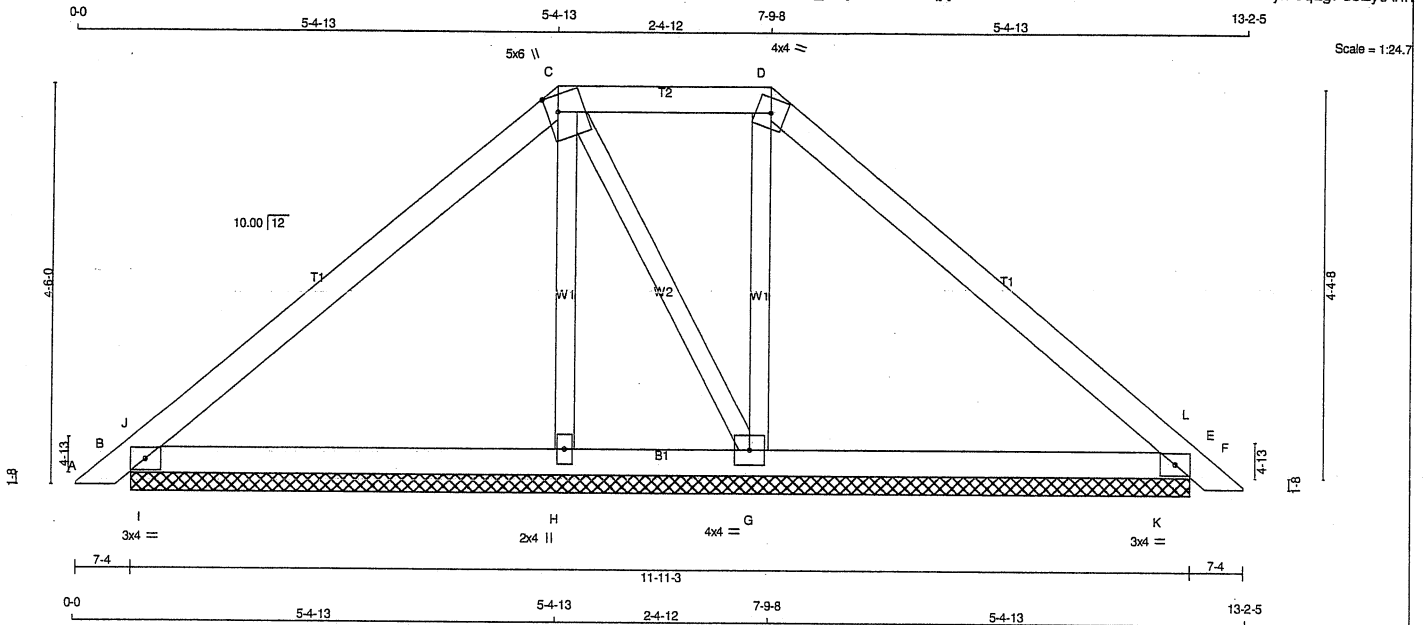


Structural component only
DWG# T-2125202

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
420295	PB4	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 29 15:35:40 2021 Page 1
ID:X73D9caach_OLyHeCIMNYqyWzN-he6Nc160U3YmmbGfSr50xw89XurYyMoqLgFd3ZytAhn



LUMBER					
N. L. G. A. RULES					
CHORDS		SIZE	LUMBER		DESCR
A - C	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
B - E	2x4	DRY	No.2	SPF	
ALL WEBS		2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.					

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B - TMB1-l	MT20	3.0	4.0				
C - TTWW+m	MT20	5.0	6.0	2.25	1.50		
D - TTW-m	MT20	4.0	4.0				
E - TMB1-l	MT20	3.0	4.0				
G - BMWW1-t	MT20	4.0	4.0				
H - BMW1-w	MT20	2.0	4.0				

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
B	414	0	414	0	11-11-3	11-11-3
E	395	0	395	0	11-11-3	11-11-3
H	205	0	205	0	11-11-3	11-11-3
G	308	0	308	0	11-11-3	11-11-3

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	291	199 / 0	0 / 0	0 / 0	0 / 0	92 / 0	0 / 0
E	278	188 / 0	0 / 0	0 / 0	0 / 0	90 / 0	0 / 0
H	148	78 / 0	0 / 0	0 / 0	0 / 0	70 / 0	0 / 0
G	219	139 / 0	0 / 0	0 / 0	0 / 0	80 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM TO				FR-TO			
A-B	0 / 13	-84.9	-84.9	0.02 (1)	10.00	H-C	-97 / 0	0.03 (1)	
B-J	-63 / 32	-84.9	-84.9	0.13 (1)	6.25	C-G	-48 / 0	0.02 (1)	
J-C	-223 / 0	-84.9	-84.9	0.24 (1)	6.25	G-D	-157 / 0	0.04 (1)	
C-D	-135 / 0	-84.9	-84.9	0.08 (1)	6.25	I-J	-548 / 0	0.00 (1)	
D-L	-194 / 0	-84.9	-84.9	0.24 (1)	6.25	K-L	-552 / 0	0.00 (1)	
L-E	-58 / 64	-84.9	-84.9	0.13 (1)	6.25				
E-F	0 / 13	-84.9	-84.9	0.02 (1)	10.00				
B-I	0 / 161	-18.5	-18.5	0.21 (1)	10.00				
I-H	0 / 161	-18.5	-18.5	0.21 (1)	10.00				
H-G	0 / 159	-18.5	-18.5	0.13 (1)	10.00				
G-K	0 / 139	-18.5	-18.5	0.21 (1)	10.00				
K-E	0 / 139	-18.5	-18.5	0.21 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.24/1.00 (C-J:1), BC=0.21/1.00 (B-I:1), WB=0.04/1.00 (D-G:1), SS=0.43/1.00 (E-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

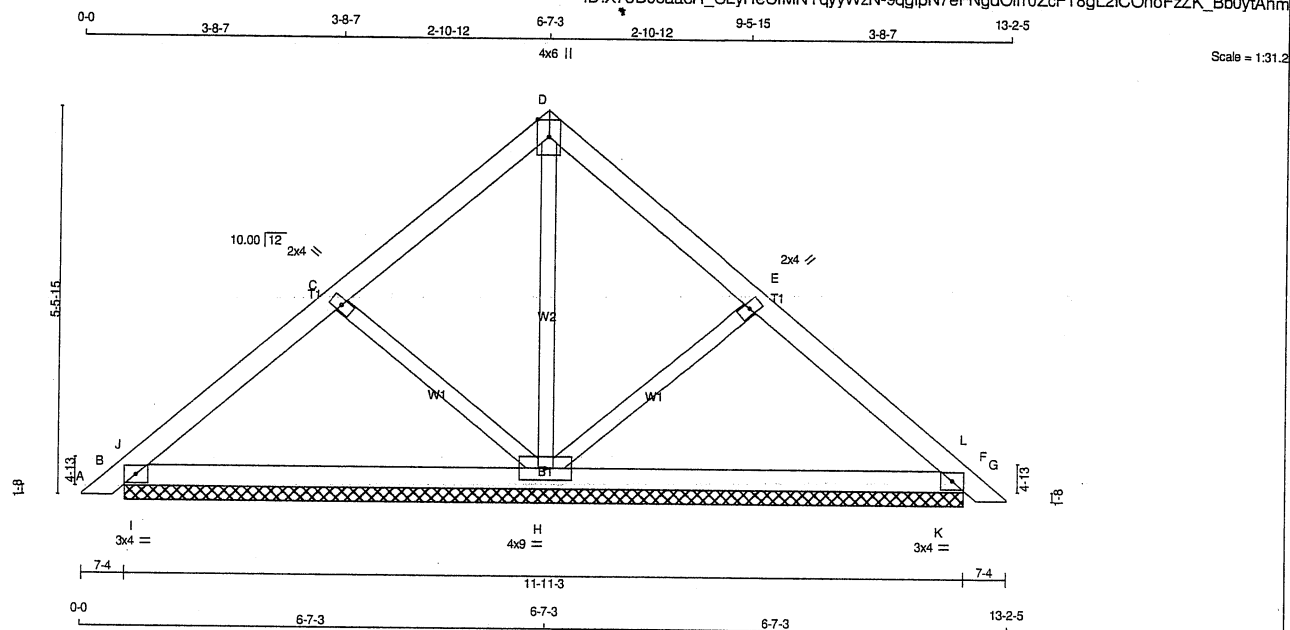
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.33 (B) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)



Structural component only
DWG# T-2125203



TOTAL WEIGHT = $4 \times 43 = 172$ lb

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-l	MT20	3.0	4.0		
C	TMW+w	MT20	2.0	4.0		
D	TTW+p	MT20	4.0	6.0		Edge
E	TMW+w	MT20	2.0	4.0		
F	TMB1-l	MT20	3.0	4.0		
H	BMWVWW1-t	MT20	4.0	9.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BUILDING BEARINGS

JT	FACTORED		MAXIMUM FACTORED		INPUT	REQRD
	GROSS	REACTION	GROSS	REACTION	BRG	BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX
F	320	0	320	0	0	11-11-3
B	320	0	320	0	0	11-11-3
H	681	0	681	0	0	11-11-3

UNFACTORED REACTIONS

1ST CASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	226	150 / 0	0 / 0	0 / 0	0 / 0	77 / 0	0 / 0
F	226	150 / 0	0 / 0	0 / 0	0 / 0	77 / 0	0 / 0
H	484	305 / 0	0 / 0	0 / 0	0 / 0	179 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED			
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO		FR-TO		
A-B	0 / 13	-84.9	-84.9	0.02 (1)	10.00	H-D	-230 / 0
B-J	-307 / 0	-84.9	-84.9	0.07 (4)	6.25	H-E	-246 / 0
J-C	-187 / 0	-84.9	-84.9	0.13 (1)	6.25	C-H	-246 / 0
C-D	-5 / 10	-84.9	-84.9	0.13 (1)	10.00	I-J	0 / 143
D-E	-5 / 10	-84.9	-84.9	0.13 (1)	10.00	K-L	0 / 143
E-L	-187 / 0	-84.9	-84.9	0.13 (1)	6.25		
L-F	-307 / 0	-84.9	-84.9	0.07 (4)	6.25		
F-G	0 / 13	-84.9	-84.9	0.02 (1)	10.00		
B-I	0 / 164	-18.5	-18.5	0.06 (4)	10.00		
I-H	0 / 164	-18.5	-18.5	0.17 (4)	10.00		
H-K	0 / 164	-18.5	-18.5	0.17 (4)	10.00		
K-F	0 / 164	-18.5	-18.5	0.06 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR
SMALL BUILDING REQUIREMENTS OF PART 9,
NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13/1.00 (C-D:1), BC=0.17/1.00 (H-K:4), WB=0.10/1.00 (D-H:1), SSI=0.11/1.00 (B-I:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

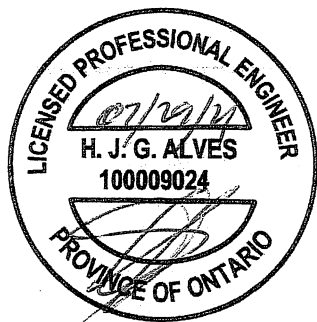
NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

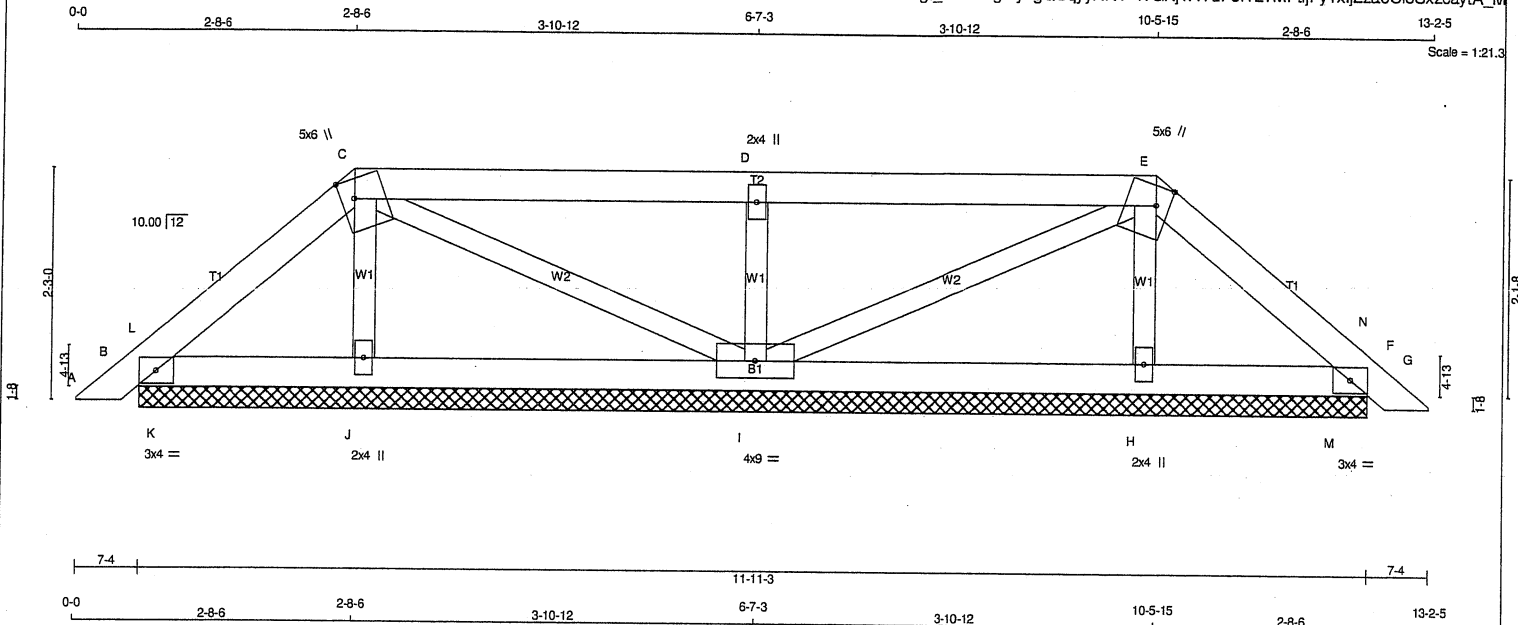


Structural component only
DWG# T-2125204

JOB NAME 420300	TRUSS NAME PB50	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 29 16:24:07 2021 Page 1
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LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				TOTAL WEIGHT = 2 X 41 = 81 lb			
N. L. G. A. RULES				BEARINGS							
CHORDS	SIZE	LUMBER	DESCR.	FACTORED	GROSS REACTION	MAXIMUM FACTORED	INPUT	REQD			
A - C	2x4	DRY	No.2	SPF	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	IN-SX
C - E	2x4	DRY	No.2	SPF	180	0	180	0	0	11-11-3	11-11-3
E - G	2x4	DRY	No.2	SPF	180	0	180	0	0	11-11-3	11-11-3
B - F	2x4	DRY	No.2	SPF	228	0	228	0	0	11-11-3	11-11-3
					507	0	507	0	0	11-11-3	11-11-3
					228	0	228	0	0	11-11-3	11-11-3
ALL WEBS 2x3 DRY No.2 SPF											
DRY: SEASONED LUMBER.											

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	
C	TTWW+m	MT20	5.0	6.0	2.25 1.50
D	TMW+w	MT20	2.0	4.0	
E	TTWW+m	MT20	5.0	6.0	2.25 1.50
F	TMB1-I	MT20	3.0	4.0	
H	BMW1+w	MT20	2.0	4.0	
I	BMWVWV-I	MT20	4.0	9.0	
J	BMW1+w	MT20	2.0	4.0	

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

UNFACTORED REACTIONS							
JT	1ST CASE	MAX/MIN. COMPONENT REACTIONS					
B	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	126	92/0	0/0	0/0	0/0	34/0	0/0
J	126	92/0	0/0	0/0	0/0	34/0	0/0
I	164	92/0	0/0	0/0	0/0	72/0	0/0
H	358	237/0	0/0	0/0	0/0	121/0	0/0
	164	92/0	0/0	0/0	0/0	72/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, J, I, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0/13	-84.9 -84.9	0.02 (1)	10.00	J-C	-147/0	0.02 (1)
B-L	-33/0	-84.9 -84.9	0.01 (1)	6.25	C-I	-30/0	0.01 (1)
L-C	-61/0	-84.9 -84.9	0.04 (1)	6.25	I-D	-413/0	0.06 (1)
C-D	-8/0	-84.9 -84.9	0.22 (1)	10.00	D-E	-30/0	0.01 (1)
D-E	-8/0	-84.9 -84.9	0.22 (1)	10.00	E-H	-147/0	0.02 (1)
E-N	-61/0	-84.9 -84.9	0.04 (1)	6.25	K-L	-125/0	0.00 (1)
N-F	-33/0	-84.9 -84.9	0.01 (1)	6.25	M-N	-125/0	0.00 (1)
F-G	0/13	-84.9 -84.9	0.02 (1)	10.00			
B-K	0/44	-18.5 -18.5	0.05 (1)	10.00			
K-J	0/44	-18.5 -18.5	0.05 (1)	10.00			
J-I	0/35	-18.5 -18.5	0.06 (4)	10.00			
I-H	0/35	-18.5 -18.5	0.06 (4)	10.00			
H-M	0/44	-18.5 -18.5	0.05 (1)	10.00			
M-F	0/44	-18.5 -18.5	0.05 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.22/1.00 (C-D:1), BC=0.06/1.00 (H-I:4), WB=0.06/1.00 (D-I:1), SS=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

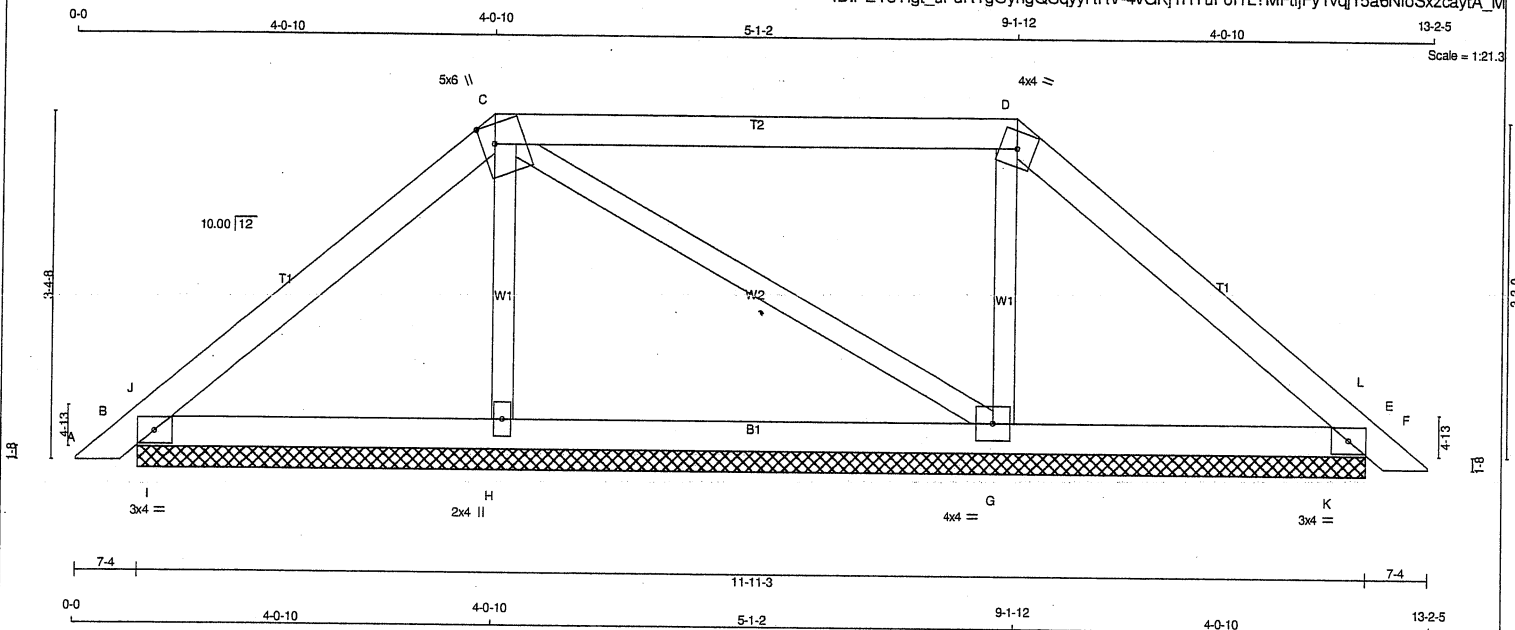
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (D) (INPUT = 0.90)
JSI METAL= 0.09 (D) (INPUT = 1.00)



Structural component only
DWG# T-2125256

JOB NAME 420300	TRUSS NAME PB51	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 29 16:24:07 2021 Page 1 ID:PEY5Tigt_uPuRtgOyngQSqyyRRV-4vGKj1HTuFol1L?MFtjFy1vqj15a6NioSxzcaytA_M			



LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				TOTAL WEIGHT = 2 X 40 = 80 lb			
N. L. G. A. RULES				BEARINGS				DESIGN CRITERIA			
CHORDS	SIZE	LUMBER	DESCR.	FACTORED	MAXIMUM FACTORED	INPUT	REQD	SPECIFIED LOADS:			
A - C	2x4	DRY	No.2	SPF	GROSS REACTION	GROSS REACTION	BRG	TOP CH. LL = 23.3 PSF			
C - D	2x4	DRY	No.2	SPF	VERT	HORZ	UPLIFT	DL = 6.0 PSF			
D - F	2x4	DRY	No.2	SPF	B	E	H	BOT CH. LL = 0.0 PSF			
B - E	2x4	DRY	No.2	SPF	306	0	306	DL = 7.4 PSF			
ALL WEBS 2x3 DRY: SEASONED LUMBER.				H				TOTAL LOAD = 36.7 PSF			
				G				SPACING = 24.0 IN./C			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	
C	TTWW+m	MT20	5.0	6.0	2.25 1.50
D	TTW-m	MT20	4.0	4.0	
E	TMB1-I	MT20	3.0	4.0	
G	BMW1-t	MT20	4.0	4.0	
H	BMW1-w	MT20	2.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

UNFACTORED REACTIONS							
JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	214	154 / 0	0 / 0	0 / 0	0 / 0	61 / 0	0 / 0
E	199	142 / 0	0 / 0	0 / 0	0 / 0	58 / 0	0 / 0
H	243	140 / 0	0 / 0	0 / 0	0 / 0	103 / 0	0 / 0
G	280	169 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX LC1 (LC)
FR-TO		FROM	TO		FR-TO		
A-B	0 / 13	-84.9	-84.9 0.02 (1)	10.00	H-C	-228 / 0	0.04 (1)
B-J	-44 / 0	-84.9	-84.9 0.06 (1)	6.25	C-G	-31 / 0	0.02 (1)
J-C	-155 / 0	-84.9	-84.9 0.12 (1)	6.25	G-D	-265 / 0	0.05 (1)
C-D	-78 / 0	-84.9	-84.9 0.38 (1)	6.25	I-J	-295 / 0	0.00 (1)
D-L	-123 / 0	-84.9	-84.9 0.12 (1)	6.25	K-L	-299 / 0	0.00 (1)
L-E	-22 / 0	-84.9	-84.9 0.05 (1)	6.25			
E-F	0 / 13	-84.9	-84.9 0.02 (1)	10.00			
B-I	0 / 113	-18.5	-18.5 0.11 (1)	10.00			
I-H	0 / 113	-18.5	-18.5 0.11 (1)	10.00			
H-G	0 / 105	-18.5	-18.5 0.09 (4)	10.00			
G-K	0 / 88	-18.5	-18.5 0.11 (1)	10.00			
K-E	0 / 88	-18.5	-18.5 0.11 (1)	10.00			

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.38/1.00 (C-D:1), BC=0.11/1.00 (B-I:1), WB=0.05/1.00 (D-G:1), SSI=0.23/1.00 (E-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

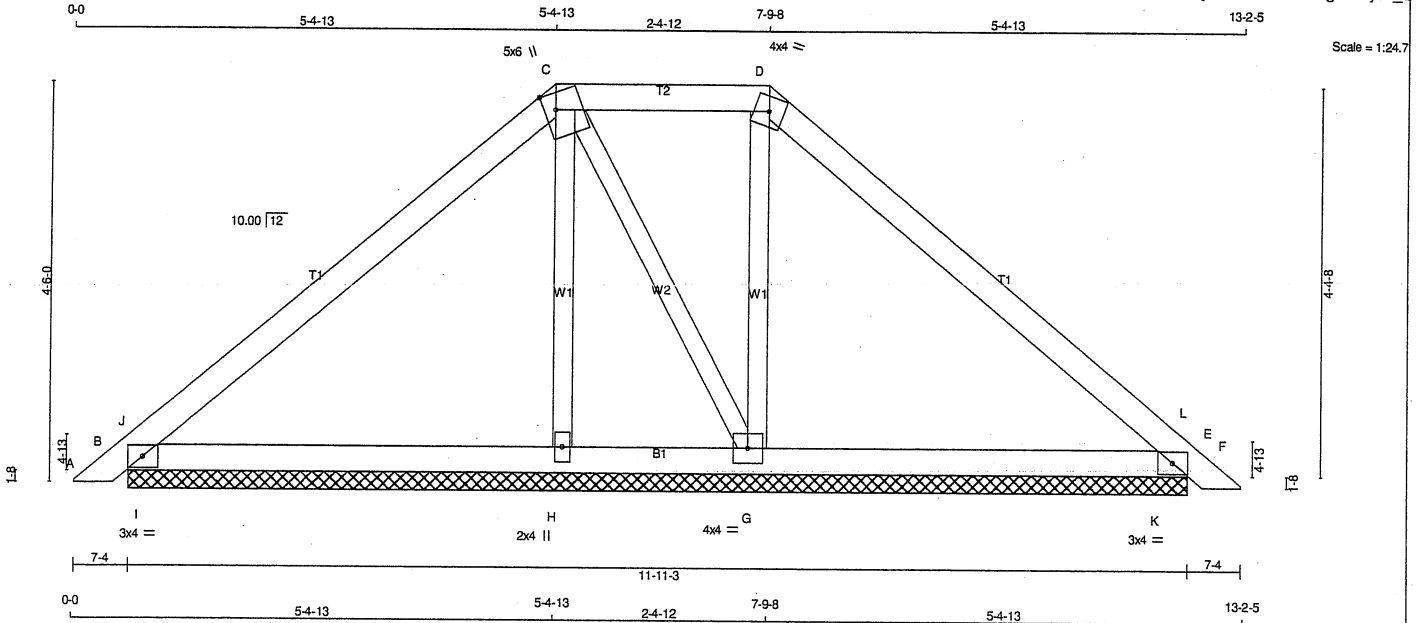


Structural component only
DWG# T-2125257

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
420300	PB52	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 29 16:24:08 2021 Page 1
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LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS	2x4	DRY	No.2
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
B - E	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-t	MT20	3.0	4.0	
C	TTWW+m	MT20	5.0	6.0	2.25 1.50
D	TTW-m	MT20	4.0	4.0	
E	TMB1-t	MT20	3.0	4.0	
G	BMW1-t	MT20	4.0	4.0	
H	BMW1-w	MT20	2.0	4.0	

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	DOWN	BRG	BRG
B	414	0	414	0	11-11-3
E	395	0	395	0	11-11-3
H	205	0	205	0	11-11-3
G	308	0	308	0	11-11-3

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	291	199 / 0	0 / 0	0 / 0	0 / 0	92 / 0	0 / 0
E	278	188 / 0	0 / 0	0 / 0	0 / 0	90 / 0	0 / 0
H	148	78 / 0	0 / 0	0 / 0	0 / 0	70 / 0	0 / 0
G	219	139 / 0	0 / 0	0 / 0	0 / 0	80 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		W E B S	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)
FR-TO		FROM TO	LENGTH	FR-TO	
A-B	0 / 13	-84.9 -84.9	0.02 (1)	H-C	-97 / 0
B-J	-63 / 32	-84.9 -84.9	0.13 (1)	C-G	-48 / 0
J-C	-223 / 0	-84.9 -84.9	0.24 (1)	G-D	-157 / 0
C-D	-135 / 0	-84.9 -84.9	0.08 (1)	I-J	-548 / 0
D-L	-194 / 0	-84.9 -84.9	0.24 (1)	K-L	-552 / 0
L-E	-58 / 64	-84.9 -84.9	0.13 (1)		
E-F	0 / 13	-84.9 -84.9	0.02 (1)		
B-I	0 / 161	-18.5 -18.5	0.21 (1)		
I-H	0 / 161	-18.5 -18.5	0.21 (1)		
H-G	0 / 159	-18.5 -18.5	0.13 (1)		
G-K	0 / 139	-18.5 -18.5	0.21 (1)		
K-E	0 / 139	-18.5 -18.5	0.21 (1)		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.24/1.00 (C-J:1), BC=0.21/1.00 (B-I:1), WB=0.04/1.00 (D-G:1), SS=0.43/1.00 (E-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

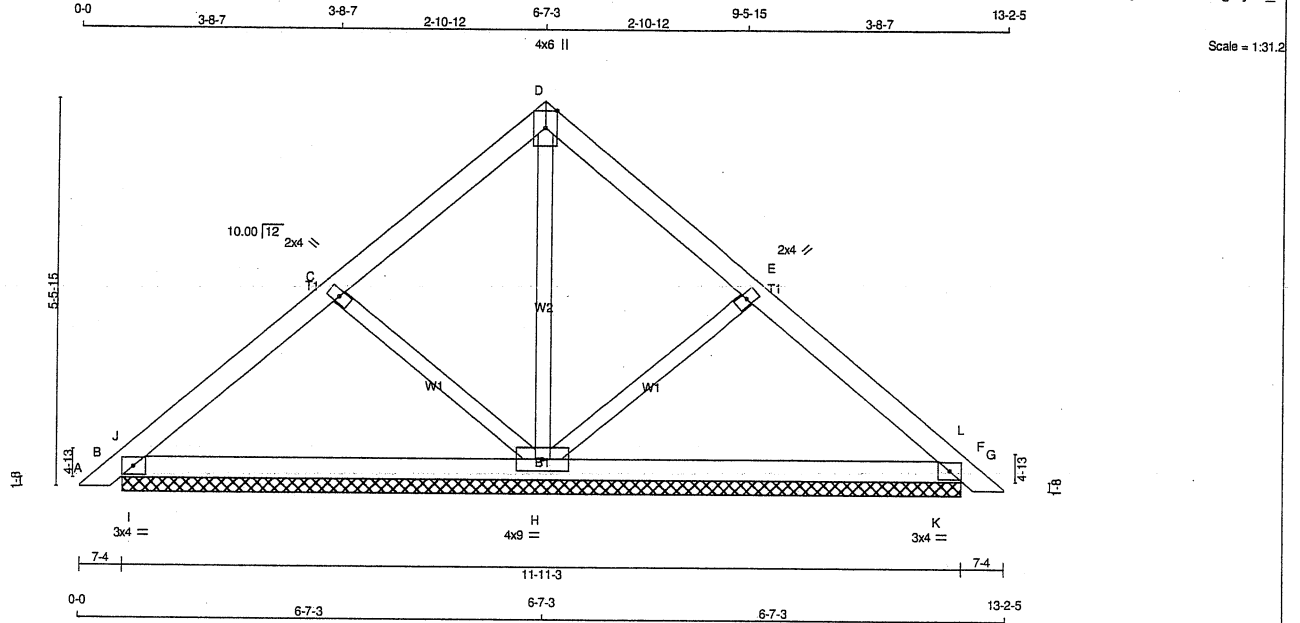
JSI GRIP= 0.33 (B) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)



Structural component only
DWG# T-2125258

JOB NAME 420300	TRUSS NAME PB53	QUANTITY 4	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.420 S Jan 21 2021 MITek Industries, Inc. Thu Jul 29 16:24:09 2021 Page 1
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TOTAL WEIGHT = 4 X 43 = 172 lb

LUMBER				N. L. G. A. RULES	
CHORDS	SIZE	LUMBER	DESCR.	SPF	SPF
A - D	2x4	DRY	No.2	SPF	SPF
D - G	2x4	DRY	No.2	SPF	SPF
B - F	2x4	DRY	No.2	SPF	SPF
ALL WEBS 2x3 DRY				No.2	SPF
DRY: SEASONED LUMBER.					

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	
C	TMW+u	MT20	2.0	4.0	
D	TTW+p	MT20	4.0	6.0	Edge
E	TMW+u	MT20	2.0	4.0	
F	TMB1-I	MT20	3.0	4.0	
H	BMWVW1-I	MT20	4.0	9.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
GROSS REACTION		VERT	HORZ	GROSS REACTION		BRG	IN-SX	BRG	IN-SX
JT		320	0	320	0	0	11-11-3	11-11-3	11-11-3
B		320	0	320	0	0	11-11-3	11-11-3	11-11-3
F		320	0	320	0	0	11-11-3	11-11-3	11-11-3
H		681	0	681	0	0	11-11-3	11-11-3	11-11-3

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
B	226	150 / 0	0 / 0	0 / 0	0 / 0	77 / 0	0 / 0	
F	226	150 / 0	0 / 0	0 / 0	0 / 0	77 / 0	0 / 0	
H	484	305 / 0	0 / 0	0 / 0	0 / 0	179 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX. FACTORED		WEBS		FACTORED		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	LC1 MAX	UNBRAC	MEMB.	FORCE (LBS)	MAX	LC1 MAX	MEMB.	FORCE (LBS)
FR-TO		FROM	TO	LENGTH	FR-TO						
A-B	0 / 13	-84.9	-84.9	0.02 (1)	10.00	H-D	-230 / 0	0.10 (1)			
B-J	-307 / 0	-84.9	-84.9	0.07 (4)	6.25	H-E	-246 / 0	0.07 (1)			
J-C	-187 / 0	-84.9	-84.9	0.13 (1)	6.25	C-H	-246 / 0	0.07 (1)			
C-D	-5 / 10	-84.9	-84.9	0.13 (1)	10.00	I-J	0 / 143	0.00 (1)			
D-E	-5 / 10	-84.9	-84.9	0.13 (1)	10.00	K-L	0 / 143	0.00 (1)			
E-L	-187 / 0	-84.9	-84.9	0.13 (1)	6.25						
L-F	-307 / 0	-84.9	-84.9	0.07 (4)	6.25						
F-G	0 / 13	-84.9	-84.9	0.02 (1)	10.00						
B-I	0 / 164	-18.5	-18.5	0.06 (4)	10.00						
I-H	0 / 164	-18.5	-18.5	0.17 (4)	10.00						
H-K	0 / 164	-18.5	-18.5	0.17 (4)	10.00						
K-F	0 / 164	-18.5	-18.5	0.06 (4)	10.00						

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13/1.00 (C-D:1), BC=0.17/1.00 (H-I:4), WB=0.10/1.00 (D-H:1), SSI=0.11/1.00 (B-I:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

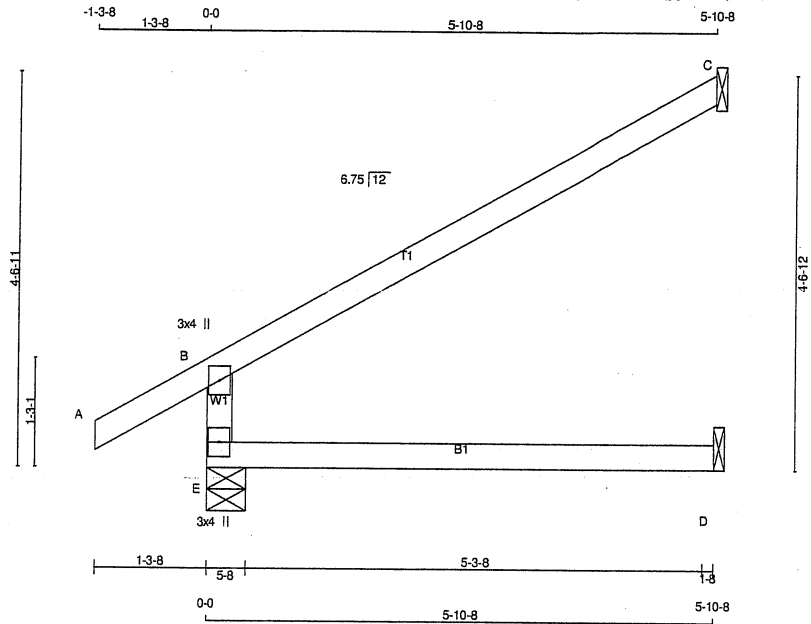
JSI GRIP= 0.22 (B) (INPUT = 0.90)
JSI METAL= 0.07 (F) (INPUT = 1.00)



Structural component only
DWG# T-2125259

JOB NAME 420295	TRUSS NAME J1	QUANTITY 18	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 29 15:35:36 2021 Page 1
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TOTAL WEIGHT = 18 X 17 = 309 lb [M]

LUMBER				DESCR.
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
E	491	0	491	0	0	5-8	5-8	
C	187	0	187	0	0	1-8	1-8	
D	45	0	51	0	0	1-8	1-8	

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS		PERM. LIVE	WIND	DEAD	SOIL
	SNOW	LIVE	SNOW	LIVE				
E	346	234 / 0	0 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0
C	129	103 / 0	0 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		WEBS		MAX. FACTORED	
	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	LC1 MAX	MEMB. UNBRAC LENGTH FR-TO	FORCE (LBS)	MAX	LC1 MAX
FR-TO		FROM TO						
E-B	-427 / 0	0.0	0.0	0.13 (4)	7.81			
A-B	0 / 28	-84.9	-84.9	0.11 (1)	10.00			
B-C	-31 / 0	-84.9	-84.9	0.50 (1)	6.25			
E-D	0 / 0	-18.5	-18.5	0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.50/1.00 (B-C:1), BC=0.13/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.21/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

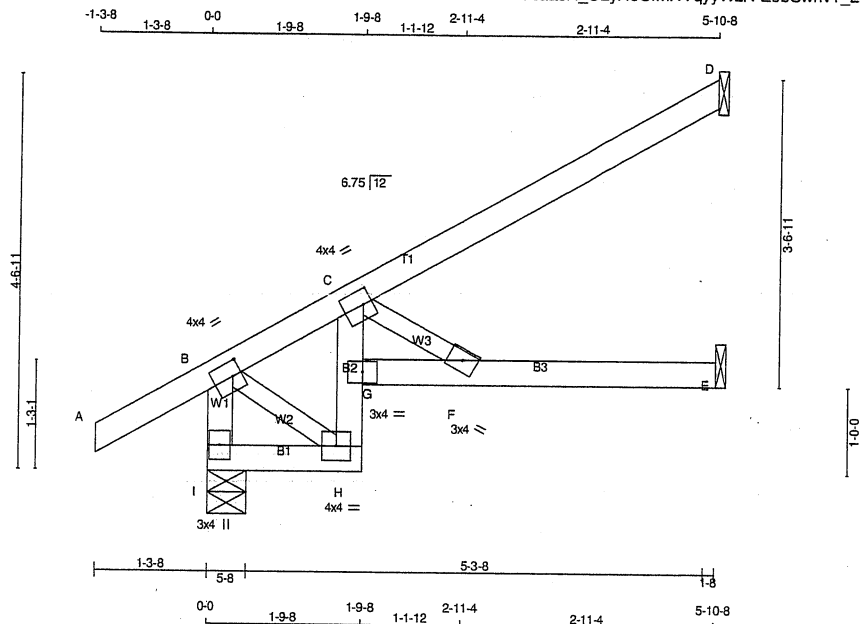
JSI GRIP= 0.17 (E) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)



Structural component only
DWG# T-2125198

JOB NAME 420297	TRUSS NAME J1S	QUANTITY 8	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.420 S Jan 21 2021 MITek Industries, Inc. Thu Jul 29 15:42:17 2021 Page 1
ID:X73D9caacH_OLyHeCIMNYqyyWzN-Z0bSwnv1_2ZRuc7yCj7We4_0wJiUwqFdxv8V3ytAba



TOTAL WEIGHT = 8 X 21 = 171 lb [M]

LUMBER					DESCR.
N. L. G. A. RULES	CHORDS	SIZE	LUMBER		
I - B	2x4	DRY	No.2	SPF	
A - D	2x4	DRY	No.2	SPF	
I - H	2x4	DRY	No.2	SPF	
H - C	2x4	DRY	No.2	SPF	
G - E	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TMVW-t	MT20	4.0	4.0	2.00	1.25
F	BMW+w	MT20	3.0	4.0		
G	BMVH	MT20	3.0	4.0		
H	BMVW-t	MT20	4.0	4.0		
I	BMV1+p	MT20	3.0	4.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT REQD	
	VERT	HORZ	DOWN	HORZ	BRG	IN-SX
I	433	0	433	0	5-8	5-8
D	181	0	181	0	1-8	1-8
E	109	0	109	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) D, E

UNFACTORED REACTIONS

JT	1ST LCASE SNOW		MAX/MIN. LIVE		PERM. LIVE		WIND	DEAD	SOIL
	COMBINED								
I	305	208 / 0	0 / 0	0 / 0	0 / 0	0 / 0	97 / 0	0 / 0	
D	126	95 / 0	0 / 0	0 / 0	0 / 0	0 / 0	31 / 0	0 / 0	
E	80	34 / 0	0 / 0	0 / 0	0 / 0	0 / 0	46 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)		FACTORED VERT. LOAD (PLF)		MAX. FACTORED CSI (LC)		WEBS MAX. FACTORED FORCE (LBS)		MAX. FACTORED CSI (LC)
	FR-TO		FROM TO		UNBRAC LENGTH		FR-TO		
I-B	-417 / 0		0.0	0.0	0.04 (1)	7.81	B-H	0 / 182	0.04 (1)
A-B	0 / 28		-84.9	-84.9	0.11 (1)	10.00	C-F	-410 / 0	0.06 (1)
B-C	-207 / 0		-84.9	-84.9	0.11 (1)	6.25			
C-D	0 / 4		-84.9	-84.9	0.26 (1)	10.00			
I-H	0 / 0		-18.5	-18.5	0.01 (4)	10.00			
H-G	-75 / 0		0.0	0.0	0.21 (1)	7.81			
G-C	0 / 123		0.0	0.0	0.23 (1)	10.00			
G-F	0 / 340		-18.5	-18.5	0.38 (1)	10.00			
F-E	0 / 0		-18.5	-18.5	0.33 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/595 (0.12")

CSI: TC=0.26/1.00 (C-D:1), BC=0.38/1.00 (F-G:1), WB=0.06/1.00 (C-F:1), SSI=0.15/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

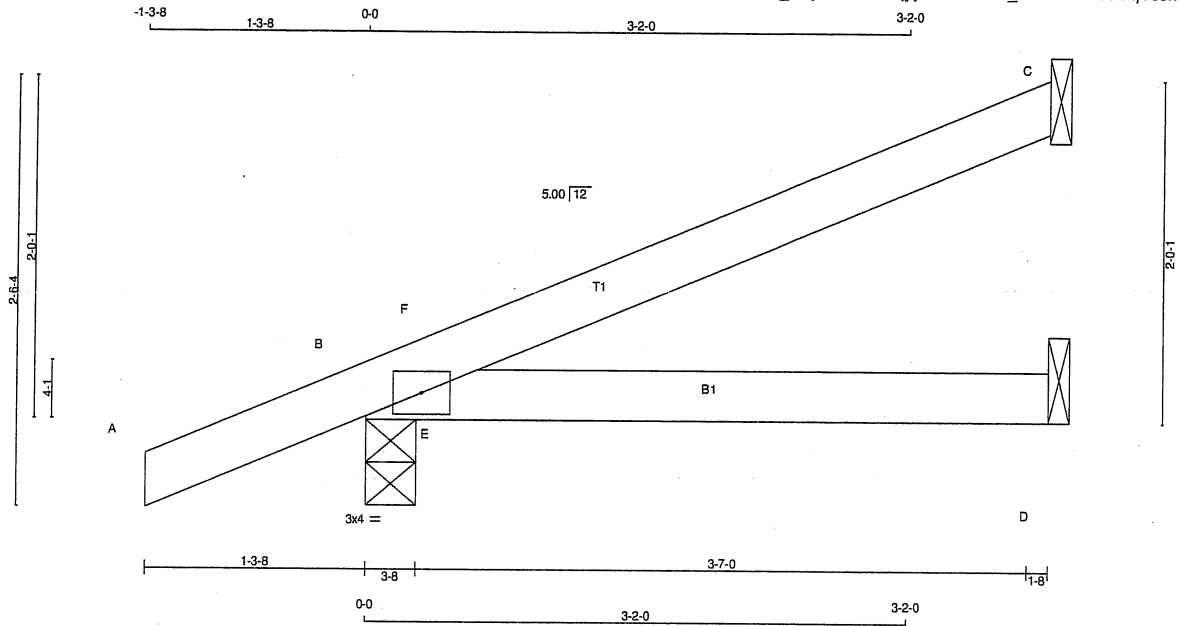
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.28 (C) (INPUT = 0.90)
JSI METAL=0.15 (F) (INPUT = 1.00)



Structural component only
DWG# T-2125223



TOTAL WEIGHT = 10 X 11 = 112 lb [M]

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS	2x4	DRY	No.2
A - C	2x4	DRY	No.2
B - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMB1-I	MT20	3.0	4.0

NOTES: (1)
 1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	HORZ	BRG	IN-SX	BRG	IN-SX
C	148	0	148	0	0	1-8	1-8	1-8	1-8
B	321	0	321	0	0	3-8	3-8	3-8	3-8
D	59	0	59	0	0	1-8	1-8	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST LOOSE		MAX./MIN. COMPONENT REACTIONS		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	DEAD	SOIL
C	102	78 / 0	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0	25 / 0	0 / 0
B	226	156 / 0	0 / 0	0 / 0	0 / 0	70 / 0	0 / 0	70 / 0	0 / 0
D	44	15 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0	29 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		W E B S		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	MAX. UNBRACED LENGTH	MEMB.	FORCE (LBS)	MAX. UNBRACED LENGTH
FR-TO					FR-TO		
A-B	0 / 21	-84.9	-84.9 0.11 (1)	10.00	E-F	-182 / 6	0.00 (1)
B-F	-15 / 5	-84.9	-84.9 0.04 (4)	6.25			
F-C	0 / 2	-84.9	-84.9 0.17 (1)	10.00			
B-E	0 / 0	-18.5	-18.5 0.13 (1)	10.00			
E-D	0 / 0	-18.5	-18.5 0.13 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 23.3 PSF
 DL = 6.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBC 2018, ABC 2019
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 086-14
 - TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
 ALLOWABLE DEFL.(TL)= L/360 (0.19")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.17/1.00 (C-F:1), BC=0.13/1.00 (D-E:1),
 WB=0.00/1.00 (E-F:1), SS=0.15/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

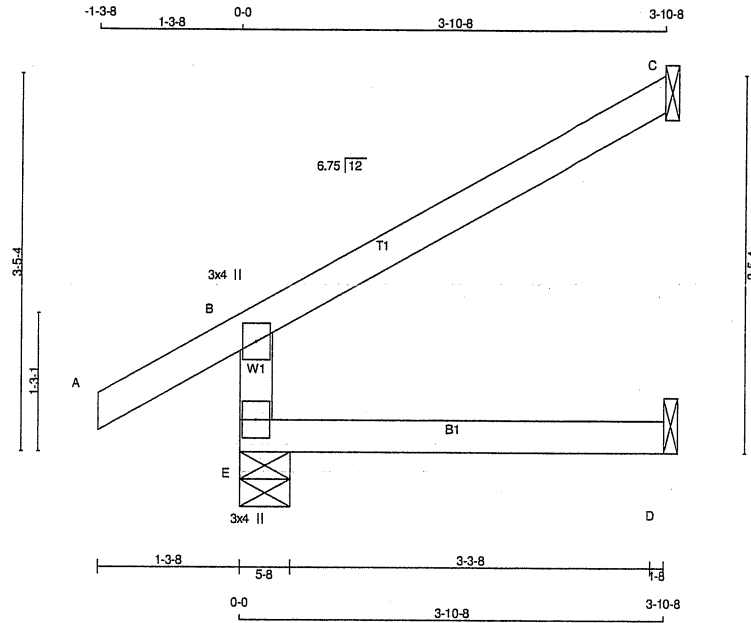
JSI GRIP= 0.23 (B) (INPUT = 0.90)
 JSI METAL= 0.05 (B) (INPUT = 1.00)



Structural component only
 DWG# T-2125199

JOB NAME 420298	TRUSS NAME J31	QUANTITY 8	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 29 15:59:30 2021 Page 1
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Scale = 1:20.0

TOTAL WEIGHT = 8 X 12 = 98 lb

LUMBER				DESCR.
N. L. G. A. RULES	SIZE	LUMBER		
CHORDS				SPF
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	3.0	4.0
E	BMV1+p	MT20	3.0	4.0

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	362	0	362	0	5-8	5-8
C	123	0	123	0	1-8	1-8
D	31	0	34	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	254	176 / 0	0 / 0	0 / 0	0 / 0	78 / 0	0 / 0
C	85	68 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	25	0 / 0	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	MAX. CSI (LC)	UNBRAC LENGTH	MEMB. MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO							
E-B	-321 / 0	0.0	0.0	0.05 (4)	7.81		
A-B	0 / 28	-84.9	-84.9	0.13 (5)	10.00		
B-C	-20 / 0	-84.9	-84.9	0.22 (1)	6.25		
E-D	0 / 0	-18.5	-18.5	0.06 (4)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.22/1.00 (B-C:1), BC=0.06/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

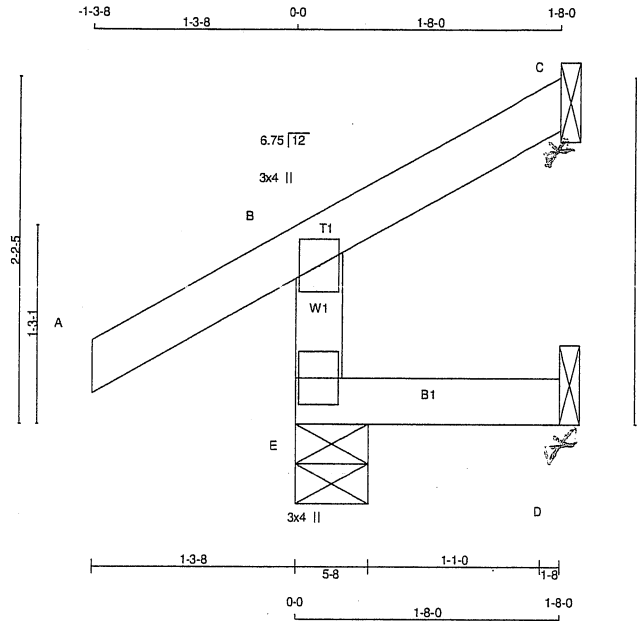
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (E) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)



Structural component only
DWG# T-2125235

JOB NAME 420299	TRUSS NAME J32	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed Aug 4 10:52:47 2021 Page 1 ID:oUu4oI0DmAZdCEGz17gXJEyyT7V-aynT8JNZjX37zBmv9_XWxdVZ9Dr6yUIKPSobyrGH			



TOTAL WEIGHT = 2 X 7 = 14 lb

LUMBER			
N. L. G. A. RULES	SIZE	DRY	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
E	249	0	249	0	5-8	5-8
C	34	0	34	0	1-8	1-8
D	5	0	14	0	1-8	1-8

SEE MITK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	COMBINED	MAX./MIN. COMPONENT REACTIONS					
		1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD
E	174	128 / 0	0 / 0	0 / 0	0 / 0	45 / 0	0 / 0
C	23	18 / -18	0 / 0	0 / 0	0 / 0	5 / 0	0 / 0
D	5	0 / -8	0 / 0	0 / 0	0 / 0	10 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS		FACTORED		WEBS	
MEMB.	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	MEMB.	MAX. FORCE (LBS)
FR-TO		FROM	TO	FR-TO	
E-B	-224 / 0	0.0	0.0 0.03 (5)	7.81	
A-B	0 / 28	-84.9	-84.9 0.11 (1)	10.00	
B-C	-18 / 0	-84.9	-84.9 0.08 (1)	6.25	
E-D	0 / 0	-18.5	-18.5 0.03 (5)	10.00	

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:		
TOP CH.	LL =	23.3 PSF
	DL =	6.0 PSF
BOT CH.	LL =	0.0 PSF
	DL =	7.4 PSF
TOTAL LOAD	=	36.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.11/1.00 (A-B:1), BC=0.03/1.00 (D-E:5),
WB=0.00/1.00 (n/a:0), SSI=0.08/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

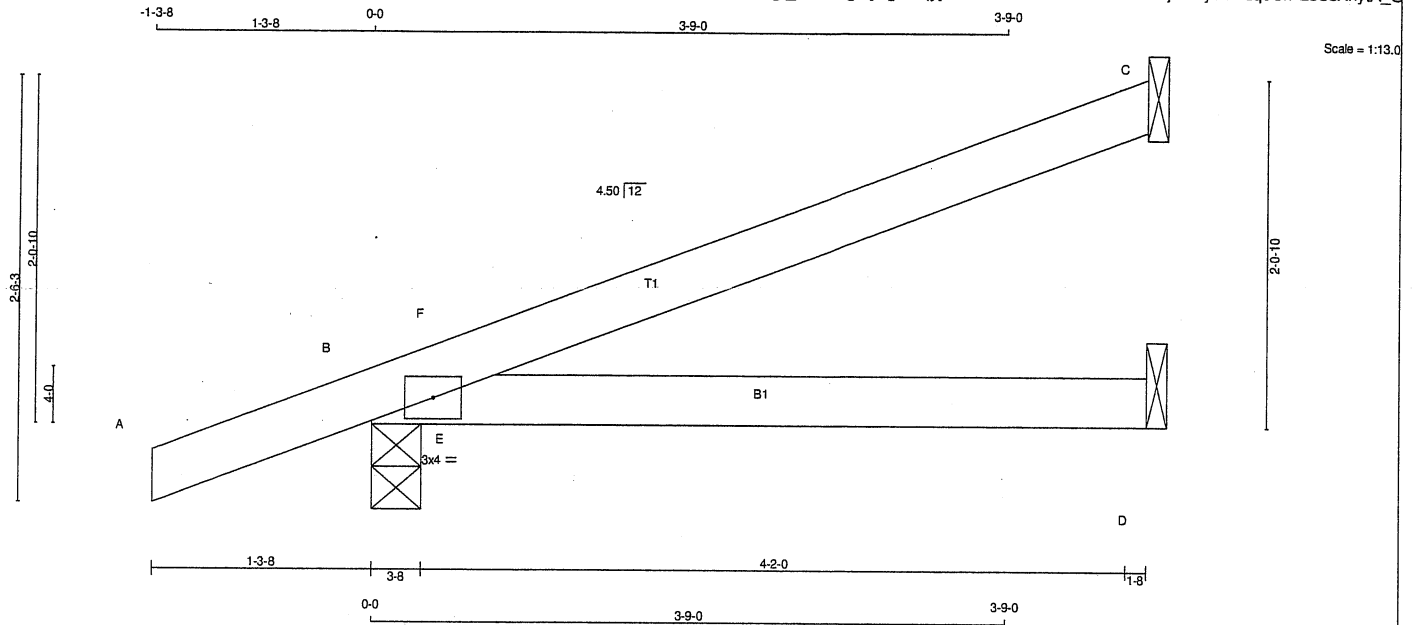
JSI GRIP= 0.09 (E) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)



Structural component only
DWG# T-2126375

JOB NAME 420300	TRUSS NAME J51	QUANTITY 6	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 29 16:24:05 2021 Page 1
ID:PEY5Tlgt_uPuRTgOyngQSqyyRRV-8W8aLGDMDYan1rz7SjFAXybbwLq6CfPL8SsXhytA_C



TOTAL WEIGHT = 6 X 13 = 75 lb

LUMBER				DESCR.	SPF
N.L.G.A. RULES	CHORDS	SIZE	LUMBER		
A - C	2x4	DRY	No.2		
B - D	2x4	DRY	No.2		

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y
B	TMB1-I	MT20	3.0	4.0	X

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX
C	170	0	170	0	1-8	1-8
B	351	0	351	0	3-8	3-8
D	67	0	67	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
C	118	90 / 0	0 / 0	0 / 0	0 / 0	28 / 0	0 / 0
B	247	169 / 0	0 / 0	0 / 0	0 / 0	78 / 0	0 / 0
D	50	17 / 0	0 / 0	0 / 0	0 / 0	33 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	0/19	-84.9	-84.9	0.11 (1)	10.00	E-F	-211/8
B-F	-16/6	-84.9	-84.9	0.05 (4)	6.25		0.00 (1)
F-C	0/2	-84.9	-84.9	0.23 (1)	10.00		
B-E	0/0	-18.5	-18.5	0.16 (1)	10.00		
E-D	0/0	-18.5	-18.5	0.17 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.23/1.00 (C-F:1), BC=0.17/1.00 (D-E:1), WB=0.00/1.00 (E-F:1), SS=0.18/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

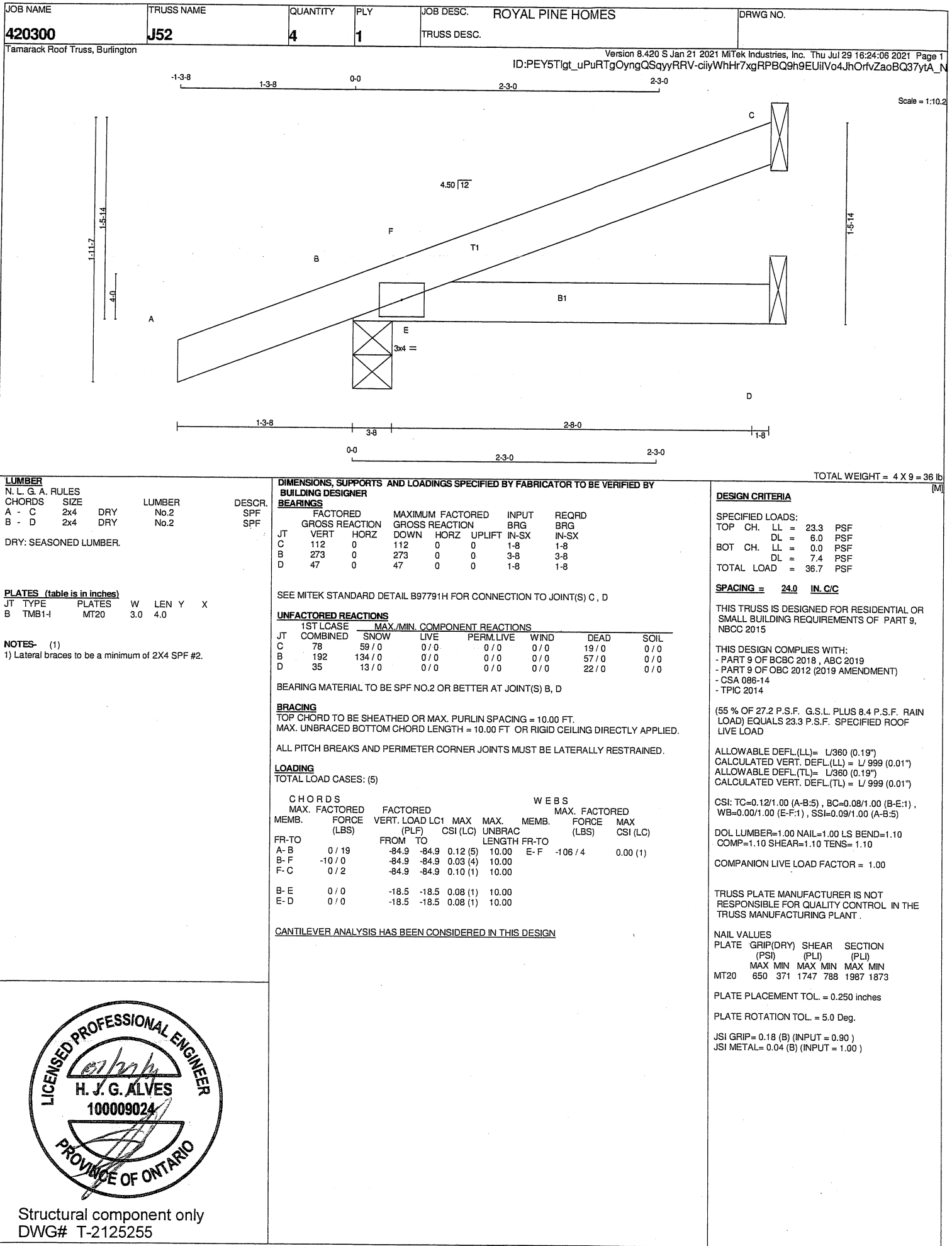
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.24 (B) (INPUT = 0.90)
JSI METAL = 0.06 (B) (INPUT = 1.00)

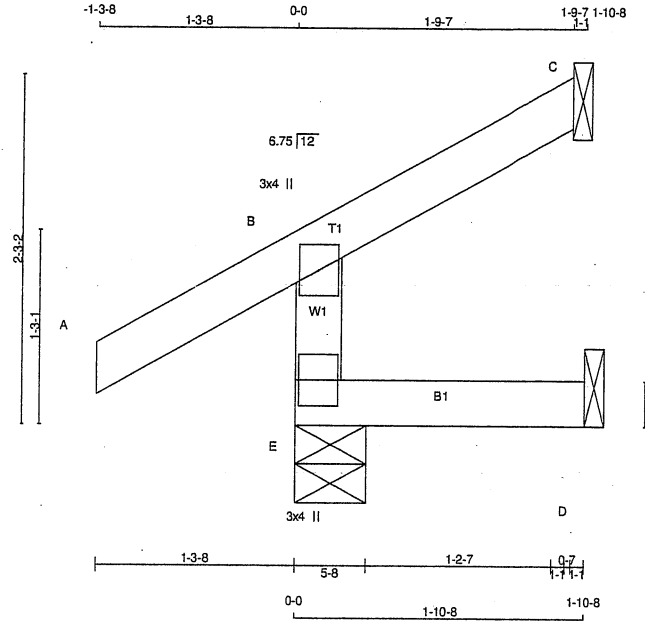


Structural component only
DWG# T-2125254



JOB NAME 420298	TRUSS NAME C30	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 29 15:59:25 2021 Page 1
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TOTAL WEIGHT = 2 X 7 = 15 lb [M]

LUMBER				DESCR.	SPF
N. L. G. A. RULES	CHORDS	SIZE	LUMBER		
E - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	253	0	253	0	5-8	5-8
C	41	0	41	0	1-8	1-8
D	9	0	17	0	1-8	1-8

SEE MITK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST CASE MAX./MIN. COMPONENT REACTIONS		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
	COMBINED	VERT.						
E	176	129 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0	
C	28	22 / -17	0 / 0	0 / 0	0 / 0	7 / 0	0 / 0	
D	8	0 / -7	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS		FACTORED		WEBS		FACTORED	
	MAX.	FORCE	VERT. LOAD	LC1 MAX	MAX.	MEMB.	FORCE	MAX
		(LBS)	(PLF)	CSI (LC)	UNBRAC LENGTH		(LBS)	CSI (LC)
FR-TO			FROM TO			FR-TO		
E-B	-226 / 0		0.0	0.0	0.03 (5)	7.81		
A-B	0 / 28		-84.9	-84.9	0.11 (5)	10.00		
B-C	-17 / 0		-84.9	-84.9	0.09 (1)	6.25		
E-D	0 / 0		-18.5	-18.5	0.03 (5)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.11/1.00 (A-B:5), BC=0.03/1.00 (D-E:5),
WB=0.00/1.00 (n/a:0), SSI=0.08/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

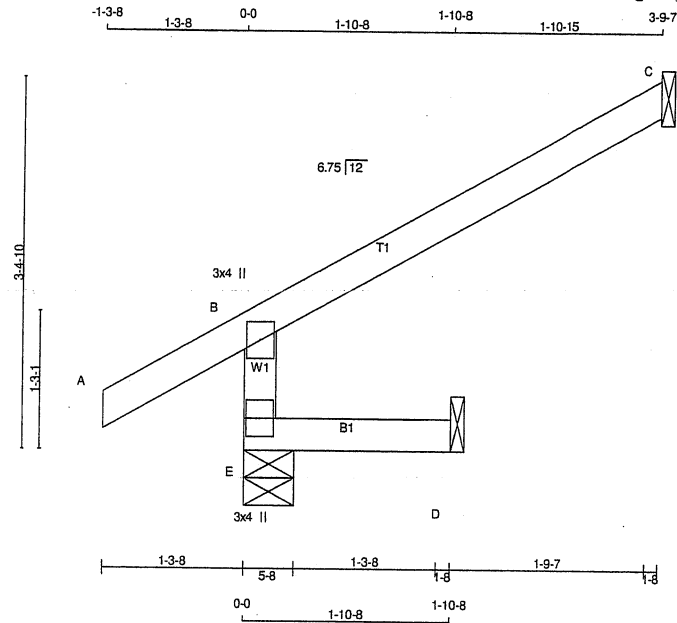
JSI GRIP= 0.09 (E) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)



Structural component only
DWG# T-2125228

JOB NAME 420298	TRUSS NAME C31	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 29 15:59:26 2021 Page 1
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Scale = 1/20.0

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS	2x4	DRY	No.2
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	3.0	4.0
E	BMV1+p	MT20	3.0	4.0

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	335	0	335	0	5-8	5-8
C	121	0	121	0	1-8	1-8
D	16	0	17	0	1-8	1-8

SEE MITTEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	WIND			
E	234	174 / 0	0 / 0	0 / 0	60 / 0	0 / 0	0 / 0
C	83	66 / 0	0 / 0	0 / 0	17 / 0	0 / 0	0 / 0
D	12	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS		FACTORED		WEBS	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	MEMB.	FORCE (LBS)
FR-TO		FROM	TO	FR-TO	
E-B	-316 / 0	0.0	0.0 0.01 (4)	7.81	
A-B	0 / 28	-84.9	-84.9 0.12 (5)	10.00	
B-C	-20 / 0	-84.9	-84.9 0.21 (1)	6.25	
E-D	0 / 0	-18.5	-18.5 0.02 (4)	10.00	

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 2 X 10 = 20 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(5% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.21/1.00 (B-C:1), BC=0.02/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

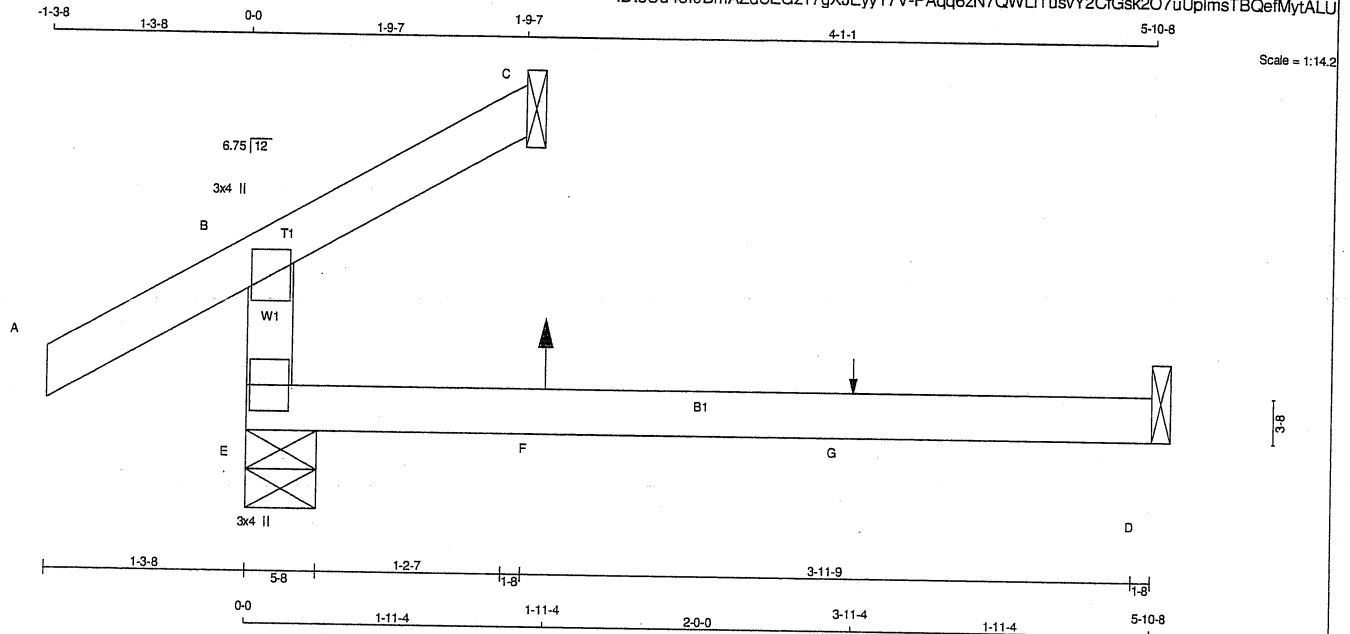
JSI GRIP= 0.13 (E) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)



Structural component only
DWG# T-2125229

JOB NAME 420298	TRUSS NAME C32	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 29 15:59:27 2021 Page 1
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LUMBER				DESCR.
N. L. G. A. RULES	SIZE	LUMBER	SPF	
CHORDS				
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	3.0	4.0
E	BMV1+p	MT20	3.0	4.0

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	267	0	267	0	5-8	5-8
C	59	0	59	0	1-8	1-8
D	44	0	52	0	1-8	1-8

SEE MITTEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	WIND			
E	188	126 / 0	0 / 0	0 / 0	63 / 0	0 / 0	0 / 0
C	43	19 / 0	0 / 0	0 / 0	25 / 0	0 / 0	0 / 0
D	36	0 / -2	0 / 0	0 / 0	37 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH
FR-TO					FR-TO		
E-B	-210 / 0	0.0	0.0	0.11 (4)	7.81		
A-B	0 / 28	-84.9	-84.9	0.12 (1)	10.00		
B-C	-9 / 10	-84.9	-84.9	0.08 (4)	10.00		
E-F	0 / 0	-18.5	-18.5	0.14 (4)	10.00		
F-G	0 / 0	-18.5	-18.5	0.14 (4)	10.00		
G-D	0 / 0	-18.5	-18.5	0.14 (4)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	1-11-4	4	1	7	FRONT	VERT	TOTAL	---	C1
G	3-11-4	1	1	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 12 = 24 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	36.7	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.12/1.00 (A-B:1), BC=0.14/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.08/1.00 (D-E:4)

DOL LUMBER=0.97 NAIL=0.97 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.09 (E) (INPUT = 0.90)

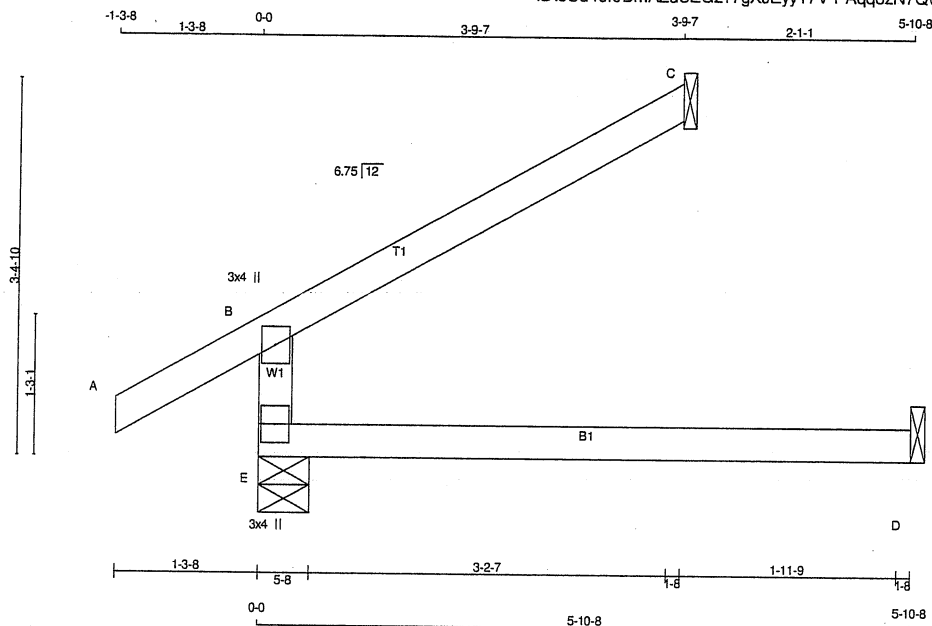
JSI METAL = 0.07 (B) (INPUT = 1.00)



Structural component only
DWG# T-2125230

JOB NAME 420298	TRUSS NAME C33	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 29 15:59:27 2021 Page 1
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TOTAL WEIGHT = 2 X 14 = 29 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	VERT	HORZ	DOWN	HORZ	BRG	IN-SX	BRG	IN-SX
E	380	0	380	0	0	5-8	5-8	5-8	5-8
C	121	0	121	0	0	1-8	1-8	1-8	1-8
D	45	0	51	0	0	1-8	1-8	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	269	174 / 0	0 / 0	0 / 0	0 / 0	96 / 0	0 / 0
C	83	66 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	MEMB.	FORCE (LBS)	MAX	LC1 MAX
FR-TO		FROM	TO	FR-TO			
E-B	-316 / 0	0.0	0.0	0.13 (4)	7.81		
A-B	0 / 28	-84.9	-84.9	0.11 (1)	10.00		
B-C	-20 / 0	-84.9	-84.9	0.21 (1)	6.25		
E-D	0 / 0	-18.5	-18.5	0.13 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.03")

CSI: TC=0.21/1.00 (B-C:1), BC=0.13/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

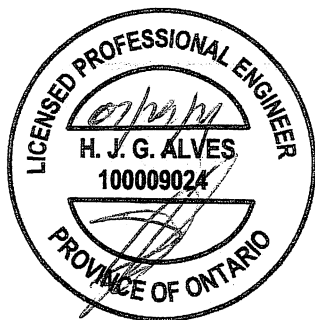
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

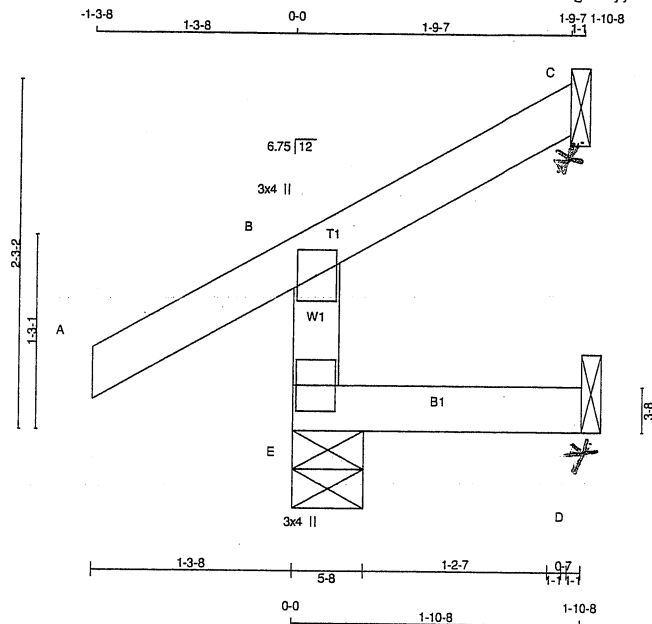
JSI GRIP= 0.13 (E) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)



Structural component only
DWG# T-2125231

JOB NAME 420298	TRUSS NAME C34	QUANTITY 4	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Thu Jul 29 15:59:28 2021 Page 1
ID:oUu4oI0DmAZdCEGz17gXJEyyT7V-tMODJJOIBqT952R66lup3HDBXGLYC?0ir9CBoytALT



TOTAL WEIGHT = 4 X 7 = 29 lb [M]

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			SPF
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0
E	BMV1+p	MT20	3.0	4.0

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
GROSS REACTION		GROSS REACTION		BRG		BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	253	0	253	0	0	5-8	5-8
C	41	0	41	0	-21	1-8	1-8
D	9	0	17	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	176	129 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	28	22 / -17	0 / 0	0 / 0	0 / 0	7 / 0	0 / 0
D	8	0 / -7	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS		MAX. FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX. UNBRACED LENGTH (LC)	MEMB.	FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
E-B	-226 / 0	0.0	0.0 0.03 (5)	7.81			
A-B	0 / 28	-84.9	-84.9 0.11 (5)	10.00			
B-C	-17 / 0	-84.9	-84.9 0.09 (1)	6.25			
E-D	0 / 0	-18.5	-18.5 0.03 (5)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.11/1.00 (A-B-5), BC=0.03/1.00 (D-E-5), WB=0.00/1.00 (n/a:0), SSI=0.08/1.00 (A-B-5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX	MIN	MAX
MT20	650 371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

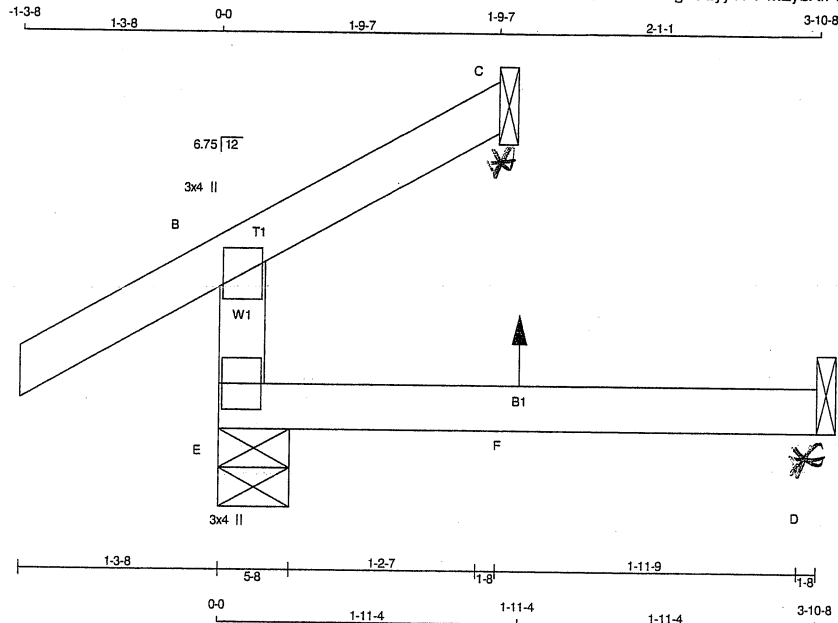
JSI GRIP= 0.09 (E) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)



Structural component only
DWG# T-2125232

JOB NAME 420298	TRUSS NAME C35	QUANTITY 4	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.420 S Jan 21 2021 MTek Industries, Inc. Thu Jul 29 15:59:29 2021 Page 1
ID:0Uu4oIDmAZdCEGz17gXJEyyT7V-MZybXfPNy7b0iC0IfTE7LHpNexC7Hf9xVWijEytALS



Scale = 1:14.2

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	
E - B	2x4 DRY	No.2	
A - C	2x4 DRY	No.2	
E - D	2x4 DRY	No.2	

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	261	0	261	0	5-8	5-8
C	45	0	45	0	1-8	1-8
D	27	0	35	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	WIND			
E	184	126 / 0	0 / 0	0 / 0	58 / 0	0 / 0	0 / 0
C	32	20 / -18	0 / 0	0 / 0	12 / 0	0 / 0	0 / 0
D	22	0 / 4	0 / 0	0 / 0	25 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (9)

MEMB.	CHORDS		FACTORED		WEBS	
	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)
FR-TO						
E-B	-223 / 0	0.0	0.0	0.05 (5)	7.81	
A-B	0 / 28	-84.9	-84.9	0.13 (5)	10.00	
B-C	-15 / 1	-84.9	-84.9	0.08 (5)	6.25	
E-F	0 / 0	-18.5	-18.5	0.06 (4)	10.00	
F-D	0 / 0	-18.5	-18.5	0.06 (4)	10.00	

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	1-11-4	4	1	7	BACK	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	36.7	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.13/1.00 (A-B-5), BC=0.06/1.00 (D-E-4), WB=0.00/1.00 (n/a:0), SSI=0.09/1.00 (A-B-5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

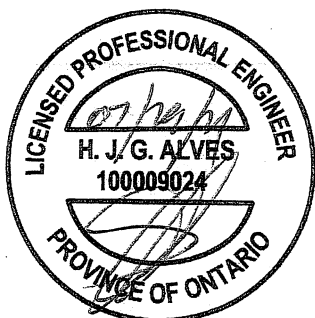
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.09 (E) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)



Structural component only
DWG# T-2125233



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering and General Safety notes.

T-1900218

Feb 09, 2018

TOE-NAIL CAPACITY DETAILS

LATERAL AND WITHDRAWAL RESISTANCE OF BEARING ANCHORAGE BY TOE-NAILS

			LATERAL RESISTANCE		WITHDRAWAL	
			SPF	D. FIR	SPF	D. FIR
COMMON WIRE	3.00	0.144	122	139	30	42
	3.25	0.144	127	144	32	45
	3.50	0.160	152	173	38	52
COMMON SPIRAL	3.00	0.122	96	108	26	36
	3.25	0.122	97	108	28	40
	3.50	0.152	142	161	36	50
3.25" Gun nail	3.25	0.120	94	105	28	39

Note: If using truss with D. Fir lumber and SPF bearing plate, use tabulated SPF values in table.

Nail type:	Common wire	Common spiral	Common wire	Common spiral	Gun Nail
Diameter (in.)	0.160	0.152	0.144	0.122	0.120
Length (in.)	3.50	3.50	3.00	3.00	3.25
LUMBER					
2x4 SPF	2	2	3	3	3
2x6 SPF	4	4	4	5	5
2x4 D. FIR	2	2	2	2	2
2x6 D. FIR	3	3	3	4	4

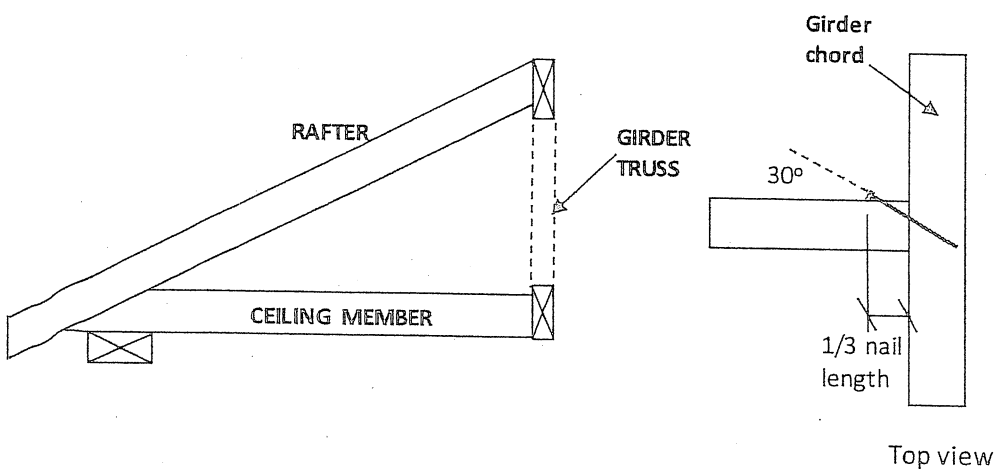
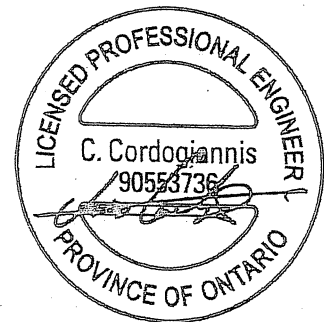


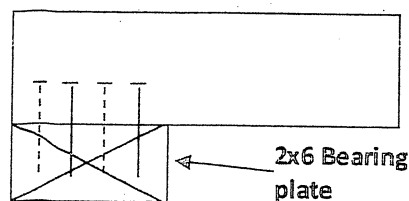
Figure 1: Toe-Nailing Rafter / Ceiling Member to Girder Truss

PEO
Certificate No. 10889485

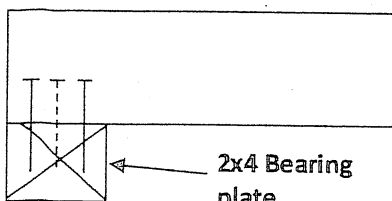
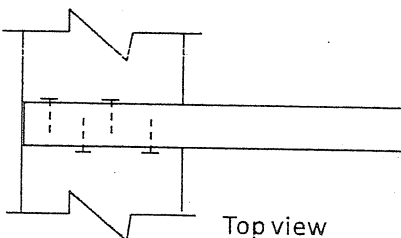


TOE-NAIL CAPACITY DETAILS

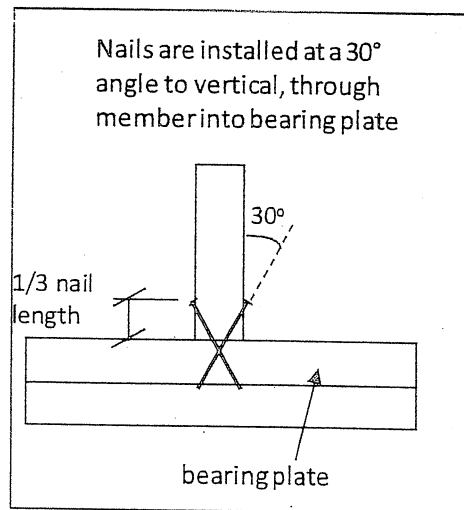
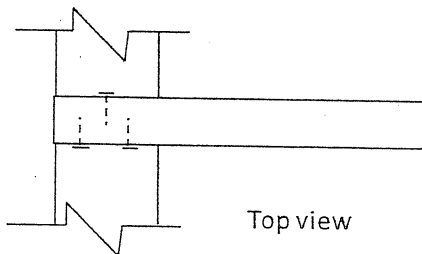
Figure 2: Toe-Nail Anchorage to Bearing Plate for Uplift



Elevation View



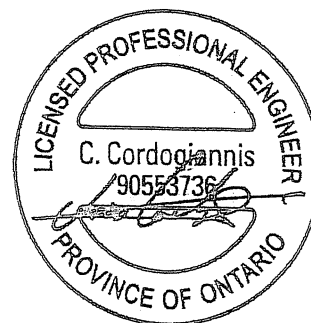
Elevation View



NOTES:

1. Rafter and ceiling members may be connected to top and bottom chords of girder truss by toe-nailing the members into the girder chords (see fig. 1), provided the factored vertical reactions of the supported members do not exceed the lateral resistance of the toe-nails. Mechanical connectors (hangers) are required if factored vertical reactions exceed the toe-nail capacity, or if the connection must resist horizontal loads (loads perpendicular to the face of girder or rafter).
2. Trusses, rafters or ceiling members may be anchored to the bearing plate with toe-nails (see fig. 2), provided that the factored uplift reactions due to **wind or earthquake loads** do not exceed the **withdrawal resistance of the toe-nails**. Mechanical anchors (tie-downs) are required for reactions that exceed the toe-nail withdrawal capacity. Toe-nail anchorage to bearing plates is **NOT** permitted if uplift reactions are generated from gravity loads (snow, floor live, dead).
3. Tabulated toe-nail resistances on page 1 are for **one** toe-nail. Multiply unit values by the number of nails used in the connection. Maximum number of nails in a connection shall not exceed the tabulated limits shown on page 1 for a given lumber size /species.
4. Nail values are based on specific gravity of $G = 0.42$ (SPF) and $G = 0.49$ (D. Fir).
5. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member.
6. For wind / earthquake loads, tabulated lateral resistances may be multiplied by 1.15 (K_D factor). No increases are permitted for tabulated withdrawal resistances.
7. Lumber must be dry ($< 19\%$ moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-19, Clause 12.9.

PEO
Certificate No. 10889485



LUL/LUS/LJS/HUS/HHUS/HGUS

Standard and Double-Shear Joist Hangers



This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 217–218.

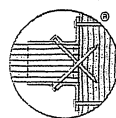
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 18–20.

Installation:

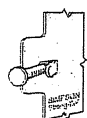
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1 1/2" nails into the header and 10d commons into the joist, and reduce the resistance to 0.84 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

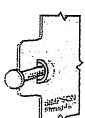
- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on pp. 105–107.



Double-Shear Nailing Top View

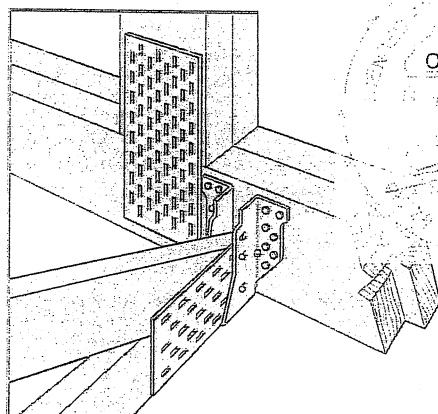


Double-Shear Nailing Side View; Do not bend tab

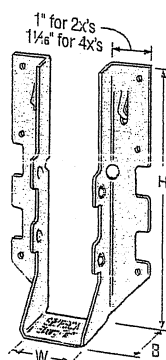


Dome Double-Shear Nailing Side View (available on some models)

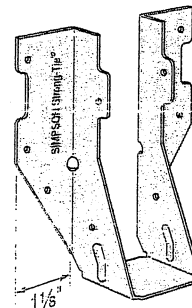
Typical HUS26 Installation with Reduced Heel Height
(Truss Designer to provide fastener quantity for connecting multiple members together)



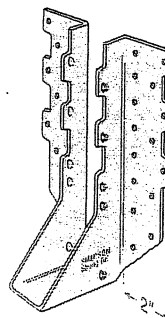
Oct. 14, 2020



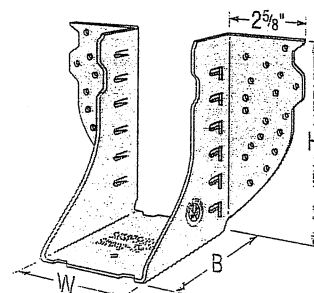
LUS28



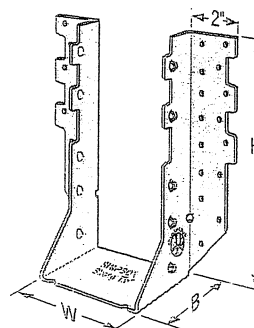
LU26L



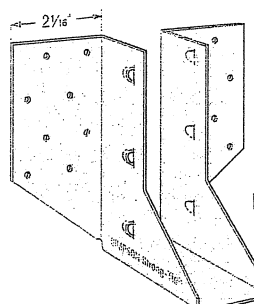
HUS210
(HUS26, HUS28, and HHUS similar)



HGUS28-2



HHUS210-2



LJS26DS

LUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

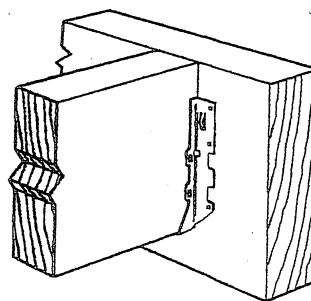
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

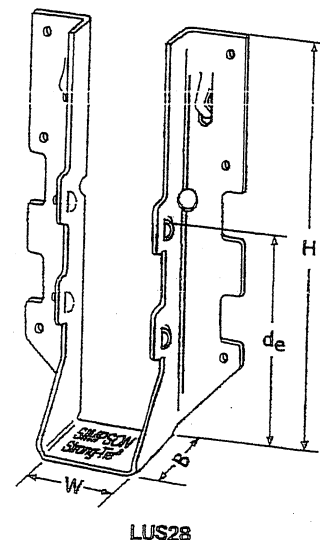
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified



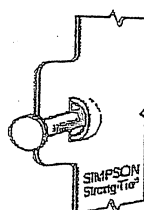
Typical LUS Installation



LUS28

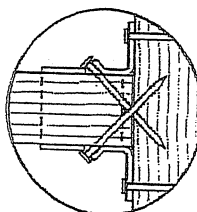
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
								(K _u =1.15)	(K _n =1.00)	(K _u =1.15)	(K _n =1.00)
LUS24	18	1⅞	3⅞	1¼	1⅞	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3⅞	3⅞	2	1⅞	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1⅞	4⅞	1¼	3⅞	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3⅞	4⅞	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4⅞	4⅞	2	3¼	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1⅞	6⅞	1¼	3⅞	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3⅞	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4⅞	6¼	2	3¼	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1⅞	7⅞	1¼	3⅞	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3⅞	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4⅞	8⅞	2	5¼	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.

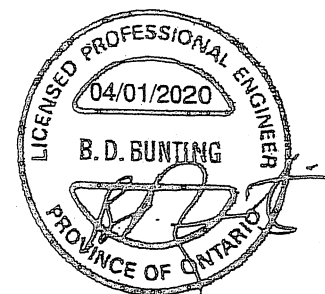


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECLUS20 3/20 exp. 6/22

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TECHNICAL BULLETIN

HUS/LJS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

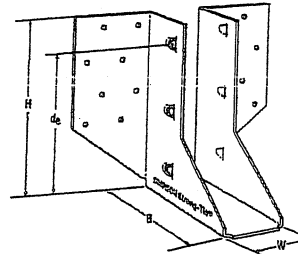
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

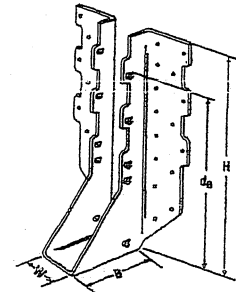
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

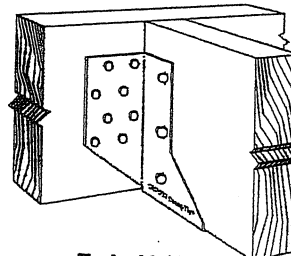
- See current catalogue for options



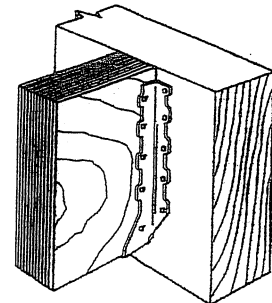
LJS26DS



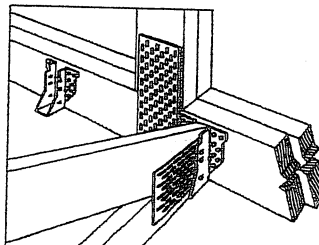
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



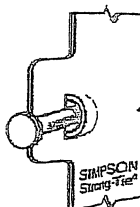
Typical HUS
Installation



Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

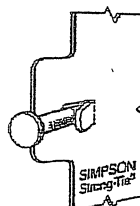
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d ₁	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K ₁ =1.15)	Normal (K ₂ =1.00)	Uplift (K ₁ =1.15)	Normal (K ₂ =1.00)
LJS26DS	18	1 1/16	5	3 1/2	4 5/8	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1 1/8	5 1/4	3	3 15/16	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1 5/8	7 3/4	3	6 9/32	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1 5/8	9 3/4	3	7 31/32	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1 13/16	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

1. d₁ is the distance from the seat of the hanger to the highest joist nail.

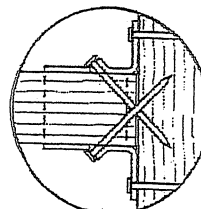


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

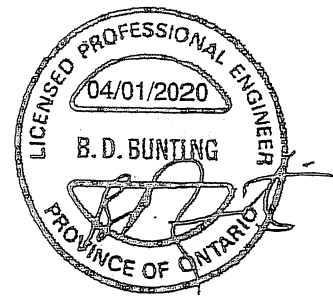
U.S. Patent
5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



**LIMIT
STATES
DESIGN**

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECHUS20 3/20 exp. 6/22

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TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

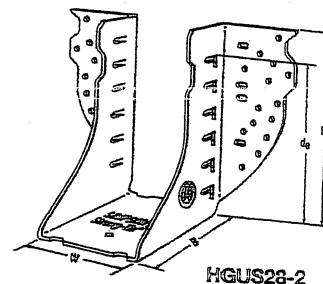
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%.
No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

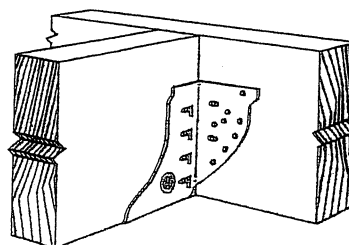
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

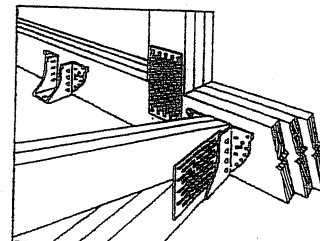
- See current catalogue for options



HGUS28-2



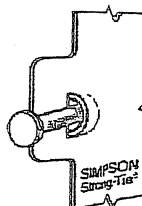
Typical HGUS Installation



Typical HGUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

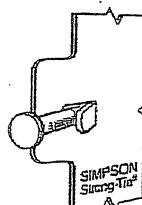
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d ₁ ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _p =1.15)	Normal (K _p =1.00)	Uplift (K _p =1.15)	Normal (K _p =1.00)
HGUS26	12	1⅞	5⅞	5	4⅞	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3⅞	5⅞	4	4⅞	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4⅞	5⅞	4	4⅞	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6⅞	5⅞	4	4⅞	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1⅞	7⅞	5	6⅞	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3⅞	7⅞	4	6⅞	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4⅞	7⅞	4	6⅞	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6⅞	7⅞	4	6⅞	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210	12	1⅞	9⅞	5	7⅞	(46) 16d	(16) 16d	3535	11070	2510	8090
HGUS210-2	12	3⅞	9⅞	4	8⅞	(46) 16d	(16) 16d	6840	14015	4855	10270
HGUS210-3	12	4⅞	9⅞	4	8⅞	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6⅞	9⅞	4	8⅞	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6⅞	10⅞	4	10⅞	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6⅞	12⅞	4	11⅞	(66) 16d	(22) 16d	10130	16400	7195	11645

1. d₁ is the distance from the seat of the hanger to the highest joist nail.

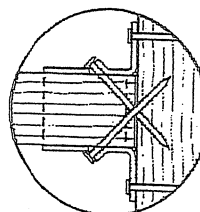


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

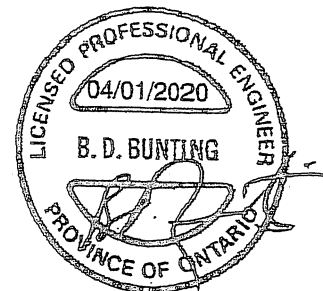
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



UNIT STATES DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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ONTARIO WOOD TRUSS
FABRICATORS ASSOCIATION

TECH-NOTES

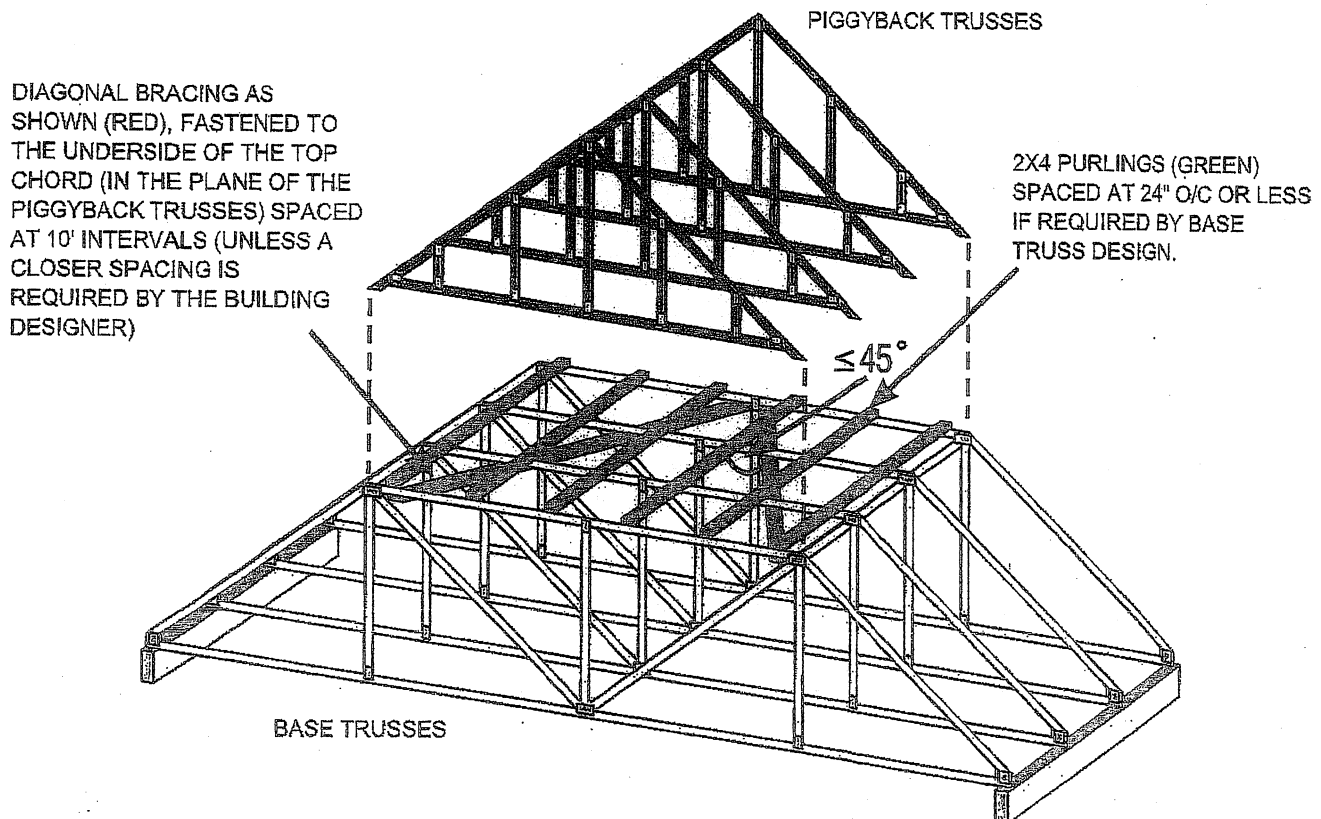
TN 15-001 Piggyback Bracing

Overview:

Where piggybacks are connected ovetop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

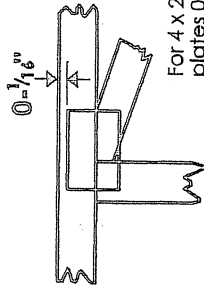
Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

Symbols

PLATE LOCATION AND ORIENTATION

Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/8" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

* Plate location details available in Mitek software or upon request.

PLATE SIZE

4 X 4

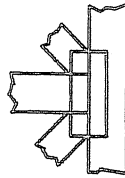
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)

