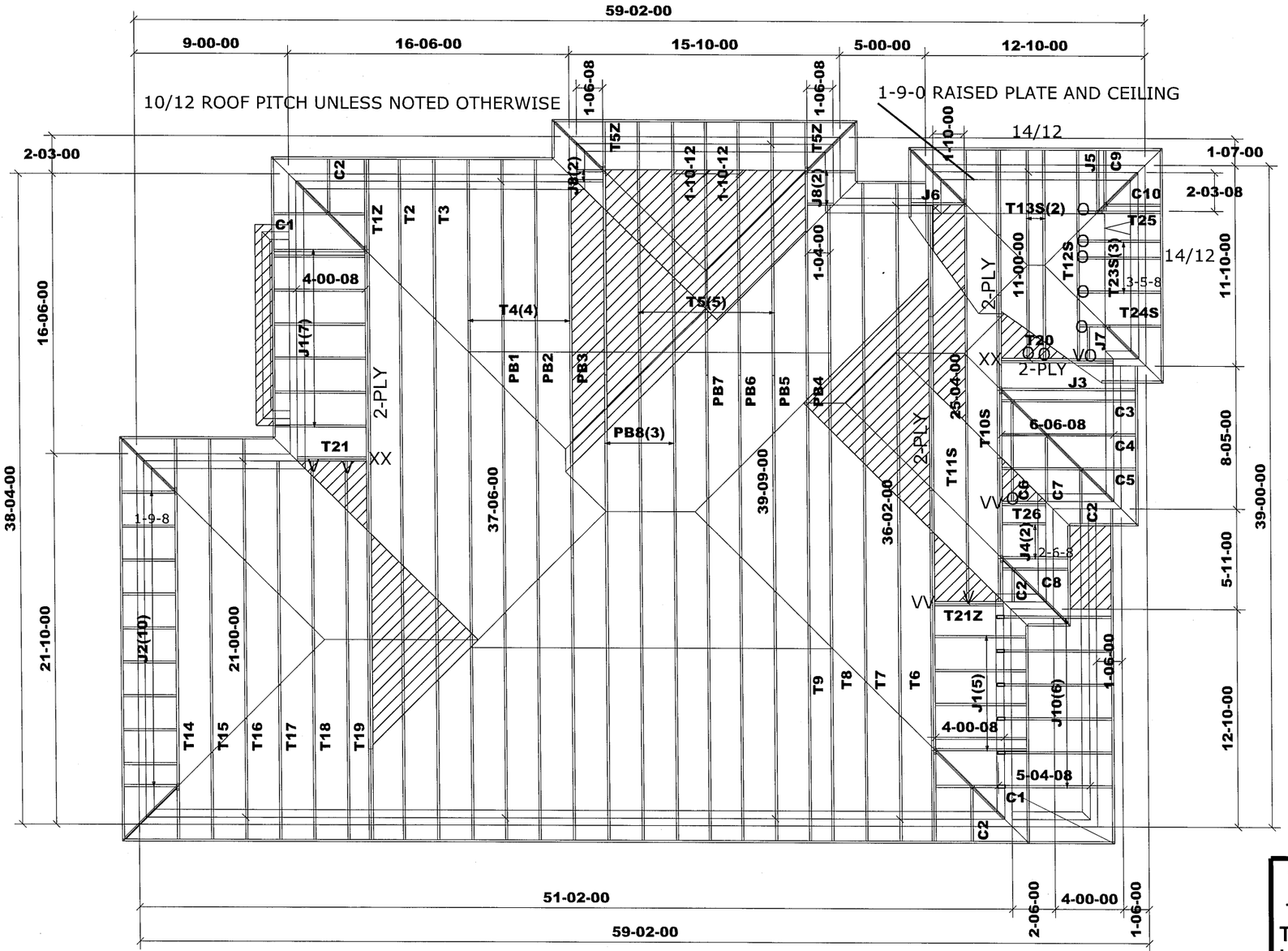


ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment). Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH OBC 2012 (2019 amendment) OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 27.2 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 23.3 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

DWG# T-2205687 to T-2205713, T-2205718 to T-2205730, T-2205736 to T-2205749, T1800218

HARDWARE:
LUS24 - (**O**)
LJS26DS - (**V**)
HGUS26-2 - (**XX**)
LUS26-2 -(VV)

DENOTES:
CONVENTIONAL FRAMING

LICENSED PROFESSIONAL ENGINEER
03-03-22
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2206146

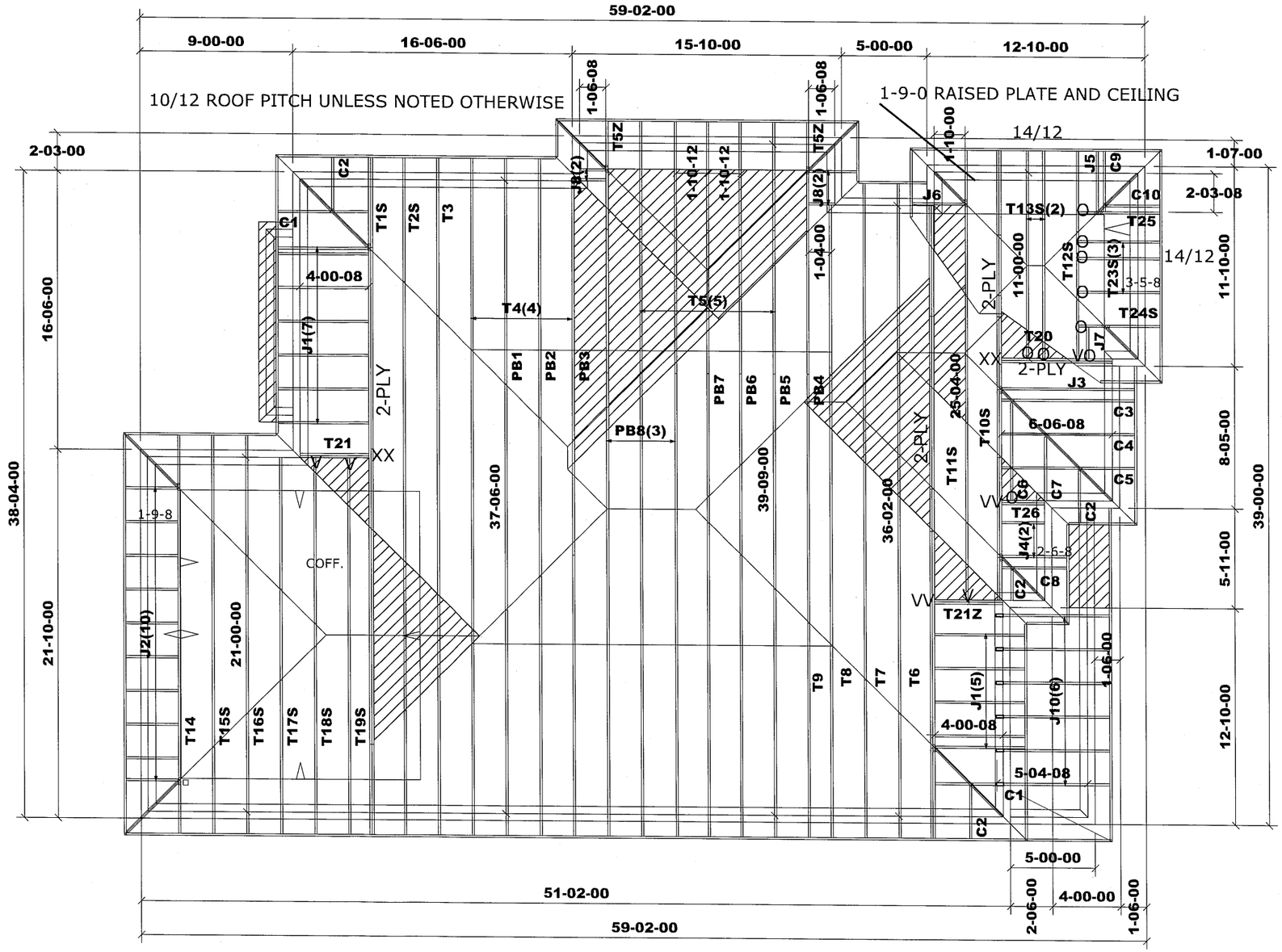
TRUSS PLACEMENT PLAN.

This is a truss placement plan only, NOT a final roof framing plan. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual truss drawings for each component identified on this placement plan.

Please verify that all dimensions match the dimensions found on the job. The Building designer is responsible for the temporary/ permanent bracing of the roof and floor system and its integration into the bracing of the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsability of the building designer unless otherwise noted in this plan. Building designer to review and aprove this plan to acertain comformity to his overall structural plan.

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

Job Track: 52545	Builder / Location: ROYAL PINE HOMES / BRAMPTON	Model / Elevation: MODEL 40-7 (CNR) / A	Mitek ver 8.4.2.286
Layout ID: 422443	Project: VALES OF HUMBER		
Plan Log: 205334	Date: 2022-03-03	Designer: YPG	



ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment). Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH OBC 2012 (2019 amendment) OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 27.2 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 23.3 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (**O**)
LJS26DS - (**V**)
HGUS26-2 - (**XX**)
LUS26-2 -(VV)

DENOTES:
CONVENTIONAL FRAMING

DWG# T-2205687 to T-2205713,
T-2205716 to T-2205742,
T1800218

LICENSED PROFESSIONAL ENGINEER
03-03-22
H. J. G. ALVES
100090242
PROVINCE OF ONTARIO

Structural component only
DWG# T-2206147

TRUSS PLACEMENT PLAN.

This is a truss placement plan only, NOT a final roof framing plan. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual truss drawings for each component identified on this placement plan.

Please verify that all dimensions match the dimensions found on the job. The Building designer is responsible for the temporary/ permanent bracing of the roof and floor system and its integration into the bracing of the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsability of the building designer unless otherwise noted in this plan. Building designer to review and aprove this plan to acertain comformity to his overall structural plan.

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

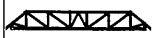
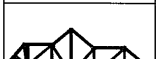
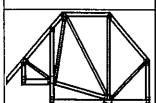
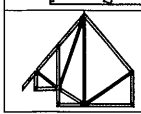
Job Track: 52545	Builder / Location: ROYAL PINE HOMES / BRAMPTON	Model / Elevation: MODEL 40-7 (CNR) / A-OPT.WITH COFF	Mitek ver 8.4.2.286
Layout ID: 422291	Project: VALES OF HUMBER		
Plan Log: 205334	Date: 2022-03-03	Designer: YPG	

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52545
Builder:	ROYAL PINE HOMES	PlanLog:	205334
Project:	VALES OF HUMBER	Layout ID:	422443
Location:	BRAMPTON	Ref #	
Model:	MODEL 40-7 (CNR)	Page:	1 of 5
Lot #:		Date:	02-28-2022
Elevation:	A	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

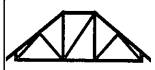
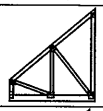
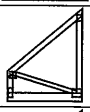
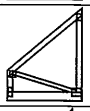
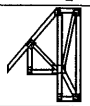
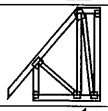
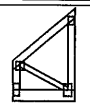
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T1Z Hip Girder	10 /12	**37-06-00**	5-00-02	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	359.84 225.33		
	1	T2 Hip	10 /12	**37-06-00**	6-08-02	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	187.58 113.67		
	1	T3 Hip	10 /12	**37-06-00**	8-04-02	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	204.75 125.67		
	4	T4 Hip	10 /12	**37-06-00**	10-00-02	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	873.93 534.67		
	5	T5 Piggyback Base	10 /12	**39-09-00**	10-00-02	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	1169.5 724.17		
	2	T5Z Piggyback Base Girder	10 /12	**39-09-00**	10-00-02	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	467.8 289.67		
	1 2-ply	T6 Roof Special Girder	10 /12	**36-02-00**	7-01-02	2 x 4 2 x 6	1-03-08	1-07-11 1-07-11	370 232.00		
	1	T7 Hip	10 /12	**36-02-00**	6-08-02	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	177.55 108.67		
	1	T8 Hip	10 /12	**36-02-00**	8-04-02	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	190.13 118.00		
	1	T9 Hip	10 /12	**36-02-00**	10-00-02	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	213.82 133.00		
	1 2-ply	T10S Roof Special Girder	14 /12	**25-04-00**	8-02-04	2 x 4 2 x 6	1-03-08	2-01-05 1-07-11	328.4 210.00		
	1	T11S Roof Special Girder	14 /12	**25-04-00**	8-09-02	2 x 4 2 x 6	1-03-08	2-01-05 1-07-11	147.37 94.17		
	1	T12S Hip Girder	14 /12	**11-00-00**	7-10-12	2 x 4 2 x 6	1-03-08	2-01-05 3-10-05	80.19 52.67		
	2	T13S Roof Special	14 /12	**11-00-00**	10-03-05	2 x 4	1-03-08	2-01-05 3-10-05	146.47 94.33		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52545
Builder:	ROYAL PINE HOMES	PlanLog:	205334
Project:	VALES OF HUMBER	Layout ID:	422443
Location:	BRAMPTON	Ref #	
Model:	MODEL 40-7 (CNR)	Page:	2 of 5
Lot #:		Date:	02-28-2022
Elevation:	A	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

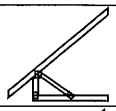
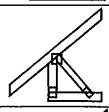
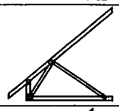
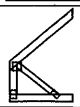
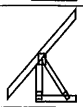
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	T14 Hip Girder	10 /12	21-00-00	3-01-10	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	85.52 55.50		
	1	T15 Hip	10 /12	21-00-00	4-09-10	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	86.47 55.50		
	1	T16 Hip	10 /12	21-00-00	6-05-10	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	94.55 60.33		
	1	T17 Hip	10 /12	21-00-00	8-01-10	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	98.51 63.67		
	1	T18 Hip	10 /12	21-00-00	9-09-10	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	112.04 72.67		
	1	T19 Common	10 /12	21-00-00	10-04-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	111.8 70.00		
	1 2-ply	T20 Jack-Closed Girder	10 /12	6-06-08	7-01-02	2 x 4 2 x 6		1-07-11 7-01-02	78.47 51.00		
	1 2-ply	T21 Jack-Closed Girder	10 /12	4-00-08	5-00-02	2 x 4 2 x 6		1-07-11 5-00-02	44.82 31.33		
	1 2-ply	T21Z Jack-Closed Girder	10 /12	4-00-08	5-00-02	2 x 4 2 x 6		1-07-11 5-00-02	44.82 31.33		
	3	T23S Jack-Closed	14 /12	3-05-08	7-10-12	2 x 4	1-03-08	2-01-05 7-10-12	95.88 59.00		
	1	T24S Half Hip Girder	14 /12	3-05-08	6-00-09	2 x 4	1-03-08	2-01-05 6-00-09	33.65 22.83		
	1	T25 Half Hip Girder	14 /12	3-05-08	4-09-06	2 x 4	1-03-08	2-01-05 4-09-06	27.7 19.17		
	1 2-ply	T26 Jack-Closed Girder	10 /12	2-06-08	3-09-02	2 x 4 2 x 6		1-07-11 3-09-02	29.87 21.00		
	1	PB1 Piggyback	10 /12	17-05-00	1-08-00	2 x 4			52.49 33.17		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52545
Builder:	ROYAL PINE HOMES	PlanLog:	205334
Project:	VALES OF HUMBER	Layout ID:	422443
Location:	BRAMPTON	Ref #	
Model:	MODEL 40-7 (CNR)	Page:	3 of 5
Lot #:		Date:	02-28-2022
Elevation:	A	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

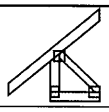
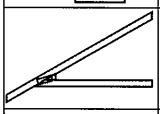
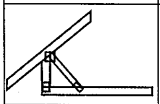
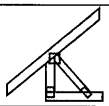
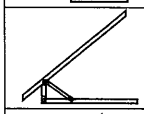
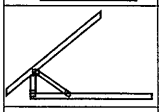
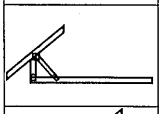
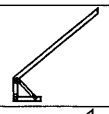
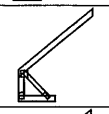
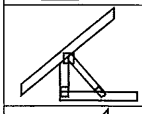
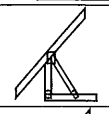
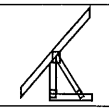
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	PB2 Piggyback	10 /12	17-05-00	3-04-00	2 x 4			55.75 36.50		
	1	PB3 Piggyback	10 /12	17-05-00	4-04-15	2 x 4			55.89 34.33		
	1	PB4 Piggyback	10 /12	16-01-00	1-01-05	2 x 4			37.1 23.50		
	1	PB5 Piggyback	10 /12	16-01-00	2-09-05	2 x 4			50.46 32.83		
	1	PB6 Piggyback	10 /12	16-01-00	4-05-05	2 x 4			50.04 30.83		
	1	PB7 Piggyback	10 /12	16-01-00	6-00-05	2 x 4			53.27 33.67		
	3	PB8 Piggyback	10 /12	16-01-00	6-08-07	2 x 4			168.83 107.00		
	12	J1 Jack-Open	10 /12	4-00-08	5-00-02	2 x 4	1-03-08	1-07-11 5-00-02	192.13 138.00		
	10	J2 Jack-Open	10 /12	1-09-08	3-01-10	2 x 4	1-03-08	1-07-11 3-01-10	96.54 70.00		
	1	J3 Jack-Open	10 /12	6-06-08	7-01-02	2 x 4	1-03-08	1-07-11 7-01-02	27.2 17.83		
	2	J4 Jack-Open	10 /12	2-06-08	3-09-02	2 x 4		1-07-11 3-09-02	20.07 15.00		
	1	J5 Jack-Open	14 /12	2-03-08	4-09-06	2 x 4	1-03-08	2-01-05 4-09-06	13.12 8.83		
	1	J6 Jack-Open	14 /12	1-10-00	4-03-00	2 x 4	1-03-08	2-01-05 4-03-00	11.63 8.17		
	1	J7 Jack-Open	14 /12	1-10-08	6-00-09	2 x 4		3-10-05 6-00-09	12.8 8.67		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52545
Builder:	ROYAL PINE HOMES	PlanLog:	205334
Project:	VALES OF HUMBER	Layout ID:	422443
Location:	BRAMPTON	Ref #	
Model:	MODEL 40-7 (CNR)	Page:	4 of 5
Lot #:		Date:	02-28-2022
Elevation:	A	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	4	J8 Jack-Open	10 /12	**1-06-08**	2-11-02	2 x 4	1-03-08	1-07-11 2-11-02	36.17 25.33		
	6	J10 Jack-Open	7 /12	**5-04-08**	4-02-12	2 x 4	1-03-08	4-05 3-05-15	94.17 60.00		
	2	C1 Jack-Open	10 /12	**1-09-07**	3-01-09	2 x 4	1-03-08 2-03-01	1-07-11 3-01-09	24.42 18.00		
	4	C2 Jack-Open	10 /12	**1-09-07**	3-01-09	2 x 4	1-03-08 1-01	1-07-11 3-01-09	38.95 28.00		
	1	C3 Jack-Open	10 /12	**5-09-07**	6-05-09	2 x 4	1-03-08 9-01	1-07-11 6-05-09	21.45 14.17		
	1	C4 Jack-Open	10 /12	**3-09-07**	4-09-09	2 x 4	1-03-08 2-09-01	1-07-11 4-09-09	18.48 12.17		
	1	C5 Jack-Open	10 /12	**1-09-07**	3-01-09	2 x 4	1-03-08 4-09-01	1-07-11 3-01-09	15.06 10.33		
	1	C6 Jack-Open	10 /12	**1-10-08**	6-05-09	2 x 4	3-10-15	1-07-11 3-02-07	13.81 9.00		
	1	C7 Jack-Open	10 /12	**1-10-08**	4-09-09	2 x 4	1-10-15	1-07-11 3-02-07	10.84 7.67		
	1	C8 Jack-Open	10 /12	**1-09-07**	3-01-09	2 x 4	1-03-08 9-01	1-07-11 3-01-09	10.5 7.67		
	1	C9 Jack-Open	14 /12	**1-09-07**	4-02-05	2 x 4	1-03-08 6-01	2-01-05 4-02-05	12.05 8.83		
	1	C10 Jack-Open	14 /12	**1-09-07**	4-02-05	2 x 4	1-03-08 1-01	2-01-05 4-02-05	11.58 8.17		

TOTAL # TRUSS= **107** TOTAL BFT OF ALL TRUSSES= **4477.02** BFT. TOTAL WEIGHT OF ALL TRSSES **7066.24** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	HGUS26-2	
3	Hardware	LJS26DS	

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52545
Builder:	ROYAL PINE HOMES	PlanLog:	205334
Project:	VALES OF HUMBER	Layout ID:	422443
Location:	BRAMPTON	Ref #	
Model:	MODEL 40-7 (CNR)	Page:	5 of 5
Lot #:		Date:	02-28-2022
Elevation:	A	Designer:	
		Sales Rep:	Rick DiCiano

HARDWARE

QTY	TYPE	MODEL	LENGTH
9	Hardware	LUS24	
2	Hardware	LUS26-2	

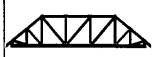
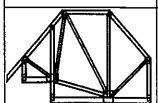
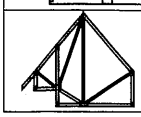
TOTAL NUMBER OF ITEMS= **16**

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52545
Builder:	ROYAL PINE HOMES	PlanLog:	205334
Project:	VALES OF HUMBER	Layout ID:	422291
Location:	BRAMPTON	Ref #	
Model:	MODEL 40-7 (CNR)	Page:	1 of 5
Lot #:		Date:	02-28-2022
Elevation:	A-OPT.WITH COFF	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

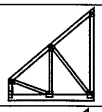
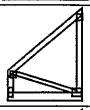
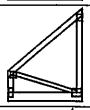
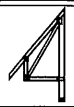
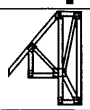
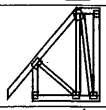
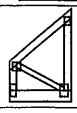
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T1S Hip Girder	10 /12	37-06-00	5-00-02	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	411.81 259.33		
	1	T2S Hip	10 /12	37-06-00	6-08-02	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	199.67 123.33		
	1	T3 Hip	10 /12	37-06-00	8-04-02	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	208.86 128.33		
	4	T4 Hip	10 /12	37-06-00	10-00-02	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	892.8 546.67		
	5	T5 Piggyback Base	10 /12	39-09-00	10-00-02	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	1195.76 740.83		
	2	T5Z Piggyback Base Girder	10 /12	39-09-00	10-00-02	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	478.3 296.33		
	1 2-ply	T6 Roof Special Girder	10 /12	36-02-00	7-01-02	2 x 4 2 x 6	1-03-08	1-07-11 1-07-11	384.44 242.67		
	1	T7 Hip	10 /12	36-02-00	6-08-02	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	179.19 111.00		
	1	T8 Hip	10 /12	36-02-00	8-04-02	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	203.96 125.67		
	1	T9 Hip	10 /12	36-02-00	10-00-02	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	215.47 134.33		
	1 2-ply	T10S Roof Special Girder	14 /12	25-04-00	8-02-04	2 x 4 2 x 6	1-03-08	2-01-05 1-07-11	317.23 201.00		
	1	T11S Roof Special Girder	14 /12	25-04-00	8-09-02	2 x 4 2 x 6	1-03-08	2-01-05 1-07-11	148.73 94.83		
	1	T12S Hip Girder	14 /12	11-00-00	7-10-12	2 x 4 2 x 6	1-03-08	2-01-05 3-10-05	80.19 52.67		
	2	T13S Roof Special	14 /12	11-00-00	10-03-05	2 x 4	1-03-08	2-01-05 3-10-05	146.47 94.33		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52545
Builder:	ROYAL PINE HOMES	PlanLog:	205334
Project:	VALES OF HUMBER	Layout ID:	422291
Location:	BRAMPTON	Ref #	
Model:	MODEL 40-7 (CNR)	Page:	2 of 5
Lot #:		Date:	02-28-2022
Elevation:	A-OPT.WITH COFF	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

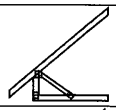
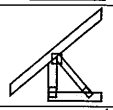
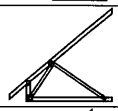
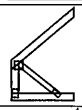
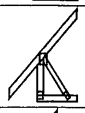
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	T14 Hip Girder	10 /12	21-00-00	3-01-10	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	85.52 55.50		
	1	T15S Hip	10 /12	21-00-00	4-09-10	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	93.27 61.67		
	1	T16S Hip	10 /12	21-00-00	6-05-10	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	100.56 65.50		
	1	T17S Hip	10 /12	21-00-00	8-01-10	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	100.67 66.50		
	1	T18S Hip	10 /12	21-00-00	9-09-10	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	114.15 74.83		
	1	T19S Roof Special	10 /12	21-00-00	10-04-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	108.86 70.33		
	1 2-ply	T20 Jack-Closed Girder	10 /12	6-06-08	7-01-02	2 x 4 2 x 6		1-07-11 7-01-02	78.47 51.00		
	1 2-ply	T21 Jack-Closed Girder	10 /12	4-00-08	5-00-02	2 x 4 2 x 6		1-07-11 5-00-02	44.82 31.33		
	1 2-ply	T21Z Jack-Closed Girder	10 /12	4-00-08	5-00-02	2 x 4 2 x 6		1-07-11 5-00-02	44.82 31.33		
	3	T23S Jack-Closed	14 /12	3-05-08	7-10-12	2 x 4	1-03-08	2-01-05 7-10-12	95.88 59.00		
	1	T24S Half Hip Girder	14 /12	3-05-08	6-00-09	2 x 4	1-03-08	2-01-05 6-00-09	33.65 22.83		
	1	T25 Half Hip Girder	14 /12	3-05-08	4-09-06	2 x 4	1-03-08	2-01-05 4-09-06	27.7 19.17		
	1 2-ply	T26 Jack-Closed Girder	10 /12	2-06-08	3-09-02	2 x 4 2 x 6		1-07-11 3-09-02	29.87 21.00		
	1	PB1 Piggyback	10 /12	17-05-00	1-08-00	2 x 4			52.49 33.17		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52545
Builder:	ROYAL PINE HOMES	PlanLog:	205334
Project:	VALES OF HUMBER	Layout ID:	422291
Location:	BRAMPTON	Ref #	
Model:	MODEL 40-7 (CNR)	Page:	3 of 5
Lot #:		Date:	02-28-2022
Elevation:	A-OPT.WITH COFF	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

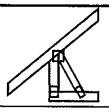
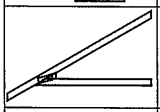
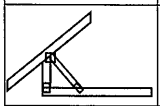
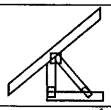
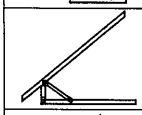
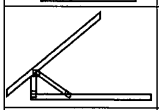
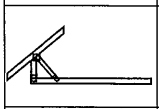
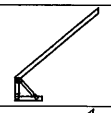
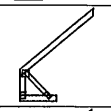
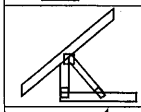
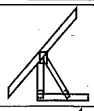
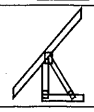
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	PB2 Piggyback	10 /12	**17-05-00**	3-04-00	2 x 4			55.75 36.50		
	1	PB3 Piggyback	10 /12	**17-05-00**	4-04-15	2 x 4			55.89 34.33		
	1	PB4 Piggyback	10 /12	**16-01-00**	1-01-05	2 x 4			37.76 24.50		
	1	PB5 Piggyback	10 /12	**16-01-00**	2-09-05	2 x 4			50.46 32.83		
	1	PB6 Piggyback	10 /12	**16-01-00**	4-05-05	2 x 4			50.04 30.83		
	1	PB7 Piggyback	10 /12	**16-01-00**	6-00-05	2 x 4			53.27 33.67		
	3	PB8 Piggyback	10 /12	**16-01-00**	6-08-07	2 x 4			168.83 107.00		
	12	J1 Jack-Open	10 /12	**4-00-08**	5-00-02	2 x 4	1-03-08	1-07-11 5-00-02	192.13 138.00		
	10	J2 Jack-Open	10 /12	**1-09-08**	3-01-10	2 x 4	1-03-08	1-07-11 3-01-10	96.54 70.00		
	1	J3 Jack-Open	10 /12	**6-06-08**	7-01-02	2 x 4	1-03-08	1-07-11 7-01-02	27.2 17.83		
	2	J4 Jack-Open	10 /12	**2-06-08**	3-09-02	2 x 4		1-07-11 3-09-02	20.07 15.00		
	1	J5 Jack-Open	14 /12	**2-03-08**	4-09-06	2 x 4	1-03-08	2-01-05 4-09-06	13.12 8.83		
	1	J6 Jack-Open	14 /12	**1-10-00**	4-03-00	2 x 4	1-03-08	2-01-05 4-03-00	11.63 8.17		
	1	J7 Jack-Open	14 /12	**1-10-08**	6-00-09	2 x 4		3-10-05 6-00-09	12.8 8.67		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52545
Builder:	ROYAL PINE HOMES	PlanLog:	205334
Project:	VALES OF HUMBER	Layout ID:	422291
Location:	BRAMPTON	Ref #	
Model:	MODEL 40-7 (CNR)	Page:	4 of 5
Lot #:		Date:	02-28-2022
Elevation:	A-OPT.WITH COFF	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	4	J8 Jack-Open	10 /12	**1-06-08**	2-11-02	2 x 4	1-03-08	1-07-11 2-11-02	35.26 25.33		
	6	J10 Jack-Open	7 /12	**5-04-08**	4-02-12	2 x 4	1-03-08	4-05 3-05-15	94.17 60.00		
	2	C1 Jack-Open	10 /12	**1-09-07**	3-01-09	2 x 4	1-03-08 2-03-01	1-07-11 3-01-09	24.42 18.00		
	4	C2 Jack-Open	10 /12	**1-09-07**	3-01-09	2 x 4	1-03-08 1-01	1-07-11 3-01-09	38.95 28.00		
	1	C3 Jack-Open	10 /12	**5-09-07**	6-05-09	2 x 4	1-03-08 9-01	1-07-11 6-05-09	21.45 14.17		
	1	C4 Jack-Open	10 /12	**3-09-07**	4-09-09	2 x 4	1-03-08 2-09-01	1-07-11 4-09-09	18.48 12.17		
	1	C5 Jack-Open	10 /12	**1-09-07**	3-01-09	2 x 4	1-03-08 4-09-01	1-07-11 3-01-09	15.06 10.33		
	1	C6 Jack-Open	10 /12	**1-10-08**	6-05-09	2 x 4	3-10-15	1-07-11 3-02-07	13.81 9.00		
	1	C7 Jack-Open	10 /12	**1-10-08**	4-09-09	2 x 4	1-10-15	1-07-11 3-02-07	10.84 7.67		
	1	C8 Jack-Open	10 /12	**1-09-07**	3-01-09	2 x 4	1-03-08 9-01	1-07-11 3-01-09	10.5 7.67		
	1	C9 Jack-Open	14 /12	**1-09-07**	4-02-05	2 x 4	1-03-08 6-01	2-01-05 4-02-05	12.05 8.83		
	1	C10 Jack-Open	14 /12	**1-09-07**	4-02-05	2 x 4	1-03-08 1-01	2-01-05 4-02-05	11.58 8.17		

TOTAL # TRUSS= **107** TOTAL BFT OF ALL TRUSSES= **4589.98** BFT. TOTAL WEIGHT OF ALL TRSSES **7225.67** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	HGUS26-2	
3	Hardware	LJS26DS	

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52545
Builder:	ROYAL PINE HOMES	PlanLog:	205334
Project:	VALES OF HUMBER	Layout ID:	422291
Location:	BRAMPTON	Ref #	
Model:	MODEL 40-7 (CNR)	Page:	5 of 5
Lot #:		Date:	02-28-2022
Elevation:	A-OPT.WITH COFF	Designer:	
		Sales Rep:	Rick DiCiano

HARDWARE

QTY	TYPE	MODEL	LENGTH
9	Hardware	LUS24	
2	Hardware	LUS26-2	

TOTAL NUMBER OF ITEMS= **16**

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
422443	T1Z	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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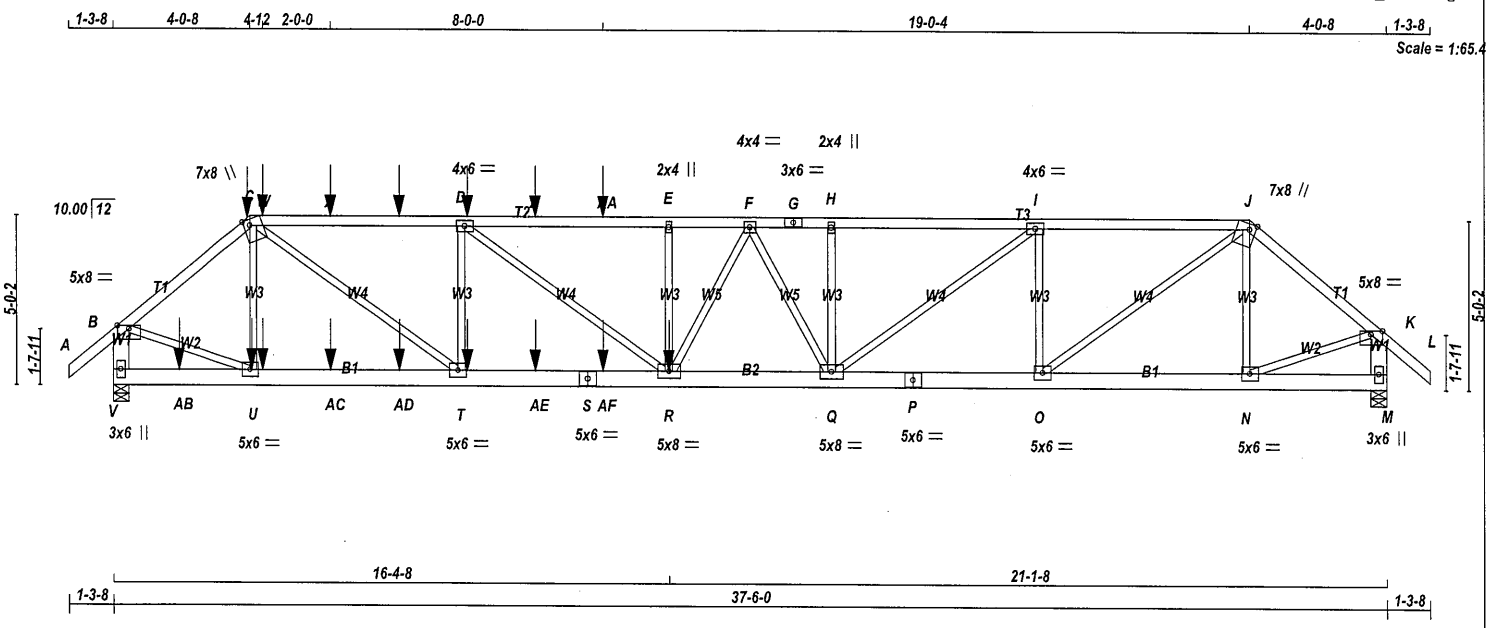

TOTAL WEIGHT = 2 X 180 = 360 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
V - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
V - S	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- G	1	12	SIDE(61.0)
G- J	1	12	TOP
J- L	1	12	TOP
V- B	2	12	TOP
M- K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
V- S	2	12	SIDE(183.1)
S- P	2	12	SIDE(183.1)
P- M	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
E- R	1	6	SIDE(150.1)

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

Structural component only
DWG# T-2205743

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	3495	0	3495	0	0	5-8	5-8
M	2801	0	2801	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	2475	1603 / 0	0 / 0	0 / 0	0 / 0	873 / 0	0 / 0
M	1983	1286 / 0	0 / 0	0 / 0	0 / 0	698 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 38	-84.9	-84.9	0.06 (1)		10.00
B- C	-3662 / 0	-84.9	-84.9	0.20 (1)		4.75
C- W	-5782 / 0	-84.9	-84.9	0.69 (1)		3.47
W- X	-5782 / 0	-84.9	-84.9	0.69 (1)		3.47
X- Y	-5782 / 0	-84.9	-84.9	0.69 (1)		3.47
Y- D	-5782 / 0	-84.9	-84.9	0.69 (1)		3.47
D- Z	-7276 / 0	-84.9	-84.9	0.83 (1)		3.00
Z-AA	-7276 / 0	-84.9	-84.9	0.83 (1)		3.00
AA- E	-7276 / 0	-84.9	-84.9	0.83 (1)		3.00
E- F	-7276 / 0	-84.9	-84.9	0.35 (1)		3.38
F- G	-6511 / 0	-84.9	-84.9	0.27 (1)		3.64
G- H	-6511 / 0	-84.9	-84.9	0.27 (1)		3.64
H- I	-6511 / 0	-84.9	-84.9	0.58 (1)		3.44
I- J	-4839 / 0	-84.9	-84.9	0.45 (1)		4.02
J- K	-2870 / 0	-84.9	-84.9	0.18 (1)		5.26
K- L	0 / 38	-84.9	-84.9	0.06 (1)		10.00
V- B	-3469 / 0	0.0	0.0	0.13 (1)		7.51
M- K	-2782 / 0	0.0	0.0	0.10 (1)		7.81
V-AB	0 / 0	-18.5	-18.5	0.05 (4)		10.00
AB- U	0 / 0	-18.5	-18.5	0.05 (4)		10.00
U-AC	0 / 2791	-18.5	-18.5	0.22 (1)		10.00
AC-AD	0 / 2791	-18.5	-18.5	0.22 (1)		10.00
AD- T	0 / 2791	-18.5	-18.5	0.22 (1)		10.00
T-AE	0 / 5782	-18.5	-18.5	0.44 (1)		10.00
AE- S	0 / 5782	-18.5	-18.5	0.44 (1)		10.00
S-AF	0 / 5782	-18.5	-18.5	0.44 (1)		10.00
AF- R	0 / 5782	-18.5	-18.5	0.44 (1)		10.00
R- Q	0 / 6902	-18.5	-18.5	0.50 (1)		10.00
Q- P	0 / 4840	-18.5	-18.5	0.36 (1)		10.00
P- O	0 / 4840	-18.5	-18.5	0.36 (1)		10.00
O- N	0 / 2187	-18.5	-18.5	0.17 (1)		10.00
N- M	0 / 0	-18.5	-18.5	0.04 (4)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
U- C	-633 / 0	0.11 (1)
C- T	0 / 3723	0.46 (1)
T- D	-2042 / 0	0.37 (1)
D- R	0 / 1875	0.23 (1)
R- E	-493 / 0	0.09 (1)
Q- H	-402 / 0	0.07 (1)
Q- I	0 / 2097	0.26 (1)
O- I	-1860 / 0	0.33 (1)
O- J	0 / 3302	0.41 (1)
N- J	-520 / 0	0.09 (1)
B- U	0 / 2925	0.36 (1)
N- K	0 / 2293	0.28 (1)
R- F	0 / 821	0.10 (1)
F- Q	-857 / 0	0.19 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	4-0-8	-146	-146	---	FRONT	VERT	TOTAL	---	C1
D	10-5-4	-60	-60	---	FRONT	VERT	TOTAL	---	C1
R	16-4-8	-901	-901	---	FRONT	VERT	TOTAL	---	C1
T	10-5-4	-15	-15	---	FRONT	VERT	TOTAL	---	C1
U	4-1-4	-15	-15	---	FRONT	VERT	TOTAL	---	C1
U	4-5-4	-15	-15	---	FRONT	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.25")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.20")
ALLOWABLE DEFL.(TL)= L/360 (1.25")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.38")

CSI: TC=0.83/1.00 (D-E:1) , BC=0.50/1.00 (Q-R:1) , WB=0.46/1.00 (C-T:1) , SSI=0.19/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (U) (INPUT = 0.90)
JSI METAL= 0.57 (S) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
422443	T1Z	1	2	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	7.0	8.0	Edge	2.25
D	TMWW-t	MT20	4.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMW+w	MT20	2.0	4.0		
I	TMWW-t	MT20	4.0	6.0		
J	TTWW+m	MT20	7.0	8.0	Edge	2.25
K	TMVW-p	MT20	5.0	8.0	Edge	
M	BMV1+p	MT20	3.0	6.0		
N, O, T, U						
N	BMWW-t	MT20	5.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	5.0	8.0		
R	BMWWW-t	MT20	5.0	8.0		
S	BS-t	MT20	5.0	6.0		
V	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
W	4-5-4	-77	-77	---	FRONT	VERT	TOTAL	---	C1
X	6-5-4	-60	-60	---	FRONT	VERT	TOTAL	---	C1
Y	8-5-4	-60	-60	---	FRONT	VERT	TOTAL	---	C1
Z	12-5-4	-60	-60	---	FRONT	VERT	TOTAL	---	C1
AA	14-5-4	-60	-60	---	FRONT	VERT	TOTAL	---	C1
AB	1-11-4	-15	-15	---	FRONT	VERT	TOTAL	---	C1
AC	6-5-4	-15	-15	---	FRONT	VERT	TOTAL	---	C1
AD	8-5-4	-15	-15	---	FRONT	VERT	TOTAL	---	C1
AE	12-5-4	-15	-15	---	FRONT	VERT	TOTAL	---	C1
AF	14-5-4	-15	-15	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

Structural component only
DWG# T-2205743

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
422291	T1S	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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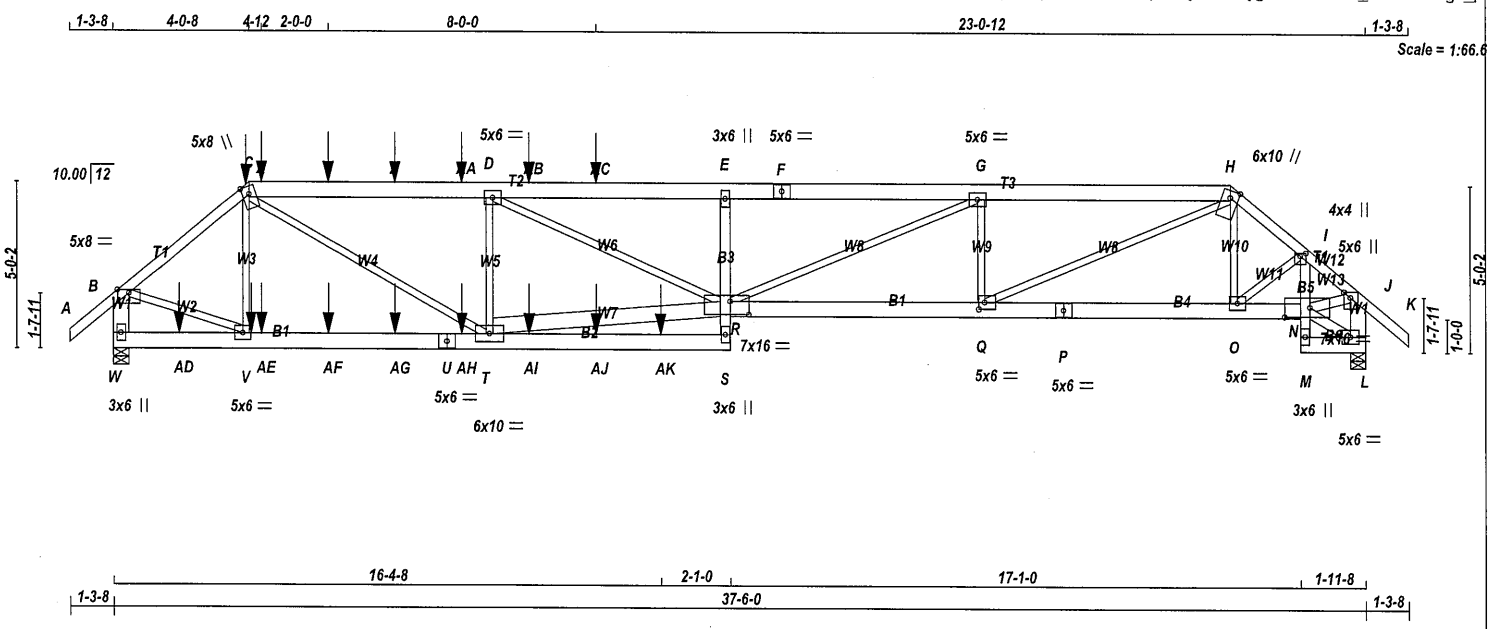

TOTAL WEIGHT = 2 X 206 = 412 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
W - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
W - U	2x6	DRY	No.2	SPF
U - S	2x6	DRY	No.2	SPF
S - E	2x4	DRY	No.2	SPF
R - P	2x6	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF
M - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
T - R	2x6	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
H- K	1	12	TOP
C- F	2	12	SIDE(61.0)
F- H	2	12	TOP
W- B	2	12	TOP
L- J	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
W- U	2	12	SIDE(183.1)
U- S	2	12	SIDE(0.0)
R- P	2	12	TOP
P- N	2	12	TOP
M- L	2	12	TOP
S- E	1	12	TOP
I- M	1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	

Structural component only
DWG# T-2205716

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
W	3492	0	3492	0	0	5-8	5-8
L	2803	0	2803	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	2473	1601 / 0	0 / 0	0 / 0	0 / 0	872 / 0	0 / 0
L	1985	1287 / 0	0 / 0	0 / 0	0 / 0	698 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.73 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	FORCE (LBS)	MAX. FACTORED MAX CSI (LC)
A- B	0 / 38	-84.9	-84.9	0.06 (1)	10.00	V- C	-804 / 0	0.14 (1)
B- C	-3581 / 0	-84.9	-84.9	0.20 (1)	4.80	O- H	0 / 97	0.02 (4)
C- X	-6350 / 0	-84.9	-84.9	0.34 (1)	4.43	O- I	0 / 179	0.02 (1)
X- Y	-6350 / 0	-84.9	-84.9	0.34 (1)	4.43	B- V	0 / 2887	0.36 (1)
Y- Z	-6350 / 0	-84.9	-84.9	0.34 (1)	4.43	C- T	0 / 4249	0.53 (1)
Z-AA	-6350 / 0	-84.9	-84.9	0.34 (1)	4.43	T- D	-2424 / 0	0.41 (1)
AA- D	-6350 / 0	-84.9	-84.9	0.34 (1)	4.43	T- R	0 / 6221	0.40 (1)
D-AB	-9105 / 0	-84.9	-84.9	0.43 (1)	3.73	D- R	0 / 3022	0.37 (1)
AB-AC	-9105 / 0	-84.9	-84.9	0.43 (1)	3.73	N- L	-85 / 0	0.00 (1)
AC- E	-9105 / 0	-84.9	-84.9	0.43 (1)	3.73	N- J	0 / 2666	0.24 (1)
E- F	-9173 / 0	-84.9	-84.9	0.36 (1)	3.81	Q- H	0 / 4482	0.55 (1)
F- G	-9173 / 0	-84.9	-84.9	0.36 (1)	3.81	R- G	0 / 2531	0.31 (1)
G- H	-6878 / 0	-84.9	-84.9	0.27 (1)	4.38	Q- G	-1821 / 0	0.21 (1)
H- I	-3667 / 0	-84.9	-84.9	0.09 (1)	4.85			
I- J	-3486 / 0	-84.9	-84.9	0.10 (1)	4.94			
J- K	0 / 38	-84.9	-84.9	0.06 (1)	10.00			
W- B	-3429 / 0	0.0	0.0	0.13 (1)	7.54			
L- J	-2745 / 0	0.0	0.0	0.10 (1)	7.81			
W-AD	0 / 0	-18.5	-18.5	0.03 (1)	10.00			
AD- V	0 / 0	-18.5	-18.5	0.03 (1)	10.00			
V-AE	0 / 2776	-18.5	-18.5	0.43 (1)	10.00			
AE-AF	0 / 2776	-18.5	-18.5	0.43 (1)	10.00			
AF-AG	0 / 2776	-18.5	-18.5	0.43 (1)	10.00			
AG- U	0 / 2776	-18.5	-18.5	0.43 (1)	10.00			
U-AH	0 / 2776	-18.5	-18.5	0.43 (1)	10.00			
AH- T	0 / 2776	-18.5	-18.5	0.43 (1)	10.00			
T-AI	0 / 243	-18.5	-18.5	0.66 (1)	10.00			
AI-AJ	0 / 243	-18.5	-18.5	0.66 (1)	10.00			
AJ-AK	0 / 243	-18.5	-18.5	0.66 (1)	10.00			
AK- S	0 / 243	-18.5	-18.5	0.66 (1)	10.00			
S- R	0 / 917	0.0	0.0	0.26 (1)	10.00			
R- E	-570 / 0	0.0	0.0	0.19 (1)	7.81			
R- Q	0 / 6878	-18.5	-18.5	0.50 (1)	10.00			
Q- P	0 / 2831	-18.5	-18.5	0.25 (1)	10.00			
P- O	0 / 2831	-18.5	-18.5	0.25 (1)	10.00			
O- N	0 / 2694	-18.5	-18.5	0.24 (1)	10.00			
M- N	0 / 19	0.0	0.0	0.06 (1)	10.00			
N- I	-348 / 0	0.0	0.0	0.06 (1)	7.81			
M- L	0 / 74	-18.5	-18.5	0.01 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.25")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.31")
ALLOWABLE DEFL.(TL)= L/360 (1.25")
CALCULATED VERT. DEFL.(TL) = L/ 773 (0.58")

CSI: TC=0.43/1.00 (D-E:1) , BC=0.66/1.00 (S-T:1) , WB=0.55/1.00 (H-Q:1) , SSI=0.30/1.00 (S-T:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (V) (INPUT = 0.90)
JSI METAL= 0.55 (R) (INPUT = 1.00)

CONTINUED ON PAGE 2

2x6 2 6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	5.0	8.0	Edge	
D	TMWW-t	MT20	5.0	6.0		
E	TMV+p	MT20	3.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMWW-t	MT20	5.0	6.0		
H	TTWW+m	MT20	6.0	10.0	Edge	
I	TMVW+p	MT20	4.0	4.0	1.00	2.00
J	TMVW+p	MT20	5.0	6.0	2.00	2.25
L	BMVW1-t	MT20	5.0	6.0		
M	BMV+p	MT20	3.0	6.0		
N	BVMWW-I	MT20	7.0	16.0	3.50	9.00
O	BMWW-t	MT20	5.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWW-t	MT20	5.0	6.0	2.50	2.00
R	BVMWWW-I	MT20	7.0	16.0	4.75	6.75
S	BMV+p	MT20	3.0	6.0		
T	BMWWW-t	MT20	6.0	10.0		
U	BS-t	MT20	5.0	6.0		
V	BMWW-t	MT20	5.0	6.0		
W	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	4-0-8	-146	-146	---	FRONT	VERT	TOTAL	---	C1
V	4-1-4	-15	-15	---	FRONT	VERT	TOTAL	---	C1
X	4-5-4	-77	-77	---	FRONT	VERT	TOTAL	---	C1
Y	6-5-4	-60	-60	---	FRONT	VERT	TOTAL	---	C1
Z	8-5-4	-60	-60	---	FRONT	VERT	TOTAL	---	C1
AA	10-5-4	-60	-60	---	FRONT	VERT	TOTAL	---	C1
AB	12-5-4	-60	-60	---	FRONT	VERT	TOTAL	---	C1
AC	14-5-4	-60	-60	---	FRONT	VERT	TOTAL	---	C1
AD	1-11-4	-15	-15	---	FRONT	VERT	TOTAL	---	C1
AE	4-5-4	-15	-15	---	FRONT	VERT	TOTAL	---	C1
AF	6-5-4	-15	-15	---	FRONT	VERT	TOTAL	---	C1
AG	8-5-4	-15	-15	---	FRONT	VERT	TOTAL	---	C1
AH	10-5-4	-15	-15	---	FRONT	VERT	TOTAL	---	C1
AI	12-5-4	-15	-15	---	FRONT	VERT	TOTAL	---	C1
AJ	14-5-4	-15	-15	---	FRONT	VERT	TOTAL	---	C1
AK	16-4-8	-901	-901	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

Structural component only
DWG# T-2205716

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
422443	T2	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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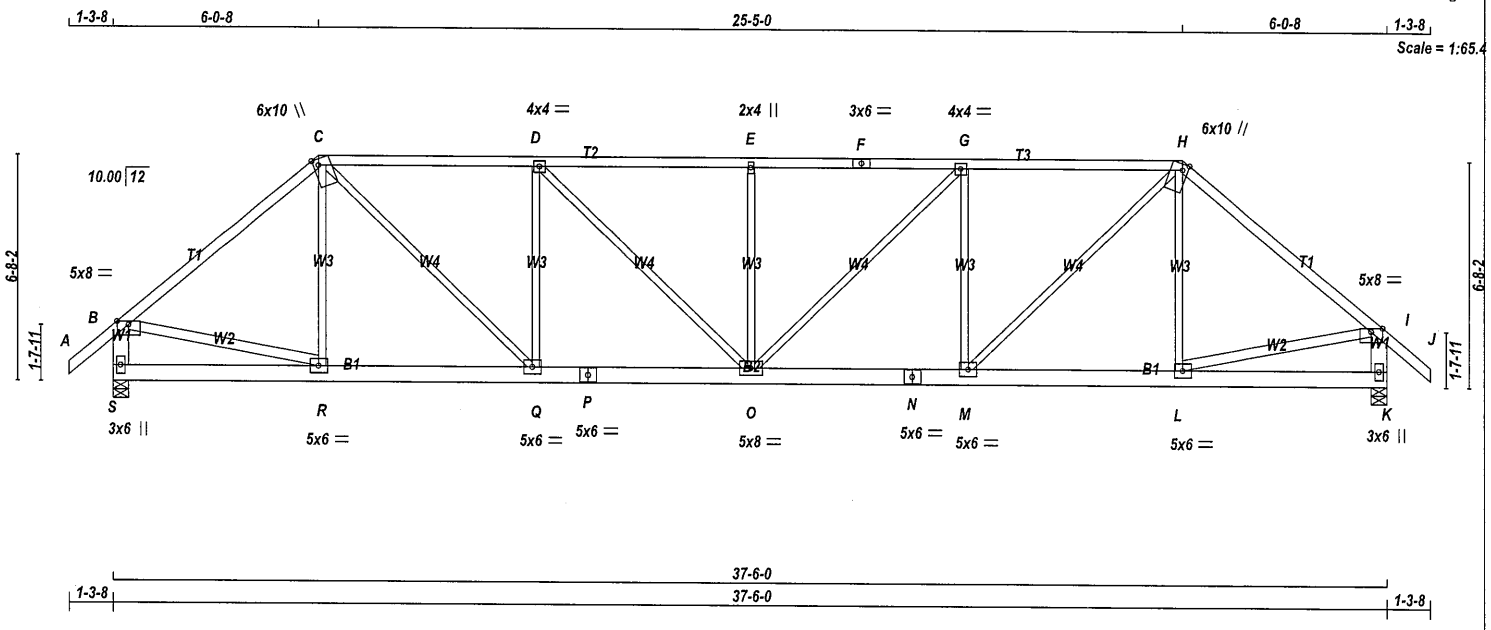

TOTAL WEIGHT = 188 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
B - R	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	6.0	10.0	Edge	1.75
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW+m	MT20	6.0	10.0	Edge	1.75
I	TMVW-p	MT20	5.0	8.0	Edge	
K	BMV1+p	MT20	3.0	6.0		
L, M, Q, R						
L	BMWW-t	MT20	5.0	6.0		
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BS-t	MT20	5.0	6.0		
S	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2205744

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	2056	0	2056	0	0	5-8	5-8
K	2056	0	2056	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1457	938 / 0	0 / 0	0 / 0	0 / 0	519 / 0	0 / 0
K	1457	938 / 0	0 / 0	0 / 0	0 / 0	519 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.31 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 38	-84.9	-84.9	0.12 (1)	10.00	R- C	-181 / 30	0.13 (1)
B- C	-2060 / 0	-84.9	-84.9	0.82 (1)	3.89	C- Q	0 / 1461	0.33 (1)
C- D	-2620 / 0	-84.9	-84.9	0.76 (1)	3.52	Q- D	-908 / 0	0.63 (1)
D- E	-2920 / 0	-84.9	-84.9	0.80 (1)	3.31	D- O	0 / 425	0.10 (1)
E- F	-2920 / 0	-84.9	-84.9	0.80 (1)	3.31	O- E	-492 / 0	0.34 (1)
F- G	-2920 / 0	-84.9	-84.9	0.80 (1)	3.31	O- G	0 / 425	0.10 (1)
G- H	-2620 / 0	-84.9	-84.9	0.76 (1)	3.52	M- G	-908 / 0	0.63 (1)
H- I	-2060 / 0	-84.9	-84.9	0.82 (1)	3.89	M- H	0 / 1461	0.33 (1)
I- J	0 / 38	-84.9	-84.9	0.12 (1)	10.00	L- H	-181 / 30	0.13 (1)
S- B	-2010 / 0	0.0	0.0	0.14 (1)	7.13	B- R	0 / 1611	0.26 (1)
K- I	-2010 / 0	0.0	0.0	0.14 (1)	7.13	L- I	0 / 1611	0.26 (1)
S- R	0 / 0	-18.5	-18.5	0.08 (4)	10.00			
R- Q	0 / 1577	-18.5	-18.5	0.23 (1)	10.00			
Q- P	0 / 2620	-18.5	-18.5	0.36 (1)	10.00			
P- O	0 / 2620	-18.5	-18.5	0.36 (1)	10.00			
O- N	0 / 2620	-18.5	-18.5	0.36 (1)	10.00			
N- M	0 / 2620	-18.5	-18.5	0.36 (1)	10.00			
M- L	0 / 1577	-18.5	-18.5	0.23 (1)	10.00			
L- K	0 / 0	-18.5	-18.5	0.08 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.25")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (1.25")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.27")

CSI: TC=0.82/1.00 (H-I:1) , BC=0.36/1.00 (O-Q:1) , WB=0.63/1.00 (D-Q:1) , SSI=0.26/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (F) (INPUT = 0.90)
JSI METAL= 0.48 (P) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
422291	T2S	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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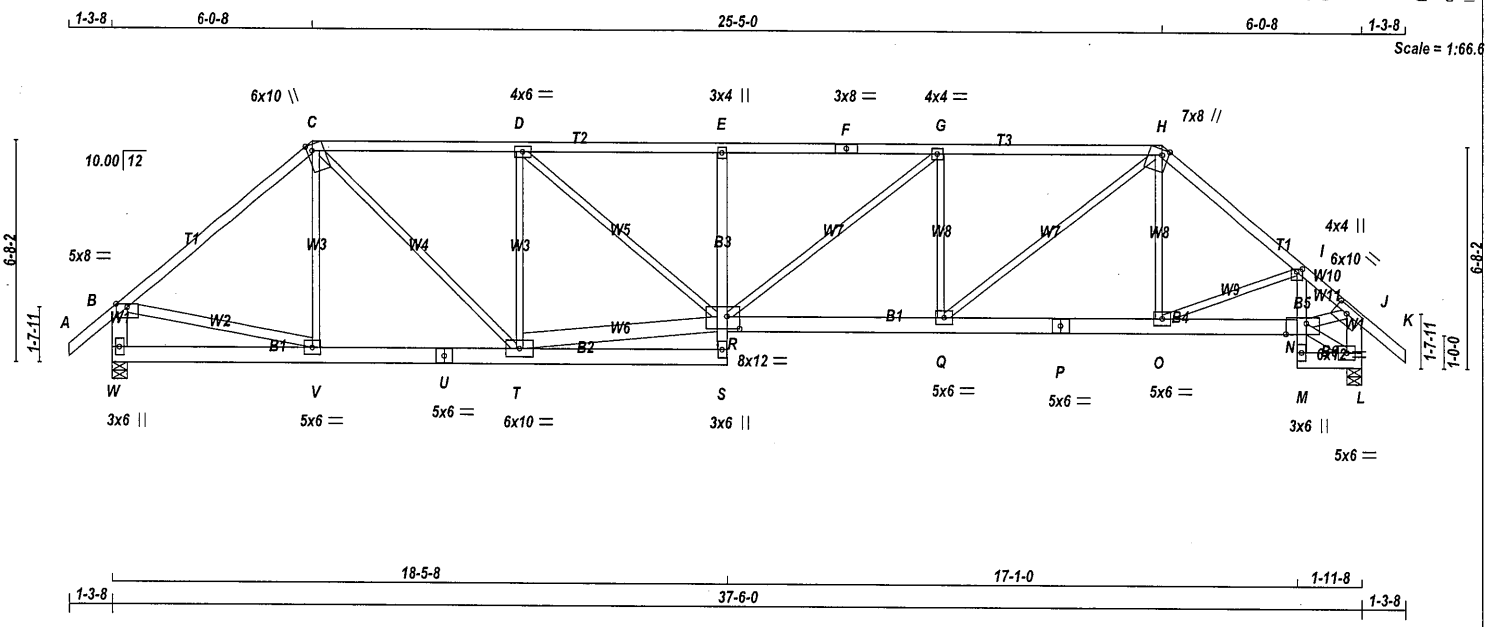

TOTAL WEIGHT = 200 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	2100F 1.8E	SPF
C - F	2x4	DRY	2100F 1.8E	SPF
F - H	2x4	DRY	2100F 1.8E	SPF
H - K	2x4	DRY	2100F 1.8E	SPF
W - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
W - U	2x6	DRY	No.2	SPF
U - S	2x6	DRY	No.2	SPF
S - E	2x4	DRY	No.2	SPF
R - P	2x6	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF
M - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
T - R	2x6	DRY	No.2	SPF
B - V	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	6.0	10.0	Edge	1.75
D	TMWW-t	MT20	4.0	6.0		
E	TMV+p	MT20	3.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW+m	MT20	7.0	8.0	Edge	2.25
I	TMVW+p	MT20	4.0	4.0	1.00	2.00
J	TMVW-t	MT20	6.0	10.0	2.50	4.50
L	BMVW1-t	MT20	5.0	6.0		
M	BMV+p	MT20	3.0	6.0		
N	BVMWW-l	MT20	6.0	12.0	4.00	7.50
O, Q, V						
O	BMWW-t	MT20	5.0	6.0		
P	BS-t	MT20	5.0	6.0		
R	BVMWWW-l	MT20	8.0	12.0	4.50	4.50
S	BMV+p	MT20	3.0	6.0		
T	BMWWW-t	MT20	6.0	10.0		

Structural component only
DWG# T-2205717

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
W	2053	0	2053	0	0	5-8	5-8
L	2058	0	2058	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	1455	937 / 0	0 / 0	0 / 0	0 / 0	519 / 0	0 / 0
L	1458	939 / 0	0 / 0	0 / 0	0 / 0	519 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.16 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 38	-84.9	-84.9	0.08 (1)	10.00
B- C	-2057 / 0	-84.9	-84.9	0.46 (1)	5.16
C- D	-2581 / 0	-84.9	-84.9	0.38 (1)	4.80
D- E	-3448 / 0	-84.9	-84.9	0.43 (1)	4.24
E- F	-3460 / 0	-84.9	-84.9	0.51 (1)	4.16
F- G	-3460 / 0	-84.9	-84.9	0.51 (1)	4.16
G- H	-3133 / 0	-84.9	-84.9	0.49 (1)	4.35
H- I	-2442 / 0	-84.9	-84.9	0.17 (1)	5.15
I- J	-2524 / 0	-84.9	-84.9	0.11 (1)	5.13
J- K	0 / 38	-84.9	-84.9	0.08 (1)	10.00
W- B	-2008 / 0	0.0	0.0	0.14 (1)	7.13
L- J	-2015 / 0	0.0	0.0	0.14 (1)	7.12
W- V	0 / 0	-18.5	-18.5	0.08 (4)	10.00
V- U	0 / 1574	-18.5	-18.5	0.23 (1)	10.00
U- T	0 / 1574	-18.5	-18.5	0.23 (1)	10.00
T- S	0 / 61	-18.5	-18.5	0.07 (4)	10.00
S- R	0 / 52	0.0	0.0	0.09 (1)	10.00
R- E	-491 / 0	0.0	0.0	0.19 (1)	7.81
R- Q	0 / 3133	-18.5	-18.5	0.43 (1)	10.00
Q- P	0 / 1863	-18.5	-18.5	0.27 (1)	10.00
P- O	0 / 1863	-18.5	-18.5	0.27 (1)	10.00
O- N	0 / 1971	-18.5	-18.5	0.28 (1)	10.00
M- N	0 / 19	0.0	0.0	0.07 (1)	10.00
N- I	-184 / 0	0.0	0.0	0.07 (1)	7.81
M- L	0 / 46	-18.5	-18.5	0.01 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V- C	-185 / 24	0.13 (1)
C- T	0 / 1440	0.32 (1)
T- D	-1328 / 0	0.92 (1)
T- R	0 / 2569	0.30 (1)
D- R	0 / 1131	0.25 (1)
R- G	0 / 421	0.09 (1)
Q- G	-896 / 0	0.41 (1)
Q- H	0 / 1626	0.37 (1)
O- H	0 / 155	0.05 (4)
O- I	-120 / 0	0.04 (1)
B- V	0 / 1608	0.26 (1)
N- L	-53 / 0	0.01 (1)
N- J	0 / 1955	0.31 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.25")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (1.25")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.30")

CSI: TC=0.51/1.00 (E-G:1) , BC=0.43/1.00 (Q-R:1) , WB=0.92/1.00 (D-T:1) , SSI=0.26/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (Q) (INPUT = 0.90)
JSI METAL= 0.53 (T) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
422291	T2S	1	1	TRUSS DESC.	

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
U	BS-t	MT20	5.0	6.0		
W	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2205717

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
422291	T3	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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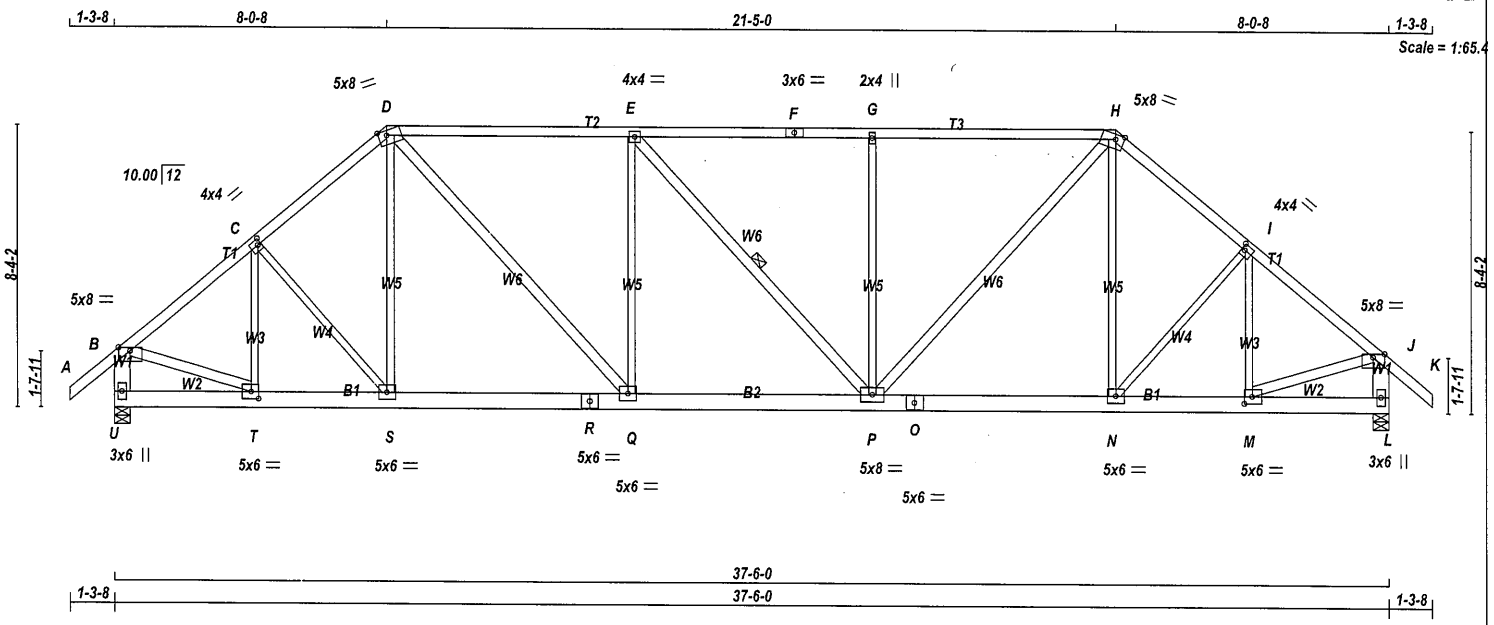

TOTAL WEIGHT = 209 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	2100F 1.8E	SPF
F - H	2x4	DRY	2100F 1.8E	SPF
H - K	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
U - R	2x6	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - Q	2x4	DRY	No.2	SPF
E - P	2x4	DRY	No.2	SPF
P - H	2x4	DRY	No.2	SPF
B - T	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW-m	MT20	5.0	8.0	Edge	3.00
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	8.0	Edge	3.00
I	TMWW-t	MT20	4.0	4.0	2.00	1.25
J	TMVW-p	MT20	5.0	8.0	Edge	
L	BMV1+p	MT20	3.0	6.0		
M	BMWW-t	MT20	5.0	6.0	2.50	2.75
N, Q, S						
N	BMWW-t	MT20	5.0	6.0		
O	BS-t	MT20	5.0	6.0		
P	BMWWW-t	MT20	5.0	8.0		
R	BS-t	MT20	5.0	6.0		
T	BMWW-t	MT20	5.0	6.0	2.50	2.75
U	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

Structural component only
DWG# T-2205718

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2056	0	2056	0	0	5-8	5-8
L	2056	0	2056	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1457	938 / 0	0 / 0	0 / 0	0 / 0	519 / 0	0 / 0
L	1457	938 / 0	0 / 0	0 / 0	0 / 0	519 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.46 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 38	-84.9	-84.9	0.12 (1)		10.00
B- C	-2001 / 0	-84.9	-84.9	0.30 (1)		4.50
C- D	-2046 / 0	-84.9	-84.9	0.30 (1)		4.46
D- E	-2261 / 0	-84.9	-84.9	0.49 (1)		4.91
E- F	-2258 / 0	-84.9	-84.9	0.49 (1)		4.91
F- G	-2258 / 0	-84.9	-84.9	0.49 (1)		4.91
G- H	-2258 / 0	-84.9	-84.9	0.48 (1)		4.93
H- I	-2047 / 0	-84.9	-84.9	0.30 (1)		4.46
I- J	-2001 / 0	-84.9	-84.9	0.30 (1)		4.50
J- K	0 / 38	-84.9	-84.9	0.12 (1)		10.00
U- B	-2012 / 0	0.0	0.0	0.14 (1)		7.13
L- J	-2011 / 0	0.0	0.0	0.14 (1)		7.13
U- T	0 / 0	-18.5	-18.5	0.04 (4)		10.00
T- S	0 / 1556	-18.5	-18.5	0.22 (1)		10.00
S- R	0 / 1549	-18.5	-18.5	0.23 (1)		10.00
R- Q	0 / 1549	-18.5	-18.5	0.23 (1)		10.00
Q- P	0 / 2261	-18.5	-18.5	0.31 (1)		10.00
P- O	0 / 1550	-18.5	-18.5	0.23 (1)		10.00
O- N	0 / 1550	-18.5	-18.5	0.23 (1)		10.00
N- M	0 / 1556	-18.5	-18.5	0.23 (1)		10.00
M- L	0 / 0	-18.5	-18.5	0.04 (4)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI
T- C	-412 / 0	0.15 (1)
C- S	-13 / 7	0.01 (1)
S- D	0 / 132	0.04 (4)
D- Q	0 / 1062	0.17 (1)
Q- E	-654 / 0	0.85 (1)
E- P	-5 / 0	0.00 (1)
P- G	-655 / 0	0.85 (1)
P- H	0 / 1056	0.17 (1)
N- H	0 / 137	0.04 (4)
N- I	-12 / 7	0.01 (1)
M- I	-414 / 0	0.15 (1)
B- T	0 / 1623	0.26 (1)
M- J	0 / 1623	0.26 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.25")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (1.25")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.49/1.00 (D-E:1) , BC=0.31/1.00 (P-Q:1) , WB=0.85/1.00 (G-P:1) , SSI=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (T) (INPUT = 0.90)
JSI METAL= 0.35 (T) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
422291	T3	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2205718

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
422291	T4	4	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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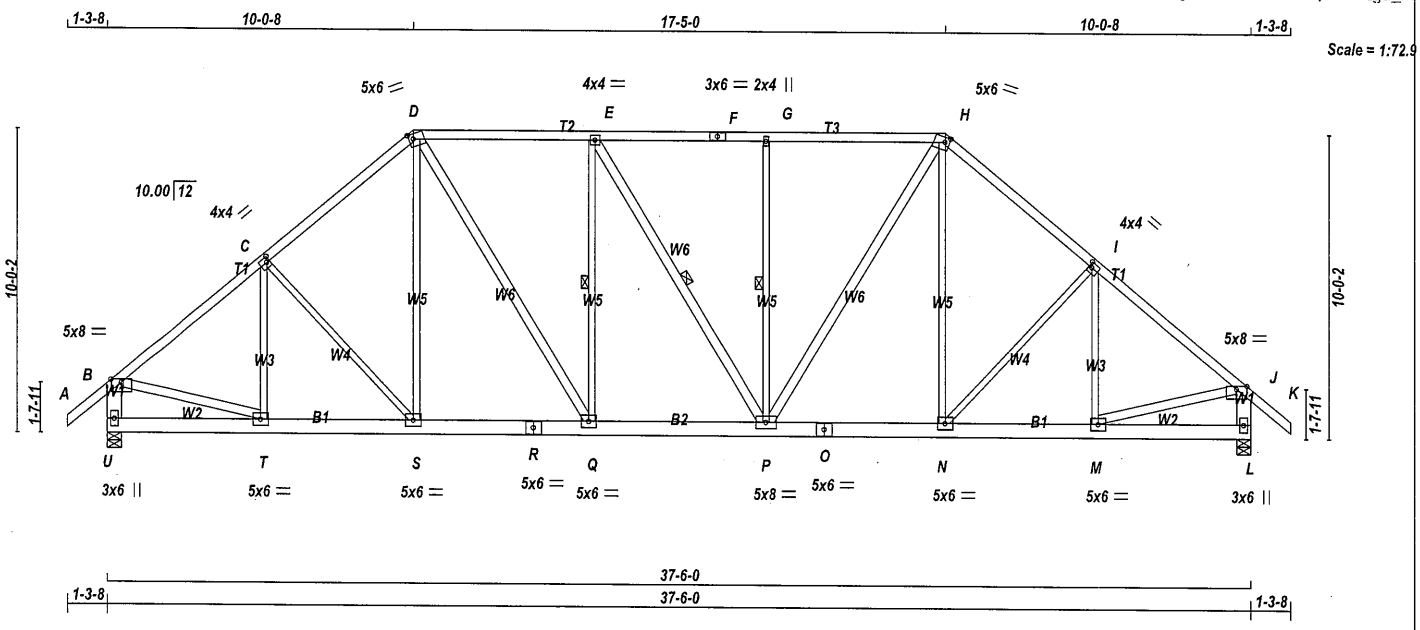

TOTAL WEIGHT = 4 X 223 = 893 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
U - R	2x6	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - Q	2x4	DRY	No.2	SPF
E - P	2x4	DRY	No.2	SPF
P - H	2x4	DRY	No.2	SPF
B - T	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW-m	MT20	5.0	6.0	2.00	1.75
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	6.0	2.00	1.75
I	TMWW-t	MT20	4.0	4.0	2.00	1.25
J	TMVW-p	MT20	5.0	8.0	Edge	
L	BMV1+p	MT20	3.0	6.0		
M, N, Q, S, T						
M	BMWW-t	MT20	5.0	6.0		
O	BS-t	MT20	5.0	6.0		
P	BMWWW-t	MT20	5.0	8.0		
R	BS-t	MT20	5.0	6.0		
U	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1)

Structural component only
DWG# T-2205719

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2056	0	2056	0	0	5-8	5-8
L	2056	0	2056	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1457	938 / 0	0 / 0	0 / 0	0 / 0	519 / 0	0 / 0
L	1457	938 / 0	0 / 0	0 / 0	0 / 0	519 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.41 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-Q, E-P, G-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 38	-84.9	-84.9	0.12 (1)	10.00
B- C	-2061 / 0	-84.9	-84.9	0.36 (1)	4.41
C- D	-1971 / 0	-84.9	-84.9	0.34 (1)	4.50
D- E	-1876 / 0	-84.9	-84.9	0.38 (1)	4.52
E- F	-1872 / 0	-84.9	-84.9	0.35 (1)	4.57
F- G	-1872 / 0	-84.9	-84.9	0.35 (1)	4.57
G- H	-1872 / 0	-84.9	-84.9	0.38 (1)	4.52
H- I	-1972 / 0	-84.9	-84.9	0.34 (1)	4.50
I- J	-2061 / 0	-84.9	-84.9	0.36 (1)	4.41
J- K	0 / 38	-84.9	-84.9	0.12 (1)	10.00
U- B	-2008 / 0	0.0	0.0	0.14 (1)	7.13
L- J	-2008 / 0	0.0	0.0	0.14 (1)	7.13
U- T	0 / 0	-18.5	-18.5	0.05 (4)	10.00
T- S	0 / 1608	-18.5	-18.5	0.22 (1)	10.00
S- R	0 / 1488	-18.5	-18.5	0.21 (1)	10.00
R- Q	0 / 1488	-18.5	-18.5	0.21 (1)	10.00
Q- P	0 / 1876	-18.5	-18.5	0.25 (1)	10.00
P- O	0 / 1488	-18.5	-18.5	0.21 (1)	10.00
O- N	0 / 1488	-18.5	-18.5	0.21 (1)	10.00
N- M	0 / 1608	-18.5	-18.5	0.22 (1)	10.00
M- L	0 / 0	-18.5	-18.5	0.05 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- C	-297 / 0	0.15 (1)
C- S	-183 / 0	0.20 (1)
S- D	0 / 242	0.05 (1)
D- Q	0 / 748	0.12 (1)
Q- E	-527 / 0	0.34 (1)
E- P	-7 / 0	0.01 (1)
P- G	-530 / 0	0.35 (1)
P- H	0 / 739	0.12 (1)
N- H	0 / 249	0.06 (1)
N- I	-181 / 0	0.19 (1)
M- I	-300 / 0	0.15 (1)
B- T	0 / 1653	0.27 (1)
M- J	0 / 1653	0.27 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.25")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (1.25")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.38/1.00 (D-E:1) , BC=0.25/1.00 (P-Q:1) , WB=0.35/1.00 (G-P:1) , SSI=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (T) (INPUT = 0.90)
JSI METAL= 0.40 (H) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
422291	T5	5	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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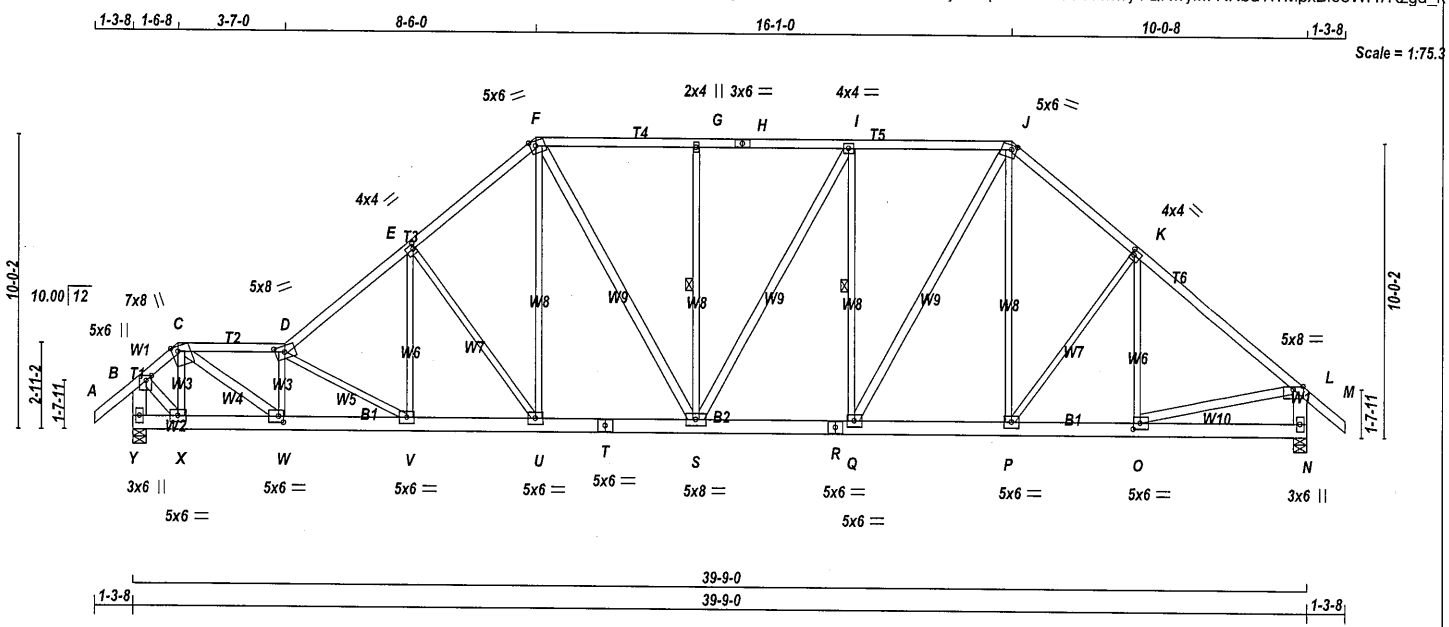

TOTAL WEIGHT = 5 X 239 = 1196 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF
Y - B	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
Y - T	2x6	DRY	No.2	SPF
T - R	2x6	DRY	No.2	SPF
R - N	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - W	2x4	DRY	No.2	SPF
B - X	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
Q - J	2x4	DRY	No.2	SPF
F - S	2x4	DRY	No.2	SPF
S - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTWW+m	MT20	7.0	8.0	Edge	2.25
D	TTWW-m	MT20	5.0	8.0	2.50	3.75
E	TMWW-t	MT20	4.0	4.0	2.00	1.00
F	TTWW-m	MT20	5.0	6.0	2.00	2.25
G	TMW+w	MT20	2.0	4.0		
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	4.0	4.0		
J	TTWW-m	MT20	5.0	6.0	1.75	2.00
K	TMWW-t	MT20	4.0	4.0	2.00	1.25
L	TMVW-p	MT20	5.0	8.0	Edge	
N	BMV1+p	MT20	3.0	6.0		
O	BMWW-t	MT20	5.0	6.0	2.50	2.75
P, Q, U, V, X						
P	BMWW-t	MT20	5.0	6.0		
R	BS-t	MT20	5.0	6.0		
S	BMWWW-t	MT20	5.0	8.0		
T	BS-t	MT20	5.0	6.0		

Structural component only
DWG# T-2205720

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Y	2174	0	2174	0	0	5-8	5-8
N	2170	0	2170	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Y	1541	991 / 0	0 / 0	0 / 0	0 / 0	550 / 0	0 / 0
N	1538	989 / 0	0 / 0	0 / 0	0 / 0	549 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Y, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF I-Q, G-S.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 38	-84.9	-84.9	0.12 (1)	10.00
B- C	-1678 / 0	-84.9	-84.9	0.11 (1)	5.04
C- D	-3459 / 0	-84.9	-84.9	0.28 (1)	3.58
D- E	-3154 / 0	-84.9	-84.9	0.34 (1)	3.70
E- F	-2511 / 0	-84.9	-84.9	0.28 (1)	4.13
F- G	-2136 / 0	-84.9	-84.9	0.34 (1)	4.34
G- H	-2137 / 0	-84.9	-84.9	0.32 (1)	4.36
H- I	-2137 / 0	-84.9	-84.9	0.32 (1)	4.36
I- J	-2039 / 0	-84.9	-84.9	0.34 (1)	4.43
J- K	-2133 / 0	-84.9	-84.9	0.36 (1)	4.32
K- L	-2215 / 0	-84.9	-84.9	0.46 (1)	4.19
L- M	0 / 38	-84.9	-84.9	0.12 (1)	10.00
Y- B	-2229 / 0	0.0	0.0	0.15 (1)	6.85
N- L	-2117 / 0	0.0	0.0	0.14 (1)	6.99
Y- X	0 / 0	-18.5	-18.5	0.08 (1)	10.00
X- W	0 / 1224	-18.5	-18.5	0.26 (1)	10.00
W- V	0 / 3528	-18.5	-18.5	0.53 (1)	10.00
V- U	0 / 2450	-18.5	-18.5	0.32 (1)	10.00
U- T	0 / 1913	-18.5	-18.5	0.25 (1)	10.00
T- S	0 / 1913	-18.5	-18.5	0.25 (1)	10.00
S- R	0 / 2039	-18.5	-18.5	0.27 (1)	10.00
R- Q	0 / 2039	-18.5	-18.5	0.27 (1)	10.00
Q- P	0 / 1606	-18.5	-18.5	0.22 (1)	10.00
P- O	0 / 1725	-18.5	-18.5	0.22 (1)	10.00
O- N	0 / 0	-18.5	-18.5	0.07 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
X- C	-727 / 0	0.12 (1)
C- W	0 / 2763	0.44 (1)
W- D	-1678 / 0	0.28 (1)
D- V	-1255 / 0	0.51 (1)
V- E	0 / 747	0.17 (1)
E- U	-934 / 0	1.00 (1)
U- F	0 / 865	0.19 (1)
P- J	0 / 262	0.06 (1)
P- K	-209 / 0	0.22 (1)
O- K	-275 / 0	0.17 (1)
B- X	0 / 1562	0.25 (1)
O- L	0 / 1763	0.28 (1)
Q- J	0 / 886	0.14 (1)
F- S	0 / 456	0.07 (1)
Q- I	-667 / 0	0.44 (1)
S- G	-486 / 0	0.32 (1)
S- I	0 / 202	0.03 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.32")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (1.32")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.26")

CSI: TC=0.46/1.00 (K-L:1) , BC=0.53/1.00 (V-W:1) , WB=1.00/1.00 (E-U:1) , SSI=0.21/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (W) (INPUT = 0.90)
JSI METAL= 0.55 (D) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
422291	T5	5	1	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
W	BMWW-t	MT20	5.0	6.0	2.50	2.00
Y	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2205720

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
422291	T5Z	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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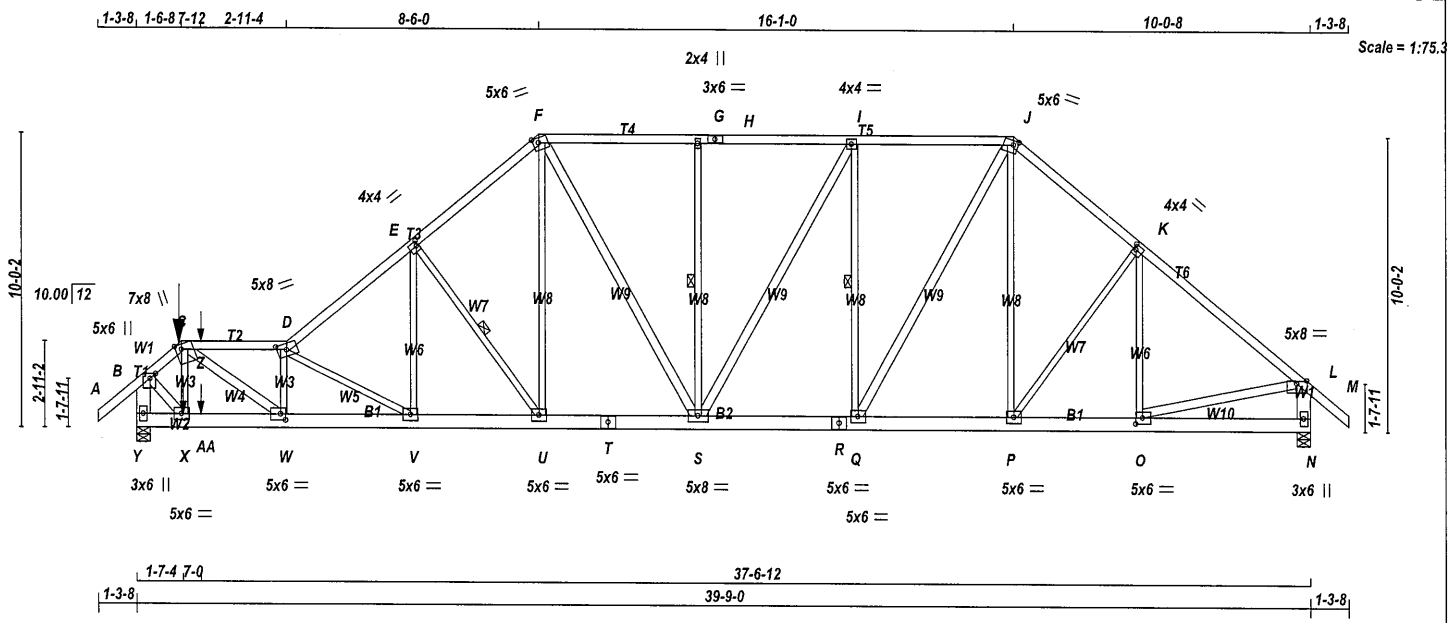

TOTAL WEIGHT = 2 X 239 = 478 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF
Y - B	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
Y - T	2x6	DRY	No.2	SPF
T - R	2x6	DRY	No.2	SPF
R - N	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - W	2x4	DRY	No.2	SPF
B - X	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
Q - J	2x4	DRY	No.2	SPF
F - S	2x4	DRY	No.2	SPF
S - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTWW+m	MT20	7.0	8.0	Edge	2.25
D	TTWW-m	MT20	5.0	8.0	2.50	3.75
E	TMWW-t	MT20	4.0	4.0	2.00	1.00
F	TTWW-m	MT20	5.0	6.0	2.00	2.25
G	TMW+w	MT20	2.0	4.0		
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	4.0	4.0		
J	TTWW-m	MT20	5.0	6.0	1.75	2.00
K	TMWW-t	MT20	4.0	4.0	2.00	1.25
L	TMVW-p	MT20	5.0	8.0	Edge	
N	BMV1+p	MT20	3.0	6.0		
O	BMWW-t	MT20	5.0	6.0	2.50	2.75
P, Q, U, V, X						
P	BMWW-t	MT20	5.0	6.0		
R	BS-t	MT20	5.0	6.0		
S	BMWWW-t	MT20	5.0	8.0		
T	BS-t	MT20	5.0	6.0		

Structural component only
DWG# T-2205721

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Y	2227	0	2227	0	0	5-8	5-8
N	2172	0	2172	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Y	1579	1015 / 0	0 / 0	0 / 0	0 / 0	564 / 0	0 / 0
N	1540	990 / 0	0 / 0	0 / 0	0 / 0	550 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Y, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.51 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-U, I-Q, G-S.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (7)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 38	-84.9	-84.9	0.12 (1)	10.00
B- C	-1722 / 0	-84.9	-84.9	0.12 (1)	4.99
C- Z	-3488 / 0	-84.9	-84.9	0.32 (1)	3.51
Z- D	-3488 / 0	-84.9	-84.9	0.32 (1)	3.51
D- E	-3168 / 0	-84.9	-84.9	0.34 (1)	3.69
E- F	-2519 / 0	-84.9	-84.9	0.28 (1)	4.13
F- G	-2141 / 0	-84.9	-84.9	0.34 (1)	4.33
G- H	-2141 / 0	-84.9	-84.9	0.32 (1)	4.36
H- I	-2141 / 0	-84.9	-84.9	0.32 (1)	4.36
I- J	-2043 / 0	-84.9	-84.9	0.34 (1)	4.43
J- K	-2136 / 0	-84.9	-84.9	0.36 (1)	4.32
K- L	-2218 / 0	-84.9	-84.9	0.46 (1)	4.19
L- M	0 / 38	-84.9	-84.9	0.12 (1)	10.00
Y- B	-2281 / 0	0.0	0.0	0.15 (1)	6.79
N- L	-2119 / 0	0.0	0.0	0.14 (1)	6.98
Y- X	0 / 0	-18.5	-18.5	0.08 (5)	10.00
X-AA	0 / 1257	-18.5	-18.5	0.27 (1)	10.00
AA- W	0 / 1257	-18.5	-18.5	0.27 (1)	10.00
W- V	0 / 3557	-18.5	-18.5	0.54 (1)	10.00
V- U	0 / 2460	-18.5	-18.5	0.32 (1)	10.00
U- T	0 / 1919	-18.5	-18.5	0.25 (1)	10.00
T- S	0 / 1919	-18.5	-18.5	0.25 (1)	10.00
S- R	0 / 2043	-18.5	-18.5	0.27 (1)	10.00
R- Q	0 / 2043	-18.5	-18.5	0.27 (1)	10.00
Q- P	0 / 1608	-18.5	-18.5	0.22 (1)	10.00
P- O	0 / 1727	-18.5	-18.5	0.23 (1)	10.00
O- N	0 / 0	-18.5	-18.5	0.07 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
X- C	-752 / 0	0.13 (1)
C- W	0 / 2759	0.44 (1)
W- D	-1677 / 0	0.28 (1)
D- V	-1276 / 0	0.52 (1)
V- E	0 / 758	0.17 (1)
E- U	-943 / 0	0.35 (1)
U- F	0 / 872	0.20 (1)
P- J	0 / 264	0.06 (5)
P- K	-210 / 0	0.23 (5)
O- K	-275 / 0	0.17 (1)
B- X	0 / 1603	0.26 (1)
O- L	0 / 1765	0.28 (1)
Q- J	0 / 888	0.14 (1)
F- S	0 / 467	0.08 (5)
Q- I	-669 / 0	0.44 (1)
S- G	-486 / 0	0.32 (5)
S- I	0 / 204	0.03 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	1-6-8	-39	-39	76	FRONT	VERT	TOTAL	---	C1
X	1-7-4	1	1	---	FRONT	VERT	TOTAL	---	C1
Z	2-2-4	1	1	67	FRONT	VERT	TOTAL	---	C1
AA	2-2-4	1	1	---	FRONT	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.32")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (1.32")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.26")

CSI: TC=0.46/1.00 (K-L:1) , BC=0.54/1.00 (V-W:1) , WB=0.52/1.00 (D-V:1) , SSI=0.21/1.00 (I-J:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (W) (INPUT = 0.90)
JSI METAL= 0.55 (D) (INPUT = 1.00)

CONTINUED ON PAGE 2

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
W	BMWW-t	MT20	5.0	6.0	2.50	2.00
Y	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

Structural component only
DWG# T-2205721

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
422291	T6	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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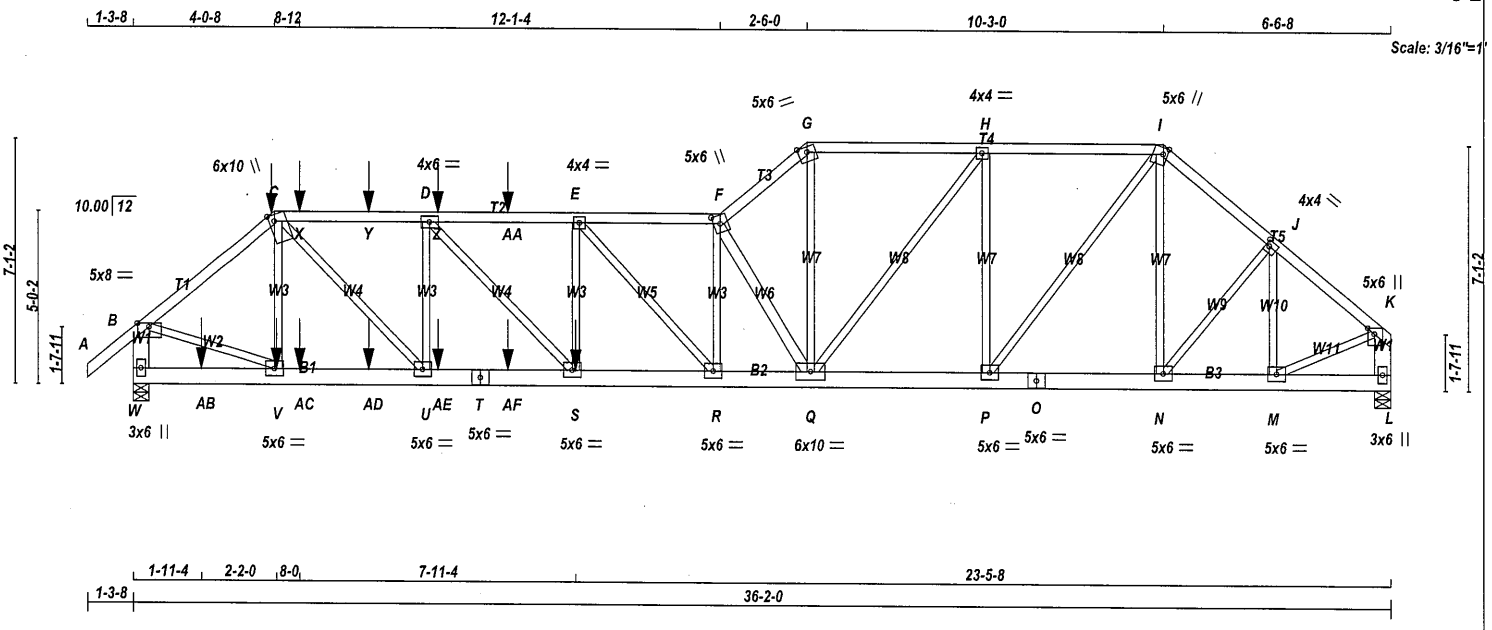

TOTAL WEIGHT = 2 X 192 = 384 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
W - B	2x6	DRY	No.2	SPF
L - K	2x6	DRY	No.2	SPF
W - T	2x6	DRY	No.2	SPF
T - O	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
F - Q	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- F	1	12	SIDE(61.0)
F- G	1	12	TOP
G- I	1	12	TOP
I- K	1	12	TOP
W- B	2	12	TOP
L- K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
W- T	2	12	SIDE(183.1)
T- O	2	12	SIDE(183.1)
O- L	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
E- S	1	6	SIDE(41.5)
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

Structural component only
DWG# T-2205722

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
W	3108	0	3108	0	0	5-8	5-8
L	2291	0	2291	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	2203	1421 / 0	0 / 0	0 / 0	0 / 0	782 / 0	0 / 0
L	1625	1037 / 0	0 / 0	0 / 0	0 / 0	588 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 38	-84.9	-84.9	0.06 (1)	10.00
B- C	-3206 / 0	-84.9	-84.9	0.19 (1)	5.02
C- X	-4323 / 0	-84.9	-84.9	0.26 (1)	4.36
X- Y	-4323 / 0	-84.9	-84.9	0.26 (1)	4.36
Y- D	-4323 / 0	-84.9	-84.9	0.26 (1)	4.36
D- Z	-5473 / 0	-84.9	-84.9	0.31 (1)	3.89
Z-AA	-5473 / 0	-84.9	-84.9	0.31 (1)	3.89
AA- E	-5473 / 0	-84.9	-84.9	0.31 (1)	3.89
E- F	-5385 / 0	-84.9	-84.9	0.22 (1)	4.02
F- G	-4572 / 0	-84.9	-84.9	0.12 (1)	4.41
G- H	-3553 / 0	-84.9	-84.9	0.26 (1)	4.73
H- I	-2971 / 0	-84.9	-84.9	0.24 (1)	5.10
I- J	-2525 / 0	-84.9	-84.9	0.10 (1)	5.61
J- K	-2302 / 0	-84.9	-84.9	0.10 (1)	5.81
W- B	-3073 / 0	0.0	0.0	0.11 (1)	7.81
L- K	-2245 / 0	0.0	0.0	0.08 (1)	7.81
W-AB	0 / 0	-18.5	-18.5	0.03 (4)	10.00
AB- V	0 / 0	-18.5	-18.5	0.03 (4)	10.00
V-AC	0 / 2443	-18.5	-18.5	0.18 (1)	10.00
AC-AD	0 / 2443	-18.5	-18.5	0.18 (1)	10.00
AD- U	0 / 2443	-18.5	-18.5	0.18 (1)	10.00
U-AE	0 / 4323	-18.5	-18.5	0.32 (1)	10.00
AE- T	0 / 4323	-18.5	-18.5	0.32 (1)	10.00
T-AF	0 / 4323	-18.5	-18.5	0.32 (1)	10.00
AF- S	0 / 4323	-18.5	-18.5	0.32 (1)	10.00
S- R	0 / 5473	-18.5	-18.5	0.41 (1)	10.00
R- Q	0 / 5382	-18.5	-18.5	0.40 (1)	10.00
Q- P	0 / 2972	-18.5	-18.5	0.22 (1)	10.00
P- O	0 / 1917	-18.5	-18.5	0.15 (1)	10.00
O- N	0 / 1917	-18.5	-18.5	0.15 (1)	10.00
N- M	0 / 1783	-18.5	-18.5	0.14 (1)	10.00
M- L	0 / 0	-18.5	-18.5	0.02 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V- C	-568 / 0	0.10 (1)
S- E	-293 / 0	0.05 (1)
E- R	-135 / 0	0.04 (1)
R- F	0 / 106	0.01 (1)
F- Q	-3803 / 0	0.63 (1)
Q- G	0 / 2638	0.33 (1)
Q- H	0 / 972	0.12 (1)
P- H	-1283 / 0	0.53 (1)
P- I	0 / 1735	0.21 (1)
N- I	-50 / 36	0.02 (1)
N- J	0 / 215	0.03 (1)
M- J	-630 / 0	0.09 (1)
B- V	0 / 2560	0.32 (1)
M- K	0 / 1896	0.23 (1)
D- S	0 / 1695	0.21 (1)
C- U	0 / 2728	0.34 (1)
U- D	-1857 / 0	0.33 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	4-0-8	-146	-146	---	FRONT	VERT	TOTAL	---	C1
S	12-8-8	-609	-609	---	FRONT	VERT	TOTAL	---	C1
V	4-1-4	-15	-15	---	FRONT	VERT	TOTAL	---	C1
X	4-9-4	-68	-68	---	FRONT	VERT	TOTAL	---	C1
Y	6-9-4	-60	-60	---	FRONT	VERT	TOTAL	---	C1
Z	8-9-4	-60	-60	---	FRONT	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.21")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (1.21")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.27")

CSI: TC=0.31/1.00 (D-E:1) , BC=0.41/1.00 (R-S:1) , WB=0.63/1.00 (F-Q:1) , SSI=0.14/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (F) (INPUT = 0.90)
JSI METAL= 0.41 (G) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
422291	T6	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	6.0	10.0	Edge	1.75
D	TMWW-t	MT20	4.0	6.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTWW+m	MT20	5.0	6.0	3.00	2.25
G	TTW-m	MT20	5.0	6.0	Edge	
H	TMWW-t	MT20	4.0	4.0		
I	TTWW+m	MT20	5.0	6.0	2.25	1.50
J	TMWW-t	MT20	4.0	4.0	2.00	1.25
K	TMVW+p	MT20	5.0	6.0	2.00	2.25
L	BMV1+p	MT20	3.0	6.0		
M, N, P, R, S, U, V						
M	BMWW-t	MT20	5.0	6.0		
O	BS-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	6.0	10.0		
T	BS-t	MT20	5.0	6.0		
W	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
AA	10-9-4	-60	-60	---	FRONT	VERT	TOTAL	---	C1
AB	1-11-4	-15	-15	---	FRONT	VERT	TOTAL	---	C1
AC	4-9-4	-15	-15	---	FRONT	VERT	TOTAL	---	C1
AD	6-9-4	-15	-15	---	FRONT	VERT	TOTAL	---	C1
AE	8-9-4	-15	-15	---	FRONT	VERT	TOTAL	---	C1
AF	10-9-4	-15	-15	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

Structural component only
DWG# T-2205722

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
422291	T7	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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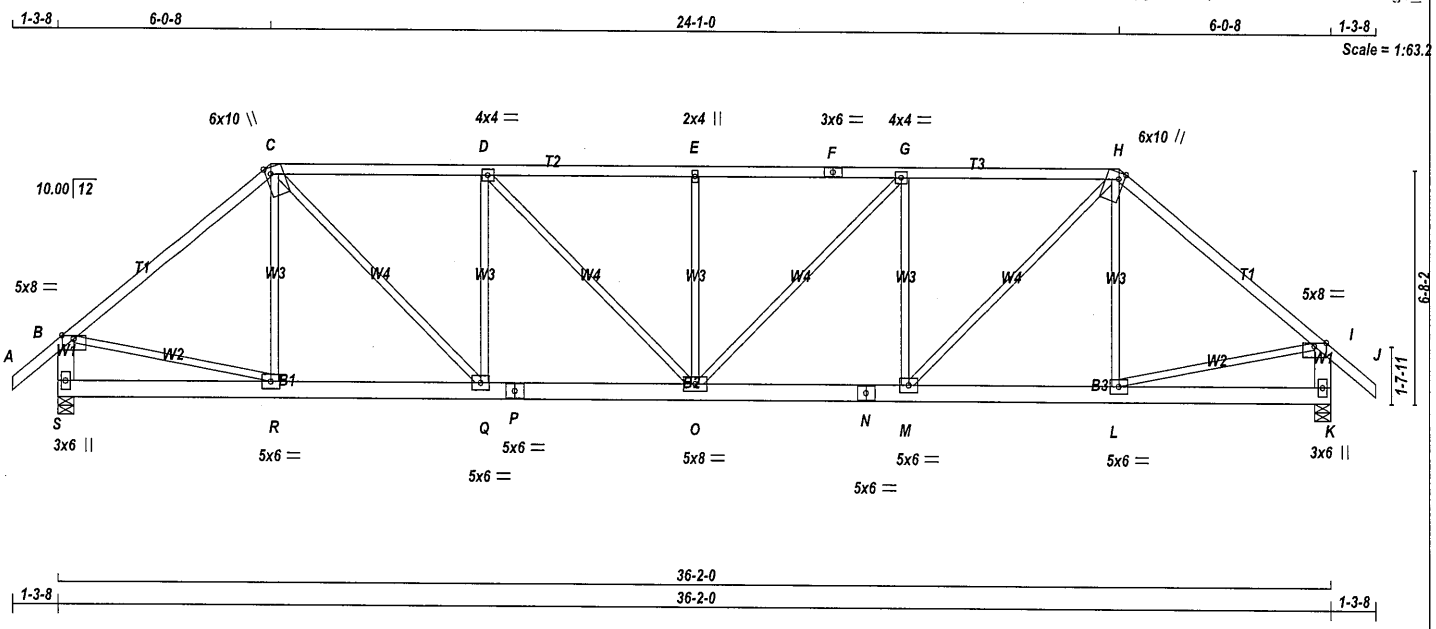

TOTAL WEIGHT = 179 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	6.0	10.0	Edge	1.75
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW+m	MT20	6.0	10.0	Edge	1.75
I	TMVW-p	MT20	5.0	8.0	Edge	
K	BMV1+p	MT20	3.0	6.0		
L, M, Q, R						
L	BMWW-t	MT20	5.0	6.0		
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BS-t	MT20	5.0	6.0		
S	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2205723

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	1987	0	1987	0	0	5-8	5-8
K	1987	0	1987	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1408	907 / 0	0 / 0	0 / 0	0 / 0	501 / 0	0 / 0
K	1408	907 / 0	0 / 0	0 / 0	0 / 0	501 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.55 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 38	-84.9	-84.9	0.12	(1)	10.00	R- C	-173 / 32	0.12 (1)
B- C	-1972 / 0	-84.9	-84.9	0.80	(1)	3.97	C- Q	0 / 1347	0.30 (1)
C- D	-2446 / 0	-84.9	-84.9	0.64	(1)	3.75	Q- D	-859 / 0	0.59 (1)
D- E	-2714 / 0	-84.9	-84.9	0.68	(1)	3.55	D- O	0 / 390	0.09 (1)
E- F	-2714 / 0	-84.9	-84.9	0.68	(1)	3.55	O- E	-466 / 0	0.32 (1)
F- G	-2714 / 0	-84.9	-84.9	0.68	(1)	3.55	O- G	0 / 390	0.09 (1)
G- H	-2446 / 0	-84.9	-84.9	0.64	(1)	3.75	M- G	-859 / 0	0.59 (1)
H- I	-1972 / 0	-84.9	-84.9	0.80	(1)	3.97	M- H	0 / 1347	0.30 (1)
I- J	0 / 38	-84.9	-84.9	0.12	(1)	10.00	L- H	-173 / 32	0.12 (1)
S- B	-1940 / 0	0.0	0.0	0.13	(1)	7.22	B- R	0 / 1542	0.35 (1)
K- I	-1940 / 0	0.0	0.0	0.13	(1)	7.22	L- I	0 / 1542	0.35 (1)
S- R	0 / 0	-18.5	-18.5	0.08	(4)	10.00			
R- Q	0 / 1509	-18.5	-18.5	0.22	(1)	10.00			
Q- P	0 / 2446	-18.5	-18.5	0.33	(1)	10.00			
P- O	0 / 2446	-18.5	-18.5	0.33	(1)	10.00			
O- N	0 / 2446	-18.5	-18.5	0.33	(1)	10.00			
N- M	0 / 2446	-18.5	-18.5	0.33	(1)	10.00			
M- L	0 / 1509	-18.5	-18.5	0.22	(1)	10.00			
L- K	0 / 0	-18.5	-18.5	0.08	(4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.21")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (1.21")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.80/1.00 (H-I:1) , BC=0.33/1.00 (M-O:1) , WB=0.59/1.00 (G-M:1) , SSI=0.24/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.71 (L) (INPUT = 0.90)
JSI METAL= 0.44 (N) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
422291	T8	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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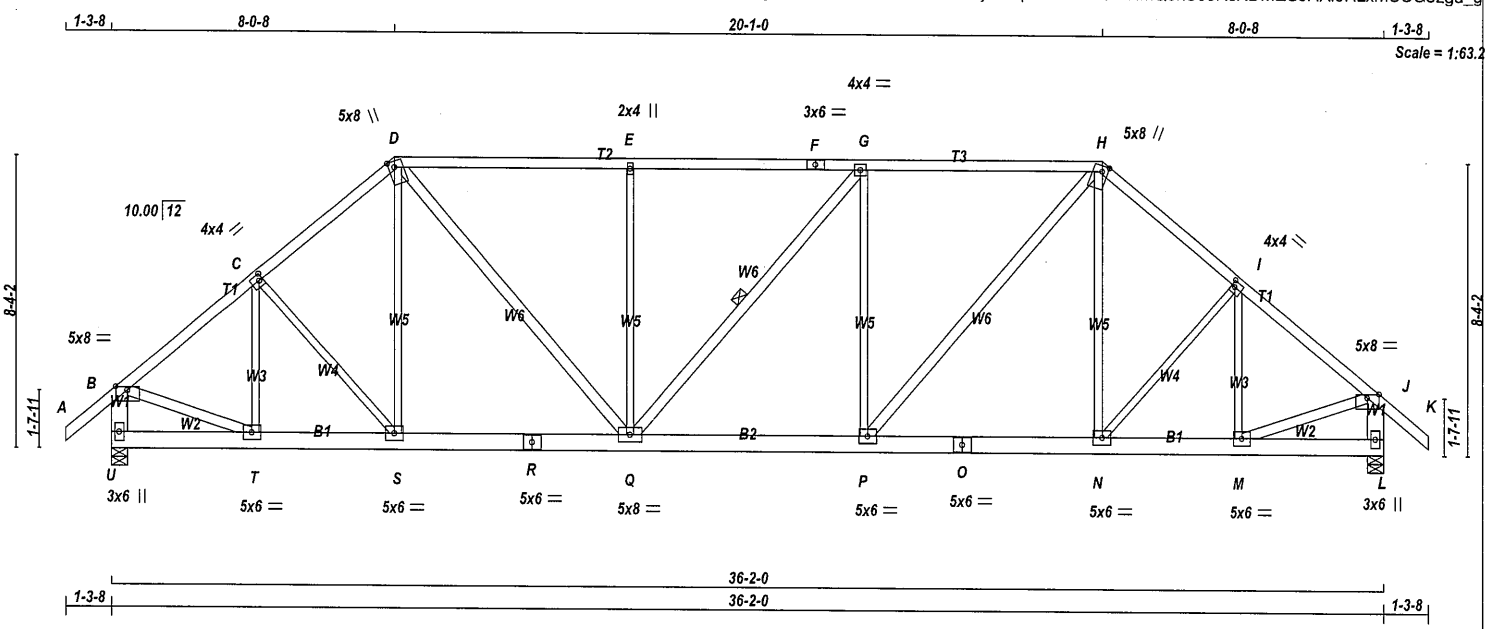

TOTAL WEIGHT = 204 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
U - R	2x6	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - Q	2x4	DRY	No.2	SPF
Q - G	2x4	DRY	No.2	SPF
P - H	2x4	DRY	No.2	SPF
B - T	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	8.0	2.00	2.00
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW+m	MT20	5.0	8.0	2.00	2.00
I	TMWW-t	MT20	4.0	4.0	2.00	1.25
J	TMVW-p	MT20	5.0	8.0	Edge	
L	BMV1+p	MT20	3.0	6.0		
M, N, P, S, T						
M	BMWW-t	MT20	5.0	6.0		
O	BS-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	5.0	8.0		
R	BS-t	MT20	5.0	6.0		
U	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1)

Structural component only
DWG# T-2205724

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	1987	0	1987	0	0	5-8	5-8
L	1987	0	1987	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1408	907 / 0	0 / 0	0 / 0	0 / 0	501 / 0	0 / 0
L	1408	907 / 0	0 / 0	0 / 0	0 / 0	501 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 38	-84.9	-84.9	0.12 (1)	10.00
B- C	-1922 / 0	-84.9	-84.9	0.30 (1)	4.58
C- D	-1955 / 0	-84.9	-84.9	0.30 (1)	4.54
D- E	-2100 / 0	-84.9	-84.9	0.71 (1)	3.91
E- F	-2100 / 0	-84.9	-84.9	0.71 (1)	3.89
F- G	-2100 / 0	-84.9	-84.9	0.71 (1)	3.89
G- H	-2104 / 0	-84.9	-84.9	0.72 (1)	3.89
H- I	-1954 / 0	-84.9	-84.9	0.30 (1)	4.54
I- J	-1922 / 0	-84.9	-84.9	0.30 (1)	4.58
J- K	0 / 38	-84.9	-84.9	0.12 (1)	10.00
U- B	-1943 / 0	0.0	0.0	0.13 (1)	7.22
L- J	-1943 / 0	0.0	0.0	0.13 (1)	7.22
U- T	0 / 0	-18.5	-18.5	0.04 (4)	10.00
T- S	0 / 1496	-18.5	-18.5	0.22 (1)	10.00
S- R	0 / 1480	-18.5	-18.5	0.21 (1)	10.00
R- Q	0 / 1480	-18.5	-18.5	0.21 (1)	10.00
Q- P	0 / 2104	-18.5	-18.5	0.29 (1)	10.00
P- O	0 / 1479	-18.5	-18.5	0.21 (1)	10.00
O- N	0 / 1479	-18.5	-18.5	0.21 (1)	10.00
N- M	0 / 1496	-18.5	-18.5	0.21 (1)	10.00
M- L	0 / 0	-18.5	-18.5	0.04 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- C	-394 / 0	0.14 (1)
C- S	-27 / 0	0.02 (1)
S- D	0 / 144	0.04 (4)
D- Q	0 / 961	0.15 (1)
Q- E	-614 / 0	0.80 (1)
Q- G	-6 / 0	0.00 (1)
P- G	-612 / 0	0.79 (1)
P- H	0 / 968	0.16 (1)
N- H	0 / 138	0.04 (4)
N- I	-29 / 0	0.02 (1)
M- I	-391 / 0	0.14 (1)
B- T	0 / 1560	0.25 (1)
M- J	0 / 1560	0.25 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.21")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (1.21")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.72/1.00 (G-H:1) , BC=0.29/1.00 (P-Q:1) , WB=0.80/1.00 (E-Q:1) , SSI=0.27/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.67 (M) (INPUT = 0.90)
JSI METAL= 0.34 (M) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
422291	T9	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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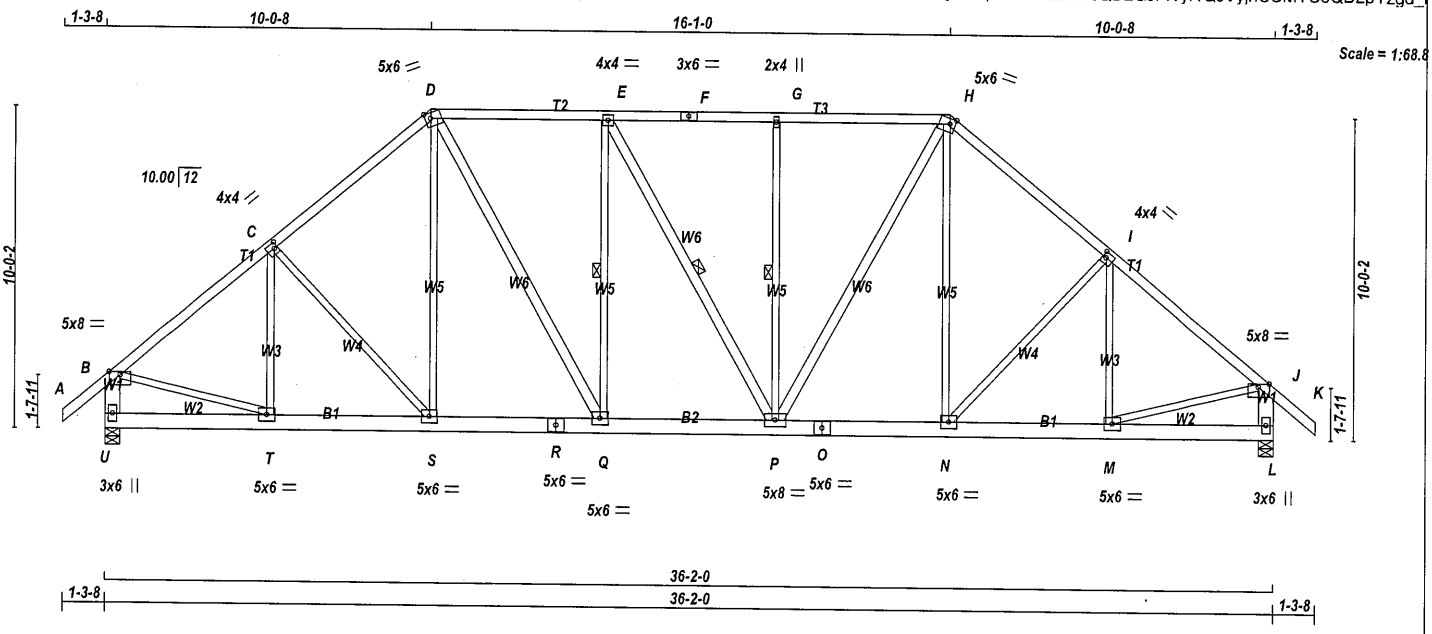

TOTAL WEIGHT = 215 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
U - R	2x6	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - Q	2x4	DRY	No.2	SPF
E - P	2x4	DRY	No.2	SPF
P - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW-m	MT20	5.0	6.0	2.00	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	6.0	2.00	2.00
I	TMWW-t	MT20	4.0	4.0	2.00	1.25
J	TMVW-p	MT20	5.0	8.0	Edge	
L	BMV1+p	MT20	3.0	6.0		
M, N, Q, S, T						
M	BMWW-t	MT20	5.0	6.0		
O	BS-t	MT20	5.0	6.0		
P	BMWWW-t	MT20	5.0	8.0		
R	BS-t	MT20	5.0	6.0		
U	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2205725

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	1987	0	1987	0	0	5-8	5-8
L	1987	0	1987	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1408	907 / 0	0 / 0	0 / 0	0 / 0	501 / 0	0 / 0
L	1408	907 / 0	0 / 0	0 / 0	0 / 0	501 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.49 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-Q, E-P, G-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 38	-84.9	-84.9	0.12 (1)		10.00
B- C	-1977 / 0	-84.9	-84.9	0.35 (1)		4.49
C- D	-1876 / 0	-84.9	-84.9	0.34 (1)		4.60
D- E	-1746 / 0	-84.9	-84.9	0.32 (1)		4.73
E- F	-1742 / 0	-84.9	-84.9	0.30 (1)		4.76
F- G	-1742 / 0	-84.9	-84.9	0.30 (1)		4.76
G- H	-1742 / 0	-84.9	-84.9	0.32 (1)		4.74
H- I	-1877 / 0	-84.9	-84.9	0.34 (1)		4.60
I- J	-1976 / 0	-84.9	-84.9	0.35 (1)		4.49
J- K	0 / 38	-84.9	-84.9	0.12 (1)		10.00
U- B	-1939 / 0	0.0	0.0	0.13 (1)		7.23
L- J	-1939 / 0	0.0	0.0	0.13 (1)		7.23
U- T	0 / 0	-18.5	-18.5	0.05 (4)		10.00
T- S	0 / 1543	-18.5	-18.5	0.21 (1)		10.00
S- R	0 / 1415	-18.5	-18.5	0.20 (1)		10.00
R- Q	0 / 1415	-18.5	-18.5	0.20 (1)		10.00
Q- P	0 / 1746	-18.5	-18.5	0.23 (1)		10.00
P- O	0 / 1416	-18.5	-18.5	0.20 (1)		10.00
O- N	0 / 1416	-18.5	-18.5	0.20 (1)		10.00
N- M	0 / 1543	-18.5	-18.5	0.21 (1)		10.00
M- L	0 / 0	-18.5	-18.5	0.05 (4)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- C	-282 / 0	0.14 (1)
C- S	-194 / 0	0.21 (1)
S- D	0 / 246	0.06 (1)
D- Q	0 / 676	0.11 (1)
Q- E	-485 / 0	0.32 (1)
E- P	-8 / 0	0.01 (1)
P- G	-488 / 0	0.32 (1)
P- H	0 / 666	0.11 (1)
N- H	0 / 254	0.06 (1)
N- I	-192 / 0	0.21 (1)
M- I	-284 / 0	0.14 (1)
B- T	0 / 1586	0.36 (1)
M- J	0 / 1586	0.36 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.21")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (1.21")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.35/1.00 (B-C:1) , BC=0.23/1.00 (P-Q:1) , WB=0.36/1.00 (B-T:1) , SSI=0.21/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (T) (INPUT = 0.90)
JSI METAL= 0.35 (H) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
422291	T10S	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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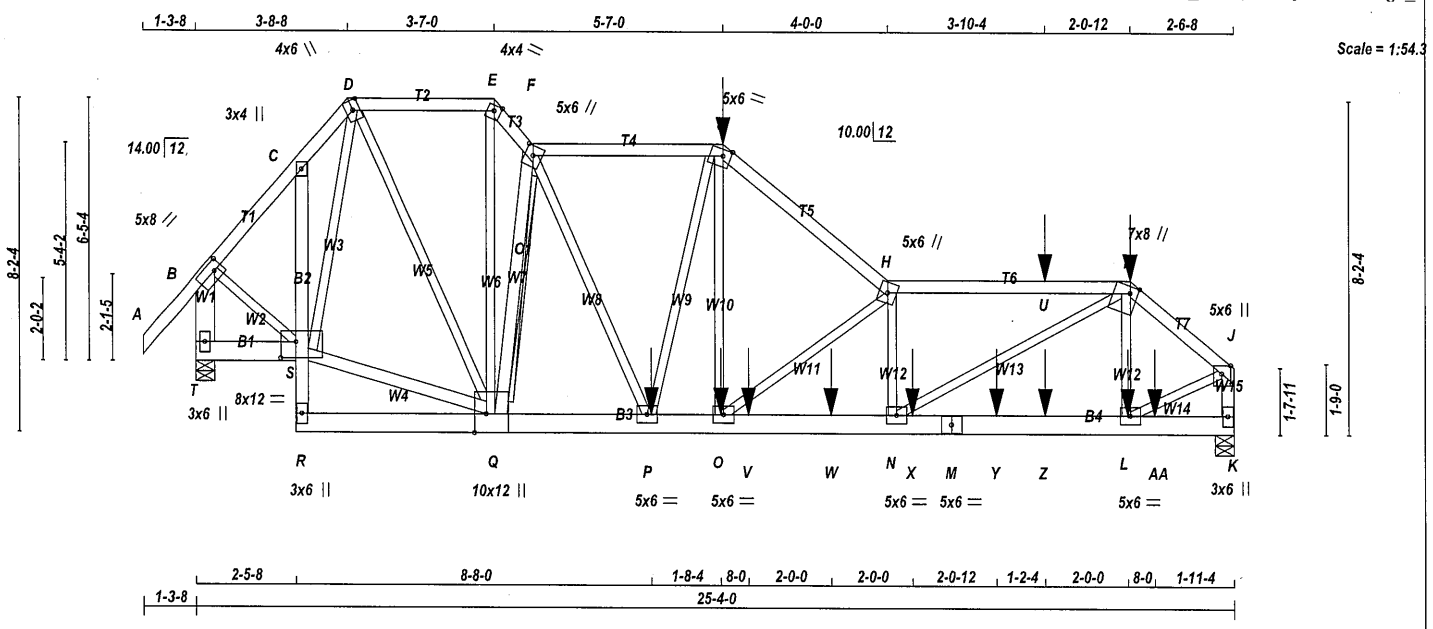

TOTAL WEIGHT = 2 X 159 = 317 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - I	2x4	DRY	No.2	SPF
I - J	2x4	DRY	No.2	SPF
T - B	2x6	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
T - S	2x6	DRY	No.2	SPF
R - C	2x4	DRY	No.2	SPF
R - M	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
Q - F	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	TOP
D- E	1	12	TOP
E- F	1	12	TOP
F- G	1	12	SIDE(61.0)
G- H	1	12	SIDE(61.0)
H- I	1	12	SIDE(61.0)
I- J	1	12	SIDE(61.0)
K- J	1	12	TOP
T- B	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
T- S	2	12	TOP
R- M	2	12	SIDE(183.1)
M- K	2	12	SIDE(183.1)
C- R	1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
O- G	1	6	SIDE(54.4)
2x3	1	6	
2x4	1	6	

Structural component only
DWG# T-2205726

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	2744	0	2744	0	0	5-8	5-8
K	2729	0	2729	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1941	1274 / 0	0 / 0	0 / 0	0 / 0	667 / 0	0 / 0
K	1933	1250 / 0	0 / 0	0 / 0	0 / 0	683 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.78 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT F-Q

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (7)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 45	-84.9	-84.9	0.07 (1)	10.00
B- C	-2128 / 0	-84.9	-84.9	0.06 (1)	6.04
C- D	-2145 / 0	-84.9	-84.9	0.03 (1)	6.04
D- E	-2106 / 0	-84.9	-84.9	0.12 (1)	5.98
E- F	-3206 / 0	-84.9	-84.9	0.04 (1)	5.18
F- G	-3364 / 0	-84.9	-84.9	0.23 (1)	4.87
G- H	-4356 / 0	-84.9	-84.9	0.22 (1)	4.41
H- U	-5299 / 0	-84.9	-84.9	0.50 (1)	3.78
U- I	-5299 / 0	-84.9	-84.9	0.50 (1)	3.78
I- J	-2727 / 0	-84.9	-84.9	0.08 (1)	5.47
T- B	-2720 / 0	0.0	0.0	0.11 (1)	7.81
K- J	-2827 / 0	0.0	0.0	0.16 (1)	6.82
T- S	0 / 0	-18.5	-18.5	0.01 (4)	10.00
R- S	0 / 41	0.0	0.0	0.01 (5)	10.00
S- C	-191 / 0	0.0	0.0	0.01 (5)	7.81
R- Q	0 / 3	-18.5	-18.5	0.02 (4)	10.00
Q- P	0 / 2719	-18.5	-18.5	0.19 (1)	10.00
P- O	0 / 3374	-18.5	-18.5	0.27 (1)	10.00
O- V	0 / 5348	-18.5	-18.5	0.41 (1)	10.00
V- W	0 / 5348	-18.5	-18.5	0.41 (1)	10.00
W- N	0 / 5348	-18.5	-18.5	0.41 (1)	10.00
N- X	0 / 2069	-18.5	-18.5	0.28 (1)	10.00
X- M	0 / 2069	-18.5	-18.5	0.28 (1)	10.00
M- Y	0 / 2069	-18.5	-18.5	0.28 (1)	10.00
Y- Z	0 / 2069	-18.5	-18.5	0.28 (1)	10.00
Z- L	0 / 2069	-18.5	-18.5	0.28 (1)	10.00
L-AA	0 / 0	-18.5	-18.5	0.12 (1)	10.00
AA- K	0 / 0	-18.5	-18.5	0.12 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- E	0 / 2210	0.27 (1)
O- G	0 / 1806	0.22 (1)
O- H	-2588 / 0	0.62 (1)
N- H	-1578 / 0	0.18 (1)
N- I	0 / 3731	0.46 (1)
L- I	-543 / 0	0.06 (1)
L- J	0 / 2297	0.28 (1)
B- S	0 / 1648	0.20 (1)
Q- F	-4250 / 0	0.79 (1)
S- Q	0 / 1500	0.13 (1)
S- D	-110 / 23	0.04 (1)
D- Q	0 / 1715	0.21 (1)
P- G	-105 / 0	0.05 (5)
F- P	0 / 1655	0.20 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.84")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.84")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.50/1.00 (H-I:1) , BC=0.41/1.00 (N-O:1) , WB=0.79/1.00 (F-Q:1) , SSI=0.11/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (H) (INPUT = 0.90)
JSI METAL= 0.54 (H) (INPUT = 1.00)

CONTINUED ON PAGE 2

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.50	2.50
C	TMV+p	MT20	3.0	4.0		
D	TTWW+m	MT20	4.0	6.0	Edge	
E	TTW-m	MT20	4.0	4.0	Edge	
F	TTWW+m	MT20	5.0	6.0	2.75	2.50
G	TTWW-m	MT20	5.0	6.0	2.00	2.25
H	TTWW+m	MT20	5.0	6.0		
I	TTWW+m	MT20	7.0	8.0	Edge	2.25
J	TMVW+p	MT20	5.0	6.0	Edge	
K	BMV1+p	MT20	3.0	6.0		
L, N, O, P						
L	BMWW-t	MT20	5.0	6.0		
M	BS-t	MT20	5.0	6.0		
Q	BMWWWW*+m	MT20	10.0	12.0	Edge	3.50
R	BMV+p	MT20	3.0	6.0		
S	BVMWWW-l	MT20	8.0	12.0	4.75	4.50
T	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	12-10-8	-468	-468	---	BACK	VERT	TOTAL	---	C1
I	22-9-8	-40	-40	99	BACK	VERT	TOTAL	---	C1
L	22-8-12	-4	-4	---	BACK	VERT	TOTAL	---	C1
O	12-9-12	-101	-101	---	BACK	VERT	TOTAL	---	C1
P	11-1-8	-982	-982	---	BACK	VERT	TOTAL	---	C1
U	20-8-12	-16	-16	---	BACK	VERT	TOTAL	---	C1
V	13-5-12	-34	-34	---	BACK	VERT	TOTAL	---	C1
W	15-5-12	-34	-34	---	BACK	VERT	TOTAL	---	C1
X	17-5-12	-35	-35	---	BACK	VERT	TOTAL	---	C1
Y	19-6-8	-210	-210	---	BACK	VERT	TOTAL	---	C1
Z	20-8-12	-4	-4	---	BACK	VERT	TOTAL	---	C1
AA	23-4-12	-6	-6	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

Structural component only
DWG# T-2205726

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
422291	T11S	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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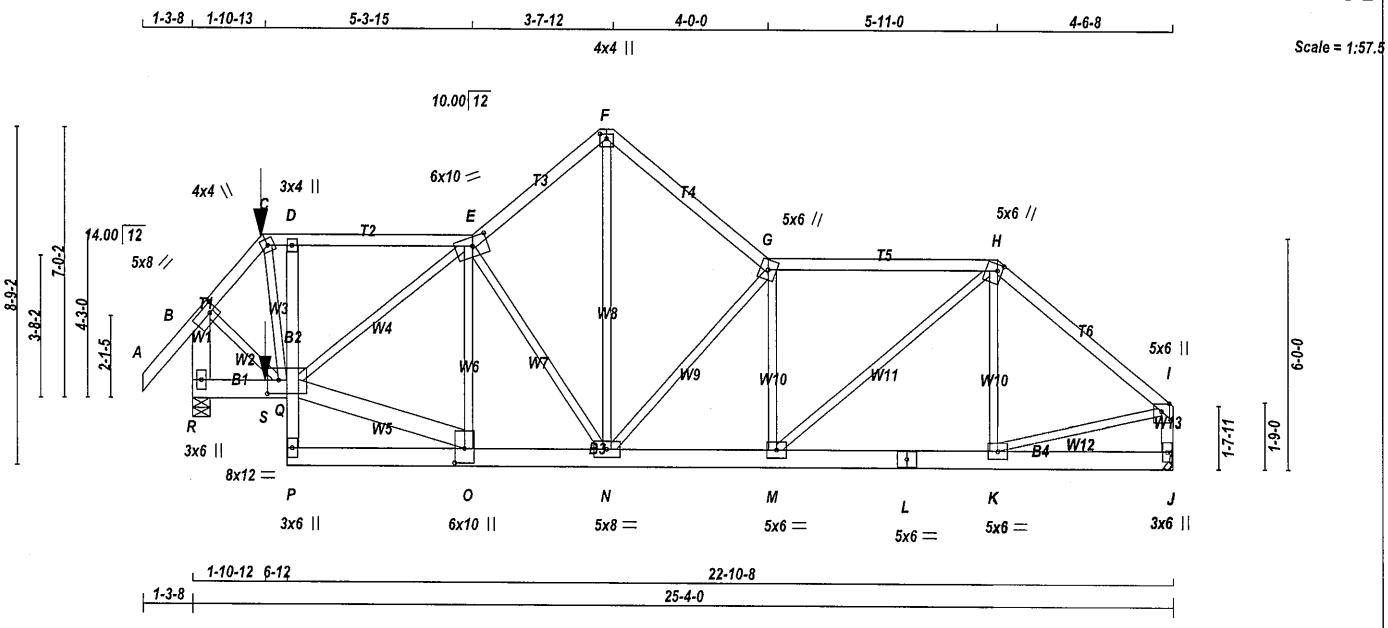

TOTAL WEIGHT = 149 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - I	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
R - Q	2x6	DRY	No.2	SPF
P - D	2x4	DRY	No.2	SPF
P - L	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
Q - O	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.50	2.50
C	TTW+m	MT20	4.0	4.0		
D	TMV+p	MT20	3.0	4.0		
E	TTWWW-m	MT20	6.0	10.0	2.75	4.75
F	TTW+p	MT20	4.0	4.0	1.50	2.00
G	TTWW+m	MT20	5.0	6.0		
H	TTWW+m	MT20	5.0	6.0	2.00	1.50
I	TMVW+p	MT20	5.0	6.0	Edge	
J	BMV1+p	MT20	3.0	6.0		
K	BMWW-t	MT20	5.0	6.0		
L	BS-t	MT20	5.0	6.0		
M	BMWW-t	MT20	5.0	6.0		
N	BMWWW-t	MT20	5.0	8.0		
O	BMWW+t	MT20	6.0	10.0	4.50	3.00
P	BMV+p	MT20	3.0	6.0		
Q	BVMWWWW+t	MT20	8.0	12.0	4.25	3.50
R	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES (1)
1)

Structural component only
DWG# T-2205727

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
R	1439	0	1439	0	0	5-8	5-8
J	1307	0	1307	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1020	657 / 0	0 / 0	0 / 0	0 / 0	363 / 0	0 / 0
J	928	588 / 0	0 / 0	0 / 0	0 / 0	340 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.28 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (7)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 45	-84.9	-84.9	0.13 (1)	10.00	Q- O	0 / 1275	0.17 (1)
B- C	-1032 / 0	-84.9	-84.9	0.14 (1)	5.99	Q- E	-525 / 0	0.31 (5)
C- D	-815 / 0	-84.9	-84.9	0.24 (1)	6.25	O- E	-376 / 0	0.20 (1)
D- E	-812 / 0	-84.9	-84.9	0.25 (1)	6.25	E- N	-477 / 0	0.37 (1)
E- F	-1232 / 0	-84.9	-84.9	0.22 (1)	5.49	N- G	-1004 / 0	0.77 (1)
F- G	-1233 / 0	-84.9	-84.9	0.32 (1)	5.36	M- G	-441 / 0	0.19 (1)
G- H	-1581 / 0	-84.9	-84.9	0.67 (1)	4.28	M- H	0 / 794	0.20 (1)
H- I	-1283 / 0	-84.9	-84.9	0.39 (1)	5.19	K- H	-140 / 29	0.06 (1)
R- B	-1415 / 0	0.0	0.0	0.12 (1)	7.81	B- Q	0 / 770	0.19 (1)
J- I	-1275 / 0	0.0	0.0	0.15 (1)	7.10	K- I	0 / 1016	0.25 (1)
						N- F	0 / 1247	0.31 (1)
R- S	0 / 0	-18.5	-18.5	0.02 (4)	10.00	C- Q	0 / 965	0.24 (5)
S- Q	0 / 0	-18.5	-18.5	0.02 (4)	10.00			
P- Q	0 / 40	0.0	0.0	0.02 (1)	10.00			
Q- D	-480 / 0	0.0	0.0	0.04 (1)	7.81			
P- O	0 / 6	-18.5	-18.5	0.04 (4)	10.00			
O- N	0 / 1201	-18.5	-18.5	0.17 (1)	10.00			
N- M	0 / 1590	-18.5	-18.5	0.24 (1)	10.00			
M- L	0 / 980	-18.5	-18.5	0.17 (1)	10.00			
L- K	0 / 980	-18.5	-18.5	0.17 (1)	10.00			
K- J	0 / 0	-18.5	-18.5	0.06 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	1-10-13	-6	-6	72	FRONT	VERT	TOTAL	---	C1
S	1-10-12	-1	-1	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.84")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.84")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.67/1.00 (G-H:1) , BC=0.24/1.00 (M-N:1) , WB=0.77/1.00 (G-N:1) , SSI=0.22/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (F) (INPUT = 0.90)
JSI METAL= 0.47 (I) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
422291	T12S	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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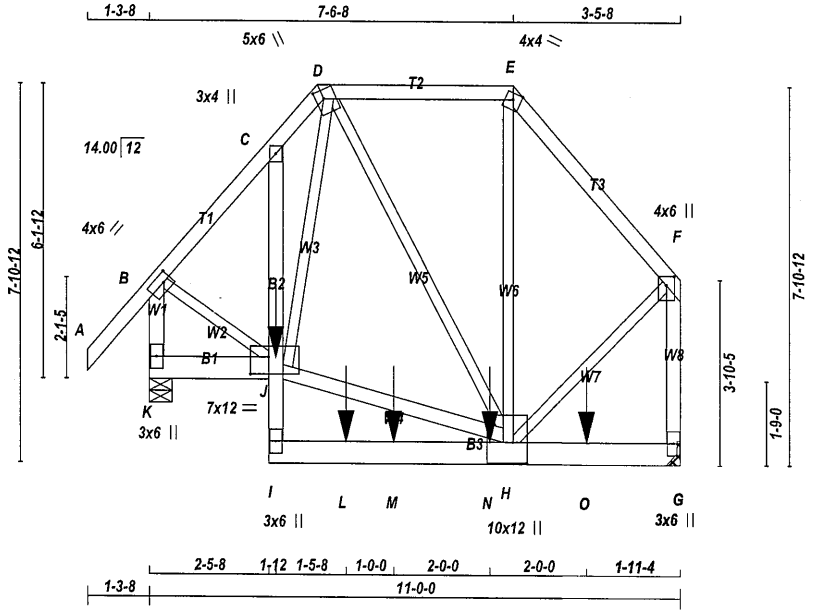

TOTAL WEIGHT = 80 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - J	2x6	DRY	No.2	SPF
I - C	2x4	DRY	No.2	SPF
I - G	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	2.00	2.00
C	TMV+p	MT20	3.0	4.0		
D	TTWW+m	MT20	5.0	6.0	2.25	2.00
E	TTW-m	MT20	4.0	4.0	Edge	
F	TMVW+p	MT20	4.0	6.0	2.25	2.00
G	BMV1+p	MT20	3.0	6.0		
H	BMWWWW*+m	MT20	10.0	12.0	5.25	3.50
I	BMV+p	MT20	3.0	6.0		
J	BVMWWW-I	MT20	7.0	12.0	4.25	4.50
K	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2205728

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
K	1113	0	1113	0	0	5-8	5-8
G	982	0	982	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	785	529 / 0	0 / 0	0 / 0	0 / 0	256 / 0	0 / 0
G	694	458 / 0	0 / 0	0 / 0	0 / 0	236 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 45	-84.9	-84.9	0.13 (1)	10.00	J- H	0 / 426	0.08 (1)
B- C	-738 / 0	-84.9	-84.9	0.09 (1)	6.25	J- D	0 / 434	0.11 (1)
C- D	-749 / 0	-84.9	-84.9	0.04 (1)	6.25	H- E	0 / 121	0.03 (4)
D- E	-376 / 0	-84.9	-84.9	0.26 (1)	6.25	H- F	0 / 519	0.13 (1)
E- F	-579 / 0	-84.9	-84.9	0.20 (1)	6.25	B- J	0 / 574	0.14 (1)
K- B	-1089 / 0	0.0	0.0	0.13 (1)	7.54	D- H	-70 / 0	0.09 (1)
G- F	-947 / 0	0.0	0.0	0.23 (1)	7.81			
K- J	0 / 0	-18.5	-18.5	0.02 (4)	10.00			
I- J	0 / 197	0.0	0.0	0.06 (1)	10.00			
J- C	-170 / 0	0.0	0.0	0.03 (1)	7.81			
I- L	0 / 8	-18.5	-18.5	0.20 (1)	10.00			
L- M	0 / 8	-18.5	-18.5	0.20 (1)	10.00			
M- N	0 / 8	-18.5	-18.5	0.20 (1)	10.00			
N- H	0 / 8	-18.5	-18.5	0.20 (1)	10.00			
H- O	0 / 0	-18.5	-18.5	0.18 (1)	10.00			
O- G	0 / 0	-18.5	-18.5	0.18 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
J	2-7-4	-142	-142	---	BACK	VERT	TOTAL	---	C1
L	4-0-12	-112	-112	---	BACK	VERT	TOTAL	---	C1
M	5-0-12	-112	-112	---	BACK	VERT	TOTAL	---	C1
N	7-0-12	-112	-112	---	BACK	VERT	TOTAL	---	C1
O	9-0-12	-112	-112	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.37")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.37")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.26/1.00 (D-E:1) , BC=0.20/1.00 (H-I:1) , WB=0.14/1.00 (B-J:1) , SSI=0.25/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.57 (B) (INPUT = 0.90)
JSI METAL= 0.23 (F) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
422291	T13S	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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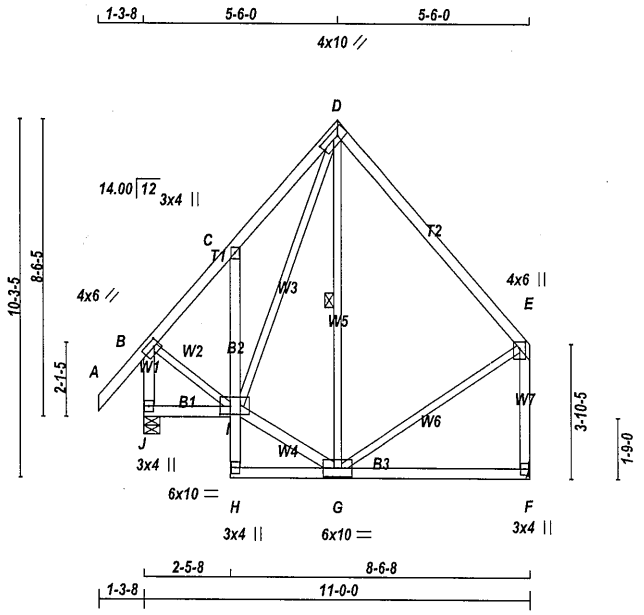

TOTAL WEIGHT = 2 X 73 = 146 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
H - C	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	2.00	2.00
C	TMV+p	MT20	3.0	4.0		
D	TTWW-I	MT20	4.0	10.0	2.00	6.75
E	TMVW+p	MT20	4.0	6.0	2.25	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	6.0	10.0		
H	BMV+p	MT20	3.0	4.0		
I	BVMWWW-I	MT20	6.0	10.0	2.75	3.50
J	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	687	0	687	0	0	5-8	5-8
F	568	0	568	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	486	321 / 0	0 / 0	0 / 0	0 / 0	164 / 0	0 / 0
F	404	256 / 0	0 / 0	0 / 0	0 / 0	147 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 45	-84.9	-84.9	0.12 (1)	10.00	I- G	0 / 183	0.03 (1)
B- C	-370 / 0	-84.9	-84.9	0.08 (1)	6.25	I- D	0 / 246	0.06 (1)
C- D	-396 / 0	-84.9	-84.9	0.08 (1)	6.25	G- D	-92 / 40	0.06 (1)
D- E	-251 / 0	-84.9	-84.9	0.33 (1)	6.25	G- E	0 / 193	0.04 (1)
J- B	-663 / 0	0.0	0.0	0.08 (1)	7.81	B- I	0 / 306	0.07 (1)
F- E	-527 / 0	0.0	0.0	0.13 (1)	7.81			
J- I	0 / 0	-18.5	-18.5	0.04 (4)	10.00			
H- I	0 / 10	0.0	0.0	0.02 (1)	10.00			
I- C	-276 / 0	0.0	0.0	0.03 (1)	7.81			
H- G	0 / 7	-18.5	-18.5	0.12 (4)	10.00			
G- F	0 / 0	-18.5	-18.5	0.12 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	23.3	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL	LOAD	=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.37")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.37")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.33/1.00 (D-E:1) , BC=0.12/1.00 (G-H:4) , WB=0.07/1.00 (B-I:1) , SSI=0.12/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.34 (B) (INPUT = 0.90)
JSI METAL= 0.14 (E) (INPUT = 1.00)

Structural component only
DWG# T-2205729

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
422291	T14	1	1	ROYAL PINE HOMES TRUSS DESC.	

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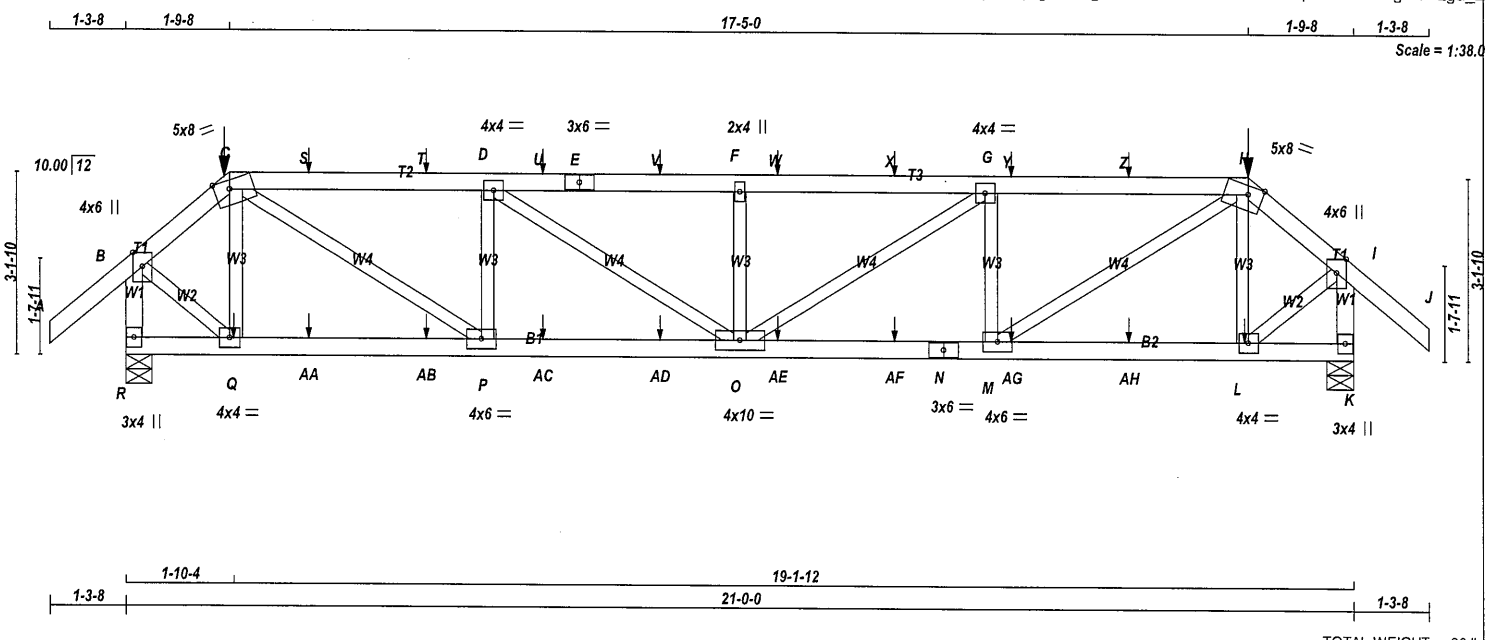

TOTAL WEIGHT = 86 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
R - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW-m	MT20	5.0	8.0	Edge	3.00
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW-m	MT20	5.0	8.0	Edge	3.00
I	TMVW+p	MT20	4.0	6.0	Edge	
K	BMV1+p	MT20	3.0	4.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMWW-t	MT20	4.0	6.0		
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	4.0	10.0		
P	BMWW-t	MT20	4.0	6.0		
Q	BMWW-t	MT20	4.0	4.0		
R	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only

DWG# T-2205730

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	1263	0	1263	0	0	5-8	5-8
K	1263	0	1263	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	893	586 / 0	0 / 0	0 / 0	0 / 0	306 / 0	0 / 0
K	893	586 / 0	0 / 0	0 / 0	0 / 0	306 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.38 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 38	-84.9	-84.9	0.13 (1)	10.00
B- C	-902 / 0	-84.9	-84.9	0.13 (1)	6.25
C- S	-1737 / 0	-84.9	-84.9	0.30 (1)	4.71
S- T	-1737 / 0	-84.9	-84.9	0.30 (1)	4.71
T- D	-1737 / 0	-84.9	-84.9	0.30 (1)	4.71
D- U	-2052 / 0	-84.9	-84.9	0.32 (1)	4.38
U- E	-2052 / 0	-84.9	-84.9	0.32 (1)	4.38
E- V	-2052 / 0	-84.9	-84.9	0.32 (1)	4.38
V- F	-2052 / 0	-84.9	-84.9	0.32 (1)	4.38
F- W	-2052 / 0	-84.9	-84.9	0.32 (1)	4.38
W- X	-2052 / 0	-84.9	-84.9	0.32 (1)	4.38
X- G	-2052 / 0	-84.9	-84.9	0.32 (1)	4.38
G- Y	-1737 / 0	-84.9	-84.9	0.30 (1)	4.71
Y- Z	-1737 / 0	-84.9	-84.9	0.30 (1)	4.71
Z- H	-1737 / 0	-84.9	-84.9	0.30 (1)	4.71
H- I	-902 / 0	-84.9	-84.9	0.13 (1)	6.25
I- J	0 / 38	-84.9	-84.9	0.13 (1)	10.00
R- B	-1261 / 0	0.0	0.0	0.15 (1)	7.13
K- I	-1261 / 0	0.0	0.0	0.15 (1)	7.13
R- Q	0 / 0	-18.5	-18.5	0.06 (4)	10.00
Q-AA	0 / 653	-18.5	-18.5	0.16 (1)	10.00
AA-AB	0 / 653	-18.5	-18.5	0.16 (1)	10.00
AB- P	0 / 653	-18.5	-18.5	0.16 (1)	10.00
P-AC	0 / 1737	-18.5	-18.5	0.35 (1)	10.00
AC-AD	0 / 1737	-18.5	-18.5	0.35 (1)	10.00
AD- O	0 / 1737	-18.5	-18.5	0.35 (1)	10.00
O-AE	0 / 1737	-18.5	-18.5	0.35 (1)	10.00
AE-AF	0 / 1737	-18.5	-18.5	0.35 (1)	10.00
AF- N	0 / 1737	-18.5	-18.5	0.35 (1)	10.00
N- M	0 / 1737	-18.5	-18.5	0.35 (1)	10.00
M-AG	0 / 653	-18.5	-18.5	0.16 (1)	10.00
AG-AH	0 / 653	-18.5	-18.5	0.16 (1)	10.00
AH- L	0 / 653	-18.5	-18.5	0.16 (1)	10.00
L- K	0 / 0	-18.5	-18.5	0.06 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-381 / 0	0.08 (1)
C- P	0 / 1287	0.32 (1)
P- D	-619 / 0	0.12 (1)
D- O	0 / 377	0.09 (1)
O- F	-336 / 0	0.07 (1)
O- G	0 / 377	0.09 (1)
M- G	-619 / 0	0.12 (1)
M- H	0 / 1287	0.32 (1)
L- H	-381 / 0	0.08 (1)
B- Q	0 / 809	0.20 (1)
L- I	0 / 809	0.20 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	1-9-8	-41	-41	72	BACK	VERT	TOTAL	---	C1
H	19-2-8	-41	-41	72	BACK	VERT	TOTAL	---	C1
L	19-1-12	1	1	---	BACK	VERT	TOTAL	---	C1
Q	1-10-4	1	1	---	BACK	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.32/1.00 (D-F:1) , BC=0.35/1.00 (M-O:1) , WB=0.32/1.00 (C-P:1) , SSI=0.19/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (H) (INPUT = 0.90)
JSI METAL= 0.52 (N) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
422291	T14	1	1	ROYAL PINE HOMES TRUSS DESC.	

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
S	3-1-12	1	1	72	BACK	VERT	TOTAL	---	C1
T	5-1-12	1	1	72	BACK	VERT	TOTAL	---	C1
U	7-1-12	1	1	72	BACK	VERT	TOTAL	---	C1
V	9-1-12	1	1	72	BACK	VERT	TOTAL	---	C1
W	11-1-12	1	1	72	BACK	VERT	TOTAL	---	C1
X	13-1-12	1	1	72	BACK	VERT	TOTAL	---	C1
Y	15-1-12	1	1	72	BACK	VERT	TOTAL	---	C1
Z	17-1-12	1	1	72	BACK	VERT	TOTAL	---	C1
AA	3-1-12	1	1	---	BACK	VERT	TOTAL	---	C1
AB	5-1-12	1	1	---	BACK	VERT	TOTAL	---	C1
AC	7-1-12	1	1	---	BACK	VERT	TOTAL	---	C1
AD	9-1-12	1	1	---	BACK	VERT	TOTAL	---	C1
AE	11-1-12	1	1	---	BACK	VERT	TOTAL	---	C1
AF	13-1-12	1	1	---	BACK	VERT	TOTAL	---	C1
AG	15-1-12	1	1	---	BACK	VERT	TOTAL	---	C1
AH	17-1-12	1	1	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

Structural component only
DWG# T-2205730

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
422443	T15	1	1	ROYAL PINE HOMES TRUSS DESC.	

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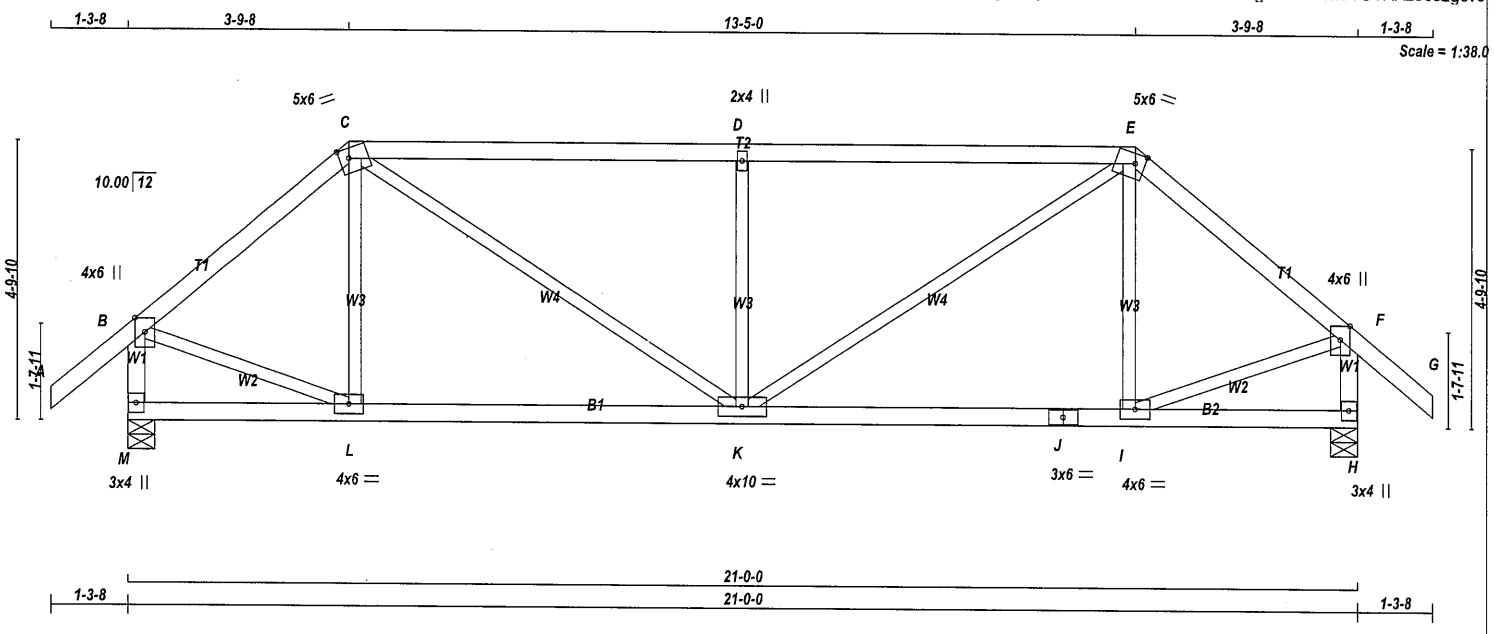

TOTAL WEIGHT = 86 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW-m	MT20	5.0	6.0	2.00	2.00
D	TMW+w	MT20	2.0	4.0		
E	TTWW-m	MT20	5.0	6.0	2.00	2.00
F	TMVW+p	MT20	4.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	10.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1203	0	1203	0	0	5-8	5-8
H	1203	0	1203	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	852	554 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
H	852	554 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.39 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 38	-84.9	-84.9	0.12 (1)	10.00		L- C	-147 / 25	0.05 (1)
B- C	-1003 / 0	-84.9	-84.9	0.23 (1)	5.95		C- K	0 / 739	0.17 (1)
C- D	-1379 / 0	-84.9	-84.9	0.75 (1)	4.39		K- D	-698 / 0	0.24 (1)
D- E	-1379 / 0	-84.9	-84.9	0.75 (1)	4.39		K- E	0 / 739	0.17 (1)
E- F	-1003 / 0	-84.9	-84.9	0.23 (1)	5.95		I- E	-147 / 25	0.05 (1)
F- G	0 / 38	-84.9	-84.9	0.12 (1)	10.00		B- L	0 / 811	0.18 (1)
M- B	-1181 / 0	0.0	0.0	0.13 (1)	7.34		I- F	0 / 811	0.18 (1)
H- F	-1181 / 0	0.0	0.0	0.13 (1)	7.34				
M- L	0 / 0	-18.5	-18.5	0.12 (4)	10.00				
L- K	0 / 765	-18.5	-18.5	0.24 (4)	10.00				
K- J	0 / 765	-18.5	-18.5	0.24 (4)	10.00				
J- I	0 / 765	-18.5	-18.5	0.24 (4)	10.00				
I- H	0 / 0	-18.5	-18.5	0.12 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.75/1.00 (C-D:1) , BC=0.24/1.00 (I-K:4) , WB=0.24/1.00 (D-K:1) , SSI=0.28/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (C) (INPUT = 0.90)
JSI METAL= 0.49 (B) (INPUT = 1.00)

Structural component only
DWG# T-2205745

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
422291	T15S	1	1	ROYAL PINE HOMES TRUSS DESC.	

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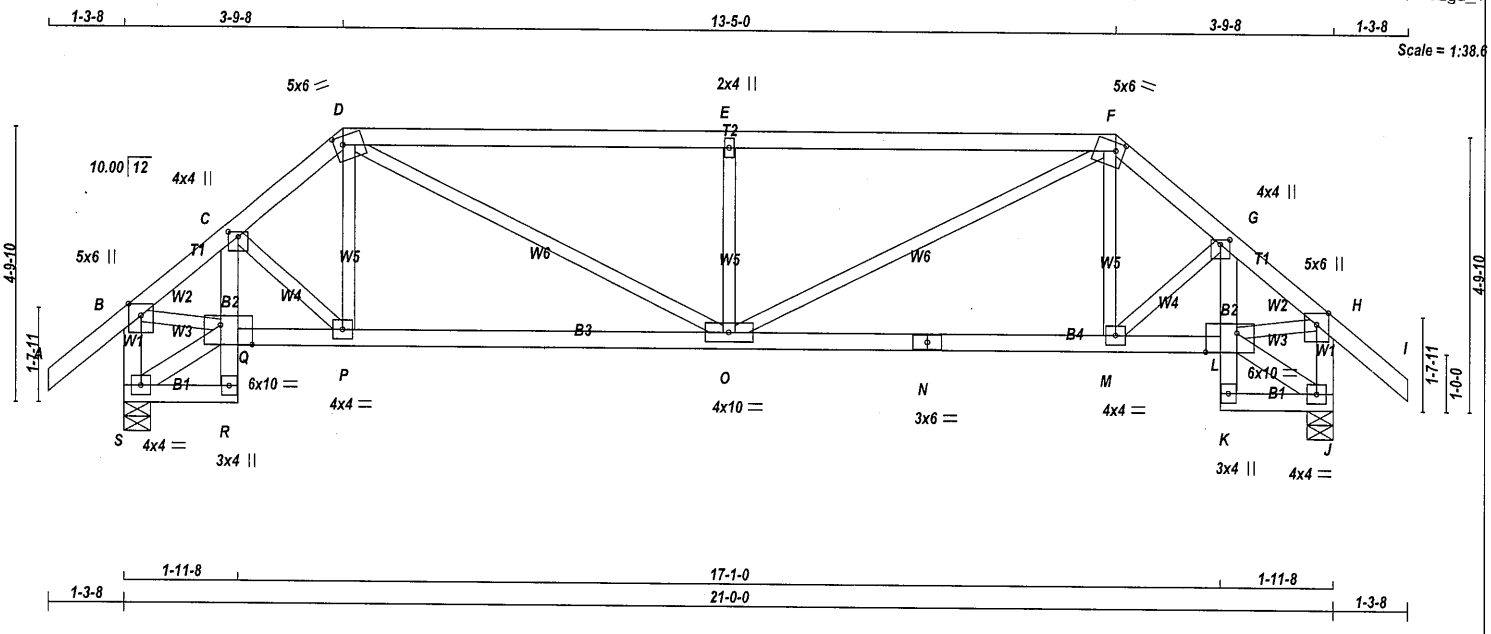

TOTAL WEIGHT = 93 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
S - R	2x4	DRY	No.2	SPF
R - C	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
K - G	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	TTWW-m	MT20	5.0	6.0	1.75	1.75
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	1.75	1.75
G	TMVW+p	MT20	4.0	4.0	1.00	2.00
H	TMVW+p	MT20	5.0	6.0	Edge	
J	BMVW1-t	MT20	4.0	4.0		
K	BMV+p	MT20	3.0	4.0		
L	BVMWW-l	MT20	6.0	10.0	4.00	6.50
M	BMWW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	4.0	10.0		
P	BMWW-t	MT20	4.0	4.0		
Q	BVMWW-l	MT20	6.0	10.0	4.00	6.50
R	BMV+p	MT20	3.0	4.0		
S	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1)

Structural component only
DWG# T-2205731

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	1203	0	1203	0	0	5-8	5-8
J	1203	0	1203	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	852	554 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
J	852	554 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.97 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 38	-84.9	-84.9	0.12 (1)	10.00
B- C	-1319 / 0	-84.9	-84.9	0.13 (1)	5.49
C- D	-1255 / 0	-84.9	-84.9	0.08 (1)	5.66
D- E	-1745 / 0	-84.9	-84.9	0.79 (1)	3.97
E- F	-1745 / 0	-84.9	-84.9	0.79 (1)	3.97
F- G	-1255 / 0	-84.9	-84.9	0.08 (1)	5.66
G- H	-1319 / 0	-84.9	-84.9	0.13 (1)	5.49
H- I	0 / 38	-84.9	-84.9	0.12 (1)	10.00
S- B	-1167 / 0	0.0	0.0	0.12 (1)	7.38
J- H	-1167 / 0	0.0	0.0	0.12 (1)	7.38
S- R	0 / 31	-18.5	-18.5	0.02 (4)	10.00
R- Q	0 / 19	0.0	0.0	0.05 (1)	10.00
Q- C	-53 / 0	0.0	0.0	0.05 (1)	7.81
Q- P	0 / 1010	-18.5	-18.5	0.24 (1)	10.00
P- O	0 / 963	-18.5	-18.5	0.25 (4)	10.00
O- N	0 / 963	-18.5	-18.5	0.25 (4)	10.00
N- M	0 / 963	-18.5	-18.5	0.25 (4)	10.00
M- L	0 / 1010	-18.5	-18.5	0.24 (1)	10.00
K- L	0 / 19	0.0	0.0	0.05 (1)	10.00
L- G	-53 / 0	0.0	0.0	0.05 (1)	7.81
K- J	0 / 31	-18.5	-18.5	0.02 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- P	-66 / 0	0.01 (1)
P- D	0 / 103	0.04 (4)
D- O	0 / 882	0.20 (1)
O- E	-694 / 0	0.16 (1)
O- F	0 / 882	0.20 (1)
M- F	0 / 103	0.04 (4)
M- G	-66 / 0	0.01 (1)
S- Q	-36 / 0	0.00 (1)
B- Q	0 / 995	0.22 (1)
L- J	-36 / 0	0.00 (1)
L- H	0 / 995	0.22 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.79/1.00 (E-F:1) , BC=0.25/1.00 (O-P:4) , WB=0.22/1.00 (B-Q:1) , SSI=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (D) (INPUT = 0.90)
JSI METAL= 0.48 (H) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
422443	T16	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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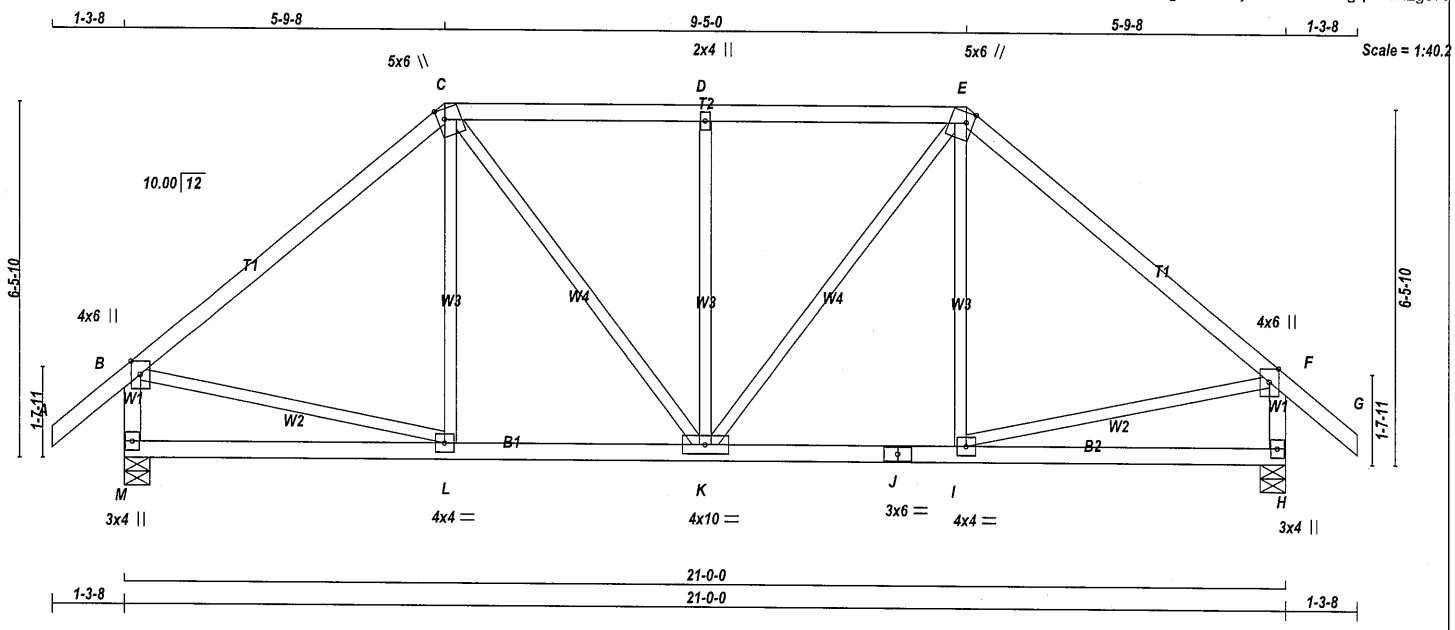

TOTAL WEIGHT = 95 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMVW+p	MT20	4.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	10.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1203	0	1203	0	0	5-8	5-8
H	1203	0	1203	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	852	554 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
H	852	554 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.80 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 38	-84.9	-84.9	0.12 (1)	10.00	L- C	-48 / 69	0.03 (1)
B- C	-980 / 0	-84.9	-84.9	0.39 (1)	5.80	C- K	0 / 346	0.08 (1)
C- D	-960 / 0	-84.9	-84.9	0.25 (1)	6.04	K- D	-485 / 0	0.33 (1)
D- E	-960 / 0	-84.9	-84.9	0.25 (1)	6.04	K- E	0 / 346	0.08 (1)
E- F	-980 / 0	-84.9	-84.9	0.39 (1)	5.80	I- E	-48 / 69	0.03 (1)
F- G	0 / 38	-84.9	-84.9	0.12 (1)	10.00	B- L	0 / 769	0.17 (1)
M- B	-1158 / 0	0.0	0.0	0.12 (1)	7.40	I- F	0 / 769	0.17 (1)
H- F	-1158 / 0	0.0	0.0	0.12 (1)	7.40			
M- L	0 / 0	-18.5	-18.5	0.14 (4)	10.00			
L- K	0 / 750	-18.5	-18.5	0.21 (4)	10.00			
K- J	0 / 750	-18.5	-18.5	0.21 (4)	10.00			
J- I	0 / 750	-18.5	-18.5	0.21 (4)	10.00			
I- H	0 / 0	-18.5	-18.5	0.14 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.39/1.00 (E-F:1) , BC=0.21/1.00 (I-K:4) , WB=0.33/1.00 (D-K:1) , SSI=0.19/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (I) (INPUT = 0.90)
JSI METAL= 0.50 (B) (INPUT = 1.00)

Structural component only
DWG# T-2205746

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
422291	T16S	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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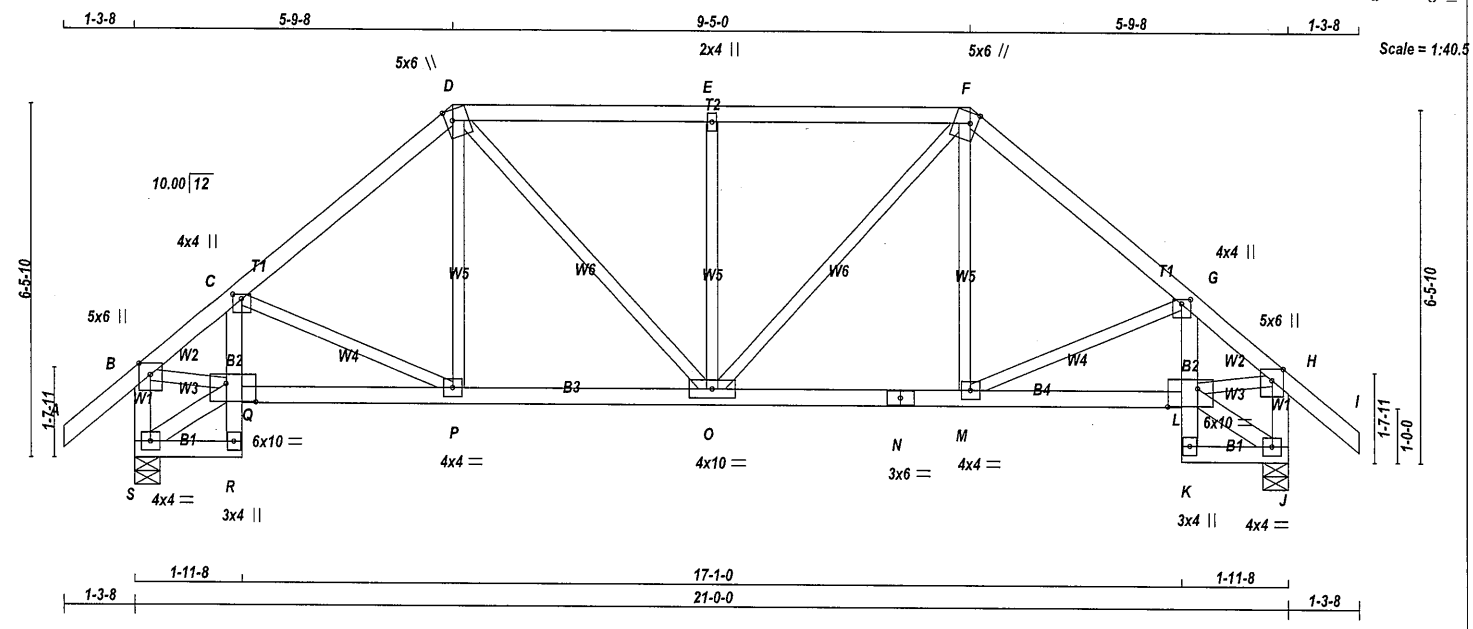

TOTAL WEIGHT = 101 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
S - R	2x4	DRY	No.2	SPF
R - C	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
K - G	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMVW+p	MT20	4.0	4.0	1.00	2.00
H	TMVW+p	MT20	5.0	6.0	Edge	
J	BMVW1-t	MT20	4.0	4.0		
K	BMV+p	MT20	3.0	4.0		
L	BVMWW-I	MT20	6.0	10.0	4.00	6.50
M	BMWW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	4.0	10.0		
P	BMWW-t	MT20	4.0	4.0		
Q	BVMWW-I	MT20	6.0	10.0	4.00	6.50
R	BMV+p	MT20	3.0	4.0		
S	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1)

Structural component only

DWG# T-2205732

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	1203	0	1203	0	0	5-8	5-8
J	1203	0	1203	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	852	554 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
J	852	554 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.46 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 38	-84.9	-84.9	0.12 (1)	10.00
B- C	-1358 / 0	-84.9	-84.9	0.12 (1)	5.46
C- D	-1153 / 0	-84.9	-84.9	0.18 (1)	5.73
D- E	-1132 / 0	-84.9	-84.9	0.25 (1)	5.68
E- F	-1132 / 0	-84.9	-84.9	0.25 (1)	5.68
F- G	-1153 / 0	-84.9	-84.9	0.18 (1)	5.73
G- H	-1358 / 0	-84.9	-84.9	0.12 (1)	5.46
H- I	0 / 38	-84.9	-84.9	0.12 (1)	10.00
S- B	-1166 / 0	0.0	0.0	0.12 (1)	7.38
J- H	-1166 / 0	0.0	0.0	0.12 (1)	7.38
S- R	0 / 32	-18.5	-18.5	0.02 (4)	10.00
R- Q	0 / 19	0.0	0.0	0.05 (1)	10.00
Q- C	-48 / 18	0.0	0.0	0.04 (1)	7.81
Q- P	0 / 1067	-18.5	-18.5	0.22 (1)	10.00
P- O	0 / 875	-18.5	-18.5	0.19 (1)	10.00
O- N	0 / 875	-18.5	-18.5	0.19 (1)	10.00
N- M	0 / 875	-18.5	-18.5	0.19 (1)	10.00
M- L	0 / 1067	-18.5	-18.5	0.22 (1)	10.00
K- L	0 / 19	0.0	0.0	0.05 (1)	10.00
L- G	-48 / 18	0.0	0.0	0.04 (1)	7.81
K- J	0 / 32	-18.5	-18.5	0.02 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- P	-213 / 0	0.07 (1)
P- D	0 / 147	0.04 (4)
D- O	0 / 382	0.09 (1)
O- E	-483 / 0	0.22 (1)
O- F	0 / 382	0.09 (1)
M- F	0 / 147	0.04 (4)
M- G	-213 / 0	0.07 (1)
S- Q	-37 / 0	0.00 (1)
B- Q	0 / 1052	0.24 (1)
L- J	-37 / 0	0.00 (1)
L- H	0 / 1052	0.24 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.25/1.00 (D-E:1) , BC=0.22/1.00 (L-M:1) , WB=0.24/1.00 (B-Q:1) , SSI=0.19/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.73 (H) (INPUT = 0.90)
JSI METAL= 0.49 (H) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
422443	T17	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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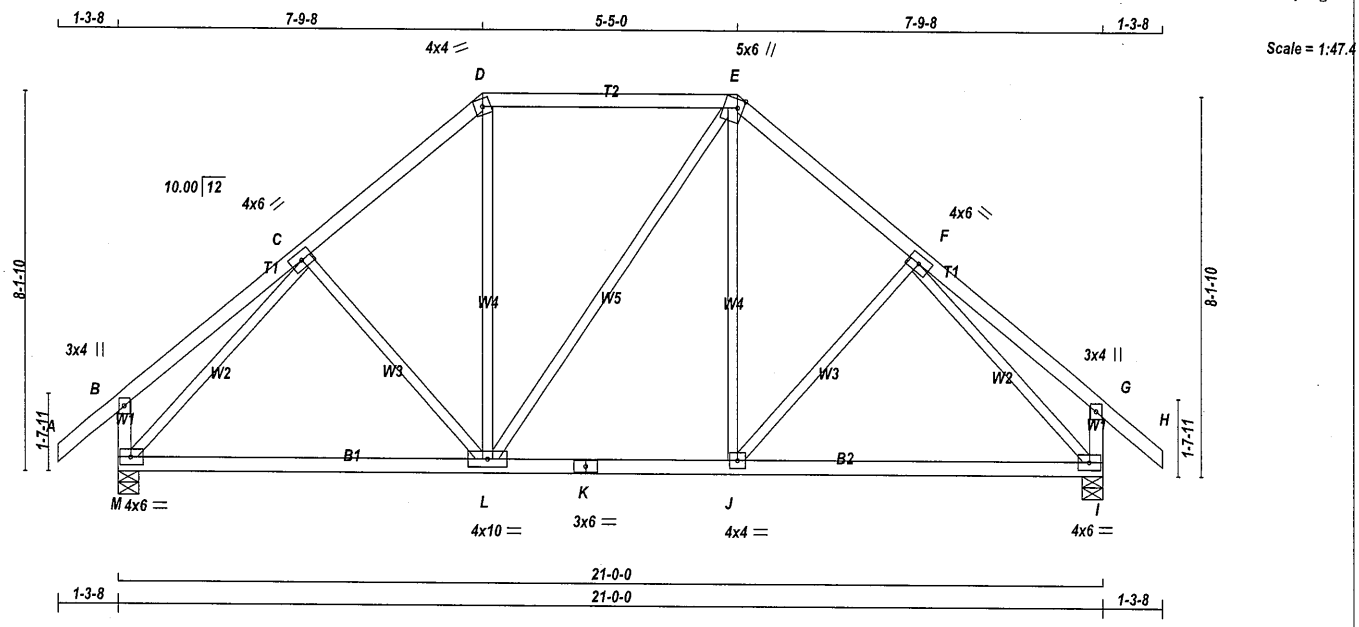

TOTAL WEIGHT = 99 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is In Inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMWW-t	MT20	4.0	6.0		
G	TMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	4.0	10.0		
M	BMVW1-t	MT20	4.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1203	0	1203	0	0	5-8	5-8
I	1203	0	1203	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	852	554 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
I	852	554 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.24 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 38	-84.9	-84.9	0.12 (1)	10.00
B- C	0 / 24	-84.9	-84.9	0.21 (1)	10.00
C- D	-924 / 0	-84.9	-84.9	0.17 (1)	6.24
D- E	-692 / 0	-84.9	-84.9	0.32 (1)	6.25
E- F	-924 / 0	-84.9	-84.9	0.17 (1)	6.24
F- G	0 / 24	-84.9	-84.9	0.21 (1)	10.00
G- H	0 / 38	-84.9	-84.9	0.12 (1)	10.00
M- B	-243 / 0	0.0	0.0	0.03 (1)	7.81
I- G	-243 / 0	0.0	0.0	0.03 (1)	7.81
M- L	0 / 767	-18.5	-18.5	0.30 (4)	10.00
L- K	0 / 692	-18.5	-18.5	0.28 (4)	10.00
K- J	0 / 692	-18.5	-18.5	0.28 (4)	10.00
J- I	0 / 767	-18.5	-18.5	0.30 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- L	-117 / 4	0.07 (1)
L- D	0 / 220	0.05 (4)
L- E	0 / 0	0.00 (1)
J- E	0 / 219	0.05 (4)
J- F	-117 / 4	0.07 (1)
M- C	-1181 / 0	0.71 (1)
F- I	-1180 / 0	0.71 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.32/1.00 (D-E:1) , BC=0.30/1.00 (I-J:4) , WB=0.71/1.00 (C-M:1) , SSI=0.18/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.71 (C) (INPUT = 0.90)
JSI METAL= 0.27 (K) (INPUT = 1.00)

Structural component only
DWG# T-2205747

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
422291	T17S	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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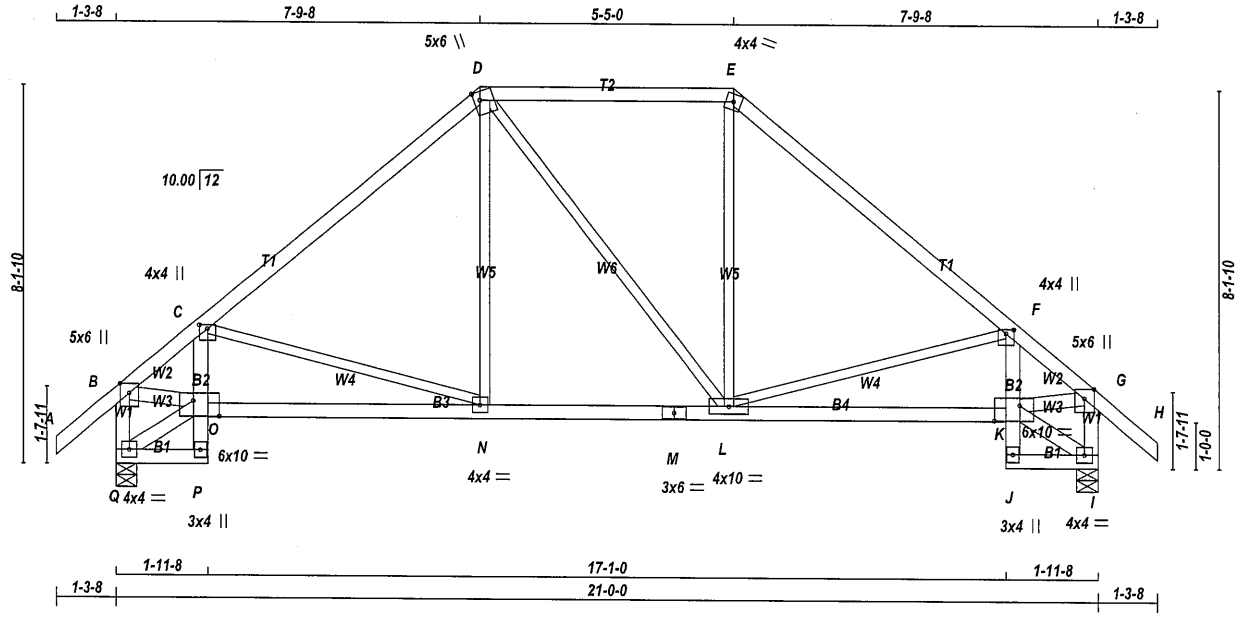

TOTAL WEIGHT = 101 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
Q - P	2x4	DRY	No.2	SPF
P - C	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
J - F	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
B - O	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
K - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TTW-m	MT20	4.0	4.0		
F	TMVW+p	MT20	4.0	4.0	1.00	2.00
G	TMVW+p	MT20	5.0	6.0	Edge	
I	BMVW1-t	MT20	4.0	4.0		
J	BMV+p	MT20	3.0	4.0		
K	BVMWW-l	MT20	6.0	10.0	4.00	6.50
L	BMWWW-t	MT20	4.0	10.0		
M	BS-t	MT20	3.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BVMWW-l	MT20	6.0	10.0	4.00	6.50
P	BMV+p	MT20	3.0	4.0		
Q	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1)

Structural component only
DWG# T-2205733

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	1203	0	1203	0	0	5-8	5-8
I	1203	0	1203	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	852	554 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
I	852	554 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.11 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 38	-84.9	-84.9	0.12 (1)	10.00	C- N	-414 / 0	0.30 (1)
B- C	-1432 / 0	-84.9	-84.9	0.31 (1)	5.11	N- D	0 / 208	0.05 (4)
C- D	-1038 / 0	-84.9	-84.9	0.39 (1)	5.68	D- L	0 / 2	0.00 (4)
D- E	-778 / 0	-84.9	-84.9	0.33 (1)	6.25	L- E	0 / 208	0.05 (4)
E- F	-1036 / 0	-84.9	-84.9	0.39 (1)	5.68	L- F	-415 / 0	0.30 (1)
F- G	-1432 / 0	-84.9	-84.9	0.31 (1)	5.11	Q- O	-36 / 0	0.00 (1)
G- H	0 / 38	-84.9	-84.9	0.12 (1)	10.00	B- O	0 / 1157	0.19 (1)
Q- B	-1167 / 0	0.0	0.0	0.12 (1)	7.38	K- I	-36 / 0	0.00 (1)
I- G	-1167 / 0	0.0	0.0	0.12 (1)	7.38	K- G	0 / 1157	0.19 (1)
Q- P	0 / 30	-18.5	-18.5	0.02 (4)	10.00			
P- O	0 / 19	0.0	0.0	0.05 (1)	10.00			
O- C	-49 / 32	0.0	0.0	0.04 (1)	7.81			
O- N	0 / 1171	-18.5	-18.5	0.27 (1)	10.00			
N- M	0 / 777	-18.5	-18.5	0.22 (4)	10.00			
M- L	0 / 777	-18.5	-18.5	0.22 (4)	10.00			
L- K	0 / 1171	-18.5	-18.5	0.27 (1)	10.00			
J- K	0 / 19	0.0	0.0	0.05 (1)	10.00			
K- F	-49 / 32	0.0	0.0	0.04 (1)	7.81			
J- I	0 / 30	-18.5	-18.5	0.02 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.39/1.00 (C-D:1) , BC=0.27/1.00 (K-L:1) , WB=0.30/1.00 (F-L:1) , SSI=0.18/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (N) (INPUT = 0.90)
JSI METAL= 0.52 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
422443	T18	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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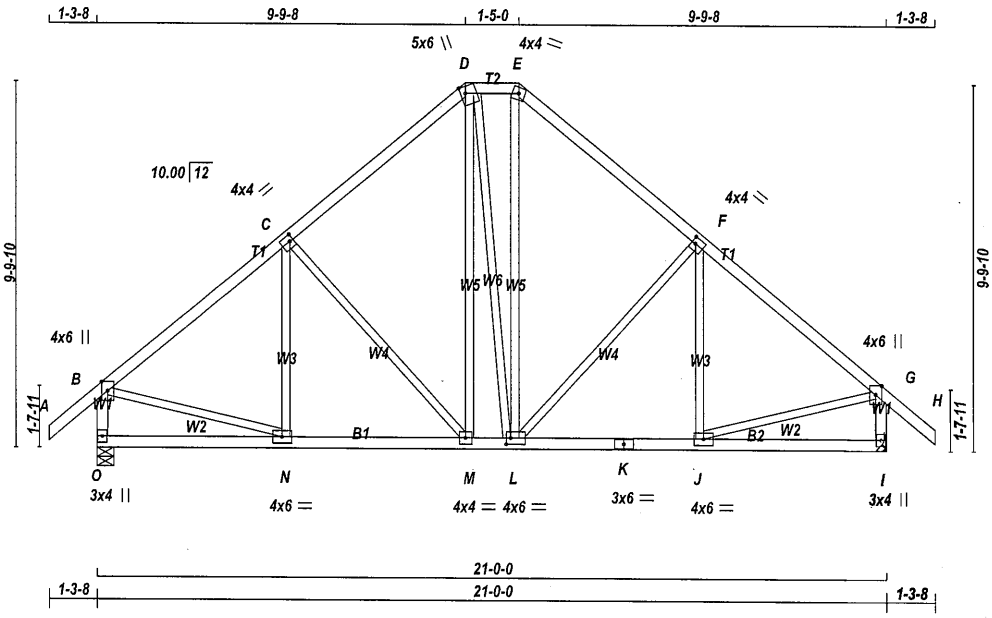

TOTAL WEIGHT = 112 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0	2.00	1.25
G	TMVW+p	MT20	4.0	6.0	Edge	
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	4.0	6.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	4.0	6.0	2.00	1.50
M	BMWW-t	MT20	4.0	4.0		
N	BMWW-t	MT20	4.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2205748

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1203	0	1203	0	0	5-8	5-8
I	1203	0	1203	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	852	554 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
I	852	554 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.87 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 38	-84.9	-84.9	0.12 (1)	10.00	N- C	-104 / 44		0.05 (1)
B- C	-1021 / 0	-84.9	-84.9	0.28 (1)	5.87	C- M	-319 / 0		0.34 (1)
C- D	-811 / 0	-84.9	-84.9	0.27 (1)	6.25	M- D	0 / 268		0.06 (1)
D- E	-602 / 0	-84.9	-84.9	0.03 (1)	6.25	D- L	0 / 20		0.00 (1)
E- F	-814 / 0	-84.9	-84.9	0.27 (1)	6.25	L- E	0 / 290		0.07 (1)
F- G	-1020 / 0	-84.9	-84.9	0.28 (1)	5.87	L- F	-314 / 0		0.33 (1)
G- H	0 / 38	-84.9	-84.9	0.12 (1)	10.00	J- F	-111 / 41		0.06 (1)
O- B	-1165 / 0	0.0	0.0	0.12 (1)	7.39	B- N	0 / 836		0.19 (1)
I- G	-1164 / 0	0.0	0.0	0.12 (1)	7.39	J- G	0 / 835		0.19 (1)
O- N	0 / 0	-18.5	-18.5	0.11 (4)	10.00				
N- M	0 / 809	-18.5	-18.5	0.19 (1)	10.00				
M- L	0 / 599	-18.5	-18.5	0.14 (1)	10.00				
L- K	0 / 808	-18.5	-18.5	0.18 (1)	10.00				
K- J	0 / 808	-18.5	-18.5	0.18 (1)	10.00				
J- I	0 / 0	-18.5	-18.5	0.11 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.28/1.00 (B-C:1) , BC=0.19/1.00 (M-N:1) , WB=0.34/1.00 (C-M:1) , SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (B) (INPUT = 0.90)
JSI METAL= 0.51 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
422291	T18S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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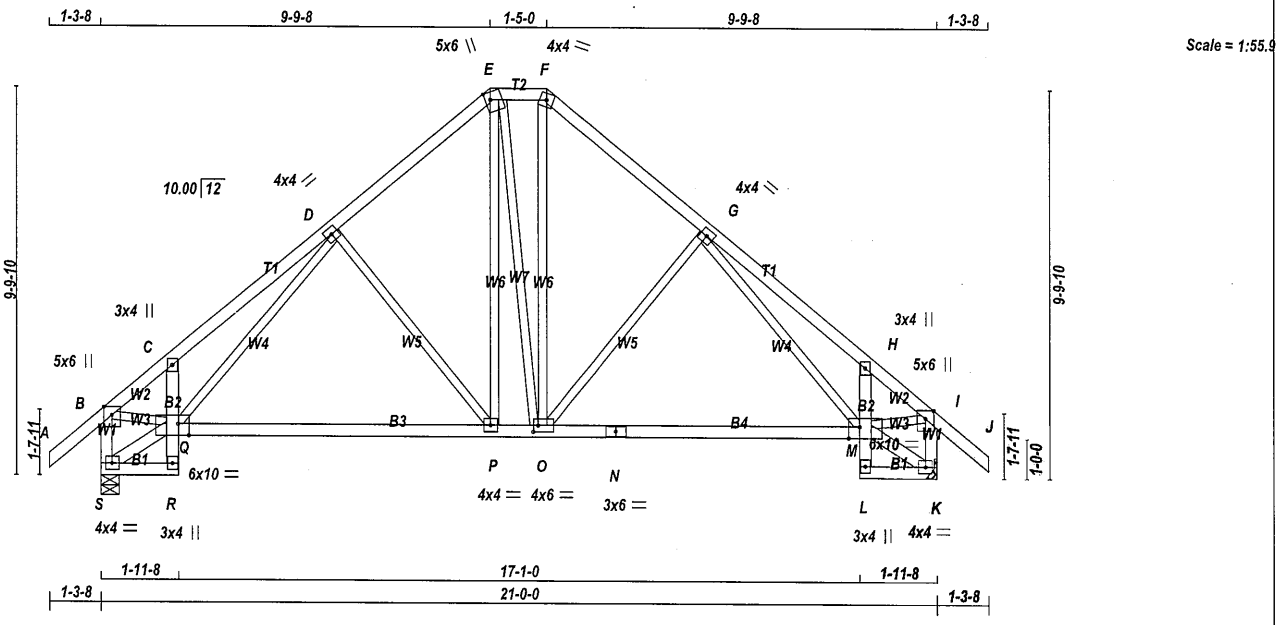

TOTAL WEIGHT = 114 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - J	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
S - R	2x4	DRY	No.2	SPF
R - C	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMV+p	MT20	3.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TTW-m	MT20	4.0	4.0		
G	TMWW-t	MT20	4.0	4.0		
H	TMV+p	MT20	3.0	4.0		
I	TMVW+p	MT20	5.0	6.0	Edge	
K	BMVW1-t	MT20	4.0	4.0		
L	BMV+p	MT20	3.0	4.0		
M	BVMWWW-l	MT20	6.0	10.0	Edge	3.25
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	4.0	6.0	2.00	1.50
P	BMWW-t	MT20	4.0	4.0		
Q	BVMWWW-l	MT20	6.0	10.0	Edge	3.25
R	BMV+p	MT20	3.0	4.0		
S	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES (1)
1)

Structural component only
DWG# T-2205734

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
S	1203	0	1203	0	0	5-8	5-8
K	1203	0	1203	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K. MINIMUM BEARING LENGTH AT JOINT K = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	852	554 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
K	852	554 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.60 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 38	-84.9	-84.9	0.12 (1)	10.00	D- P	-345 / 0	0.27 (1)	
B- C	-1254 / 0	-84.9	-84.9	0.10 (1)	5.65	P- E	0 / 354	0.08 (1)	
C- D	-1227 / 0	-84.9	-84.9	0.19 (1)	5.60	E- O	0 / 14	0.00 (1)	
D- E	-896 / 0	-84.9	-84.9	0.18 (1)	6.25	O- F	0 / 369	0.08 (1)	
E- F	-674 / 0	-84.9	-84.9	0.03 (1)	6.25	O- G	-344 / 0	0.27 (1)	
F- G	-899 / 0	-84.9	-84.9	0.18 (1)	6.25	G- M	0 / 116	0.03 (1)	
G- H	-1227 / 0	-84.9	-84.9	0.19 (1)	5.60	Q- D	0 / 118	0.03 (1)	
H- I	-1254 / 0	-84.9	-84.9	0.10 (1)	5.65	S- Q	-35 / 0	0.00 (1)	
I- J	0 / 38	-84.9	-84.9	0.12 (1)	10.00	B- Q	0 / 964	0.22 (1)	
S- B	-1169 / 0	0.0	0.0	0.12 (1)	7.37	M- K	-35 / 0	0.00 (1)	
K- I	-1169 / 0	0.0	0.0	0.12 (1)	7.37	M- I	0 / 964	0.22 (1)	
S- R	0 / 31	-18.5	-18.5	0.02 (4)	10.00				
R- Q	0 / 19	0.0	0.0	0.04 (1)	10.00				
Q- C	-192 / 0	0.0	0.0	0.03 (1)	7.81				
Q- P	0 / 886	-18.5	-18.5	0.35 (4)	10.00				
P- O	0 / 672	-18.5	-18.5	0.36 (4)	10.00				
O- N	0 / 887	-18.5	-18.5	0.37 (4)	10.00				
N- M	0 / 887	-18.5	-18.5	0.37 (4)	10.00				
L- M	0 / 19	0.0	0.0	0.04 (1)	10.00				
M- H	-192 / 0	0.0	0.0	0.03 (1)	7.81				
L- K	0 / 31	-18.5	-18.5	0.02 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.19/1.00 (C-D:1) , BC=0.37/1.00 (M-O:4) , WB=0.27/1.00 (D-P:1) , SSI=0.13/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.72 (O) (INPUT = 0.90)
JSI METAL= 0.46 (I) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
422443	T19	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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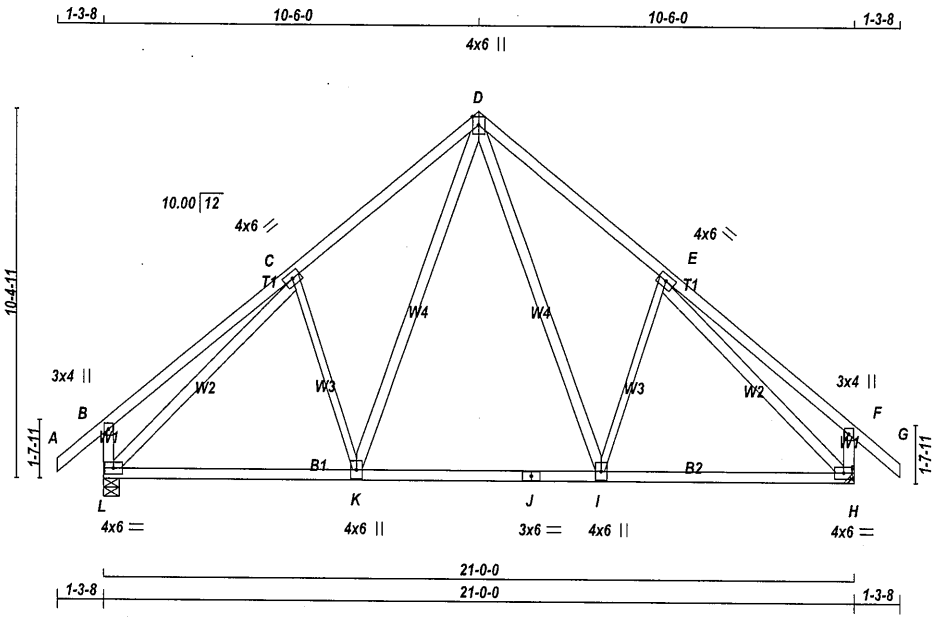

TOTAL WEIGHT = 112 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
I - E	2x3	DRY	No.2	SPF
C - K	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BMWW+t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW+t	MT20	4.0	6.0		
L	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1203	0	1203	0	0	5-8	5-8
H	1203	0	1203	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	852	554 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
H	852	554 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.96 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 38	-84.9	-84.9	0.12 (1)	10.00		D- I	0 / 439	0.07 (1)
B- C	0 / 35	-84.9	-84.9	0.39 (1)	10.00		I- E	-290 / 0	0.19 (1)
C- D	-953 / 0	-84.9	-84.9	0.32 (1)	5.96		K- D	0 / 439	0.07 (1)
D- E	-953 / 0	-84.9	-84.9	0.32 (1)	5.96		C- K	-290 / 0	0.19 (1)
E- F	0 / 35	-84.9	-84.9	0.39 (1)	10.00		L- C	-1178 / 0	0.99 (1)
F- G	0 / 38	-84.9	-84.9	0.12 (1)	10.00		E- H	-1178 / 0	0.99 (1)
L- B	-283 / 0	0.0	0.0	0.03 (1)	7.81				
H- F	-283 / 0	0.0	0.0	0.03 (1)	7.81				
L- K	0 / 798	-18.5	-18.5	0.28 (4)	10.00				
K- J	0 / 564	-18.5	-18.5	0.26 (4)	10.00				
J- I	0 / 564	-18.5	-18.5	0.26 (4)	10.00				
I- H	0 / 798	-18.5	-18.5	0.28 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.39/1.00 (B-C:1) , BC=0.28/1.00 (H-I:4) , WB=0.99/1.00 (E-H:1) , SSI=0.17/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (C) (INPUT = 0.90)
JSI METAL= 0.29 (E) (INPUT = 1.00)

Structural component only
DWG# T-2205749

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
422291	T19S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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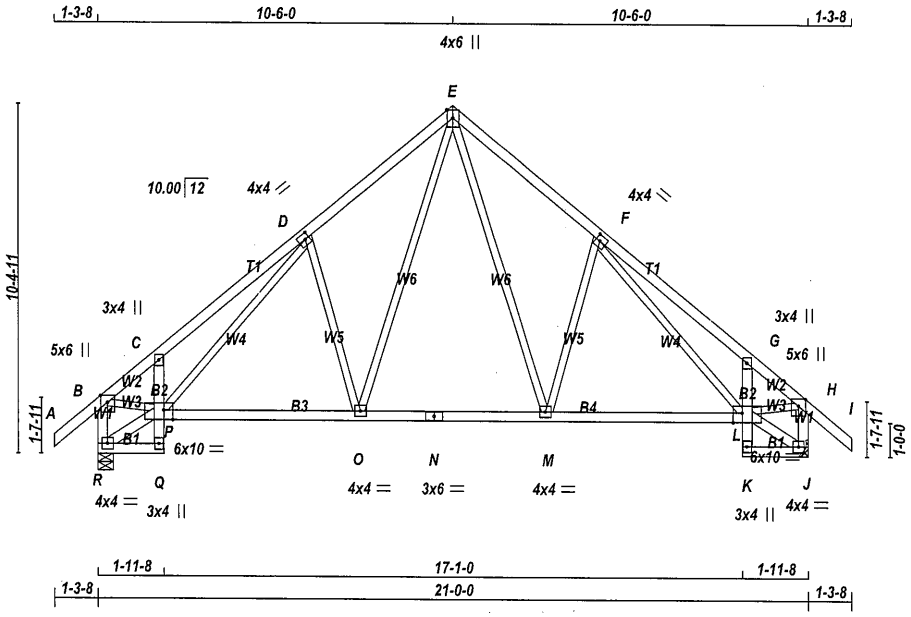

TOTAL WEIGHT = 109 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
R - Q	2x4	DRY	No.2	SPF
Q - C	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
K - G	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMV+p	MT20	3.0	4.0		
D	TMWW-t	MT20	4.0	4.0	2.00	1.50
E	TTWW+p	MT20	4.0	6.0	Edge	
F	TMWW-t	MT20	4.0	4.0	2.00	1.50
G	TMV+p	MT20	3.0	4.0		
H	TMVW+p	MT20	5.0	6.0	Edge	
J	BMVW1-t	MT20	4.0	4.0		
K	BMV+p	MT20	3.0	4.0		
L	BVMWWW-I	MT20	6.0	10.0	Edge	3.25
M	BMWW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BVMWWW-I	MT20	6.0	10.0	Edge	3.25
Q	BMV+p	MT20	3.0	4.0		
R	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2205735

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	1203	0	1203	0	0	5-8	5-8
J	1203	0	1203	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	852	554 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
J	852	554 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.55 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 38	-84.9	-84.9	0.12 (1)	10.00	E- M	0 / 521	0.12 (1)
B- C	-1260 / 0	-84.9	-84.9	0.10 (1)	5.64	M- F	-399 / 0	0.22 (1)
C- D	-1236 / 0	-84.9	-84.9	0.22 (1)	5.55	O- E	0 / 521	0.12 (1)
D- E	-1033 / 0	-84.9	-84.9	0.21 (1)	5.94	D- O	-399 / 0	0.22 (1)
E- F	-1033 / 0	-84.9	-84.9	0.21 (1)	5.94	P- D	0 / 123	0.03 (1)
F- G	-1236 / 0	-84.9	-84.9	0.22 (1)	5.55	F- L	0 / 123	0.03 (1)
G- H	-1260 / 0	-84.9	-84.9	0.10 (1)	5.64	R- P	-35 / 0	0.00 (1)
H- I	0 / 38	-84.9	-84.9	0.12 (1)	10.00	B- P	0 / 972	0.22 (1)
R- B	-1169 / 0	0.0	0.0	0.12 (1)	7.37	L- J	-35 / 0	0.00 (1)
J- H	-1169 / 0	0.0	0.0	0.12 (1)	7.37	L- H	0 / 972	0.22 (1)
R- Q	0 / 31	-18.5	-18.5	0.02 (4)	10.00			
Q- P	0 / 19	0.0	0.0	0.04 (1)	10.00			
P- C	-213 / 0	0.0	0.0	0.03 (1)	7.81			
P- O	0 / 888	-18.5	-18.5	0.22 (1)	10.00			
O- N	0 / 619	-18.5	-18.5	0.20 (4)	10.00			
N- M	0 / 619	-18.5	-18.5	0.20 (4)	10.00			
M- L	0 / 888	-18.5	-18.5	0.22 (1)	10.00			
K- L	0 / 19	0.0	0.0	0.04 (1)	10.00			
L- G	-213 / 0	0.0	0.0	0.03 (1)	7.81			
K- J	0 / 31	-18.5	-18.5	0.02 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.22/1.00 (F-G:1) , BC=0.22/1.00 (O-P:1) , WB=0.22/1.00 (F-M:1) , SSI=0.14/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.68 (H) (INPUT = 0.90)
JSI METAL= 0.46 (H) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
422291	T20	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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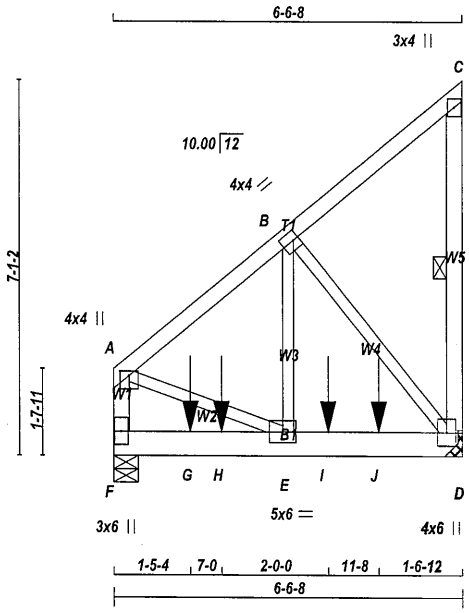

Scale = 1:41.8

TOTAL WEIGHT = 2 X 39 = 78 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
F- A	1	12	TOP
A- C	1	12	TOP
C- D	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMWW-t	MT20	5.0	6.0		

LICENSED PROFESSIONAL ENGINEER
02-28-22
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2205736

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	1410	0	1410	0	0	5-8	5-8
D	1410	0	1410	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	996	663 / 0	0 / 0	0 / 0	0 / 0	333 / 0	0 / 0
D	997	656 / 0	0 / 0	0 / 0	0 / 0	341 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-D.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FR-TO	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	FR-TO	MAX CSI (LC)
F- A	-1124 / 0		0.0	0.0	0.06 (1)	7.81	A- E	0 / 898		0.11 (1)
A- B	-1076 / 0		-84.9	-84.9	0.08 (1)	6.25	E- B	0 / 1378		0.17 (1)
B- C	-20 / 0		-84.9	-84.9	0.08 (1)	6.25	B- D	-1319 / 0		0.27 (1)
D- C	-108 / 0		0.0	0.0	0.01 (1)	6.25				
F- G	0 / 0		-18.5	-18.5	0.19 (1)	10.00				
G- H	0 / 0		-18.5	-18.5	0.19 (1)	10.00				
H- E	0 / 0		-18.5	-18.5	0.19 (1)	10.00				
E- I	0 / 842		-18.5	-18.5	0.24 (1)	10.00				
I- J	0 / 842		-18.5	-18.5	0.24 (1)	10.00				
J- D	0 / 842		-18.5	-18.5	0.24 (1)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-5-4	-56	-56	---	FRONT	VERT	TOTAL	---	C1
H	2-0-4	-679	-679	---	FRONT	VERT	TOTAL	---	C1
I	4-0-4	-389	-389	---	FRONT	VERT	TOTAL	---	C1
J	4-11-12	-389	-389	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.08/1.00 (A-B:1) , BC=0.24/1.00 (D-E:1) , WB=0.27/1.00 (B-D:1) , SSI=0.29/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (B) (INPUT = 0.90)
JSI METAL= 0.19 (D) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
422291	T20	1	2	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
F	BMV1+p	MT20	3.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2205736

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
422291	T21	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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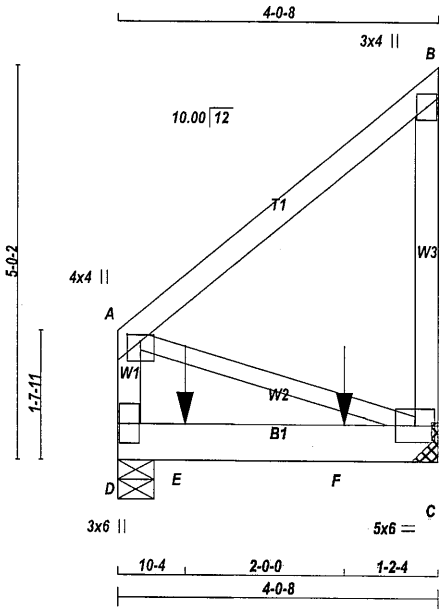

Scale = 1:28.2

TOTAL WEIGHT = 2 X 22 = 45 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
D - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
C - B	2x4	DRY	No.2	SPF
D - C	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
D- A	1	12	TOP
A- B	1	12	TOP
B- C	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
D- C	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TMV+p	MT20	3.0	4.0		
C	BMVW1-t	MT20	5.0	6.0		
D	BMV1+p	MT20	3.0	6.0		

Structural component only
DWG# T-2205737

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	1492	0	1492	0	0	5-8	5-8
C	1296	0	1296	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	1055	693 / 0	0 / 0	0 / 0	0 / 0	362 / 0	0 / 0
C	916	602 / 0	0 / 0	0 / 0	0 / 0	314 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D- A	-171 / 0	0.0	0.0	0.01 (1)	7.81	A- C	0 / 0	0.00 (1)
A- B	0 / 0	-84.9	-84.9	0.13 (1)	10.00			
C- B	-171 / 0	0.0	0.0	0.03 (1)	7.81			
D- E	0 / 0	-18.5	-18.5	0.49 (1)	10.00			
E- F	0 / 0	-18.5	-18.5	0.49 (1)	10.00			
F- C	0 / 0	-18.5	-18.5	0.49 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	10-4	-838	-838	---	FRONT	VERT	TOTAL	---	C1
F	2-10-4	-837	-837	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 810 (0.06")

CSI: TC=0.13/1.00 (A-B:1) , BC=0.49/1.00 (C-D:1) , WB=0.00/1.00 (A-C:1) , SSI=0.44/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.06 (A) (INPUT = 0.90)
JSI METAL= 0.03 (B) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
422291	T21Z	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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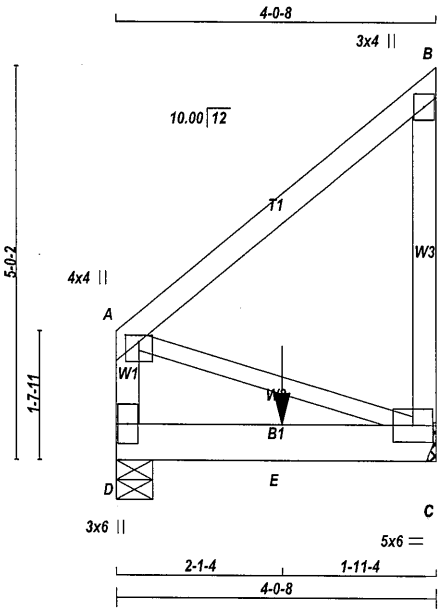

Scale = 1:28.2

TOTAL WEIGHT = 2 X 22 = 45 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
D - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
C - B	2x4	DRY	No.2	SPF
D - C	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
D- A	1	12	TOP
A- B	1	12	TOP
B- C	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
D- C	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TMV+p	MT20	3.0	4.0		
C	BMVW1-t	MT20	5.0	6.0		
D	BMV1+p	MT20	3.0	6.0		

Structural component only
DWG# T-2205738

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	826	0	826	0	0	5-8	5-8
C	879	0	879	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	586	376 / 0	0 / 0	0 / 0	0 / 0	210 / 0	0 / 0
C	623	400 / 0	0 / 0	0 / 0	0 / 0	223 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
D- A	-171 / 0	0.0	0.0	0.01 (1)	7.81	A- C	0 / 0	0.00 (1)
A- B	0 / 0	-84.9	-84.9	0.13 (1)	10.00			
C- B	-171 / 0	0.0	0.0	0.03 (1)	7.81			
D- E	0 / 0	-18.5	-18.5	0.49 (1)	10.00			
E- C	0 / 0	-18.5	-18.5	0.49 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	2-1-4	-913	-913	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.13/1.00 (A-B:1) , BC=0.49/1.00 (C-D:1) , WB=0.00/1.00 (A-C:1) , SSI=0.23/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.06 (A) (INPUT = 0.90)
JSI METAL= 0.03 (B) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
422291	T23S	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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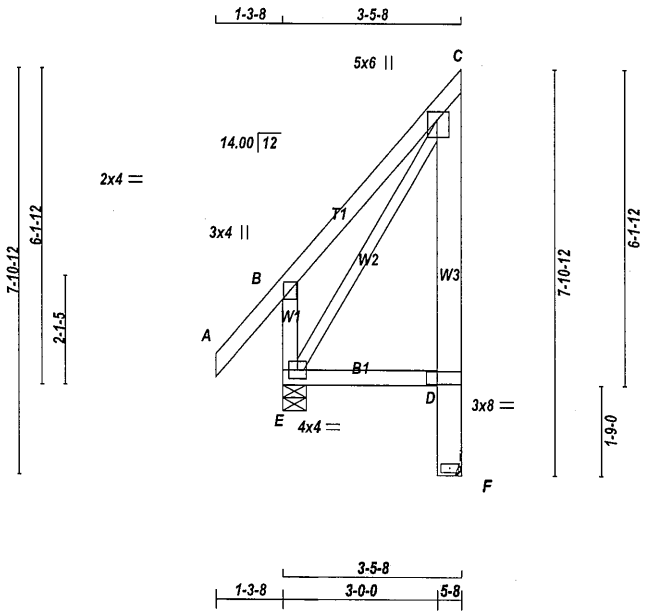

TOTAL WEIGHT = 3 X 32 = 96 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - C	2x6	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
B	TP-p	MT20	2.0	4.0		
C	TMVW+p	MT20	5.0	6.0	2.00	2.25
D	BVM-I	MT20	3.0	8.0	1.50	2.50
E	BMVW1-t	MT20	4.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
E	298	0	298	0	0	5-8	5-8
F	179	0	179	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	209	146 / 0	0 / 0	0 / 0	0 / 0	63 / 0	0 / 0
F	127	81 / 0	0 / 0	0 / 0	0 / 0	46 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-266 / 0	0.0	0.0	0.03 (1)	7.81	E- C	0 / 0	0.00 (1)
A- B	0 / 45	-84.9	-84.9	0.14 (5)	10.00			
B- C	0 / 0	-84.9	-84.9	0.17 (1)	10.00			
F- D	-179 / 0	0.0	0.0	0.01 (1)	7.81			
D- C	-147 / 0	0.0	0.0	0.08 (1)	7.81			
E- D	0 / 0	-18.5	-18.5	0.06 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.17/1.00 (B-C:1) , BC=0.06/1.00 (D-E:4) , WB=0.00/1.00 (C-E:1) , SSI=0.07/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (E) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)

Structural component only
DWG# T-2205739

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
422291	T24S	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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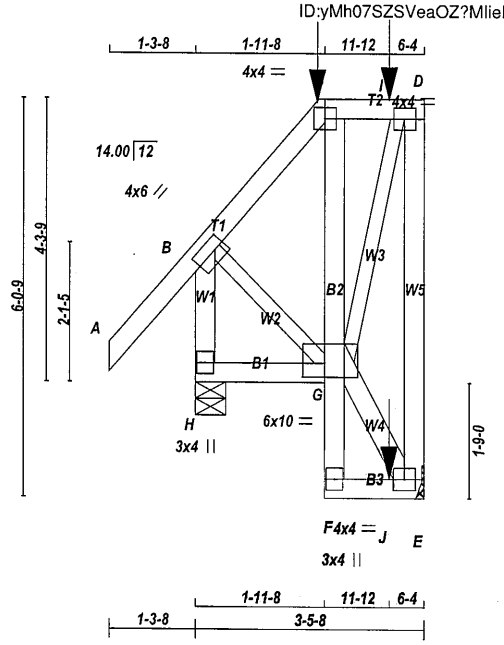

Scale = 1:33.6

TOTAL WEIGHT = 34 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
F - C	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
G - D	2x3	DRY	No.2	SPF
B - G	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	2.00	2.00
C	TTV-I	MT20	4.0	4.0		
D	TMVW-t	MT20	4.0	4.0		
E	BMVW1-t	MT20	4.0	4.0		
F	BMV+p	MT20	3.0	4.0		
G	BVMWWW-I	MT20	6.0	10.0	2.50	4.00
H	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
02-28-22
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2205740

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	177	0	177	0	0	MECHANICAL	
H	328	0	328	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	126	75 / -13	0 / 0	0 / 0	0 / 0	52 / 0	0 / 0
H	231	160 / 0	0 / 0	0 / 0	0 / 0	70 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 45	-84.9	-84.9	0.17 (5)	10.00	G- E	-4 / 0	0.00 (1)
B- C	-82 / 0	-84.9	-84.9	0.16 (5)	6.25	G- D	0 / 89	0.02 (1)
C- I	-28 / 0	-84.9	-84.9	0.04 (1)	6.25	B- G	0 / 42	0.01 (1)
I- D	-28 / 0	-84.9	-84.9	0.04 (1)	6.25			
E- D	-158 / 0	0.0	0.0	0.11 (1)	7.81			
H- B	-309 / 0	0.0	0.0	0.04 (1)	7.81			
H- G	0 / 0	-18.5	-18.5	0.03 (4)	10.00			
F- G	0 / 16	0.0	0.0	0.01 (1)	10.00			
G- C	-80 / 19	0.0	0.0	0.01 (1)	7.81			
F- J	0 / 2	-18.5	-18.5	0.01 (4)	10.00			
J- E	0 / 2	-18.5	-18.5	0.01 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	1-11-8	-6	-6	---	BACK	VERT	TOTAL	---	C1
I	2-11-4	-11	-11	---	BACK	VERT	TOTAL	---	C1
J	2-11-4	-4	-4	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.17/1.00 (A-B:5) , BC=0.03/1.00 (G-H:4) , WB=0.02/1.00 (D-G:1) , SSI=0.09/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.16 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
422291	T25	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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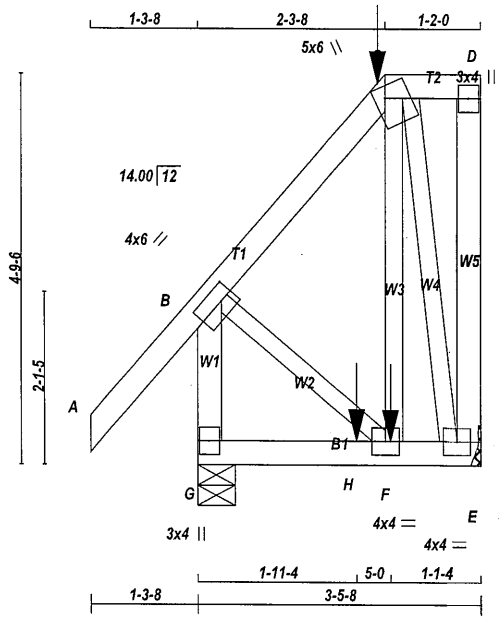

Scale = 1:27.1

TOTAL WEIGHT = 28 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	2.00	2.00
C	TTWW+m	MT20	5.0	6.0	1.75	1.50
D	TMV+p	MT20	3.0	4.0		
E	BMVW1-t	MT20	4.0	4.0		
F	BMWW-t	MT20	4.0	4.0		
G	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	212	0	212	0	0	MECHANICAL	
G	316	0	316	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	151	93 / 0	0 / 0	0 / 0	0 / 0	58 / 0	0 / 0
G	222	152 / 0	0 / 0	0 / 0	0 / 0	70 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (9)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 45	-84.9	-84.9	0.18 (5)	10.00	F- C	0 / 44	0.02 (4)
B- C	-63 / 0	-84.9	-84.9	0.08 (1)	6.25	C- E	-161 / 0	0.06 (1)
C- D	0 / 0	-84.9	-84.9	0.02 (1)	10.00	B- F	0 / 50	0.01 (1)
E- D	-50 / 0	0.0	0.0	0.02 (1)	7.81			
G- B	-293 / 0	0.0	0.0	0.04 (1)	7.81			
G- H	0 / 0	-18.5	-18.5	0.03 (4)	10.00			
H- F	0 / 0	-18.5	-18.5	0.03 (4)	10.00			
F- E	0 / 41	-18.5	-18.5	0.02 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-3-8	-30	-30	102	FRONT	VERT	TOTAL	---	C1
F	2-4-4	-2	-2	---	FRONT	VERT	TOTAL	---	C1
H	1-11-4	-5	-5	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.18/1.00 (A-B:5) , BC=0.03/1.00 (F-G:4) , WB=0.06/1.00 (C-E:1) , SSI=0.09/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

Structural component only
DWG# T-2205741

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
422291	T26	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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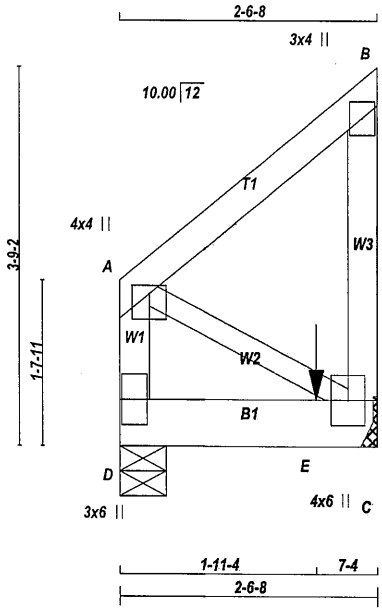

Scale = 1:22.0

TOTAL WEIGHT = 2 X 15 = 30 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
D - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
C - B	2x4	DRY	No.2	SPF
D - C	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
D- A	1	12	TOP
A- B	1	12	TOP
B- C	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
D- C	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TMV+p	MT20	3.0	4.0		
C	BMVW1+p	MT20	4.0	6.0		
D	BMV1+p	MT20	3.0	6.0		

Structural component only
DWG# T-2205742

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	191	0	191	0	0	5-8	5-8
C	322	0	322	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 2-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	134	91 / 0	0 / 0	0 / 0	0 / 0	43 / 0	0 / 0
C	225	162 / 0	0 / 0	0 / 0	0 / 0	63 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D- A	-108 / 0	0.0	0.0	0.01 (1)		7.81	A- C	0 / 0	0.00 (1)
A- B	0 / 0	-84.9	-84.9	0.05 (1)		10.00			
C- B	-108 / 0	0.0	0.0	0.01 (1)		7.81			
D- E	0 / 0	-18.5	-18.5	0.05 (1)		10.00			
E- C	0 / 0	-18.5	-18.5	0.05 (1)		10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	1-11-4	-173	-173	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.05/1.00 (A-B:1) , BC=0.05/1.00 (C-D:1) , WB=0.00/1.00 (A-C:1) , SSI=0.07/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.04 (A) (INPUT = 0.90)
JSI METAL= 0.02 (B) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
422291	PB1	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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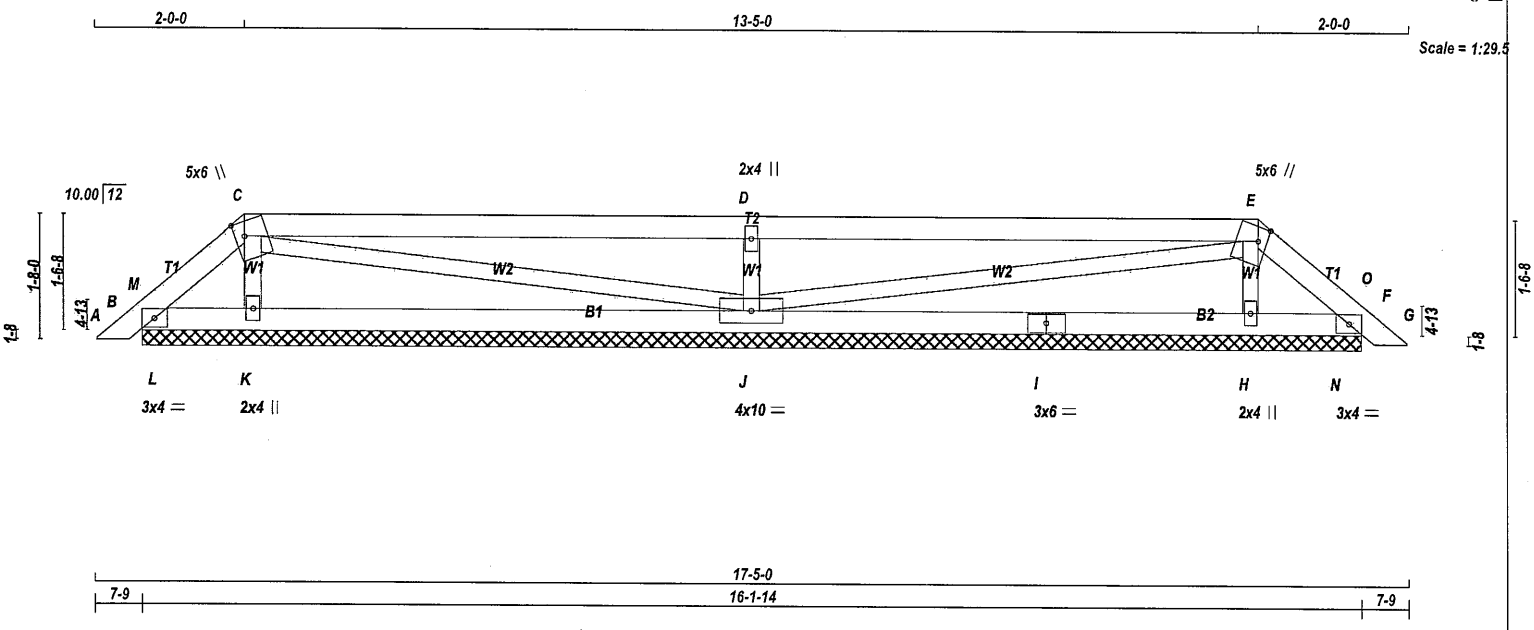

TOTAL WEIGHT = 52 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
B - I	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY::SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMB1-I	MT20	3.0	4.0		
H	BMW1+w	MT20	2.0	4.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW1-t	MT20	4.0	10.0		
K	BMW1+w	MT20	2.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	111	0	111	0	-2	16-1-14 (4-2-2	6-1-14
K	347	0	347	0	0	16-1-14 (4-2-2	6-1-14
J	842	0	842	0	0	16-1-14 (4-2-2	6-1-14
H	347	0	347	0	0	16-1-14 (4-2-2	6-1-14
F	111	0	111	0	-2	16-1-14 (4-2-2	6-1-14

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT F FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	74	75 / 0	0 / 0	0 / 0	0 / 0	0 / -1	0 / 0
K	252	129 / 0	0 / 0	0 / 0	0 / 0	123 / 0	0 / 0
J	595	392 / 0	0 / 0	0 / 0	0 / 0	203 / 0	0 / 0
H	252	129 / 0	0 / 0	0 / 0	0 / 0	123 / 0	0 / 0
F	74	75 / 0	0 / 0	0 / 0	0 / 0	0 / -1	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, K, J, H, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 13	-84.9	-84.9	0.02 (1)	10.00
B- M	-9 / 33	-84.9	-84.9	0.03 (4)	10.00
M- C	-54 / 0	-84.9	-84.9	0.03 (4)	6.25
C- D	-6 / 0	-84.9	-84.9	0.65 (1)	10.00
D- E	-6 / 0	-84.9	-84.9	0.65 (1)	10.00
E- O	-54 / 0	-84.9	-84.9	0.03 (4)	6.25
O- F	-9 / 33	-84.9	-84.9	0.03 (4)	10.00
F- G	0 / 13	-84.9	-84.9	0.02 (1)	10.00
B- L	0 / 35	-18.5	-18.5	0.03 (1)	10.00
L- K	0 / 35	-18.5	-18.5	0.13 (4)	10.00
K- J	0 / 17	-18.5	-18.5	0.17 (4)	10.00
J- I	0 / 17	-18.5	-18.5	0.17 (4)	10.00
I- H	0 / 17	-18.5	-18.5	0.17 (4)	10.00
H- N	0 / 35	-18.5	-18.5	0.13 (4)	10.00
N- F	0 / 35	-18.5	-18.5	0.03 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-217 / 0	0.03 (1)
C- J	-11 / 0	0.01 (1)
J- D	-711 / 0	0.10 (1)
J- E	-11 / 0	0.01 (1)
H- E	-217 / 0	0.03 (1)
L- M	-121 / 0	0.00 (1)
N- O	-121 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.65/1.00 (C-D:1) , BC=0.17/1.00 (J-K:4) , WB=0.10/1.00 (D-J:1) , SSI=0.28/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.59 (E) (INPUT = 0.90)
JSI METAL= 0.16 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
02-28-22
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2205706

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
422291	PB2	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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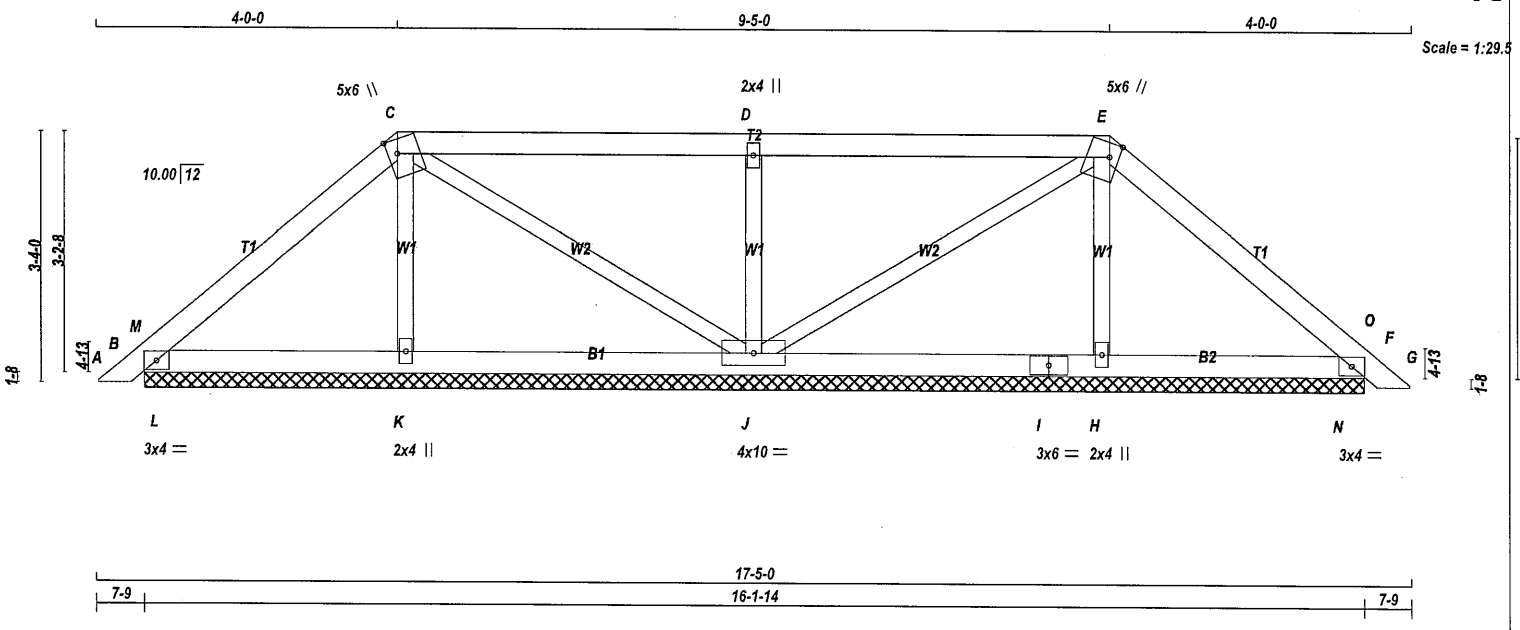

TOTAL WEIGHT = 56 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
B - I	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMB1-I	MT20	3.0	4.0		
H	BMW1+w	MT20	2.0	4.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW1-t	MT20	4.0	10.0		
K	BMW1+w	MT20	2.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	273	0	273	0	0	16-1-14 (4-2-2)	6-1-14
K	289	0	289	0	0	16-1-14 (4-2-2)	6-1-14
J	634	0	634	0	0	16-1-14 (4-2-2)	6-1-14
H	289	0	289	0	0	16-1-14 (4-2-2)	6-1-14
F	273	0	273	0	0	16-1-14 (4-2-2)	6-1-14

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	191	135 / 0	0 / 0	0 / 0	0 / 0	56 / 0	0 / 0
K	208	116 / 0	0 / 0	0 / 0	0 / 0	92 / 0	0 / 0
J	448	299 / 0	0 / 0	0 / 0	0 / 0	148 / 0	0 / 0
H	208	116 / 0	0 / 0	0 / 0	0 / 0	92 / 0	0 / 0
F	191	135 / 0	0 / 0	0 / 0	0 / 0	56 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, K, J, H, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 13	-84.9	-84.9	0.02 (1)	10.00	K- C	-175 / 0	0.03 (1)
B- M	-21 / 2	-84.9	-84.9	0.05 (1)	6.25	C- J	-58 / 0	0.03 (1)
M- C	-110 / 0	-84.9	-84.9	0.12 (1)	6.25	J- D	-499 / 0	0.09 (1)
C- D	-22 / 0	-84.9	-84.9	0.32 (1)	6.25	J- E	-58 / 0	0.03 (1)
D- E	-22 / 0	-84.9	-84.9	0.32 (1)	6.25	H- E	-175 / 0	0.03 (1)
E- O	-110 / 0	-84.9	-84.9	0.12 (1)	6.25	L- M	-294 / 0	0.00 (1)
O- F	-21 / 2	-84.9	-84.9	0.05 (1)	6.25	N- O	-294 / 0	0.00 (1)
F- G	0 / 13	-84.9	-84.9	0.02 (1)	10.00			
B- L	0 / 78	-18.5	-18.5	0.11 (1)	10.00			
L- K	0 / 78	-18.5	-18.5	0.11 (1)	10.00			
K- J	0 / 72	-18.5	-18.5	0.08 (4)	10.00			
J- I	0 / 72	-18.5	-18.5	0.08 (4)	10.00			
I- H	0 / 72	-18.5	-18.5	0.08 (4)	10.00			
H- N	0 / 78	-18.5	-18.5	0.11 (1)	10.00			
N- F	0 / 78	-18.5	-18.5	0.11 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.32/1.00 (D-E:1) , BC=0.11/1.00 (F-N:1) , WB=0.09/1.00 (D-J:1) , SSI=0.23/1.00 (F-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (E) (INPUT = 0.90)
JSI METAL= 0.10 (D) (INPUT = 1.00)

Structural component only
DWG# T-2205707

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
422291	PB3	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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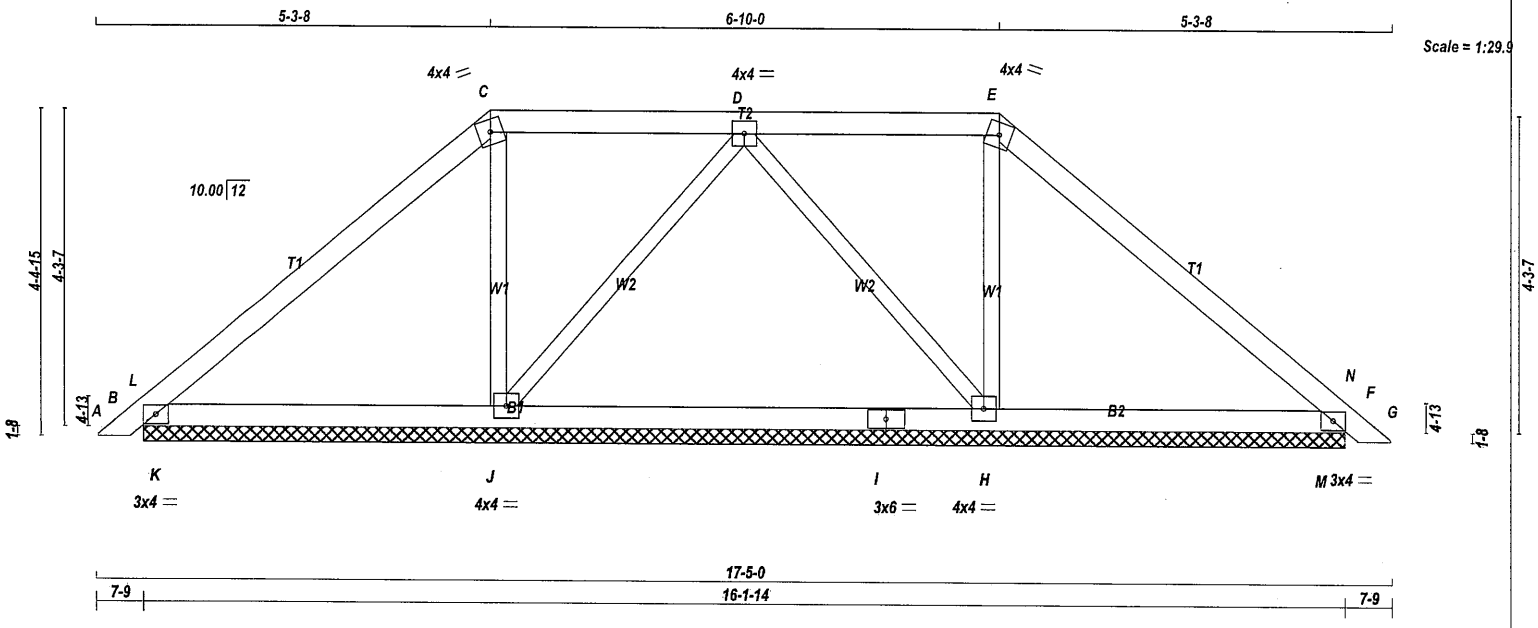

TOTAL WEIGHT = 56 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
B - I	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB1-I	MT20	3.0	4.0	
C	TTW-m	MT20	4.0	4.0	
D	TMWW-t	MT20	4.0	4.0	
E	TTW-m	MT20	4.0	4.0	
F	TMB1-I	MT20	3.0	4.0	
H	BMWW1-t	MT20	4.0	4.0	
I	BS-t	MT20	3.0	6.0	
J	BMWW1-t	MT20	4.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	359	0	359	0	0	16-1-14 (6-2-2)	6-1-14
J	520	0	520	0	0	16-1-14 (6-2-2)	6-1-14
H	520	0	520	0	0	16-1-14 (6-2-2)	6-1-14
F	359	0	359	0	0	16-1-14 (6-2-2)	6-1-14

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	252	177 / 0	0 / 0	0 / 0	0 / 0	75 / 0	0 / 0
J	371	224 / 0	0 / 0	0 / 0	0 / 0	148 / 0	0 / 0
H	371	224 / 0	0 / 0	0 / 0	0 / 0	148 / 0	0 / 0
F	252	177 / 0	0 / 0	0 / 0	0 / 0	75 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, J, H, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 13	-84.9	-84.9	0.02 (1)	10.00
B- L	-16 / 104	-84.9	-84.9	0.13 (1)	6.25
L- C	-151 / 0	-84.9	-84.9	0.23 (1)	6.25
C- D	-101 / 0	-84.9	-84.9	0.16 (1)	6.25
D- E	-101 / 0	-84.9	-84.9	0.16 (1)	6.25
E- N	-151 / 0	-84.9	-84.9	0.23 (1)	6.25
N- F	-16 / 104	-84.9	-84.9	0.13 (1)	6.25
F- G	0 / 13	-84.9	-84.9	0.02 (1)	10.00
B- K	0 / 106	-18.5	-18.5	0.20 (1)	10.00
K- J	0 / 106	-18.5	-18.5	0.20 (1)	10.00
J- I	0 / 250	-18.5	-18.5	0.17 (4)	10.00
I- H	0 / 250	-18.5	-18.5	0.17 (4)	10.00
H- M	0 / 106	-18.5	-18.5	0.20 (1)	10.00
M- F	0 / 106	-18.5	-18.5	0.20 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- C	-190 / 0	0.05 (1)
J- D	-233 / 0	0.11 (1)
D- H	-233 / 0	0.11 (1)
H- E	-190 / 0	0.05 (1)
K- L	-543 / 0	0.00 (1)
M- N	-543 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.23/1.00 (E-N:1) , BC=0.20/1.00 (F-M:1) , WB=0.11/1.00 (D-H:1) , SSI=0.42/1.00 (F-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.62 (E) (INPUT = 0.90)
JSI METAL= 0.11 (I) (INPUT = 1.00)

Structural component only
DWG# T-2205708

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
422291	PB4	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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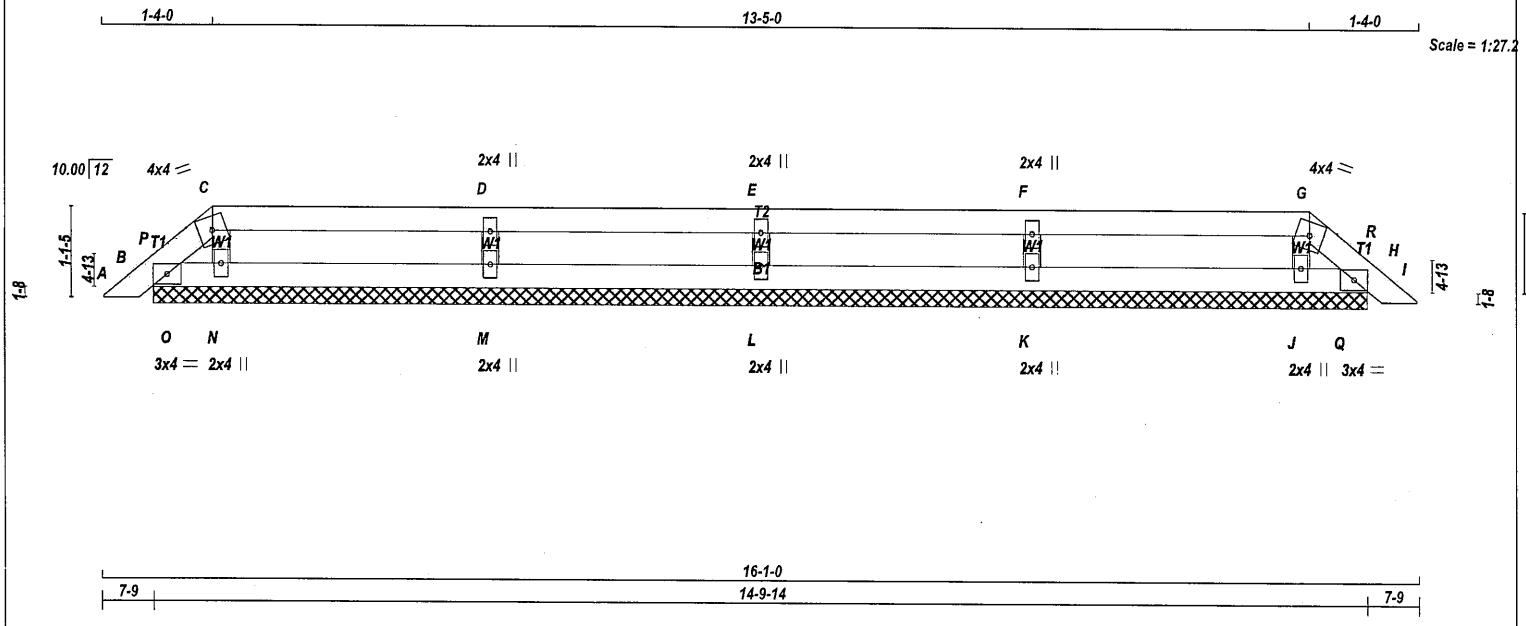

TOTAL WEIGHT = 38 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
B - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB1-I	MT20	3.0	4.0	
C	TTW-m	MT20	4.0	4.0	
D, E, F					
D	TMW+w	MT20	2.0	4.0	
G	TTW-m	MT20	4.0	4.0	
H	TMB1-I	MT20	3.0	4.0	
J, K, L, M, N					
J	BMW1+w	MT20	2.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	99	0	99	0	0	14-9-14	14-9-14
H	99	0	99	0	0	14-9-14	14-9-14
N	164	0	164	0	0	14-9-14	14-9-14
L	317	0	317	0	0	14-9-14	14-9-14
J	164	0	164	0	0	14-9-14	14-9-14
M	389	0	389	0	0	14-9-14	14-9-14
K	389	0	389	0	0	14-9-14	14-9-14

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	67	60 / 0	0 / 0	0 / 0	0 / 0	7 / 0	0 / 0
H	67	60 / 0	0 / 0	0 / 0	0 / 0	7 / 0	0 / 0
N	119	60 / 0	0 / 0	0 / 0	0 / 0	59 / 0	0 / 0
L	225	141 / 0	0 / 0	0 / 0	0 / 0	85 / 0	0 / 0
J	119	60 / 0	0 / 0	0 / 0	0 / 0	59 / 0	0 / 0
M	276	179 / 0	0 / 0	0 / 0	0 / 0	96 / 0	0 / 0
K	276	179 / 0	0 / 0	0 / 0	0 / 0	96 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, H, N, L, J, M, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 13	-84.9	-84.9	0.02 (1)	10.00	N- C	-104 / 0	0.01 (1)
B- P	-40 / 0	-84.9	-84.9	0.02 (1)	6.25	L- E	-256 / 0	0.04 (1)
P- C	-33 / 0	-84.9	-84.9	0.01 (1)	6.25	J- G	-104 / 0	0.01 (1)
C- D	-1 / 0	-84.9	-84.9	0.14 (1)	10.00	M- D	-327 / 0	0.05 (1)
D- E	-2 / 0	-84.9	-84.9	0.14 (1)	10.00	K- F	-327 / 0	0.05 (1)
E- F	-2 / 0	-84.9	-84.9	0.14 (1)	10.00	O- P	-22 / 0	0.00 (1)
F- G	-1 / 0	-84.9	-84.9	0.14 (1)	10.00	Q- R	-22 / 0	0.00 (1)
G- R	-33 / 0	-84.9	-84.9	0.01 (1)	6.25			
R- H	-40 / 0	-84.9	-84.9	0.02 (1)	6.25			
H- I	0 / 13	-84.9	-84.9	0.02 (1)	10.00			
B- O	0 / 18	-18.5	-18.5	0.00 (1)	10.00			
O- N	0 / 18	-18.5	-18.5	0.03 (4)	10.00			
N- M	0 / 2	-18.5	-18.5	0.04 (4)	10.00			
M- L	0 / 2	-18.5	-18.5	0.04 (4)	10.00			
L- K	0 / 2	-18.5	-18.5	0.04 (4)	10.00			
K- J	0 / 2	-18.5	-18.5	0.04 (4)	10.00			
J- Q	0 / 18	-18.5	-18.5	0.03 (4)	10.00			
Q- H	0 / 18	-18.5	-18.5	0.00 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (E-F:1) , BC=0.04/1.00 (J-K:4) , WB=0.05/1.00 (F-K:1) , SSI=0.14/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.16 (M) (INPUT = 0.90)
JSI METAL= 0.07 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
02-28-22
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2205709

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
422291	PB5	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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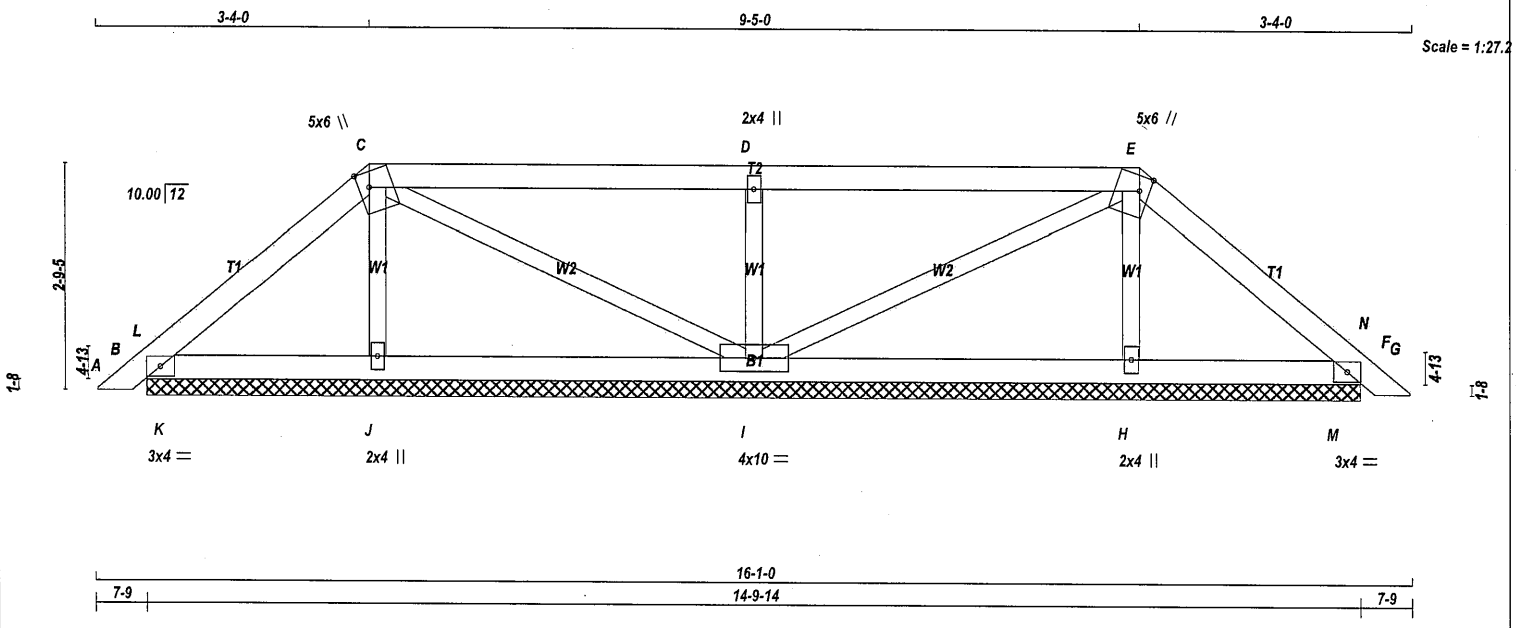

TOTAL WEIGHT = 50 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMB1-I	MT20	3.0	4.0		
H	BMW1+w	MT20	2.0	4.0		
I	BMWWW1-t	MT20	4.0	10.0		
J	BMW1+w	MT20	2.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
B	224	0	224	0	0	14-9-14	14-9-14
F	224	0	224	0	0	14-9-14	14-9-14
J	277	0	277	0	0	14-9-14	14-9-14
I	618	0	618	0	0	14-9-14	14-9-14
H	277	0	277	0	0	14-9-14	14-9-14

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	156	113 / 0	0 / 0	0 / 0	0 / 0	43 / 0	0 / 0
F	156	113 / 0	0 / 0	0 / 0	0 / 0	43 / 0	0 / 0
J	200	112 / 0	0 / 0	0 / 0	0 / 0	88 / 0	0 / 0
I	437	290 / 0	0 / 0	0 / 0	0 / 0	147 / 0	0 / 0
H	200	112 / 0	0 / 0	0 / 0	0 / 0	88 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, J, I, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 13	-84.9	-84.9	0.02 (1)	10.00	J- C	-177 / 0	0.03 (1)
B- L	-20 / 0	-84.9	-84.9	0.03 (1)	6.25	C- I	-42 / 0	0.02 (1)
L- C	-83 / 0	-84.9	-84.9	0.07 (1)	6.25	I- D	-499 / 0	0.08 (1)
C- D	-14 / 0	-84.9	-84.9	0.32 (1)	6.25	I- E	-42 / 0	0.02 (1)
D- E	-14 / 0	-84.9	-84.9	0.32 (1)	6.25	H- E	-177 / 0	0.03 (1)
E- N	-83 / 0	-84.9	-84.9	0.07 (1)	6.25	K- L	-201 / 0	0.00 (1)
N- F	-20 / 0	-84.9	-84.9	0.03 (1)	6.25	M- N	-201 / 0	0.00 (1)
F- G	0 / 13	-84.9	-84.9	0.02 (1)	10.00			
B- K	0 / 59	-18.5	-18.5	0.07 (1)	10.00			
K- J	0 / 59	-18.5	-18.5	0.07 (1)	10.00			
J- I	0 / 51	-18.5	-18.5	0.09 (4)	10.00			
I- H	0 / 51	-18.5	-18.5	0.09 (4)	10.00			
H- M	0 / 59	-18.5	-18.5	0.07 (1)	10.00			
M- F	0 / 59	-18.5	-18.5	0.07 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.32/1.00 (C-D:1) , BC=0.09/1.00 (I-J:4) , WB=0.08/1.00 (D-I:1) , SSI=0.19/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (D) (INPUT = 0.90)
JSI METAL= 0.10 (D) (INPUT = 1.00)

Structural component only
DWG# T-2205710

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
422291	PB6	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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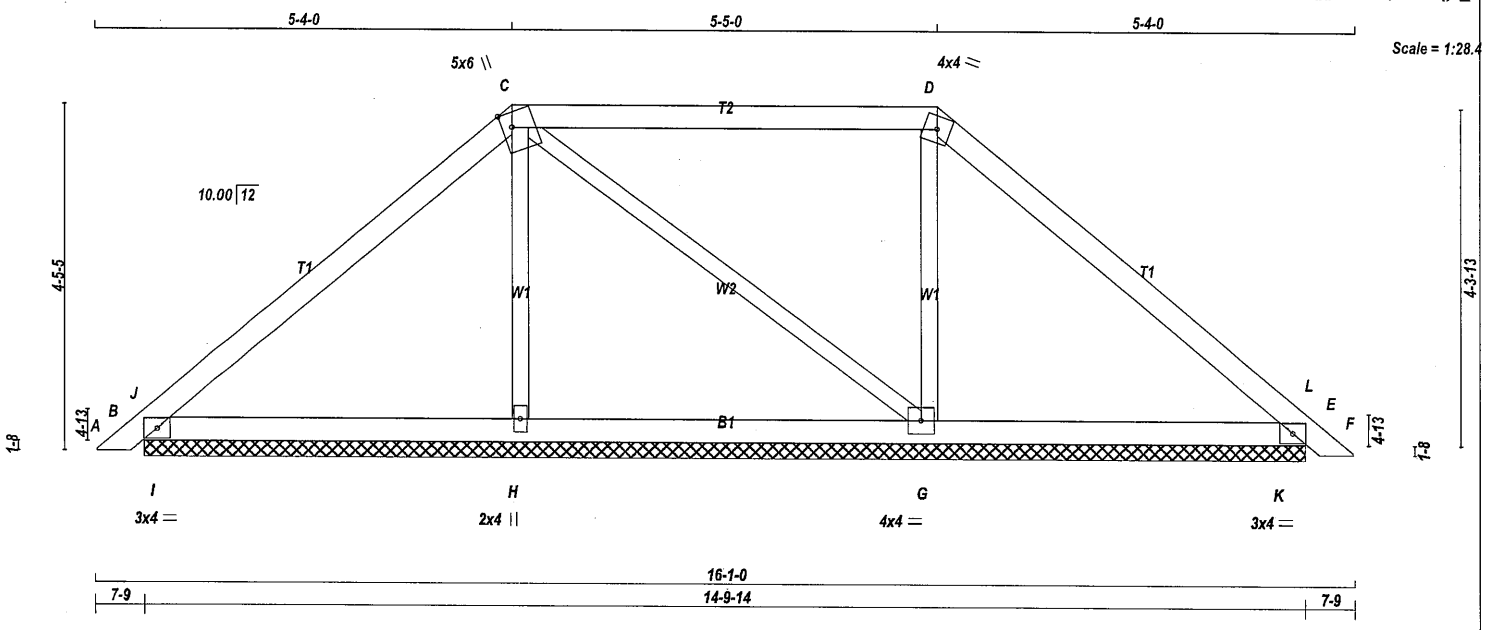

TOTAL WEIGHT = 50 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMB1-I	MT20	3.0	4.0		
G	BMWW1-t	MT20	4.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	421	0	421	0	0	14-9-14	14-9-14
E	393	0	393	0	0	14-9-14	14-9-14
H	363	0	363	0	0	14-9-14	14-9-14
G	444	0	444	0	0	14-9-14	14-9-14

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	295	207 / 0	0 / 0	0 / 0	0 / 0	88 / 0	0 / 0
E	276	191 / 0	0 / 0	0 / 0	0 / 0	85 / 0	0 / 0
H	261	148 / 0	0 / 0	0 / 0	0 / 0	113 / 0	0 / 0
G	316	193 / 0	0 / 0	0 / 0	0 / 0	123 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 13	-84.9	-84.9	0.02 (1)	10.00	H- C	-229 / 0	0.06 (1)
B- J	-52 / 7	-84.9	-84.9	0.12 (1)	6.25	C- G	-43 / 0	0.03 (1)
J- C	-237 / 0	-84.9	-84.9	0.24 (1)	6.25	G- D	-283 / 0	0.08 (1)
C- D	-132 / 0	-84.9	-84.9	0.43 (1)	6.25	I- J	-529 / 0	0.00 (1)
D- L	-194 / 0	-84.9	-84.9	0.23 (1)	6.25	K- L	-535 / 0	0.00 (1)
L- E	-43 / 54	-84.9	-84.9	0.13 (1)	6.25			
E- F	0 / 13	-84.9	-84.9	0.02 (1)	10.00			
B- I	0 / 172	-18.5	-18.5	0.20 (1)	10.00			
I- H	0 / 172	-18.5	-18.5	0.20 (1)	10.00			
H- G	0 / 166	-18.5	-18.5	0.12 (1)	10.00			
G- K	0 / 139	-18.5	-18.5	0.20 (1)	10.00			
K- E	0 / 139	-18.5	-18.5	0.20 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.43/1.00 (C-D:1) , BC=0.20/1.00 (B-I:1) , WB=0.08/1.00 (D-G:1) , SSI=0.42/1.00 (E-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.33 (B) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)

Structural component only
DWG# T-2205711

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
422291	PB7	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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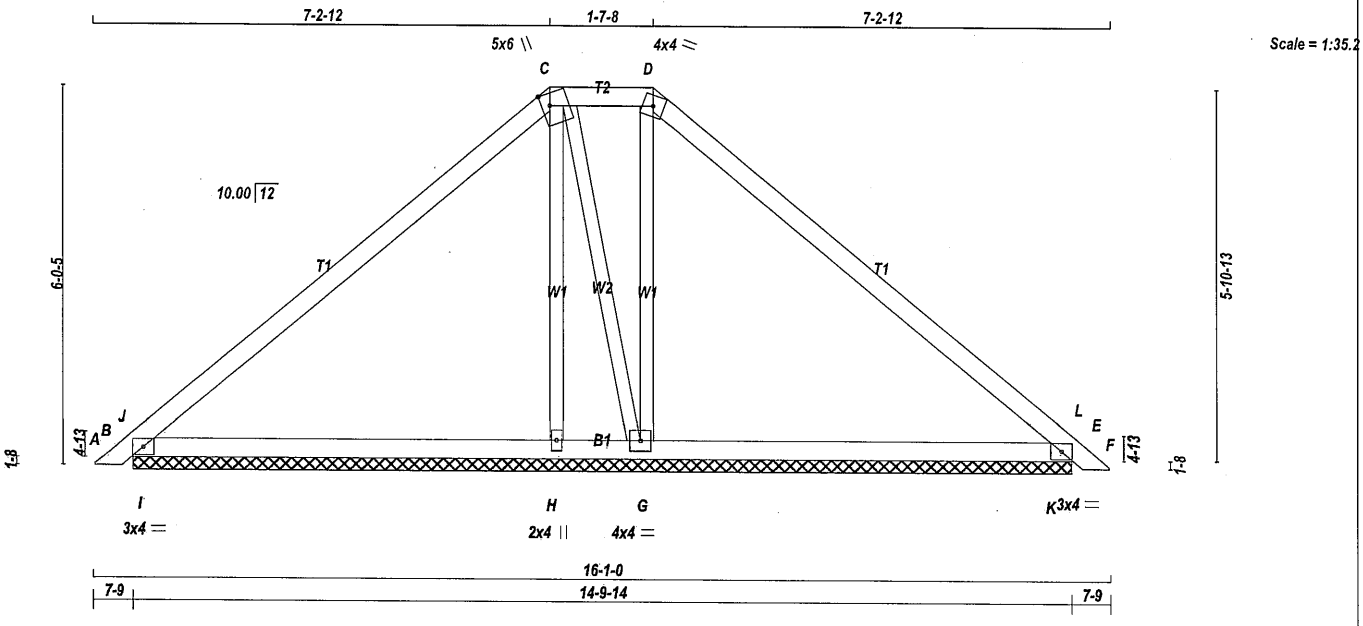

TOTAL WEIGHT = 53 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMB1-I	MT20	3.0	4.0		
G	BMWW1-t	MT20	4.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	578	0	578	0	0	14-9-14	14-9-14
E	570	0	570	0	0	14-9-14	14-9-14
H	193	0	193	0	0	14-9-14	14-9-14
G	280	0	280	0	0	14-9-14	14-9-14

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	408	276 / 0	0 / 0	0 / 0	0 / 0	131 / 0	0 / 0
E	402	271 / 0	0 / 0	0 / 0	0 / 0	131 / 0	0 / 0
H	141	67 / 0	0 / 0	0 / 0	0 / 0	73 / 0	0 / 0
G	199	125 / 0	0 / 0	0 / 0	0 / 0	74 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 13	-84.9	-84.9	0.02	(1)	10.00	H- C	-52 / 0	0.03 (1)
B- J	-79 / 225	-84.9	-84.9	0.31	(1)	6.25	C- G	-41 / 0	0.02 (1)
J- C	-356 / 0	-84.9	-84.9	0.48	(1)	6.25	G- D	-100 / 0	0.05 (1)
C- D	-246 / 0	-84.9	-84.9	0.04	(1)	6.25	I- J	-1066 / 0	0.00 (1)
D- L	-343 / 0	-84.9	-84.9	0.48	(1)	6.25	K- L	-1068 / 0	0.00 (1)
L- E	-79 / 239	-84.9	-84.9	0.31	(1)	6.25			
E- F	0 / 13	-84.9	-84.9	0.02	(1)	10.00			
B- I	0 / 258	-18.5	-18.5	0.39	(1)	10.00			
I- H	0 / 258	-18.5	-18.5	0.39	(1)	10.00			
H- G	0 / 257	-18.5	-18.5	0.28	(1)	10.00			
G- K	0 / 248	-18.5	-18.5	0.39	(1)	10.00			
K- E	0 / 248	-18.5	-18.5	0.39	(1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.48/1.00 (C-J:1) , BC=0.39/1.00 (H-I:1) , WB=0.05/1.00 (D-G:1) , SSI=0.83/1.00 (E-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.47 (B) (INPUT = 0.90)
JSI METAL= 0.14 (B) (INPUT = 1.00)

Structural component only
DWG# T-2205712

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
422291	PB8	3	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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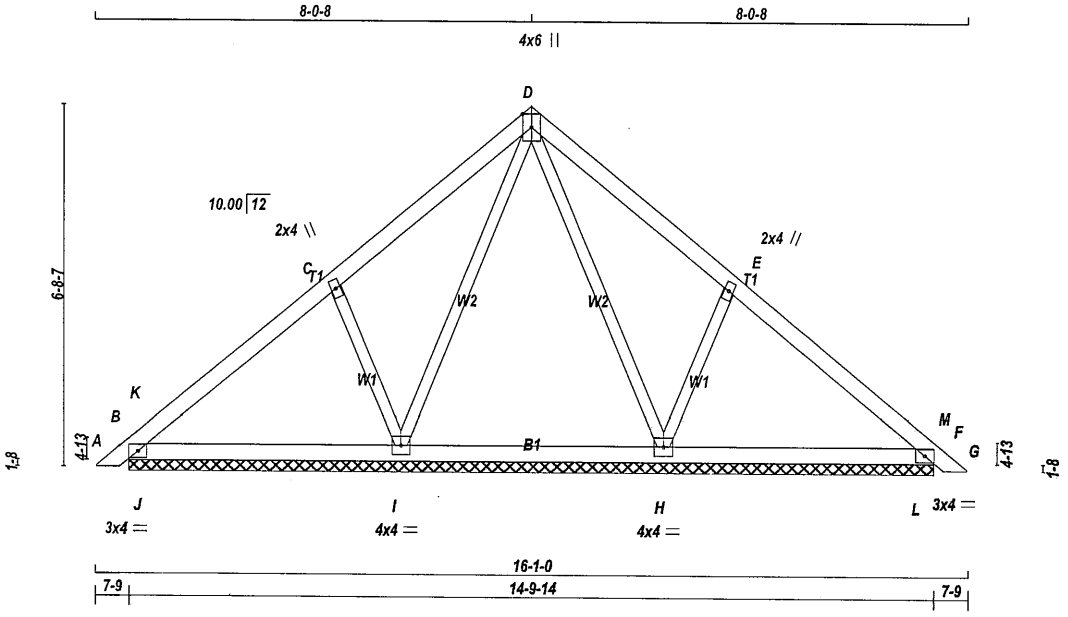

TOTAL WEIGHT = 3 X 56 = 169 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TMW+w	MT20	2.0	4.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMW+w	MT20	2.0	4.0		
F	TMB1-I	MT20	3.0	4.0		
H	BMWW1-t	MT20	4.0	4.0		
I	BMWW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	307	0	307	0	0	14-9-14	14-9-14
F	307	0	307	0	0	14-9-14	14-9-14
H	503	0	503	0	0	14-9-14	14-9-14
I	503	0	503	0	0	14-9-14	14-9-14

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	217	145 / 0	0 / 0	0 / 0	0 / 0	72 / 0	0 / 0
F	217	145 / 0	0 / 0	0 / 0	0 / 0	72 / 0	0 / 0
H	357	224 / 0	0 / 0	0 / 0	0 / 0	133 / 0	0 / 0
I	357	224 / 0	0 / 0	0 / 0	0 / 0	133 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, H, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 13	-84.9	-84.9	0.02 (1)	10.00	D- H	-123 / 0	0.10 (1)	
B- K	-187 / 0	-84.9	-84.9	0.04 (4)	6.25	H- E	-307 / 0	0.07 (1)	
K- C	-136 / 0	-84.9	-84.9	0.19 (1)	6.25	I- D	-123 / 0	0.10 (1)	
C- D	-33 / 0	-84.9	-84.9	0.19 (1)	6.25	C- I	-307 / 0	0.07 (1)	
D- E	-33 / 0	-84.9	-84.9	0.19 (1)	6.25	J- K	-80 / 70	0.00 (1)	
E- M	-136 / 0	-84.9	-84.9	0.19 (1)	6.25	L- M	-80 / 70	0.00 (1)	
M- F	-187 / 0	-84.9	-84.9	0.04 (4)	6.25				
F- G	0 / 13	-84.9	-84.9	0.02 (1)	10.00				
B- J	0 / 124	-18.5	-18.5	0.06 (1)	10.00				
J- I	0 / 124	-18.5	-18.5	0.11 (4)	10.00				
I- H	0 / 51	-18.5	-18.5	0.10 (4)	10.00				
H- L	0 / 124	-18.5	-18.5	0.11 (4)	10.00				
L- F	0 / 124	-18.5	-18.5	0.06 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.19/1.00 (C-K:1) , BC=0.11/1.00 (I-J:4) , WB=0.10/1.00 (D-H:1) , SSI=0.12/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (F) (INPUT = 0.90)
JSI METAL= 0.08 (E) (INPUT = 1.00)

Structural component only
DWG# T-2205713

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
422291	J1	12	1	ROYAL PINE HOMES TRUSS DESC.	

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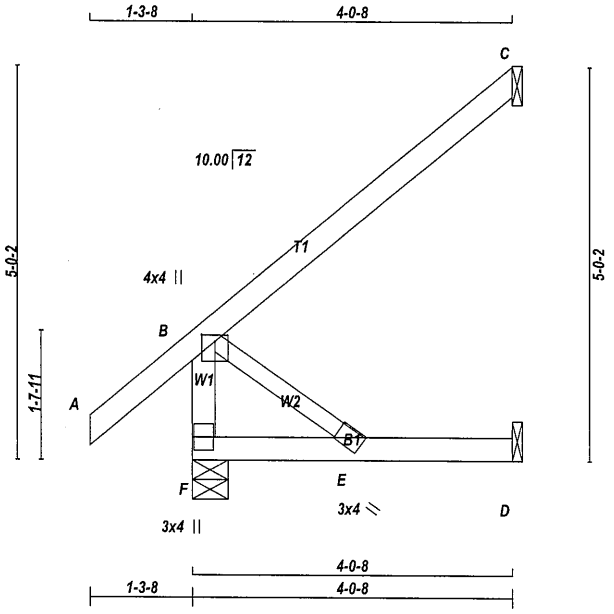

Scale = 1:28.2

TOTAL WEIGHT = 12 X 16 = 192 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMW+w	MT20	3.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	326	0	326	0	0	5-8	5-8
C	171	0	171	0	0	1-8	1-8
D	37	0	42	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	229	159 / 0	0 / 0	0 / 0	0 / 0	71 / 0	0 / 0
C	118	94 / 0	0 / 0	0 / 0	0 / 0	24 / 0	0 / 0
D	30	0 / 0	0 / 0	0 / 0	0 / 0	30 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-289 / 0	0.0	0.0	0.03 (1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 38	-84.9	-84.9	0.12 (1)	10.00			
B- C	0 / 0	-84.9	-84.9	0.24 (1)	10.00			
F- E	0 / 0	-18.5	-18.5	0.09 (4)	10.00			
E- D	0 / 0	-18.5	-18.5	0.09 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.24/1.00 (B-C:1) , BC=0.09/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.10/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

Structural component only
DWG# T-2205697

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
422291	J2	10	1	ROYAL PINE HOMES TRUSS DESC.	

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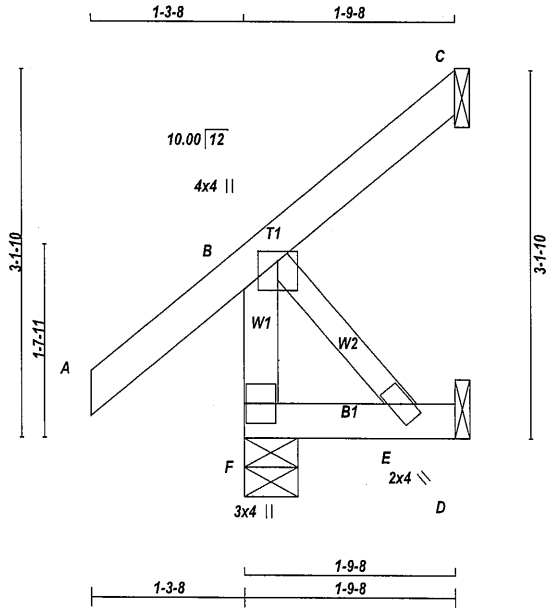

Scale = 1:18.8

TOTAL WEIGHT = 10 X 10 = 97 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	256	0	256	0	0	5-8	5-8
C	31	0	31	0	-36	1-8	1-8
D	17	0	19	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	178	131 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	21	17 / -25	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	13	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-239 / 0	0.0	0.0	0.03 (1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 38	-84.9	-84.9	0.12 (1)	10.00			
B- C	-29 / 0	-84.9	-84.9	0.11 (1)	6.25			
F- E	0 / 0	-18.5	-18.5	0.02 (4)	10.00			
E- D	0 / 0	-18.5	-18.5	0.01 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.02/1.00 (E-F:4) , WB=0.00/1.00 (B-E:1) , SSI=0.07/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

Structural component only
DWG# T-2205698

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
422291	J3	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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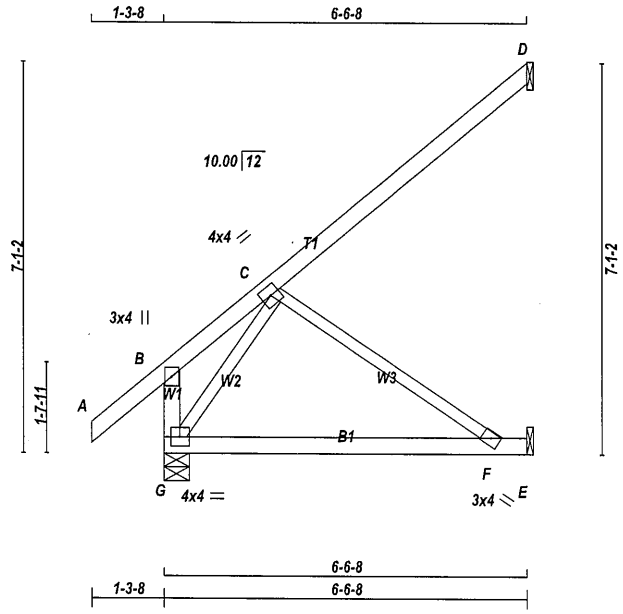

Scale = 1:40.1

TOTAL WEIGHT = 27 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
G - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMV+p	MT20	3.0	4.0	
C	TMWW-t	MT20	4.0	4.0	
F	BMW+w	MT20	3.0	4.0	
G	BMVW1-t	MT20	4.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	468	0	468	0	0	5-8	5-8
D	167	0	167	0	0	1-8	1-8
E	158	0	158	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) D , E

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	330	224 / 0	0 / 0	0 / 0	0 / 0	106 / 0	0 / 0
D	116	91 / 0	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0
E	116	55 / 0	0 / 0	0 / 0	0 / 0	61 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- B	-165 / 0	0.0	0.0		0.02 (1)	7.81	C- F	-186 / 0	0.08 (1)
A- B	0 / 38	-84.9	-84.9		0.12 (1)	10.00	G- C	-277 / 0	0.06 (1)
B- C	0 / 20	-84.9	-84.9		0.20 (1)	10.00			
C- D	-19 / 0	-84.9	-84.9		0.22 (1)	6.25			
G- F	0 / 152	-18.5	-18.5		0.25 (4)	10.00			
F- E	0 / 0	-18.5	-18.5		0.13 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 651 (0.12")

CSI: TC=0.22/1.00 (C-D:1) , BC=0.25/1.00 (F-G:4) , WB=0.08/1.00 (C-F:1) , SSI=0.14/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (G) (INPUT = 0.90)
JSI METAL= 0.07 (C) (INPUT = 1.00)

Structural component only
DWG# T-2205699

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
422291	J4	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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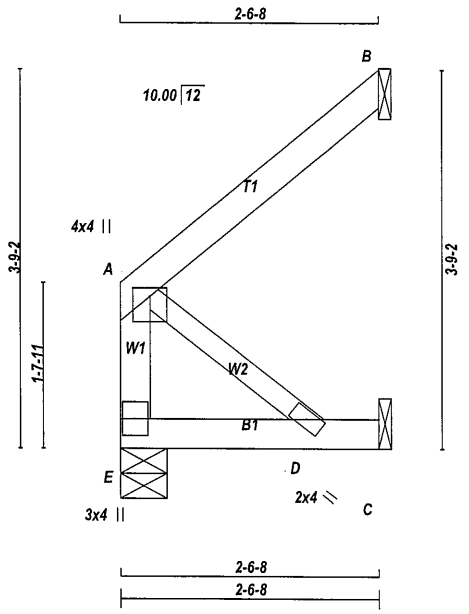

Scale = 1:22.0

TOTAL WEIGHT = 2 X 10 = 20 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
E - C	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	BMW+w	MT20	2.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	131	0	131	0	0	5-8	5-8
B	108	0	108	0	0	1-8	1-8
C	24	0	26	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) B , C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	93	59 / 0	0 / 0	0 / 0	0 / 0	34 / 0	0 / 0
B	74	59 / 0	0 / 0	0 / 0	0 / 0	15 / 0	0 / 0
C	19	0 / 0	0 / 0	0 / 0	0 / 0	19 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- A	-108 / 0	0.0	0.0	0.01 (1)	7.81	A- D	0 / 0	0.00 (1)
A- B	0 / 0	-84.9	-84.9	0.09 (1)	10.00			
E- D	0 / 0	-18.5	-18.5	0.04 (4)	10.00			
D- C	0 / 0	-18.5	-18.5	0.03 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.09/1.00 (A-B:1) , BC=0.04/1.00 (D-E:4) , WB=0.00/1.00 (A-D:1) , SSI=0.06/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.08 (A) (INPUT = 0.90)
JSI METAL= 0.02 (A) (INPUT = 1.00)

Structural component only
DWG# T-2205700

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
422291	J5	1	1	ROYAL PINE HOMES TRUSS DESC.	

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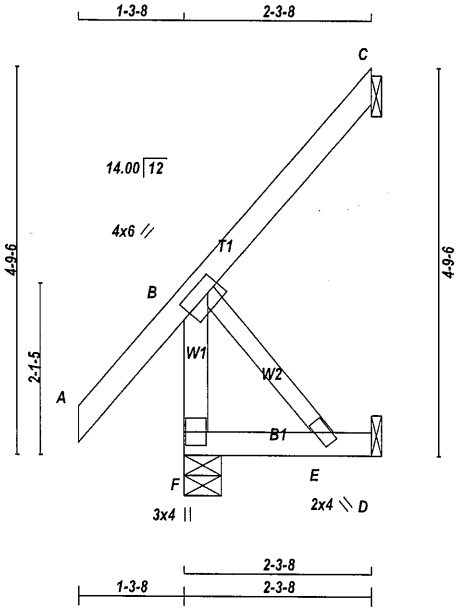

Scale = 1:27.3

TOTAL WEIGHT = 13 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	2.00	2.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	237	0	237	0	0	5-8	5-8
C	97	0	97	0	0	1-8	1-8
D	21	0	24	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	166	119 / 0	0 / 0	0 / 0	0 / 0	48 / 0	0 / 0
C	67	53 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0
D	17	0 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-216 / 0	0.0	0.0	0.03 (1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 45	-84.9	-84.9	0.13 (5)	10.00			
B- C	0 / 0	-84.9	-84.9	0.08 (1)	10.00			
F- E	0 / 0	-18.5	-18.5	0.03 (4)	10.00			
E- D	0 / 0	-18.5	-18.5	0.02 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.13/1.00 (A-B:5) , BC=0.03/1.00 (E-F:4) , WB=0.00/1.00 (B-E:1) , SSI=0.06/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.11 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)

Structural component only
DWG# T-2205701

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
422291	J6	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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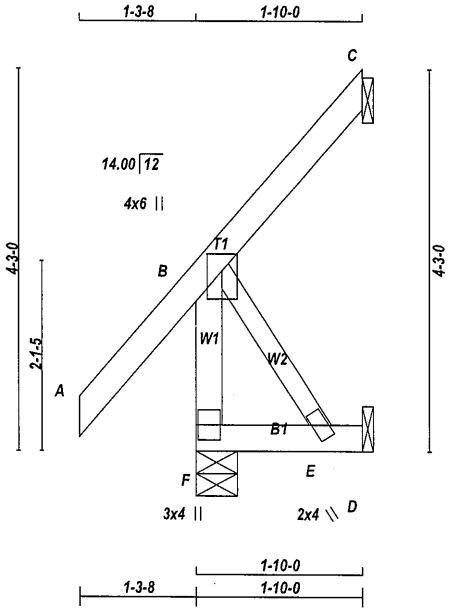

TOTAL WEIGHT = 12 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.25	2.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	259	0	259	0	0	5-8	5-8
C	32	0	32	0	-36	1-8	1-8
D	17	0	19	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	181	133 / 0	0 / 0	0 / 0	0 / 0	48 / 0	0 / 0
C	22	18 / -25	0 / 0	0 / 0	0 / 0	5 / 0	0 / 0
D	14	0 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-242 / 0	0.0	0.0	0.03 (1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 45	-84.9	-84.9	0.12 (1)	10.00			
B- C	-35 / 0	-84.9	-84.9	0.11 (1)	6.25			
F- E	0 / 0	-18.5	-18.5	0.02 (4)	10.00			
E- D	0 / 0	-18.5	-18.5	0.01 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.02/1.00 (E-F:4) , WB=0.00/1.00 (B-E:1) , SSI=0.06/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

Structural component only
DWG# T-2205702

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
422291	J7	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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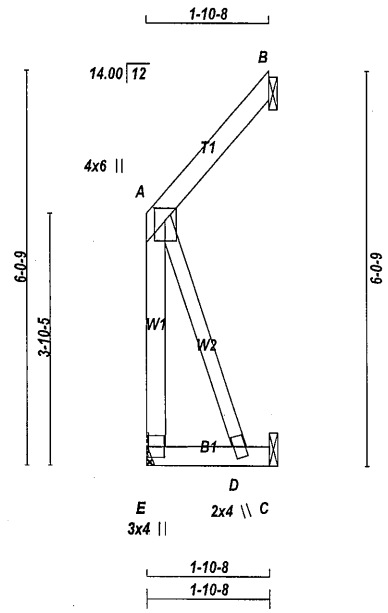

Scale = 1:33.9

TOTAL WEIGHT = 13 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
E - C	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.25	2.00
D	BMW+w	MT20	2.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	97	0	97	0	0	MECHANICAL	
B	80	0	80	0	0	1-8	1-8
C	17	0	19	0	0	1-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) B , C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	69	44 / 0	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0
B	55	44 / 0	0 / 0	0 / 0	0 / 0	11 / 0	0 / 0
C	14	0 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
E- A	-80 / 0	0.0	0.0	0.02 (1)	7.81
A- B	0 / 0	-84.9	-84.9	0.05 (1)	10.00
E- D	0 / 0	-18.5	-18.5	0.02 (4)	10.00
D- C	0 / 0	-18.5	-18.5	0.01 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- D	0 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.05/1.00 (A-B:1) , BC=0.02/1.00 (D-E:4) , WB=0.00/1.00 (A-D:1) , SSI=0.04/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.05 (A) (INPUT = 0.90)
JSI METAL= 0.02 (A) (INPUT = 1.00)

Structural component only
DWG# T-2205703

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
422291	J8	4	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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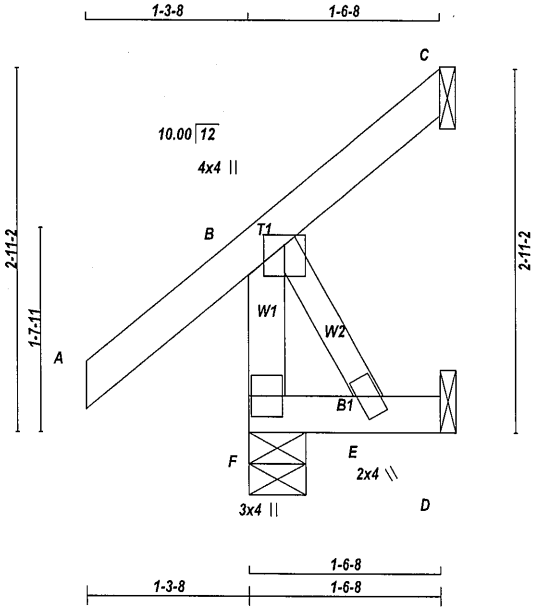

Scale = 1:17.8

TOTAL WEIGHT = 4 X 9 = 35 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	250	0	250	0	0	5-8	5-8
C	13	0	13	0	-44	1-8	1-8
D	14	0	16	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	174	129 / 0	0 / 0	0 / 0	0 / 0	45 / 0	0 / 0
C	9	7 / -29	0 / 0	0 / 0	0 / 0	2 / 0	0 / 0
D	11	0 / 0	0 / 0	0 / 0	0 / 0	11 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-236 / 0	0.0	0.0	0.03 (1)		7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 38	-84.9	-84.9	0.12 (1)		10.00			
B- C	-34 / 0	-84.9	-84.9	0.11 (1)		6.25			
F- E	0 / 0	-18.5	-18.5	0.01 (4)		10.00			
E- D	0 / 0	-18.5	-18.5	0.01 (4)		10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.01/1.00 (E-F:4) , WB=0.00/1.00 (B-E:1) , SSI=0.07/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

Structural component only
DWG# T-2205704

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
422291	J10	6	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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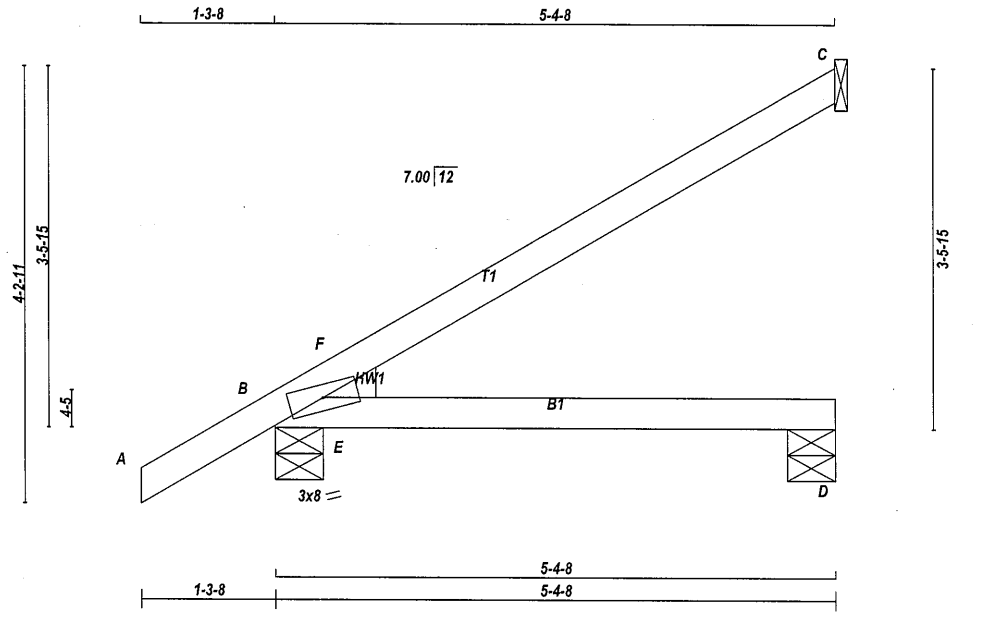

TOTAL WEIGHT = 6 X 16 = 94 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-m	MT20	3.0	8.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
C	196	0	196	0	0	1-8	1-8	
B	394	0	394	0	0	5-8	5-8	2x4 L
D	82	0	82	0	0	5-8	5-8	

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	136	104 / 0	0 / 0	0 / 0	0 / 0	33 / 0	0 / 0
B	277	189 / 0	0 / 0	0 / 0	0 / 0	88 / 0	0 / 0
D	61	22 / 0	0 / 0	0 / 0	0 / 0	39 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 28	-84.9	-84.9	0.11 (1)	10.00	E- F	-238 / 4	0.00 (1)
B- F	-23 / 4	-84.9	-84.9	0.06 (4)	6.25			
F- C	0 / 3	-84.9	-84.9	0.31 (1)	10.00			
B- E	0 / 0	-18.5	-18.5	0.24 (1)	10.00			
E- D	0 / 0	-18.5	-18.5	0.25 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 635 (0.10")

CSI: TC=0.31/1.00 (C-F:1) , BC=0.25/1.00 (D-E:1) , WB=0.00/1.00 (E-F:1) , SSI=0.20/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (B) (INPUT = 0.90)
JSI METAL= 0.03 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
02-28-22
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2205705

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
422291	C1	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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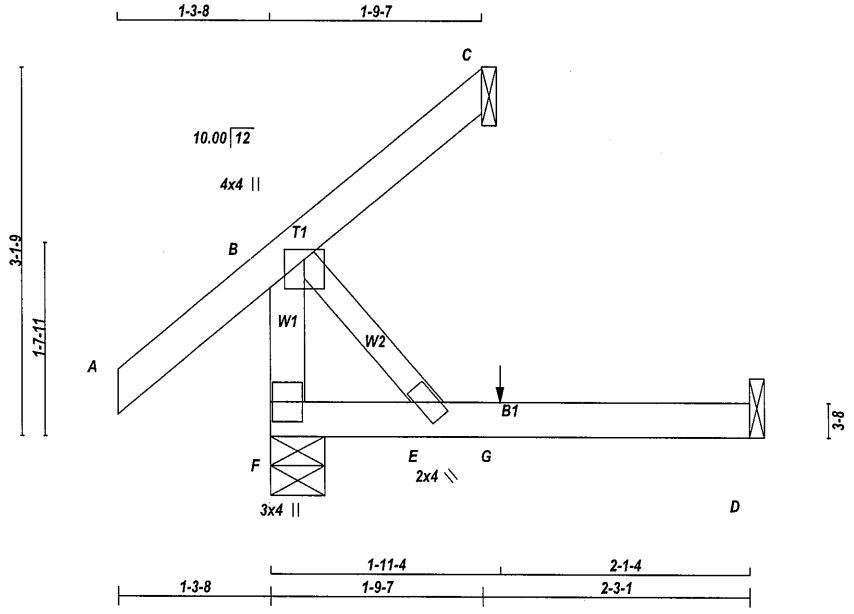

TOTAL WEIGHT = 2 X 12 = 24 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	276	0	276	0	0	5-8	5-8
C	30	0	30	0	0	1-8	1-8
D	37	0	42	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	195	131 / 0	0 / 0	0 / 0	0 / 0	64 / 0	0 / 0
C	21	17 / 0	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	30	0 / 0	0 / 0	0 / 0	0 / 0	30 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-239 / 0	0.0	0.0	0.03 (1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 38	-84.9	-84.9	0.12 (1)	10.00			
B- C	-29 / 0	-84.9	-84.9	0.11 (1)	6.25			
F- E	0 / 0	-18.5	-18.5	0.08 (4)	10.00			
E- G	0 / 0	-18.5	-18.5	0.09 (4)	10.00			
G- D	0 / 0	-18.5	-18.5	0.09 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11-4	1	1	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.12/0.80 (A-B:1) , BC=0.09/0.80 (D-E:4) , WB=0.00/0.80 (B-E:1) , SSI=0.07/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.80)
JSI METAL= 0.05 (B) (INPUT = 0.80)

Structural component only
DWG# T-2205687

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
422291	C2	4	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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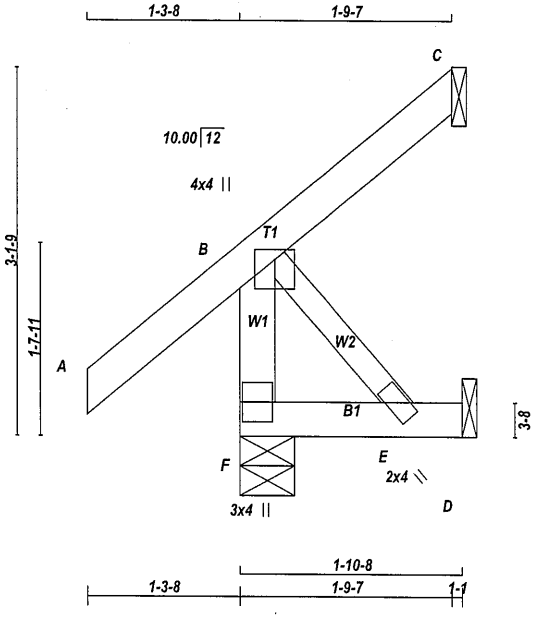

Scale = 1:18.8

TOTAL WEIGHT = 4 X 10 = 39 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	256	0	256	0	0	5-8	5-8
C	30	0	30	0	-36	1-8	1-8
D	17	0	19	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	179	131 / 0	0 / 0	0 / 0	0 / 0	48 / 0	0 / 0
C	21	17 / -25	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	14	0 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-239 / 0	0.0	0.0	0.03 (1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 38	-84.9	-84.9	0.12 (1)	10.00			
B- C	-29 / 0	-84.9	-84.9	0.11 (1)	6.25			
F- E	0 / 0	-18.5	-18.5	0.02 (4)	10.00			
E- D	0 / 0	-18.5	-18.5	0.02 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.12/0.80 (A-B:1) , BC=0.02/0.80 (E-F:4) , WB=0.00/0.80 (B-E:1) , SSI=0.07/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.80)
JSI METAL= 0.05 (B) (INPUT = 0.80)

LICENSED PROFESSIONAL ENGINEER
02-28-22
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2205688

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
422291	C3	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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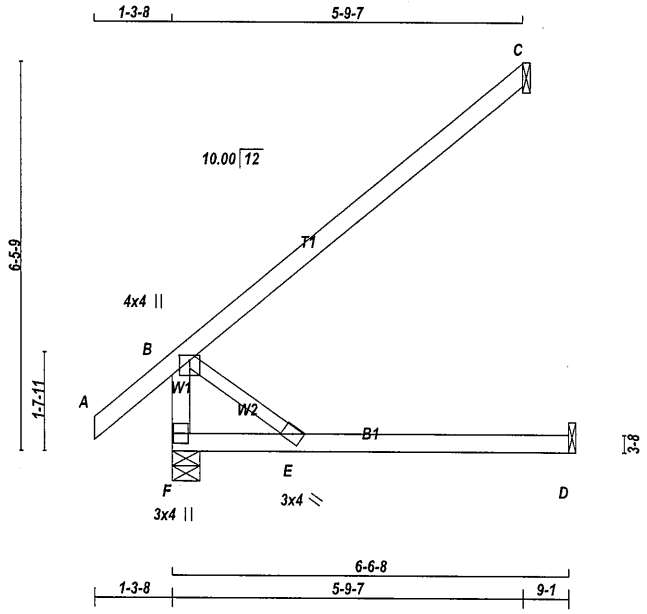

Scale = 1:36.8

TOTAL WEIGHT = 21 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMW+w	MT20	3.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	424	0	424	0	0	5-8	5-8
C	246	0	246	0	0	1-8	1-8
D	61	0	68	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	299	199 / 0	0 / 0	0 / 0	0 / 0	100 / 0	0 / 0
C	169	135 / 0	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0
D	48	0 / 0	0 / 0	0 / 0	0 / 0	48 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-363 / 0	0.0	0.0	0.04 (1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 38	-84.9	-84.9	0.12 (1)	10.00			
B- C	0 / 0	-84.9	-84.9	0.48 (1)	10.00			
F- E	0 / 0	-18.5	-18.5	0.20 (4)	10.00			
E- D	0 / 0	-18.5	-18.5	0.23 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 951 (0.08")

CSI: TC=0.48/0.80 (B-C:1) , BC=0.23/0.80 (D-E:4) , WB=0.00/0.80 (B-E:1) , SSI=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (B) (INPUT = 0.80)
JSI METAL= 0.07 (B) (INPUT = 0.80)

Structural component only
DWG# T-2205689

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
422291	C4	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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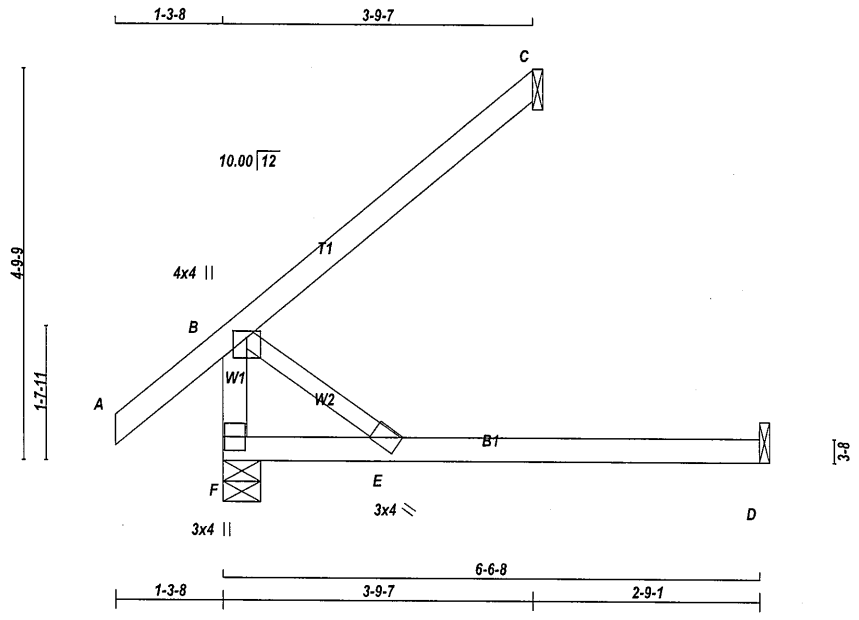

TOTAL WEIGHT = 18 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMW+w	MT20	3.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	339	0	339	0	0	5-8	5-8
C	161	0	161	0	0	1-8	1-8
D	61	0	68	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	240	153 / 0	0 / 0	0 / 0	0 / 0	88 / 0	0 / 0
C	111	88 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
D	48	0 / 0	0 / 0	0 / 0	0 / 0	48 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-278 / 0	0.0	0.0	0.03 (1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 38	-84.9	-84.9	0.12 (1)	10.00			
B- C	0 / 0	-84.9	-84.9	0.21 (1)	10.00			
F- E	0 / 0	-18.5	-18.5	0.20 (4)	10.00			
E- D	0 / 0	-18.5	-18.5	0.23 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 951 (0.08")

CSI: TC=0.21/0.80 (B-C:1) , BC=0.23/0.80 (D-E:4) , WB=0.00/0.80 (B-E:1) , SSI=0.10/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (B) (INPUT = 0.80)
JSI METAL= 0.06 (B) (INPUT = 0.80)

Structural component only
DWG# T-2205690

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
422291	C5	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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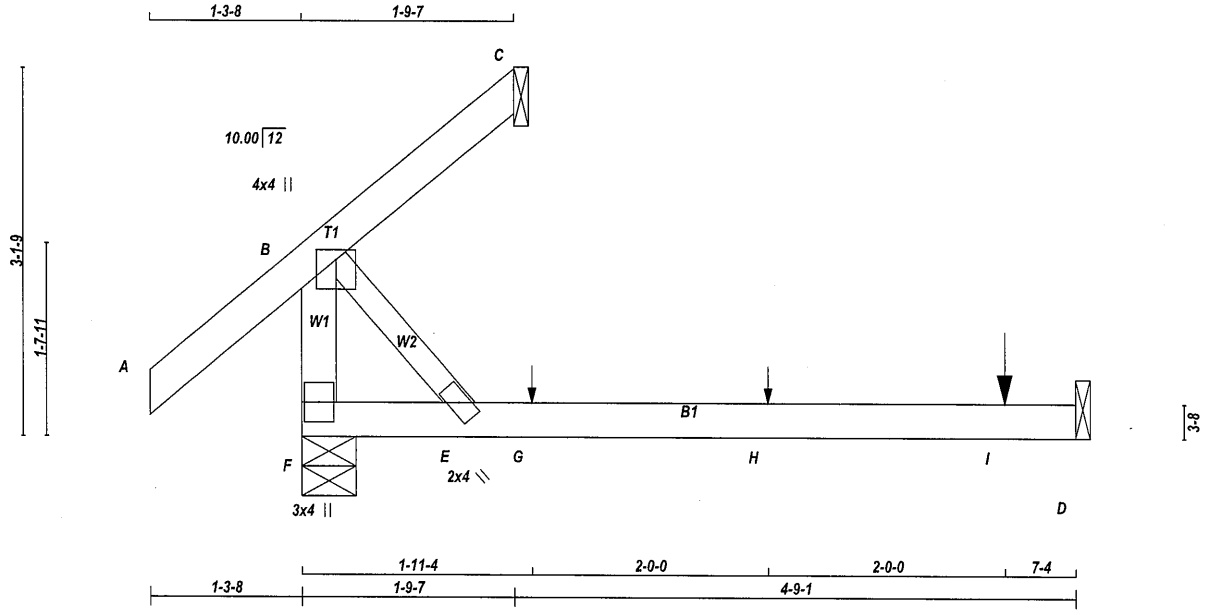

TOTAL WEIGHT = 15 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	300	0	300	0	0	5-8	5-8
C	30	0	30	0	0	1-8	1-8
D	63	0	70	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	213	131 / 0	0 / 0	0 / 0	0 / 0	82 / 0	0 / 0
C	21	17 / 0	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	50	0 / 0	0 / 0	0 / 0	0 / 0	50 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-239 / 0	0.0	0.0	0.03 (1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 38	-84.9	-84.9	0.13 (1)	10.00			
B- C	-29 / 0	-84.9	-84.9	0.09 (1)	6.25			
F- E	0 / 0	-18.5	-18.5	0.15 (4)	10.00			
E- G	0 / 0	-18.5	-18.5	0.23 (4)	10.00			
G- H	0 / 0	-18.5	-18.5	0.23 (4)	10.00			
H- I	0 / 0	-18.5	-18.5	0.23 (4)	10.00			
I- D	0 / 0	-18.5	-18.5	0.23 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11-4	1	1	---	BACK	VERT	TOTAL	---	C1
H	3-11-4	1	1	---	BACK	VERT	TOTAL	---	C1
I	5-11-4	-2	-2	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 942 (0.08")

CSI: TC=0.13/0.80 (A-B:1) , BC=0.23/0.80 (D-E:4) , WB=0.00/0.80 (B-E:1) , SSI=0.08/1.00 (D-E:4)

DOL LUMBER=0.92 NAIL=0.92 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (B) (INPUT = 0.80)
JSI METAL= 0.05 (B) (INPUT = 0.80)

Structural component only
DWG# T-2205691

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
422291	C6	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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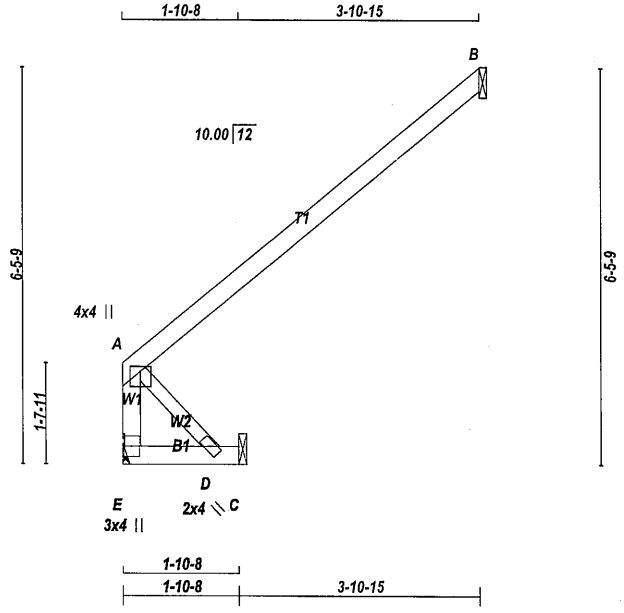

TOTAL WEIGHT = 14 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
E - C	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	BMW+w	MT20	2.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	263	0	263	0	0	MECHANICAL	
B	246	0	246	0	0	1-8	1-8
C	17	0	19	0	0	1-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) B , C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	183	135 / 0	0 / 0	0 / 0	0 / 0	49 / 0	0 / 0
B	169	135 / 0	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0
C	14	0 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF) CSI (LC)	MAX. UNBRAC LENGTH
E- A	-246 / 0	0.0	0.0	0.03 (1)	7.81
A- B	0 / 0	-84.9	-84.9	0.48 (1)	10.00
E- D	0 / 0	-18.5	-18.5	0.02 (4)	10.00
D- C	0 / 0	-18.5	-18.5	0.01 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- D	0 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.48/0.80 (A-B:1) , BC=0.02/0.80 (D-E:4) , WB=0.00/0.80 (A-D:1) , SSI=0.15/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (A) (INPUT = 0.80)
JSI METAL= 0.05 (A) (INPUT = 0.80)

Structural component only
DWG# T-2205692

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
422291	C7	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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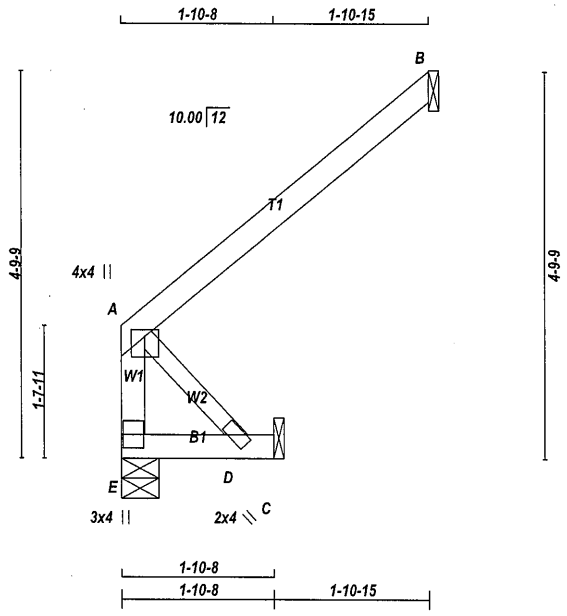

Scale = 1:27.4

TOTAL WEIGHT = 11 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
E - C	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	BMW+w	MT20	2.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	178	0	178	0	0	5-8	5-8
B	161	0	161	0	0	1-8	1-8
C	17	0	19	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) B , C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	125	88 / 0	0 / 0	0 / 0	0 / 0	37 / 0	0 / 0
B	111	88 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
C	14	0 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- A	-161 / 0	0.0	0.0		0.02 (1)	7.81	A- D	0 / 0	0.00 (1)
A- B	0 / 0	-84.9	-84.9		0.21 (1)	10.00			
E- D	0 / 0	-18.5	-18.5		0.02 (4)	10.00			
D- C	0 / 0	-18.5	-18.5		0.01 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.21/0.80 (A-B:1) , BC=0.02/0.80 (D-E:4) , WB=0.00/0.80 (A-D:1) , SSI=0.10/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.12 (A) (INPUT = 0.80)
JSI METAL= 0.03 (A) (INPUT = 0.80)

Structural component only
DWG# T-2205693

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
422291	C8	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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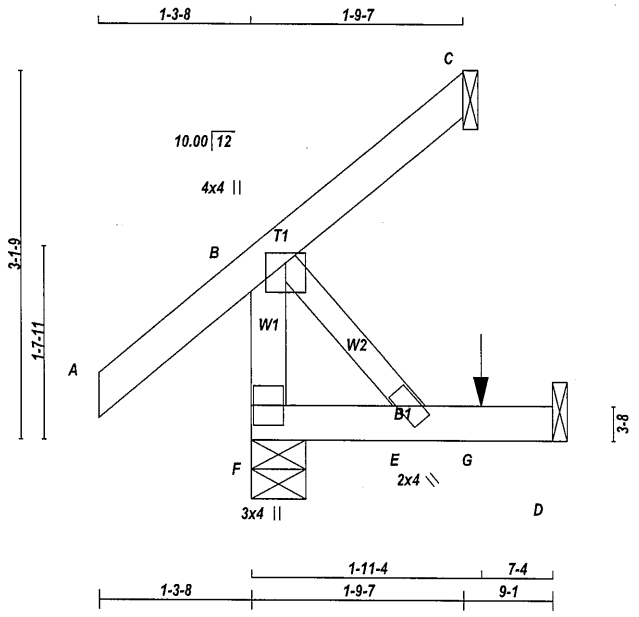

TOTAL WEIGHT = 10 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	263	0	263	0	0	5-8	5-8
C	30	0	30	0	-36	1-8	1-8
D	25	0	28	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	184	131 / 0	0 / 0	0 / 0	0 / 0	53 / 0	0 / 0
C	21	17 / -25	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	20	0 / 0	0 / 0	0 / 0	0 / 0	20 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-239 / 0	0.0	0.0	0.03 (1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 38	-84.9	-84.9	0.13 (5)	10.00			
B- C	-29 / 0	-84.9	-84.9	0.12 (5)	6.25			
F- E	0 / 0	-18.5	-18.5	0.04 (4)	10.00			
E- G	0 / 0	-18.5	-18.5	0.04 (4)	10.00			
G- D	0 / 0	-18.5	-18.5	0.04 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11-4	-2	-2	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.13/0.80 (A-B:5) , BC=0.04/0.80 (E-F:4) , WB=0.00/0.80 (B-E:1) , SSI=0.08/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.80)
JSI METAL= 0.05 (B) (INPUT = 0.80)

Structural component only
DWG# T-2205694

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
422291	C9	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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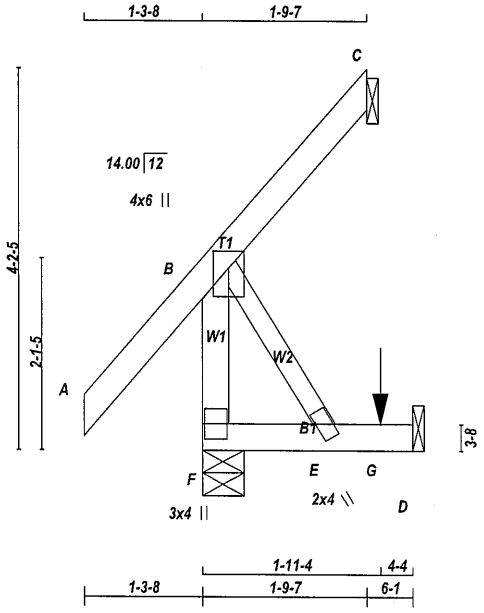

Scale = 1:24.3

TOTAL WEIGHT = 12 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.25	2.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	263	0	263	0	0	5-8	5-8
C	29	0	29	0	-37	1-8	1-8
D	25	0	28	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	184	133 / 0	0 / 0	0 / 0	0 / 0	52 / 0	0 / 0
C	20	16 / -26	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	20	0 / 0	0 / 0	0 / 0	0 / 0	20 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-242 / 0	0.0	0.0	0.03 (1)		7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 45	-84.9	-84.9	0.13 (5)		10.00			
B- C	-35 / 0	-84.9	-84.9	0.12 (5)		6.25			
F- E	0 / 0	-18.5	-18.5	0.03 (4)		10.00			
E- G	0 / 0	-18.5	-18.5	0.03 (4)		10.00			
G- D	0 / 0	-18.5	-18.5	0.03 (4)		10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11-4	-4	-4	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.13/0.80 (A-B:5) , BC=0.03/0.80 (E-F:4) , WB=0.00/0.80 (B-E:1) , SSI=0.07/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (B) (INPUT = 0.80)
JSI METAL= 0.06 (B) (INPUT = 0.80)

Structural component only
DWG# T-2205695

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
422291	C10	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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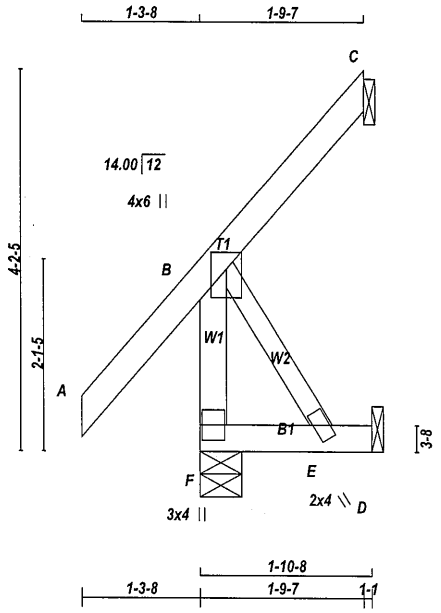


Scale = 1:24.3

TOTAL WEIGHT = 12 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.25	2.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	259	0	259	0	0	5-8	5-8
C	29	0	29	0	-37	1-8	1-8
D	17	0	19	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	181	133 / 0	0 / 0	0 / 0	0 / 0	48 / 0	0 / 0
C	20	16 / -26	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	14	0 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-242 / 0	0.0	0.0		0.03 (1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 45	-84.9	-84.9		0.12 (1)	10.00			
B- C	-35 / 0	-84.9	-84.9		0.11 (1)	6.25			
F- E	0 / 0	-18.5	-18.5		0.02 (4)	10.00			
E- D	0 / 0	-18.5	-18.5		0.02 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.02/1.00 (E-F:4) , WB=0.00/1.00 (B-E:1) , SSI=0.06/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

Structural component only
DWG# T-2205696

Alves Engineering Services Inc.
5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

LICENSED PROFESSIONAL ENGINEER
06/11/18
H.J.G. ALVES
PROVINCE OF ONTARIO

RESPONSABILITIES

1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components

2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.

3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.

4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.

5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.

2- Lumber is to be the sizes and grade specified on the truss drawing.

3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.

4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings

5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.

6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)

7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.

8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering tem and General Safety notes.

T-1900218 Feb 09, 2018

MiTek®

STANDARD DETAIL MSD2015-H

Issued: **SEPTEMBER 22, 2020**
Expiry: **APRIL 30, 2022**

TOE-NAIL CAPACITY DETAILS

LATERAL AND WITHDRAWAL RESISTANCE OF BEARING ANCHORAGE BY TOE-NAILS

NAIL TYPE	Length (in)	Diameter (in)	LATERAL RESISTANCE PER NAIL (LBS)		WITHDRAWAL RESISTANCE PER NAIL (LBS)	
			SPF	D. FIR	SPF	D. FIR
COMMON WIRE	3.00	0.144	122	139	30	42
	3.25	0.144	127	144	32	45
	3.50	0.160	152	173	38	52
COMMON SPIRAL	3.00	0.122	96	108	26	36
	3.25	0.122	97	108	28	40
	3.50	0.152	142	161	36	50
3.25" Gun nail	3.25	0.120	94	105	28	39

Note: If using truss with D. Fir lumber and SPF bearing plate, use tabulated SPF values in table.

Nail type:	Common wire	Common spiral	Common wire	Common spiral	Gun Nail
Diameter (in.)	0.160	0.152	0.144	0.122	0.120
Length (in.)	3.50	3.50	3.00	3.00	3.25
LUMBER	MAXIMUM NUMBER OF TOE NAILS				
2x4 SPF	2	2	3	3	3
2x6 SPF	4	4	4	5	5
2x4 D. FIR	2	2	2	2	2
2x6 D. FIR	3	3	3	4	4

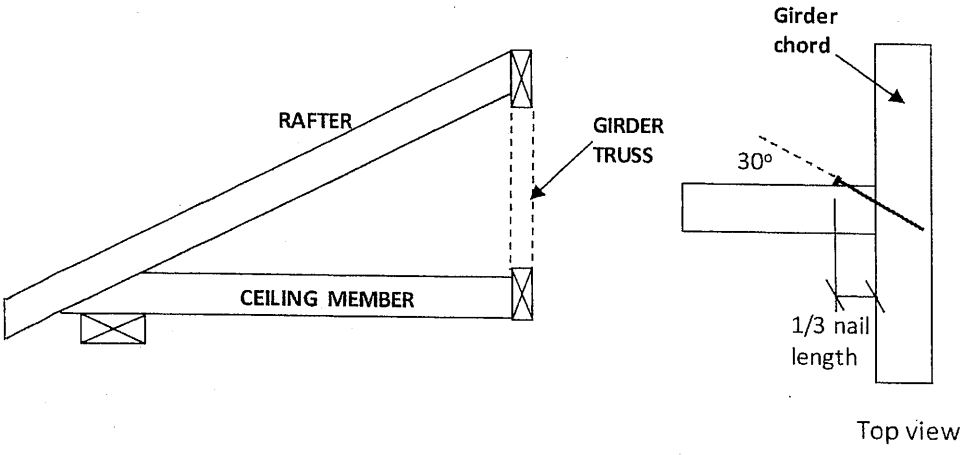


Figure 1: Toe-Nailing Rafter / Ceiling Member to Girder Truss

PEO
Certificate No. 10889485
LICENSED PROFESSIONAL ENGINEER
C. Cordogiannis
90553736
PROVINCE OF ONTARIO

December 21, 2020

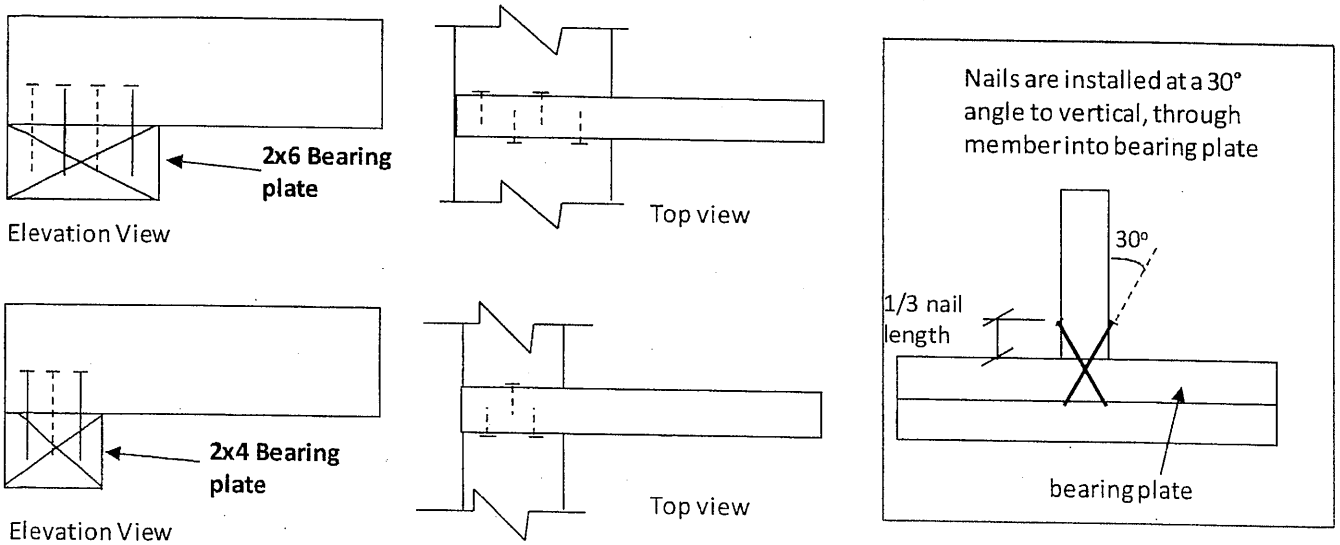
MiTek®

STANDARD DETAIL MSD2015-H

Issued: **SEPTEMBER 22, 2020**
Expiry: **APRIL 30, 2022**

TOE-NAIL CAPACITY DETAILS

Figure 2: Toe-Nail Anchorage to Bearing Plate for Uplift



NOTES:

1. Rafter and ceiling members may be connected to top and bottom chords of girder truss by toe-nailing the members into the girder chords (see fig. 1), provided the factored vertical reactions of the supported members do not exceed the lateral resistance of the toe-nails. Mechanical connectors (hangers) are required if factored vertical reactions exceed the toe-nail capacity, or if the connection must resist horizontal loads (loads perpendicular to the face of girder or rafter).

2. Trusses, rafters or ceiling members may be anchored to the bearing plate with toe-nails (see fig. 2), provided that the factored uplift reactions due to **wind or earthquake loads** do not exceed the **withdrawal resistance of the toe-nails.** Mechanical anchors (tie-downs) are required for reactions that exceed the toe-nail withdrawal capacity. Toe-nail anchorage to bearing plates is **NOT** permitted if uplift reactions are generated from gravity loads (snow, floor live, dead).

3. Tabulated toe-nail resistances on page 1 are for **one** toe-nail. Multiply unit values by the number of nails used in the connection. Maximum number of nails in a connection shall not exceed the tabulated limits shown on page 1 for a given lumber size /species.

4. Nail values are based on specific gravity of G = 0.42 (SPF) and G = 0.49 (D. Fir).

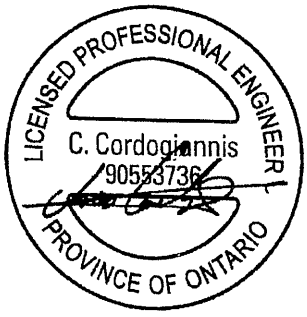
5. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member.

6. For wind / earthquake loads, tabulated lateral resistances may be multiplied by 1.15 (K_D factor). No increases are permitted for tabulated withdrawal resistances.

7. Lumber must be dry (< 19% moisture content) at the time of nail installation.

8. Nail values in this table comply with CSA O86-19, Clause 12.9.

PEO
Certificate No. 10889485



December 21, 2020

LUL/LUS/LJS/HUS/HHUS/HGUS

Standard and Double-Shear Joist Hangers

This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 217–218.

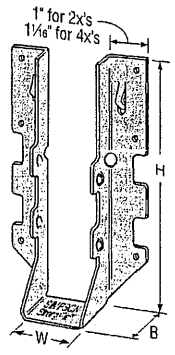
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 18–20.

Installation:

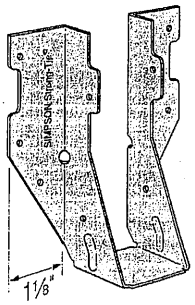
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

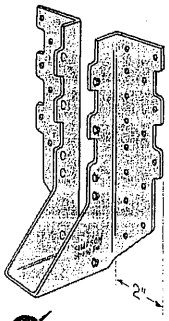
- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on pp. 105–107.



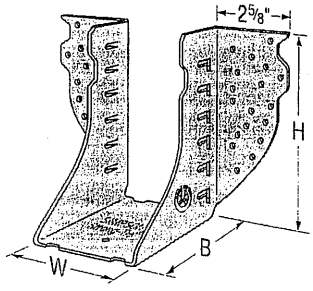
LUS28



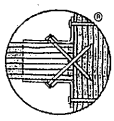
LU26L



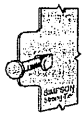
HUS210
(HUS26, HUS28, and HHUS similar)



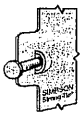
HGUS28-2



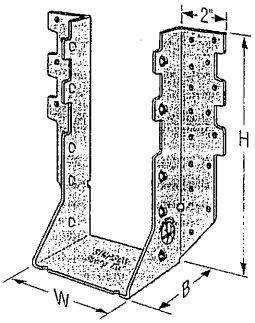
Double-Shear Nailing Top View



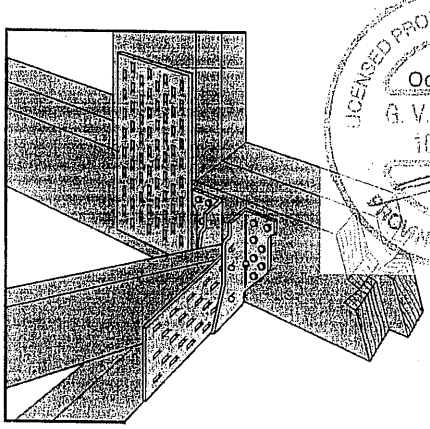
Double-Shear Nailing Side View; Do not bend tab



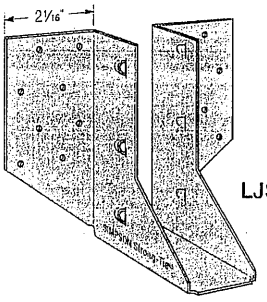
Dome Double-Shear Nailing Side View *(available on some models)*



HHUS210-2



Typical HUS26 Installation with Reduced Heel Height
(Truss Designer to provide fastener quantity for connecting multiple members together)



LJS26DS

TECHNICAL BULLETIN

LUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

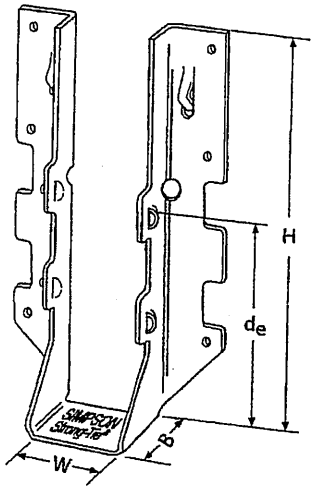
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

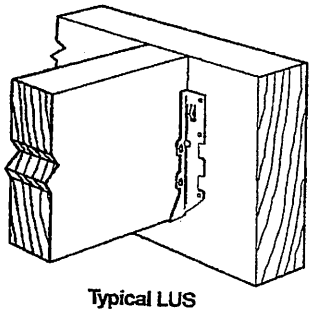
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified

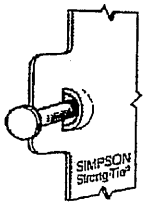


LUS28


Typical LUS Installation

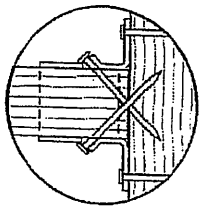
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LUS24	18	1 9/16	3 1/8	1 3/4	1 15/16	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 13/16	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1 9/16	4 3/4	1 3/4	3 5/8	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 7/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4 5/8	4 9/16	2	3 1/4	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 9/16	6 5/8	1 3/4	3 3/4	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4 5/8	6 1/4	2	3 1/4	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 9/16	7 13/16	1 3/4	3 7/8	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4 5/8	8 3/16	2	5 1/4	(8) 16d	(6) 16d	2580	3345	2320	2375

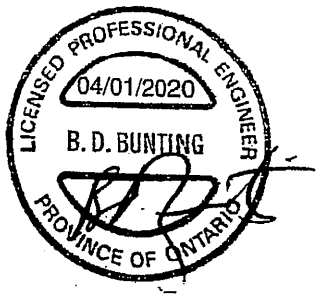
1. d_e is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580


Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HUS/LJS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

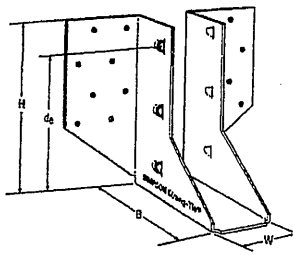
- Factored resistances are in accordance with CSA O86 -14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

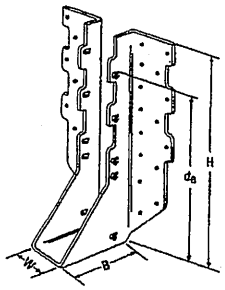
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

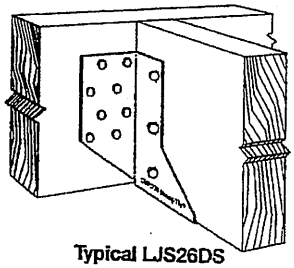
- See current catalogue for options



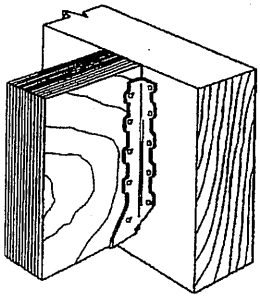
LJS26DS



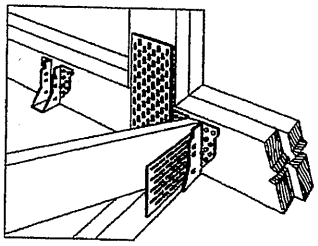
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS Installation



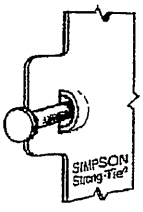
Typical HUS Installation



Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

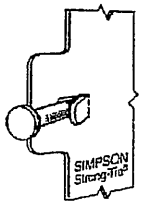
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
								lb.	lb.	lb.	lb.
LJS26DS	18	1 9/16	5	3½	4⅝	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1⅝	5⅜	3	3 15/16	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1⅝	7 3/32	3	6 3/32	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1⅝	9 3/32	3	7 31/32	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1 13/16	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

1. d_e is the distance from the seat of the hanger to the highest joist nail.

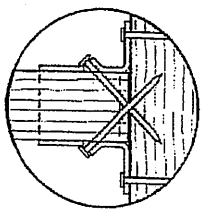


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

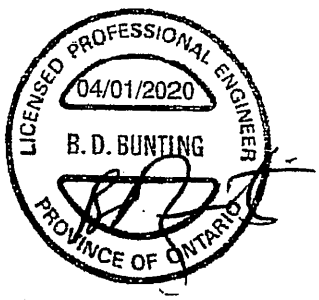
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT STATES DESIGN

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(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

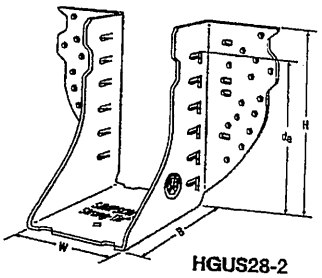
All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

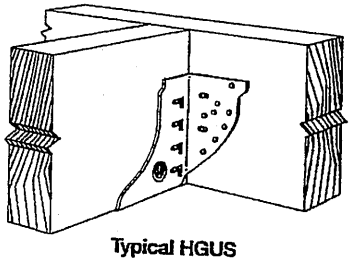
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.



HGUS28-2

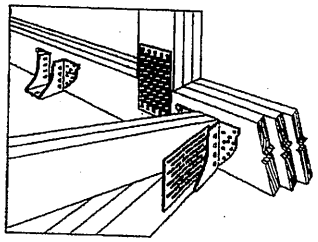
Installation:

- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications


Typical HGUS Installation

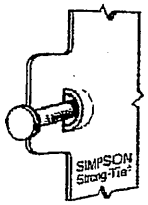
Options:

- See current catalogue for options


Typical HGUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
HGUS26	12	1⅝	5⅜	5	4 5/32	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3 5/16	5 7/16	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4 15/16	5½	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6 5/16	5 7/16	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1⅝	7⅛	5	6⅛	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3 5/16	7 3/16	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4 15/16	7¼	4	6⅜	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6 5/16	7 3/16	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210	12	1⅝	9⅛	5	7⅞	(46) 16d	(16) 16d	3535	11070	2510	8090
HGUS210-2	12	3 5/16	9 3/16	4	8⅛	(46) 16d	(16) 16d	6840	14015	4855	10270
HGUS210-3	12	4 15/16	9¼	4	8⅜	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6 5/16	9 3/16	4	8⅛	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6 5/16	10⅝	4	10⅛	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6 5/16	12⅝	4	11⅛	(66) 16d	(22) 16d	10130	16400	7195	11645

1. d_e is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).

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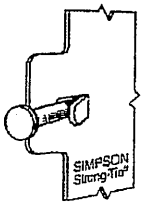
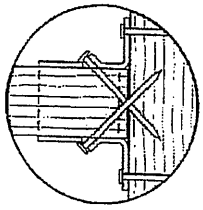
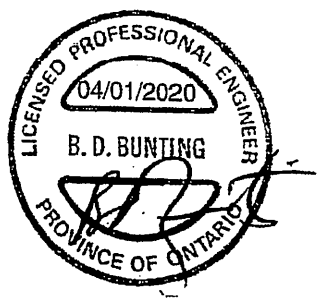


Double Shear Nailing Side View. Do not bend tab back.


Double Shear Nailing Top View.



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strongtie.com

ONTARIO WOOD TRUSS FABRICATORS ASSOCIATION

TECH-NOTES

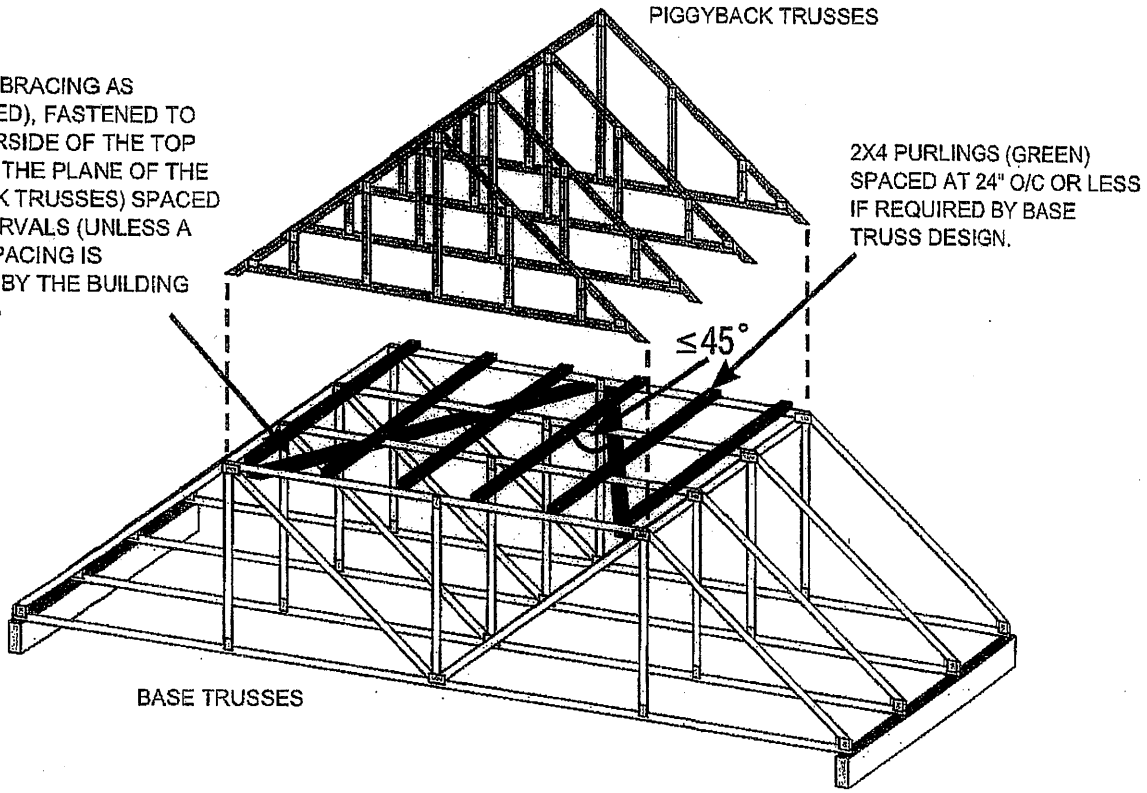
TN 15-001
Piggyback Bracing

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:

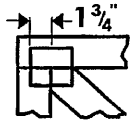


NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

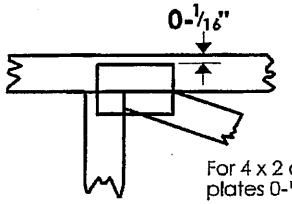
SKETCH FROM BCSI-CANADA 2013

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

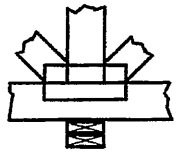
4 X 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

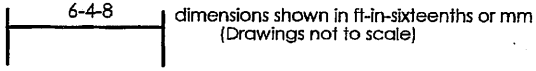
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses

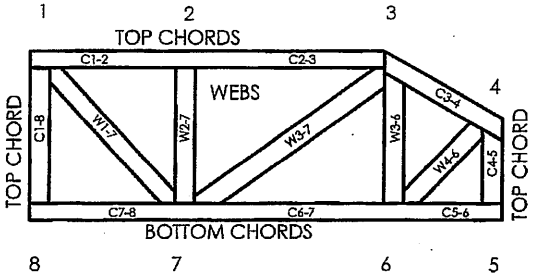
DSB-89: Design Standard for Bracing.

BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

MiTek®

POWER TO PERFORM.™

⚠ General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.