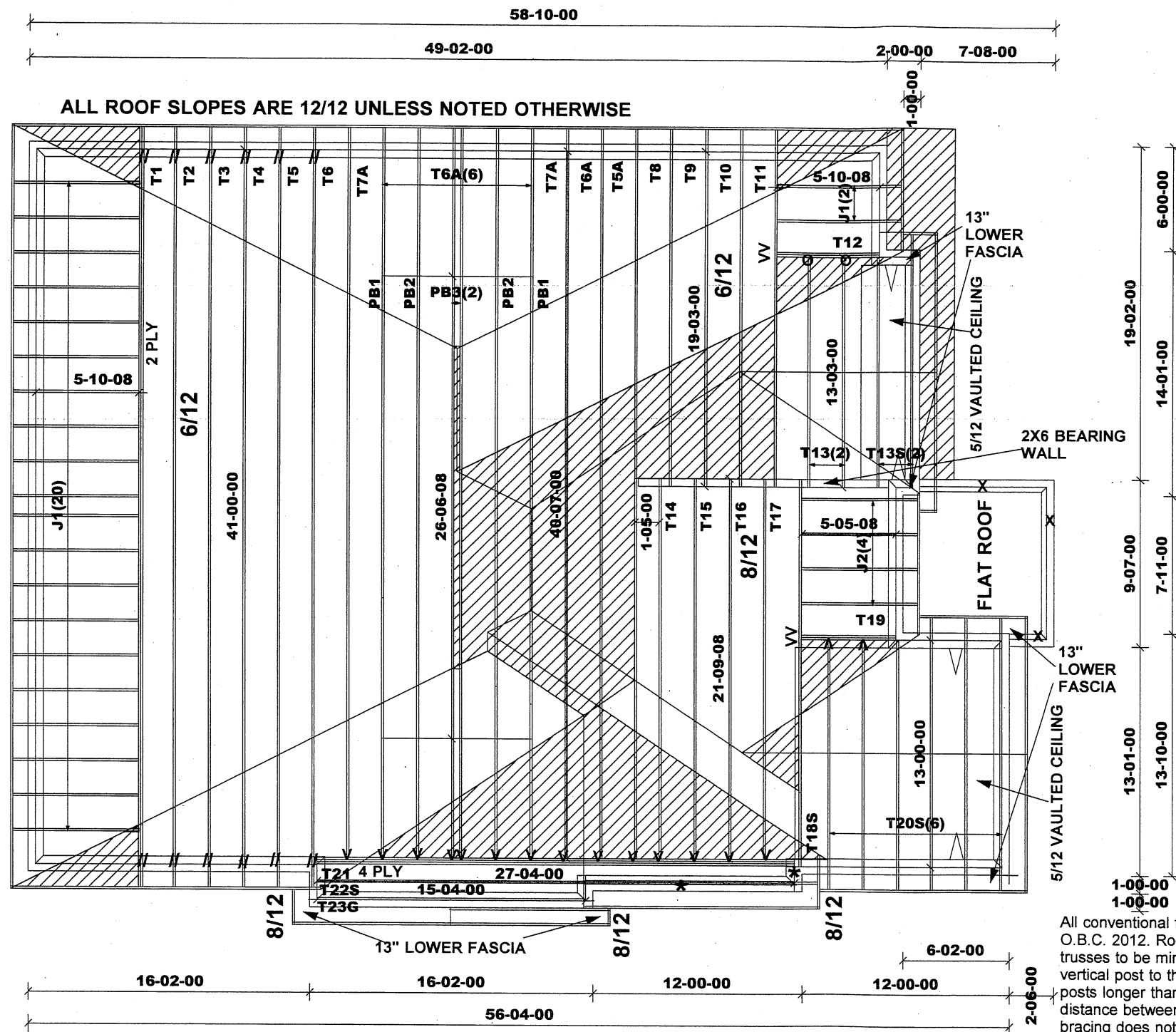




ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

DESIGN CONFORMS WITH OBC 2012
(2019 AMENDMENT)
OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 27.2 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 23.3 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

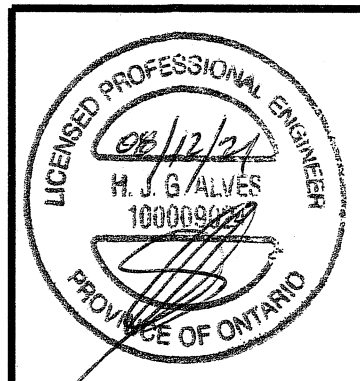
HARDWARE:
LUS24 - (O)
LJS26DS - (V)
LUS26-2 - (VV)
H2.5A - (I)

DENOTES:
* : 12" RAISED TOP OF PLATE
X: DROPPED BEAM BY OTHERS

DWG# T-2126619 to T-2126648, T-2126664,
T1800218

DENOTES:
CONVENTIONAL
FRAMING

All conventional framing to conform with Part 9 of
O.B.C. 2012. Roof rafters that cross over or meet
trusses to be min. 2x4 SPF #2 @ 24" o/c with a
vertical post to the truss at each cross point. Vertical
posts longer than 6' to have lateral bracing so that the
distance between the post end points and lateral
bracing does not exceed 6'.



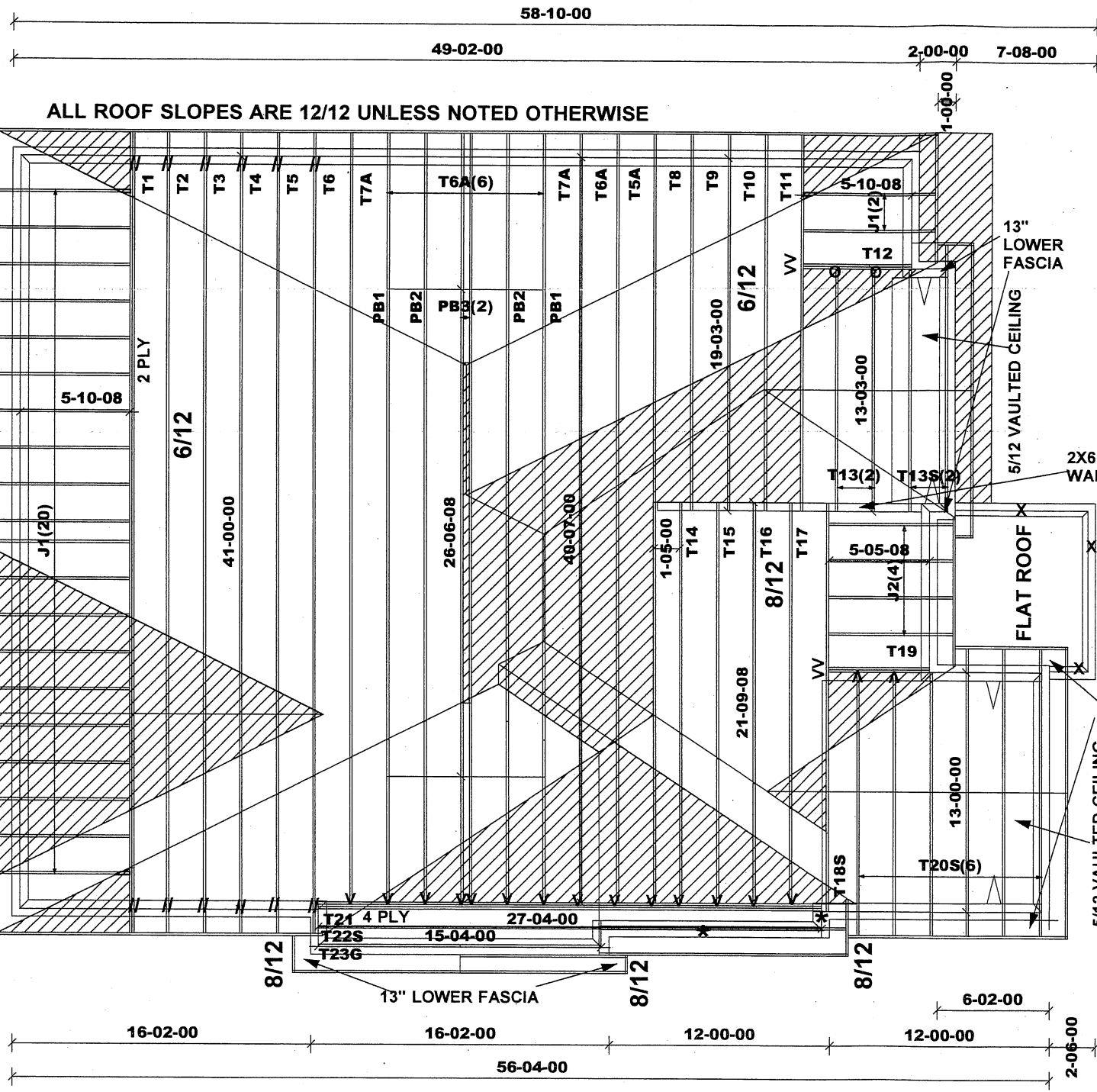
DWG NO. TAM **T-212777**
STRUCTURAL
COMPONENT ONLY

TRUSS PLACEMENT PLAN.

This is a truss placement plan only, NOT a final roof framing plan. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual truss drawings for each component identified on this placement plan.

Please verify that all dimensions match the dimensions found on the job. The Building designer is responsible for the temporary/permanent bracing of the roof and floor system and its integration into the bracing of the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer unless otherwise noted in this plan. Building designer to review and approve this plan to ascertain conformity to his overall structural plan.

	Job Track: 52430	Builder / Location: ROYAL PINE HOMES / BRAMPTON	Model / Elevation: UNIT 5003 (CORNER) / A STD OR OPT
	Layout ID: 420538	Project: VALES OF CASTLEMORE NORTH	THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.
	Plan Log: 205064	Date: 2021-08-10	Designer: ND
	M140225		



ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

DESIGN CONFORMS WITH OBC 2012
(2019 AMENDMENT)
OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 27.2 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 23.3 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
LUS26-2 - (VV)
H2.5A - (J)

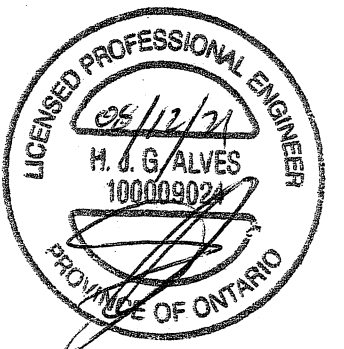
DENOTES:
* : 12" RAISED TOP OF PLATE
X: DROPPED BEAM BY OTHERS

NO CHANGE TO TRUSSES
SAME AS STANDARD
ND. 07/27/2021

DENOTES:
CONVENTIONAL
FRAMING

All conventional framing to conform with Part 9 of
O.B.C. 2012. Roof rafters that cross over or meet
trusses to be min. 2x4 SPF #2 @ 24" o/c with a
vertical post to the truss at each cross point. Vertical
posts longer than 6' to have lateral bracing so that the
distance between the post end points and lateral
bracing does not exceed 6'.

DWG# T-2126619 to T-2126648, T-2126664,
T1800218



DWG NO. TAM 7-2127778
STRUCTURAL
COMPONENT ONLY

TRUSS PLACEMENT PLAN.

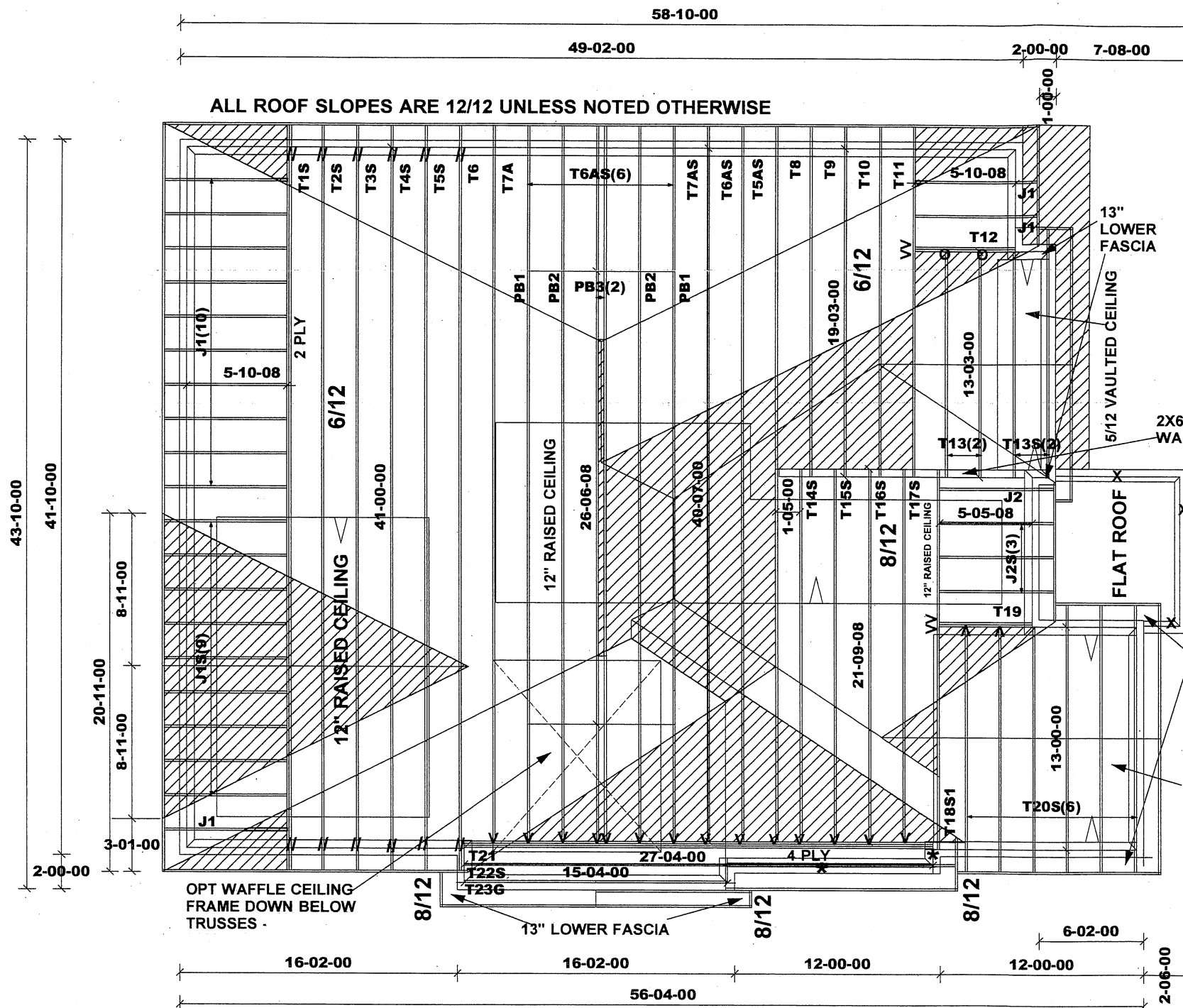
This is a truss placement plan only, NOT a final roof framing plan. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual truss drawings for each component identified on this placement plan.

Please verify that all dimensions match the dimensions found on the job. The Building designer is responsible for the temporary/permanent bracing of the roof and floor system and its integration into the bracing of the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer unless otherwise noted in this plan. Building designer to review and approve this plan to ascertain conformity to his overall structural plan.



Job Track: 52430
Layout ID: 420538
Plan Log: 205064
Builder / Location: ROYAL PINE HOMES / BRAMPTON
Project: VALES OF CASTLEMORE NORTH
Date: 2021-08-10
Designer: ND

Model / Elevation: UNIT 5003 (CORNER) / A STD OR OPT - REAR UPGRADE
Mitek ver 8.4.2.286
THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.



ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

DESIGN CONFORMS WITH OBC 2012
(2019 AMENDMENT)
OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 27.2 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 23.3 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
LUS26-2 - (VV)
H2.5A - (J)

DENOTES:

* : 12" RAISED TOP OF PLATE

X: DROPPED BEAM BY OTHERS

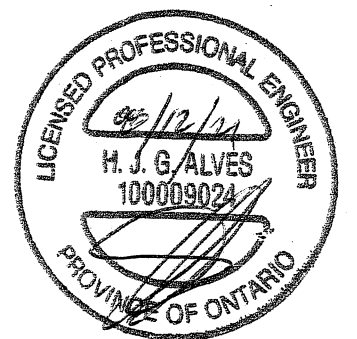
NO CHANGE TO TRUSSES
SAME AS STANDARD.

ND. 07/28/2021

DENOTES:
CONVENTIONAL
FRAMING

All conventional framing to conform with Part 9 of O.B.C. 2012. Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DWG# T-2126619 to T-2126623, T-2126630, T-2126632 to T-2126639, T-2126645 to T-2126664, T1800218

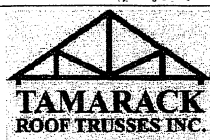


DWG NO. TAM T-2127180
STRUCTURAL
COMPONENT ONLY

TRUSS PLACEMENT PLAN.

This is a truss placement plan only, NOT a final roof framing plan. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual truss drawings for each component identified on this placement plan.

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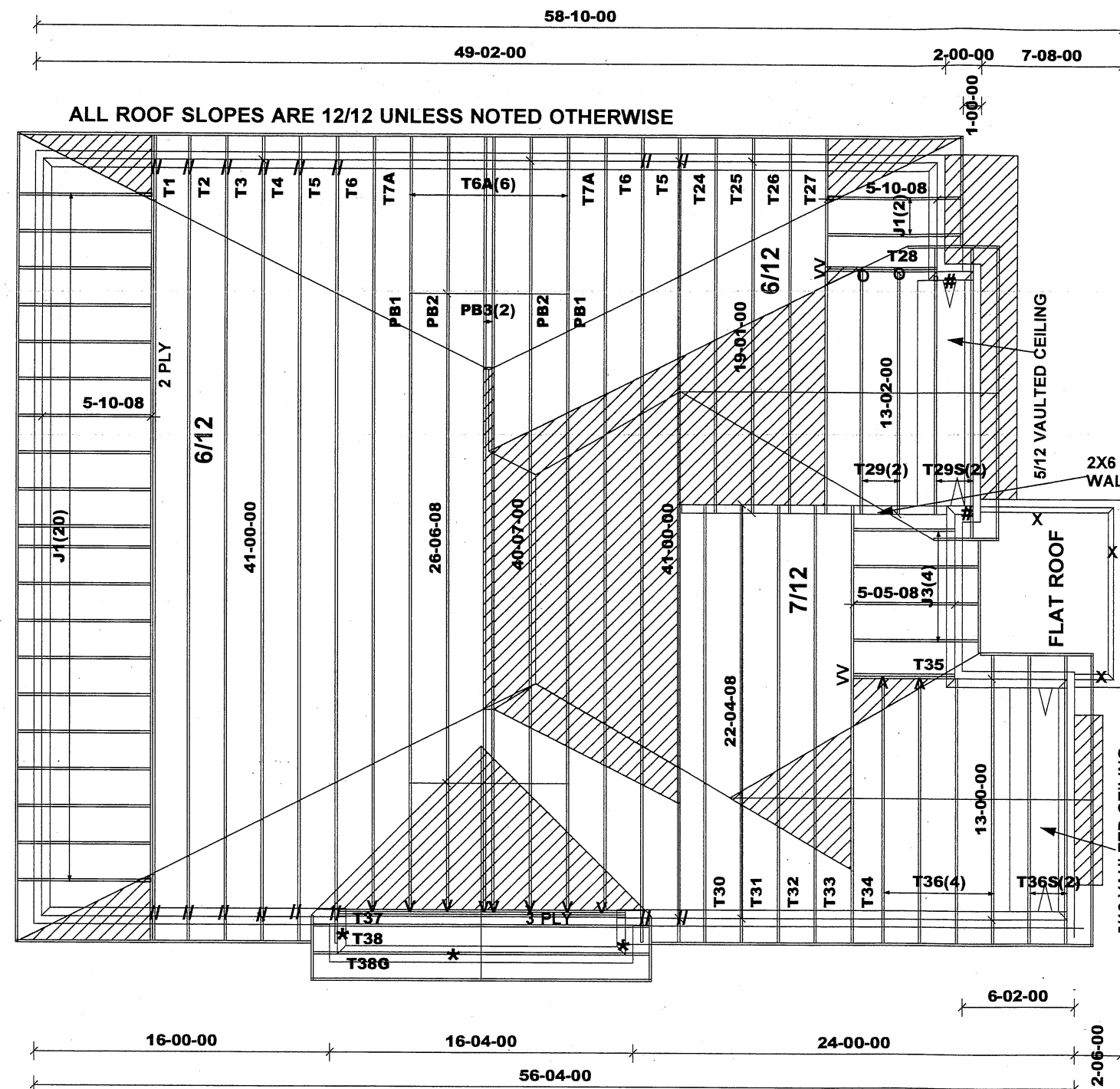


Job Track: 52430
Layout ID: 420540
Plan Log: 205064

Builder / Location: ROYAL PINE HOMES / BRAMPTON
Project: VALES OF CASTLEMORE NORTH
Date: 2021-08-10
Designer: ND

Model / Elevation: A STD OR OPT (W/COFF & REAR)
UNIT 5003 (CORNER) / UPGRADE
THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.
Mitek ver 8.4.2.286

M14225



ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

DESIGN CONFORMS WITH OBC 2012
(2019 AMENDMENT)
OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 27.2 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 23.3 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
LUS26-2 - (VV)
H2.5A - (I)

DENOTES:

* : 18" RAISED TOP OF PLATE

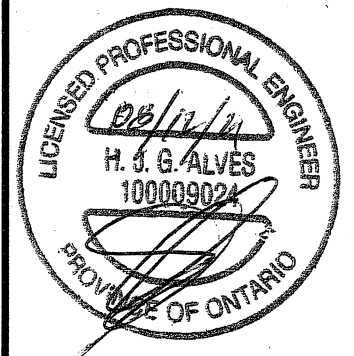
: 18" RAISED HEEL

X: DROPPED BEAM BY OTHERS

 DENOTES:
CONVENTIONAL
FRAMING

DWG# T-2126619, T-2126621 to T-2126628, T-
2126630 to T-2126632, T-2126665 to T-
2126683, T1800218

All conventional framing to conform with Part 9 of
O.B.C. 2012. Roof rafters that cross over or meet
trusses to be min. 2x4 SPF #2 @ 24" o/c with a
vertical post to the truss at each cross point. Vertical
posts longer than 6' to have lateral bracing so that the
distance between the post end points and lateral
bracing does not exceed 6'.



DWG NO. TAM *T-2127781*
STRUCTURAL
COMPONENT ONLY

TRUSS PLACEMENT PLAN.

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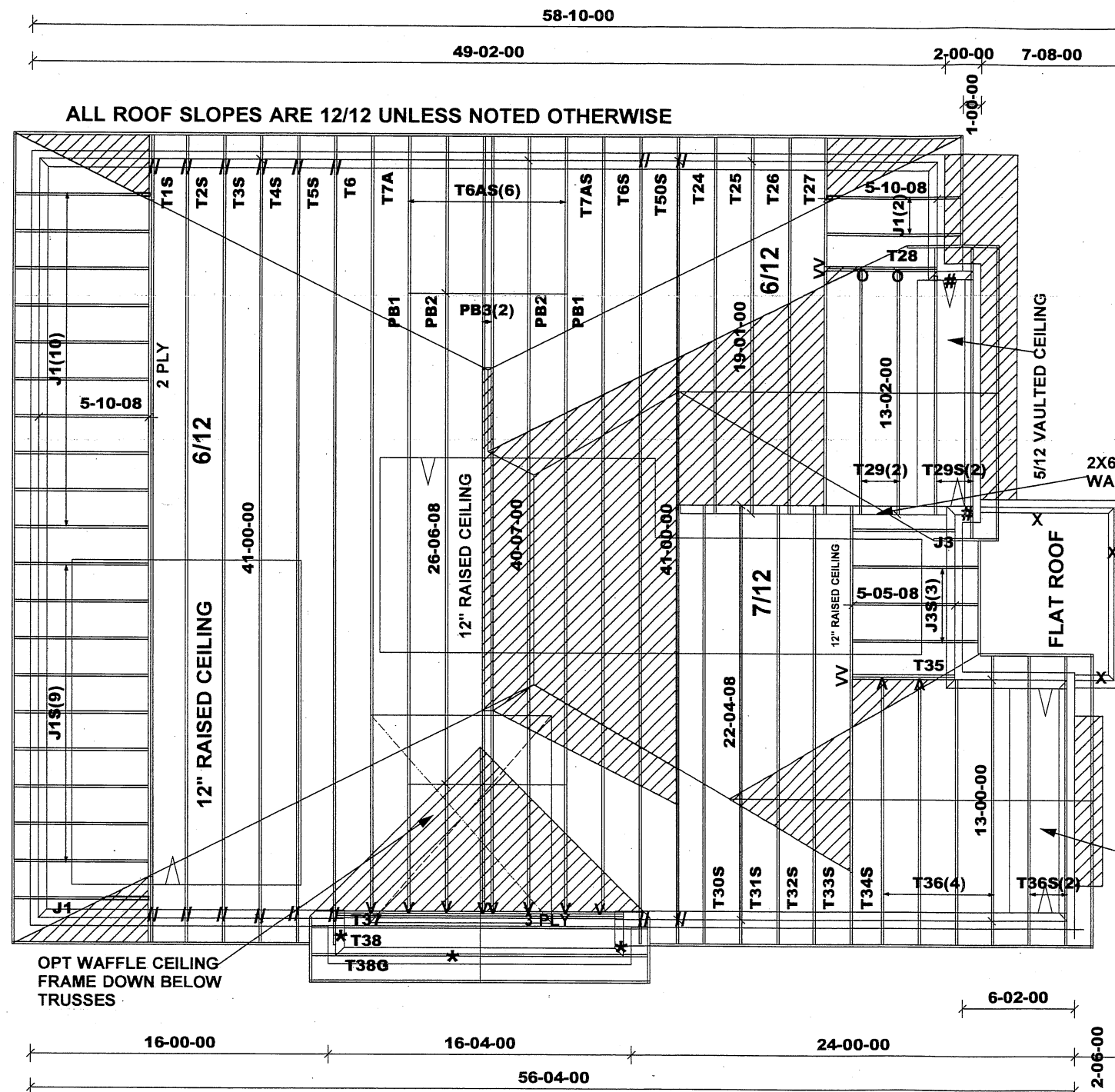


Job Track: **52430**
Layout ID: **420542**
Plan Log: **205064**

Builder / Location: **ROYAL PINE HOMES / BRAMPTON**
Project: **VALES OF CASTLEMORE NORTH**
Date: 2021-08-10 Designer: **ND**

Model / Elevation: **UNIT 5003 (CORNER) / B STD OR OPT**
Mitek ver 8.4.2.286
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M14225



ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

DESIGN CONFORMS WITH OBC 2012
(2019 AMENDMENT)
OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 27.2 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 23.3 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
LUS26-2 - (VV)
H2.5A - (I)

DENOTES:

* : 18" RAISED TOP OF PLATE

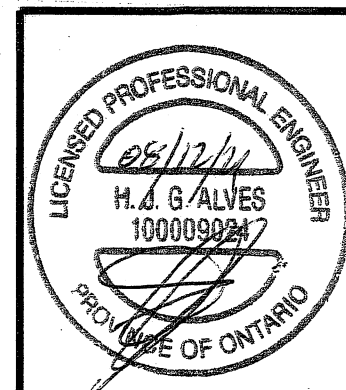
: 18" RAISED HEEL

X: DROPPED BEAM BY OTHERS

DENOTES:
CONVENTIONAL
FRAMING

All conventional framing to conform with Part 9 of O.B.C. 2012. Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DWG# T-2126619, T-2126621 to T-2126623, T-2126630, T-2126632, T-2126649, T-2126651 to T-2126654, T-2126656 to T-2126658, T-2126665 to T-2126672, T-2126678 to T-2126690, T-2127361, T1800218



DWG NO. TAM **T-2127183**
STRUCTURAL
COMPONENT ONLY

TRUSS PLACEMENT PLAN.

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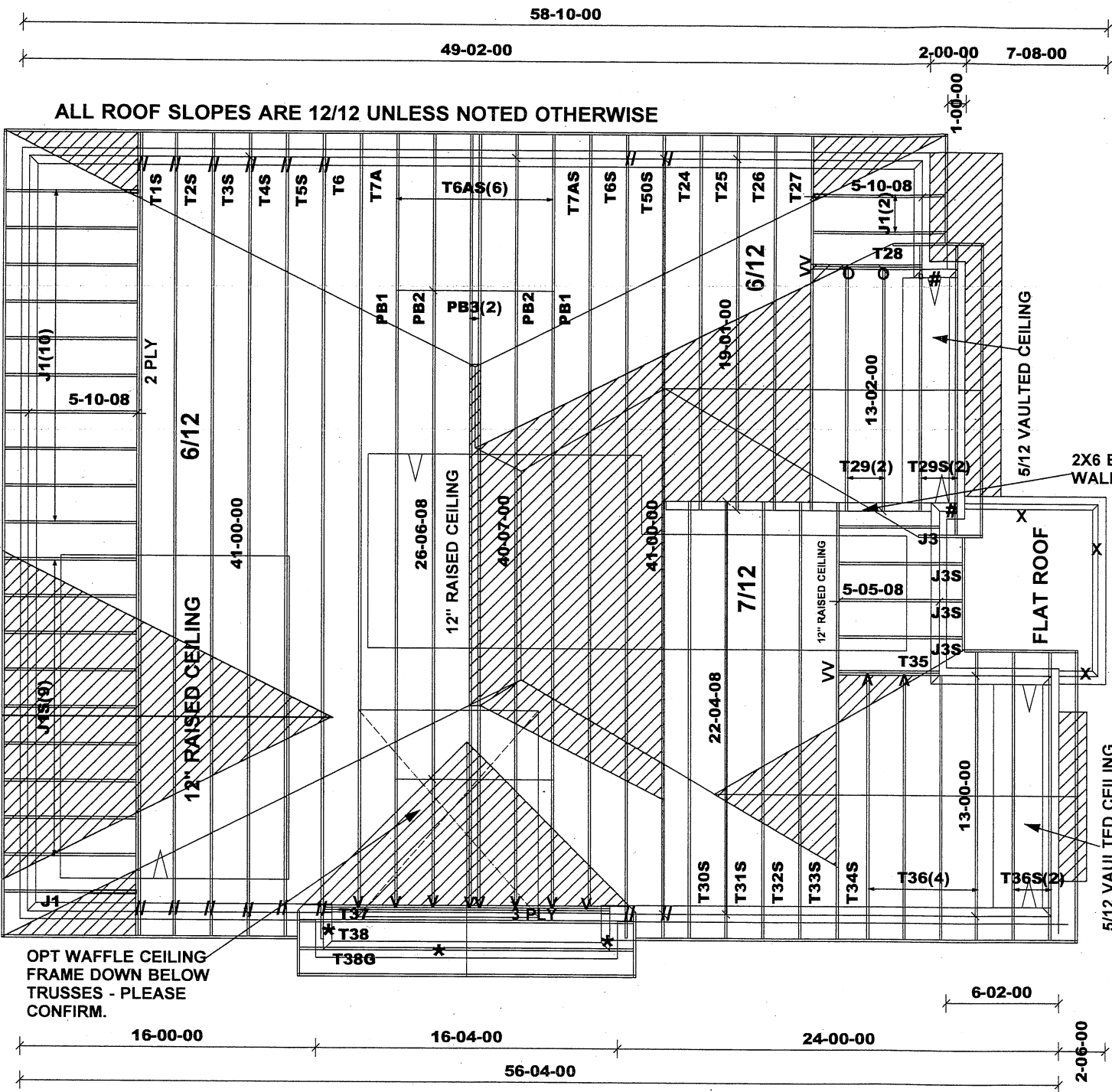


Job Track: **52430**
Layout ID: **420547**
Plan Log: **205064**

Builder / Location:
ROYAL PINE HOMES / BRAMPTON
Project: **VALES OF CASTLEMORE NORTH**
Date: 2021-08-10 Designer: **ND**

Model / Elevation:
UNIT 5003 (CORNER) / B STD OR OPT (W/COFF)
Model ver 8.4.2.286
THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

M114225



ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

DESIGN CONFORMS WITH OBC 2012
(2019 AMENDMENT)
OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 27.2 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 23.3 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
LUS26-2 - (VV)
H2.5A - (I)

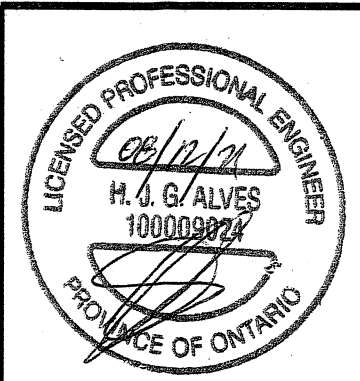
DENOTES:
* : 18" RAISED TOP OF PLATE
#: 18" RAISED HEEL
X: DROPPED BEAM BY OTHERS

NO CHANGE TO TRUSSES
SAME AS STANDARD
ND. 07/29/2021

DENOTES:
CONVENTIONAL
FRAMING

All conventional framing to conform with Part 9 of
O.B.C. 2012. Roof rafters that cross over or meet
trusses to be min. 2x4 SPF #2 @ 24" o/c with a
vertical post to the truss at each cross point. Vertical
posts longer than 6' to have lateral bracing so that the
distance between the post end points and lateral
bracing does not exceed 6'.

DWG# T-2126619, T-2126621 to T-2126623, T-
2126630, T-2126632, T-2126649, T-2126651 to
T-2126654, T-2126656 to T-2126658, T-
2126665 to T-2126672, T-2126678 to T-
2126690, T-2127361, T1800218

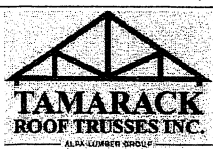


DWG NO. TAM T-2127784
STRUCTURAL
COMPONENT ONLY

TRUSS PLACEMENT PLAN.

This is a truss placement plan only, NOT a final roof framing plan. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual truss drawings for each component identified on this placement plan.

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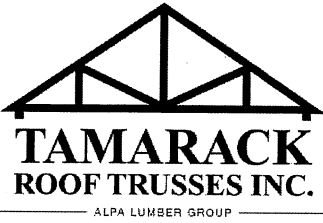


M14225

Job Track:	52430	Builder / Location:	ROYAL PINE HOMES / BRAMPTON
Layout ID:	420547	Project:	VALES OF CASTLEMORE NORTH
Plan Log:	205064	Date: 2021-08-10	Designer: ND

Model / Elevation:
UNIT 5003 (CORNER) / B STD OR OPT (W/COFF & REAR UPGRADE)
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Mitek ver 8.4.2.286

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: VALES OF CASTLEMORE NORTH
 Location: BRAMPTON
 Model: UNIT 5003 (CORNER)
 Lot #:
 Elevation: A STD OR OPT

Job Track: 52430
 PlanLog: 205064
 Layout ID: 420538
 Ref #
 Page: 1 of 3
 Date: 08-10-2021
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	12 /12	41-00-00	4-02-00	2 x 6	1-03-08 1-03-08	1-10-08 1-10-08	491.79 304.00		
	1	T2 Hip	12 /12	41-00-00	5-02-00	2 x 6	1-03-08 1-03-08	1-10-08 1-10-08	242.47 145.83		
	1	T3 Hip	12 /12	41-00-00	6-02-00	2 x 6	1-03-08 1-03-08	1-10-08 1-10-08	254.58 157.33		
	1	T4 Hip	12 /12	41-00-00	7-02-00	2 x 6	1-03-08 1-03-08	1-10-08 1-10-08	243.45 148.17		
	1	T5 Hip	12 /12	41-00-00	8-02-00	2 x 6	1-03-08 1-03-08	1-10-08 1-10-08	257.32 159.33		
	1	T5A Hip	12 /12	40-07-00	8-02-00	2 x 6	1-03-08	1-10-08 2-03-08	252.76 157.33		
	1	T6 Hip	12 /12	41-00-00	9-02-00	2 x 6	1-03-08 1-03-08	1-10-08 1-10-08	270.32 166.50		
	7	T6A Piggyback Base	12 /12	40-07-00	9-02-00	2 x 6	1-03-08	1-10-08 2-03-08	1852.95 1142.17		
	2	T7A Hip	12 /12	40-07-00	10-02-00	2 x 6	1-03-08	1-10-08 2-03-08	547.32 329.33		
	1	T8 Half Hip	12 /12	19-03-00	7-01-04	2 x 4	1-03-08	1-10-08 7-01-04	97.19 61.50		
	1	T9 Half Hip	12 /12	19-03-00	6-01-04	2 x 4	1-03-08	1-10-08 6-01-04	90.9 58.83		
	1	T10 Half Hip	12 /12	19-03-00	5-01-04	2 x 4	1-03-08	1-10-08 5-01-04	84.91 54.00		
	1	T11 Half Hip Girder	12 /12	19-03-00	4-01-04	2 x 4 2 x 6	1-03-08	1-10-08 4-01-04	96.09 62.17		
	1 2-ply	T12 Jack-Closed Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	57.64 37.00		

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: VALES OF CASTLEMORE NORTH
 Location: BRAMPTON
 Model: UNIT 5003 (CORNER)
 Lot #:
 Elevation: A STD OR OPT

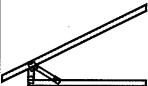
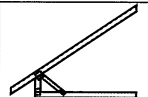
Job Track: 52430
 PlanLog: 205064
 Layout ID: 420538
 Ref #
 Page: 2 of 3
 Date: 08-10-2021
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	T13 Common	12 /12	13-03-00	7-05-00	2 x 4		9-08 9-08	119 76.33		
	2	T13S Scissor	12 /12 5 /12	13-03-00	8-03-15	2 x 4	1-03-08 1-03-08	9-08 9-08	125.15 84.33		
	1	T14 Roof Special	-12 /12	21-09-08	10-04-08	2 x 4		10-04-08 1-02-08	121.99 75.00		
	1	T15 Roof Special	-12 /12	21-09-08	9-00-08	2 x 4		9-00-08 1-02-08	117.7 73.17		
	1	T16 Roof Special	-12 /12	21-09-08	7-08-08	2 x 4		7-08-08 1-02-08	108.51 69.17		
	1	T17 Roof Special	-12 /12	21-09-08	6-04-08	2 x 4		6-04-08 1-02-08	97.23 59.50		
	1	T18S Half Hip Girder	12 /12 5 /12	22-02-08	5-00-08	2 x 6		9-08 5-00-08	121.26 75.17		
	1 2-ply	T19 Jack-Closed Girder	8 /12	5-05-08	5-00-08	2 x 4 2 x 6		1-04-13 5-00-08	60.01 39.33		
	6	T20S Scissor	12 /12 5 /12	13-00-00	8-02-07	2 x 4	1-03-08 1-03-08	9-08 9-08	369.19 247.00		
	1 3-ply	T21 Common Girder	8 /12	27-04-00	10-05-10	2 x 6		3-13 2-04-13	561.4 349.00		
	1	T22S Roof Special	8 /12	27-04-00	10-05-10	2 x 4	1-03-08	3-13 1-04-13	122.5 76.00		
	1	T23G GABLE	8 /12	15-04-00	5-05-02	2 x 4		3-13 3-13	51.74 33.50		
	2	PB1 Piggyback	12 /12	26-06-08	2-00-00	2 x 4			168.74 107.33		
	2	PB2 Piggyback	12 /12	26-06-08	3-00-00	2 x 4			173.05 109.00		

 TAMARACK ROOF TRUSSES INC. ALPHA LUMBER GROUP	DELIVERY SHIPLIST										
	Lumber Yard: TAMARACK LUMBER Builder: ROYAL PINE HOMES Project: VALES OF CASTLEMORE NORTH Location: BRAMPTON Model: UNIT 5003 (CORNER) Lot #: Elevation: A STD OR OPT							Job Track: 52430 PlanLog: 205064 Layout ID: 420538 Ref # Page: 3 of 3 Date: 08-10-2021 Designer: Sales Rep: Rick DiCiano			

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	PB3 Piggyback	12 /12	26-06-08	4-00-00	2 x 4			180.48 113.00		
	22	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	396.36 256.67		
	4	J2 Jack-Open	8 /12	5-05-08	5-00-08	2 x 4	1-03-08	1-04-13 5-00-08	72.46 46.67		

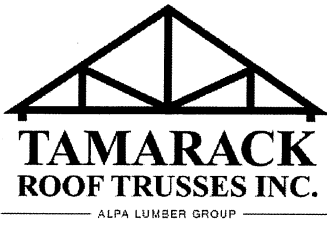
TOTAL # TRUSS= 77 TOTAL BFT OF ALL TRUSSES= 4873.66 BFT. TOTAL WEIGHT OF ALL TRSSES 7806.49 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
16	Hardware	LJS26DS	
2	Hardware	LUS24	
2	Hardware	LUS26-2	
24	Hardware	H2.5A	

TOTAL NUMBER OF ITEMS= 44

DELIVERY SHIPLIST



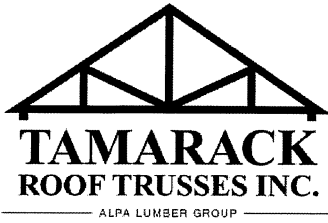
Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: VALES OF CASTLEMORE NORTH
 Location: BRAMPTON
 Model: UNIT 5003 (CORNER)
 Lot #:
 Elevation: A STD OR OPT (W/COFF)

Job Track: 52430
 PlanLog: 205064
 Layout ID: 420540
 Ref #
 Page: 1 of 3
 Date: 08-10-2021
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1S Hip Girder	12 /12	41-00-00	4-02-00	2 x 6	1-03-08 1-03-08	1-10-08 1-10-08	509.31 320.00		
	1	T2S Hip	12 /12	41-00-00	5-02-00	2 x 6	1-03-08 1-03-08	1-10-08 1-10-08	265.62 164.00		
	1	T3S Hip	12 /12	41-00-00	6-02-00	2 x 6	1-03-08 1-03-08	1-10-08 1-10-08	274.19 171.33		
	1	T4S Hip	12 /12	41-00-00	7-02-00	2 x 6	1-03-08 1-03-08	1-10-08 1-10-08	291.72 178.00		
	1	T5AS Hip	12 /12	40-07-00	8-02-00	2 x 6	1-03-08	1-10-08 2-03-08	318.02 198.67		
	1	T5S Hip	12 /12	41-00-00	8-02-00	2 x 6	1-03-08 1-03-08	1-10-08 1-10-08	291.97 178.67		
	1	T6 Hip	12 /12	41-00-00	9-02-00	2 x 6	1-03-08 1-03-08	1-10-08 1-10-08	270.32 165.50		
	7	T6AS Piggyback Base	12 /12	40-07-00	9-02-00	2 x 6	1-03-08	1-10-08 2-03-08	2195.83 1374.33		
	1	T7A Hip	12 /12	40-07-00	10-02-00	2 x 6	1-03-08	1-10-08 2-03-08	272.51 164.00		
	1	T7AS Hip	12 /12	40-07-00	10-02-00	2 x 4 2 x 6	1-03-08	1-10-08 2-03-08	312.22 190.67		
	1	T8 Half Hip	12 /12	19-03-00	7-01-04	2 x 4	1-03-08	1-10-08 7-01-04	97.19 61.50		
	1	T9 Half Hip	12 /12	19-03-00	6-01-04	2 x 4	1-03-08	1-10-08 6-01-04	90.9 58.83		
	1	T10 Half Hip	12 /12	19-03-00	5-01-04	2 x 4	1-03-08	1-10-08 5-01-04	84.91 54.00		
	1	T11 Half Hip Girder	12 /12	19-03-00	4-01-04	2 x 4 2 x 6	1-03-08	1-10-08 4-01-04	96.09 62.17		

DELIVERY SHIPLIST

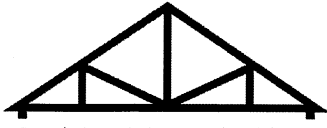


Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: VALES OF CASTLEMORE NORTH
 Location: BRAMPTON
 Model: UNIT 5003 (CORNER)
 Lot #:
 Elevation: A STD OR OPT (W/COFF)

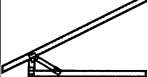
Job Track: 52430
 PlanLog: 205064
 Layout ID: 420540
 Ref #
 Page: 2 of 3
 Date: 08-10-2021
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T12 Jack-Closed Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	57.64 37.00		
	2	T13 Common	12 /12	13-03-00	7-05-00	2 x 4		9-08 9-08	117.17 75.00		
	2	T13S Scissor	12 /12 5 /12	13-03-00	7-05-00	2 x 4		9-08 9-08	114.9 77.67		
	1	T14S Roof Special	-12 /12	21-09-08	10-04-08	2 x 4		10-04-08 1-02-08	144.41 89.50		
	1	T15S Roof Special	-12 /12	21-09-08	9-00-08	2 x 4		9-00-08 1-02-08	129.88 81.17		
	1	T16S Roof Special	-12 /12	21-09-08	7-08-08	2 x 4		7-08-08 1-02-08	136.99 87.83		
	1	T17S Roof Special	-12 /12	21-09-08	6-04-08	2 x 4		6-04-08 1-02-08	123.51 78.83		
	1	T18S1 Half Hip Girder	12 /12 5 /12	22-02-08	5-00-08	2 x 6		9-08 5-00-08	135.47 86.67		
	1 2-ply	T19 Jack-Closed Girder	8 /12	5-05-08	5-00-08	2 x 4 2 x 6		1-04-13 5-00-08	60.01 39.33		
	6	T20S Scissor	12 /12 5 /12	13-00-00	8-02-07	2 x 4	1-03-08 1-03-08	9-08 9-08	369.19 247.00		
	1 3-ply	T21 Common Girder	8 /12	27-04-00	10-05-10	2 x 6		3-13 2-04-13	561.4 349.00		
	1	T22S Roof Special	8 /12	27-04-00	10-05-10	2 x 4	1-03-08	3-13 1-04-13	122 75.33		
	1	T23G GABLE	8 /12	15-04-00	5-05-02	2 x 4		3-13 3-13	51.74 33.50		
	2	PB1 Piggyback	12 /12	26-06-08	2-00-00	2 x 4			168.74 107.33		

 TAMARACK ROOF TRUSSES INC. ALPHA LUMBER GROUP	DELIVERY SHIPLIST				
	Lumber Yard:	TAMARACK LUMBER		Job Track:	52430
	Builder:	ROYAL PINE HOMES		PlanLog:	205064
	Project:	VALES OF CASTLEMORE NORTH		Layout ID:	420540
	Location:	BRAMPTON		Ref #	
	Model:	UNIT 5003 (CORNER)		Page:	3 of 3
Lot #:			Date:	08-10-2021	
Elevation:	A STD OR OPT (W/COFF)		Designer:		
			Sales Rep:	Rick DiCiano	

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	PB2 Piggyback	12 /12	26-06-08	3-00-00	2 x 4			173.05 109.00		
	2	PB3 Piggyback	12 /12	26-06-08	4-00-00	2 x 4			180.48 113.00		
	13	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	234.21 151.67		
	9	J1S Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 3-01-04	182.77 129.00		
	1	J2 Jack-Open	8 /12	5-05-08	5-00-08	2 x 4	1-03-08	1-04-13 5-00-08	18.12 11.67		
	3	J2S Jack-Open	8 /12	5-05-08	5-00-08	2 x 4	1-03-08	1-04-13 4-00-08	63.04 41.00		

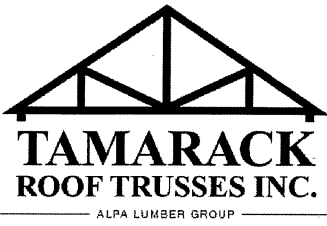
TOTAL # TRUSS= 77
 TOTAL BFT OF ALL TRUSSES= 5362.17
 BFT.
 TOTAL WEIGHT OF ALL TRSSES 8515.56
 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
16	Hardware	LJS26DS	
2	Hardware	LUS24	
2	Hardware	LUS26-2	
24	Hardware	H2.5A	

TOTAL NUMBER OF ITEMS= 44

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: VALES OF CASTLEMORE NORTH
 Location: BRAMPTON
 Model: UNIT 5003 (CORNER)
 Lot #:
 Elevation: B STD OR OPT

Job Track: 52430
 PlanLog: 205064
 Layout ID: 420542
 Ref #
 Page: 1 of 3
 Date: 08-10-2021
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	12 /12	41-00-00	4-02-00	2 x 6	1-03-08 1-03-08	1-10-08 1-10-08	491.79 304.00		
	1	T2 Hip	12 /12	41-00-00	5-02-00	2 x 6	1-03-08 1-03-08	1-10-08 1-10-08	242.47 145.83		
	1	T3 Hip	12 /12	41-00-00	6-02-00	2 x 6	1-03-08 1-03-08	1-10-08 1-10-08	254.58 157.33		
	1	T4 Hip	12 /12	41-00-00	7-02-00	2 x 6	1-03-08 1-03-08	1-10-08 1-10-08	243.45 148.17		
	2	T5 Hip	12 /12	41-00-00	8-02-00	2 x 6	1-03-08 1-03-08	1-10-08 1-10-08	514.64 318.67		
	2	T6 Hip	12 /12	41-00-00	9-02-00	2 x 6	1-03-08 1-03-08	1-10-08 1-10-08	540.65 333.00		
	6	T6A Piggyback Base	12 /12	40-07-00	9-02-00	2 x 6	1-03-08	1-10-08 2-03-08	1588.24 979.00		
	2	T7A Hip	12 /12	40-07-00	10-02-00	2 x 6	1-03-08	1-10-08 2-03-08	547.32 329.33		
	1	T24 Half Hip	12 /12	19-01-00	7-01-04	2 x 4	1-03-08	1-10-08 7-01-04	96.75 60.83		
	1	T25 Half Hip	12 /12	19-01-00	6-01-04	2 x 4	1-03-08	1-10-08 6-01-04	90.44 58.17		
	1	T26 Half Hip	12 /12	19-01-00	5-01-04	2 x 4	1-03-08	1-10-08 5-01-04	84.44 53.33		
	1	T27 Half Hip Girder	12 /12	19-01-00	4-01-04	2 x 4 2 x 6	1-03-08	1-10-08 4-01-04	94.4 60.83		
	1 2-ply	T28 Jack-Closed Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	57.64 37.00		
	2	T29 Common	12 /12	13-02-00	9-11-08	2 x 4		3-04-08 3-04-08	146.57 91.33		

DELIVERY SHIPLIST

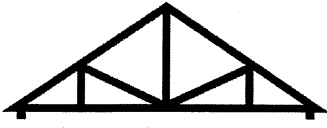


Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: VALES OF CASTLEMORE NORTH
 Location: BRAMPTON
 Model: UNIT 5003 (CORNER)
 Lot #:
 Elevation: B STD OR OPT

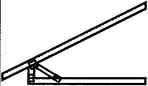
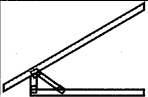
Job Track: 52430
 PlanLog: 205064
 Layout ID: 420542
 Ref #
 Page: 2 of 3
 Date: 08-10-2021
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	T29S Scissor	12 /12 5 /12	13-02-00	9-11-08	2 x 4	1-03-08 1-03-08	3-04-08 3-04-08	147.33 95.00		
	1	T30 Half Hip	12 /12	22-04-08	9-01-10	2 x 4	1-03-08	1-10-08 9-01-10	122.81 77.00		
	1	T31 Half Hip	12 /12	22-04-08	7-11-10	2 x 4	1-03-08	1-10-08 7-11-10	114.89 72.17		
	1	T32 Half Hip	12 /12	22-04-08	6-09-10	2 x 4	1-03-08	1-10-08 6-09-10	103.99 66.50		
	1	T33 Half Hip	12 /12	22-04-08	5-07-10	2 x 4	1-03-08	1-10-08 5-07-10	97.04 61.67		
	1	T34 Half Hip Girder	12 /12	22-04-08	4-05-10	2 x 4 2 x 6	1-03-08	1-10-08 4-05-10	109.77 66.50		
	1 2-ply	T35 Jack-Closed Girder	7 /12	5-05-08	4-05-10	2 x 4 2 x 6		1-03-07 4-05-10	56.85 38.00		
	4	T36 Common	12 /12	13-00-00	8-04-08	2 x 4	1-03-08 1-03-08	1-10-08 1-10-08	271.39 173.33		
	2	T36S Scissor	12 /12 5 /12	13-00-00	8-04-08	2 x 4	1-03-08 1-03-08	1-10-08 1-10-08	129.28 84.67		
	1 3-ply	T37 Common Girder	12 /12	15-06-00	11-01-08	2 x 4 2 x 6		3-04-08 3-04-08	354.85 216.00		
	1	T38 Common	12 /12	15-06-00	9-07-08	2 x 4	1-03-08 1-03-08	1-10-08 1-10-08	78.58 50.50		
	1	T38G GABLE	12 /12	15-06-00	9-07-08	2 x 4	1-03-08 1-03-08	1-10-08 1-10-08	85.91 56.50		
	2	PB1 Piggyback	12 /12	26-06-08	2-00-00	2 x 4			168.74 107.33		
	2	PB2 Piggyback	12 /12	26-06-08	3-00-00	2 x 4			173.05 109.00		

DELIVERY SHIPLIST		
 TAMARACK ROOF TRUSSES INC. ALPHA LUMBER GROUP	Lumber Yard:	TAMARACK LUMBER
	Builder:	ROYAL PINE HOMES
	Project:	VALES OF CASTLEMORE NORTH
	Location:	BRAMPTON
	Model:	UNIT 5003 (CORNER)
	Lot #:	
Elevation:		B STD OR OPT
Job Track:		52430
PlanLog:		205064
Layout ID:		420542
Ref #		
Page:		3 of 3
Date:		08-10-2021
Designer:		
Sales Rep:		Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	PB3 Piggyback	12 /12	26-06-08	4-00-00	2 x 4			180.48 113.00		
	22	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	396.36 256.67		
	4	J3 Jack-Open	7 /12	5-05-08	4-05-10	2 x 4	1-03-08	1-03-07 4-05-10	70.19 44.00		

TOTAL # TRUSS= 77

TOTAL BFT OF ALL TRUSSES= 4764.66

BFT.

TOTAL WEIGHT OF ALL TRSSES 7654.92 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
10	Hardware	LJS26DS	
2	Hardware	LUS24	
2	Hardware	LUS26-2	
32	Hardware	H2.5A	

TOTAL NUMBER OF ITEMS= 46

DELIVERY SHIPLIST



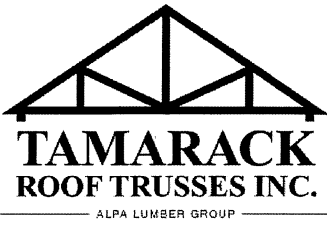
Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: VALES OF CASTLEMORE NORTH
 Location: BRAMPTON
 Model: UNIT 5003 (CORNER)
 Lot #:
 Elevation: B STD OR OPT (W/COFF)

Job Track: 52430
 PlanLog: 205064
 Layout ID: 420547
 Ref #
 Page: 1 of 3
 Date: 08-10-2021
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1S Hip Girder	12 /12	41-00-00	4-02-00	2 x 6	1-03-08 1-03-08	1-10-08 1-10-08	509.31 320.00		
	1	T2S Hip	12 /12	41-00-00	5-02-00	2 x 6	1-03-08 1-03-08	1-10-08 1-10-08	265.62 164.00		
	1	T3S Hip	12 /12	41-00-00	6-02-00	2 x 6	1-03-08 1-03-08	1-10-08 1-10-08	274.19 171.33		
	1	T4S Hip	12 /12	41-00-00	7-02-00	2 x 6	1-03-08 1-03-08	1-10-08 1-10-08	291.72 178.00		
	1	T5S Hip	12 /12	41-00-00	8-02-00	2 x 6	1-03-08 1-03-08	1-10-08 1-10-08	293.92 180.00		
	1	T6 Hip	12 /12	41-00-00	9-02-00	2 x 6	1-03-08 1-03-08	1-10-08 1-10-08	270.32 166.50		
	6	T6AS Piggyback Base	12 /12	40-07-00	9-02-00	2 x 6	1-03-08	1-10-08 2-03-08	1882.14 1178.00		
	1	T6S Hip	12 /12	41-00-00	9-02-00	2 x 6	1-03-08 1-03-08	1-10-08 1-10-08	317.26 200.00		
	1	T7A Hip	12 /12	40-07-00	10-02-00	2 x 6	1-03-08	1-10-08 2-03-08	272.51 164.00		
	1	T7AS Hip	12 /12	40-07-00	10-02-00	2 x 4 2 x 6	1-03-08	1-10-08 2-03-08	312.22 190.67		
	1	T24 Half Hip	12 /12	19-01-00	7-01-04	2 x 4	1-03-08	1-10-08 7-01-04	96.75 60.83		
	1	T25 Half Hip	12 /12	19-01-00	6-01-04	2 x 4	1-03-08	1-10-08 6-01-04	90.44 58.17		
	1	T26 Half Hip	12 /12	19-01-00	5-01-04	2 x 4	1-03-08	1-10-08 5-01-04	84.44 53.33		
	1	T27 Half Hip Girder	12 /12	19-01-00	4-01-04	2 x 4 2 x 6	1-03-08	1-10-08 4-01-04	94.4 60.83		

DELIVERY SHIPLIST



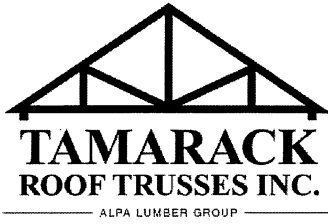
Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: VALES OF CASTLEMORE NORTH
 Location: BRAMPTON
 Model: UNIT 5003 (CORNER)
 Lot #:
 Elevation: B STD OR OPT (W/COFF)

Job Track: 52430
 PlanLog: 205064
 Layout ID: 420547
 Ref #
 Page: 2 of 3
 Date: 08-10-2021
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T28 Jack-Closed Girder	6 /12	5-10-08	4-01-04	2 x 4		1-02-00 4-01-04	51.25 34.00		
	2	T29 Common	12 /12	13-02-00	9-11-08	2 x 4		3-04-08 3-04-08	146.57 91.33		
	2	T29S Scissor	12 /12 5 /12	13-02-00	9-11-08	2 x 4	1-03-08 1-03-08	3-04-08 3-04-08	147.33 95.00		
	1	T30S Half Hip	12 /12	22-04-08	9-01-10	2 x 4	1-03-08	1-10-08 9-01-10	145.4 92.67		
	1	T31S Half Hip	12 /12	22-04-08	7-11-10	2 x 4	1-03-08	1-10-08 7-11-10	135.31 86.50		
	1	T32S Half Hip	12 /12	22-04-08	6-09-10	2 x 4	1-03-08	1-10-08 6-09-10	122.4 79.17		
	1	T33S Half Hip	12 /12	22-04-08	5-07-10	2 x 4	1-03-08	1-10-08 5-07-10	113.42 74.00		
	1	T34S Half Hip Girder	12 /12	22-04-08	4-05-10	2 x 4 2 x 6	1-03-08	1-10-08 4-05-10	127.46 81.67		
	1 2-ply	T35 Jack-Closed Girder	7 /12	5-05-08	4-05-10	2 x 4 2 x 6		1-03-07 4-05-10	56.85 38.00		
	4	T36 Common	12 /12	13-00-00	8-04-08	2 x 4	1-03-08 1-03-08	1-10-08 1-10-08	271.39 173.33		
	2	T36S Scissor	12 /12 5 /12	13-00-00	8-04-08	2 x 4	1-03-08 1-03-08	1-10-08 1-10-08	129.28 84.67		
	1 3-ply	T37 Common Girder	12 /12	15-06-00	11-01-08	2 x 4 2 x 6		3-04-08 3-04-08	318.19 196.00		
	1	T38 Common	12 /12	15-06-00	9-07-08	2 x 4	1-03-08 1-03-08	1-10-08 1-10-08	78.58 50.50		
	1	T38G GABLE	12 /12	15-06-00	9-07-08	2 x 4	1-03-08 1-03-08	1-10-08 1-10-08	85.91 56.50		

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: VALES OF CASTLEMORE NORTH
 Location: BRAMPTON
 Model: UNIT 5003 (CORNER)
 Lot #:
 Elevation: B STD OR OPT (W/COFF)

Job Track: 52430
 PlanLog: 205064
 Layout ID: 420547
 Ref #
 Page: 3 of 3
 Date: 08-10-2021
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T50S Hip	12 /12	41-00-00	8-02-00	2 x 6	1-03-08 1-03-08	1-10-08 1-10-08	299.01 186.67		
	2	PB1 Piggyback	12 /12	26-06-08	2-00-00	2 x 4			168.74 107.33		
	2	PB2 Piggyback	12 /12	26-06-08	3-00-00	2 x 4			173.05 109.00		
	2	PB3 Piggyback	12 /12	26-06-08	4-00-00	2 x 4			180.48 113.00		
	13	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	234.21 151.67		
	9	J1S Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 3-01-04	182.77 129.00		
	1	J3 Jack-Open	7 /12	5-05-08	4-05-10	2 x 4	1-03-08	1-03-07 4-05-10	17.55 11.00		
	3	J3S Jack-Open	7 /12	5-05-08	4-05-10	2 x 4	1-03-08	1-03-07 3-05-10	60.4 39.00		

TOTAL # TRUSS= 77 TOTAL BFT OF ALL TRUSSES= 5225.67 BFT. TOTAL WEIGHT OF ALL TRSSES 8300.79 LBS

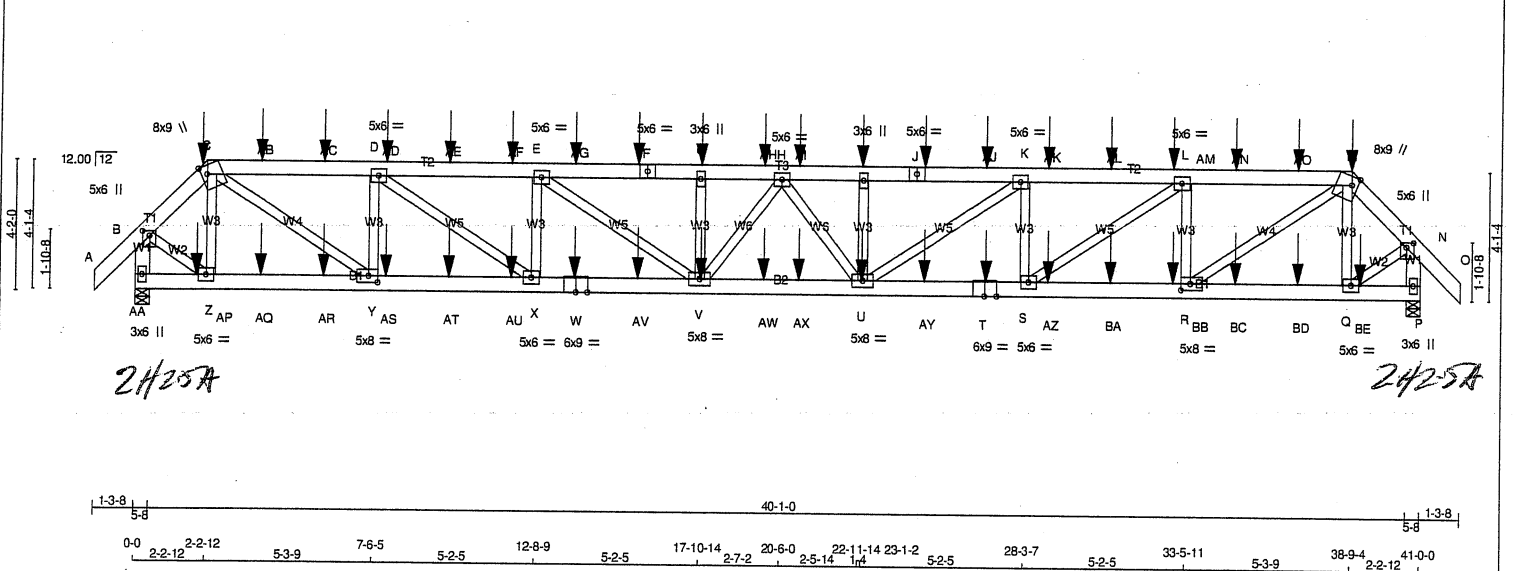
HARDWARE

QTY	TYPE	MODEL	LENGTH
10	Hardware	LJS26DS	
2	Hardware	LUS24	
2	Hardware	LUS26-2	
32	Hardware	H2.5A	

TOTAL NUMBER OF ITEMS= 46

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420538	T1	1	2	ROYAL PINE HOMES	

Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Aug 5 09:07:23 2021 Page 1	
				ID:X496mqPCjhJQnqfx7YDh0Lyvsh-GHVL4SRjuoGm8Ot6N8dM9kpVHxbqMzJsjJGyqyo	
				Scale = 1:70.1	



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. A - C 2x6 DRY No.2 SPF C - F 2x6 DRY No.2 SPF F - J 2x6 DRY No.2 SPF J - M 2x6 DRY No.2 SPF M - O 2x6 DRY No.2 SPF AA - B 2x6 DRY No.2 SPF P - N 2x6 DRY No.2 SPF AA - W 2x6 DRY 2100F 1.8E SPF W - T 2x6 DRY 2100F 1.8E SPF T - P 2x6 DRY 2100F 1.8E SPF ALL WEBS 2x4 DRY No.2 SPF EXCEPT DRY: SEASONED LUMBER. DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS: CHORDS #ROWS SURFACE SPACING (IN) LOAD(PLF) TOP CHORDS : (0.122"x3") SPIRAL NAILS A-C 2 12 SIDE(140.3) C-F 2 12 SIDE(70.1) F-J 2 12 SIDE(210.5) J-M 2 12 SIDE(70.1) M-O 2 12 SIDE(140.3) AA-B 2 12 TOP P-N 2 12 TOP BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS AA-W 2 12 SIDE(0.0) W-T 2 12 SIDE(227.4) T-P 2 12 SIDE(0.0) WEBS : (0.122"x3") SPIRAL NAILS 2x4 1 6 NAILS TO BE DRIVEN FROM ONE SIDE ONLY. GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS. TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.			
DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS FACTORED GROSS REACTION MAXIMUM FACTORED INPUT REQD JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX AA 5117 0 5309 -228 -1096 5-8 5-8 P 5117 0 5309 0 -1096 5-8 5-8 PROVIDE ANCHORAGE AT BEARING JOINT AA FOR 1096 LBS. FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT P FOR 1096 LBS. FACTORED UPLIFT PROVIDE FOR 228 LBS. FACTORED HORIZONTAL REACTION AT JOINT AA UNFACTORED REACTIONS 1ST LCASE MAX./MIN. COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL AA 3740 2325 / 0 431 / 0 0 / 0 254 / -1070 1112 / 0 0 / 0 P 3740 2325 / 0 431 / 0 0 / 0 165 / -1070 1112 / 0 0 / 0 HORIZONTAL REACTIONS AA --- 0 / 0 0 / 0 0 / 0 163 / -163 0 / 0 0 / 0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) AA, P BRACING MAX. UNBRACED TOP CHORD LENGTH = 2.96 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (18) CHORDS MAX. FACTORED WEBS MEMB. FORCE VERT. LOAD LC1 MAX. MEMB. FORCE MAX. FACTORED (LBS) (PLF) CSI (LC) UNBRAC LENGTH FR-TO (LBS) CSI (LC) FR-TO A-B 0 / 62 -105.2 -105.2 0.05 (2) 10.00 Z-C -1405 / 308 0.12 (2) B-C -4573 / 984 -105.2 -105.2 0.08 (3) 5.36 C-Y -1438 / 6705 0.59 (3) C-AB -8680 / 1927 -105.2 -105.2 0.32 (2) 3.91 Y-D -3532 / 873 0.30 (3) AB-AC -8680 / 1927 -105.2 -105.2 0.32 (2) 3.91 D-X -933 / 4414 0.39 (3) AC-D -8680 / 1927 -105.2 -105.2 0.32 (2) 3.91 X-E -2228 / 579 0.19 (3) D-AD -12285 / 2674 -105.2 -105.2 0.45 (2) 3.24 E-V -456 / 2197 0.19 (3) AD-AE -12285 / 2674 -105.2 -105.2 0.45 (2) 3.24 V-G -777 / 251 0.06 (2) AE-AF -12285 / 2674 -105.2 -105.2 0.45 (2) 3.24 U-I -777 / 251 0.06 (3) AF-E -12285 / 2674 -105.2 -105.2 0.45 (2) 3.24 U-K -456 / 2198 0.19 (2) E-AG -14076 / 3029 -105.2 -105.2 0.55 (2) 2.96 S-K -2228 / 579 0.19 (2) AG-F -14076 / 3029 -105.2 -105.2 0.55 (2) 2.96 S-L -933 / 4414 0.39 (2) F-G -14076 / 3029 -105.2 -105.2 0.55 (2) 2.96 R-L -3532 / 873 0.30 (2) G-AH -14076 / 3029 -105.2 -105.2 0.40 (2) 3.08 R-M -1438 / 6705 0.59 (2) AH-H -14076 / 3029 -105.2 -105.2 0.40 (2) 3.08 Q-M -1405 / 308 0.12 (3) H-AI -14076 / 3029 -105.2 -105.2 0.40 (3) 3.08 B-Z -618 / 3662 0.32 (2) AI-I -14076 / 3029 -105.2 -105.2 0.40 (3) 3.08 Q-N -617 / 3662 0.32 (3) I-J -14076 / 3029 -105.2 -105.2 0.55 (3) 2.96 V-H -280 / 75 0.03 (3) J-AJ -14076 / 3029 -105.2 -105.2 0.55 (3) 2.96 H-U -259 / 75 0.03 (2) AJ-K -14076 / 3029 -105.2 -105.2 0.55 (3) 2.96 K-AK -12285 / 2674 -105.2 -105.2 0.45 (3) 3.24 AK-AL -12285 / 2674 -105.2 -105.2 0.45 (3) 3.24 AL-AM -12285 / 2674 -105.2 -105.2 0.45 (3) 3.24 AM-L -12285 / 2674 -105.2 -105.2 0.45 (3) 3.24 L-AN -8680 / 1927 -105.2 -105.2 0.32 (3) 3.91 AN-AO -8680 / 1927 -105.2 -105.2 0.32 (3) 3.91 AO-M -8680 / 1927 -105.2 -105.2 0.32 (3) 3.91 M-N -4573 / 984 -105.2 -105.2 0.08 (14) 5.36 N-O 0 / 62 -105.2 -105.2 0.05 (3) 10.00 AA-B -5360 / 1116 0.0 0.0 0.20 (2) 6.32 P-N -5361 / 1116 0.0 0.0 0.20 (3) 6.32 AA-AP -201 / 215 -39.5 -39.5 0.05 (3) 6.25 AP-Z -201 / 215 -39.5 -39.5 0.05 (3) 6.25 Z-AQ -566 / 3143 -39.5 -39.5 0.13 (2) 6.25 AQ-AR -566 / 3143 -39.5 -39.5 0.13 (2) 6.25 AR-Y -566 / 3143 -39.5 -39.5 0.13 (2) 6.25			
DESIGN CRITERIA *** SPECIAL LOADS ANALYSIS *** GEOMETRY AND/OR BASIC LOADS CHANGED BY USER. LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE SPECIFIED LOADS: TOP CH. LL = 30.1 PSF DL = 6.0 PSF BOT CH. LL = 10.5 PSF DL = 7.4 PSF TOTAL LOAD = 54.0 PSF SPACING = 24.0 IN. C/C LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12 *** NON STANDARD GIRDER *** ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES. THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBC 2015 THIS DESIGN COMPLIES WITH: - PART 4 OF CBC 2018, ABC 2019 - PART 4 OF CBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 DESIGN ASSUMPTIONS - SLOPE REDUCTION FACTOR NOT USED (80 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 30.1 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL)= L/360 (1.37") CALCULATED VERT. DEFL.(LL)= L/999 (0.36") ALLOWABLE DEFL.(TL)= L/180 (2.73") CALCULATED VERT. DEFL.(TL)= L/913 (0.54") CSI: TC=0.55/1.00 (E-G-2), BC=0.43/1.00 (U-V-2), WB=0.59/1.00 (C-Y-3), SS=0.18/1.00 (K-L-2) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00 SNOW LOAD IMPORTANCE FACTOR = 1.00 WIND LOAD IMPORTANCE FACTOR = 1.00 LIVE LOAD IMPORTANCE FACTOR = 1.00 COMPANION LIVE LOAD FACTOR = 1.00 AUTOSOLVE HEELS OFF TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. CONTINUED ON PAGE 2			



Structural component only
DWG# T-2126624

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	1.75	2.75
C	TTWW+m	MT20	8.0	9.0	Edge	2.25
D, E, H, K, L						
D	TMVW-t	MT20	5.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMVW+w	MT20	3.0	6.0		
I	TMVW-w	MT20	3.0	6.0		
J	TS-t	MT20	5.0	6.0		
M	TTWW+m	MT20	8.0	9.0	Edge	2.25
N	TMVW+p	MT20	5.0	6.0	1.75	2.75
P	BMV1+p	MT20	3.0	6.0		
Q, S, X, Z						
Q	BMVW-t	MT20	5.0	6.0		
R	BMVW-t	MT20	5.0	8.0	2.50	3.50
T	BS-t	MT20	6.0	9.0		
U	BMVW-t	MT20	5.0	8.0		
V	BMVW-t	MT20	5.0	8.0		
W	BS-t	MT20	6.0	9.0		
Y	BMVW-t	MT20	5.0	8.0	2.50	3.50
AA	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LOADING
TOTAL LOAD CASES: (18)

CHORDS			WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO	CS (LC)	UNBRAC LENGTH	FR-TO
Y-AS	-1736 / 8681	-39.5 -39.5	0.28 (2)	6.25	
AS-AT	-1736 / 8681	-39.5 -39.5	0.28 (2)	6.25	
AT-AU	-1736 / 8681	-39.5 -39.5	0.28 (2)	6.25	
AU-X	-1736 / 8681	-39.5 -39.5	0.28 (2)	6.25	
X-W	-2481 / 12285	-39.5 -39.5	0.37 (2)	6.25	
W-AV	-2481 / 12285	-39.5 -39.5	0.37 (2)	6.25	
AV-V	-2481 / 12285	-39.5 -39.5	0.37 (2)	6.25	
V-AW	-2870 / 14222	-39.5 -39.5	0.43 (2)	6.25	
AW-AX	-2870 / 14222	-39.5 -39.5	0.43 (2)	6.25	
AX-U	-2870 / 14222	-39.5 -39.5	0.43 (2)	6.25	
U-AY	-2453 / 12285	-39.5 -39.5	0.37 (3)	6.25	
AY-T	-2453 / 12285	-39.5 -39.5	0.37 (3)	6.25	
T-S	-2453 / 12285	-39.5 -39.5	0.37 (3)	6.25	
S-AZ	-1690 / 8681	-39.5 -39.5	0.28 (3)	6.25	
AZ-BA	-1690 / 8681	-39.5 -39.5	0.28 (3)	6.25	
BA-BB	-1690 / 8681	-39.5 -39.5	0.28 (3)	6.25	
BB-R	-1690 / 8681	-39.5 -39.5	0.28 (3)	6.25	
R-BC	-500 / 3143	-39.5 -39.5	0.13 (3)	6.25	
BC-BD	-500 / 3143	-39.5 -39.5	0.13 (3)	6.25	
BD-Q	-500 / 3143	-39.5 -39.5	0.13 (3)	6.25	
Q-BE	-12 / 27	-39.5 -39.5	0.05 (2)	6.25	
BE-P	-12 / 27	-39.5 -39.5	0.05 (2)	6.25	

SPECIFIED CONCENTRATED LOADS (LBS)

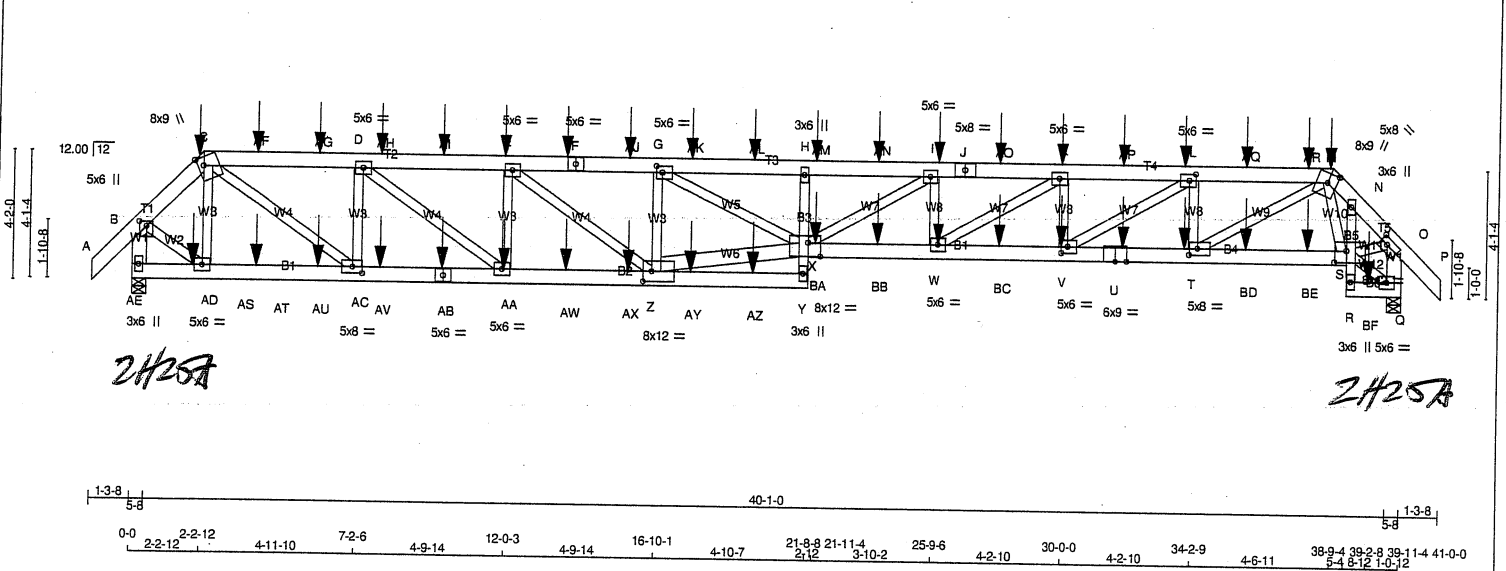
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-2-12	-22	-139	---	FRONT	VERT	DEAD	---	C1
C	2-2-12	-104	-110	---	FRONT	VERT	TOTAL	---	C1
C	2-2-12	-110	-110	---	FRONT	VERT	SNOW	---	C1
F	15-11-4	-100	-100	---	FRONT	VERT	TOTAL	---	C1
G	17-11-4	-100	-100	---	FRONT	VERT	TOTAL	---	C1
I	23-0-12	-100	-100	---	FRONT	VERT	TOTAL	---	C1
J	25-0-12	-100	-100	---	FRONT	VERT	TOTAL	---	C1
M	38-9-4	-22	-139	---	FRONT	VERT	DEAD	---	C1
M	38-9-4	-104	-110	---	FRONT	VERT	TOTAL	---	C1
M	38-9-4	-110	-110	---	FRONT	VERT	SNOW	---	C1
T	27-0-12	-29	-29	---	FRONT	VERT	TOTAL	---	C1
U	23-0-12	-29	-29	---	FRONT	VERT	TOTAL	---	C1
V	17-11-4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
W	13-11-4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AB	3-11-4	-100	-100	---	FRONT	VERT	TOTAL	---	C1
AC	5-11-4	-100	-100	---	FRONT	VERT	TOTAL	---	C1
AD	7-11-4	-100	-100	---	FRONT	VERT	TOTAL	---	C1
AE	9-11-4	-100	-100	---	FRONT	VERT	TOTAL	---	C1
AF	11-11-4	-100	-100	---	FRONT	VERT	TOTAL	---	C1
AG	13-11-4	-100	-100	---	FRONT	VERT	TOTAL	---	C1
AH	19-11-4	-100	-100	---	FRONT	VERT	TOTAL	---	C1
AI	21-0-12	-100	-100	---	FRONT	VERT	TOTAL	---	C1
AJ	27-0-12	-100	-100	---	FRONT	VERT	TOTAL	---	C1
AK	29-0-12	-100	-100	---	FRONT	VERT	TOTAL	---	C1
AL	31-0-12	-100	-100	---	FRONT	VERT	TOTAL	---	C1
AM	33-0-12	-100	-100	---	FRONT	VERT	TOTAL	---	C1
AN	35-0-12	-100	-100	---	FRONT	VERT	TOTAL	---	C1
AO	37-0-12	-100	-100	---	FRONT	VERT	TOTAL	---	C1
AP	1-11-4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AQ	3-11-4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AR	5-11-4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AS	7-11-4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AT	9-11-4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AU	11-11-4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AV	15-11-4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AW	19-11-4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AX	21-0-12	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AY	25-0-12	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AZ	29-0-12	-29	-29	---	FRONT	VERT	TOTAL	---	C1
BA	31-0-12	-29	-29	---	FRONT	VERT	TOTAL	---	C1
BB	33-0-12	-29	-29	---	FRONT	VERT	TOTAL	---	C1
BC	35-0-12	-29	-29	---	FRONT	VERT	TOTAL	---	C1
BD	37-0-12	-29	-29	---	FRONT	VERT	TOTAL	---	C1
BE	39-0-12	-29	-29	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2.(8)
WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.2) PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.

JSI GRIP= 0.89 (S) (INPUT = 0.90)
JSI METAL= 0.93 (W) (INPUT = 1.00)





LUMBER N.L.G.A. RULES CHORDS SIZE LUMBER DESCR. A - C 2x6 DRY No.2 SPF C - F 2x6 DRY No.2 SPF F - J 2x6 DRY No.2 SPF J - M 2x6 DRY No.2 SPF M - P 2x6 DRY No.2 SPF AE - B 2x6 DRY No.2 SPF Q - O 2x6 DRY No.2 SPF AE - AB 2x6 DRY 2100F 1.8E SPF AB - Y 2x6 DRY 2100F 1.8E SPF Y - H 2x4 DRY No.2 SPF X - U 2x6 DRY 2100F 1.8E SPF U - S 2x6 DRY 2100F 1.8E SPF R - N 2x4 DRY No.2 SPF R - Q 2x6 DRY 2100F 1.8E SPF ALL WEBS 2x4 DRY No.2 SPF EXCEPT Z - X 2x6 DRY 2100F 1.8E SPF DRY: SEASONED LUMBER. DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS: CHORDS #ROWS SURFACE SPACING (IN) LOAD (PLF) TOP CHORDS : (0.122"x3") SPIRAL NAILS A-C 2 12 SIDE (140.3) C-F 2 12 SIDE (210.5) F-J 2 12 SIDE (210.5) J-M 2 12 SIDE (210.5) M-P 2 12 SIDE (0.0) AE-B 2 12 TOP Q-O 2 12 TOP BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS AE-AB 2 12 SIDE (0.0) AB-Y 2 12 SIDE (227.4) X-U 2 12 SIDE (227.4) U-S 2 12 SIDE (227.4) R-Q 2 12 SIDE (0.0) Y-H 1 12 TOP N-R 1 12 TOP WEBS : (0.122"x3") SPIRAL NAILS 2x4 1 6 2x6 2 6	DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS FACTORED GROSS REACTION MAXIMUM FACTORED GROSS REACTION INPUT REORD JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX AE 5041 0 5235 230 -1095 5-8 5-8 Q 4947 0 5153 0 -1099 5-8 5-8 PROVIDE ANCHORAGE AT BEARING JOINT AE FOR 1095 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT Q FOR 1099 LBS FACTORED UPLIFT PROVIDE FOR 230 LBS FACTORED HORIZONTAL REACTION AT JOINT AE UNFACTORED REACTIONS 1ST LCASE MAX/MIN COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM LIVE WIND DEAD SOIL AE 3687 2289 / 0 431 / 0 0 / 0 255 / -1069 1097 / 0 0 / 0 Q 3623 2238 / 0 431 / 0 0 / 0 162 / -1072 1092 / 0 0 / 0 HORIZONTAL REACTIONS AE 0 / 0 0 / 0 0 / 0 164 / -163 0 / 0 0 / 0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) AE, Q BRACING MAX. UNBRACED TOP CHORD LENGTH = 2.22 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (18) CHORDS WEBS MAX. FACTORED MAX. FACTORED MEMB. FORCE VERT. LOAD LC1 MAX. MAX. MEMB. FORCE MAX. (LBS) (PLF) CSI (LC) UNBRAC (LBS) CSI (LC) FR-TO FROM TO LENGTH FR-TO A-B 0 / 62 -105.2 -105.2 0.05 (2) 10.00 AD-C -1397 / 306 0.12 (2) B-C -4498 / 982 -105.2 -105.2 0.08 (13) 5.40 Z-G -3959 / 933 0.34 (3) C-AF -8232 / 1861 -105.2 -105.2 0.29 (2) 4.03 Z-X -2798 / 13566 0.36 (3) AF-AG -8232 / 1861 -105.2 -105.2 0.29 (2) 4.03 G-X -1240 / 5929 0.52 (3) AG-D -8232 / 1861 -105.2 -105.2 0.29 (2) 4.03 B-AD -616 / 3601 0.32 (2) D-AH -11623 / 2581 -105.2 -105.2 0.40 (2) 3.37 S-Q -100 / 49 0.01 (3) AH-AI -11623 / 2581 -105.2 -105.2 0.40 (2) 3.37 S-O -671 / 3892 0.34 (3) AI-E -11623 / 2581 -105.2 -105.2 0.40 (2) 3.37 W-I -1512 / 455 0.10 (2) E-F -13562 / 2986 -105.2 -105.2 0.50 (3) 3.05 T-L -3439 / 888 0.22 (2) F-AJ -13562 / 2986 -105.2 -105.2 0.50 (3) 3.05 X-I -341 / 1501 0.13 (2) AJ-G -13562 / 2986 -105.2 -105.2 0.50 (3) 3.05 T-M -1635 / 7593 0.67 (2) G-AK -18873 / 4101 -105.2 -105.2 0.87 (3) 2.22 V-K -2388 / 625 0.15 (2) AK-AL -18873 / 4101 -105.2 -105.2 0.87 (3) 2.22 W-K -723 / 3365 0.30 (2) AL-H -18873 / 4101 -105.2 -105.2 0.87 (3) 2.22 V-L -1120 / 5110 0.45 (2) H-AM -19029 / 4134 -105.2 -105.2 0.84 (3) 2.24 AC-D -3507 / 873 0.30 (3) AM-AN -19029 / 4134 -105.2 -105.2 0.84 (3) 2.24 AA-E -2321 / 598 0.20 (3) AN-I -19029 / 4134 -105.2 -105.2 0.84 (3) 2.24 AC-E -1385 / 6361 0.56 (3) I-J -17768 / 3845 -105.2 -105.2 0.71 (3) 2.48 D-AA -921 / 4262 0.38 (3) J-AO -17768 / 3845 -105.2 -105.2 0.71 (3) 2.48 E-Z -531 / 2465 0.22 (3) AO-K -14929 / 3232 -105.2 -105.2 0.53 (3) 2.89 M-S -821 / 181 0.05 (3) K-AP -14929 / 3232 -105.2 -105.2 0.53 (3) 2.89 AP-L -14929 / 3232 -105.2 -105.2 0.53 (3) 2.89 L-AQ -10612 / 2299 -105.2 -105.2 0.34 (3) 3.56 AQ-AR -10612 / 2299 -105.2 -105.2 0.34 (3) 3.56 AR-M -10612 / 2299 -105.2 -105.2 0.34 (3) 3.56 M-N -5331 / 1153 -105.2 -105.2 0.11 (3) 4.99 N-O -5559 / 1137 -105.2 -105.2 0.11 (3) 4.90 O-P 0 / 62 -105.2 -105.2 0.05 (3) 10.00 AE-B -5277 / 1114 0.0 0.0 0.20 (2) 6.36 Q-O -5055 / 1081 0.0 0.0 0.19 (3) 6.48 AE-AS -203 / 216 -39.5 -39.5 0.05 (3) 6.25 AS-AD -203 / 216 -39.5 -39.5 0.05 (3) 6.25 AD-AT -566 / 3090 -39.5 -39.5 0.13 (2) 6.25 AT-AU -566 / 3090 -39.5 -39.5 0.13 (2) 6.25 AU-AC -566 / 3090 -39.5 -39.5 0.13 (2) 6.25	DESIGN CRITERIA *** SPECIAL LOADS ANALYSIS *** GEOMETRY AND/OR BASIC LOADS CHANGED BY USER. LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE SPECIFIED LOADS: TOP CH. LL = 30.1 PSF DL = 6.0 PSF BOT CH. LL = 10.5 PSF DL = 7.4 PSF TOTAL LOAD = 54.0 PSF SPACING = 24.0 IN. C/C LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12 *** NON STANDARD GIRDER *** ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES. THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015 THIS DESIGN COMPLIES WITH: - PART 4 OF BCBC 2018 , ABC 2019 - PART 4 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 DESIGN ASSUMPTIONS - SLOPE REDUCTION FACTOR NOT USED - OVERHANG NOT TO BE ALTERED OR CUT OFF. (80 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 30.1 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL)= L/360 (1.37") CALCULATED VERT. DEFL.(LL) = L/938 (0.52") ALLOWABLE DEFL.(TL)= L/180 (2.73") CALCULATED VERT. DEFL.(TL) = L/622 (0.79") CSI: TC=0.87/1.00 (G-H:3), BC=0.53/1.00 (W-X:3), WB=0.67/1.00 (M-T:2), SSI=0.18/1.00 (G-H:3) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00 SNOW LOAD IMPORTANCE FACTOR = 1.00 WIND LOAD IMPORTANCE FACTOR = 1.00 LIVE LOAD IMPORTANCE FACTOR = 1.00 COMPANION LIVE LOAD FACTOR = 1.00 AUTOSOLVE HEELS OFF TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873
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Structural component only
DWG# T-2126651

JOB NAME 420540	TRUSS NAME T1S	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	1.75	2.75
C	TTWW+m	MT20	8.0	9.0	Edge	2.25
D, E, I, K						
D	TMVW-t	MT20	5.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMVW-t	MT20	5.0	6.0	2.50	2.25
H	TMV+p	MT20	3.0	6.0		
J	TS-t	MT20	5.0	8.0		
L	TMVW-t	MT20	5.0	6.0	2.50	2.50
M	TTWW+m	MT20	8.0	9.0	3.75	3.50
N	TMV+p	MT20	3.0	6.0		
O	TMVW-t	MT20	5.0	8.0	2.50	3.00
Q	BMVW1-t	MT20	5.0	6.0		
R	BMV+p	MT20	3.0	6.0		
S	BVMWVW-t	MT20	8.0	12.0	4.50	4.50
T	BMVW-t	MT20	5.0	8.0	2.50	3.25
U	BS-t	MT20	6.0	9.0		
V	BMVW-t	MT20	5.0	6.0	2.50	2.50
W, AA, AD						
W	BMVW-t	MT20	5.0	6.0		
X	BVMWVW-t	MT20	8.0	12.0	5.25	4.75
Y	BMV+p	MT20	3.0	6.0		
Z	BVMWVW-t	MT20	8.0	12.0	4.00	3.25
AB	BS-t	MT20	5.0	6.0		
AC	BMVW-t	MT20	5.0	8.0	2.50	3.75
AE	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES - (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

LOADING

TOTAL LOAD CASES: (18)

CHORDS			WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO		LENGTH FR-TO	
AC-AV	-1671 / 8233	-39.5	-39.5	0.26 (2)	6.25
AV-AB	-1671 / 8233	-39.5	-39.5	0.26 (2)	6.25
AB-AA	-1671 / 8233	-39.5	-39.5	0.26 (2)	6.25
AA-AW	-2390 / 11623	-39.5	-39.5	0.34 (2)	6.25
AW-AX	-2390 / 11623	-39.5	-39.5	0.34 (2)	6.25
AX-Z	-2390 / 11623	-39.5	-39.5	0.34 (2)	6.25
Z-AY	-87 / 413	-39.5	-39.5	0.05 (17)	6.25
AY-AZ	-87 / 413	-39.5	-39.5	0.05 (17)	6.25
AZ-Y	-87 / 413	-39.5	-39.5	0.05 (17)	6.25
Y-X	0 / 142	0.0	0.0	0.32 (3)	10.00
X-H	-584 / 229	0.0	0.0	0.31 (3)	7.81
X-BA	-3653 / 17768	-39.5	-39.5	0.53 (3)	6.25
BA-BB	-3653 / 17768	-39.5	-39.5	0.53 (3)	6.25
BB-W	-3653 / 17768	-39.5	-39.5	0.53 (3)	6.25
W-BC	-3041 / 14929	-39.5	-39.5	0.47 (3)	6.25
BC-V	-3041 / 14929	-39.5	-39.5	0.47 (3)	6.25
V-U	-2092 / 10612	-39.5	-39.5	0.33 (3)	6.25
U-T	-2092 / 10612	-39.5	-39.5	0.33 (3)	6.25
T-BD	-685 / 4093	-39.5	-39.5	0.16 (3)	6.25
BD-BE	-685 / 4093	-39.5	-39.5	0.16 (3)	6.25
BE-S	-685 / 4093	-39.5	-39.5	0.16 (3)	6.25
R-S	0 / 67	0.0	0.0	0.07 (3)	10.00
S-N	-3 / 267	0.0	0.0	0.09 (3)	10.00
R-BF	-15 / 85	-39.5	-39.5	0.01 (17)	6.25
BF-Q	-15 / 85	-39.5	-39.5	0.01 (17)	6.25

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-2-12	-22	-22	---	FRONT	VERT	DEAD	---	C1
C	2-2-12	-104	-139	---	FRONT	VERT	TOTAL	---	C1
C	2-2-12	-110	-110	---	FRONT	VERT	SNOW	---	C1
E	11-11-4	-100	-100	---	FRONT	VERT	TOTAL	---	C1
F	13-11-4	-100	-100	---	FRONT	VERT	TOTAL	---	C1
I	25-11-4	-56	-56	---	FRONT	VERT	TOTAL	---	C1
K	29-11-4	-56	-56	---	FRONT	VERT	TOTAL	---	C1
L	33-11-4	-56	-56	---	FRONT	VERT	TOTAL	---	C1
M	38-8-2	-22	-22	---	FRONT	VERT	DEAD	---	C1
M	38-8-2	-110	-110	---	FRONT	VERT	SNOW	---	C1
T	33-11-4	-64	-64	---	FRONT	VERT	TOTAL	---	C1
U	31-11-4	-64	-64	---	FRONT	VERT	TOTAL	---	C1
V	29-11-4	-64	-64	---	FRONT	VERT	TOTAL	---	C1
W	25-11-4	-64	-64	---	FRONT	VERT	TOTAL	---	C1
AA	11-11-4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AB	9-11-4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AF	3-11-4	-100	-100	---	FRONT	VERT	TOTAL	---	C1
AG	5-11-4	-100	-100	---	FRONT	VERT	TOTAL	---	C1
AH	7-11-4	-100	-100	---	FRONT	VERT	TOTAL	---	C1
AI	9-11-4	-100	-100	---	FRONT	VERT	TOTAL	---	C1
AJ	15-11-4	-100	-100	---	FRONT	VERT	TOTAL	---	C1
AK	17-11-4	-100	-100	---	FRONT	VERT	TOTAL	---	C1
AL	19-11-4	-100	-100	---	FRONT	VERT	TOTAL	---	C1
AM	21-11-4	-56	-56	---	FRONT	VERT	TOTAL	---	C1
AN	23-11-4	-56	-56	---	FRONT	VERT	TOTAL	---	C1
AO	27-11-4	-56	-56	---	FRONT	VERT	TOTAL	---	C1
AP	31-11-4	-56	-56	---	FRONT	VERT	TOTAL	---	C1
AQ	35-11-4	-56	-56	---	FRONT	VERT	TOTAL	---	C1
AR	37-11-4	-65	-65	---	FRONT	VERT	TOTAL	---	C1
AS	1-11-4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AT	3-11-4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AU	5-11-4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AV	7-11-4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AW	13-11-4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AX	15-11-4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AY	17-11-4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AZ	19-11-4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
BA	21-11-4	-68	-68	---	FRONT	VERT	TOTAL	---	C1
BB	23-11-4	-64	-64	---	FRONT	VERT	TOTAL	---	C1
BC	27-11-4	-64	-64	---	FRONT	VERT	TOTAL	---	C1
BD	35-11-4	-64	-64	---	FRONT	VERT	TOTAL	---	C1
BE	37-11-4	-64	-64	---	FRONT	VERT	TOTAL	---	C1
BF	39-11-4	-32	-32	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.2) PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpG, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.



Structural component only
DWG# T-2126651

amarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Aug 5 09:07:24 2021 Page 1
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Scale = 1/64.0

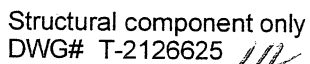


PLATES (table is in inches)					
UT	TYPE	PLATES	W	LEN	Y X
B	TMWV-t	MT20	6.0	10.0	
C	TTWW-w	MT20	6.0	10.0	3.25 1.50
D, E, I, J					
D	TMWV-t	MT20	5.0	6.0	
F	TS-t	MT20	5.0	6.0	
G	TMW-w	MT20	3.0	6.0	
H	TS-t	MT20	5.0	6.0	
K	TTWW-w	MT20	6.0	10.0	3.25 1.50
L	TMWV-t	MT20	6.0	10.0	
N	BMV1-t	MT20	6.0	9.0	Edge 0.50
P	BMWV-t	MT20	5.0	6.0	2.50 2.75
Q	BMWV-t	MT20	6.0	9.0	
R	BS-t	MT20	5.0	6.0	
S	BMWVW-t	MT20	5.0	8.0	
T	BS-t	MT20	6.0	9.0	
U	BMWV-t	MT20	5.0	6.0	
V	BMWV-t	MT20	6.0	9.0	

TOTAL LOAD CASES: (18)

(80 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 30.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ISI GRIP= 0.89 (K) (INPUT = 0.90)
ISI METAL= 0.99 (T) (INPUT = 1.00)



JOB NAME 420538	TRUSS NAME T2	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Aug 5 09:07:24 2021 Page 2
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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
W	BMWW-t	MT20	5.0	6.0	2.50	2.75
X	BMV1+t	MT20	6.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

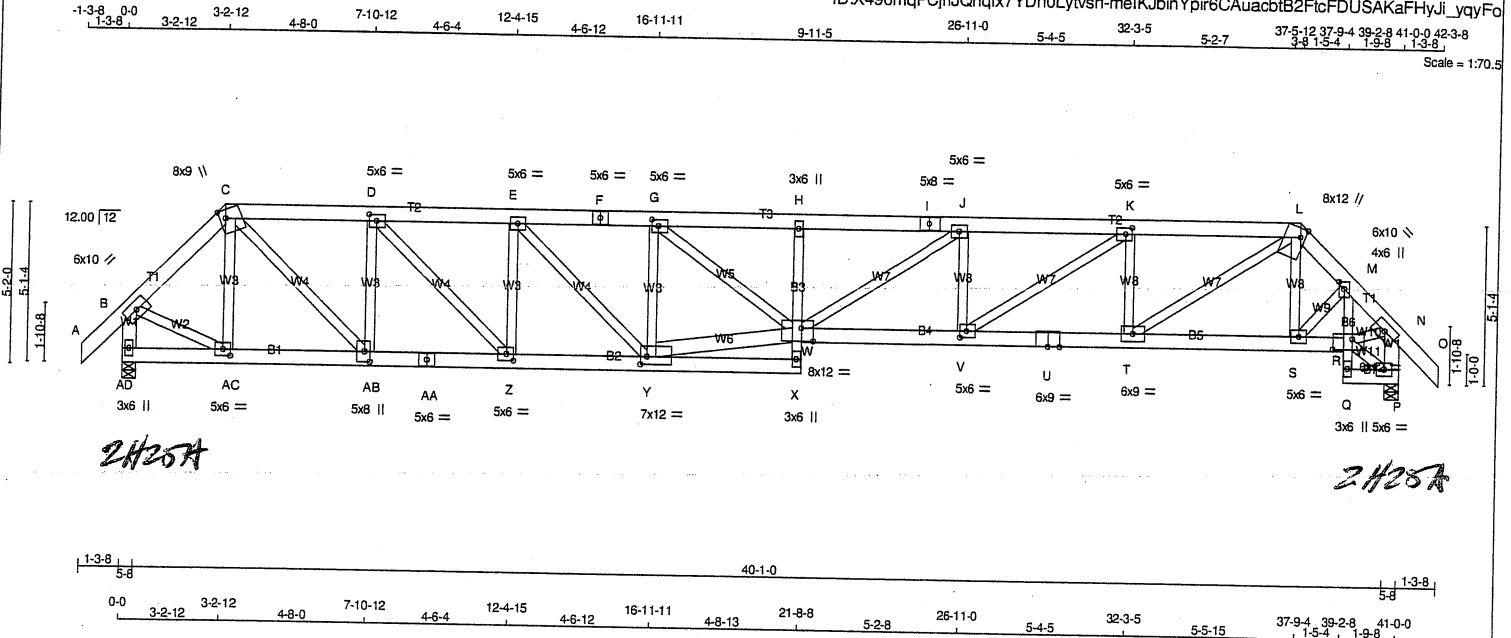
WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.2) PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2), BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.



Structural component only
DWG# T-2126625 *me*

JOB NAME 420540	TRUSS NAME T2S	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.420 S Jan 21 2021 MITek Industries, Inc. Thu Aug 5 09:39:23 2021 Page 1
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 Scale = 1:70.5



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF	
C - F	2x6	DRY	No.2	SPF	
F - I	2x6	DRY	No.2	SPF	
I - L	2x6	DRY	No.2	SPF	
L - O	2x6	DRY	No.2	SPF	
AD - B	2x6	DRY	No.2	SPF	
P - N	2x6	DRY	No.2	SPF	
AD - AA	2x6	DRY	2100F 1.8E	SPF	
AA - X	2x6	DRY	2100F 1.8E	SPF	
X - H	2x4	DRY	No.2	SPF	
W - U	2x6	DRY	2100F 1.8E	SPF	
U - R	2x6	DRY	2100F 1.8E	SPF	
Q - M	2x4	DRY	No.2	SPF	
Q - P	2x6	DRY	No.2	SPF	
ALL WEBS EXCEPT Y - W	2x4	DRY	No.2	SPF	
	2x6	DRY	2100F 1.8E	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-t	MT20	6.0	10.0		
C TTWW+m	MT20	8.0	9.0	Edge	2.25
D TMVW-t	MT20	5.0	6.0	2.50	2.75
E TMVW-t	MT20	5.0	6.0		
F TS-t	MT20	5.0	6.0		
G TMVW-t	MT20	5.0	6.0	2.50	2.50
H TMV+p	MT20	3.0	6.0		
I TS-t	MT20	5.0	8.0		
J TMVW-t	MT20	5.0	6.0		
K TMVW-t	MT20	5.0	6.0	2.50	2.50
L TTWW+m	MT20	8.0	12.0	Edge	2.00
M TMVW+p	MT20	4.0	6.0	2.75	2.00
N TMVW-t	MT20	6.0	10.0		
P BMVW-t	MT20	5.0	6.0		
Q BMV+p	MT20	3.0	6.0		
R BMVW-t	MT20	6.0	12.0	4.00	7.50
S BMVW-t	MT20	5.0	6.0		
T BMVW-t	MT20	6.0	9.0		
U BS-t	MT20	6.0	9.0		
V BMVW-t	MT20	5.0	6.0	2.50	2.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQRD
JT	VERT	HORZ	DOWN	HORZ
AD	3117	0	3464	-268
P	3123	0	3467	0

PROVIDE ANCHORAGE AT BEARING JOINT AD FOR 1111 LBS FACTORED UPLIFT
 PROVIDE ANCHORAGE AT BEARING JOINT P FOR 1114 LBS FACTORED UPLIFT

PROVIDE FOR 268 LBS FACTORED HORIZONTAL REACTION AT JOINT AD

UNFACTORED REACTIONS

JT	1ST LCASE	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
AD	2316	1550 / 0	431 / 0	0 / 0	267 / -1066	567 / 0	0 / 0	0 / 0
P	2320	1552 / 0	431 / 0	0 / 0	156 / -1069	567 / 0	0 / 0	0 / 0

HORIZONTAL REACTIONS

AD	—	0 / 0	0 / 0	0 / 0	191 / -191	0 / 0	0 / 0
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BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) AD, P

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 2.44 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 6.21 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (18)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED HORIZ. LOAD (LBS)	MAX. FACTORED UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)
FR-TO						FR-TO			
A-B		0 / 62	-105.2	-105.2	0.09 (2)	10.00	AC-C	-645 / 250	0.15 (2)
B-C		-3094 / 1063	-105.2	-105.2	0.21 (13)	4.69	S-L	-80 / 168	0.03 (17)
C-D		-4554 / 1625	-105.2	-105.2	0.29 (2)	3.89	S-M	-18 / 196	0.03 (2)
D-E		-6136 / 2125	-105.2	-105.2	0.38 (2)	3.33	B-AC	-614 / 2332	0.37 (2)
E-F		-7040 / 2403	-105.2	-105.2	0.44 (2)	3.09	R-P	-62 / 50	0.01 (14)
F-G		-7040 / 2403	-105.2	-105.2	0.44 (2)	3.09	R-N	-706 / 2648	0.43 (3)
G-H		-9156 / 3064	-105.2	-105.2	0.66 (2)	2.54	Y-G	-2234 / 794	0.53 (3)
H-I		-9195 / 3076	-105.2	-105.2	0.76 (2)	2.44	Y-W	-2186 / 7097	0.34 (2)
I-J		-9195 / 3076	-105.2	-105.2	0.76 (2)	2.44	G-W	-818 / 2659	0.43 (3)
J-K		-8345 / 2768	-105.2	-105.2	0.66 (3)	2.66	V-J	-1258 / 536	0.20 (2)
K-L		-6229 / 2098	-105.2	-105.2	0.49 (3)	3.21	T-K	-2136 / 819	0.34 (2)
L-M		-3878 / 1302	-105.2	-105.2	0.17 (14)	4.27	W-J	-373 / 1062	0.18 (13)
M-N		-3824 / 1209	-105.2	-105.2	0.18 (14)	4.30	V-K	-838 / 2601	0.42 (2)
N-O		0 / 62	-105.2	-105.2	0.09 (3)	10.00	T-L	-1382 / 4211	0.68 (13)
AD-B		-3427 / 1126	0.0	0.0	0.23 (2)	5.75	AB-D	-2227 / 829	0.53 (3)
P-N		-3400 / 1094	0.0	0.0	0.23 (3)	5.77	Z-E	-1496 / 574	0.36 (3)
AD-AC		-241 / 255	-39.5	-39.5	0.04 (6)	6.25	C-AB	-1112 / 3409	0.55 (3)
AC-AB		-620 / 2151	-39.5	-39.5	0.14 (2)	6.25	D-Z	-743 / 2302	0.37 (3)
AB-AA		-1391 / 4554	-39.5	-39.5	0.26 (2)	6.25	E-Y	-432 / 1345	0.22 (3)
AA-Z		-1391 / 4554	-39.5	-39.5	0.26 (2)	6.25			
Z-Y		-1889 / 6136	-39.5	-39.5	0.33 (2)	6.25			
Y-X		-46 / 145	-39.5	-39.5	0.05 (6)	6.25			
X-W		0 / 104	0.0	0.0	0.21 (2)	10.00			
W-H		-587 / 288	0.0	0.0	0.23 (2)	7.81			
V-W		-2532 / 8345	-39.5	-39.5	0.45 (3)	6.21			
V-U		-1839 / 6230	-39.5	-39.5	0.36 (3)	6.25			
U-T		-1839 / 6230	-39.5	-39.5	0.36 (3)	6.25			
T-S		-683 / 2743	-39.5	-39.5	0.20 (3)	6.25			
S-R		-682 / 2655	-39.5	-39.5	0.20 (3)	6.25			
Q-R		0 / 41	0.0	0.0	0.08 (3)	10.00			
R-M		-255 / 106	0.0	0.0	0.08 (3)	7.81			
Q-P		-15 / 52	-39.5	-39.5	0.02 (6)	6.25			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 30.1	PSF
DL = 6.0	PSF	
BOT CH.	LL = 10.5	PSF
DL = 7.4	PSF	
TOTAL LOAD = 54.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBC 2015

THIS DESIGN COMPLIES WITH:
 - PART 4 OF CBC 2018, ABC 2019
 - PART 4 OF CBC 2012 (2019 AMENDMENT)
 - CSA 086-14
 - TPIC 2014

DESIGN ASSUMPTIONS
 - SLOPE REDUCTION FACTOR NOT USED
 - OVERHANG NOT TO BE ALTERED OR CUT OFF.

(80 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR, EQUALS 30.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (1.37")
 CALCULATED VERT. DEFL.(LL) = $L/999$ (0.45")
 ALLOWABLE DEFL.(TL) = $L/180$ (2.73")
 CALCULATED VERT. DEFL.(TL) = $L/808$ (0.61")

CSI: TC=0.76/1.00 (H-J:2), BC=0.45/1.00 (V-W:3),
 WB=0.68/1.00 (L-T:13), SSI=0.23/1.00 (K-L:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
 WIND LOAD IMPORTANCE FACTOR = 1.00
 LIVE LOAD IMPORTANCE FACTOR = 1.00
 COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (G) (INPUT = 0.90)
 JSI METAL= 0.99 (U) (INPUT = 1.00)



Structural component only
 DWG# T-2126652

CONTINUED ON PAGE 2

JOB NAME 420540	TRUSS NAME T2S	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
W	BMVWW-I	MT20	8.0	12.0	5.00	4.50
X	BMV+p	MT20	3.0	6.0		
Y	BMVWW-t	MT20	7.0	12.0	3.00	2.50
Z	BMVWW-t	MT20	5.0	6.0	2.50	2.75
AA	BS-t	MT20	5.0	6.0		
AB	BMVWW-t	MT20	5.0	8.0	4.00	2.00
AC	BMVWW-t	MT20	5.0	6.0	2.50	2.75
AD	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

**TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2.(8)**

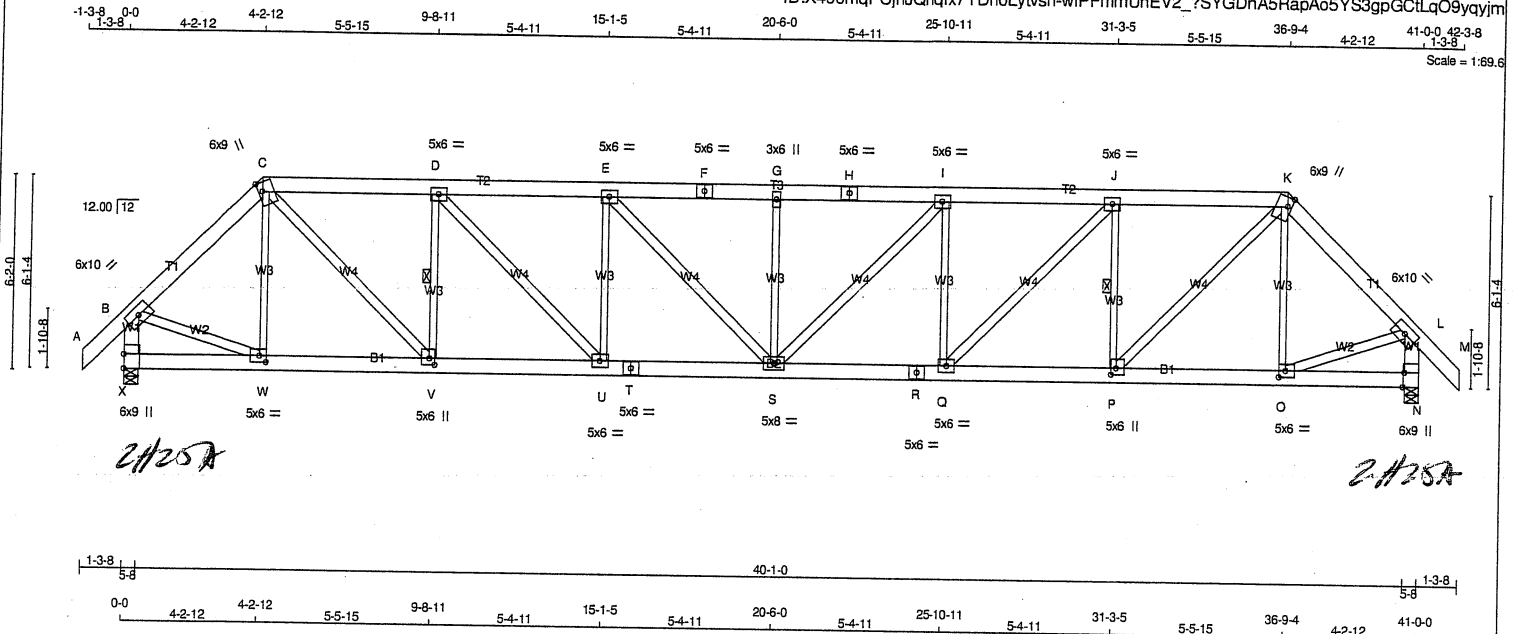
WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.2) PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, $C_p C_g$, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.



Structural component only
DWG# T-2126652 3/2

JOB NAME 420538	TRUSS NAME T3	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF	
C - F	2x6	DRY	No.2	SPF	
F - H	2x6	DRY	No.2	SPF	
H - K	2x6	DRY	No.2	SPF	
K - M	2x6	DRY	No.2	SPF	
X - B	2x6	DRY	No.2	SPF	
N - L	2x6	DRY	No.2	SPF	
X - T	2x6	DRY	No.2	SPF	
T - R	2x6	DRY	No.2	SPF	
R - N	2x6	DRY	No.2	SPF	

ALL WEBS EXCEPT	SIZE	LUMBER	DESCR.
W - C	2x3	DRY	No.2
V - D	2x3	DRY	No.2
U - E	2x3	DRY	No.2
S - G	2x3	DRY	No.2
Q - I	2x3	DRY	No.2
P - J	2x3	DRY	No.2
O - K	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMW-t	MT20	6.0	10.0		
C TTWW+m	MT20	6.0	9.0	3.50	1.50
D, E, I, J					
D TMW-t	MT20	5.0	6.0		
F TS-t	MT20	5.0	6.0		
G TMW+w	MT20	3.0	6.0		
H TS-t	MT20	5.0	6.0		
K TTWW+m	MT20	6.0	9.0	3.50	1.50
L TMW-t	MT20	6.0	10.0		
N BMW-t	MT20	6.0	9.0	Edge	0.50
O BMW-t	MT20	5.0	6.0	2.50	2.50
P BMW-t	MT20	5.0	6.0	2.50	2.00
Q BMW-t	MT20	5.0	6.0		
R BS-t	MT20	5.0	6.0		
S BMW-t	MT20	5.0	6.0		
T BS-t	MT20	5.0	6.0		
U BMW-t	MT20	5.0	6.0		
V BMW-t	MT20	5.0	6.0	2.50	2.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQD
JT	VERT	HORZ	DOWN	HORZ
X	3120	0	3454	-308
N	3120	0	3454	0

PROVIDE ANCHORAGE AT BEARING JOINT X FOR 1106 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT N FOR 1106 LBS. FACTORED UPLIFT

PROVIDE FOR 308 LBS. FACTORED HORIZONTAL REACTION AT JOINT X

UNFACTORED REACTIONS

1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED					
X	2318	1543 / 0	431 / 0	0 / 0	276 / -1063	567 / 0
N	2318	1543 / 0	431 / 0	0 / 0	151 / -1063	567 / 0

HORIZONTAL REACTIONS

X	0 / 0	0 / 0	0 / 0	220 / -220	0 / 0	0 / 0
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BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, N

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 3.37 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.09 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-V, J-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (18)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC2)	MAX. FACTORED VERT. LOAD (LC3)	MAX. FACTORED VERT. LOAD (LC4)	MAX. FACTORED VERT. LOAD (LC5)	MAX. FACTORED VERT. LOAD (LC6)	MAX. FACTORED VERT. LOAD (LC7)	MAX. FACTORED VERT. LOAD (LC8)	MAX. FACTORED VERT. LOAD (LC9)	MAX. FACTORED VERT. LOAD (LC10)	MAX. FACTORED VERT. LOAD (LC11)	MAX. FACTORED VERT. LOAD (LC12)	MAX. FACTORED VERT. LOAD (LC13)	MAX. FACTORED VERT. LOAD (LC14)	MAX. FACTORED VERT. LOAD (LC15)	MAX. FACTORED VERT. LOAD (LC16)	MAX. FACTORED VERT. LOAD (LC17)	MAX. FACTORED VERT. LOAD (LC18)
FR-TO																				
A-B	0 / 62	-105.2	-105.2	0.09 (2)	10.00	W-C	-481 / 216	0.24 (9)												
B-C	-3198 / 1087	-105.2	-105.2	0.28 (13)	4.53	C-V	-1022 / 3099	0.85 (14)												
C-D	-4364 / 1594	-105.2	-105.2	0.35 (2)	3.89	V-D	-2004 / 781	0.46 (3)												
D-E	-5564 / 1968	-105.2	-105.2	0.43 (2)	3.43	D-U	-586 / 1800	0.49 (14)												
E-F	-5955 / 2084	-105.2	-105.2	0.40 (2)	3.37	U-E	-1090 / 471	0.56 (3)												
F-G	-5955 / 2084	-105.2	-105.2	0.40 (2)	3.37	E-S	-213 / 631	0.18 (14)												
G-H	-5955 / 2084	-105.2	-105.2	0.40 (2)	3.37	S-G	-682 / 329	0.34 (2)												
H-I	-5955 / 2084	-105.2	-105.2	0.40 (2)	3.37	S-I	-214 / 631	0.18 (13)												
I-J	-5564 / 1968	-105.2	-105.2	0.43 (3)	3.43	Q-I	-1090 / 471	0.56 (2)												
J-K	-4364 / 1594	-105.2	-105.2	0.35 (3)	3.89	Q-J	-586 / 1800	0.49 (13)												
K-L	-3198 / 1088	-105.2	-105.2	0.28 (14)	4.53	P-J	-2004 / 781	0.46 (2)												
L-M	0 / 62	-105.2	-105.2	0.09 (3)	10.00	P-K	-1022 / 3099	0.85 (13)												
X-B	-3393 / 1122	0.0	0.0	0.23 (2)	5.78	O-K	-502 / 216	0.25 (10)												
N-L	-3393 / 1122	0.0	0.0	0.23 (3)	5.78	B-W	-594 / 2351	0.38 (2)												
X-W	-281 / 296	-39.5	-39.5	0.09 (6)	6.25	O-L	-594 / 2351	0.38 (3)												
W-V	-635 / 2240	-39.5	-39.5	0.34 (2)	6.25															
V-U	-1317 / 4365	-39.5	-39.5	0.58 (2)	6.25															
U-T	-1690 / 5564	-39.5	-39.5	0.73 (2)	6.09															
T-S	-1690 / 5564	-39.5	-39.5	0.73 (2)	6.09															
S-R	-1658 / 5564	-39.5	-39.5	0.73 (3)	6.13															
R-Q	-1658 / 5564	-39.5	-39.5	0.73 (3)	6.13															
Q-P	-1253 / 4365	-39.5	-39.5	0.58 (3)	6.25															
P-O	-538 / 2240	-39.5	-39.5	0.34 (3)	6.25															
O-N	-12 / 27	-39.5	-39.5	0.09 (5)	6.25															

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2.(8)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 30.1 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.4 PSF
TOTAL LOAD = 54.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF BCBC 2018, ABC 2019
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- SLOPE REDUCTION FACTOR NOT USED

(80 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 30.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.37")
CALCULATED VERT. DEFL.(LL)= L/999 (0.26")
ALLOWABLE DEFL.(TL)= L/180 (2.73")
CALCULATED VERT. DEFL.(TL)= L/999 (0.35")

CSI: TC=0.43/1.00 (D-E-2), BC=0.73/1.00 (S-U-2), WB=0.85/1.00 (C-V:14), SSI=0.24/1.00 (J-K-2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE (PSI) DRY (PLI) SHEAR SECTION (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (T) (INPUT = 0.90)
JSI METAL= 0.97 (T) (INPUT = 1.00)



Structural component only
DWG# T-2126626

CONTINUED ON PAGE 2

JOB NAME 420538	TRUSS NAME T3	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
W	BMW-t	MT20	5.0	6.0	2.50	2.50
X	BMV1-t	MT20	6.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.2) PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, $C_p C_g$, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.



Structural component only
DWG# T-2126626 *m*

CONTINUED ON PAGE 2

JOB NAME 420540	TRUSS NAME T3S	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
X	BS-t	MT20	5.0	6.0		
Y	BMWW-t	MT20	5.0	8.0	4.00	2.00
Z	BMWW-t	MT20	5.0	6.0	2.50	2.00
AA	BMV1-p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.2) PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.

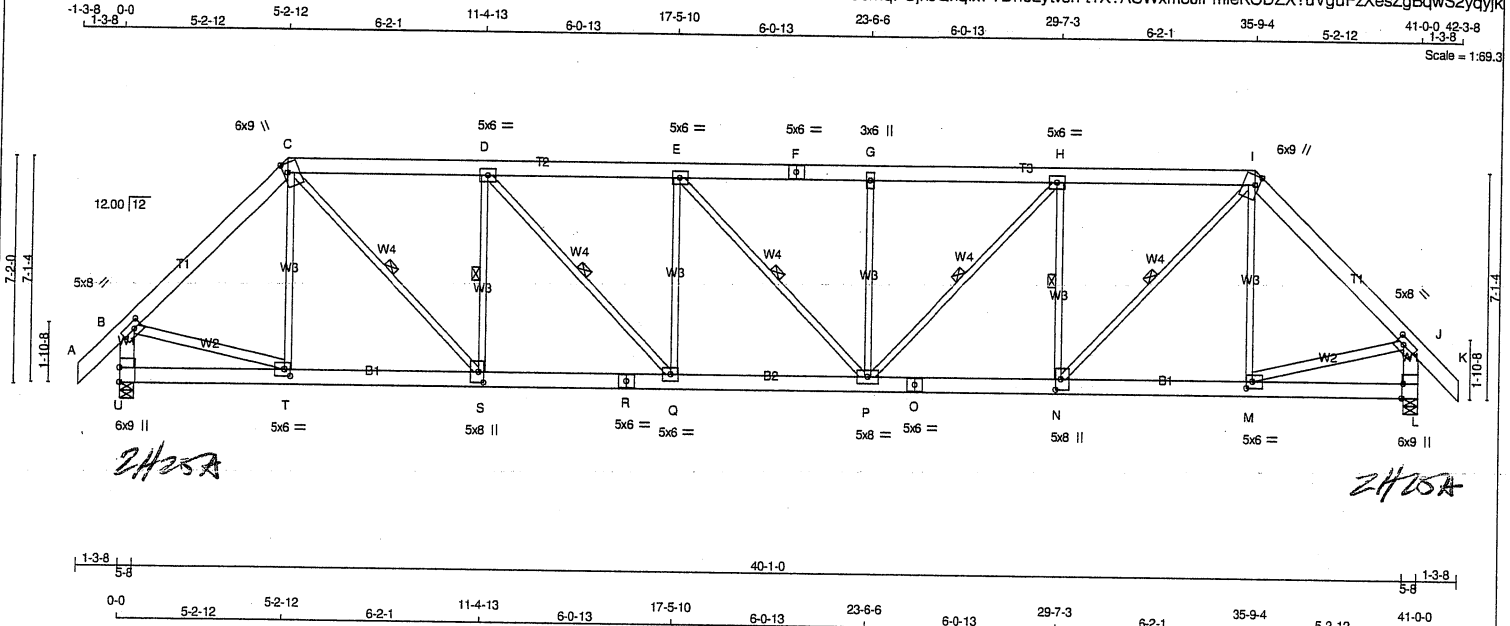


Structural component only
DWG# T-2126653 *in*

JOB NAME 420538	TRUSS NAME T4	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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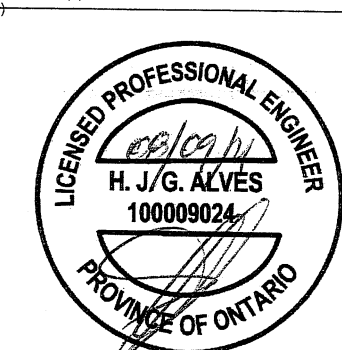


LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x6	DRY No.2	SPF
C - F	2x6	DRY No.2	SPF
F - I	2x6	DRY No.2	SPF
I - K	2x6	DRY No.2	SPF
U - B	2x6	DRY No.2	SPF
L - J	2x6	DRY No.2	SPF
U - R	2x6	DRY No.2	SPF
R - O	2x6	DRY No.2	SPF
O - L	2x6	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF
B - T	2x4	DRY No.2	SPF
M - J	2x4	DRY No.2	SPF

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMWV-t	MT20	5.0	8.0	2.50 3.00
C	TTWW+m	MT20	6.0	9.0	3.50 1.50
D, E, H					
D	TMWV-t	MT20	5.0	6.0	
F	TS-t	MT20	5.0	6.0	
G	TMW+w	MT20	3.0	6.0	
I	TTWW+m	MT20	6.0	9.0	3.50 1.50
J	TMWV-t	MT20	5.0	8.0	2.50 3.00
L	BMV1+t	MT20	6.0	9.0	Edge 0.50
M	BMWV-t	MT20	5.0	6.0	2.50 2.25
N	BMWV-t	MT20	5.0	8.0	4.00 2.00
O	BS-t	MT20	5.0	6.0	
P	BMWVW-t	MT20	5.0	8.0	
Q	BMWV-t	MT20	5.0	6.0	
R	BS-t	MT20	5.0	8.0	
S	BMWVW-t	MT20	5.0	8.0	4.00 2.00
T	BMWV-t	MT20	5.0	6.0	2.50 2.25
U	BMV1+t	MT20	6.0	9.0	5.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)



Structural component only
DWG# T-2126627

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
U	3120	0	3442	-348	-1099
L	3120	0	3442	0	-1099

PROVIDE ANCHORAGE AT BEARING JOINT U FOR 1099 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT L FOR 1099 LBS FACTORED UPLIFT

PROVIDE FOR 348 LBS FACTORED HORIZONTAL REACTION AT JOINT U

UNFACTORED REACTIONS

1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED					
U	2318	1535 / 0	431 / 0	0 / 0	285 / -1058	567 / 0
L	2318	1535 / 0	431 / 0	0 / 0	146 / -1058	567 / 0

HORIZONTAL REACTIONS

U	0 / 0	0 / 0	0 / 0	249 / -249	0 / 0	0 / 0
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BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 3.57 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-S, D-S, D-Q, E-P, H-P, H-N, I-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (18)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 62	-105.2 -105.2	0.09 (2)	10.00	T-C	-359 / 188 0.27 (9)	
B-C	-3245 / 1093	-105.2 -105.2	0.37 (13)	4.38	C-S	-923 / 2767 0.62 (3)	
C-D	-4102 / 1537	-105.2 -105.2	0.41 (2)	3.92	S-D	-1792 / 732 0.52 (3)	
D-E	-4945 / 1795	-105.2 -105.2	0.47 (2)	3.57	D-Q	-444 / 1345 0.30 (3)	
E-F	-4938 / 1791	-105.2 -105.2	0.39 (3)	3.67	Q-E	-765 / 380 0.58 (3)	
F-G	-4938 / 1791	-105.2 -105.2	0.39 (3)	3.67	E-P	-57 / 53 0.03 (2)	
G-H	-4938 / 1791	-105.2 -105.2	0.47 (3)	3.57	P-G	-736 / 354 0.55 (10)	
H-I	-4103 / 1538	-105.2 -105.2	0.42 (3)	3.92	P-H	-437 / 1334 0.30 (2)	
I-J	-3244 / 1093	-105.2 -105.2	0.37 (14)	4.38	H-N	-1789 / 730 0.52 (2)	
J-K	0 / 62	-105.2 -105.2	0.09 (3)	10.00	N-I	-924 / 2769 0.62 (2)	
U-B	-3365 / 1119	0.0	0.0 0.23 (2)	5.80	M-I	-377 / 188 0.28 (10)	
L-J	-3365 / 1119	0.0	0.0 0.23 (3)	5.80	B-T	-569 / 2353 0.38 (2)	
					M-J	-569 / 2353 0.38 (3)	
U-T	-322 / 336	-39.5	-39.5 0.12 (6)	6.25			
T-S	-633 / 2279	-39.5	-39.5 0.37 (2)	6.25			
S-R	-1218 / 4102	-39.5	-39.5 0.55 (2)	6.25			
R-Q	-1218 / 4102	-39.5	-39.5 0.55 (2)	6.25			
Q-P	-1473 / 4945	-39.5	-39.5 0.66 (2)	6.25			
P-O	-1154 / 4104	-39.5	-39.5 0.56 (3)	6.25			
O-N	-1154 / 4104	-39.5	-39.5 0.56 (3)	6.25			
N-M	-522 / 2279	-39.5	-39.5 0.37 (3)	6.25			
M-L	-12 / 27	-39.5	-39.5 0.12 (5)	6.25			

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2.(8)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 30.1 PSF
DL = 6.0 PSF	
BOT CH.	LL = 10.5 PSF
DL = 7.4 PSF	
TOTAL LOAD = 54.0 PSF	

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 4 OF BCBC 2018, ABC 2019
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- SLOPE REDUCTION FACTOR NOT USED

(80 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR = 1.00
30.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.37")
CALCULATED VERT. DEFL.(LL) = L/999 (0.22")
ALLOWABLE DEFL.(TL) = L/180 (2.73")
CALCULATED VERT. DEFL.(TL) = L/999 (0.30")

CSI: TC=0.47/1.00 (G-H:3), BC=0.66/1.00 (P-Q:2), WB=0.62/1.00 (H-N:2), SS=0.27/1.00 (H-I:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (N) (INPUT = 0.90)
JSI METAL= 0.76 (O) (INPUT = 1.00)

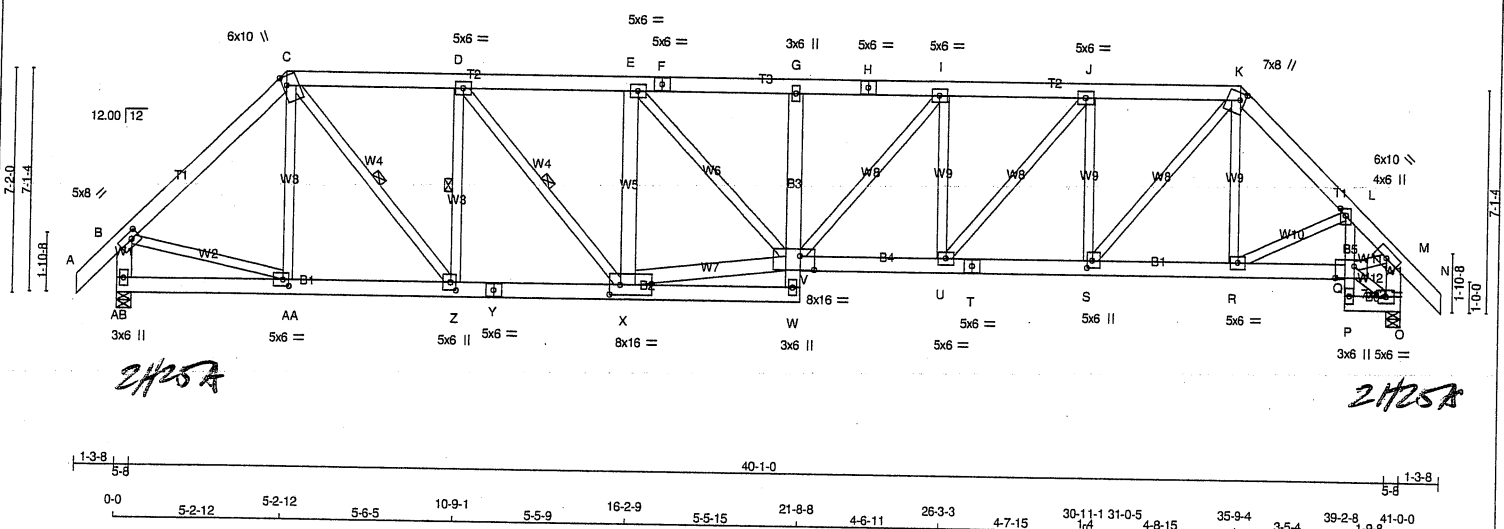
JOB NAME 420538	TRUSS NAME T4	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.2) PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.



Structural component only
DWG# T-2126627



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x6	DRY No.2	SPF
C - F	2x6	DRY No.2	SPF
F - H	2x6	DRY No.2	SPF
H - K	2x6	DRY No.2	SPF
K - N	2x6	DRY No.2	SPF
AB - B	2x6	DRY No.2	SPF
O - M	2x6	DRY No.2	SPF
AB - Y	2x6	DRY 2100F 1.8E	SPF
Y - W	2x6	DRY 2100F 1.8E	SPF
W - G	2x6	DRY No.2	SPF
V - T	2x6	DRY 2100F 1.8E	SPF
T - Q	2x6	DRY 2100F 1.8E	SPF
P - L	2x4	DRY No.2	SPF
P - O	2x4	DRY No.2	SPF
ALL WEBS EXCEPT X - E	2x4	DRY No.2	SPF
X - V	2x6	DRY 2100F 1.8E	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.50	3.00
C	TTWW+m	MT20	6.0	10.0	3.50	1.50
D, E, I, J						
D	TMVW-t	MT20	5.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMV+p	MT20	3.0	6.0		
H	TS-t	MT20	5.0	6.0		
K	TTWW+m	MT20	7.0	8.0	2.75	2.00
L	TMVW+p	MT20	4.0	6.0	2.75	2.00
M	TMVW-t	MT20	6.0	10.0		
O	BMVW1-t	MT20	5.0	6.0		
P	BMV+p	MT20	3.0	6.0		
Q	BMVWV-t	MT20	7.0	12.0	4.50	7.00
R	BMVW-t	MT20	5.0	6.0		
S	BMVW-t	MT20	5.0	6.0	2.75	2.00
T	BS-t	MT20	5.0	6.0		
U	BMVW-t	MT20	5.0	6.0		
V	BMVWVW-t	MT20	8.0	16.0	5.25	5.50
W	BMV+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	BRG	REQRD
AB	3117	0	3442	-348	-1098	5-8	5-8	
O	3123	0	3445	0	-1101	5-8	5-8	

PROVIDE ANCHORAGE AT BEARING JOINT AB FOR 1098 LBS. FACTORED UPLIFT
 PROVIDE ANCHORAGE AT BEARING JOINT O FOR 1101 LBS. FACTORED UPLIFT

PROVIDE FOR 348 LBS. FACTORED HORIZONTAL REACTION AT JOINT AB

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN.	COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
AB	2316	1535 / 0	430 / 0	0 / 0	286 / -1057	567 / 0	0 / 0	0 / 0	0 / 0
O	2320	1537 / 0	431 / 0	0 / 0	145 / -1060	567 / 0	0 / 0	0 / 0	0 / 0

HORIZONTAL REACTIONS
 AB --- 0 / 0 0 / 0 0 / 0 249 / -249 0 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) AB, O

BRACING
 MAX. UNBRACED TOP CHORD LENGTH = 3.39 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-Z, C-Z, D-X.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
 TOTAL LOAD CASES: (18)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO				FR-TO			
A-B	0 / 62	-105.2	-105.2 0.09 (2)	AA-C	-365 / 186	0.20 (9)	
B-C	-3242 / 1093	-105.2	-105.2 0.37 (13)	X-E	-1772 / 647	0.62 (3)	
C-D	-3941 / 1486	-105.2	-105.2 0.34 (2)	X-V	-1386 / 4676	0.22 (2)	
D-E	-4817 / 1751	-105.2	-105.2 0.39 (2)	E-V	-443 / 1531	0.34 (14)	
E-F	-5832 / 2055	-105.2	-105.2 0.40 (2)	R-K	-38 / 236	0.04 (17)	
F-G	-5832 / 2055	-105.2	-105.2 0.40 (2)	R-L	-108 / 201	0.03 (14)	
G-H	-5876 / 2068	-105.2	-105.2 0.33 (3)	B-AA	-567 / 2350	0.38 (2)	
H-I	-5876 / 2068	-105.2	-105.2 0.33 (3)	V-I	-1920 / 738	0.70 (2)	
I-J	-5445 / 1900	-105.2	-105.2 0.35 (3)	U-I	-1133 / 487	0.41 (2)	
J-K	-4407 / 1580	-105.2	-105.2 0.29 (3)	V-I	-276 / 757	0.19 (13)	
K-L	-3779 / 1288	-105.2	-105.2 0.23 (14)	U-J	-558 / 1712	0.38 (13)	
L-M	-3844 / 1236	-105.2	-105.2 0.19 (14)	S-J	-920 / 2770	0.63 (13)	
M-N	0 / 62	-105.2	-105.2 0.09 (3)	Z-D	-1864 / 733	0.39 (3)	
AB-B	-3359 / 1118	0.0	0.0 0.23 (2)	C-Z	-895 / 2684	0.43 (3)	
O-M	-3377 / 1081	0.0	0.0 0.23 (3)	D-X	-477 / 1481	0.24 (3)	
AB-AA	-322 / 336	-39.5	-39.5 0.06 (17)	Q-O	-63 / 50	0.01 (14)	
AA-Z	-631 / 2275	-39.5	-39.5 0.15 (2)	Q-M	-719 / 2708	0.44 (3)	
Z-Y	-1168 / 3941	-39.5	-39.5 0.22 (2)				
Y-X	-1168 / 3941	-39.5	-39.5 0.22 (2)				
X-W	-76 / 246	-39.5	-39.5 0.06 (6)				
W-V	0 / 113	0.0	0.0 0.18 (3)				
V-G	-620 / 301	0.0	0.0 0.25 (3)				
V-U	-1569 / 5445	-39.5	-39.5 0.30 (3)				
U-T	-1212 / 4408	-39.5	-39.5 0.24 (3)				
T-S	-1212 / 4408	-39.5	-39.5 0.24 (3)				
S-R	-611 / 2656	-39.5	-39.5 0.16 (3)				
R-Q	-696 / 2717	-39.5	-39.5 0.16 (3)				
P-Q	0 / 41	0.0	0.0 0.08 (3)				
Q-L	-239 / 123	0.0	0.0 0.08 (3)				
P-O	-15 / 52	-39.5	-39.5 0.02 (6)				

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 30.1 PSF
 DL = 6.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 54.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 4 OF CBC 2018, ABC 2019
 - PART 4 OF OBC 2012 (2019 AMENDMENT)
 - CSA 086-14
 - TPIC 2014

DESIGN ASSUMPTIONS
 - SLOPE REDUCTION FACTOR NOT USED
 - OVERHANG NOT TO BE ALTERED OR CUT OFF.

(80 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR. EQUALS 30.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.37")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.23")
 ALLOWABLE DEFL.(TL) = L/180 (2.73")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.31")

CSI: TC=0.40/1.00 (E-G-2), BC=0.30/1.00 (U-V-3), WB=0.70/1.00 (J-S-2), GSI=0.24/1.00 (C-D-3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
 WIND LOAD IMPORTANCE FACTOR = 1.00
 LIVE LOAD IMPORTANCE FACTOR = 1.00
 COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF
 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (Z) (INPUT = 0.90)
 JSI METAL= 0.79 (T) (INPUT = 1.00)



Structural component only
 DWG# T-2126654

JOB NAME 420540	TRUSS NAME T4S	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
X	BMWVW-t	MT20	8.0	16.0	3.75	4.00
Y	BS-t	MT20	5.0	6.0		
Z	BMWVW-t	MT20	5.0	6.0	3.00	2.00
AA	BMWVW-t	MT20	5.0	6.0	2.50	2.25
AB	BMV1+p	MT20	3.0	6.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

**TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2.(8)**

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.2) PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, $C_p C_g$, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.

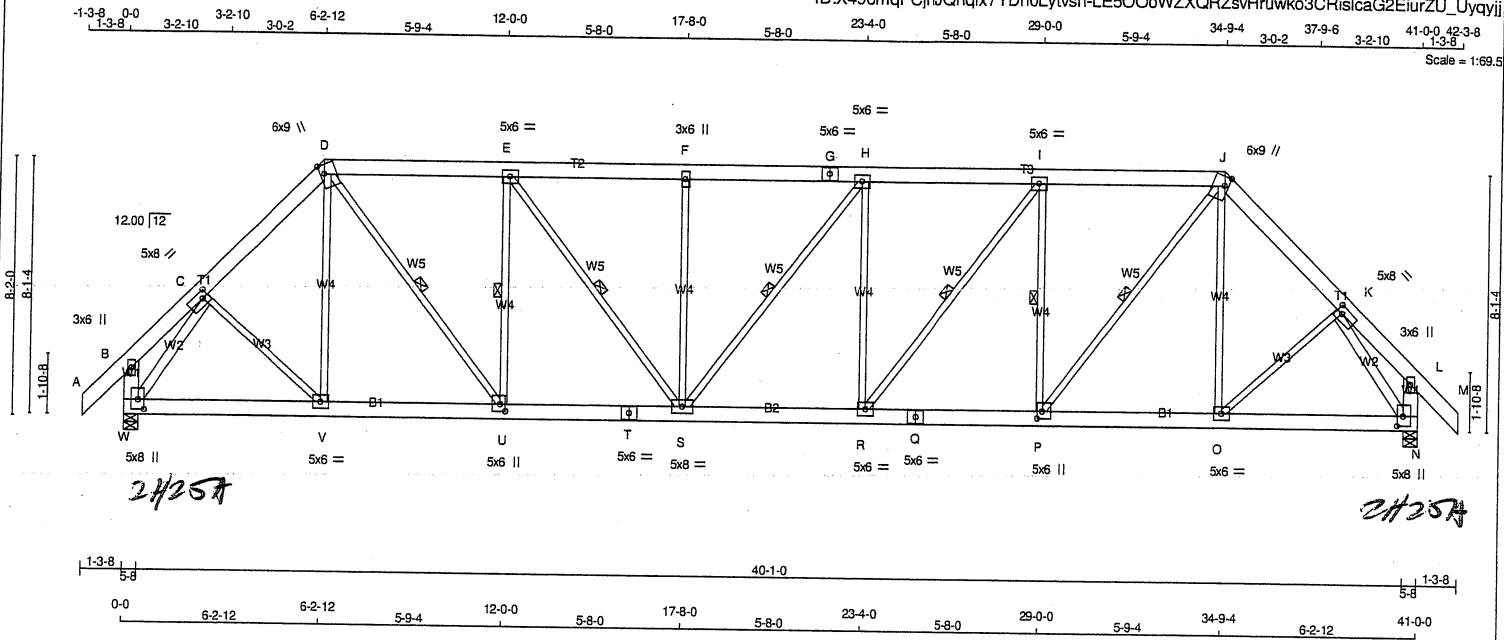


Structural component only
DWG# T-2126654 *ML*

JOB NAME 420538	TRUSS NAME T5	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - G	2x6	DRY	No.2	SPF
G - J	2x6	DRY	No.2	SPF
J - M	2x6	DRY	No.2	SPF
W - B	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
W - T	2x6	DRY	No.2	SPF
T - Q	2x6	DRY	No.2	SPF
Q - N	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
W - C	2x4	DRY	No.2	SPF
K - N	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	6.0	
C	TMWW-t	MT20	5.0	8.0	2.50 2.25
D	TTWW+m	MT20	6.0	9.0	3.50 1.50
E, H, I					
E	TMWW-t	MT20	5.0	6.0	
F	TMWW+m	MT20	3.0	6.0	
G	TS-t	MT20	5.0	6.0	
J	TTWW+m	MT20	6.0	9.0	3.50 1.50
K	TMWW-t	MT20	5.0	8.0	2.50 2.25
L	TMV+p	MT20	3.0	6.0	
N	BMWW1+p	MT20	5.0	8.0	3.75 2.25
O, R, V					
O	BMWW-t	MT20	5.0	6.0	
P	BMWW+m	MT20	5.0	6.0	2.75 2.00
Q	BS-t	MT20	5.0	6.0	
S	BMWWW-t	MT20	5.0	8.0	
T	BS-t	MT20	5.0	6.0	
U	BMWWW-t	MT20	5.0	6.0	2.75 2.00
W	BMWW1+p	MT20	5.0	8.0	3.75 2.25

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQRD
	VERT	HORZ	DOWN	HORZ
JT	3120	0	3427	-388
W	3120	0	3427	0
N	3120	0	3427	0

PROVIDE ANCHORAGE AT BEARING JOINT W FOR 1091 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT N FOR 1091 LBS. FACTORED UPLIFT

PROVIDE FOR 388 LBS. FACTORED HORIZONTAL REACTION AT JOINT W

UNFACTORED REACTIONS

1ST LCASE	SNOW	MAX. MIN. LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	2318	1525 / 0	431 / 0	0 / 0	293 / -1052	567 / 0
W	2318	1525 / 0	431 / 0	0 / 0	143 / -1052	567 / 0
N	2318	1525 / 0	431 / 0	0 / 0	143 / -1052	567 / 0

HORIZONTAL REACTIONS
W 0 / 0 0 / 0 0 / 0 277 / -277 0 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, N

BRACING
MAX. UNBRACED TOP CHORD LENGTH = 3.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.
1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-U, E-U, E-S, H-S, I-R, I-P, J-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (18)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 62	-105.2 -105.2	0.09 (2)	C-V	-40 / 631	0.14 (3)	
B-C	-46 / 215	-105.2 -105.2	0.11 (13)	V-D	-195 / 190	0.22 (10)	
C-D	-3253 / 1223	-105.2 -105.2	0.23 (13)	D-U	-809 / 2386	0.54 (3)	
D-E	-3644 / 1421	-105.2 -105.2	0.35 (2)	U-E	-1686 / 689	0.64 (3)	
E-F	-4262 / 1603	-105.2 -105.2	0.38 (2)	E-S	-384 / 1165	0.26 (3)	
F-G	-4262 / 1603	-105.2 -105.2	0.31 (2)	S-F	-686 / 330	0.76 (10)	
G-H	-4262 / 1603	-105.2 -105.2	0.31 (2)	F-H	-74 / 64	0.05 (3)	
H-I	-4269 / 1607	-105.2 -105.2	0.37 (3)	R-H	-719 / 365	0.81 (2)	
I-J	-3642 / 1420	-105.2 -105.2	0.34 (3)	R-I	-392 / 1178	0.27 (2)	
J-K	-3253 / 1224	-105.2 -105.2	0.23 (14)	P-I	-1691 / 691	0.64 (2)	
K-L	-45 / 214	-105.2 -105.2	0.11 (14)	P-J	-807 / 2384	0.54 (2)	
L-M	0 / 62	-105.2 -105.2	0.09 (3)	O-J	-179 / 204	0.20 (9)	
W-B	-284 / 254	0.0 0.0	0.03 (13)	O-K	-116 / 631	0.14 (2)	
N-L	-284 / 253	0.0 0.0	0.03 (14)	W-C	-3607 / 1019	0.72 (2)	
				K-N	-3608 / 1020	0.72 (3)	
W-V	-728 / 1938	-39.5 -39.5	0.34 (2)				
V-U	-618 / 2276	-39.5 -39.5	0.37 (2)				
U-T	-1058 / 3644	-39.5 -39.5	0.50 (2)				
T-S	-1058 / 3644	-39.5 -39.5	0.50 (2)				
S-R	-1219 / 4269	-39.5 -39.5	0.58 (3)				
R-Q	-986 / 3642	-39.5 -39.5	0.49 (3)				
Q-P	-986 / 3642	-39.5 -39.5	0.49 (3)				
P-O	-498 / 2276	-39.5 -39.5	0.38 (3)				
O-N	-521 / 1938	-39.5 -39.5	0.34 (3)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 30.1 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.4 PSF
TOTAL LOAD = 54.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF CBC 2018, ABC 2019
- PART 4 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR NOT USED

(80 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 30.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.37")
CALCULATED VERT. DEFL.(LL) = L/999 (0.18")
ALLOWABLE DEFL.(TL)= L/180 (2.73")
CALCULATED VERT. DEFL.(TL) = L/999 (0.25")

CSI: TC=0.38/1.00 (E-F:2), BC=0.58/1.00 (R-S:3), WB=0.81/1.00 (H-R:2), SS=0.25/1.00 (D-E:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	MAX MIN	MAX MIN	MAX MIN
MT20	650	371	1747	788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (K) (INPUT = 0.90)
JSI METAL= 0.70 (N) (INPUT = 1.00)



Structural component only
DWG# T-2126628

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
420538	T5	1	1	TRUSS DESC.		

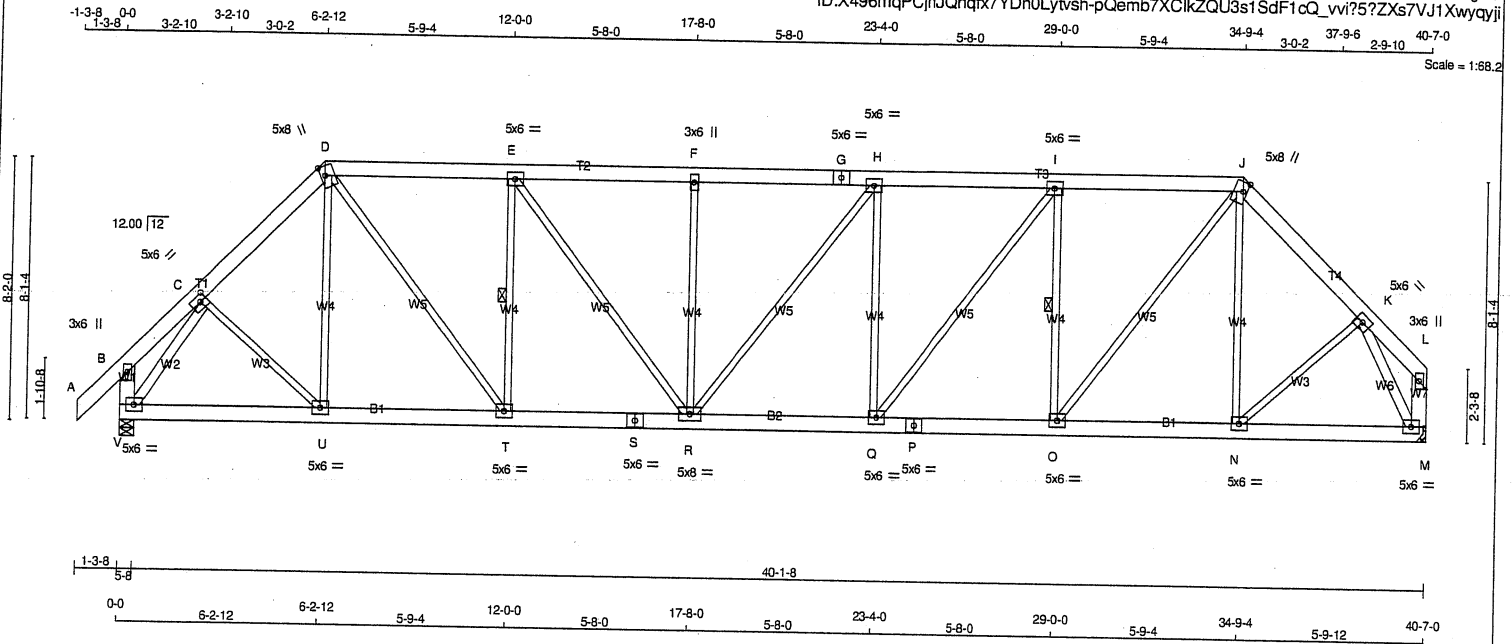
Tamarack Roof Truss, Burlington

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WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.2) PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_{pe} , BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.



Structural component only
DWG# T-2126628 *mm*



TOTAL WEIGHT = 253 lb

LUMBER

N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - G	2x6	DRY	No.2	SPF
G - J	2x6	DRY	No.2	SPF
J - L	2x6	DRY	No.2	SPF
V - B	2x6	DRY	No.2	SPF
M - L	2x6	DRY	No.2	SPF
V - S	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
V - C	2x4	DRY	No.2	SPF
K - M	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	6.0		
C TMWW-t	MT20	5.0	6.0	2.50	2.50
D TTWW+m	MT20	5.0	8.0	3.50	1.50
E, H, I, K					
E TMWW-t	MT20	5.0	6.0		
F TMW+w	MT20	3.0	6.0		
G TS-t	MT20	5.0	6.0		
J TTWW+m	MT20	5.0	8.0	3.50	1.50
L TMV+p	MT20	3.0	6.0		
M BMWW-t	MT20	5.0	6.0		
N, O, Q, T, U					
N BMWW-t	MT20	5.0	6.0		
P BS-t	MT20	5.0	6.0		
R BMWWW-t	MT20	5.0	8.0		
S BS-t	MT20	5.0	6.0		
V BMWW-t	MT20	5.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
V	2221	0	2221	0	5-8	5-8
M	2097	0	2097	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT M. MINIMUM BEARING LENGTH AT JOINT M = 3-8.

UNFACTORED REACTIONS								
JT	1ST CASE		MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
V	1574	1013 / 0	0 / 0	0 / 0	0 / 0	561 / 0	0 / 0	
M	1489	945 / 0	0 / 0	0 / 0	0 / 0	544 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.83 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF I-O, E-T.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (PLF)
	FR-TO	FROM TO	FR-TO	FROM TO
A-B	0 / 44	-84.9	-84.9	0.07 (1)
B-C	0 / 18	-84.9	-84.9	0.04 (1)
C-D	-2098 / 0	-84.9	-84.9	0.11 (1)
D-E	-2357 / 0	-84.9	-84.9	0.22 (1)
E-F	-2761 / 0	-84.9	-84.9	0.23 (1)
F-G	-2761 / 0	-84.9	-84.9	0.18 (1)
G-H	-2761 / 0	-84.9	-84.9	0.18 (1)
H-I	-2749 / 0	-84.9	-84.9	0.23 (1)
I-J	-2308 / 0	-84.9	-84.9	0.21 (1)
J-K	-1986 / 0	-84.9	-84.9	0.11 (1)
K-L	0 / 22	-84.9	-84.9	0.04 (1)
L-M	-196 / 0	0.0	0.0	0.01 (1)
M-L	-50 / 0	0.0	0.0	0.00 (1)
V-U	0 / 1260	-18.5	-18.5	0.20 (1)
U-T	0 / 1466	-18.5	-18.5	0.22 (1)
T-S	0 / 2357	-18.5	-18.5	0.32 (1)
S-R	0 / 2357	-18.5	-18.5	0.32 (1)
R-Q	0 / 2749	-18.5	-18.5	0.37 (1)
Q-P	0 / 2308	-18.5	-18.5	0.30 (1)
P-O	0 / 2308	-18.5	-18.5	0.30 (1)
O-N	0 / 1385	-18.5	-18.5	0.21 (1)
N-M	0 / 1053	-18.5	-18.5	0.18 (1)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.35")
CALCULATED VERT. DEFL.(LL) = L/999 (0.12")
ALLOWABLE DEFL.(TL) = L/360 (1.35")
CALCULATED VERT. DEFL.(TL) = L/999 (0.22")

CSI: TC=0.23/1.00 (E-F:1), BC=0.37/1.00 (Q-R:1), WB=0.55/1.00 (H-Q:1), SS=0.18/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

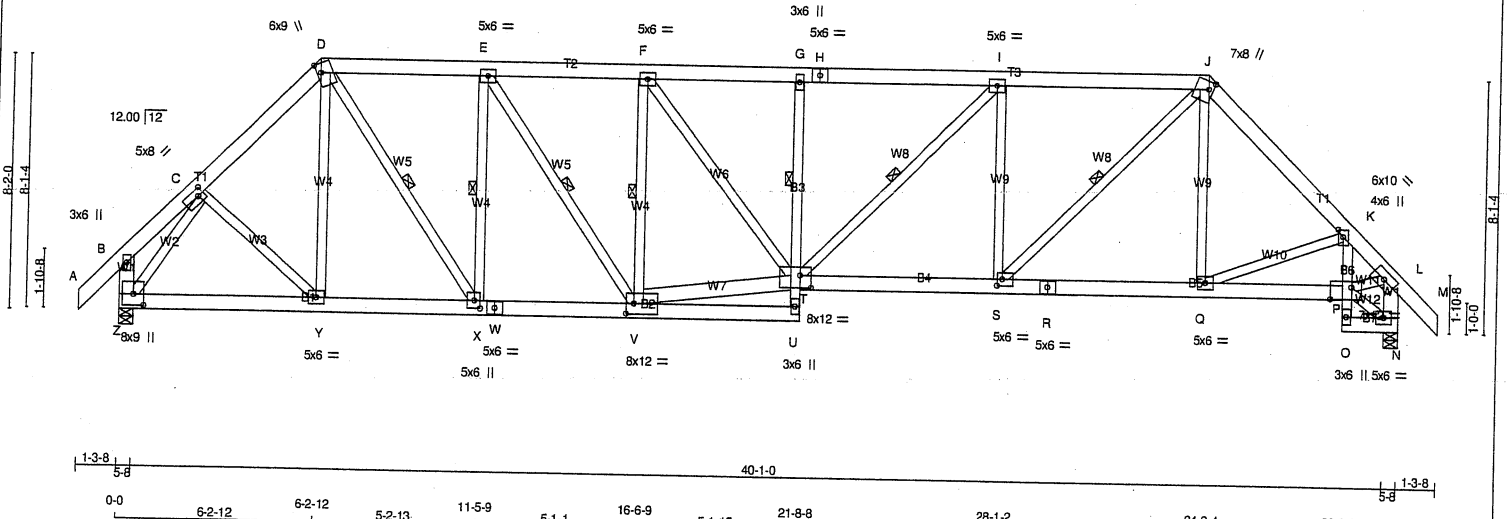
JSI GRIP= 0.90 (K) (INPUT = 0.90)
JSI METAL= 0.52 (C) (INPUT = 1.00)



JOB NAME 420540	TRUSS NAME T5S	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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Scale = 1:70.4



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x6 DRY	No.2	SPF
D - H	2x6 DRY	No.2	SPF
H - J	2x6 DRY	No.2	SPF
J - M	2x6 DRY	No.2	SPF
Z - B	2x6 DRY	No.2	SPF
N - L	2x6 DRY	No.2	SPF
Z - W	2x6 DRY	2100F 1.8E	SPF
W - U	2x6 DRY	2100F 1.8E	SPF
U - G	2x4 DRY	No.2	SPF
T - R	2x6 DRY	2100F 1.8E	SPF
R - P	2x6 DRY	2100F 1.8E	SPF
O - K	2x4 DRY	No.2	SPF
O - N	2x6 DRY	2100F 1.8E	SPF
ALL WEBS	2x4 DRY	No.2	SPF
EXCEPT			
V - T	2x6 DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	3.0	6.0
C	TMVW-t	MT20	5.0	8.0 2.50 2.25
D	TTVW+m	MT20	6.0	9.0 3.50 1.50
E, F, I				
E	TMVW-t	MT20	5.0	6.0
G	TMV+p	MT20	3.0	6.0
H	TS-t	MT20	5.0	6.0
J	TTVW+m	MT20	7.0	8.0 3.00 1.75
K	TMVW+p	MT20	4.0	6.0 2.75 2.00
L	TMVW-t	MT20	5.0	10.0
N	BMVW-t	MT20	5.0	6.0
O	BMV+p	MT20	3.0	6.0
P	BMVW-t	MT20	7.0	12.0 4.75 8.25
Q	BMVW-t	MT20	5.0	6.0
R	BS-t	MT20	5.0	6.0
S	BMVW-t	MT20	5.0	6.0 2.50 2.00
T	BMVW-t	MT20	8.0	12.0 4.75 4.25
U	BMV+p	MT20	3.0	6.0
V	BMVW-t	MT20	8.0	12.0 4.00 3.00
W	BS-t	MT20	5.0	6.0
X	BMVW-t	MT20	5.0	6.0 3.00 2.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	3117	0	3427	-388
Z	HORZ	3123	0	3430	0
N	UPLIFT			-1093	5-8

PROVIDE ANCHORAGE AT BEARING JOINT Z FOR 1089 LBS FACTORED UPLIFT

PROVIDE ANCHORAGE AT BEARING JOINT N FOR 1093 LBS FACTORED UPLIFT

PROVIDE FOR 388 LBS FACTORED HORIZONTAL REACTION AT JOINT Z

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
Z	2316	1525 / 0	431 / 0	0 / 0	294 / -1051	567 / 0	0 / 0
N	2320	1527 / 0	431 / 0	0 / 0	142 / -1054	567 / 0	0 / 0

HORIZONTAL REACTIONS

Z	0 / 0	0 / 0	0 / 0	277 / -277	0 / 0	0 / 0
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BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Z, N

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 3.44 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-T, F-V, E-X, D-X, E-V, I-T, J-S.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (18)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED UNBRACED LENGTH (FT)
FR-TO			
A-B	0 / 62	-105.2 -105.2 0.09 (2)	10.00
B-C	-46 / 212	-105.2 -105.2 0.11 (13)	6.25
C-D	-3252 / 1220	-105.2 -105.2 0.23 (13)	4.56
D-E	-3536 / 1386	-105.2 -105.2 0.30 (2)	4.30
E-F	-4176 / 1574	-105.2 -105.2 0.33 (2)	3.99
F-G	-4949 / 1800	-105.2 -105.2 0.33 (3)	3.71
G-H	-4958 / 1802	-105.2 -105.2 0.57 (3)	3.44
H-I	-4958 / 1802	-105.2 -105.2 0.57 (3)	3.44
I-J	-4381 / 1598	-105.2 -105.2 0.53 (3)	3.68
J-K	-3710 / 1267	-105.2 -105.2 0.27 (14)	4.25
K-L	-3861 / 1262	-105.2 -105.2 0.25 (14)	4.28
L-M	0 / 62	-105.2 -105.2 0.09 (3)	10.00
Z-B	-281 / 250	0.0 0.0 0.03 (13)	7.81
N-L	-3362 / 1072	0.0 0.0 0.23 (3)	5.80
FR-TO			
Y-X	-727 / 1930	-39.5 -39.5 0.14 (2)	6.25
X-W	-616 / 2275	-39.5 -39.5 0.16 (2)	6.25
W-V	-1024 / 3536	-39.5 -39.5 0.19 (2)	6.25
V-U	-1024 / 3536	-39.5 -39.5 0.19 (2)	6.25
U-T	-18 / 60	-39.5 -39.5 0.05 (17)	6.25
T-G	0 / 107	0.0 0.0 0.10 (3)	10.00
G-T	-692 / 334	0.0 0.0 0.11 (3)	6.25
T-S	-1199 / 4382	-39.5 -39.5 0.26 (3)	6.25
S-R	-569 / 2602	-39.5 -39.5 0.17 (3)	6.25
R-Q	-569 / 2602	-39.5 -39.5 0.17 (3)	6.25
Q-P	-747 / 2761	-39.5 -39.5 0.18 (3)	6.25
P-O	0 / 41	0.0 0.0 0.08 (3)	10.00
O-P	-250 / 136	0.0 0.0 0.08 (3)	7.81
O-N	-16 / 53	-39.5 -39.5 0.01 (6)	6.25

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 30.1 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.4 PSF
TOTAL LOAD = 54.0 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF CBC 2018, ABC 2019
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR NOT USED
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(80% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 30.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.37")
CALCULATED VERT. DEFL.(LL) = L/999 (0.19")
ALLOWABLE DEFL.(TL) = L/180 (2.73")
CALCULATED VERT. DEFL.(TL) = L/999 (0.26")

CSI: TC=0.57/1.00 (G-I:3), BC=0.26/1.00 (S-T:3), WB=0.87/1.00 (I-S:2), SS=0.29/1.00 (I-J:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (J) (INPUT = 0.90)
JSI METAL= 0.62 (C) (INPUT = 1.00)



Structural component only
DWG# T-2126656

CONTINUED ON PAGE 2

JOB NAME 420540	TRUSS NAME T5S	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Aug 5 09:39:28 2021 Page 2
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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
Y	BMWV-t	MT20	5.0	6.0		
Z	BMWV1+p	MT20	8.0	9.0	4.25	3.75

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

**TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2.(8)**

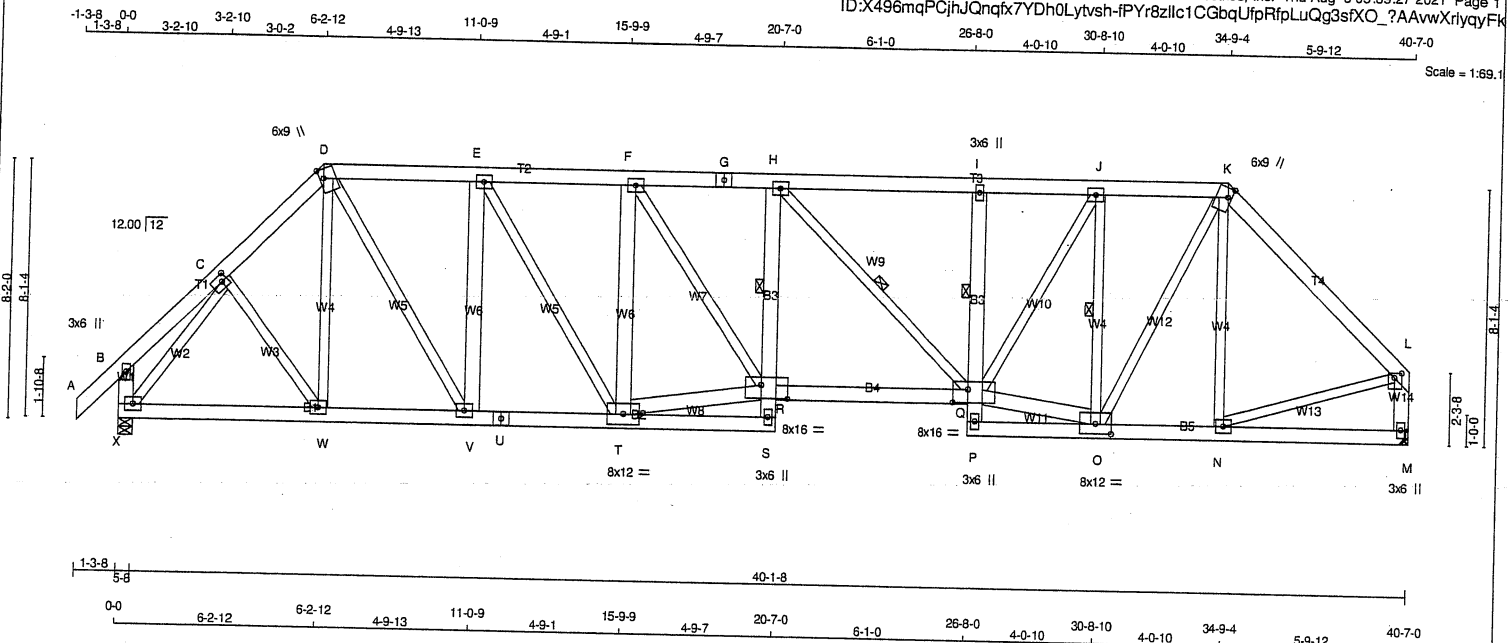
WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.2) PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.



Structural component only
DWG# T-2126656 *m*

JOB NAME 420540	TRUSS NAME T5AS	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID:X496mqPCjhQnqfx7YDh0Lytvsh-fPYr8zllc1CGbqUfpRfLuQg3sfXO_?AAvwXrlyqyFK



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x6	DRY	No.2
D - G	2x6	DRY	No.2
G - K	2x6	DRY	No.2
K - L	2x6	DRY	No.2
X - B	2x6	DRY	No.2
M - L	2x6	DRY	No.2
X - U	2x6	DRY	2100F 1.8E
U - S	2x6	DRY	2100F 1.8E
S - H	2x6	DRY	No.2
R - Q	2x6	DRY	2100F 1.8E
P - I	2x6	DRY	No.2
P - M	2x6	DRY	2100F 1.8E
ALL WEBS EXCEPT	2x4	DRY	No.2
T - F	2x6	DRY	No.2
T - R	2x6	DRY	2100F 1.8E
Q - O	2x6	DRY	2100F 1.8E
V - E	2x6	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	6.0		
C	TMVW-t	MT20	5.0	6.0	2.50	2.00
D	TTWW+m	MT20	6.0	9.0	3.50	1.50
E, F, J						
E	TMVW-t	MT20	5.0	6.0		
G	TS-t	MT20	5.0	6.0		
H	TMVW-t	MT20	5.0	6.0		
I	TMV+p	MT20	3.0	6.0		
K	TTWW+m	MT20	6.0	9.0	3.50	1.50
L	TMVW+p	MT20	5.0	6.0	1.75	2.75
M	BMV1+p	MT20	3.0	6.0		
N, V, W						
N	BMVW-t	MT20	5.0	6.0		
O	BMVWV-t	MT20	8.0	12.0	3.75	6.00
P	BMV+p	MT20	3.0	6.0		
Q	BMVWVW-I	MT20	8.0	16.0	5.00	5.75
R	BMVWVW-I	MT20	8.0	16.0	5.00	10.00
S	BMV+p	MT20	3.0	6.0		
T	BMVWVW-t	MT20	8.0	12.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQD	
GROSS	REACTION	GROSS	REACTION	BRG	BRG	IN-SX	IN-SX
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
M	2097	0	2097	0	0	MECHANICAL	
X	2221	0	2221	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT M. MINIMUM BEARING LENGTH AT JOINT M = 3-8.

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN.	COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1489	945 / 0	0 / 0	0 / 0	0 / 0	544 / 0	0 / 0
X	1574	1013 / 0	0 / 0	0 / 0	0 / 0	561 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.54 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF H-R, I-Q, H-Q, J-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED	MEMB.	MAX. FACTORED
FORCE (LBS)	VERT. LOAD (PLF)	FORCE (LBS)	MAX. FACTORED
			CS (LC)
FR-TO	FROM TO	FR-TO	FROM TO
A-B	0 / 44	C-W	0 / 200
B-C	0 / 10	W-D	-33 / 54
C-D	-2097 / 0	T-F	-1166 / 0
D-E	-2231 / 0	T-R	0 / 2620
E-F	-2655 / 0	F-R	0 / 997
F-G	-3234 / 0	H-Q	-509 / 0
G-H	-3234 / 0	Q-O	0 / 2000
H-I	-2894 / 0	Q-J	0 / 1622
I-J	-2875 / 0	O-J	-1697 / 0
J-K	-2048 / 0	O-K	0 / 1433
K-L	-1973 / 0	N-K	-295 / 0
X-B	-237 / 0	X-C	-2355 / 0
M-L	-2053 / 0	N-L	0 / 1448
		V-E	-1139 / 0
		D-V	0 / 1432
		E-T	0 / 814
X-W	0 / 1342		
W-V	0 / 1465		
V-U	0 / 2232		
U-T	0 / 2232		
T-S	0 / 111		
S-R	0 / 39		
R-H	-142 / 31		
R-Q	0 / 3251		
P-Q	0 / 33		
Q-I	-472 / 0		
P-O	0 / 128		
O-N	0 / 1384		
N-M	0 / 0		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.35")
CALCULATED VERT. DEFL.(LL) = L/999 (0.12")
ALLOWABLE DEFL.(TL) = L/360 (1.35")
CALCULATED VERT. DEFL.(TL) = L/999 (0.23")

CSI: TC=0.28/1.00 (K-L-1), BC=0.19/1.00 (Q-R-1), WB=0.70/1.00 (C-X-1), SSI=0.17/1.00 (H-I-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.85 (J) (INPUT = 0.90)
JSI METAL = 0.60 (L) (INPUT = 1.00)



Structural component only
DWG# T-2126655

CONTINUED ON PAGE 2

JOB NAME 420540	TRUSS NAME T5AS	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
U	BS-t	MT20	5.0	6.0		
X	BMVW1-t	MT20	5.0	6.0		



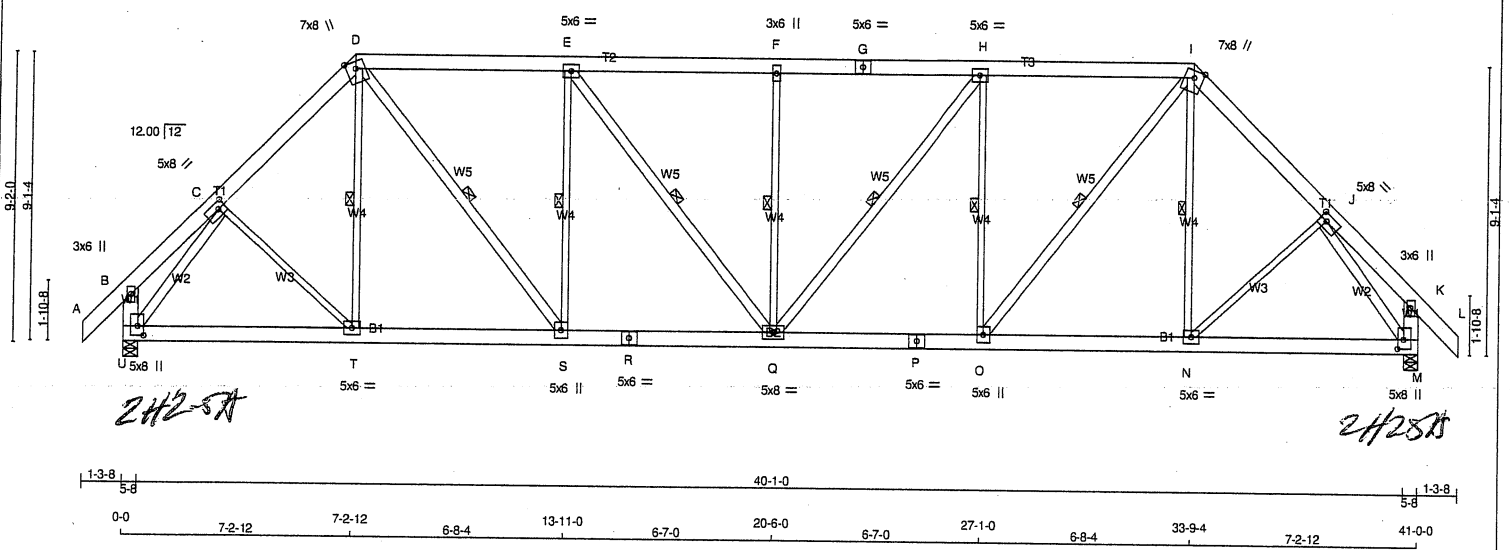
Structural component only
DWG# T-2126655

Tamarack Roof Truss, Burlington

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Scale = 1:69.5



LUMBER				N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x6	DRY	No.2	D - S	2x4	DRY	No.2
D - G	2x6	DRY	No.2	E - Q	2x4	DRY	No.2
G - I	2x6	DRY	No.2	Q - H	2x4	DRY	No.2
I - L	2x6	DRY	No.2	O - I	2x4	DRY	No.2
U - B	2x6	DRY	No.2	U - C	2x4	DRY	No.2
M - K	2x6	DRY	No.2	J - M	2x4	DRY	No.2
U - R	2x6	DRY	No.2				
R - P	2x6	DRY	No.2				
P - M	2x6	DRY	No.2				
ALL WEBS EXCEPT	2x3	DRY	No.2				
D - S	2x4	DRY	No.2				
E - Q	2x4	DRY	No.2				
Q - H	2x4	DRY	No.2				
O - I	2x4	DRY	No.2				
U - C	2x4	DRY	No.2				
J - M	2x4	DRY	No.2				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	6.0		
C	TMWW-t	MT20	5.0	8.0	2.50	2.75
D	TTWW+m	MT20	7.0	8.0	Edge	
E	TMWW-t	MT20	5.0	6.0		
F	TMW+w	MT20	3.0	6.0		
G	TS-t	MT20	5.0	6.0		
H	TMWW-t	MT20	5.0	6.0		
J	TTWW+m	MT20	7.0	8.0	Edge	
K	TMWW-t	MT20	5.0	8.0	2.50	2.75
L	TMV+p	MT20	3.0	6.0		
M	BMVV1+p	MT20	5.0	8.0	3.50	2.25
N	BMWW-t	MT20	5.0	6.0		
O	BMWW-t	MT20	5.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWW-t	MT20	5.0	8.0		
R	BS-t	MT20	5.0	6.0		
S	BMWW-t	MT20	5.0	6.0		
T	BMWW-t	MT20	5.0	6.0		
U	BMVV1+p	MT20	5.0	8.0	3.50	2.25

Structural component only
DWG# T-2126630

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
U	3120	0	3410	-428
M	3120	0	3410	0

PROVIDE ANCHORAGE AT BEARING JOINT U FOR 1081 LBS FACTORED UPLIFT

PROVIDE ANCHORAGE AT BEARING JOINT M FOR 1081 LBS FACTORED UPLIFT

PROVIDE FOR 428 LBS FACTORED HORIZONTAL REACTION AT JOINT U

UNFACTORED REACTIONS

1ST LCASE MAX/MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	2318	1514 / 0	431 / 0	0 / 0	299 / -1045	567 / 0	0 / 0
M	2318	1514 / 0	431 / 0	0 / 0	141 / -1045	567 / 0	0 / 0

HORIZONTAL REACTIONS

U	—	0 / 0	0 / 0	0 / 0	306 / -306	0 / 0	0 / 0
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BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 3.95 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-T, D-S, E-S, E-Q, F-Q, H-Q, H-O, I-O, I-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (18)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CS (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 62	-105.2 -105.2	0.09 (2)	10.00	C-T	-37 / 515	0.12 (3)
B-C	-43 / 234	-105.2 -105.2	0.15 (13)	6.25	T-D	-108 / 284	0.06 (17)
C-D	-3223 / 1232	-105.2 -105.2	0.26 (13)	4.55	D-S	-748 / 2164	0.45 (14)
D-E	-3477 / 1394	-105.2 -105.2	0.47 (2)	4.12	S-E	-1455 / 640	0.72 (3)
E-F	-3800 / 1477	-105.2 -105.2	0.49 (2)	3.95	E-Q	-238 / 705	0.14 (14)
F-G	-3800 / 1477	-105.2 -105.2	0.49 (3)	3.95	Q-F	-761 / 366	0.36 (2)
G-H	-3800 / 1477	-105.2 -105.2	0.49 (3)	3.95	H-Q	-239 / 705	0.14 (13)
H-I	-3477 / 1395	-105.2 -105.2	0.47 (3)	4.12	O-H	-1455 / 640	0.72 (2)
I-J	-3223 / 1232	-105.2 -105.2	0.26 (14)	4.55	O-I	-748 / 2164	0.45 (13)
J-K	-42 / 233	-105.2 -105.2	0.15 (14)	6.25	N-I	-96 / 296	0.06 (17)
K-L	0 / 62	-105.2 -105.2	0.09 (3)	10.00	N-J	-98 / 515	0.12 (2)
U-B	-300 / 278	0.0	0.0 (3)	7.81	U-C	-3618 / 1002	0.97 (2)
M-K	-300 / 277	0.0	0.0 (3)	7.81	J-M	-3618 / 1003	0.97 (3)
U-T	-771 / 2036	-39.5 -39.5	0.37 (2)	6.25			
T-S	-595 / 2253	-39.5 -39.5	0.39 (2)	6.25			
S-R	-989 / 3477	-39.5 -39.5	0.49 (2)	6.25			
R-Q	-989 / 3477	-39.5 -39.5	0.49 (2)	6.25			
Q-P	-925 / 3477	-39.5 -39.5	0.49 (3)	6.25			
P-O	-925 / 3477	-39.5 -39.5	0.49 (3)	6.25			
O-N	-466 / 2253	-39.5 -39.5	0.39 (3)	6.25			
N-M	-538 / 2036	-39.5 -39.5	0.37 (3)	6.25			

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2.(8)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL =	30.1	PSF
DL =	6.0	PSF
BOT CH. LL =	10.5	PSF
DL =	7.4	PSF
TOTAL LOAD =	54.0	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 4 OF BCBC 2018, ABC 2019
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- SLOPE REDUCTION FACTOR NOT USED

(80 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 30.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.37")

CALCULATED VERT. DEFL.(LL)= L/999 (0.14")

ALLOWABLE DEFL.(TL)= L/180 (2.73")

CALCULATED VERT. DEFL.(TL)= L/999 (0.20")

CSI: TC=0.49/1.00 (F-H:3), BC=0.49/1.00 (O-Q:3),
WB=0.97/1.00 (J-M:3), SSI=0.29/1.00 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)

JSI METAL= 0.73 (U) (INPUT = 1.00)

CONTINUED ON PAGE 5



JOB NAME 420538	TRUSS NAME T6	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

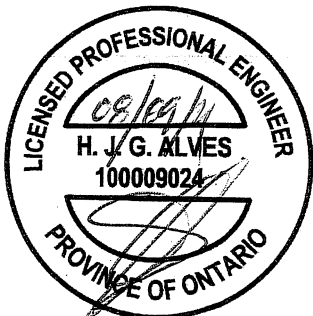
Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Aug 5 09:07:30 2021 Page 2
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Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)

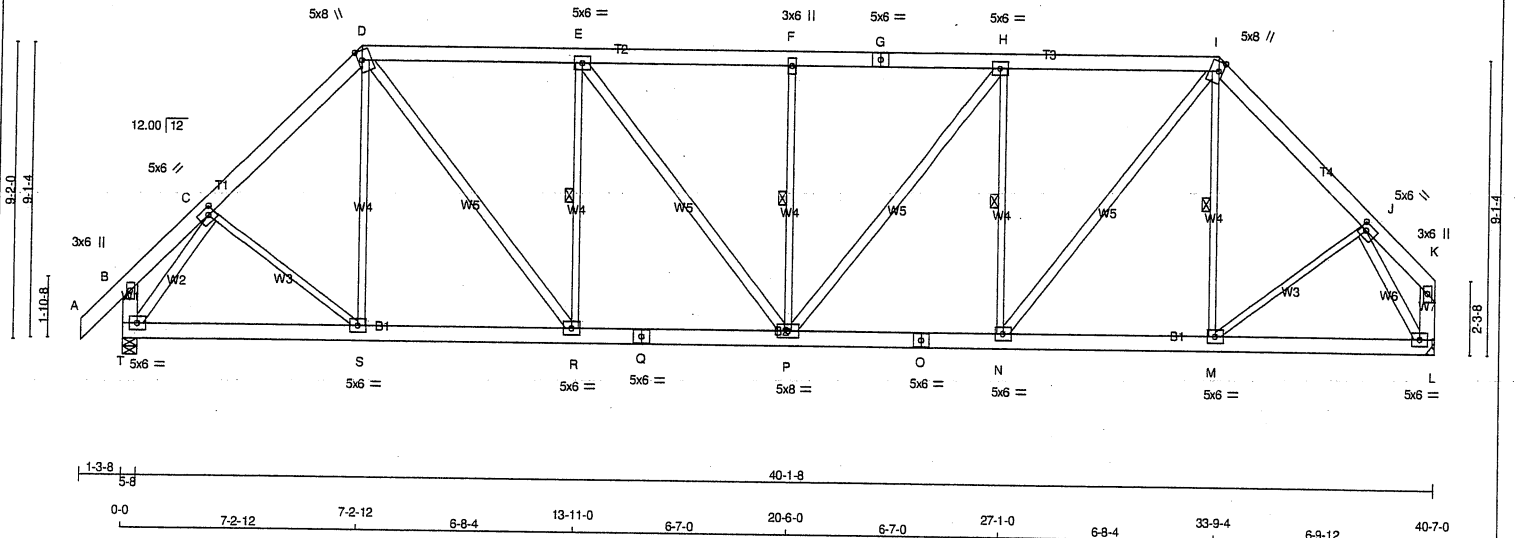
1) Lateral braces to be a minimum of 2X4 SPF #2.

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.2} PSF AT
{40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK
COEFFICIENTS, $C_p C_g$, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL
WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON
(OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY
FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF
AND 5.0 PSF RESPECTIVELY.



Structural component only
DWG# T-2126630 *ML*

JOB NAME 420538	TRUSS NAME T6A	QUANTITY 7	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Aug 5 09:07:31 2021 Page 1	
ID: X496mqPCjhuQnqk7YDh0Lyvsh-lpmW0pZSqLp7jN0QZ2HVhr3ECWISTTZ9bpo8bpyqyig				Scale = 1:67.9	



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x6	DRY	No.2
D - G	2x6	DRY	No.2
G - I	2x6	DRY	No.2
I - K	2x6	DRY	No.2
L - B	2x6	DRY	No.2
L - K	2x6	DRY	No.2
T - Q	2x6	DRY	No.2
Q - O	2x6	DRY	No.2
O - L	2x6	DRY	No.2

ALL WEBS EXCEPT			
CHORDS	SIZE	LUMBER	DESCR.
D - R	2x4	DRY	No.2
E - P	2x4	DRY	No.2
P - H	2x4	DRY	No.2
N - I	2x4	DRY	No.2
T - C	2x4	DRY	No.2
J - L	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	6.0		
C TMWW-t	MT20	5.0	6.0	2.50	2.50
D TTWW+m	MT20	5.0	8.0	3.50	1.50
E TMWW-t	MT20	5.0	6.0		
F TMW+w	MT20	3.0	6.0		
G TS-t	MT20	5.0	6.0		
H TMWW-t	MT20	5.0	6.0		
I TTWW+m	MT20	5.0	8.0	3.50	1.50
J TMWW-t	MT20	5.0	6.0	2.50	2.25
K BMV+p	MT20	3.0	6.0		
L BMV1-t	MT20	5.0	6.0		
M, N, R, S					
M BMWW-t	MT20	5.0	6.0		
O BS-t	MT20	5.0	6.0		
P BMWW-t	MT20	5.0	6.0		
Q BS-t	MT20	5.0	6.0		
T BMV1-t	MT20	5.0	6.0		

NOTES: (1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
T	2221	0	2221	0	0
L	2097	0	2097	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT L. MINIMUM BEARING LENGTH AT JOINT L = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	DEAD	SOIL
T	COMBINED	SNOW	LIVE	PERM. LIVE
T	1574	1013 / 0	0 / 0	0 / 0
L	1489	945 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.93 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-R, F-P, H-N, I-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	FORCE (LBS)	MEMB.	FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 44	C-S	0 / 212
B-C	0 / 35	D-R	0 / 93
C-D	-2093 / 0	E-P	0 / 1317
D-E	-2268 / 0	F-G	-918 / 0
E-F	-2491 / 0	G-H	0 / 367
F-G	-2491 / 0	H-I	-514 / 0
G-H	-2491 / 0	I-J	0 / 421
H-I	-2236 / 0	J-K	-961 / 0
I-J	-2001 / 0	K-L	0 / 1372
J-K	0 / 43	L-M	-94 / 51
K-L	-179 / 0	M-N	0 / 383
L-M	-26 / 0	N-O	-2370 / 0
		O-P	-2296 / 0
		P-Q	
		Q-R	
		R-S	
		S-R	
		R-Q	
		Q-P	
		P-O	
		O-N	
		N-M	
		M-L	

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
DL	=	6.0	PSF	
BOT CH.	LL	=	0.0	PSF
DL	=	7.4	PSF	
TOTAL LOAD	=	36.7	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.35")
CALCULATED VERT. DEFL.(LL) = L/999 (0.09")
ALLOWABLE DEFL.(TL) = L/360 (1.35")
CALCULATED VERT. DEFL.(TL) = L/999 (0.17")

CSI: TC=0.31/1.00 (E-F:1), BC=0.31/1.00 (P-R:1), WB=0.51/1.00 (C-T:1), SSI=0.21/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(FLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.87 (J) (INPUT = 0.90)

JSI METAL = 0.53 (C) (INPUT = 1.00)

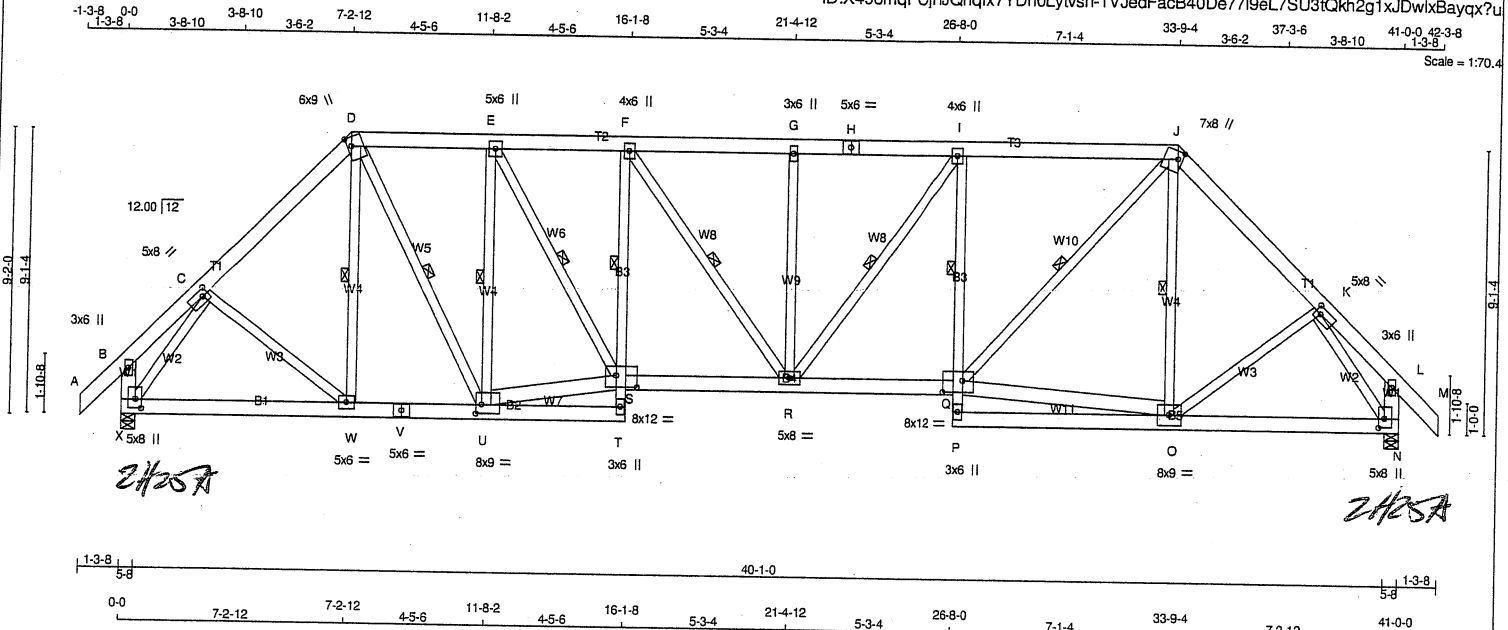


Structural component only
DWG# T-2126631

JOB NAME 420547	TRUSS NAME T6S	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

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LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - D	2x6	DRY	No.2	SPF
D - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
J - M	2x6	DRY	No.2	SPF
N - B	2x6	DRY	No.2	SPF
X - L	2x6	DRY	No.2	SPF
X - V	2x6	DRY	2100F 1.8E	SPF
V - T	2x6	DRY	2100F 1.8E	SPF
T - F	2x4	DRY	No.2	SPF
S - Q	2x6	DRY	2100F 1.8E	SPF
P - I	2x4	DRY	No.2	SPF
P - N	2x6	DRY	2100F 1.8E	SPF
ALL WEBS	2x4	DRY	No.2	SPF
EXCEPT				
U - S	2x6	DRY	2100F 1.8E	SPF
Q - O	2x6	DRY	2100F 1.8E	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	6.0		
C	TMVW-t	MT20	5.0	8.0	2.50	2.25
D	TTVW+m	MT20	6.0	9.0	3.50	1.50
E	TMVW+t	MT20	5.0	6.0		
F	TMVW+p	MT20	4.0	6.0		
G	TMV+w	MT20	3.0	6.0		
H	TS-t	MT20	5.0	6.0		
I	TMVW+p	MT20	4.0	6.0		
J	TTVW+m	MT20	7.0	8.0	3.00	1.75
K	TMVW-t	MT20	5.0	8.0	2.50	2.25
L	TMV+p	MT20	3.0	6.0		
N	BMVW1+p	MT20	5.0	8.0	3.50	2.25
O	BMVW1-t	MT20	8.0	9.0		
P	BMV+p	MT20	3.0	6.0		
Q	BMVW1-t	MT20	8.0	12.0	4.50	7.75
R	BMVW1-t	MT20	5.0	8.0		
S	BMVW1-t	MT20	8.0	12.0	4.50	8.00
T	BMV+p	MT20	3.0	6.0		
U	BMVW1-t	MT20	8.0	9.0	3.50	2.25
V	BS-t	MT20	5.0	6.0		
W	BMVW-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	RECORD
		GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
X	3120	0	3410	-428	-1081
N	3120	0	3410	0	-1081

PROVIDE ANCHORAGE AT BEARING JOINT X FOR 1081 LBS FACTORED UPLIFT

PROVIDE ANCHORAGE AT BEARING JOINT N FOR 1081 LBS FACTORED UPLIFT

PROVIDE FOR 428 LBS FACTORED HORIZONTAL REACTION AT JOINT X

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN COMPONENT REACTIONS			
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND
X	2318	1514 / 0	431 / 0	0 / 0	299 / -1045
N	2318	1514 / 0	431 / 0	0 / 0	141 / -1045

HORIZONTAL REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
X	---	0 / 0	0 / 0	0 / 0	306 / -306	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, N

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 3.76 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 5.37 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-S, I-Q, D-W, D-U, E-U, E-S, F-R, I-R, J-Q, J-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (18)

CHORDS		FACTORED		WEBS	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX	MEMB.	FORCE (LBS)
FR-TO		FROM	TO	FR-TO	
A-B	0 / 62	-105.2	-105.2	C-W	-41 / 574
B-C	-38 / 198	-105.2	-105.2	W-D	-127 / 241
C-D	-3224 / 1216	-105.2	-105.2	D-U	-706 / 2046
D-E	-3110 / 1279	-105.2	-105.2	E-U	-2356 / 790
E-F	-4099 / 1575	-105.2	-105.2	U-S	-891 / 3172
F-G	-4277 / 1601	-105.2	-105.2	S-E	-596 / 2090
G-H	-4277 / 1601	-105.2	-105.2	F-R	-145 / 437
H-I	-4277 / 1601	-105.2	-105.2	R-G	-622 / 298
I-J	-3947 / 1505	-105.2	-105.2	R-I	-226 / 728
J-K	-3233 / 1217	-105.2	-105.2	Q-O	-460 / 2243
K-L	-36 / 196	-105.2	-105.2	Q-J	-881 / 2645
L-M	0 / 62	-105.2	-105.2	O-J	-328 / 202
X-B	-255 / 227	0.0	0.0	O-K	-108 / 575
N-L	-254 / 226	0.0	0.0	X-C	-3616 / 1049
X-W	-779 / 1969	-39.5	-39.5	K-N	-3630 / 1048
W-V	-595 / 2251	-39.5	-39.5		
V-U	-595 / 2251	-39.5	-39.5		
U-T	-13 / 42	-39.5	-39.5		
T-S	0 / 94	0.0	0.0		
S-F	-953 / 419	0.0	0.0		
S-R	-1170 / 4105	-39.5	-39.5		
R-Q	-1063 / 3952	-39.5	-39.5		
P-Q	0 / 130	0.0	0.0		
Q-I	-1449 / 616	0.0	0.0		
P-O	-10 / 37	-39.5	-39.5		
O-N	-544 / 1977	-39.5	-39.5		

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	30.1	PSF
DL	=	6.0	PSF	
BOT CH.	LL	=	10.5	PSF
DL	=	7.4	PSF	
TOTAL LOAD	=	54.0	PSF	

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 4 OF BCBC 2018, OBC 2012, ABC 2019
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- SLOPE REDUCTION FACTOR NOT USED

(80% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 30.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (1.37")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.16")
ALLOWABLE DEFL.(TL) = $L/180$ (2.73")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.22")

CSI: TC=0.48/1.00 (I-J-3), BC=0.25/1.00 (I-Q-2), WB=0.83/1.00 (E-U-3), SSI=0.30/1.00 (I-J-2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	(PSI)	(PLI)	SHEAR	SECTION
MT20	618	354	1667	788	1987

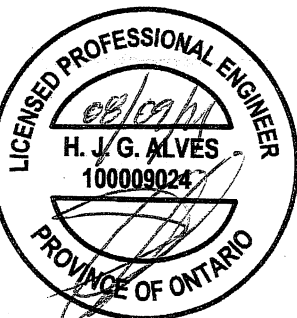
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (N) (INPUT = 0.90)

JSI METAL= 0.71 (N) (INPUT = 1.00)

CONTINUED ON PAGE 2



Structural component only
DWG# T-2126684

JOB NAME 420547	TRUSS NAME T6S	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:X496mqPCihJQnqfx7YDh0Lytvsh-Vht0qbbEyO84GHiyiLsM ib2981HPUBTRaVUk0yqx?t

PLATES (table is in inches)

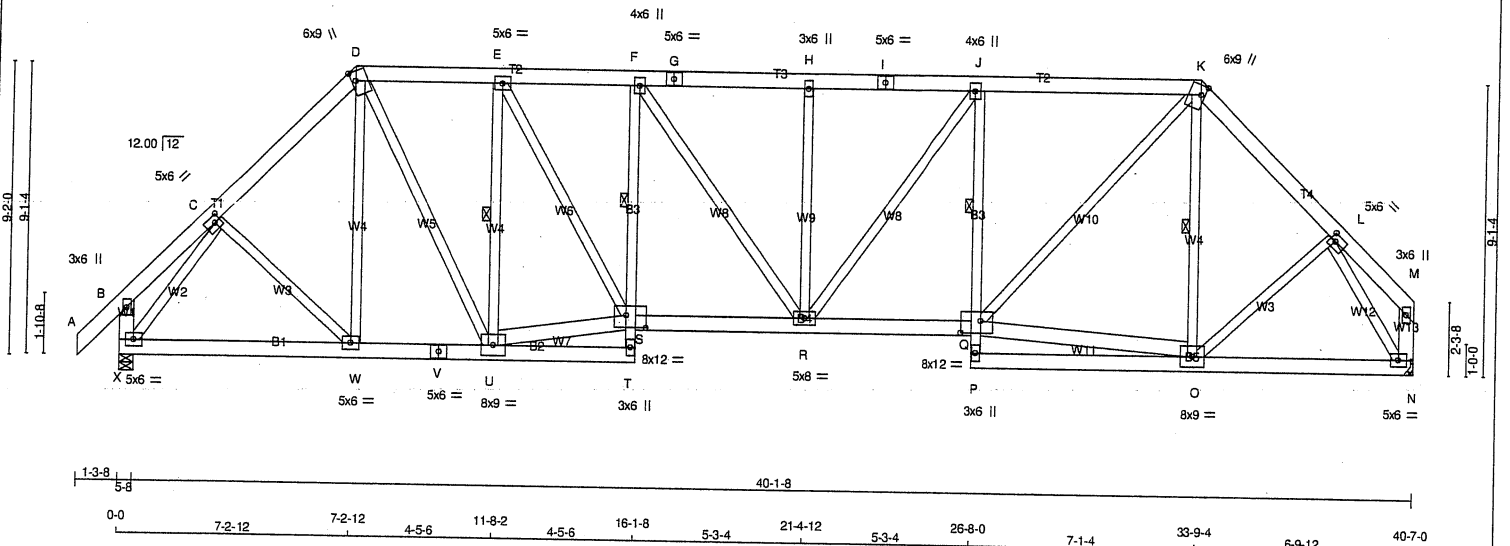
JT	TYPE	PLATES	W	LEN	Y	X
X	BMVW1+p	MT20	5.0	8.0	3.50	2.25

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.2} PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.



Structural component only
DWG# T-2126684

JOB NAME 420540	TRUSS NAME T6AS	QUANTITY 7	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Aug 5 09:39:30 2021 Page 1	
ID: X496mqPCjhJQnqfx7YDh0Lytvsh-3_D_n_nAvyarSHDEUaDWzW2A33hsbMycst8BS4yqyFh				Scale = 1:68.8	



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x6	DRY	No.2
D - G	2x6	DRY	No.2
G - I	2x6	DRY	No.2
I - K	2x6	DRY	No.2
K - M	2x6	DRY	No.2
X - B	2x6	DRY	No.2
N - M	2x6	DRY	No.2
X - V	2x6	DRY	2100F 1.8E
V - T	2x6	DRY	2100F 1.8E
T - F	2x4	DRY	No.2
S - Q	2x6	DRY	2100F 1.8E
P - J	2x4	DRY	No.2
P - N	2x6	DRY	2100F 1.8E
ALL WEBS	2x4	DRY	No.2
EXCEPT			
U - S	2x6	DRY	2100F 1.8E
Q - O	2x6	DRY	2100F 1.8E

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	6.0		
C TMVW+t	MT20	5.0	6.0	2.50	2.25
D TTWW+m	MT20	6.0	9.0	3.50	1.50
E TMVW+t	MT20	5.0	6.0		
F TMVW+p	MT20	4.0	6.0		
G TS-t	MT20	5.0	6.0		
H TMV+w	MT20	3.0	6.0		
I TS-t	MT20	5.0	6.0		
J TMVW+p	MT20	4.0	6.0		
K TTWW+m	MT20	6.0	9.0	3.50	1.50
L TMVW+t	MT20	5.0	6.0	2.50	2.00
M TMV+p	MT20	3.0	6.0		
N BMVW1-t	MT20	5.0	6.0		
O BMVWV-t	MT20	8.0	9.0		
P BMV+p	MT20	3.0	6.0		
Q BVMWV-t	MT20	8.0	12.0	4.50	7.50
R BVMWV-t	MT20	5.0	8.0		
S BVMWV-t	MT20	8.0	12.0	4.50	7.50
T BMV+p	MT20	3.0	6.0		
U BVMWV-t	MT20	8.0	9.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT REQRD	
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX
X	2221	0	2221	0	5-8	5-8
N	2097	0	2097	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT N. MINIMUM BEARING LENGTH AT JOINT N = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
X	1574	1013 / 0	0 / 0	0 / 0	0 / 0	561 / 0	0 / 0
N	1489	945 / 0	0 / 0	0 / 0	0 / 0	544 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.72 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-S, J-Q, E-U, K-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED LOAD LC1 (PLF)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO		FROM TO	FR-TO	
A-B	0 / 44	-84.9	0.07 (1)	10.00
B-C	0 / 26	-84.9	0.07 (1)	10.00
C-D	-2091 / 0	-84.9	0.13 (1)	5.49
D-E	-2026 / 0	-84.9	0.13 (1)	5.57
E-F	-2679 / 0	-84.9	0.14 (1)	4.98
F-G	-2795 / 0	-84.9	0.16 (1)	4.88
G-H	-2795 / 0	-84.9	0.16 (1)	4.88
H-I	-2795 / 0	-84.9	0.30 (1)	4.72
I-J	-2795 / 0	-84.9	0.30 (1)	4.72
J-K	-2543 / 0	-84.9	0.30 (1)	4.91
K-L	-2001 / 0	-84.9	0.13 (1)	5.58
L-M	0 / 30	-84.9	0.07 (1)	10.00
X-B	-206 / 0	0.0	0.01 (1)	7.81
N-M	-58 / 0	0.0	0.00 (1)	7.81
X-W	0 / 1330	-18.5	-18.5	0.10 (1)
W-V	0 / 1459	-18.5	-18.5	0.10 (1)
V-U	0 / 1459	-18.5	-18.5	0.10 (1)
U-T	0 / 28	-18.5	-18.5	0.02 (4)
T-S	0 / 39	0.0	0.0	0.04 (1)
S-F	-575 / 0	0.0	0.0	0.07 (1)
S-R	0 / 2683	-18.5	-18.5	0.15 (1)
R-Q	0 / 2546	-18.5	-18.5	0.14 (1)
P-Q	0 / 54	0.0	0.0	0.04 (1)
Q-J	-949 / 0	0.0	0.0	0.12 (1)
P-O	0 / 24	-18.5	-18.5	0.07 (4)
O-N	0 / 1152	-18.5	-18.5	0.10 (4)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
DL = 6.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.4	PSF	
TOTAL LOAD = 36.7	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.35")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL) = L/360 (1.35")
CALCULATED VERT. DEFL.(TL) = L/999 (0.20")

CSI: TC=0.30/1.00 (J-K:1), BC=0.15/1.00 (R-S:1), WB=0.62/1.00 (C-X:1), SSI=0.21/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

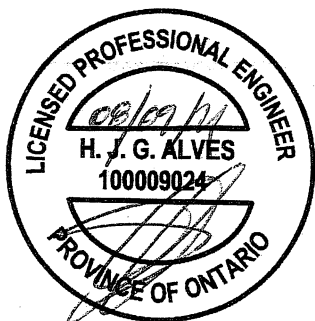
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 788
	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (L) (INPUT = 0.90)
JSI METAL= 0.58 (C) (INPUT = 1.00)

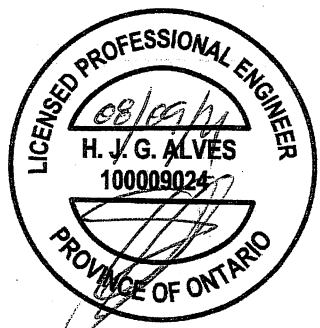


Structural component only
DWG# T-2126657

JOB NAME 420540	TRUSS NAME T6AS	QUANTITY 7	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
V	BS-t	MT20	5.0	6.0		
W	BMWW-t	MT20	5.0	6.0		
X	BMVW1-t	MT20	5.0	6.0		

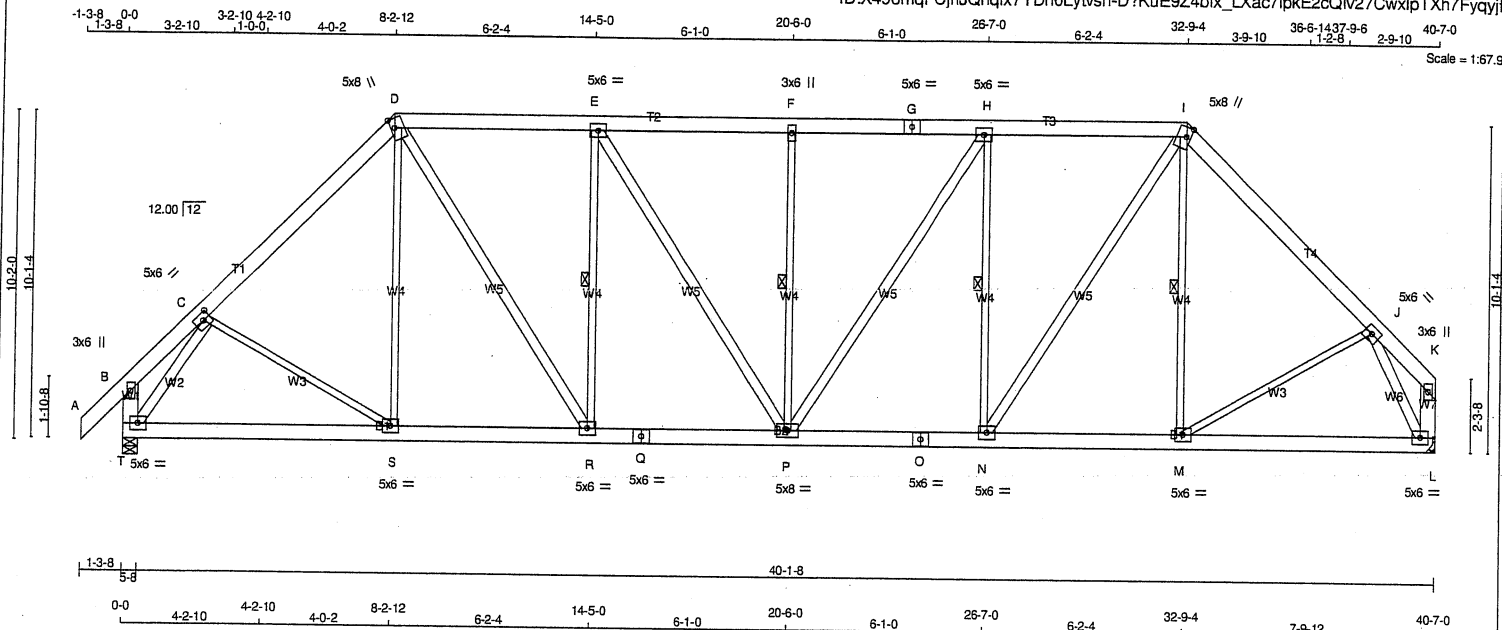


Structural component only
DWG# T-2126657 212

JOB NAME 420538	TRUSS NAME T7A	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

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ID:X496mqPCjhJQnqfx7YDh0Lytvsh-D?KuE9Z4b5x_LXac7lpkE2cQlv27CwXlpTXh7Fyqyif



TOTAL WEIGHT = 2 X 274 = 547 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF	
D - G	2x6	DRY	No.2	SPF	
G - I	2x6	DRY	No.2	SPF	
I - K	2x6	DRY	No.2	SPF	
T - B	2x6	DRY	No.2	SPF	
L - K	2x6	DRY	No.2	SPF	
T - Q	2x6	DRY	No.2	SPF	
Q - O	2x6	DRY	No.2	SPF	
O - L	2x6	DRY	No.2	SPF	

ALL WEBS EXCEPT	SIZE	LUMBER	DESCR.
D - R	2x4	DRY	No.2
E - P	2x4	DRY	No.2
P - H	2x4	DRY	No.2
N - I	2x4	DRY	No.2
T - C	2x4	DRY	No.2
J - L	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	6.0		
C TMWW-t	MT20	5.0	6.0	2.50	2.75
D TTWW+m	MT20	5.0	8.0	3.50	1.50
E, H, J					
F TMWW-t	MT20	5.0	6.0		
F TMWW+w	MT20	3.0	6.0		
G TS-t	MT20	5.0	6.0		
I TTWW+m	MT20	5.0	8.0	3.50	1.50
K TMV+p	MT20	3.0	6.0		
L BMVW1-t	MT20	5.0	6.0		
M, N, R, S					
M BMWW-t	MT20	5.0	6.0		
O BS-t	MT20	5.0	6.0		
P BMWW-t	MT20	5.0	8.0		
Q BS-t	MT20	5.0	6.0		
T BMVW1-t	MT20	5.0	6.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT		REQRD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	BRG	IN-SX	MECH
L	2097	0	2097	0	0	5-8			
T	2221	0	2221	0	0	5-8			

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT L. MINIMUM BEARING LENGTH AT JOINT L = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX	MIN	COMPONENT REACTIONS					
L	1489	945 / 0	0 / 0	LIVE	PERM	LIVE	WIND	DEAD	SOIL
T	1574	1013 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	544 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.21 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-R, F-P, H-N, I-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CS (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS (LC)		
FR-TO		FROM TO			FR-TO				
A-B	0 / 44	-84.9	-84.9	0.07 (1)	10.00	S-D	0 / 120	0.04 (4)	
B-C	0 / 69	-84.9	-84.9	0.16 (1)	10.00	D-R	0 / 1133	0.18 (1)	
C-D	-2079 / 0	-84.9	-84.9	0.22 (1)	5.41	R-E	-845 / 0	0.54 (1)	
D-E	-2059 / 0	-84.9	-84.9	0.25 (1)	5.38	E-P	0 / 315	0.05 (1)	
E-F	-2228 / 0	-84.9	-84.9	0.26 (1)	5.21	P-F	-474 / 0	0.30 (1)	
F-G	-2228 / 0	-84.9	-84.9	0.25 (1)	5.21	P-H	0 / 366	0.06 (1)	
G-H	-2228 / 0	-84.9	-84.9	0.25 (1)	5.21	N-H	-888 / 0	0.57 (1)	
H-I	-2032 / 0	-84.9	-84.9	0.25 (1)	5.41	N-I	0 / 1186	0.19 (1)	
I-J	-2003 / 0	-84.9	-84.9	0.22 (1)	5.49	M-I	-33 / 82	0.03 (4)	
J-K	0 / 91	-84.9	-84.9	0.17 (1)	10.00	C-S	0 / 177	0.04 (1)	
K-L	-124 / 0	0.0	0.0	0.01 (1)	7.81	T-C	-2409 / 0	0.48 (1)	
L-K	0 / 50	0.0	0.0	0.01 (1)	10.00	M-J	0 / 362	0.08 (1)	
						J-L	-2350 / 0	0.43 (1)	
T-S	0 / 1294	-18.5	-18.5	0.23 (1)	10.00				
S-R	0 / 1446	-18.5	-18.5	0.24 (1)	10.00				
R-Q	0 / 2059	-18.5	-18.5	0.28 (1)	10.00				
Q-P	0 / 2059	-18.5	-18.5	0.28 (1)	10.00				
P-O	0 / 2032	-18.5	-18.5	0.28 (1)	10.00				
O-N	0 / 2032	-18.5	-18.5	0.28 (1)	10.00				
N-M	0 / 1390	-18.5	-18.5	0.23 (1)	10.00				
M-L	0 / 1079	-18.5	-18.5	0.20 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.35")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (1.35")
CALCULATED VERT. DEFL.(TL) = L/999 (0.15")

CSI: TC=0.26/1.00 (E-F:1), BC=0.28/1.00 (P-R:1), WB=0.57/1.00 (H-N:1), SSI=0.19/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

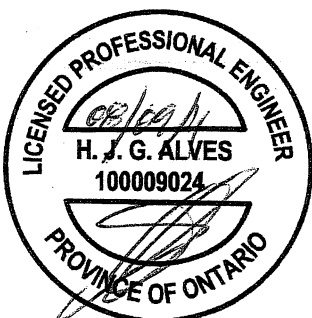
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

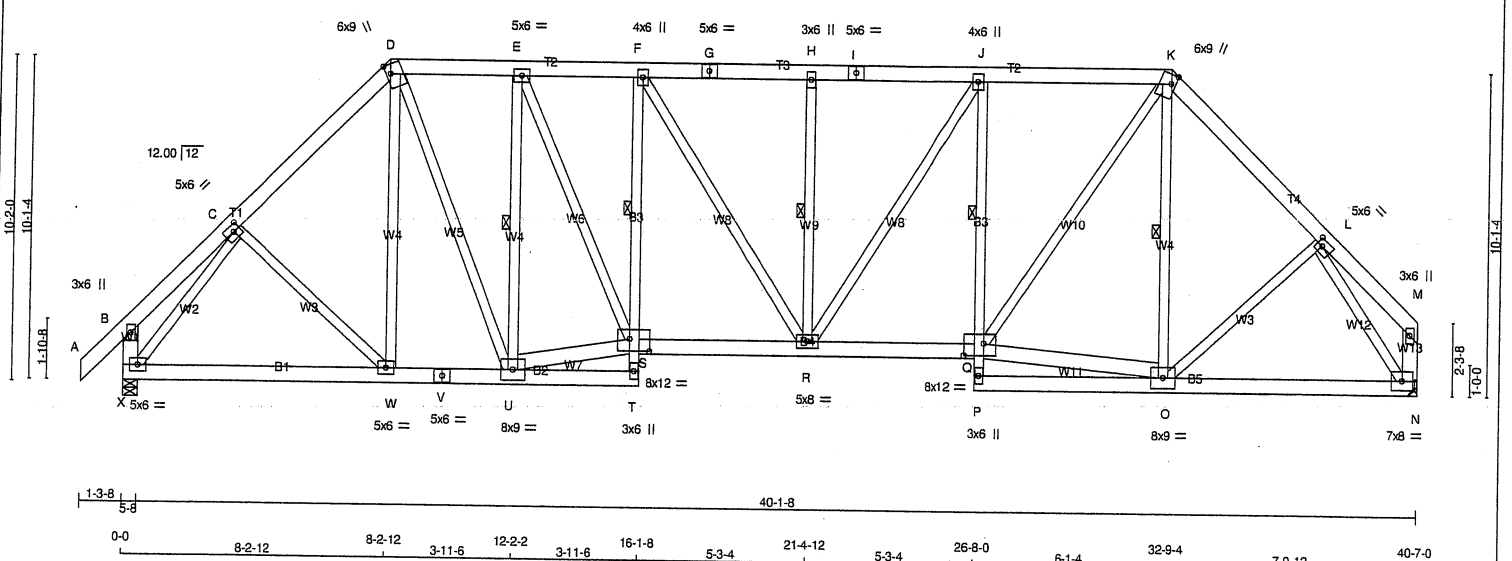
JSI GRIP = 0.85 (C) (INPUT = 0.90)
JSI METAL = 0.54 (C) (INPUT = 1.00)



Structural component only
DWG# T-2126632

JOB NAME 420540	TRUSS NAME T7AS	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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 Scale = 1:58.8



LUMBER				N. L. G. A. RULES	
CHORDS	SIZE	LUMBER	DESCR.	SPF	SPF
A - D	2x6	DRY	No.2	SPF	SPF
D - G	2x6	DRY	No.2	SPF	SPF
G - I	2x6	DRY	No.2	SPF	SPF
I - K	2x6	DRY	No.2	SPF	SPF
K - M	2x6	DRY	No.2	SPF	SPF
X - B	2x6	DRY	No.2	SPF	SPF
N - M	2x6	DRY	No.2	SPF	SPF
X - V	2x6	DRY	2100F 1.8E	SPF	SPF
V - T	2x6	DRY	2100F 1.8E	SPF	SPF
T - F	2x4	DRY	No.2	SPF	SPF
S - Q	2x6	DRY	2100F 1.8E	SPF	SPF
P - J	2x4	DRY	No.2	SPF	SPF
P - N	2x6	DRY	2100F 1.8E	SPF	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF	SPF
U - S	2x6	DRY	2100F 1.8E	SPF	SPF
Q - O	2x6	DRY	2100F 1.8E	SPF	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	6.0		
C TMWW+t	MT20	5.0	6.0	2.50	2.50
D TTWW+m	MT20	6.0	9.0	3.50	1.50
E TMWW-t	MT20	5.0	6.0		
F TMVW+p	MT20	4.0	6.0		
G TS-t	MT20	5.0	6.0		
H TMW+w	MT20	3.0	6.0		
I TS-t	MT20	5.0	6.0		
J TMVW+p	MT20	4.0	6.0		
K TTWW+m	MT20	6.0	9.0	3.50	1.50
L TMWW-t	MT20	5.0	6.0	2.50	2.25
M TMV+p	MT20	3.0	6.0		
N BMVW1-t	MT20	7.0	8.0		
O BMWWW-t	MT20	8.0	9.0		
P BMV+p	MT20	3.0	6.0		
Q BMWWW-t	MT20	8.0	12.0	4.50	7.50
R BMWWW-t	MT20	5.0	8.0		
S BMWWW-t	MT20	8.0	12.0	4.50	7.50
T BMV+p	MT20	3.0	6.0		
U BMWWW-t	MT20	8.0	9.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQ'D
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
X	2221	0	2221	0	5-8
N	2097	0	2097	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT N. MINIMUM BEARING LENGTH AT JOINT N = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN.	COMPONENT REACTIONS	WIND	DEAD	SOIL
X	COMBINED	SNOW	LIVE	PERM.LIVE		
X	1574	1013 / 0	0 / 0	0 / 0	561 / 0	0 / 0
N	1489	945 / 0	0 / 0	0 / 0	544 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.04 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-S, J-Q, E-U, H-R, K-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 44	C-W	0 / 82
B-C	0 / 32	D-U	0 / 118
C-D	-2073 / 0	E-S	0 / 1104
D-E	-1858 / 0	F-R	0 / 197
E-F	-2368 / 0	G-H	0 / 1908
F-G	-2475 / 0	H-I	0 / 1235
G-H	-2475 / 0	I-J	0 / 197
H-I	-2475 / 0	J-K	0 / 1908
I-J	-2475 / 0	K-L	0 / 1491
J-K	-2239 / 0	L-M	0 / 1391
K-L	-1997 / 0	M-N	0 / 1491
L-M	0 / 36	N-M	-226 / 0
X-B	-219 / 0	O-L	0 / 222
N-M	-72 / 0	X-C	-2380 / 0
		L-N	-2319 / 0
X-W	0 / 1383		
W-V	0 / 1445		
V-U	0 / 1445		
U-T	0 / 24		
T-S	0 / 35		
S-F	-562 / 0		
S-R	0 / 2371		
R-Q	0 / 2242		
P-Q	0 / 40		
Q-J	-901 / 0		
P-O	0 / 19		
O-N	0 / 1228		

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 23.3 PSF
 DL = 6.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBC 2018, OBC 2012, ABC 2019
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 086-09, CSA 086-14
 - TPIC 2011, TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.35")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.10")
 ALLOWABLE DEFL.(TL) = L/360 (1.35")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.18")

CSI: TC=0.22/1.00 (H-J-1), BC=0.16/1.00 (J-Q-1), WB=0.84/1.00 (C-X-1), SSI=0.18/1.00 (J-K-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION
 (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
 JSI METAL= 0.59 (C) (INPUT = 1.00)



Structural component only
 DWG# T-2126658 1/12

CONTINUED ON PAGE 2

JOB NAME 420540	TRUSS NAME T7AS	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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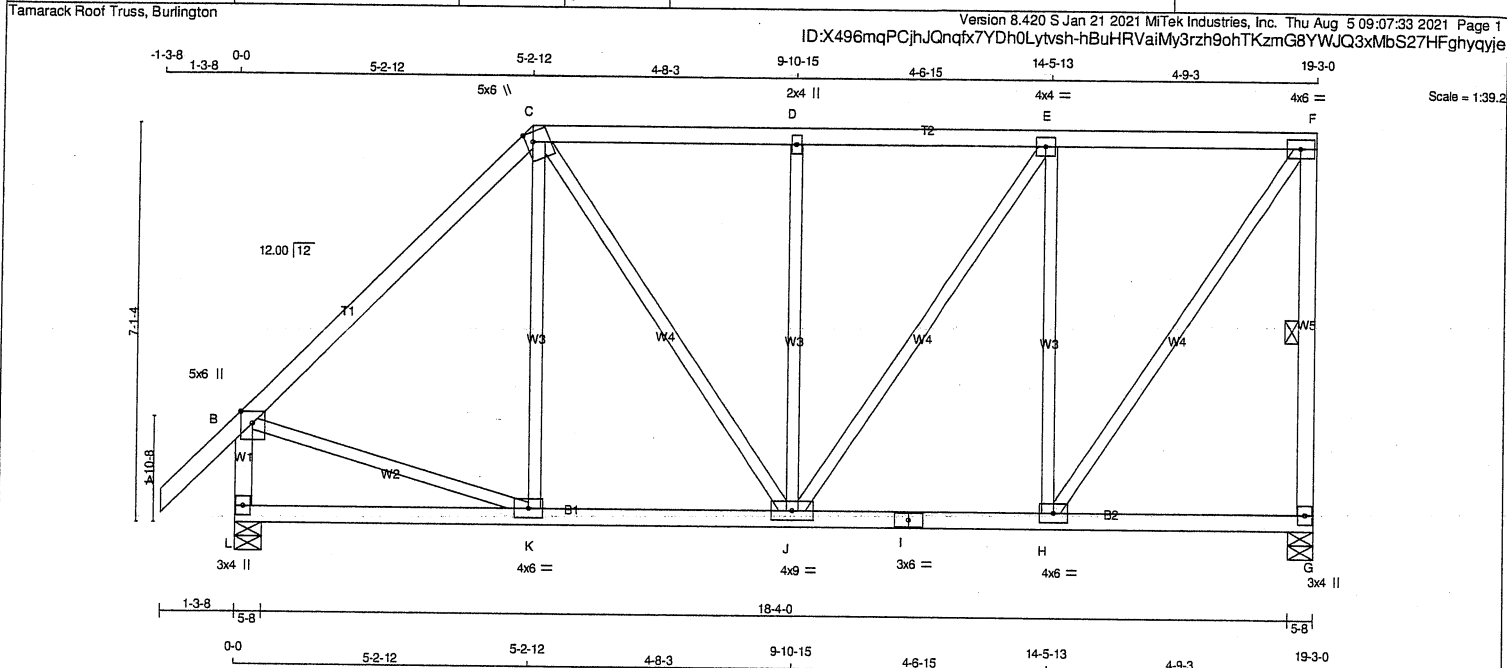
PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
V	BS-t	MT20	5.0	6.0		
W	BMWW-t	MT20	5.0	6.0		
X	BMVW1-t	MT20	5.0	6.0		



Structural component only
 DWG# T-2126658 *TH*

JOB NAME 420538	TRUSS NAME T8	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. TOTAL WEIGHT = 97 lb

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - F	2x4	DRY No.2	SPF
G - F	2x4	DRY No.2	SPF
L - B	2x4	DRY No.2	SPF
L - I	2x4	DRY No.2	SPF
I - G	2x4	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	2.50
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMVW-t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMVW-t	MT20	4.0	9.0		
K	BMVW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG IN-SX	REQD BRG IN-SX
JT	VERT	HORIZ	DOWN	HORIZ
G	995	0	995	0
L	1113	0	1113	0

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
G	706	448 / 0	0 / 0	0 / 0	258 / 0	0 / 0
L	788	513 / 0	0 / 0	0 / 0	275 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.11 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 42	-84.9 -84.9	0.12 (1)	10.00	K-C	-53 / 60	0.05 (1)
B-C	-805 / 0	-84.9 -84.9	0.45 (1)	6.11	B-K	0 / 590	0.13 (1)
C-D	-729 / 0	-84.9 -84.9	0.25 (1)	6.25	J-D	-423 / 0	0.37 (1)
D-E	-730 / 0	-84.9 -84.9	0.26 (1)	6.25	H-E	-695 / 0	0.61 (1)
E-F	-556 / 0	-84.9 -84.9	0.26 (1)	6.25	C-J	0 / 286	0.06 (1)
G-F	-959 / 0	0.0 0.0	0.22 (1)	6.25	H-F	0 / 971	0.22 (1)
L-B	-1073 / 0	0.0 0.0	0.12 (1)	7.61	J-E	0 / 311	0.07 (1)
L-K	0 / 0	-18.5 -18.5	0.12 (4)	10.00			
K-J	0 / 567	-18.5 -18.5	0.17 (4)	10.00			
J-I	0 / 556	-18.5 -18.5	0.15 (4)	10.00			
I-H	0 / 556	-18.5 -18.5	0.15 (4)	10.00			
H-G	0 / 0	-18.5 -18.5	0.10 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.64")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.64")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.45/1.00 (B-C:1), BC=0.17/1.00 (J-K:4), WB=0.61/1.00 (E-H:1), SSI=0.19/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE	GRIP (DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.86 (H) (INPUT = 0.90)
JSI METAL = 0.31 (B) (INPUT = 1.00)



Structural component only
DWG# T-2126633

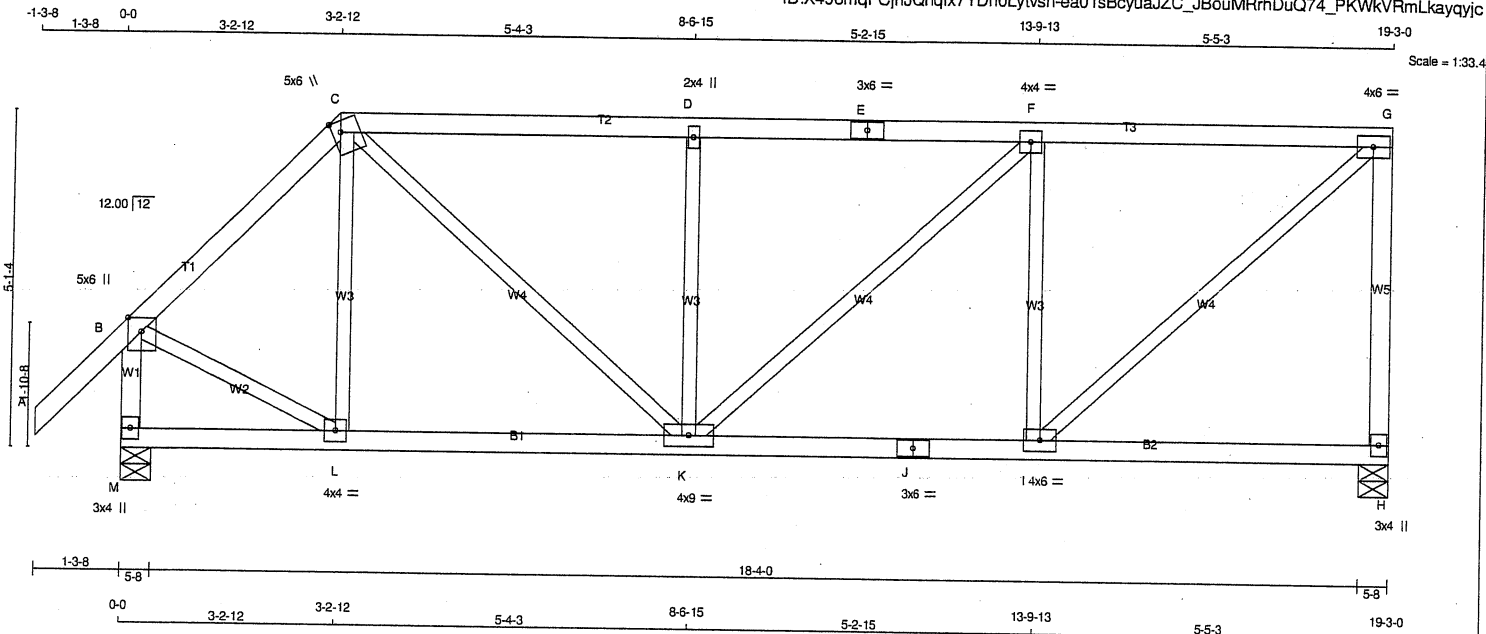
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Structural component only
DWG# T-2126634

JOB NAME 420538	TRUSS NAME T10	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - E	2x4	DRY No.2	SPF
E - G	2x4	DRY No.2	SPF
H - G	2x4	DRY No.2	SPF
M - B	2x4	DRY No.2	SPF
M - J	2x4	DRY No.2	SPF
J - H	2x4	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
 EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	2.50
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMVW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMVW-t	MT20	4.0	4.0		
G	TMVW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMVW-t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMVW-t	MT20	4.0	9.0		
L	BMVW-t	MT20	4.0	4.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
 1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQD
JT	VERT	HORZ	DOWN	HORZ
H	995	0	995	0
M	1113	0	1113	0

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	COMBINED							
H	706	448 / 0	0 / 0	0 / 0	0 / 0	0 / 0	258 / 0	0 / 0
M	788	513 / 0	0 / 0	0 / 0	0 / 0	0 / 0	275 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.68 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)
FR-TO					FR-TO		
A-B	0 / 42	-84.9	-84.9 0.12 (1)	10.00	L-C	-169 / 4	0.06 (1)
B-C	-812 / 0	-84.9	-84.9 0.16 (1)	6.25	B-L	0 / 629	0.14 (1)
C-D	-1036 / 0	-84.9	-84.9 0.34 (1)	5.71	C-K	0 / 628	0.14 (1)
D-E	-1036 / 0	-84.9	-84.9 0.36 (1)	5.68	K-D	-484 / 0	0.19 (1)
E-F	-1036 / 0	-84.9	-84.9 0.36 (1)	5.68	F-G	0 / 232	0.05 (1)
F-G	-865 / 0	-84.9	-84.9 0.35 (1)	6.09	I-F	-655 / 0	0.25 (1)
H-G	-954 / 0	0.0	0.0 0.42 (1)	7.81	I-G	0 / 1158	0.26 (1)
M-B	-1094 / 0	0.0	0.0 0.12 (1)	7.56			
M-L	0 / 0	-18.5	-18.5 0.09 (4)	10.00			
L-K	0 / 568	-18.5	-18.5 0.14 (1)	10.00			
K-J	0 / 865	-18.5	-18.5 0.21 (1)	10.00			
J-I	0 / 865	-18.5	-18.5 0.21 (1)	10.00			
I-H	0 / 0	-18.5	-18.5 0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 23.3 PSF
 DL = 6.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF CBC 2018, ABC 2019
 - PART 9 OF CBC 2012 (2019 AMENDMENT)
 - CSA 086-14
 - TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.64")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
 ALLOWABLE DEFL.(TL)= L/360 (0.64")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.06")

CSI: TC=0.42/1.00 (G-H:1), BC=0.21/1.00 (I-K:1), WB=0.26/1.00 (G-I:1), SS=0.21/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747 788 1987 1873

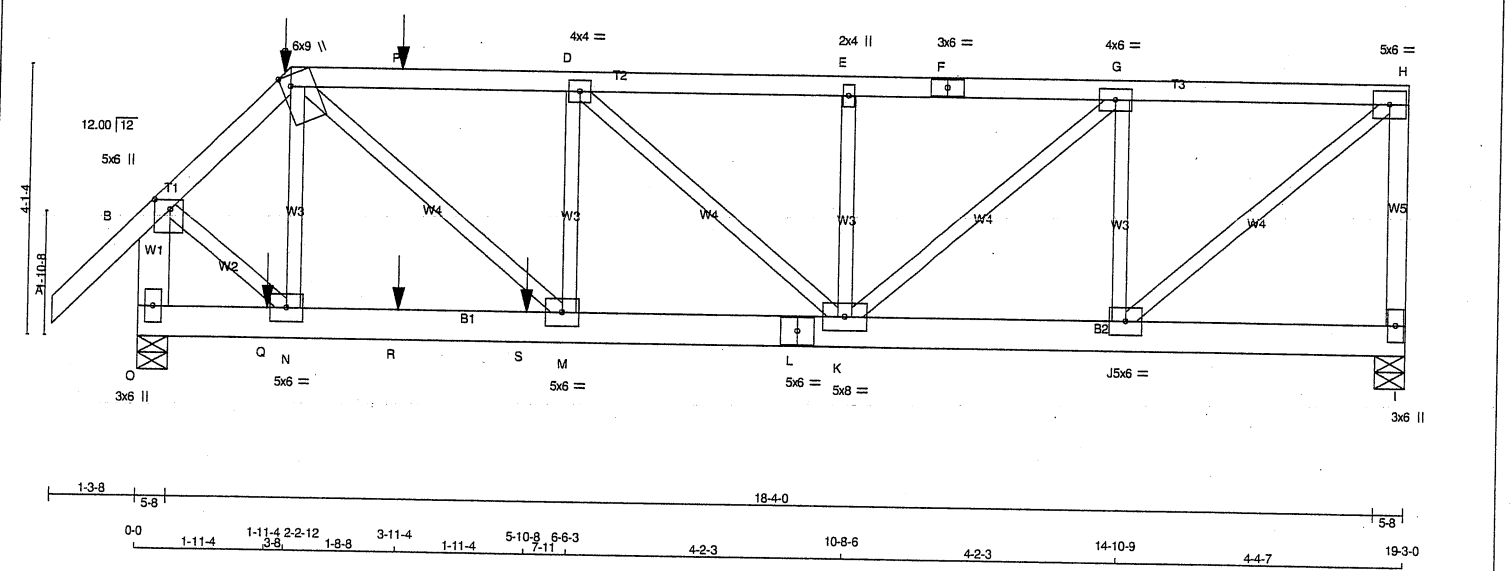
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.72 (I) (INPUT = 0.90)
 JSI METAL = 0.29 (B) (INPUT = 1.00)



Structural component only
 DWG# T-2126635



LUMBER

N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
I - H	2x4	DRY	No.2
O - B	2x6	DRY	No.2
O - L	2x6	DRY	No.2
L - I	2x6	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	1.75	2.75
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMVW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMVW-t	MT20	4.0	6.0		
H	TMVW-t	MT20	5.0	6.0		
I	BMV1+p	MT20	3.0	6.0		
J, M, N						
J	BMVW-t	MT20	5.0	6.0		
K	BMVWW-t	MT20	5.0	8.0		
L	BS-t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER							
<u>BEARINGS</u>							
	FACTORED		MAXIMUM FACTORED		INPUT	REQRD	
	GROSS REACTION		GROSS REACTION		BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
I	1376	0	1376	0	0	5-8	5-8
O	2272	0	2272	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
I	975	627 / 0	0 / 0	0 / 0	0 / 0	348 / 0	0 / 0
O	1606	1058 / 0	0 / 0	0 / 0	0 / 0	548 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	CS (LC)
FR-TO				FR-TO			
A-B	0 / 42	-84.9	-84.9 0.13 (1)	10.00	N-C	-442 / 0	0.11 (1)
B-C	-1850 / 0	-84.9	-84.9 0.13 (1)	4.80	B-N	0 / 1526	0.38 (1)
C-P	-2547 / 0	-84.9	-84.9 0.55 (1)	3.66	M-D	-123 / 23	0.03 (1)
P-D	-2547 / 0	-84.9	-84.9 0.55 (1)	3.66	K-E	-304 / 0	0.08 (1)
D-E	-2142 / 0	-84.9	-84.9 0.40 (1)	4.19	J-G	-1086 / 0	0.28 (1)
E-F	-2142 / 0	-84.9	-84.9 0.33 (1)	4.28	C-M	0 / 1665	0.41 (1)
F-G	-2142 / 0	-84.9	-84.9 0.33 (1)	4.28	D-K	-543 / 0	0.29 (1)
G-H	-1395 / 0	-84.9	-84.9 0.28 (1)	5.15	K-G	0 / 1001	0.25 (1)
H-I	-1339 / 0	0.0	0.0 0.33 (1)	6.97	J-H	0 / 1834	0.45 (1)
O-B	-2331 / 0	0.0	0.0 0.18 (1)	6.69			
O-Q	0 / 0	-18.5	-18.5 0.15 (1)	10.00			
Q-N	0 / 0	-18.5	-18.5 0.15 (1)	10.00			
N-R	0 / 1287	-18.5	-18.5 0.46 (1)	10.00			
R-S	0 / 1287	-18.5	-18.5 0.46 (1)	10.00			
S-M	0 / 1287	-18.5	-18.5 0.46 (1)	10.00			
M-L	0 / 2547	-18.5	-18.5 0.44 (1)	10.00			
L-K	0 / 2547	-18.5	-18.5 0.44 (1)	10.00			
K-J	0 / 1395	-18.5	-18.5 0.24 (1)	10.00			
J-I	0 / 0	-18.5	-18.5 0.04 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-2-12	-22	-22		FRONT	VERT	DEAD		C1
C	2-2-12	-118	-118		BACK	VERT	TOTAL		C1
C	2-2-12	-85	-85		FRONT	VERT	SNOW		C1
P	3-11-4	-113	-113		BACK	VERT	TOTAL		C1
O	1-11-4	-29	-29		BACK	VERT	TOTAL		C1
R	3-11-4	-29	-29		BACK	VERT	TOTAL		C1
S	5-10-8	-692	-692		BACK	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.64")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL) = L/360 (0.64")
CALCULATED VERT. DEFL.(TL) = L/999 (0.12")

CSI: TC=0.55/1.00 (C-D:1), BC=0.46/1.00 (M-N:1), WB=0.45/1.00 (H-J:1), SSI=0.58/1.00 (M-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

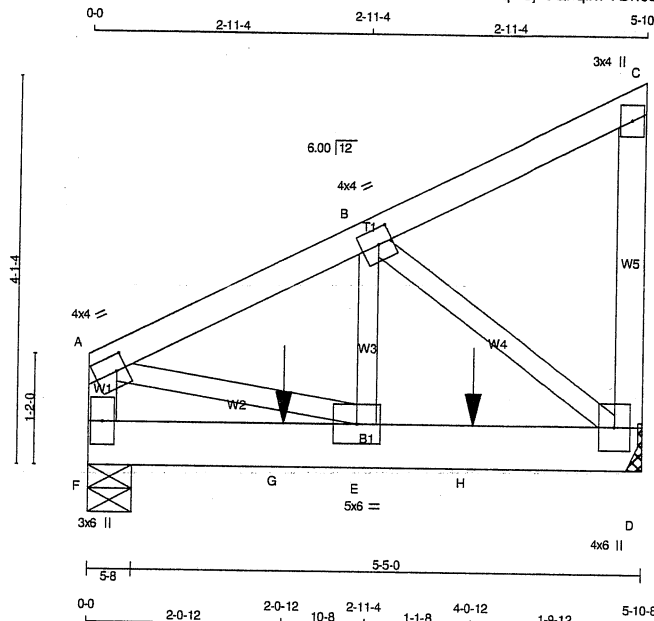
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (J) (INPUT = 0.90)
JSI METAL= 0.56 (B) (INPUT = 1.00)



JOB NAME 420538	TRUSS NAME T12	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
TAMARACK ROOF TRUSS, BURLINGTON					

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LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS	2x4	DRY	No.2
F - A	2x4	DRY	No.2
A - C	2x4	DRY	No.2
D - C	2x4	DRY	No.2
F - D	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
SPF			

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS			
F - A	1	12	TOP
A - C	1	12	TOP
C - D	1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS			
F - D	2	12	SIDE (0.0)
WEBS : (0.122"x3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0	2.00	1.25
B	TMVW-t	MT20	4.0	4.0	2.00	1.75
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMVW-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQD
JT	VERT	HORZ	DOWN	HORZ
F	942	0	942	0
D	998	0	998	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 2-0.

UNFACTORED REACTIONS

JT	1ST LOASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	667	432 / 0	0 / 0	0 / 0	0 / 0	235 / 0	0 / 0
D	707	458 / 0	0 / 0	0 / 0	0 / 0	248 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	LC1 MAX (LC)	
FR-TO				FR-TO			
F-A	-801 / 0	0.0	0.0 0.04 (1)	A-E	0 / 948	0.12 (1)	
A-B	-1014 / 0	-84.9	-84.9 0.06 (1)	E-B	0 / 861	0.11 (1)	
B-C	-11 / 0	-84.9	-84.9 0.05 (1)	B-D	-1153 / 0	0.14 (1)	
D-C	-101 / 0	0.0	0.0 0.01 (1)				
F-G	0 / 0	-18.5	-18.5 0.09 (1)				
G-E	0 / 0	-18.5	-18.5 0.09 (1)				
E-H	0 / 917	-18.5	-18.5 0.18 (1)				
H-D	0 / 917	-18.5	-18.5 0.18 (1)				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	2-0-12	-471	-471	---	BACK	VERT	TOTAL	---	C1
H	4-0-12	-471	-471	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 29 = 58 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 23.3	PSF
	DL	= 6.0	PSF
BOT CH.	LL	= 0.0	PSF
	DL	= 7.4	PSF
TOTAL LOAD		= 36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.08/1.00 (A-B:1), BC=0.18/1.00 (D-E:1), WB=0.14/1.00 (B-D:1), SS=0.19/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
		788	1987
			1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (A) (INPUT = 0.90)
JSI METAL= 0.21 (D) (INPUT = 1.00)



Structural component only
DWG# T-2126637

CONTINUED ON PAGE 2

JOB NAME 420538	TRUSS NAME T12	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
F	BMV1+p	MT20	3.0	6.0		

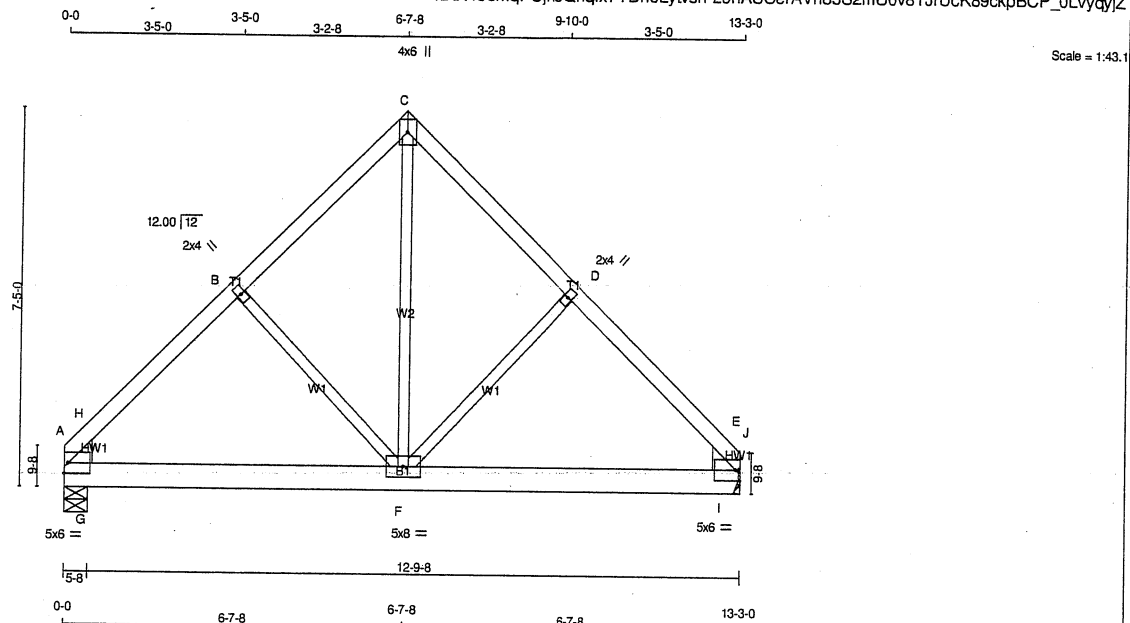
NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only
DWG# T-2126637 *mn*

JOB NAME 420538	TRUSS NAME T13	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Aug 5 09:07:38 2021 Page 1
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LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	SPF	SPF
A - C	2x4	DRY	No.2	SPF	SPF
C - E	2x4	DRY	No.2	SPF	SPF
A - E	2x6	DRY	No.2	SPF	SPF
ALL WEBS	2x3	DRY	No.2	SPF	SPF
DRY: SEASONED LUMBER.					

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y
A	TMBH1-I	MT20	5.0	6.0	Edge
B	TMW+w	MT20	2.0	4.0	
C	TTW+p	MT20	4.0	6.0	
D	TMW+w	MT20	2.0	4.0	
E	TMBH1-I	MT20	5.0	6.0	Edge
F	BMWWV-t	MT20	5.0	8.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	HORZ	GROSS REACTION	DOWN	HORZ	BRG	BRG	HEEL	WEDGE
A	685	0	685	0	0	5-8	5-8	2x6 L	2x6 R
E	685	0	685	0	0	MECHANICAL			

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	486	309 / 0	0 / 0	0 / 0	0 / 0	178 / 0	0 / 0
E	486	309 / 0	0 / 0	0 / 0	0 / 0	178 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.21 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		W E B S		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PL)	LC1 MAX	MAX. UNBRAC	MEMB.	FORCE (LBS)	MAX
FR-TO		FROM	TO	LENGTH	FR-TO		
A-H	-911 / 0	-84.9	-84.9	0.06 (1)	F-C	0 / 431	0.10 (1)
H-B	-661 / 0	-84.9	-84.9	0.09 (1)	F-D	-215 / 0	0.08 (1)
B-C	-485 / 0	-84.9	-84.9	0.10 (1)	B-F	-214 / 0	0.08 (1)
C-D	-485 / 0	-84.9	-84.9	0.10 (1)	G-H	0 / 179	0.00 (1)
D-J	-665 / 0	-84.9	-84.9	0.09 (1)	I-J	0 / 264	0.00 (1)
J-E	-987 / 0	-84.9	-84.9	0.07 (1)			
A-G	0 / 474	-18.5	-18.5	0.09 (1)			
G-F	0 / 474	-18.5	-18.5	0.11 (4)			
F-I	0 / 474	-18.5	-18.5	0.11 (4)			
I-E	0 / 474	-18.5	-18.5	0.09 (1)			

TOTAL WEIGHT = 2 X 60 = 120 lb [MIF]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCG 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.44")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.44")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.10/1.00 (C-D:1) , BC=0.11/1.00 (F-G:4) , WB=0.10/1.00 (C-F:1) , SS=0.12/1.00 (E-I:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (E) (INPUT = 0.90)

JSI METAL= 0.16 (E) (INPUT = 1.00)

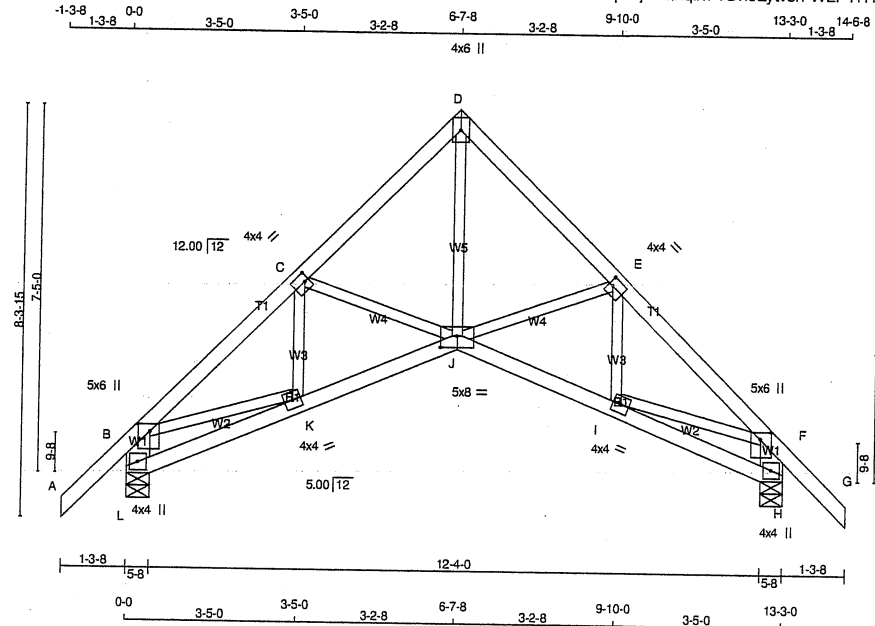


Structural component only
DWG# T-2126638

JOB NAME 420538	TRUSS NAME T13S	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1:44.4

<u>LUMBER</u>			
N. L. G. A. RULES			
CHORDS	SIZE		LUMBER
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
L - B	2x6	DRY	No.2
H - F	2x6	DRY	No.2
L - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)							
JT	TYPE	PLATES	W	LEN	Y	X	
B	TMVW+p	MT20	5.0	6.0	1.75	2.75	
C	TMVW-t	MT20	4.0	4.0	2.00	1.00	
D	TTW+p	MT20	4.0	6.0			
E	TMVW-t	MT20	4.0	4.0	2.00	1.00	
F	TMVW+p	MT20	5.0	6.0	1.75	2.75	
H	BMV1+p	MT20	4.0	4.0			
I	BMVW-t	MT20	4.0	4.0			
J	BBVWVW-p	MT20	5.0	8.0	2.75	4.00	
K	BMVW-t	MT20	4.0	4.0			
L	BMV1+p	MT20	4.0	4.0			

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
L	803	0	803	0	0	5-8	5-8	5-8	5-8
H	803	0	803	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS		1ST LCASE		MAX./MIN. COMPONENT REACTIONS		PERM. LIVE		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND
L	568	374 / 0	0 / 0	0 / 0	0 / 0	194 / 0	0 / 0	L	568	374 / 0	0 / 0	0 / 0	0 / 0
H	568	374 / 0	0 / 0	0 / 0	0 / 0	194 / 0	0 / 0	H	568	374 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.16 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED		FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (CSI (LC))	MAX. UNBRACED LENGTH	MEMB.	FORCE (LBS)	MAX. UNBRACED LENGTH	MEMB.	FORCE (LBS)
FR-TO					FR-TO			FR-TO	
A-B	0 / 42	-84.9	-84.9 0.12 (1)	10.00	J-D	0 / 803	0.18 (1)	J-D	0 / 803
B-C	-984 / 0	-84.9	-84.9 0.12 (1)	6.16	J-E	-218 / 0	0.05 (1)	J-E	-218 / 0
C-D	-738 / 0	-84.9	-84.9 0.11 (1)	6.25	I-E	-57 / 37	0.01 (4)	I-E	-57 / 37
D-E	-738 / 0	-84.9	-84.9 0.11 (1)	6.25	C-J	-218 / 0	0.05 (1)	C-J	-218 / 0
E-F	-984 / 0	-84.9	-84.9 0.12 (1)	6.16	K-C	-57 / 37	0.01 (4)	K-C	-57 / 37
F-G	0 / 42	-84.9	-84.9 0.12 (1)	10.00	B-K	0 / 730	0.16 (1)	B-K	0 / 730
L-B	-774 / 0	0.0	0.0 0.05 (1)	7.81	I-F	0 / 730	0.16 (1)	I-F	0 / 730
H-F	-774 / 0	0.0	0.0 0.05 (1)	7.81					
L-K	0 / 3	-18.5	-18.5 0.05 (4)	10.00					
K-J	0 / 766	-18.5	-18.5 0.15 (1)	10.00					
J-I	0 / 766	-18.5	-18.5 0.15 (1)	10.00					
I-H	0 / 3	-18.5	-18.5 0.05 (4)	10.00					

TOTAL WEIGHT = 2 X 63 = 125 lb [M][F]

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.44")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.44")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.12/1.00 (E-F:1), BC=0.15/1.00 (J-K:1), WB=0.18/1.00 (D-J:1), SS=0.10/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES		PLATE GRIP(DRY)		SHEAR		SECTION	
(PSI)	(PLI)	(PSI)	(PLI)	(PSI)	(PLI)	(PSI)	(PLI)
MAX	MIN	MAX	MIN	MAX	MIN	MAX	MIN
650	371	1747	788	1987	1873		

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.62 (F) (INPUT = 0.90)
JSI METAL= 0.33 (B) (INPUT = 1.00)

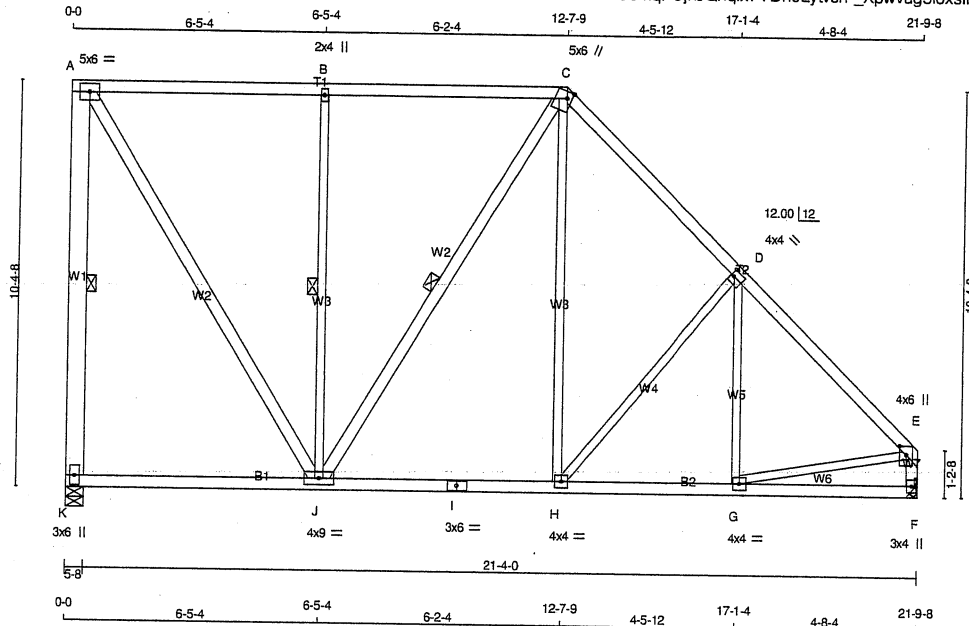


Structural component only
DWG# T-2126639

JOB NAME 420538	TRUSS NAME T14	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1:56.2

LUMBER				DESIGN CRITERIA			
N. L. G. A. RULES				SPECIFIED LOADS:			
CHORDS	SIZE	LUMBER	DESCR.	TOP CH.	LL	=	23.3 PSF
K - A	2x6	DRY	No.2	DL	=	6.0	PSF
A - C	2x4	DRY	No.2	BOT CH.	LL	=	0.0 PSF
C - E	2x4	DRY	No.2	DL	=	7.4	PSF
F - E	2x4	DRY	No.2	TOTAL LOAD	=	36.7	PSF
K - I	2x4	DRY	No.2	SPACING = 24.0 IN. C/C			
I - F	2x4	DRY	No.2	LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12			
ALL WEBS	2x3	DRY	No.2	THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015			
EXCEPT				THIS DESIGN COMPLIES WITH:			
A - J	2x4	DRY	No.2	- PART 9 OF BCBC 2018, ABC 2019			
J - C	2x4	DRY	No.2	- PART 9 OF OBC 2012 (2019 AMENDMENT)			
DRY: SEASONED LUMBER.				- CSA 086-14			
				- TPIC 2014			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0		
B	TMVW-w	MT20	2.0	4.0		
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMVW-t	MT20	4.0	4.0	2.00	1.00
E	TMVW+p	MT20	4.0	6.0	2.75	2.00
F	BMV1-p	MT20	3.0	4.0		
G	BMVW-t	MT20	4.0	4.0		
H	BMVW-t	MT20	4.0	4.0		
I	BS-t	MT20	3.0	6.0		
J	BMVWW-t	MT20	4.0	9.0		
K	BMV1-p	MT20	3.0	6.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQD
K	1126	0	1126	0
F	1126	0	1126	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 3'-8".

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
K	800	508 / 0	0 / 0	0 / 0	292 / 0	0 / 0
F	800	508 / 0	0 / 0	0 / 0	292 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.63 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF A-K, B-J, C-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS1 (LC)	
FR-TO		FROM TO		FR-TO			
K-A	-1079 / 0	0.0	0.0 0.37 (1)	A-J	0 / 1032	0.17 (1)	
A-B	-555 / 0	-84.9	-84.9 0.59 (1)	J-B	-664 / 0	0.50 (1)	
B-C	-555 / 0	-84.9	-84.9 0.59 (1)	J-C	-84 / 0	0.07 (1)	
C-D	-880 / 0	-84.9	-84.9 0.33 (1)	H-C	0 / 330	0.07 (1)	
D-E	-1087 / 0	-84.9	-84.9 0.34 (1)	H-D	-307 / 0	0.31 (1)	
F-E	-1089 / 0	0.0	0.0 0.11 (1)	G-D	-54 / 52	0.03 (1)	
				G-E	0 / 805	0.18 (1)	
K-J	0 / 0	-18.5	-18.5 0.19 (4)				
J-I	0 / 599	-18.5	-18.5 0.24 (4)				
I-H	0 / 599	-18.5	-18.5 0.24 (4)				
H-G	0 / 792	-18.5	-18.5 0.18 (1)				
G-F	0 / 0	-18.5	-18.5 0.09 (4)				

55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")

CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")

ALLOWABLE DEFL.(TL)= L/360 (0.73")

CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.59/1.00 (A-B:1), BC=0.24/1.00 (H-J:4), WB=0.50/1.00 (B-J:1), SS=0.26/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (G) (INPUT = 0.90)

JSI METAL= 0.46 (E) (INPUT = 1.00)

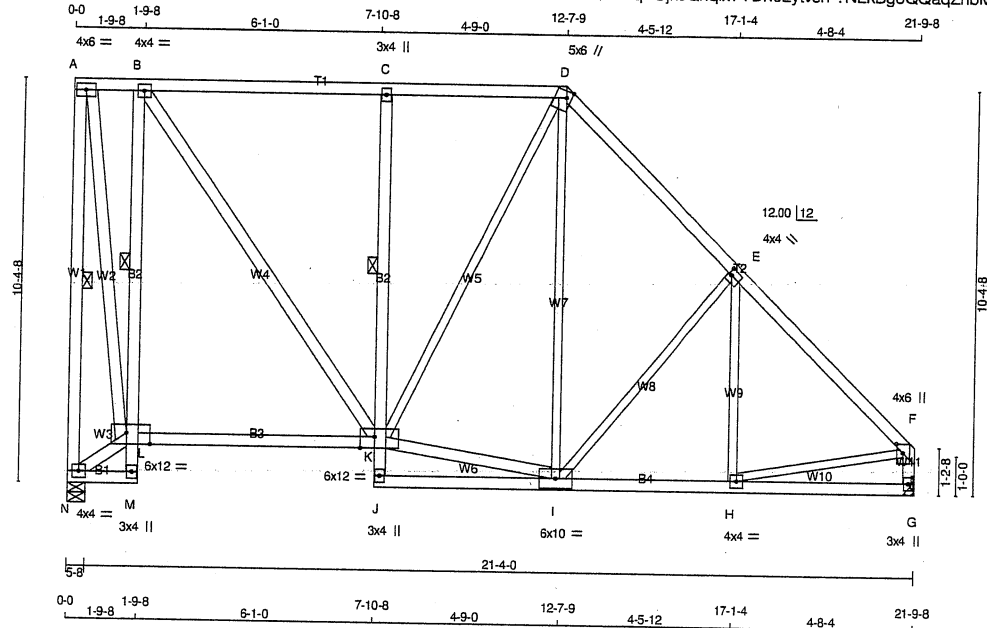


Structural component only
DWG# T-2126640

JOB NAME 420540	TRUSS NAME T14S	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID: X496mqPCjhJQnqfx7YDh0Lytvsh-?NLkBgQqQaqZhbMcC_F_2x7R5tLF3KMvJBdlWzyqyFf



Scale = 1:56.6

LUMBER	N.L.G.A. RULES	SIZE	LUMBER	DESCR.
CHORDS				
N - A	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
J - C	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
K - D	2x3	DRY	No.2	SPF
I - D	2x3	DRY	No.2	SPF
I - E	2x3	DRY	No.2	SPF
H - E	2x3	DRY	No.2	SPF
H - F	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0		
B	TMVW-t	MT20	4.0	4.0		
C	TMV+p	MT20	3.0	4.0		
D	TTVW+m	MT20	5.0	6.0	2.00	1.50
E	TMVW-t	MT20	4.0	4.0	2.00	1.00
F	TMVW+p	MT20	4.0	6.0	2.75	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMVW-t	MT20	4.0	4.0		
I	BMVW-t	MT20	6.0	10.0		
J	BMV+p	MT20	3.0	4.0		
K	BMVW-t	MT20	6.0	12.0	Edge	4.50
L	BMVW-t	MT20	6.0	12.0	Edge	7.25
M	BMV+p	MT20	3.0	4.0		
N	BMVW-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
N	1126	0	1126	0
G	1126	0	1126	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN	COMPONENT REACTIONS	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	800	508 / 0	0 / 0	0 / 0	0 / 0	0 / 0	292 / 0	0 / 0
G	800	508 / 0	0 / 0	0 / 0	0 / 0	0 / 0	292 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.63 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF A-N, B-L, C-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX	CS1 (LC)	UNBRAC LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CS1 (LC)
FR-TO											
N-A	-1094 / 0	0.0	0.0	0.58 (1)	6.04	N-L	-33 / 0	0.00 (1)			
A-B	-209 / 0	-84.9	-84.9	0.29 (1)	6.25	A-L	0 / 1172	0.19 (1)			
B-C	-659 / 0	-84.9	-84.9	0.40 (1)	6.25	B-K	0 / 778	0.12 (1)			
C-D	-658 / 0	-84.9	-84.9	0.39 (1)	6.25	K-I	0 / 606	0.10 (1)			
D-E	-882 / 0	-84.9	-84.9	0.33 (1)	6.10	K-D	0 / 131	0.03 (1)			
E-F	-1087 / 0	-84.9	-84.9	0.35 (1)	5.63	I-D	0 / 203	0.05 (4)			
G-F	-1089 / 0	0.0	0.0	0.11 (1)	7.57	I-E	-307 / 0	0.31 (1)			
						H-E	-55 / 51	0.03 (1)			
						H-F	0 / 805	0.18 (1)			
N-M	0 / 28	-18.5	-18.5	0.02 (4)	10.00						
M-L	0 / 17	0.0	0.0	0.04 (1)	10.00						
L-B	-1096 / 0	0.0	0.0	0.28 (1)	6.00						
L-K	0 / 212	-18.5	-18.5	0.24 (4)	10.00						
J-K	0 / 38	0.0	0.0	0.01 (1)	10.00						
K-C	-531 / 0	0.0	0.0	0.07 (1)	6.25						
J-I	0 / 6	-18.5	-18.5	0.09 (4)	10.00						
I-H	0 / 792	-18.5	-18.5	0.18 (1)	10.00						
H-G	0 / 0	-18.5	-18.5	0.08 (4)	10.00						

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
DL	=	6.0	PSF	
BOT CH.	LL	=	0.0	PSF
DL	=	7.4	PSF	
TOTAL LOAD	=	36.7	PSF	

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL)= L/999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL)= L/999 (0.12")

CSI: TC=0.58/1.00 (A-N:1), BC=0.28/1.00 (B-L:1), WB=0.31/1.00 (E-I:1), SSI=0.22/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

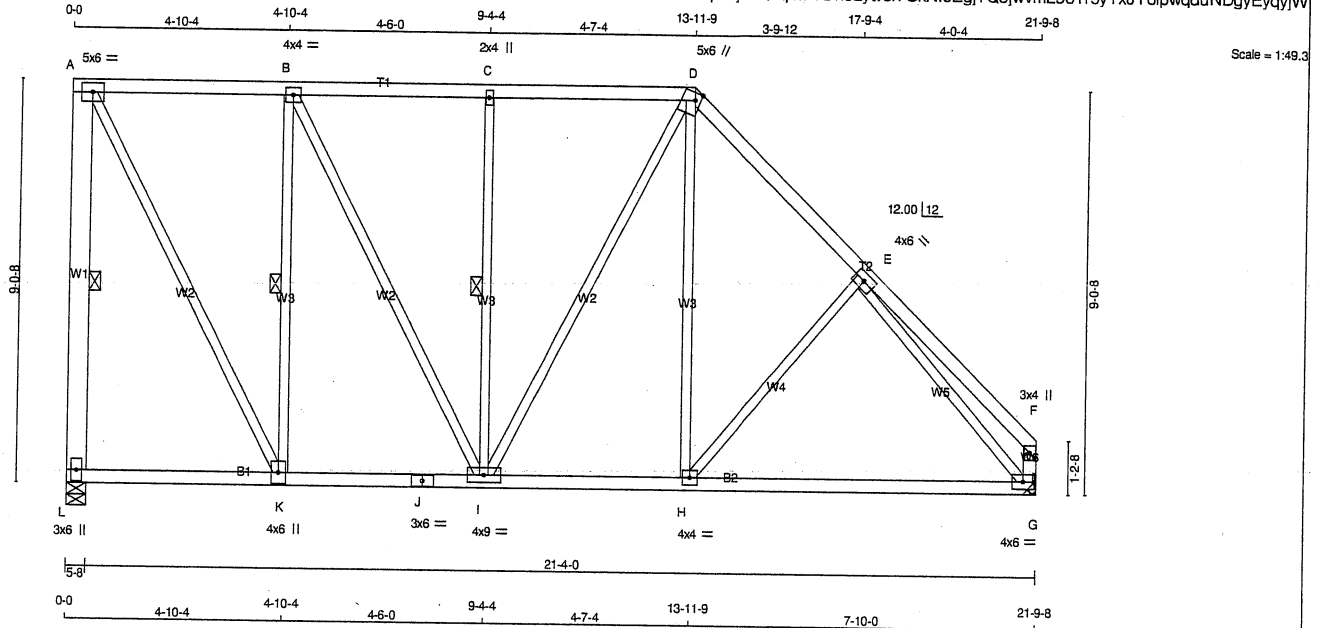
JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.46 (F) (INPUT = 1.00)



Structural component only
DWG# T-2126659

JOB NAME 420538	TRUSS NAME T15	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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TOTAL WEIGHT = 123 lb [M]

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
L - A	2x6	DRY	No.2
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
G - F	2x4	DRY	No.2
L - J	2x4	DRY	No.2
J - G	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0		
B	TMVW-t	MT20	4.0	4.0		
C	TMVW-w	MT20	2.0	4.0		
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMVW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	6.0		
H	BMVW-t	MT20	4.0	4.0		
I	BMVW-t	MT20	4.0	9.0		
J	BS-t	MT20	3.0	6.0		
K	BMVW-t	MT20	4.0	6.0		
L	BMV1-p	MT20	3.0	6.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
L	1126	0	1126	0	5-8
G	1126	0	1126	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	800	508 / 0	0 / 0	0 / 0	0 / 0	292 / 0	0 / 0
G	800	508 / 0	0 / 0	0 / 0	0 / 0	292 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF A-L, B-K, C-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
L-A	-1091 / 0	0.0	0.0 0.26 (1)	6.25	H-D	0 / 282	0.06 (1)
A-B	-513 / 0	-84.9	-84.9 0.26 (1)	6.25	H-E	-173 / 0	0.12 (1)
B-C	-708 / 0	-84.9	-84.9 0.27 (1)	6.25	E-G	-1215 / 0	0.78 (1)
C-D	-708 / 0	-84.9	-84.9 0.23 (1)	6.25	K-B	-817 / 0	0.43 (1)
D-E	-960 / 0	-84.9	-84.9 0.23 (1)	6.05	I-C	-412 / 0	0.22 (1)
E-F	0 / 27	-84.9	-84.9 0.21 (1)	10.00	A-K	0 / 1056	0.24 (1)
G-F	-126 / 0	0.0	0.0 0.01 (1)	7.81	B-I	0 / 423	0.10 (1)
					I-D	0 / 97	0.02 (1)
L-K	0 / 0	-18.5	-18.5 0.11 (4)	10.00			
K-J	0 / 513	-18.5	-18.5 0.16 (4)	10.00			
J-I	0 / 513	-18.5	-18.5 0.16 (4)	10.00			
I-H	0 / 661	-18.5	-18.5 0.31 (4)	10.00			
H-G	0 / 770	-18.5	-18.5 0.31 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
DL =	6.0	PSF	
BOT CH.	LL =	0.0	PSF
DL =	7.4	PSF	
TOTAL LOAD =	36.7	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/999 (0.13")

CSI: TC=0.27/1.00 (B-C:1), BC=0.31/1.00 (G-H:4), WB=0.78/1.00 (E-G:1), SS=0.19/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

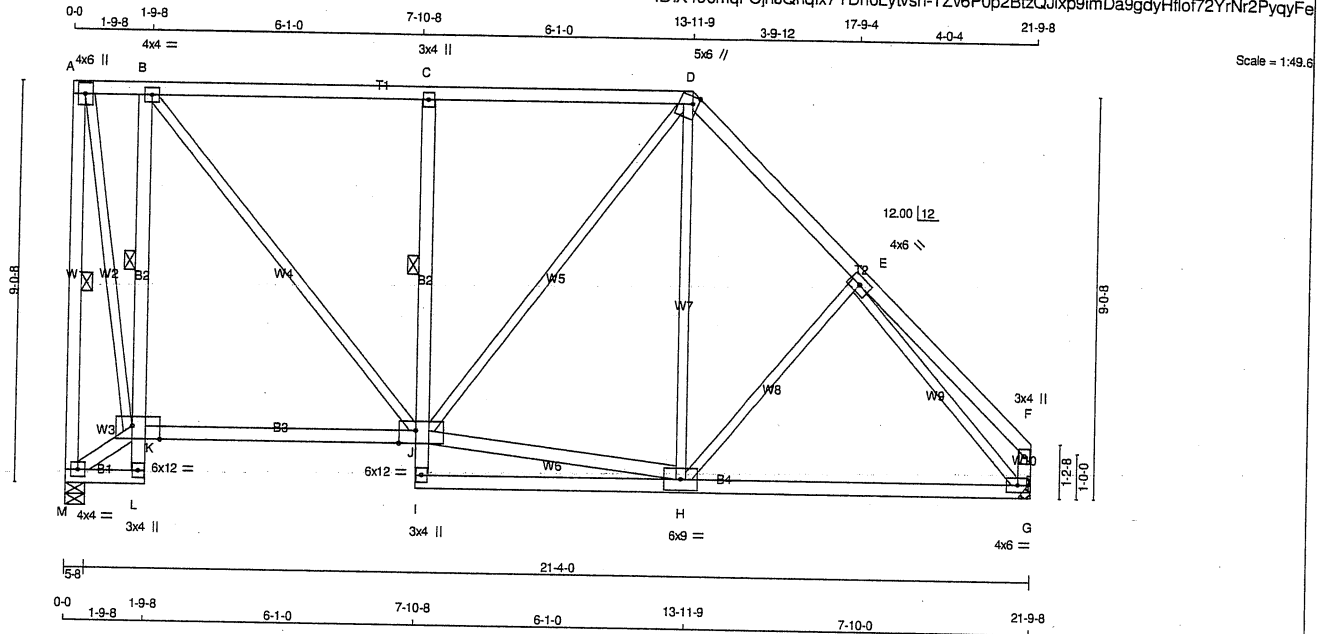
JSI GRIP= 0.75 (A) (INPUT = 0.90)
JSI METAL= 0.28 (E) (INPUT = 1.00)



Structural component only
DWG# T-2126641

JOB NAME 420540	TRUSS NAME T15S	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID: X496mqPCjhJQnqfx7YDh0Lytvsh-TZv6P0p2BtzQJxp9imDa9gdyHfio72YrNr2PyqyFe



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
M - A	2x4	DRY	No.2
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
G - F	2x4	DRY	No.2
M - L	2x4	DRY	No.2
L - B	2x4	DRY	No.2
K - J	2x4	DRY	No.2
I - C	2x4	DRY	No.2
I - G	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
M - K	2x4	DRY	No.2
J - H	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW+p	MT20	4.0	6.0	
B	TMVW-t	MT20	4.0	4.0	
C	TMV+p	MT20	3.0	4.0	
D	TTVW+m	MT20	5.0	6.0	2.00 1.50
E	TMVW-t	MT20	4.0	6.0	
F	TMV+p	MT20	3.0	4.0	
G	BMVW1-t	MT20	4.0	6.0	
H	BMVWVW-t	MT20	6.0	9.0	
I	BMV+p	MT20	3.0	4.0	
J	BVMWVW-I	MT20	6.0	12.0	Edge 4.50
K	BVMWVW-I	MT20	6.0	12.0	Edge 7.50
L	BMV+p	MT20	3.0	4.0	
M	BMVW1-t	MT20	4.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
M	1126	0	1126	0	0	5-8	5-8
G	1126	0	1126	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
M	800	508 / 0	0 / 0	0 / 0	0 / 0	0 / 0	292 / 0	0 / 0
G	800	508 / 0	0 / 0	0 / 0	0 / 0	0 / 0	292 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.01 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.02 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF A-M, B-K, C-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)	MEMB.	FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)
FR-TO		FROM TO		FR-TO			
M-A	-1091 / 0	0.0	0.0 0.41 (1)	6.05	M-K	-39 / 0	0.00 (1)
A-B	-241 / 0	-84.9	-84.9 0.25 (1)	6.25	A-K	0 / 1158	0.26 (1)
B-C	-782 / 0	-84.9	-84.9 0.51 (1)	6.01	B-J	0 / 844	0.19 (1)
C-D	-780 / 0	-84.9	-84.9 0.51 (1)	6.01	J-H	0 / 659	0.11 (1)
D-E	-964 / 0	-84.9	-84.9 0.23 (1)	6.05	J-D	0 / 196	0.04 (1)
E-F	0 / 27	-84.9	-84.9 0.21 (1)	10.00	H-D	0 / 182	0.05 (4)
G-F	-126 / 0	0.0	0.0 0.01 (1)	7.81	H-E	-173 / 0	0.12 (1)
					E-G	-1217 / 0	0.79 (1)
M-L	0 / 33	-18.5	-18.5 0.02 (4)	10.00			
L-K	0 / 17	0.0	0.0 0.05 (1)	10.00			
K-B	-1079 / 0	0.0	0.0 0.17 (1)	6.02			
K-J	0 / 245	-18.5	-18.5 0.24 (4)	10.00			
I-J	0 / 40	0.0	0.0 0.02 (1)	10.00			
J-C	-602 / 0	0.0	0.0 0.05 (1)	6.25			
I-H	0 / 11	-18.5	-18.5 0.27 (4)	10.00			
H-G	0 / 771	-18.5	-18.5 0.33 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/999 (0.13")

CSI: TC=0.51/1.00 (B-C:1), BC=0.33/1.00 (G-H:4), WB=0.79/1.00 (E-G:1), SS=0.24/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

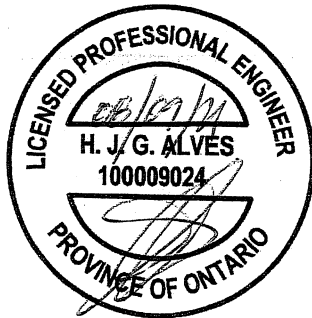
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.88 (B) (INPUT = 0.90)
JSI METAL = 0.35 (K) (INPUT = 1.00)

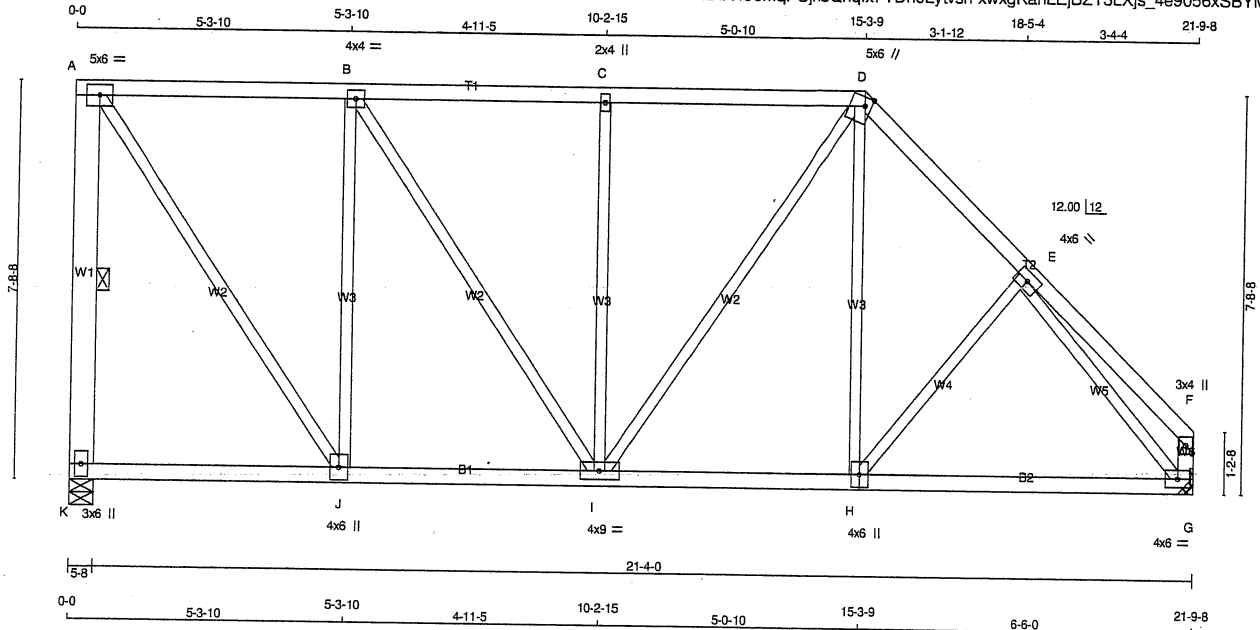


Structural component only
DWG# T-2126660

JOB NAME 420538	TRUSS NAME T16	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

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Scale = 1:42.6

TOTAL WEIGHT = 113 lb [M]

LUMBER					
N. L. G. A. RULES					
CHORDS		SIZE	LUMBER	DESCR	
K - A		2x6	DRY	No.2	SPF
A - D		2x4	DRY	No.2	SPF
D - F		2x4	DRY	No.2	SPF
G - F		2x4	DRY	No.2	SPF
K - H		2x4	DRY	No.2	SPF
H - G		2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0		
B	TMVW-t	MT20	4.0	4.0		
C	TMVW-w	MT20	2.0	4.0		
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMVW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	6.0		
H	BSWW-t	MT20	4.0	6.0		
I	BMVW-t	MT20	4.0	9.0		
J	BMVW-t	MT20	4.0	6.0		
K	BMV1+p	MT20	3.0	6.0		

NOTES: (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
K	1126 0	1126 0	5-8	5-8
G	1126 0	1126 0	MECHANICAL	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
K	800	508 / 0	0 / 0	0 / 0	0 / 0	0 / 0	292 / 0	0 / 0
G	800	508 / 0	0 / 0	0 / 0	0 / 0	0 / 0	292 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF A-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX	CSi (LC)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSi (LC)
FR-TO			FROM TO				FR-TO			
K-A	-1086 / 0	0.0	0.0	0.18 (1)	6.25	H-D	0 / 178	0.04 (1)		
A-B	-646 / 0	-84.9	-84.9	0.32 (1)	6.25	H-E	-99 / 0	0.05 (1)		
B-C	-856 / 0	-84.9	-84.9	0.33 (1)	6.15	E-G	-1222 / 0	0.53 (1)		
C-D	-856 / 0	-84.9	-84.9	0.29 (1)	6.21	J-B	-797 / 0	0.87 (1)		
D-E	-1006 / 0	-84.9	-84.9	0.15 (1)	6.05	I-C	-453 / 0	0.50 (1)		
E-F	0 / 21	-84.9	-84.9	0.14 (1)	10.00	A-J	0 / 1111	0.25 (1)		
G-F	-106 / 0	0.0	0.0	0.01 (1)	7.81	B-I	0 / 379	0.09 (1)		
						I-D	0 / 284	0.06 (1)		
K-J	0 / 0	-18.5	-18.5	0.11 (4)	10.00					
J-I	0 / 646	-18.5	-18.5	0.17 (4)	10.00					
I-H	0 / 696	-18.5	-18.5	0.17 (1)	10.00					
H-G	0 / 757	-18.5	-18.5	0.30 (4)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 36.7 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CSI: TC=0.33/1.00 (B-C:1), BC=0.30/1.00 (G-H:4), WB=0.87/1.00 (B-J:1), SSI=0.21/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

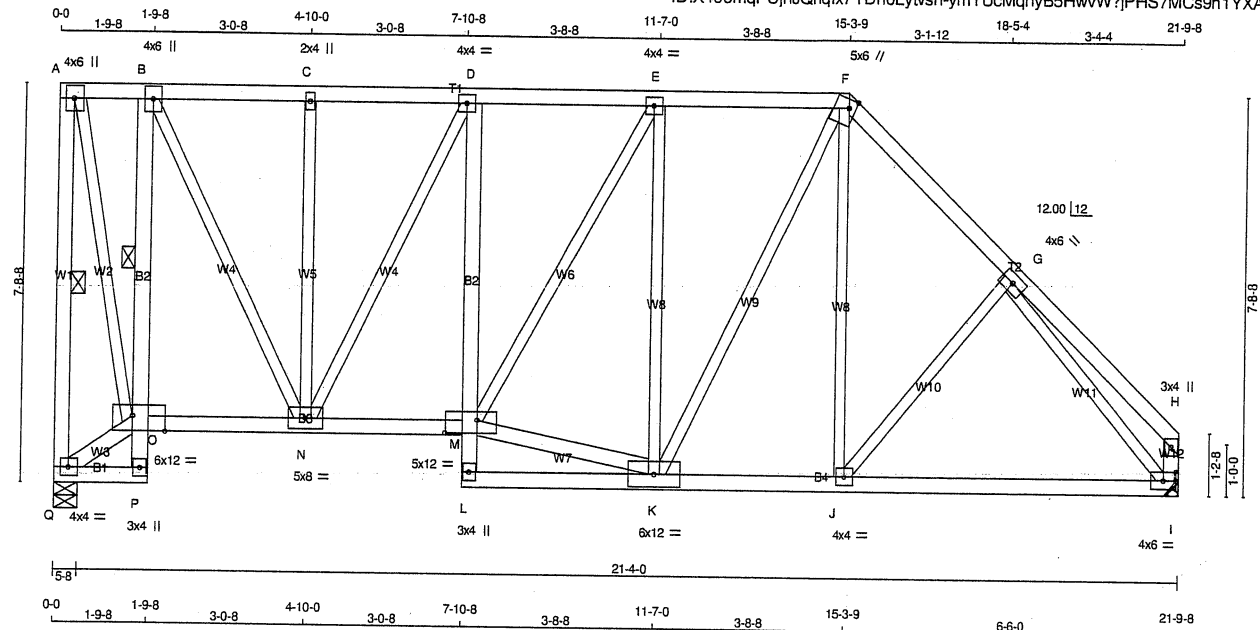
JSI GRIP= 0.73 (E) (INPUT = 0.90)
JSI METAL= 0.29 (J) (INPUT = 1.00)



Structural component only
DWG# T-2126642

JOB NAME 420540	TRUSS NAME T16S	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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TOTAL WEIGHT = 137 lb [M]

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
Q - A	2x4	DRY	No.2
A - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
I - H	2x4	DRY	No.2
Q - P	2x4	DRY	No.2
P - B	2x4	DRY	No.2
O - M	2x4	DRY	No.2
L - D	2x4	DRY	No.2
L - I	2x4	DRY	No.2

ALL WEBS EXCEPT			
	SIZE		
Q - O	2x4	DRY	No.2
M - K	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0		
B	TMVW+p	MT20	4.0	6.0		
C	TMVW+w	MT20	2.0	4.0		
D	TMVW-t	MT20	4.0	4.0		
E	TMVW-t	MT20	4.0	4.0		
F	TTVW+m	MT20	5.0	6.0	2.00	1.50
G	TMVW-t	MT20	4.0	6.0		
H	TMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	6.0		
J	BMVW-t	MT20	4.0	4.0		
K	BMVW-t	MT20	6.0	12.0		
L	BMV+p	MT20	3.0	4.0		
M	BMVW-t	MT20	5.0	12.0	3.00	7.50
N	BMVW-t	MT20	5.0	8.0		
O	BMVW-t	MT20	6.0	12.0	Edge	7.50
P	BMV+p	MT20	3.0	4.0		
Q	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT		REQD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
Q	1126	0	1126	0	0	5-8	5-8
I	1126	0	1126	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	MAX/MIN	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	800	508 / 0	0 / 0	0 / 0	0 / 0	0 / 0	292 / 0	0 / 0
I	800	508 / 0	0 / 0	0 / 0	0 / 0	0 / 0	292 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.03 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.16 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF A-Q, B-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MAX. FACTORED		FACTORED		MAX. FACTORED				
MEMB.	FORCE	VERT. LOAD	MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PLF)	CSI (LC)	UNBRAC		(LBS)	CSI (LC)	
FR-TO		FROM TO		LENGTH	FR-TO			
Q-A	-1092 / 0	0.0	0.0	0.29 (1)	6.05	Q-O	-37 / 0	0.00 (1)
A-B	-267 / 0	-84.9	-84.9	0.04 (1)	6.25	A-O	0 / 1073	0.24 (1)
B-C	-673 / 0	-84.9	-84.9	0.10 (1)	6.25	B-N	0 / 903	0.20 (1)
C-D	-673 / 0	-84.9	-84.9	0.10 (1)	6.25	N-C	-285 / 0	0.21 (1)
D-E	-902 / 0	-84.9	-84.9	0.17 (1)	6.25	N-D	-520 / 0	0.50 (1)
E-F	-839 / 0	-84.9	-84.9	0.17 (1)	6.25	M-K	0 / 861	0.14 (1)
F-G	-1019 / 0	-84.9	-84.9	0.15 (1)	6.03	M-E	0 / 122	0.03 (1)
G-H	0 / 21	-84.9	-84.9	0.13 (1)	10.00	K-E	-451 / 0	0.50 (1)
H-I	-106 / 0	0.0	0.0	0.01 (1)	7.81	K-F	0 / 303	0.07 (1)
						J-F	0 / 194	0.05 (4)
Q-P	0 / 32	-18.5	-18.5	0.02 (4)	10.00	J-G	-98 / 2	0.05 (1)
P-O	0 / 17	0.0	0.0	0.05 (1)	10.00	G-I	-1235 / 0	0.54 (1)
O-B	-1018 / 0	0.0	0.0	0.10 (1)	6.16			
O-N	0 / 272	-18.5	-18.5	0.07 (4)	10.00			
N-M	0 / 905	-18.5	-18.5	0.17 (1)	10.00			
L-M	0 / 34	0.0	0.0	0.03 (1)	10.00			
M-D	0 / 192	0.0	0.0	0.05 (1)	10.00			
L-K	0 / 15	-18.5	-18.5	0.06 (4)	10.00			
K-J	0 / 705	-18.5	-18.5	0.23 (4)	10.00			
J-I	0 / 765	-18.5	-18.5	0.23 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
DL	=	6.0	PSF	
BOT CH.	LL	=	0.0	PSF
DL	=	7.4	PSF	
TOTAL LOAD	=	36.7	PSF	

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/999 (0.08")

CSI: TC=0.29/1.00 (A-Q:1), BC=0.23/1.00 (J-K:4), WB=0.54/1.00 (G-I:1), SS=0.15/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	618	354	1667 788 1987 1656

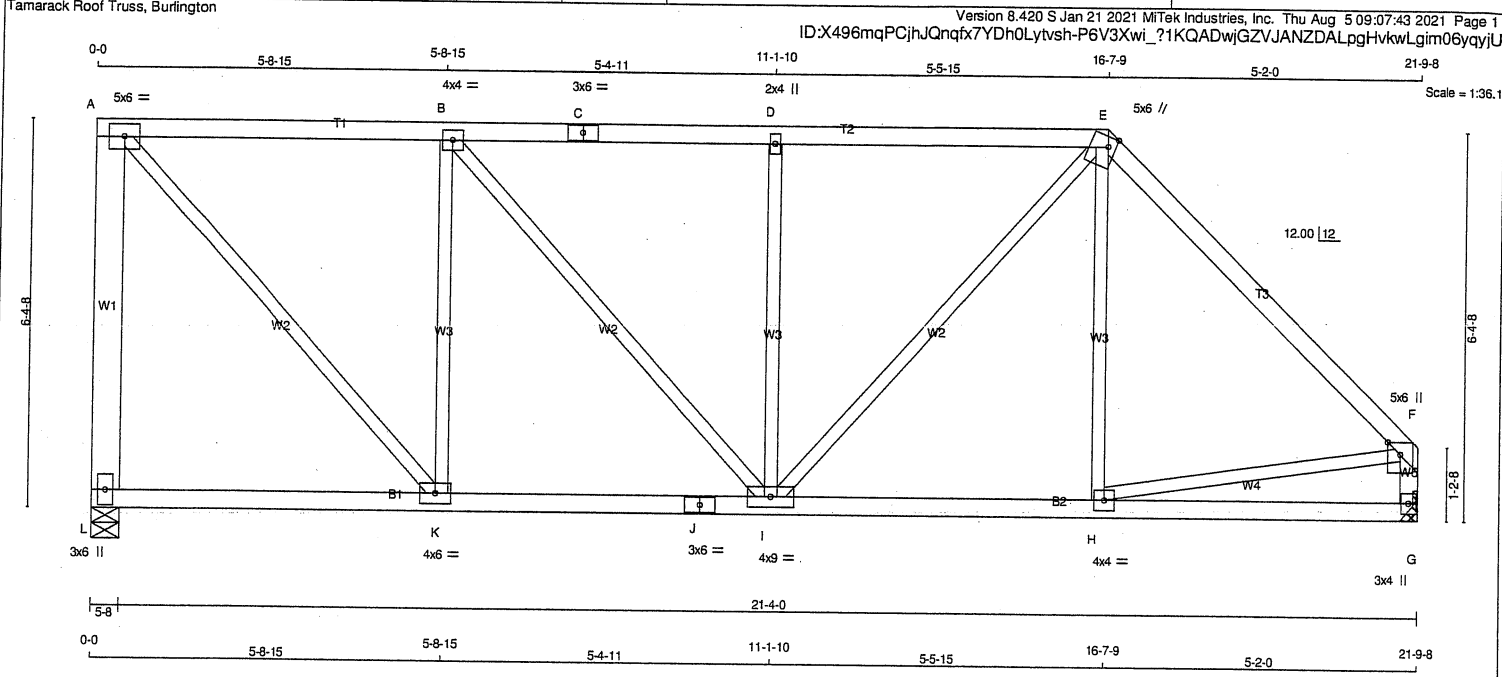
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (G) (INPUT = 0.90)
JSI METAL= 0.32 (O) (INPUT = 1.00)



Structural component only
DWG# T-2126661



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
L - A	2x6	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0		
B	TMVW-t	MT20	4.0	4.0		
C	TS-t	MT20	3.0	6.0		
D	TMVW-w	MT20	2.0	4.0		
E	TTVW+m	MT20	5.0	6.0	2.00	1.50
F	TMVW+p	MT20	5.0	6.0	Edge	
G	BMV1+p	MT20	3.0	4.0		
H	BMVW-t	MT20	4.0	4.0		
I	BMVW-t	MT20	4.0	9.0		
J	BS-t	MT20	3.0	6.0		
K	BMVW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

TOTAL WEIGHT = 101 b

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
L	1126	0	1126	0
G	1126	0	1126	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS
JT	COMBINED
L	800
G	800

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.53 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 MAX (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LC1 MAX (LC)	
FR-TO				FR-TO			
L-A	-1083 / 0	0.0	0.0 0.56 (1)	H-E	-7 / 81	0.03 (4)	
A-B	-836 / 0	-84.9	-84.9 0.39 (1)	H-F	0 / 753	0.17 (1)	
B-C	-1052 / 0	-84.9	-84.9 0.40 (1)	I-D	-495 / 0	0.32 (1)	
C-D	-1052 / 0	-84.9	-84.9 0.40 (1)	K-B	-767 / 0	0.50 (1)	
D-E	-1052 / 0	-84.9	-84.9 0.36 (1)	A-K	0 / 1218	0.27 (1)	
E-F	-1055 / 0	-84.9	-84.9 0.46 (1)	B-I	0 / 326	0.07 (1)	
G-F	-1088 / 0	0.0	0.0 0.11 (1)	I-E	0 / 460	0.10 (1)	
L-K	0 / 0	-18.5	-18.5 0.14 (4)				
K-J	0 / 836	-18.5	-18.5 0.22 (4)				
J-I	0 / 836	-18.5	-18.5 0.22 (4)				
I-H	0 / 743	-18.5	-18.5 0.20 (4)				
H-G	0 / 0	-18.5	-18.5 0.13 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL (LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.56/1.00 (A-L:1), BC=0.22/1.00 (I-K:4), WB=0.50/1.00 (B-K:1), SSI=0.23/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

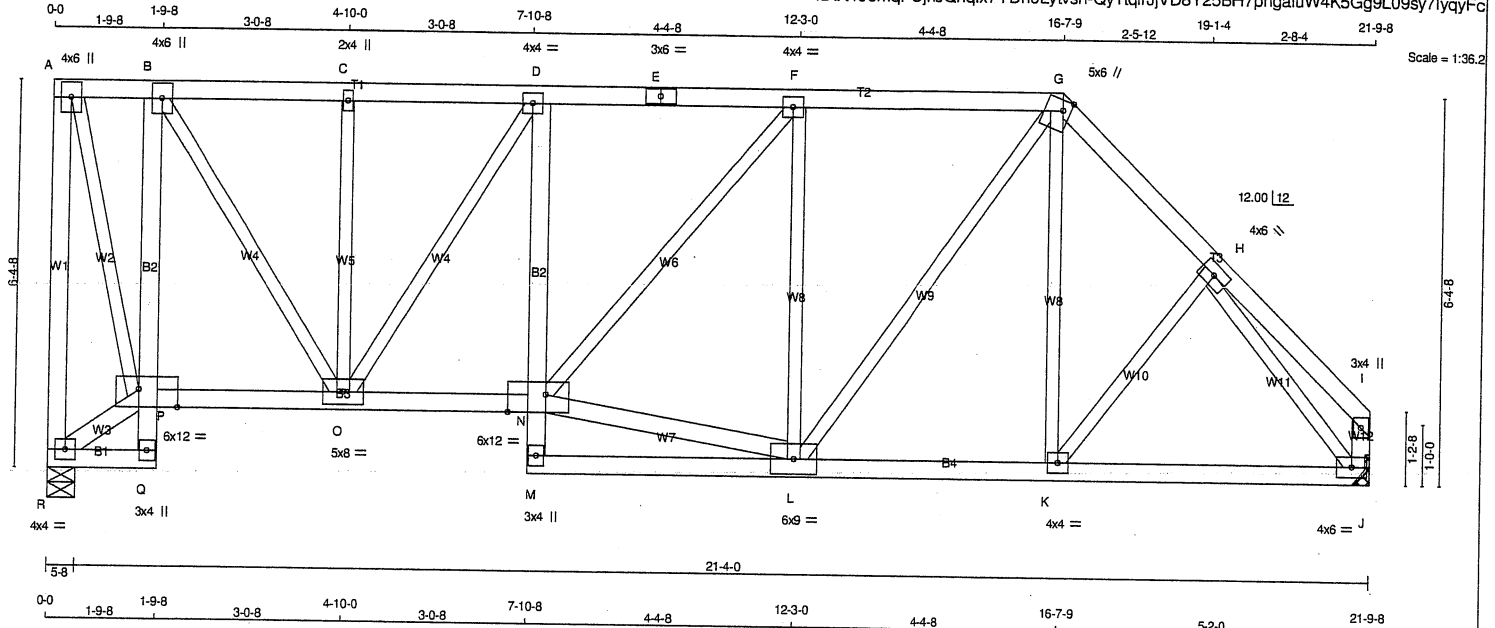
JSI GRIP= 0.87 (K) (INPUT = 0.90)
JSI METAL= 0.36 (F) (INPUT = 1.00)



JOB NAME 420540	TRUSS NAME T17S	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
R - A	2x4	DRY	No.2	SPF	
A - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
G - I	2x4	DRY	No.2	SPF	
I - J	2x4	DRY	No.2	SPF	
J - Q	2x4	DRY	No.2	SPF	
Q - B	2x4	DRY	No.2	SPF	
P - N	2x4	DRY	No.2	SPF	
M - D	2x4	DRY	No.2	SPF	
M - J	2x4	DRY	No.2	SPF	

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
R - P	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0		
B	TMVW+p	MT20	4.0	6.0		
C	TMVW+w	MT20	2.0	4.0		
D	TMVW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMVW-t	MT20	4.0	4.0		
G	TTVW+m	MT20	5.0	6.0	2.00	1.50
H	TMVW-t	MT20	4.0	6.0		
I	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	4.0	6.0		
K	BMVW-t	MT20	4.0	4.0		
L	BMVW-t	MT20	6.0	9.0		
M	BMV+p	MT20	3.0	4.0		
N	BMVW-t	MT20	6.0	12.0	Edge	7.50
O	BMVW-t	MT20	5.0	8.0		
P	BMVW-t	MT20	6.0	12.0	Edge	7.50
Q	BMV+p	MT20	3.0	4.0		
R	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED		MAXIMUM FACTORED			INPUT	REQD
	GROSS REACTION		GROSS REACTION			BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
R	1126	0	1126	0	0	5-8	5-8
J	1126	0	1126	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 3-8.

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	800	508 / 0	0 / 0	0 / 0	0 / 0	292 / 0	0 / 0
J	800	508 / 0	0 / 0	0 / 0	0 / 0	292 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.63 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.68 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED	FORCE	VERT. LOAD	LC1	MAX	MAX.	MEMB.	MAX. FACTORED	FORCE	MAX
			(LBS)	(PLF)		(LC)	UNBRAC			(LBS)	(LC)
FR-TO				FROM	TO		LENGTH	FR-TO			
R-A	-1091 / 0	0.0	0.0	0.89 (1)	7.57	R-P	-38 / 0	0.00 (1)			
A-B	-338 / 0	-84.9	-84.9	0.05 (1)	6.25	A-P	0 / 1097	0.25 (1)			
B-C	-849 / 0	-84.9	-84.9	0.10 (1)	6.25	B-O	0 / 949	0.21 (1)			
C-D	-849 / 0	-84.9	-84.9	0.12 (1)	6.25	O-C	-274 / 0	0.12 (1)			
D-E	-1141 / 0	-84.9	-84.9	0.25 (1)	5.63	O-D	-558 / 0	0.34 (1)			
E-F	-1141 / 0	-84.9	-84.9	0.25 (1)	5.63	N-L	0 / 1031	0.17 (1)			
F-G	-1021 / 0	-84.9	-84.9	0.25 (1)	5.88	N-F	0 / 176	0.04 (1)			
G-H	-1066 / 0	-84.9	-84.9	0.09 (1)	6.01	L-F	-551 / 0	0.36 (1)			
H-I	0 / 15	-84.9	-84.9	0.07 (1)	10.00	L-G	0 / 483	0.11 (1)			
J-I	-87 / 0	0.0	0.0	0.01 (1)	7.81	K-G	0 / 108	0.03 (4)			
						K-H	-12 / 25	0.01 (4)			
						H-J	-1247 / 0	0.37 (1)			
R-Q	0 / 32	-18.5	-18.5	0.02 (4)	10.00						
Q-P	0 / 17	0.0	0.0	0.05 (1)	10.00						
P-B	-1022 / 0	0.0	0.0	0.40 (1)	7.68						
P-O	0 / 344	-18.5	-18.5	0.08 (1)	10.00						
O-N	0 / 1145	-18.5	-18.5	0.21 (1)	10.00						
M-N	0 / 37	0.0	0.0	0.04 (1)	10.00						
N-D	0 / 171	0.0	0.0	0.06 (1)	10.00						
M-L	0 / 24	-18.5	-18.5	0.07 (4)	10.00						
L-K	0 / 741	-18.5	-18.5	0.18 (1)	10.00						
K-J	0 / 747	-18.5	-18.5	0.18 (1)	10.00						

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	36.7	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/999 (0.08")

CSI: TC=0.89/1.00 (A-R:1), BC=0.40/1.00 (B-P:1), WB=0.37/1.00 (H-J:1), SS=0.17/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.1Q SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

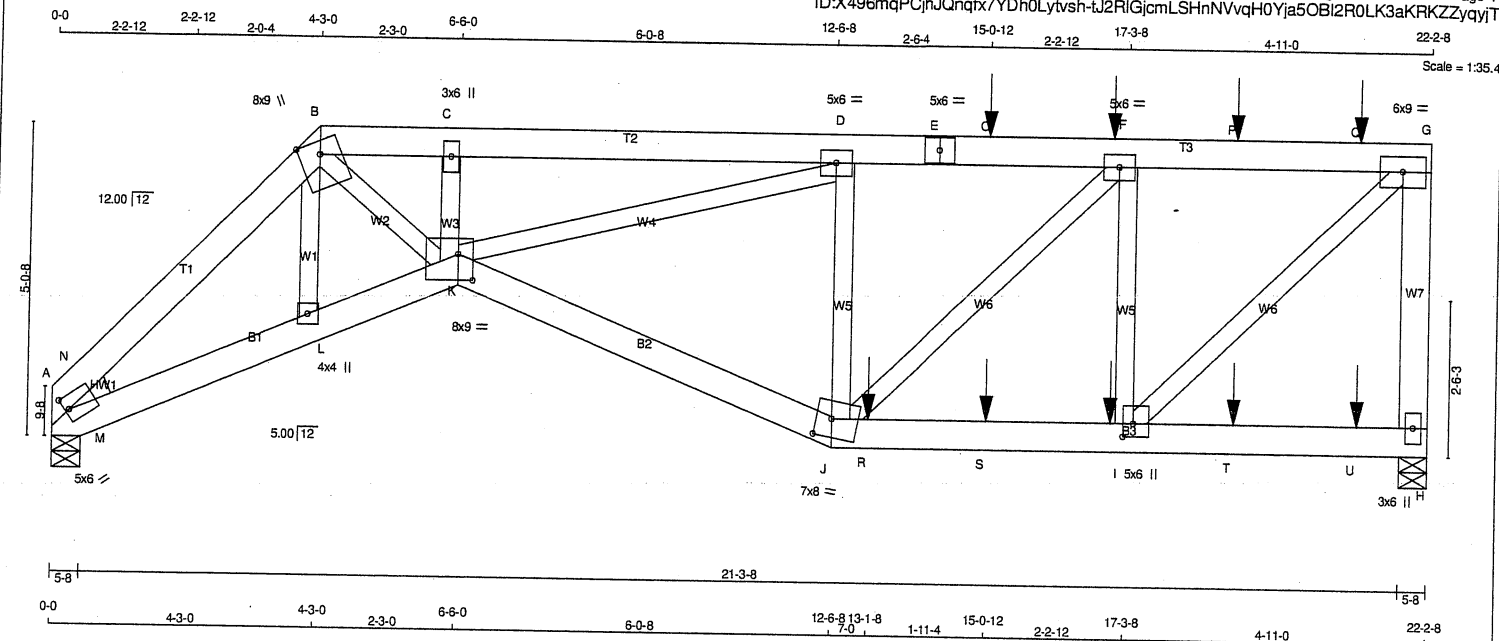
JSI GRIP= 0.82 (H) (INPUT = 0.90)
JSI METAL= 0.33 (F) (INPUT = 1.00)



Structural component only
DWG# T-2126662

Tamarack Roof Truss, Burlington

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ID: X496mqPCjhJQnqfx7YDh0Lyvsh-tj2RlGcmLSHnNVvqH0Yja5OBI2R0LK3aKRKZyqyJT



LUMBER

N. L. G. A. RULES

CHORDS SIZE LUMBER

A - B 2x6 DRY No.2

B - E 2x6 DRY No.2

E - G 2x6 DRY No.2

H - G 2x6 DRY No.2

A - K 2x6 DRY No.2

K - J 2x6 DRY No.2

J - H 2x6 DRY No.2

ALL WEBS 2x4 DRY No.2

SPF

SPF

SPF

SPF

SPF

SPF

SPF

SPF

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DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	HEEL
JT VERT	DOWN	IN-SX	WEDGE
H 2373 0	2373 0	5-8	5-8
A 1724 0	1724 0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
H 1681	1086 / 0	0 / 0 0 / 0 0 / 0 596 / 0 0 / 0
A 1221	788 / 0	0 / 0 0 / 0 0 / 0 433 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, A

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.81 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED	FACTORED	VERT. LOAD LC1	MAX	MAX. UNBRAC	MEMB.	MAX. FACTORED	MAX
FR-TO	LOC.	FORCE (LBS)	VERT. (PLF)	CS1 (LC)	LENGTH	FR-TO	LOC.	FORCE (LBS)	MAX
A-N	-3704 / 0	-84.9	-84.9	0.28 (1)	4.16	L-B	-161 / 0	0.02 (1)	0.02 (1)
N-B	-3063 / 0	-84.9	-84.9	0.28 (1)	4.53	B-K	0 / 3070	0.54 (1)	0.54 (1)
B-C	-4412 / 0	-84.9	-84.9	0.15 (1)	4.02	K-C	-216 / 0	0.03 (1)	0.03 (1)
C-D	-4396 / 0	-84.9	-84.9	0.37 (1)	3.81	K-D	0 / 1484	0.26 (1)	0.26 (1)
D-E	-3020 / 0	-84.9	-84.9	0.28 (1)	4.57	J-D	-1043 / 0	0.25 (1)	0.25 (1)
E-O	-3020 / 0	-84.9	-84.9	0.28 (1)	4.57	J-F	0 / 1190	0.21 (1)	0.21 (1)
O-F	-3020 / 0	-84.9	-84.9	0.28 (1)	4.57	I-F	-1640 / 0	0.40 (1)	0.40 (1)
F-P	-2163 / 0	-84.9	-84.9	0.28 (1)	5.20	I-G	0 / 2956	0.52 (1)	0.52 (1)
P-Q	-2163 / 0	-84.9	-84.9	0.28 (1)	5.20	M-N	0 / 653	0.00 (1)	0.00 (1)
Q-G	-2163 / 0	-84.9	-84.9	0.28 (1)	5.20				
H-G	-2331 / 0	0.0	0.0	0.55 (1)	6.69				

A-M	0 / 2473	-18.5	-18.5	0.54 (1)	10.00
M-L	0 / 2236	-18.5	-18.5	0.51 (1)	10.00
L-K	0 / 2322	-18.5	-18.5	0.44 (1)	10.00
K-J	0 / 3238	-18.5	-18.5	0.48 (1)	10.00
J-R	0 / 2164	-18.5	-18.5	0.70 (1)	10.00
R-S	0 / 2164	-18.5	-18.5	0.70 (1)	10.00
S-I	0 / 2164	-18.5	-18.5	0.70 (1)	10.00
I-T	0 / 0	-18.5	-18.5	0.15 (4)	10.00
T-U	0 / 0	-18.5	-18.5	0.15 (4)	10.00
U-H	0 / 0	-18.5	-18.5	0.15 (4)	10.00

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	17-0-12	-101	-101	---	FRONT	VERT	TOTAL	---	C1
I	17-0-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
O	15-0-12	-101	-101	---	FRONT	VERT	TOTAL	---	C1
P	19-0-12	-101	-101	---	FRONT	VERT	TOTAL	---	C1
R	21-0-12	-101	-101	---	FRONT	VERT	TOTAL	---	C1
S	13-1-8	-766	-766	---	FRONT	VERT	TOTAL	---	C1
T	15-0-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
U	19-0-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
U	21-0-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/999 (0.12")
ALLOWABLE DEFL.(TL) = L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/999 (0.23")

CSI: TC=0.55/1.00 (G-H:1), BC=0.70/1.00 (I-J:1), WB=0.54/1.00 (B-K:1), SS=0.64/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

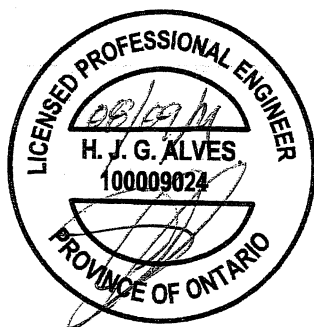
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.90 (J) (INPUT = 0.90)

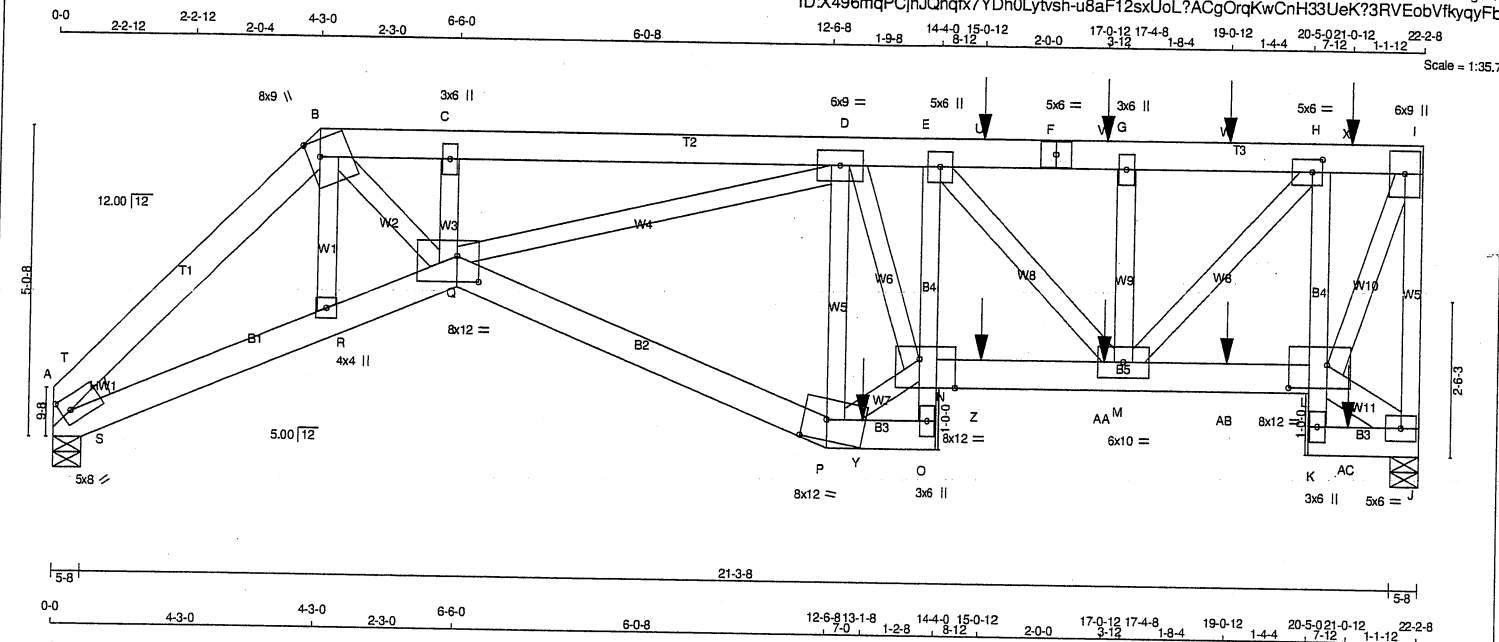
JSI METAL = 0.76 (I) (INPUT = 1.00)



Structural component only
DWG# T-2126644

JOB NAME 420540	TRUSS NAME T18S1	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID:X496mqPCjhJQnqfx7YDh0Lytvsh-u8aF12sxUoL?ACgOrqKwCnH33UeK?3RVeobVfkyqYb



LUMBER				DESCR.	
N. L. G. A. RULES	SIZE	LUMBER			
CHORDS					
A - B	2x6	DRY	No.2	SPF	
B - F	2x6	DRY	No.2	SPF	
F - I	2x6	DRY	No.2	SPF	
J - I	2x4	DRY	No.2	SPF	
A - Q	2x6	DRY	No.2	SPF	
Q - P	2x6	DRY	No.2	SPF	
P - O	2x6	DRY	No.2	SPF	
O - E	2x4	DRY	No.2	SPF	
N - L	2x6	DRY	No.2	SPF	
K - H	2x4	DRY	No.2	SPF	
K - J	2x6	DRY	No.2	SPF	
ALL WEBS	2x4	DRY	No.2	SPF	
EXCEPT					
L - J	2x6	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBH1-m	MT20	5.0	8.0	2.50	1.75
B	TTWW+m	MT20	8.0	9.0	Edge	2.25
C	TMW+w	MT20	3.0	6.0		
D	TMWVW-t	MT20	6.0	9.0		
E	TMVW+p	MT20	5.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMW+w	MT20	3.0	6.0		
H	TMVW-t	MT20	5.0	6.0	2.50	2.00
I	TMVW+p	MT20	6.0	9.0		
J	BMVW-t	MT20	5.0	6.0		
K	BMV+p	MT20	3.0	6.0		
L	BMVWV-t	MT20	8.0	12.0	4.50	7.50
M	BMVWV-t	MT20	6.0	10.0		
N	BMVWV-t	MT20	8.0	12.0	Edge	7.25
O	BMV+p	MT20	3.0	6.0		
P	BBWVW-m	MT20	8.0	12.0	4.00	4.50
Q	BBWVW-p	MT20	8.0	12.0	5.00	4.25
R	BMV+w	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
		GROSS REACTION	GROSS REACTION	BRG	HEEL
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
J	2343	0	2343	0	5-8
A	1716	0	1716	0	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN	COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
J	1861	1066 / 0	0 / 0	0 / 0	0 / 0	594 / 0	0 / 0
A	1216	783 / 0	0 / 0	0 / 0	0 / 0	433 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, A

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.80 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 5.69 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PLATE BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRAC	MEMB.	FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM	TO	LENGTH	FR-TO		
A-T	-3687 / 0	-84.9	-84.9	0.29 (1)	4.16	R-B	-146 / 0
T-B	-3068 / 0	-84.9	-84.9	0.29 (1)	4.52	B-Q	0 / 3057
B-C	-4389 / 0	-84.9	-84.9	0.15 (1)	4.03	Q-C	-219 / 0
C-D	-4372 / 0	-84.9	-84.9	0.39 (1)	3.80	Q-D	0 / 1495
D-E	-3528 / 0	-84.9	-84.9	0.31 (1)	4.22	P-D	-2277 / 0
E-U	-2588 / 0	-84.9	-84.9	0.16 (1)	4.99	P-N	0 / 3514
U-F	-2588 / 0	-84.9	-84.9	0.16 (1)	4.99	D-N	0 / 1221
F-V	-2588 / 0	-84.9	-84.9	0.16 (1)	4.99	E-M	-1450 / 0
V-G	-2588 / 0	-84.9	-84.9	0.14 (1)	4.99	M-G	-488 / 0
G-W	-2588 / 0	-84.9	-84.9	0.14 (1)	5.02	M-H	0 / 2376
W-H	-2588 / 0	-84.9	-84.9	0.14 (1)	5.02	L-J	-69 / 0
H-X	-991 / 0	-84.9	-84.9	0.07 (1)	6.25	L-I	0 / 2374
X-I	-991 / 0	-84.9	-84.9	0.07 (1)	6.25	S-T	0 / 627
I-J	-2280 / 0	0.0	0.0	0.84 (1)	5.58		

A-S	0 / 2469	-18.5	-18.5	0.53 (1)	10.00
S-R	0 / 2244	-18.5	-18.5	0.50 (1)	10.00
R-Q	0 / 2310	-18.5	-18.5	0.43 (1)	10.00
Q-P	0 / 3197	-18.5	-18.5	0.48 (1)	10.00
P-Y	0 / 88	-18.5	-18.5	0.32 (1)	10.00
Y-O	0 / 88	-18.5	-18.5	0.32 (1)	10.00
O-N	0 / 400	0.0	0.0	0.20 (1)	10.00
N-E	0 / 1170	0.0	0.0	0.34 (1)	10.00
N-Z	0 / 3552	-18.5	-18.5	0.52 (1)	10.00
Z-AA	0 / 3552	-18.5	-18.5	0.52 (1)	10.00
AA-M	0 / 3552	-18.5	-18.5	0.52 (1)	10.00
M-AB	0 / 1008	-18.5	-18.5	0.20 (1)	10.00
AB-L	0 / 1008	-18.5	-18.5	0.20 (1)	10.00
K-L	0 / 46	0.0	0.0	0.10 (1)	10.00
L-H	-2078 / 0	0.0	0.0	0.43 (1)	5.69
K-AC	0 / 59	-18.5	-18.5	0.03 (4)	10.00
AC-J	0 / 59	-18.5	-18.5	0.03 (4)	10.00

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
U	15-0-12	-54	-54	---	FRONT	VERT	TOTAL	---	C1
V	17-0-12	-54	-54	---	FRONT	VERT	TOTAL	---	C1
W	19-0-12	-54	-54	---	FRONT	VERT	TOTAL	---	C1
X	21-0-12	-101	-101	---	FRONT	VERT	TOTAL	---	C1
Y	13-1-8	-766	-766	---	FRONT	VERT	TOTAL	---	C1
Z	15-0-12	-64	-64	---	FRONT	VERT	TOTAL	---	C1
AA	17-0-12	-63	-63	---	FRONT	VERT	TOTAL	---	C1
AB	19-0-12	-63	-63	---	FRONT	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
DL = 6.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.4	PSF	
TOTAL LOAD = 36.7	PSF	

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.74")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.13")
ALLOWABLE DEFL.(TL) = $L/360$ (0.74")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.25")

CSI: TC=0.84/1.00 (H-J:1), BC=0.53/1.00 (A-S:1), WB=0.62/1.00 (N-P:1), SSI=0.48/1.00 (O-P:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
MIN	MIN	MIN
MT20	618	354
	1667	788
	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.88 (H) (INPUT = 0.90)

JSI METAL = 0.55 (P) (INPUT = 1.00)



Structural component only
DWG# T-2126663

CONTINUED ON PAGE 2

JOB NAME 420540	TRUSS NAME T18S1	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
AC	21-0-12	-29	-29	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

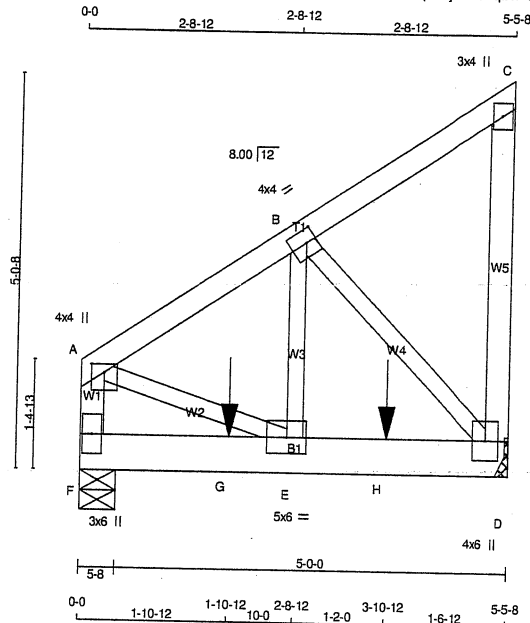
- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2126663 *m*

JOB NAME 420538	TRUSS NAME T19	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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Scale = 1:28.0

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
F-A	12	TOP
A-C	12	TOP
C-D	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F-D	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
B	TMVW-t	MT20	4.0	4.0	2.00	1.50
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMVW-t	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	6.0		

NOTES-

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
F	1007	0	0	5-8
D	1101	0	0	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 2-0.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	711	472 / 0	0 / 0	0 / 0	0 / 0	0 / 0	239 / 0	0 / 0
D	777	517 / 0	0 / 0	0 / 0	0 / 0	0 / 0	260 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED			MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	
FR-TO		VERT. LOAD (PLF)	LC1	MAX CSI (LC)		FR-TO		
F-A	-849 / 0	0.0	0.0	0.05 (1)	7.81	A-E	0 / 781	0.10 (1)
A-B	-868 / 0	-84.9	-84.9	0.05 (1)	6.25	E-B	0 / 967	0.12 (1)
B-C	-13 / 0	-84.9	-84.9	0.05 (1)	6.25	B-D	-1054 / 0	0.14 (1)
D-C	-92 / 0	0.0	0.0	0.02 (1)	7.81			
F-G	0 / 0	-18.5	-18.5	0.10 (1)	10.00			
G-E	0 / 0	-18.5	-18.5	0.10 (1)	10.00			
E-H	0 / 733	-18.5	-18.5	0.18 (1)	10.00			
H-D	0 / 733	-18.5	-18.5	0.18 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-10-12	-544	-544	---	TOP	VERT	TOTAL	---	C1
H	3-10-12	-544	-544	---	TOP	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 30 = 60 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
DL = 6.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.4	PSF	
TOTAL LOAD = 36.7	PSF	

SPACING = 24.0 IN C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.05/1.00 (A-B:1), BC=0.18/1.00 (D-E:1), WB=0.14/1.00 (B-D:1), SSI=0.22/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

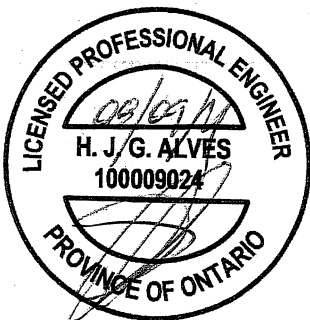
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.58 (B) (INPUT = 0.90)
JSI METAL = 0.17 (D) (INPUT = 1.00)

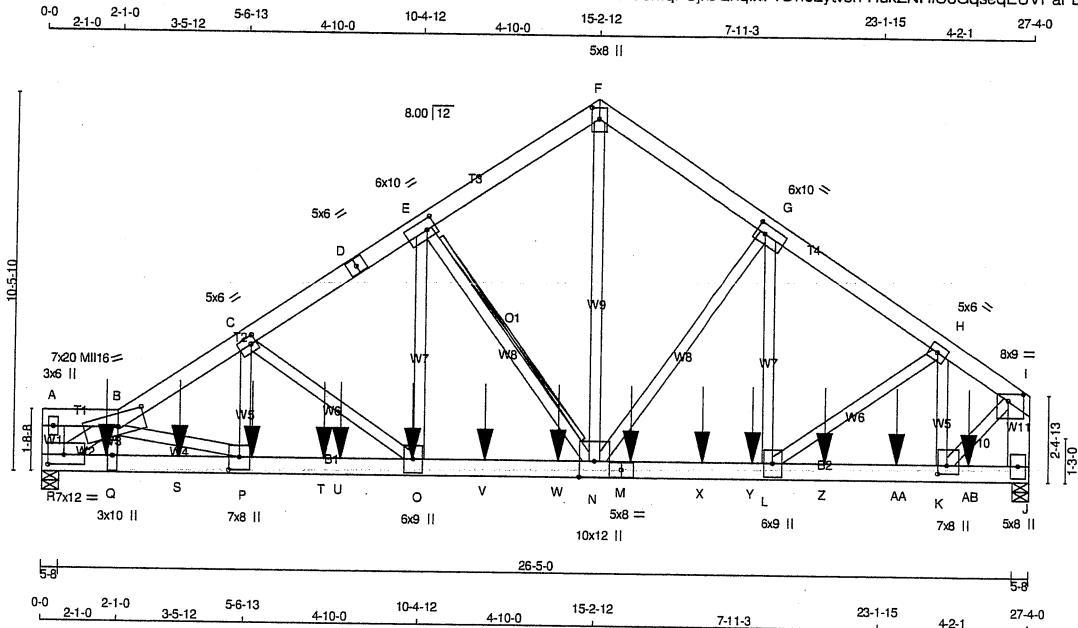


Structural component only
DWG# T-2126645

JOB NAME 420538	TRUSS NAME T21	QUANTITY 1	PLY 3	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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LUMBER				N. L. G. A. RULES	
CHORDS	SIZE	LUMBER	DESCR.		
B - D	2x6	DRY	No.2	SPF	
D - F	2x6	DRY	No.2	SPF	
F - I	2x6	DRY	No.2	SPF	
J - I	2x8	DRY	No.2	SPF	
A - B	2x6	DRY	No.2	SPF	
R - M	2x6	DRY	2100F 1.8E	SPF	
M - J	2x6	DRY	2100F 1.8E	SPF	
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF	
C - O	2x4	DRY	2100F 1.8E	SPF	
E - N	2x4	DRY	2100F 1.8E	SPF	
N - G	2x4	DRY	2100F 1.8E	SPF	
L - H	2x4	DRY	2100F 1.8E	SPF	
R - A	2x8	DRY	No.2	SPF	
R - B	2x6	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF **3** TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
B-D	2 12	TOP
D-F	2 12	TOP
F-I	2 12	TOP
A-B	2 12	TOP
J-I	2 12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
R-M	2 5	SIDE(1195.1)
M-J	2 7	SIDE(831.3)
WEBS : (0.122"x3") SPIRAL NAILS		
2x4	1 6	
2x6	2 6	
2x8	2 6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG			
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX		
J	12391	0	12391	0	0	5-8	5-8		
R	15623	0	15623	0	0	5-8	5-8		

UNFACTORED REACTIONS							
JT	1ST LCASE	MAX./MIN.	COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	8785	5640 / 0	0 / 0	0 / 0	0 / 0	3144 / 0	0 / 0
R	11079	7099 / 0	0 / 0	0 / 0	0 / 0	3979 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, R
BEARING SIZE FACTOR = 1.15 AT JNT(S) R (BASED ON SUPPORT DEPTH = 1-8)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x6 DRY SPF No.2 T-BRACE AT E-N

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	
FR-TO		FROM TO		FR-TO			
B-C	-24233 / 0	-84.9 -84.9 0.39 (1)	2.89	P-C	0 / 6508	0.35 (1)	
C-D	-18321 / 0	-84.9 -84.9 0.29 (1)	3.41	C-O	-6099 / 0	0.53 (1)	
D-E	-18321 / 0	-84.9 -84.9 0.29 (1)	3.41	O-E	0 / 9502	0.51 (1)	
E-F	-12545 / 0	-84.9 -84.9 0.15 (1)	4.16	E-N	-8290 / 0	0.48 (1)	
F-G	-12539 / 0	-84.9 -84.9 0.16 (1)	4.15	N-F	0 / 13494	0.72 (1)	
G-H	-13689 / 0	-84.9 -84.9 0.18 (1)	3.99	N-G	-1627 / 0	0.34 (1)	
H-I	-10123 / 0	-84.9 -84.9 0.07 (1)	4.61	L-G	0 / 1564	0.08 (1)	
I-J	-12277 / 0	0.0 0.0 0.24 (1)	6.00	L-H	0 / 3674	0.09 (1)	
A-B	0 / 0	-84.9 -84.9 0.01 (1)	10.00	K-H	-4446 / 0	0.23 (1)	
R-Q	0 / 21311	-18.5 -18.5 0.51 (1)	10.00	K-I	0 / 10865	0.57 (1)	
Q-S	0 / 21148	-18.5 -18.5 0.56 (1)	10.00	R-A	-76 / 0	0.00 (1)	
S-P	0 / 21148	-18.5 -18.5 0.56 (1)	10.00	Q-B	0 / 1384	0.07 (1)	
P-T	0 / 20193	-18.5 -18.5 0.65 (1)	10.00	R-B	-25865 / 0	0.58 (1)	
T-U	0 / 20193	-18.5 -18.5 0.65 (1)	10.00	B-P	-1012 / 0	0.06 (1)	
U-O	0 / 20193	-18.5 -18.5 0.65 (1)	10.00				
O-V	0 / 15266	-18.5 -18.5 0.52 (1)	10.00				
V-W	0 / 15266	-18.5 -18.5 0.52 (1)	10.00				
W-N	0 / 15266	-18.5 -18.5 0.52 (1)	10.00				
N-M	0 / 11387	-18.5 -18.5 0.46 (1)	10.00				
M-X	0 / 11387	-18.5 -18.5 0.46 (1)	10.00				
X-Y	0 / 11387	-18.5 -18.5 0.46 (1)	10.00				
Y-L	0 / 11387	-18.5 -18.5 0.46 (1)	10.00				
L-Z	0 / 8417	-18.5 -18.5 0.30 (1)	10.00				
Z-AA	0 / 8417	-18.5 -18.5 0.30 (1)	10.00				
AA-K	0 / 8417	-18.5 -18.5 0.30 (1)	10.00				
K-AB	0 / 0	-18.5 -18.5 0.07 (1)	10.00				
AB-J	0 / 0	-18.5 -18.5 0.07 (1)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
M	16-2-12	-1474	-1474	---	BACK	VERT	TOTAL	---	C1
O	10-2-12	-1474	-1474	---	BACK	VERT	TOTAL	---	C1

DESIGN CRITERIA			
SPECIFIED LOADS:			
TOP CH.	LL	=	23.3 PSF
	DL	=	6.0 PSF
BOT CH.	LL	=	0.0 PSF
	DL	=	7.4 PSF
TOTAL LOAD = 36.7 PSF			

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.90")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (0.90")
CALCULATED VERT. DEFL.(TL) = L/ 979 (0.33")

CSI: TC=0.39/1.00 (B-C:1) , BC=0.65/1.00 (O-P:1) , WB=0.72/1.00 (F-N:1) , SSI=0.58/1.00 (L-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY)	SHEAR	SECTION		
(PSI)	(PLI)	(PLI)			
	MAX MIN	MAX MIN	MAX MIN		
MT20	650 371	1747 788	1987 1873		
MI16	438 302	2547 1256	4283 1816		

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (E) (INPUT = 0.90)
JSI METAL= 0.85 (F) (INPUT = 1.00)

Structural component only
DWG# T-2126647

CONTINUED ON PAGE 2



Structural component only
DWG# T-2126647

JOB NAME 420538	TRUSS NAME T21	QUANTITY 1	PLY 3	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMW+w	MT20	3.0	6.0		
B	TTWWW-m	MIH6	7.0	20.0	4.25	9.00
C	TMWW-t	MT20	5.0	6.0	2.50	1.75
D	TS-t	MT20	5.0	6.0		
E	TMWW-t	MT20	6.0	10.0	3.25	3.00
F	TTW+p	MT20	5.0	8.0	3.75	2.50
G	TMWW-t	MT20	6.0	10.0	3.00	3.00
H	TMWW-t	MT20	5.0	6.0		
I	TMVW-p	MT20	8.0	9.0	2.25	5.25
J	BMV1+p	MT20	5.0	8.0		
K	BMWW-t	MT20	7.0	8.0	2.75	3.00
L	BMWW-t	MT20	6.0	9.0		
M	BS-t	MT20	5.0	8.0		
N	BMWWW+t	MT20	10.0	12.0	5.25	5.00
O	BMWW-t	MT20	6.0	9.0		
P	BMWW-t	MT20	7.0	8.0	4.25	3.50
Q	BMW+w	MT20	3.0	10.0		
R	BMWW1-t	MT20	7.0	12.0	3.25	5.25

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
P	5-9-4	-1474	-1474	---	BACK	VERT	TOTAL	---	C1
Q	1-9-4	-1474	-1474	---	BACK	VERT	TOTAL	---	C1
S	3-9-4	-1474	-1474	---	BACK	VERT	TOTAL	---	C1
T	7-9-4	-1474	-1474	---	BACK	VERT	TOTAL	---	C1
U	8-2-12	-1474	-1474	---	BACK	VERT	TOTAL	---	C1
V	12-2-12	-1474	-1474	---	BACK	VERT	TOTAL	---	C1
W	14-2-12	-1474	-1474	---	BACK	VERT	TOTAL	---	C1
X	18-2-12	-1474	-1474	---	BACK	VERT	TOTAL	---	C1
Y	19-7-12	-785	-785	---	BACK	VERT	TOTAL	---	C1
Z	21-7-12	-785	-785	---	BACK	VERT	TOTAL	---	C1
AA	23-7-12	-785	-785	---	BACK	VERT	TOTAL	---	C1
AB	25-7-12	-785	-785	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2126647 3/12

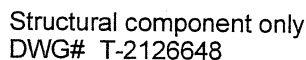
Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Aug 5 09:07:48 2021 Page 1
ID:X496mqPCjhJQnqfx7YDh0Lytvsh-l4Iybdm6qZyiG ph375VtQG0ZML3v4CFvPvXiKvovvE



1) Lateral braces to be a minimum of 2X4 SPF #2.

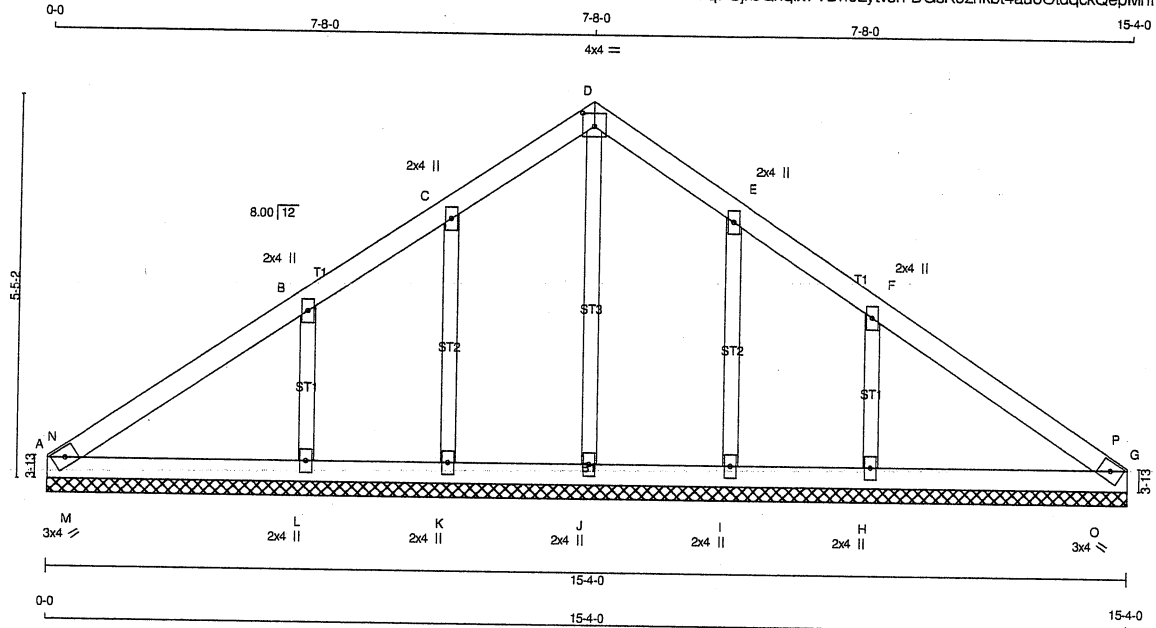
C H O R D S				W E B S				
MAX. FACTORED		FACTORED		MAX. FACTORED		MAX. FACTORED		
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC	MEMB.	FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM	TO	LENGTH	FR-TO			
A-Q	-3329 / 0	-84.9	-84.9	0.09 (4)	4.27	O-B	0 / 199	0.07 (4)
Q-B	-3326 / 0	-84.9	-84.9	0.39 (1)	4.00	B-N	-562 / 0	0.38 (1)
B-C	-2801 / 0	-84.9	-84.9	0.40 (1)	4.27	N-D	0 / 447	0.17 (4)
C-D	-2801 / 0	-84.9	-84.9	0.40 (1)	4.27	N-L	0 / 2346	0.46 (1)
D-E	-2258 / 0	-84.9	-84.9	0.36 (1)	4.69	L-F	-731 / 0	0.87 (1)
E-F	-2262 / 0	-84.9	-84.9	0.67 (1)	4.27	F-G	-781 / 0	0.35 (1)
F-G	-2905 / 0	-84.9	-84.9	0.78 (1)	3.80	K-F	0 / 492	0.19 (4)
G-H	-2905 / 0	-84.9	-84.9	0.78 (1)	3.80	K-H	0 / 2481	0.67 (1)
H-I	0 / 32	-84.9	-84.9	1.1 (1)	10.00	P-Q	-224 / 0	0.00 (1)
J-H	-2377 / 0	0.0	0.0	0.29 (1)	5.42			
A-P	0 / 2783	-18.5	-18.5	0.42 (1)	10.00			
P-O	0 / 2783	-18.5	-18.5	0.42 (1)	10.00			
O-N	0 / 2783	-43.8	-92.8	0.67 (4)	10.00			
N-M	0 / 27	-92.8	-143.5	0.44 (4)	10.00			
M-L	0 / 293	0.0	0.0	0.12 (4)	10.00			
L-E	0 / 2116	0.0	0.0	0.49 (4)	10.00			
L-K	0 / 2444	-143.5	-94.2	0.91 (4)	10.00			
K-J	0 / 0	-94.2	-43.5	0.69 (4)	10.00			

JSI METAL= 0.61 (H) (INPUT = 1.00)



JOB NAME 420538	TRUSS NAME T23G	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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TOTAL WEIGHT = 52 lb [M]

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
A - G	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

GABLE STUDS SPACED AT 2'-0" OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	4.0		
B, C, E, F						
B	TMW+w	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
G	TBM1-h	MT20	3.0	4.0		
H, I, J, K, L						
H	BMW1+w	MT20	2.0	4.0		

NOTES (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	
FR-TO		FROM TO	CSI (LC)	FR-TO		FROM TO	CSI (LC)
A-N	-23 / 0	-84.9 -84.9	0.02 (4)	6.25	J-D	-125 / 0	0.05 (1)
N-B	-27 / 0	-84.9 -84.9	0.11 (1)	6.25	K-C	-156 / 0	0.04 (1)
B-C	-43 / 0	-84.9 -84.9	0.11 (1)	6.25	L-B	-264 / 0	0.04 (1)
C-D	-31 / 0	-84.9 -84.9	0.05 (1)	6.25	I-E	-156 / 0	0.04 (1)
D-E	-31 / 0	-84.9 -84.9	0.05 (1)	6.25	H-F	-264 / 0	0.04 (1)
E-F	-43 / 0	-84.9 -84.9	0.11 (1)	6.25	M-N	-160 / 2	0.00 (1)
F-P	-27 / 0	-84.9 -84.9	0.11 (1)	6.25	O-P	-160 / 2	0.00 (1)
P-G	-23 / 0	-84.9 -84.9	0.02 (4)	6.25			
A-M	0 / 33	-18.5 -18.5	0.09 (1)	10.00			
M-L	0 / 33	-18.5 -18.5	0.09 (1)	10.00			
L-K	0 / 24	-18.5 -18.5	0.06 (1)	10.00			
K-J	0 / 20	-18.5 -18.5	0.02 (4)	10.00			
J-I	0 / 20	-18.5 -18.5	0.02 (4)	10.00			
I-H	0 / 24	-18.5 -18.5	0.06 (1)	10.00			
H-O	0 / 33	-18.5 -18.5	0.09 (1)	10.00			
O-G	0 / 33	-18.5 -18.5	0.09 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
DL	=	6.0	PSF	
BOT CH.	LL	=	0.0	PSF
DL	=	7.4	PSF	
TOTAL LOAD	=	36.7	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11/1.00 (B-N:1), BC=0.09/1.00 (A-M:1), WB=0.05/1.00 (D-J:1), SS=0.13/1.00 (A-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

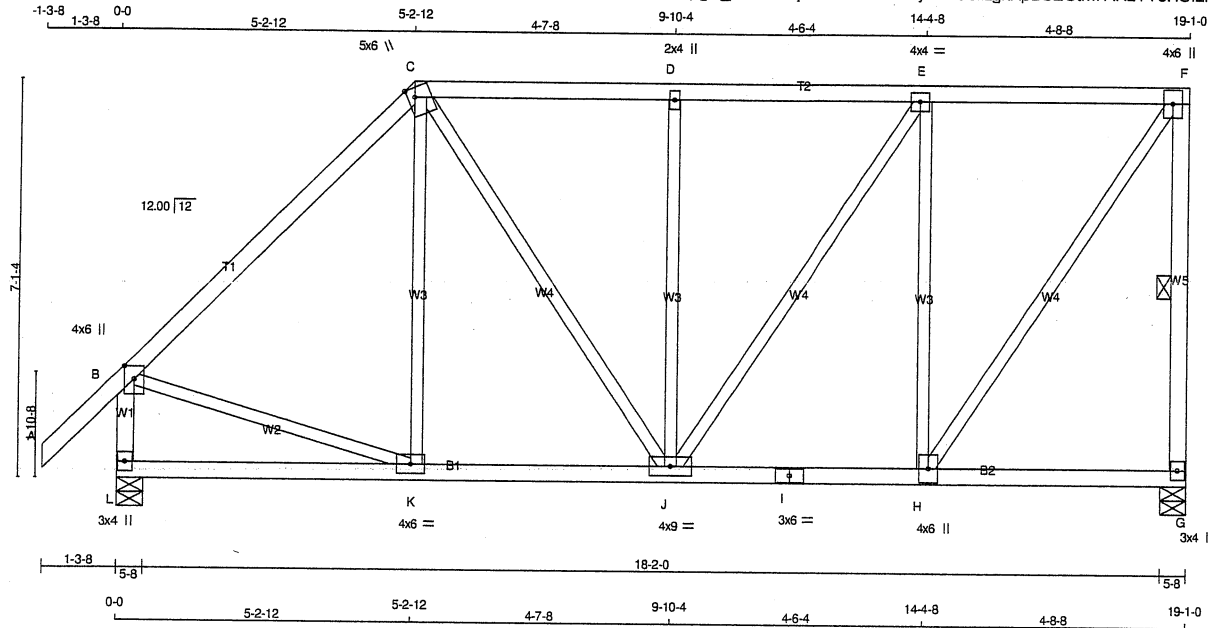
JSI GRIP= 0.17 (F) (INPUT = 0.90)

JSI METAL= 0.13 (B) (INPUT = 1.00)



Structural component only
DWG# T-2126664

JOB NAME 420542	TRUSS NAME T24	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



TOTAL WEIGHT = 97 lb
[M]

LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER		
A - C	2x4	DRY	No.2	SPF	
C - F	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
L - B	2x4	DRY	No.2	SPF	
L - I	2x4	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	4.0	6.0	2.75 2.00
C	TTWW+m	MT20	5.0	6.0	2.00 1.50
D	TMVW+w	MT20	2.0	4.0	
E	TMVW-t	MT20	4.0	4.0	
F	TMVW+p	MT20	4.0	6.0	
G	BMV1+p	MT20	3.0	4.0	
H	BMVW+t	MT20	4.0	6.0	
I	BS-t	MT20	3.0	6.0	
J	BMVWW-t	MT20	4.0	9.0	
K	BMVW-t	MT20	4.0	6.0	
L	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX		REQD BRG IN-SX	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
G	986	0	986	0	0	5-8	5-8		
L	1105	0	1105	0	0	5-8	5-8		

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	700	444 / 0	0 / 0	0 / 0	0 / 0	256 / 0	0 / 0
L	782	509 / 0	0 / 0	0 / 0	0 / 0	272 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.13 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO					FR-TO		
A-B	0 / 42	-84.9	-84.9 0.12 (1)	10.00	K-C	-52 / 60	0.05 (1)
B-C	-796 / 0	-84.9	-84.9 0.45 (1)	6.13	B-K	0 / 584	0.13 (1)
C-D	-717 / 0	-84.9	-84.9 0.24 (1)	6.25	C-J	0 / 278	0.06 (1)
D-E	-717 / 0	-84.9	-84.9 0.26 (1)	6.25	J-D	-417 / 0	0.36 (1)
E-F	-545 / 0	-84.9	-84.9 0.25 (1)	6.25	J-E	0 / 310	0.07 (1)
G-F	-951 / 0	0.0	0.0 0.22 (1)	6.25	H-E	-690 / 0	0.60 (1)
L-B	-1065 / 0	0.0	0.0 0.12 (1)	7.64	H-F	0 / 959	0.22 (1)
L-K	0 / 0	-18.5	-18.5 0.12 (4)	10.00			
K-J	0 / 560	-18.5	-18.5 0.17 (4)	10.00			
J-I	0 / 545	-18.5	-18.5 0.15 (4)	10.00			
I-H	0 / 545	-18.5	-18.5 0.15 (4)	10.00			
H-G	0 / 0	-18.5	-18.5 0.10 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3 PSF
DL = 6.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.4 PSF	
TOTAL LOAD = 36.7 PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.64")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.64")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.45/1.00 (B-C:1), BC=0.17/1.00 (J-K:4), WB=0.60/1.00 (E-H:1), SSI=0.19/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 788
		1987 1656

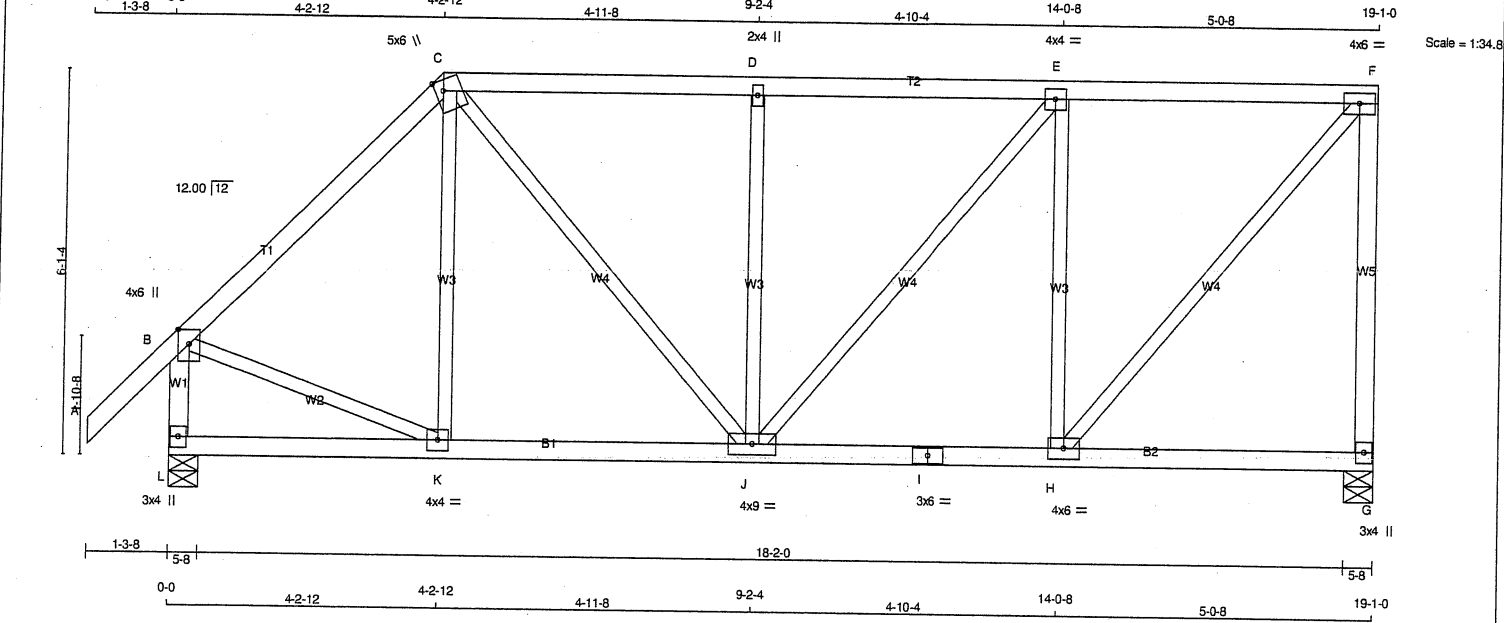
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (B) (INPUT = 0.90)
JSI METAL= 0.38 (B) (INPUT = 1.00)



Structural component only
DWG# T-2126666



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT VERT	986	0	5-8
G	986	0	5-8
L	1105	0	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX. MIN. COMPONENT REACTIONS	DEAD	SOIL
JT COMBINED	700	444 / 0	0 / 0
G	782	509 / 0	0 / 0
L			

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF
DL = 6.0 PSF

BOT CH. LL = 0.0 PSF
DL = 7.4 PSF

TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.64")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.64")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.68/1.00 (F-G:1) , BC=0.17/1.00 (H-J:4) , WB=0.39/1.00 (E-H:1) , SS=0.20/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR (PSI)	SECTION (PL)	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1667	788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (F) (INPUT = 0.90)
JSI METAL= 0.38 (B) (INPUT = 1.00)

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMVW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMVW-t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMVW-t	MT20	4.0	9.0		
K	BMVW-t	MT20	4.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

LOADING

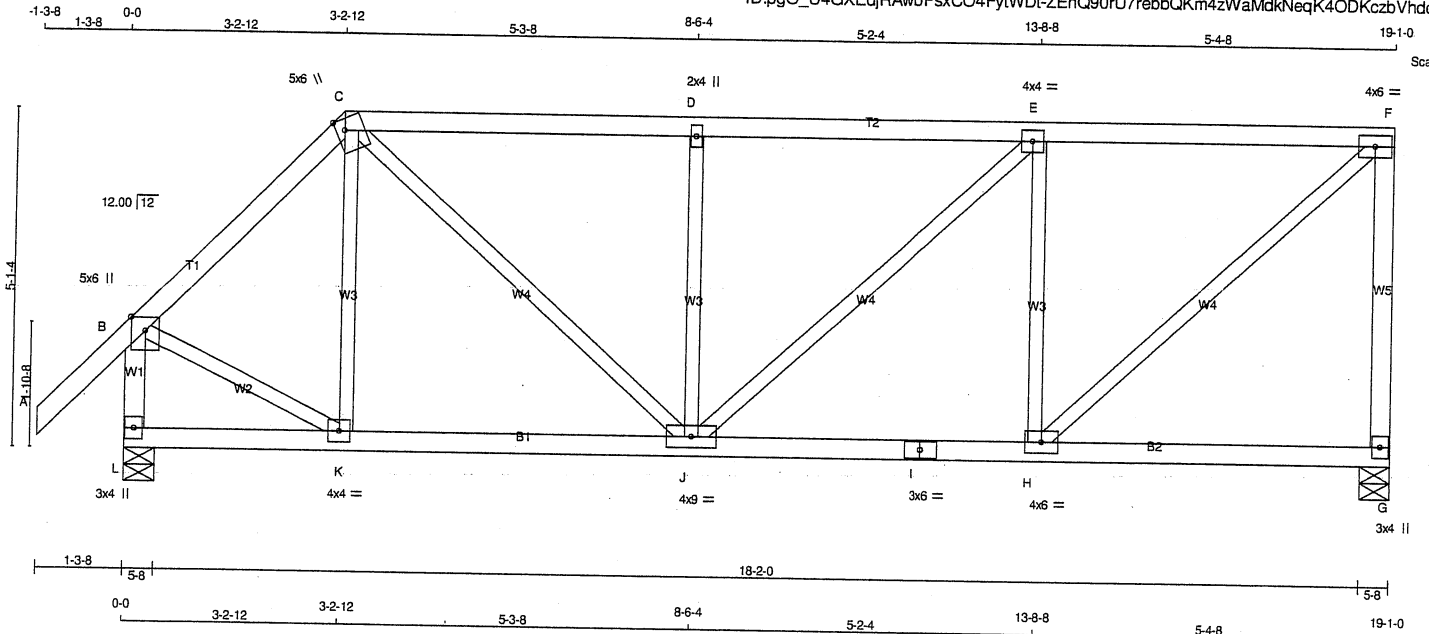
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRAC. LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRAC. LENGTH (LC)	
FR-TO				FR-TO			
A-B	0 / 42	-84.9	-84.9 0.12 (1)	10.00	K-C	-103 / 33	0.06 (1)
B-C	-811 / 0	-84.9	-84.9 0.28 (1)	6.25	B-K	0 / 606	0.14 (1)
C-D	-846 / 0	-84.9	-84.9 0.29 (1)	6.24	C-J	0 / 425	0.10 (1)
D-E	-846 / 0	-84.9	-84.9 0.30 (1)	6.22	J-D	-448 / 0	0.26 (1)
E-F	-672 / 0	-84.9	-84.9 0.29 (1)	6.25	J-E	0 / 271	0.06 (1)
G-F	-948 / 0	0.0	0.0 0.88 (1)	7.81	H-E	-670 / 0	0.39 (1)
L-B	-1074 / 0	0.0	0.0 0.12 (1)	7.61	H-F	0 / 1026	0.23 (1)
L-K	0 / 0	-18.5	-18.5 0.09 (4)	10.00			
K-J	0 / 569	-18.5	-18.5 0.15 (1)	10.00			
J-I	0 / 672	-18.5	-18.5 0.17 (4)	10.00			
I-H	0 / 672	-18.5	-18.5 0.17 (4)	10.00			
H-G	0 / 0	-18.5	-18.5 0.11 (4)	10.00			



JOB NAME 420542	TRUSS NAME T26	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER		
A - C	2x4	DRY	No.2	SPF	
C - F	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
L - B	2x4	DRY	No.2	SPF	
L - I	2x4	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMVW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMVW-t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMVWW-t	MT20	4.0	9.0		
K	BMVW-t	MT20	4.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT		VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
G		986	0	986	0	0	5-8	5-8	
L		1105	0	1105	0	0	5-8	5-8	

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN.	COMPONENT REACTIONS						
G	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
L	700	444 / 0	0 / 0	0 / 0	0 / 0	256 / 0	0 / 0		
G	782	509 / 0	0 / 0	0 / 0	0 / 0	272 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.73 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC) (LC)	MEMB.	MAX. FORCE (LBS)	MAX. UNBRACED LENGTH	MAX. (LC)
FR-TO		FROM	TO	FR-TO			
A-B	0 / 42	-84.9	-84.9 0.12 (1)	10.00	K-C	-167 / 4	0.06 (1)
B-C	-804 / 0	-84.9	-84.9 0.16 (1)	6.25	B-K	0 / 622	0.14 (1)
C-D	-1019 / 0	-84.9	-84.9 0.33 (1)	5.76	C-J	0 / 615	0.14 (1)
D-E	-1019 / 0	-84.9	-84.9 0.35 (1)	5.73	J-D	-479 / 0	0.18 (1)
E-F	-850 / 0	-84.9	-84.9 0.34 (1)	6.14	J-E	0 / 231	0.05 (1)
G-F	-946 / 0	0.0	0.0 0.42 (1)	7.81	H-E	-650 / 0	0.25 (1)
L-B	-1085 / 0	0.0	0.0 0.12 (1)	7.58	H-F	0 / 1141	0.26 (1)
L-K	0 / 0	-18.5	-18.5 0.08 (4)	10.00			
K-J	0 / 562	-18.5	-18.5 0.14 (1)	10.00			
J-I	0 / 850	-18.5	-18.5 0.20 (1)	10.00			
I-H	0 / 850	-18.5	-18.5 0.20 (1)	10.00			
H-G	0 / 0	-18.5	-18.5 0.12 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN/C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.64")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.64")
CALCULATED VERT. DEFL.(TL) = L/999 (0.06")

CSI: TC=0.42/1.00 (F-G:1), BC=0.20/1.00 (H-J:1), WB=0.26/1.00 (F-H:1), SSI=0.21/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (H) (INPUT = 0.90)
JSI METAL= 0.29 (B) (INPUT = 1.00)



Structural component only
DWG# T-2126668

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 -1-3-8 1-3-8 0-0 2-2-12 2-2-12 1-8-8 3-11-4 2-6-7 6-5-11 4-1-11 10-7-6 4-1-11 14-9-1 4-3-15 19-1-0
 Scale = 1:33



ALL WEBS 2x3 DRY				No.2	SPF	<u>UNFACTORED REACTIONS</u>							<u>SPECIFIED LOADS:</u>				
EXCEPT						1ST LCASE	MAX./MIN. COMPONENT REACTIONS						TOP CH.	LL =	23.3	PSF	
						JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	BOT CH.	LL =	6.0	PSF
						I	968	622 / 0	0 / 0	0 / 0	0 / 0	346 / 0	0 / 0		LL =	0.0	PSF
DRY: SEASONED LUMBER.						O	1599	1054 / 0	0 / 0	0 / 0	0 / 0	545 / 0	0 / 0		LL =	7.4	PSF
												TOTAL	LL =	23.3	PSF		

PLATES (table is in inches) JT TYPE PLATES B TMVW-40 MT20 5.0 6.0 1.75 2.50					BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.70 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.		LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12
--	--	--	--	--	---	--	---

TMW-W-t	MT20	4.0	6.0											THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015
TMV-W-t	MT20	5.0	6.0											
BMV1+p	MT20	3.0	6.0											
J, M, N														
				CHORDS					WEBS					
				MAX. FACTORED		FACTORED			MAX. FACTORED		FACTORED			
				MEMB.	FORCE	VERT. LOAD	LC1	MAX	MAX	MEMB.	FORCE	MAX	MAX	

Edge - INDICATES REFERENCE CORNER OF PLATE	C-P	-2511 / 0	-84.9	-84.9	0.54 (1)	3.70	C-M	0 / 1632	0.38 (1)	- CSA 086-09, CSA 086-14
TOUCHES EDGE OF CHORD	P-D	-2511 / 0	-84.9	-84.9	0.54 (1)	3.70	C-M	0 / 1632	0.40 (1)	- TPIC 2011, TPIC 2014
	D-E	-2511 / 0	-84.9	-84.9	0.54 (1)	3.70	M-D	-117 / 25	0.03 (1)	

O - Q	0 / 0	-18.5	-18.5	0.15 (1)	10.00	ALLOWABLE DEFL.(TL)= L/360 (0.64") CALCULATED VERT. DEFL.(TL)= L/999 (0.11")
Q - N	0 / 0	-18.5	-18.5	0.15 (1)	10.00	
N - P	0 / 1280	19.5	19.5	0.13 (1)		

E-R	0/1371	-18.5	-18.5	0.24 (1)	10.00
K-J	0/1371	-18.5	-18.5	0.24 (1)	10.00
J-I	0/0	-18.5	-18.5	0.04 (4)	10.00

COMP=1.00 SHEAR=1.00 TENS=1.00

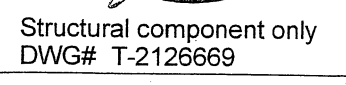
COMPANION LIVE LOAD FACTOR = 1.00

	C	2-2-12	-118	-118	--	BACK	VERT	TOTAL	--	C1	RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.
	P	2-2-12	-85	-85	--	FRONT	VERT	SNOW	--	C1	
	C	2-11-4	-112	-112	--	BACK	VERT	SNOW	--	C1	

CONNECTION REQUIREMENTS

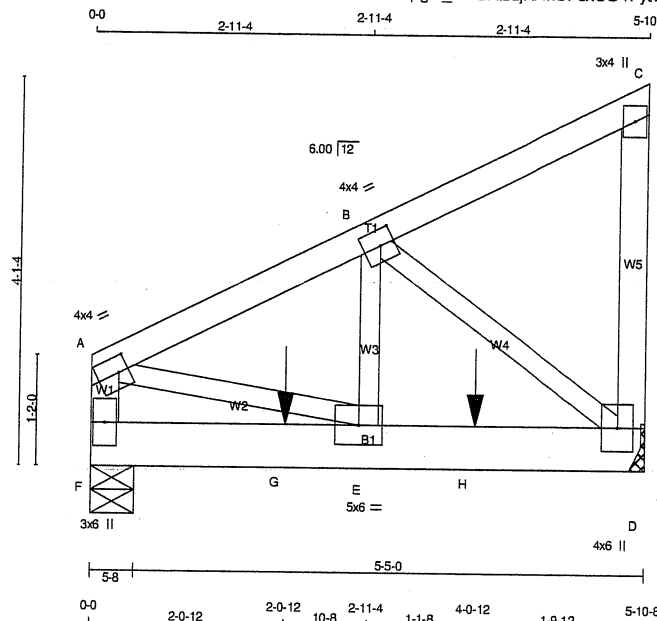
JSI GRIP = 0.88 (H) (INPUT = 0.90)
JSI METAL = 0.59 (R) (INPUT = 1.00)

DWG# T-2126669		
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JOB NAME 420542	TRUSS NAME T28	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1:23.3

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
F - A	2x4	DRY	No.2
A - C	2x4	DRY	No.2
D - C	2x4	DRY	No.2
F - D	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

DESIGN CONSISTS OF **2** TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
F-A 1	12	TOP
A-C 1	12	TOP
C-D 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F-D 2	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0	2.00	1.25
B	TMVW-t	MT20	4.0	4.0	2.00	1.75
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMVW-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG IN-SX	REQD BRG IN-SX
JT	VERT	HORZ	DOWN	HORZ
F	938	0	938	0
D	994	0	994	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 2-0.

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	664	430 / 0	0 / 0	0 / 0	0 / 0	234 / 0	0 / 0
D	704	456 / 0	0 / 0	0 / 0	0 / 0	247 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
F-A	-798 / 0	0.0	0.0 0.04 (1)	A-E	0 / 944	0.12 (1)	
A-B	-1009 / 0	-84.9	-84.9 0.06 (1)	E-B	0 / 856	0.11 (1)	
B-C	-11 / 0	-84.9	-84.9 0.05 (1)	B-D	-1148 / 0	0.14 (1)	
D-C	-101 / 0	0.0	0.0 0.01 (1)				
F-G	0 / 0	-18.5	-18.5 0.09 (1)				
G-E	0 / 0	-18.5	-18.5 0.09 (1)				
E-H	0 / 912	-18.5	-18.5 0.18 (1)				
H-D	0 / 912	-18.5	-18.5 0.18 (1)				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	2-0-12	-468	-468	---	BACK	VERT	TOTAL	---	C1
H	4-0-12	-468	-468	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 29 = 58 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = **24.0 IN. C/C**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.08/1.00 (A-B:1), BC=0.18/1.00 (D-E:1), WB=0.14/1.00 (B-D:1), SS=0.19/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (A) (INPUT = 0.90)
JSI METAL= 0.21 (D) (INPUT = 1.00)



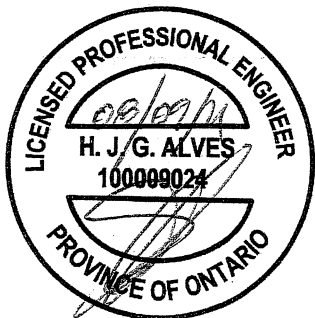
Structural component only
DWG# T-2126670

CONTINUED ON PAGE 2

JOB NAME 420542	TRUSS NAME T28	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Aug 5 10:41:35 2021 Page 2 ID:pgO U4GXEdiRAwbPxsCO4FytWDt-VdoBZhskfTuJrkU8COY2R2ppne0ws9hvQv ohgyqxLU	

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
F	BMV1+p	MT20	3.0	6.0	

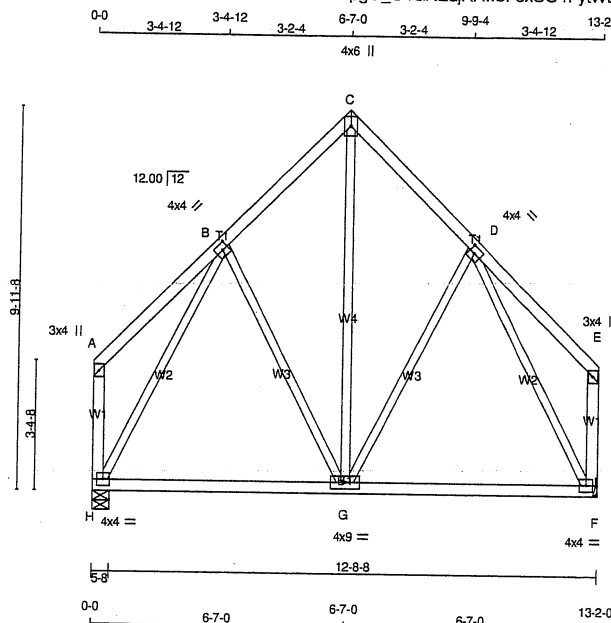
NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only
DWG# T-2126670 *mn*

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Scale = 1:57.2



DRY: SEASONED LUMBER.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MAX. FACTORED		FACTORED			MAX. FACTORED				
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM	TO			FR-TO			
A-B	0 / 23	-84.9	-84.9	0.15 (1)	10.00	G-C	0 / 275	0.06 (1)	
B-C	-367 / 0	-84.9	-84.9	0.11 (1)	6.25	G-D	-70 / 11	0.07 (1)	
C-D	-367 / 0	-84.9	-84.9	0.11 (1)	6.25	G-E	-70 / 11	0.07 (1)	
D-E	0 / 23	-84.9	-84.9	0.15 (1)	10.00	H-B	-592 / 0	0.56 (1)	
H-A	-109 / 0	0.0	0.0	0.02 (1)	7.81	D-F	-592 / 0	0.56 (1)	
F-E	-109 / 0	0.0	0.0	0.02 (1)	7.81				
H-G	0 / 275	-18.5	-18.5	0.25 (4)	10.00				
G-F	0 / 275	-18.5	-18.5	0.25 (4)	10.00				

JSI GRIP= 0.62 (B) (INPUT = 0.90)
JSI METAL= 0.20 (B) (INPUT = 1.00)

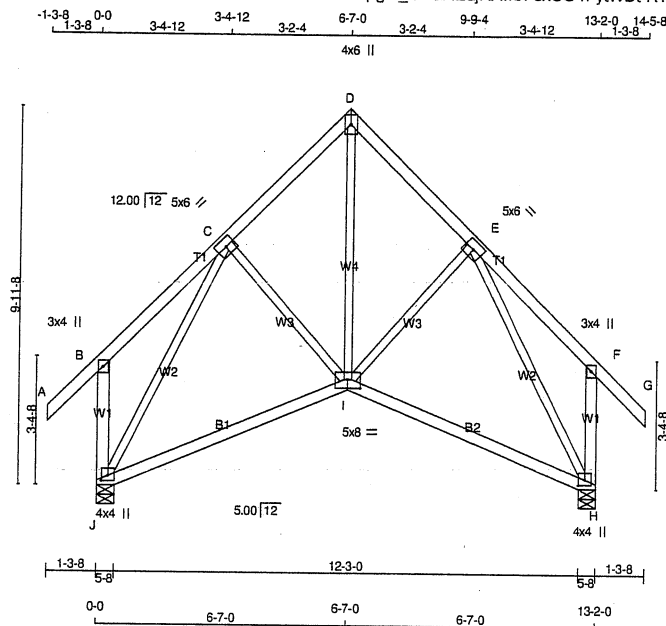


Structural component only
DWG# T-2126671

JOB NAME 420542	TRUSS NAME T29S	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 74 = 147 lb

LUMBER

N.L.G.A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - G	2x4	DRY No.2	SPF
J - B	2x4	DRY No.2	SPF
H - F	2x4	DRY No.2	SPF
J - I	2x4	DRY No.2	SPF
I - H	2x4	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTW+p	MT20	4.0	6.0		
E	TMWW-t	MT20	5.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1+p	MT20	4.0	4.0	1.75	2.00
I	BBWWVW+p	MT20	5.0	8.0	2.75	4.00
J	BMVW1+p	MT20	4.0	4.0	1.75	2.00

NOTES:

(1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
J	799	0	799	0
H	799	0	799	0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. SNOW	MAX./MIN. LIVE	MAX./MIN. PERM. LIVE	WIND	DEAD	SOIL
J	565	372 / 0	0 / 0	0 / 0	0 / 0	193 / 0	0 / 0
H	565	372 / 0	0 / 0	0 / 0	0 / 0	193 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH
FR-TO		FROM TO		FR-TO		FROM TO	
A-B	0 / 42	-84.9 -84.9	0.12 (1)	10.00	I-D	0 / 420	0.09 (1)
B-C	0 / 22	-84.9 -84.9	0.14 (1)	10.00	I-E	-16 / 22	0.01 (4)
C-D	-472 / 0	-84.9 -84.9	0.11 (1)	6.25	C-I	-16 / 22	0.01 (4)
D-E	-472 / 0	-84.9 -84.9	0.11 (1)	6.25	J-C	-726 / 0	0.69 (1)
E-F	0 / 22	-84.9 -84.9	0.14 (1)	10.00	E-H	-726 / 0	0.69 (1)
F-G	0 / 42	-84.9 -84.9	0.12 (1)	10.00			
J-B	-227 / 0	0.0 0.0	0.05 (1)	7.81			
H-F	-227 / 0	0.0 0.0	0.05 (1)	7.81			
J-I	0 / 356	-18.5 -18.5	0.26 (4)	10.00			
I-H	0 / 356	-18.5 -18.5	0.26 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.44")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.01")
ALLOWABLE DEFL.(TL) = $L/360$ (0.44")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.11")

CSI: TC=0.14/1.00 (B-C:1), BC=0.26/1.00 (I-J:4), WB=0.69/1.00 (C-J:1), SSI=0.09/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

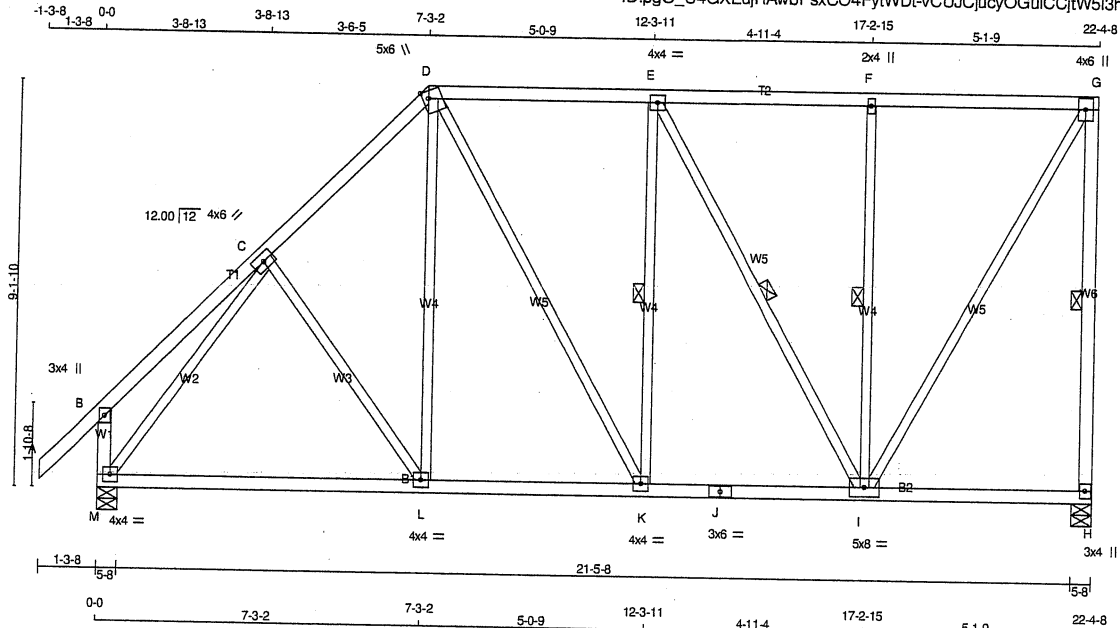
JSI GRIP= 0.65 (H) (INPUT = 0.90)
JSI METAL= 0.30 (H) (INPUT = 1.00)



Structural component only
DWG# T-2126672

JOB NAME 420542	TRUSS NAME T30	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMVW-t	MT20	4.0	4.0		
F	TMVW-w	MT20	2.0	4.0		
G	TMVW+p	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMVW-w-t	MT20	5.0	8.0		
J	BS-t	MT20	3.0	6.0		
K	BMVW-t	MT20	4.0	4.0		
L	BMVW-t	MT20	4.0	4.0		
M	BMVW1-t	MT20	4.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	DOWN	IN-SX	IN-SX
H	1156	0	5-8	5-8
M	1275	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
H	821	521 / 0	0 / 0	0 / 0	300 / 0	0 / 0
M	903	586 / 0	0 / 0	0 / 0	317 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.13 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H, E-K, F-I, E-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC2)	MAX. FACTORED VERT. LOAD (LC3)	MAX. FACTORED VERT. LOAD (LC4)	MAX. FACTORED VERT. LOAD (LC5)	MAX. FACTORED VERT. LOAD (LC6)	MAX. FACTORED VERT. LOAD (LC7)	MAX. FACTORED VERT. LOAD (LC8)	MAX. FACTORED VERT. LOAD (LC9)	MAX. FACTORED VERT. LOAD (LC10)
FR-TO													
A-B	0 / 42												
B-C	0 / 25												
C-D	-950 / 0												
D-E	-749 / 0												
E-F	-545 / 0												
F-G	-545 / 0												
H-G	-1117 / 0												
M-B	-235 / 0												
M-L	0 / 690												
L-K	0 / 654												
K-J	0 / 750												
J-I	0 / 750												
I-H	0 / 0												

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
DL	=	6.0	PSF	
BOT CH.	LL	=	0.0	PSF
DL	=	7.4	PSF	
TOTAL LOAD	=	36.7	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.75")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.75")
CALCULATED VERT. DEFL.(TL) = L/999 (0.10")

CSI: TC=0.43/1.00 (G-H:1), BC=0.28/1.00 (K-L:4), WB=0.82/1.00 (C-M:1), SSI=0.20/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

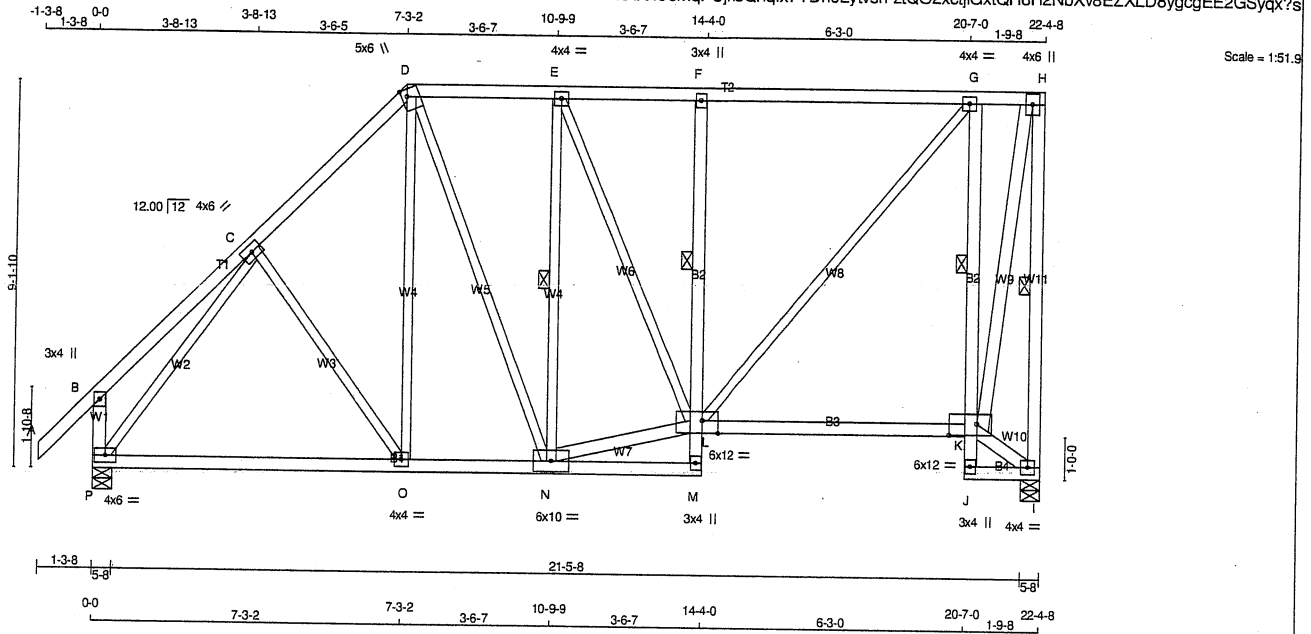
JSI GRIP= 0.88 (M) (INPUT = 0.90)
JSI METAL= 0.27 (J) (INPUT = 1.00)



Structural component only
DWG# T-2126673

JOB NAME 420547	TRUSS NAME T30S	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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TOTAL WEIGHT = 148 lb [M]

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - H	2x4	DRY	No.2	SPF	
I - H	2x4	DRY	No.2	SPF	
P - B	2x4	DRY	No.2	SPF	
P - M	2x4	DRY	No.2	SPF	
M - F	2x4	DRY	No.2	SPF	
L - K	2x4	DRY	No.2	SPF	
J - G	2x4	DRY	No.2	SPF	
J - I	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	
N - L	2x4	DRY	No.2	SPF	
K - I	2x4	DRY	No.2	SPF	
K - H	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMVW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	TMVW-t	MT20	4.0	4.0		
H	TMVW+p	MT20	4.0	6.0		
I	BMVW-t	MT20	4.0	4.0		
J	BMV+p	MT20	3.0	4.0		
K	BMVW-t	MT20	6.0	12.0	3.25	8.00
L	BMVW-t	MT20	6.0	12.0	Edge	4.50
M	BMV+p	MT20	3.0	4.0		
N	BMVW-t	MT20	6.0	10.0		
O	BMVW-t	MT20	4.0	4.0		
P	BMVW-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
I	1156	0	1156	0	5-8	5-8
P	1275	0	1275	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
I	821	521 / 0	0 / 0	0 / 0	0 / 0	300 / 0	0 / 0
P	903	586 / 0	0 / 0	0 / 0	0 / 0	317 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, P

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.13 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 5.89 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF H-I, F-L, G-K, E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		MAX. UNBRACED LENGTH	MEMB.	WEBS	
	FORCE (LBS)	VERT. LOAD (PLF)	LC1 (PLF)	LC2 (PLF)			FORCE (LBS)	MAX. FACTORED (LBS)
FR-TO						FR-TO		
A-B	0 / 42	-84.9	-84.9	0.12 (1)	10.00	C-O	-65 / 17	0.05 (1)
B-C	0 / 25	-84.9	-84.9	0.18 (1)	10.00	O-D	0 / 186	0.05 (4)
C-D	-950 / 0	-84.9	-84.9	0.19 (1)	6.13	D-N	0 / 227	0.05 (1)
D-E	-737 / 0	-84.9	-84.9	0.15 (1)	6.25	N-E	-380 / 0	0.21 (1)
E-F	-793 / 0	-84.9	-84.9	0.33 (1)	6.25	N-L	0 / 758	0.12 (1)
F-G	-795 / 0	-84.9	-84.9	0.38 (1)	6.24	E-L	0 / 135	0.03 (1)
G-H	-253 / 0	-84.9	-84.9	0.34 (1)	6.25	L-G	0 / 840	0.19 (1)
H-I	-1123 / 0	0.0	0.0	0.44 (1)	5.99	K-I	-34 / 0	0.00 (1)
P-B	-235 / 0	0.0	0.0	0.03 (1)	7.81	K-H	0 / 1233	0.20 (1)
						P-C	-1201 / 0	0.82 (1)
P-O	0 / 690	-18.5	-18.5	0.27 (4)	10.00			
O-N	0 / 654	-18.5	-18.5	0.27 (4)	10.00			
N-M	0 / 12	-18.5	-18.5	0.06 (4)	10.00			
M-L	0 / 34	0.0	0.0	0.02 (1)	10.00			
L-F	-469 / 0	0.0	0.0	0.04 (1)	6.25			
L-K	0 / 257	-18.5	-18.5	0.25 (4)	10.00			
J-K	0 / 17	0.0	0.0	0.04 (1)	10.00			
K-G	-1148 / 0	0.0	0.0	0.18 (1)	5.89			
J-I	0 / 29	-18.5	-18.5	0.02 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.75")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.75")
CALCULATED VERT. DEFL.(TL) = L/999 (0.13")

CSI: TC=0.44/1.00 (H-I:1), BC=0.27/1.00 (N-O:4), WB=0.82/1.00 (C-P:1), SSI=0.22/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

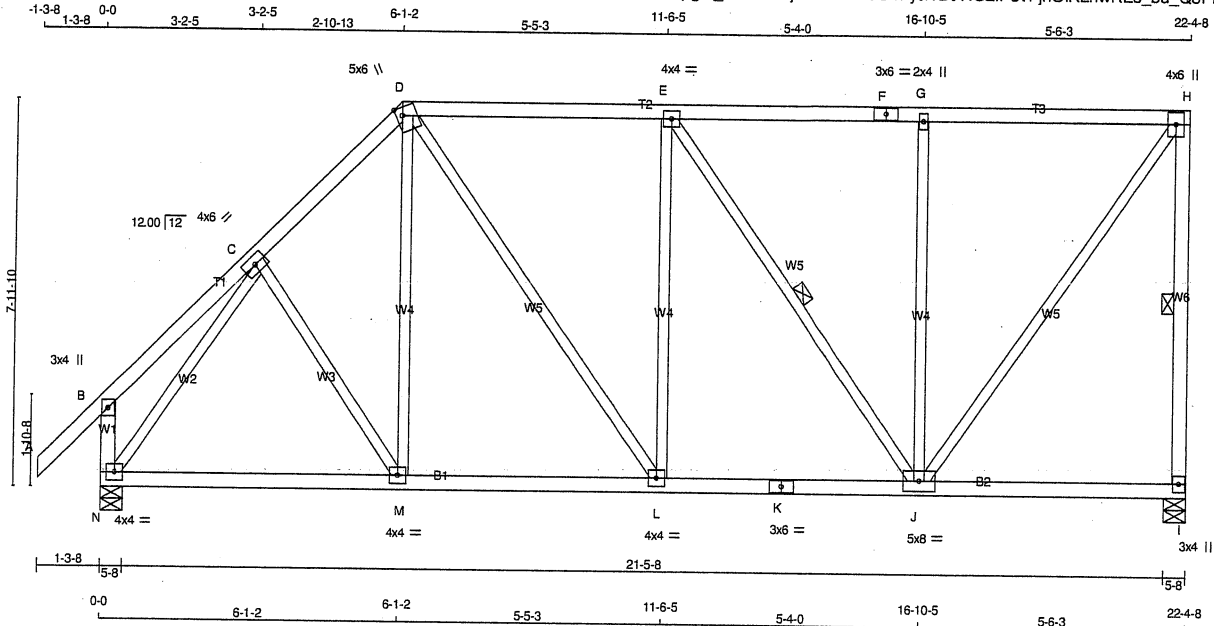
JSI GRIP= 0.87 (G) (INPUT = 0.90)
JSI METAL= 0.34 (K) (INPUT = 1.00)



Structural component only
DWG# T-2126685

JOB NAME 420542	TRUSS NAME T31	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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TOTAL WEIGHT = 115 lb [M]

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY No.2	SPF
D - F	2x4	DRY No.2	SPF
F - H	2x4	DRY No.2	SPF
I - H	2x4	DRY No.2	SPF
N - B	2x4	DRY No.2	SPF
N - K	2x4	DRY No.2	SPF
K - I	2x4	DRY No.2	SPF
ALL WEBS	2x3	DRY No.2	SPF
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMVW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMV+w	MT20	2.0	4.0		
H	TMVW+p	MT20	4.0	6.0		
I	BMV1+p	MT20	3.0	4.0		
J	BMVWW-t	MT20	5.0	8.0		
K	BS-t	MT20	3.0	6.0		
L	BMVW-t	MT20	4.0	4.0		
M	BMVW-t	MT20	4.0	4.0		
N	BMVW-t	MT20	4.0	4.0		

NOTES:

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQ'D
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
I	1156	0	1156	0	5-8
N	1275	0	1275	0	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
I	821	521 / 0	0 / 0	0 / 0	300 / 0	0 / 0
N	903	586 / 0	0 / 0	0 / 0	317 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.04 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF H-I, E-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	
FR-TO				FR-TO			
A-B	0 / 42	-84.9 -84.9	0.12 (1)	10.00	M-D	0 / 114	0.04 (4)
B-C	0 / 20	-84.9 -84.9	0.12 (1)	10.00	L-E	-189 / 10	0.23 (1)
C-D	-983 / 0	-84.9 -84.9	0.13 (1)	6.13	J-G	-504 / 0	0.61 (1)
D-E	-877 / 0	-84.9 -84.9	0.36 (1)	6.04	D-L	0 / 341	0.08 (1)
E-F	-684 / 0	-84.9 -84.9	0.35 (1)	6.25	E-J	-374 / 0	0.23 (1)
F-G	-684 / 0	-84.9 -84.9	0.35 (1)	6.25	J-H	0 / 1138	0.26 (1)
G-H	-684 / 0	-84.9 -84.9	0.35 (1)	6.25	C-M	0 / 38	0.01 (4)
I-H	-1114 / 0	0.0 0.0	0.32 (1)	6.01	N-C	-1211 / 0	0.60 (1)
N-B	-220 / 0	0.0 0.0	0.02 (1)	7.81			
N-M	0 / 672	-18.5 -18.5	0.22 (4)	10.00			
M-L	0 / 680	-18.5 -18.5	0.22 (4)	10.00			
L-K	0 / 877	-18.5 -18.5	0.21 (4)	10.00			
K-J	0 / 877	-18.5 -18.5	0.21 (4)	10.00			
J-I	0 / 0	-18.5 -18.5	0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
DL = 6.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.4	PSF	
TOTAL LOAD = 36.7	PSF	

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.75")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.75")
CALCULATED VERT. DEFL.(TL) = L/999 (0.06")

CSI: TC=0.36/1.00 (D-E:1), BC=0.22/1.00 (L-M:4), WB=0.61/1.00 (G-J:1), SSI=0.22/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	PLATE GRIP(WET)	SECTION (PL)
MAX	MIN	MAX
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

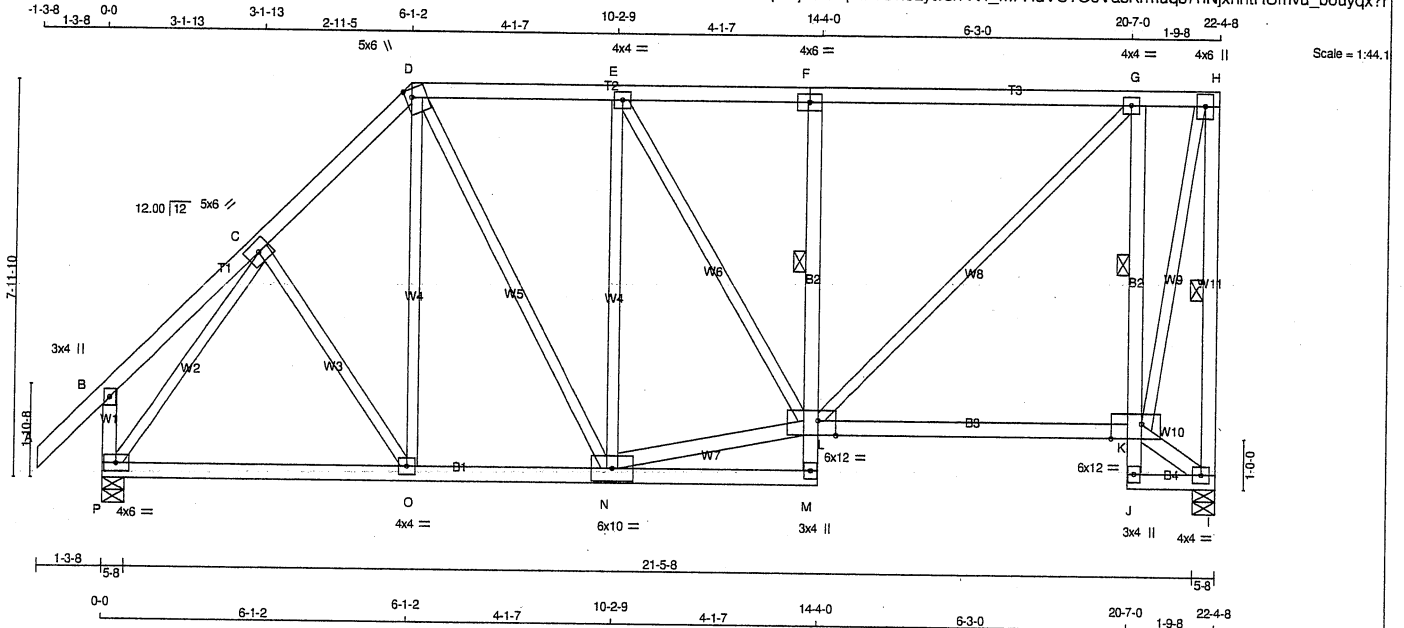
JSI GRIP= 0.88 (N) (INPUT = 0.90)
JSI METAL= 0.31 (K) (INPUT = 1.00)



Structural component only
DWG# T-2126674

JOB NAME 420547	TRUSS NAME T31S	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Aug 5 11:04:40 2021 Page 1
ID: X496mqPCjhJQnqfx7YDh0Lytvsh-R4_mFhdVU?OoVasKrmuq37hNjxhtRUMvu_bouyqx?r



LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
C	TMVW-t	MT20	5.0	6.0	
D	TTWW+m	MT20	5.0	6.0	2.00 1.50
E	TMVW-t	MT20	4.0	4.0	
F	TSV-i	MT20	4.0	6.0	
G	TMVW-t	MT20	4.0	4.0	
H	TMVW+p	MT20	4.0	6.0	
I	BMVW1-t	MT20	4.0	4.0	
J	BMV+p	MT20	3.0	4.0	
K	BMVW1-t	MT20	6.0	12.0	Edge 7.50
L	BMVW1-t	MT20	6.0	12.0	Edge 4.50
M	BMV+p	MT20	3.0	4.0	
N	BMVW1-t	MT20	6.0	10.0	
O	BMVW1-t	MT20	4.0	4.0	
P	BMVW1-t	MT20	4.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
I	1156	0	1156	0	5-8
P	1275	0	1275	0	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
I	821	521 / 0	0 / 0	0 / 0	0 / 0	300 / 0	0 / 0
P	903	586 / 0	0 / 0	0 / 0	0 / 0	317 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, P

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.68 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 5.76 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF H-I, F-L, G-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 42	-84.9 -84.9	0.12 (1)	C-O	0 / 38	0.01 (4)	
B-C	0 / 20	-84.9 -84.9	0.12 (1)	D-N	0 / 102	0.03 (4)	
C-D	-982 / 0	-84.9 -84.9	0.13 (1)	E-N	0 / 381	0.09 (1)	
D-E	-857 / 0	-84.9 -84.9	0.24 (1)	F-N	-477 / 0	0.58 (1)	
E-F	-898 / 0	-84.9 -84.9	0.24 (1)	G-L	0 / 871	0.14 (1)	
F-G	-895 / 0	-84.9 -84.9	0.54 (1)	H-L	0 / 73	0.02 (1)	
G-H	-312 / 0	-84.9 -84.9	0.48 (1)	I-L	0 / 826	0.19 (1)	
H-I	-1119 / 0	0.0 0.0	0.32 (1)	J-K	-42 / 0	0.00 (1)	
P-B	-218 / 0	0.0 0.0	0.02 (1)	K-H	0 / 1303	0.29 (1)	
				P-C	-1211 / 0	0.59 (1)	
P-O	0 / 670	-18.5 -18.5	0.20 (4)				
O-N	0 / 679	-18.5 -18.5	0.21 (4)				
N-M	0 / 16	-18.5 -18.5	0.07 (4)				
M-L	0 / 36	0.0 0.0	0.03 (1)				
L-F	-351 / 0	0.0 0.0	0.03 (1)				
L-K	0 / 317	-18.5 -18.5	0.26 (4)				
J-K	0 / 17	0.0 0.0	0.05 (1)				
K-G	-1211 / 0	0.0 0.0	0.14 (1)				
J-I	0 / 36	-18.5 -18.5	0.02 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.75")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.75")
CALCULATED VERT. DEFL.(TL) = L/999 (0.14")

CSI: TC=0.54/1.00 (F-G:1), BC=0.26/1.00 (K-L:4), WB=0.59/1.00 (C-P:1), SS=0.26/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

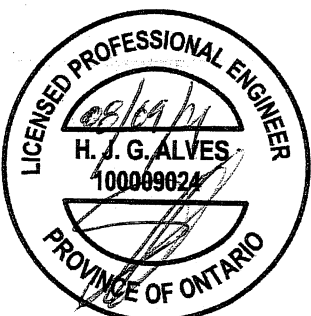
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	618	354	1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.85 (H) (INPUT = 0.90)
JSI METAL = 0.39 (K) (INPUT = 1.00)

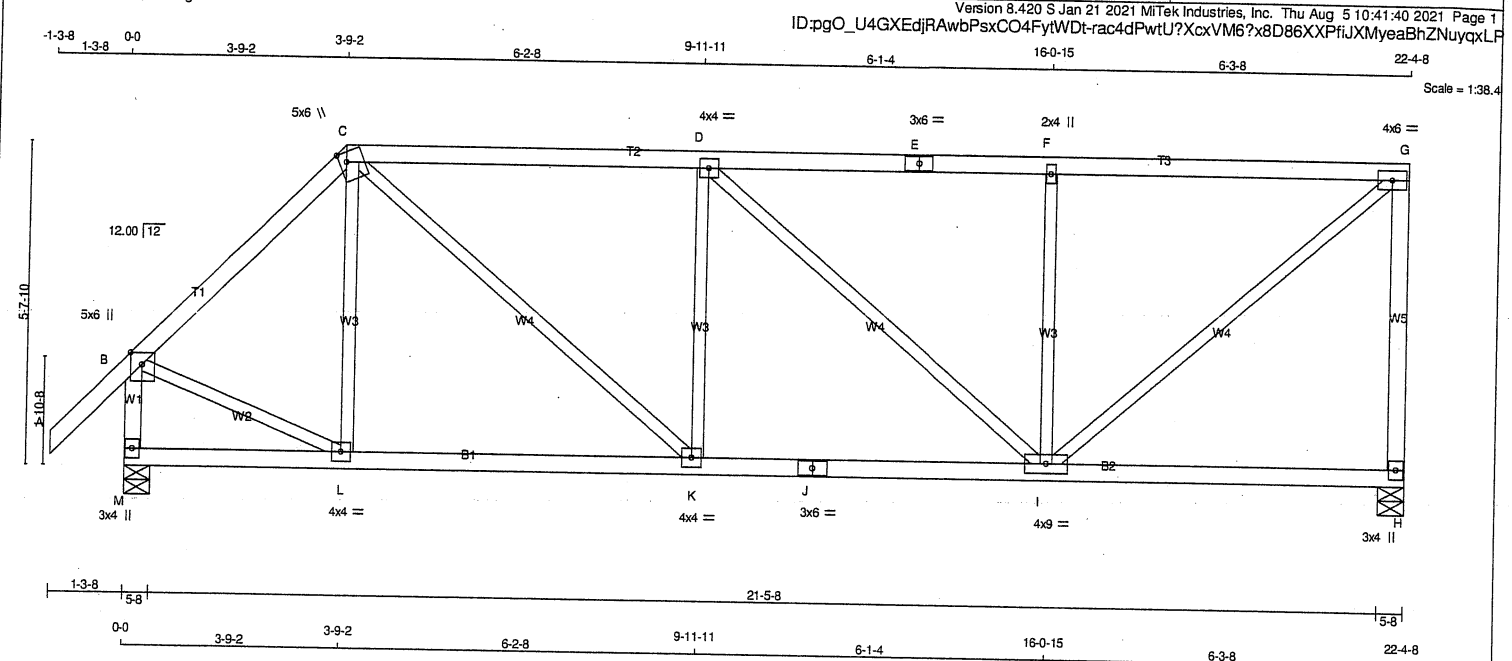


Structural component only
DWG# T-2126686

Structural component only
DWG# T-2126675

Structural component only
DWG# T-2126687

JOB NAME 420542	TRUSS NAME T33	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	



LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER DESCR.
A - C 2x4 DRY No.2 SPF
C - E 2x4 DRY No.2 SPF
E - G 2x4 DRY No.2 SPF
H - G 2x4 DRY No.2 SPF
M - B 2x4 DRY No.2 SPF
M - J 2x4 DRY No.2 SPF
J - H 2x4 DRY No.2 SPF
ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
BEARINGS				SPECIFIED LOADS:			
FACTORED	MAXIMUM FACTORED	INPUT	REQD	TOP CH. LL	LL	23.3	PSF
GROSS REACTION	GROSS REACTION	BRG	BRG	DL	DL	6.0	PSF
VERT	DOWN	UP	IN-SX	BOT CH. LL	LL	0.0	PSF
H 1156	0	1156	0	DL	DL	7.4	PSF
M 1275	0	1275	0	TOTAL LOAD		36.7	PSF

UNFACTORED REACTIONS							
1ST LCASE	MAX.	MIN.	COMPONENT	REACTIONS			
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
H	821	521 / 0	0 / 0	0 / 0	0 / 0	300 / 0	0 / 0
M	903	586 / 0	0 / 0	0 / 0	0 / 0	317 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.07 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 42	-84.9 -84.9	0.12 (1)	L-C	-168 / 15	0.08 (1)	
B-C	-986 / 0	-84.9 -84.9	0.23 (1)	B-L	0 / 746	0.17 (1)	
C-D	-1263 / 0	-84.9 -84.9	0.50 (1)	C-K	0 / 753	0.17 (1)	
D-E	-1050 / 0	-84.9 -84.9	0.49 (1)	K-D	-380 / 0	0.18 (1)	
E-F	-1050 / 0	-84.9 -84.9	0.49 (1)	D-I	-283 / 0	0.39 (1)	
F-G	-1050 / 0	-84.9 -84.9	0.49 (1)	I-F	-578 / 0	0.29 (1)	
H-G	-1108 / 0	0.0 0.0	0.64 (1)	I-G	0 / 1378	0.31 (1)	
M-B	-1252 / 0	0.0 0.0	0.14 (1)				
M-L	0 / 0	-18.5 -18.5	0.11 (4)				
L-K	0 / 691	-18.5 -18.5	0.19 (4)				
K-J	0 / 1263	-18.5 -18.5	0.29 (1)				
J-I	0 / 1263	-18.5 -18.5	0.29 (1)				
I-H	0 / 0	-18.5 -18.5	0.17 (4)				

NOTES: (1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2126676

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.75")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.75")
CALCULATED VERT. DEFL.(TL) = L/999 (0.09")

CSI: TC=0.64/1.00 (G-H:1) , BC=0.29/1.00 (I-K:1) , WB=0.39/1.00 (D-I:1) , SS=0.25/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

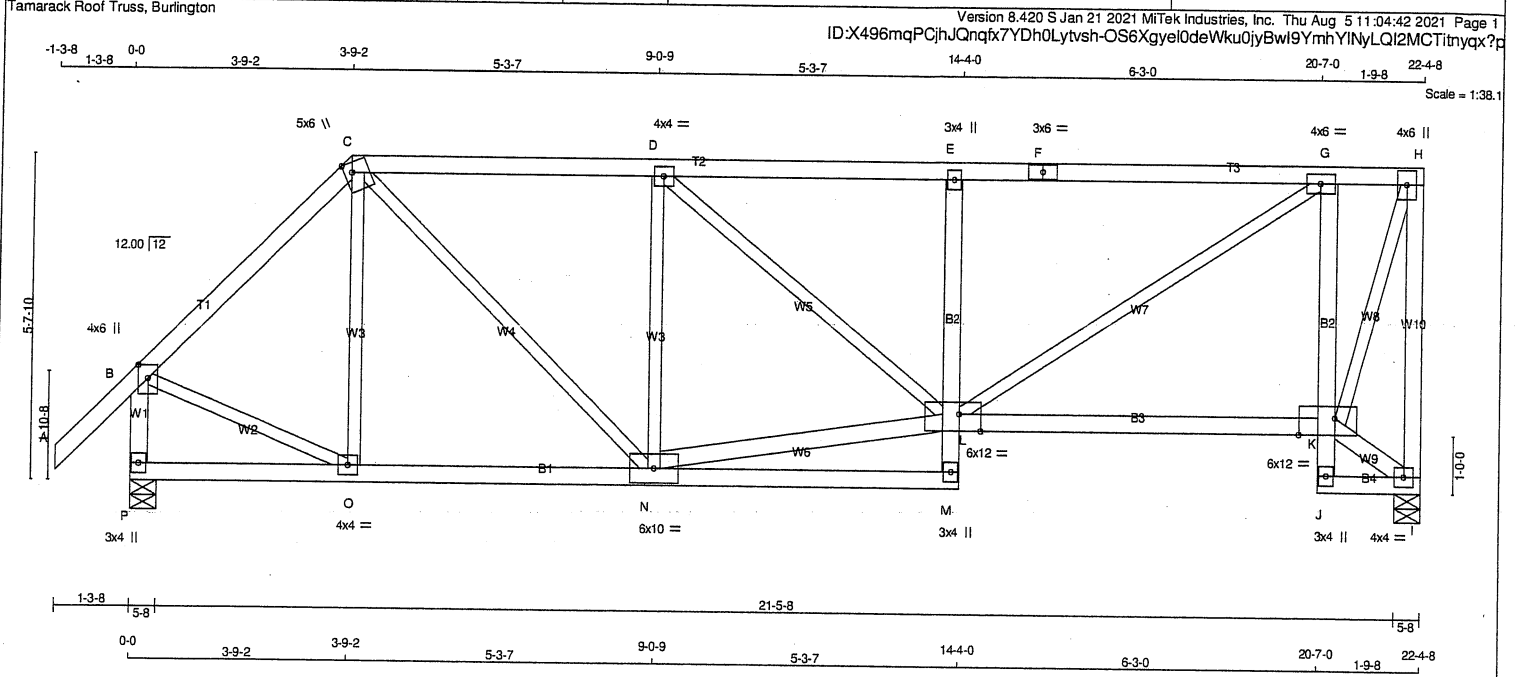
PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.84 (G) (INPUT = 0.90)
JSI METAL = 0.41 (J) (INPUT = 1.00)

JOB NAME 420547	TRUSS NAME T33S	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
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LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - E	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMVW-t	MT20	4.0	4.0		
E	TMV+p	MT20	3.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMVW-t	MT20	4.0	6.0		
H	TMVW+p	MT20	4.0	6.0		
I	BMVW-t	MT20	4.0	4.0		
J	BMV+p	MT20	3.0	4.0		
K	BMVWV-t	MT20	6.0	12.0	Edge	7.50
L	BMVWVW-t	MT20	6.0	12.0	Edge	4.50
M	BMV+p	MT20	3.0	4.0		
N	BMVWVW-t	MT20	6.0	10.0		
O	BMVW-t	MT20	4.0	4.0		
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
I	1156	0	1156	0	5-8
P	1275	0	1275	0	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. SNOW	MAX/MIN. LIVE	MAX/MIN. PERM. LIVE	MAX/MIN. WIND	MAX/MIN. DEAD	MAX/MIN. SOIL
I	821	521 / 0	0 / 0	0 / 0	0 / 0	300 / 0	0 / 0
P	903	586 / 0	0 / 0	0 / 0	0 / 0	317 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, P

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.94 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.33 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 42	O-C	-183 / 0
B-C	-982 / 0	C-N	0 / 744
C-D	-1209 / 0	N-D	-651 / 0
D-E	-1428 / 0	E-L	0 / 1201
E-F	-1437 / 0	L-L	0 / 275
F-G	-1437 / 0	L-G	0 / 1166
G-H	-457 / 0	K-I	-42 / 0
I-H	-1119 / 0	K-H	0 / 1290
P-B	-1248 / 0	B-O	0 / 743
P-O	0 / 0		
O-N	0 / 888		
N-M	0 / 39		
M-L	0 / 43		
L-E	-497 / 0		
L-K	0 / 465		
J-K	0 / 17		
K-G	-1152 / 0		
J-I	0 / 36		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.75")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.75")
CALCULATED VERT. DEFL.(TL) = L/999 (0.15")

CSI: TC=0.65/1.00 (H-I:1), BC=0.27/1.00 (K-L:4), WB=0.31/1.00 (D-N:1), SSI=0.22/1.00 (E-G:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (B) (INPUT = 0.90)
JSI METAL= 0.44 (B) (INPUT = 1.00)

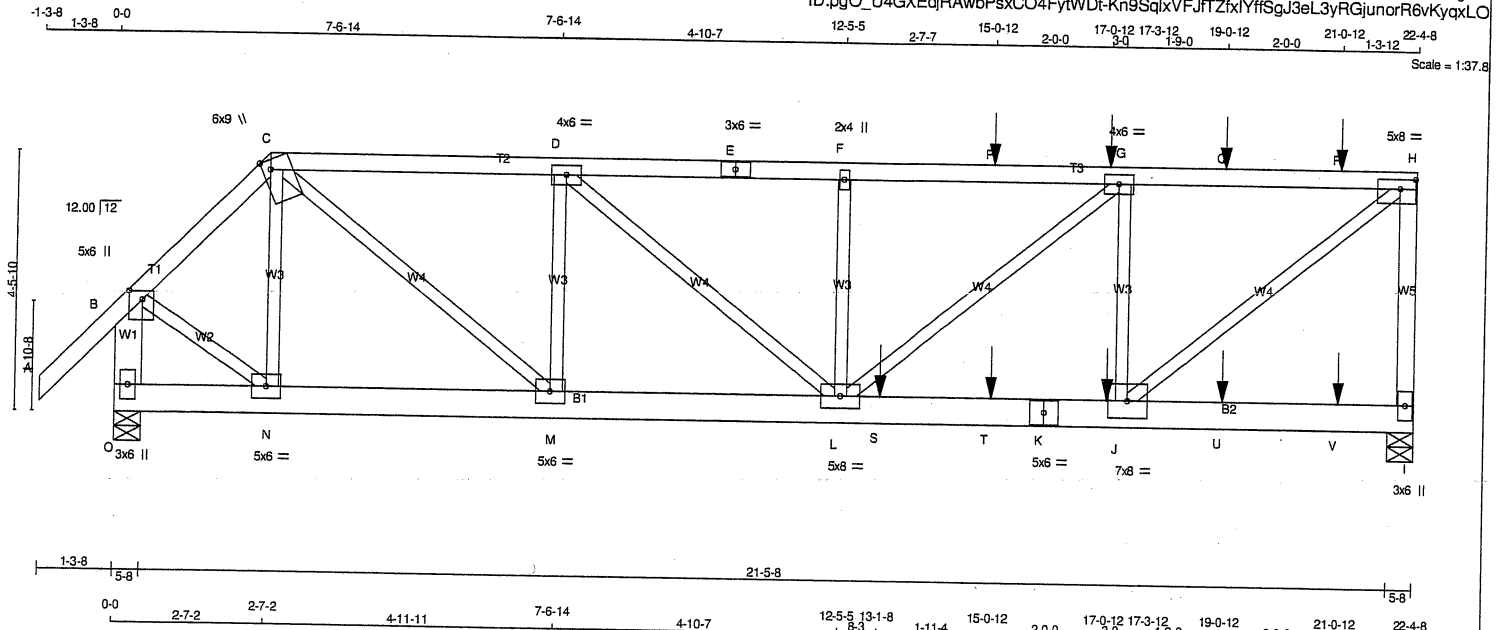


Structural component only
DWG# T-2126688

JOB NAME 420542	TRUSS NAME T34	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER DESCR.
A - C 2x4 DRY No.2 SPF
C - E 2x4 DRY No.2 SPF
E - H 2x4 DRY No.2 SPF
I - H 2x4 DRY No.2 SPF
O - B 2x6 DRY No.2 SPF
O - K 2x6 DRY No.2 SPF
K - I 2x6 DRY No.2 SPF
ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - E	2x4	DRY No.2	SPF
E - H	2x4	DRY No.2	SPF
I - H	2x4	DRY No.2	SPF
O - B	2x6	DRY No.2	SPF
O - K	2x6	DRY No.2	SPF
K - I	2x6	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	1.75	2.75
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMWW-t	MT20	4.0	6.0		
E	TS-t	MT20	3.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TMWW-t	MT20	4.0	6.0		
H	TMWW-t	MT20	5.0	8.0	2.00	3.25
I	BMV1+p	MT20	3.0	6.0		
J	BMWW-t	MT20	7.0	8.0		
K	BS-t	MT20	5.0	6.0		
L	BMWW-t	MT20	5.0	8.0		
M	BMWW-t	MT20	5.0	6.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
I	2370	0	2370	0	0	5-8	5-8
O	1858	0	1858	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN	COMPONENT REACTIONS
I	COMBINED	SNOW	LIVE
I	1679	1085 / 0	0 / 0
O	1314	861 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.69 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	FORCE (LBS)	VERT. LOAD LC1 (PLF)	MAX. UNBRACED LENGTH (LC) (LBS)	MEMB.	FORCE (LBS)
FR-TO		FROM	TO	FR-TO	
A-B	0 / 42	-84.9	-84.9 0.13 (1)	N-C	-432 / 0
B-C	-1506 / 0	-84.9	-84.9 0.14 (1)	B-N	0 / 1200
C-D	-2526 / 0	-84.9	-84.9 0.46 (1)	C-M	0 / 1910
D-E	-3411 / 0	-84.9	-84.9 0.57 (1)	M-D	-1203 / 0
E-F	-3411 / 0	-84.9	-84.9 0.57 (1)	D-L	0 / 1155
F-P	-3411 / 0	-84.9	-84.9 0.88 (1)	L-F	-414 / 0
P-G	-3411 / 0	-84.9	-84.9 0.88 (1)	G-L	0 / 1198
G-Q	-2494 / 0	-84.9	-84.9 0.77 (1)	J-G	-1680 / 0
Q-R	-2494 / 0	-84.9	-84.9 0.77 (1)	J-H	0 / 3208
R-H	-2494 / 0	-84.9	-84.9 0.77 (1)		
I-H	-2316 / 0	0.0	0.0 0.70 (1)		
O-B	-1864 / 0	0.0	0.0 0.14 (1)		

JT	LOC	LC1	MAX	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	17-0-12	-101	-101	---	FRONT	VERT	TOTAL	---	C1
J	17-0-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
P	15-0-12	-101	-101	---	FRONT	VERT	TOTAL	---	C1
Q	19-0-12	-101	-101	---	FRONT	VERT	TOTAL	---	C1
R	21-0-12	-101	-101	---	FRONT	VERT	TOTAL	---	C1
S	13-1-8	-763	-763	---	FRONT	VERT	TOTAL	---	C1
T	15-0-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
U	19-0-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
V	21-0-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC	LC1	MAX	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	17-0-12	-101	-101	---	FRONT	VERT	TOTAL	---	C1
J	17-0-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
P	15-0-12	-101	-101	---	FRONT	VERT	TOTAL	---	C1
Q	19-0-12	-101	-101	---	FRONT	VERT	TOTAL	---	C1
R	21-0-12	-101	-101	---	FRONT	VERT	TOTAL	---	C1
S	13-1-8	-763	-763	---	FRONT	VERT	TOTAL	---	C1
T	15-0-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
U	19-0-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
V	21-0-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.75")
CALCULATED VERT. DEFL.(LL) = L/999 (0.12")
ALLOWABLE DEFL.(TL) = L/360 (0.75")
CALCULATED VERT. DEFL.(TL) = L/999 (0.23")

CSI: TC=0.88/1.00 (F-G:1), BC=0.68/1.00 (J-L:1), WB=0.79/1.00 (H-J:1), SS=0.66/1.00 (J-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

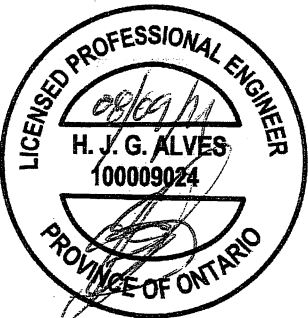
AUTOSOLVE RIGHT HEEL ONLY
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

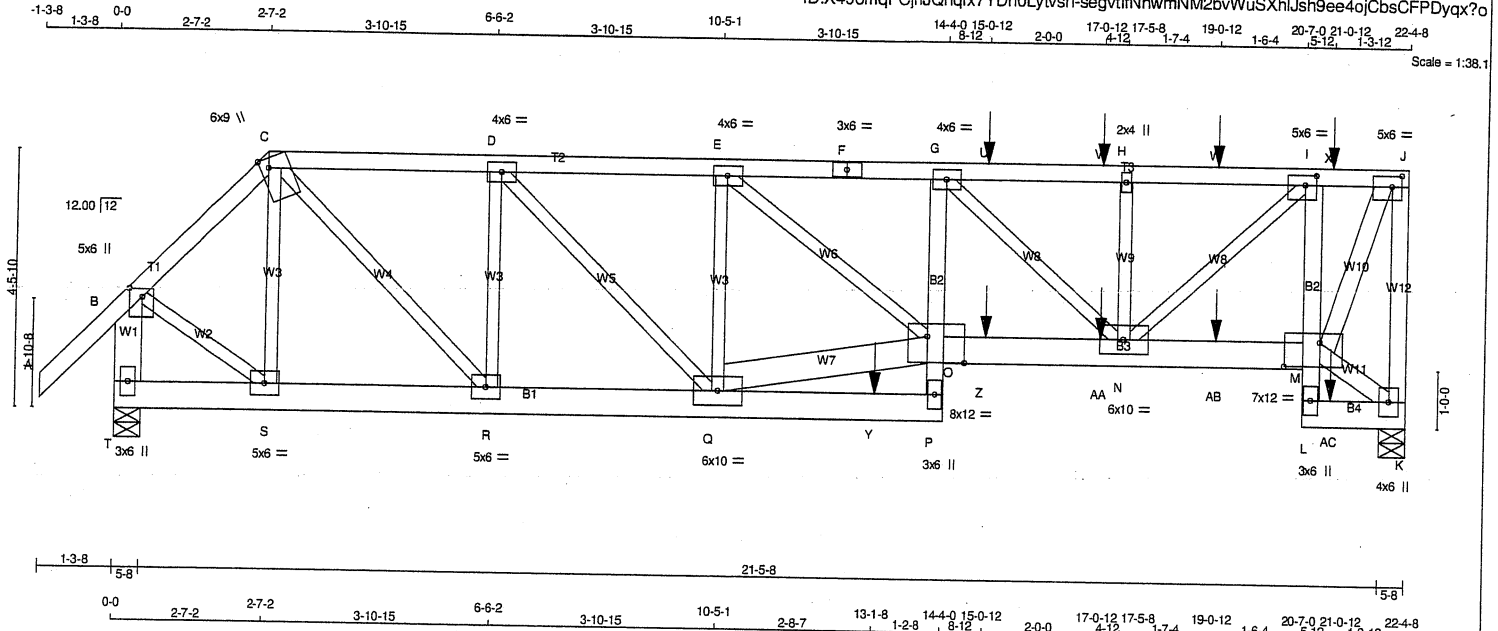
JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.53 (K) (INPUT = 1.00)



Structural component only
DWG# T-2126677

JOB NAME 420547	TRUSS NAME T34S	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
F - J	2x4	DRY	No.2
K - J	2x4	DRY	No.2
T - B	2x6	DRY	No.2
T - P	2x6	DRY	No.2
P - G	2x4	DRY	No.2
O - M	2x6	DRY	No.2
L - I	2x4	DRY	No.2
L - K	2x6	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
Q - O	2x6	DRY	No.2
M - K	2x4	DRY	No.2
M - J	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	1.75	2.75
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMVW-t	MT20	4.0	6.0		
E	TMVW-t	MT20	4.0	6.0		
F	TS-t	MT20	3.0	6.0		
G	TMVW-t	MT20	4.0	6.0		
H	TMVW-w	MT20	2.0	4.0		
I	TMVW-t	MT20	5.0	6.0	2.00	2.25
J	TMVW-t	MT20	5.0	6.0	2.25	2.00
K	BMVW1+p	MT20	4.0	6.0		
L	BMV+p	MT20	3.0	6.0		
M	BVMWV+m	MT20	7.0	12.0	5.00	7.50
N	BVMWV-t	MT20	6.0	10.0		
O	BVMWV-t	MT20	8.0	12.0	5.25	7.75
P	BMV+p	MT20	3.0	6.0		
Q	BMVWV-t	MT20	6.0	10.0		
R	BMVW-t	MT20	5.0	6.0		
S	BMVW-t	MT20	5.0	6.0		
T	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

Structural component only
 DWG# T-2126689

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	DOWN	UP	IN-SX
K	2342	0	2342	0
T	1850	0	1850	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
K	COMBINED	SNOW LIVE	PERM. LIVE	0/0	594/0
T	1660	1066/0	0/0	0/0	0/0
	1309	856/0	0/0	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, T

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.10 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 5.61 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX UNBRAC LENGTH (LC)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX UNBRAC LENGTH (LC)
FR-TO	A-B	0/42	-84.9	-84.9 0.13 (1)	10.00	S-C	-432/0	0.13 (1)
	B-C	-1495/0	-84.9	-84.9 0.14 (1)	5.20	C-R	0/1713	0.42 (1)
	C-D	-2228/0	-84.9	-84.9 0.28 (1)	4.28	P-D	-1295/0	0.38 (1)
	D-E	-3105/0	-84.9	-84.9 0.36 (1)	3.63	D-Q	0/1283	0.32 (1)
	E-F	-4223/0	-84.9	-84.9 0.41 (1)	3.10	Q-E	-1214/0	0.35 (1)
	F-G	-4223/0	-84.9	-84.9 0.41 (1)	3.10	Q-O	0/3136	0.41 (1)
	G-U	-3033/0	-84.9	-84.9 0.29 (1)	3.74	E-O	0/1415	0.35 (1)
	U-V	-3033/0	-84.9	-84.9 0.29 (1)	3.74	G-N	-1683/0	0.56 (1)
	V-H	-3033/0	-84.9	-84.9 0.29 (1)	3.74	N-H	-423/0	0.09 (1)
	H-W	-3033/0	-84.9	-84.9 0.29 (1)	3.74	N-I	0/2516	0.62 (1)
	W-I	-3033/0	-84.9	-84.9 0.29 (1)	3.74	M-K	-74/0	0.01 (1)
	I-X	-1185/0	-84.9	-84.9 0.14 (1)	5.68	M-J	0/2524	0.45 (1)
	X-J	-1185/0	-84.9	-84.9 0.14 (1)	5.68	B-S	0/1191	0.29 (1)
	K-J	-2281/0	0.0	0.0 0.68 (1)	5.58			
	T-B	-1852/0	0.0	0.0 0.14 (1)	7.32			

T-S	0/0	-18.5	-18.5 0.05 (1)	10.00
S-R	0/1040	-18.5	-18.5 0.20 (1)	10.00
R-Q	0/2228	-18.5	-18.5 0.48 (1)	10.00
Q-Y	0/96	-18.5	-18.5 0.59 (1)	10.00
Y-P	0/96	-18.5	-18.5 0.59 (1)	10.00
P-O	0/751	0.0	0.0 0.28 (1)	10.00
O-G	0/804	0.0	0.0 0.29 (1)	10.00
O-Z	0/4255	-18.5	-18.5 0.63 (1)	10.00
Z-AA	0/4255	-18.5	-18.5 0.63 (1)	10.00
AA-N	0/4255	-18.5	-18.5 0.63 (1)	10.00
N-AB	0/1206	-18.5	-18.5 0.24 (1)	10.00
AB-M	0/1206	-18.5	-18.5 0.24 (1)	10.00
L-M	0/51	0.0	0.0 0.10 (1)	10.00
M-I	-2136/0	0.0	0.0 0.32 (1)	5.61
L-AC	0/63	-18.5	-18.5 0.02 (4)	10.00
AC-K	0/63	-18.5	-18.5 0.02 (4)	10.00

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
U	15-0-12	-55	-55	---	FRONT	VERT	TOTAL	---	C1
V	17-0-12	-55	-55	---	FRONT	VERT	TOTAL	---	C1
W	19-0-12	-55	-55	---	FRONT	VERT	TOTAL	---	C1
X	21-0-12	-101	-101	---	FRONT	VERT	TOTAL	---	C1
Y	13-1-8	-763	-763	---	FRONT	VERT	TOTAL	---	C1
Z	15-0-12	-62	-62	---	FRONT	VERT	TOTAL	---	C1
AA	17-0-12	-61	-61	---	FRONT	VERT	TOTAL	---	C1
AB	19-0-12	-61	-61	---	FRONT	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 23.3 PSF
 DL = 6.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBC 2018, OBC 2012, ABC 2019
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 086-09, CSA 086-14
 - TPIC 2011, TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.75")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.14")
 ALLOWABLE DEFL.(TL)= L/360 (0.75")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.27")

CSI: TC=0.68/1.00 (J-K:1), BC=0.63/1.00 (N-O:1), WB=0.62/1.00 (H-N:1), SS=0.50/1.00 (P-Q:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.00
 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (D) (INPUT = 0.90)
 JSI METAL= 0.63 (M) (INPUT = 1.00)



Structural component only
DWG# T-2126689

JOB NAME 420547	TRUSS NAME T34S	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
AC	21-0-12	-31	-31	---	FRONT	VERT	TOTAL	---	C1

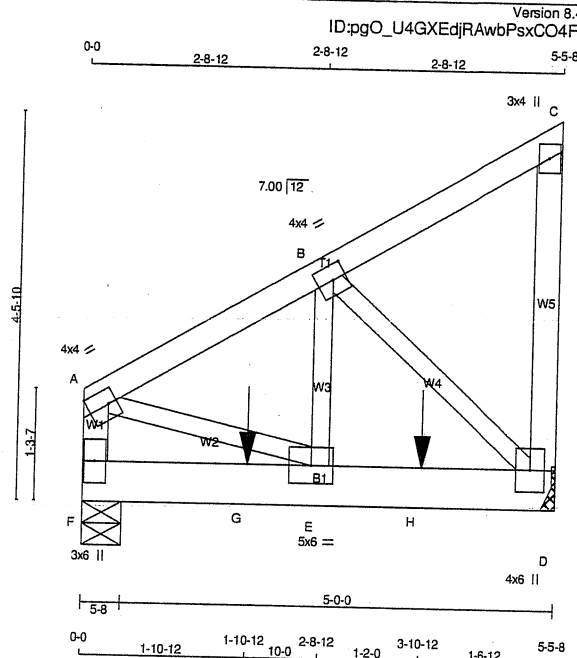
CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2126689 *mn*

JOB NAME 420542	TRUSS NAME T35	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
F - A	2x4	DRY	No.2
A - C	2x4	DRY	No.2
D - C	2x4	DRY	No.2
F - D	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
F-A	12	TOP
A-C	12	TOP
C-D	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F-D	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0	2.00	Edge
B	TMVW-t	MT20	4.0	4.0	2.00	1.75
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMVW-t	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES - (1)

- 1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
VERT	HORZ	DOWN	HORZ	UPLIFT
D	1007	0	1007	0
F	1101	0	1101	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 2-0.

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	711	472 / 0	0 / 0	0 / 0	0 / 0	0 / 0	239 / 0	0 / 0
D	777	517 / 0	0 / 0	0 / 0	0 / 0	0 / 0	260 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
F-A	-846 / 0	0.0	0.0 0.05 (1)	7.81	A-E	0 / 871	0.11 (1)
A-B	-948 / 0	-84.9	-84.9 0.05 (1)	6.25	E-B	0 / 960	0.12 (1)
B-C	-11 / 0	-84.9	-84.9 0.05 (1)	6.25	B-D	-1119 / 0	0.13 (1)
D-C	-94 / 0	0.0	0.0 0.01 (1)	7.81			
F-G	0 / 0	-18.5	-18.5 0.10 (1)	10.00			
G-E	0 / 0	-18.5	-18.5 0.10 (1)	10.00			
E-H	0 / 828	-18.5	-18.5 0.19 (1)	10.00			
H-D	0 / 828	-18.5	-18.5 0.19 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-10-12	-544	-544	---	TOP	VERT	TOTAL	---	C1
H	3-10-12	-544	-544	---	TOP	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 28 = 57 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.05/1.00 (A-B:1), BC=0.19/1.00 (D-E:1), WB=0.13/1.00 (B-D:1), SSI=0.22/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

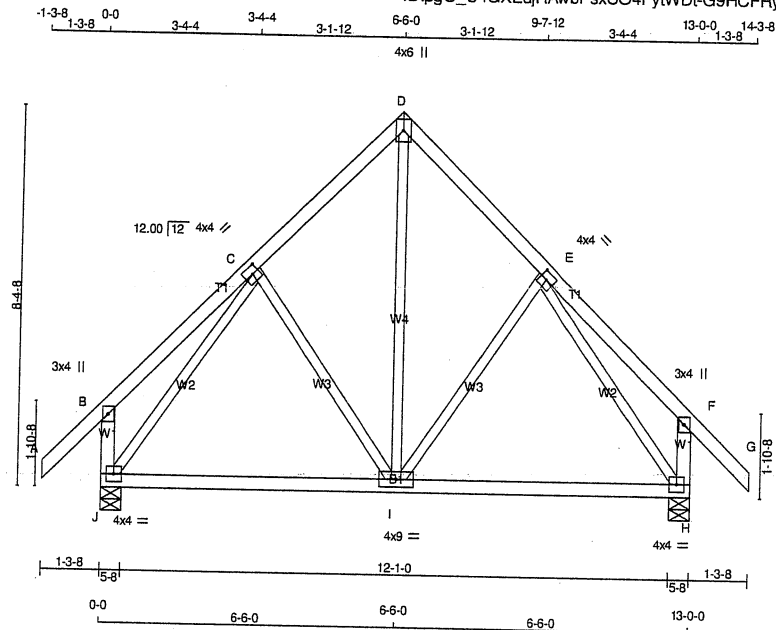
JSI GRIP = 0.75 (A) (INPUT = 0.90)
JSI METAL = 0.19 (D) (INPUT = 1.00)



Structural component only
DWG# T-2126678

JOB NAME 420542	TRUSS NAME T36	QUANTITY 4	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID:pgO_U4GXEdjRAwbPsxCO4FytWDT-G9HCFRylnwvAoz5hg3hwmk99PskdkkQ4G9wDzCyxqLm



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. TOTAL WEIGHT = 4 X 68 = 271 lb [M][F]

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
J - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

SPF

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMW+W-t	MT20	4.0	4.0	2.00	1.75
D	TTW+p	MT20	4.0	6.0		
E	TMW+W-t	MT20	4.0	4.0	2.00	1.75
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMVW1-t	MT20	4.0	9.0		
J	BMVW1-t	MT20	4.0	4.0		

NOTES (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
J	790	0	790	0	0	5-8	5-8	5-8	5-8
H	790	0	790	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
J	559	368 / 0	0 / 0	0 / 0	0 / 0	0 / 0	191 / 0	0 / 0
H	559	368 / 0	0 / 0	0 / 0	0 / 0	0 / 0	191 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED FORCE (LBS)		FACTORED VERT. LOAD LC1 MAX (PLF)		MAX. UNBRACED LENGTH FR-TO		WEBS		MAX. FACTORED FORCE (LBS)		MAX. UNBRACED LENGTH FR-TO	
MEMB.	FR-TO	FROM	TO	FROM	TO	FROM	TO	MEMB.	FR-TO	FROM	TO	FROM	TO
A-B	0 / 42	-84.9	-84.9	0.12 (1)	10.00	I-D	0 / 349	0.08 (1)					
B-C	0 / 23	-84.9	-84.9	0.14 (1)	10.00	I-E	-123 / 0	0.07 (1)					
C-D	-423 / 0	-84.9	-84.9	0.11 (1)	6.25	C-I	-123 / 0	0.07 (1)					
D-E	-423 / 0	-84.9	-84.9	0.11 (1)	6.25	J-C	-630 / 0	0.34 (1)					
E-F	0 / 23	-84.9	-84.9	0.12 (1)	10.00	E-H	-630 / 0	0.34 (1)					
F-G	0 / 42	-84.9	-84.9	0.12 (1)	10.00								
J-B	-222 / 0	0.0	0.0	0.02 (1)	7.81								
H-F	-222 / 0	0.0	0.0	0.02 (1)	7.81								
J-I	0 / 354	-18.5	-18.5	0.25 (4)	10.00								
I-H	0 / 354	-18.5	-18.5	0.25 (4)	10.00								

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.14/1.00 (E-F:1), BC=0.25/1.00 (I-J:4), WB=0.34/1.00 (E-H:1), SSI=0.10/1.00 (H-I:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

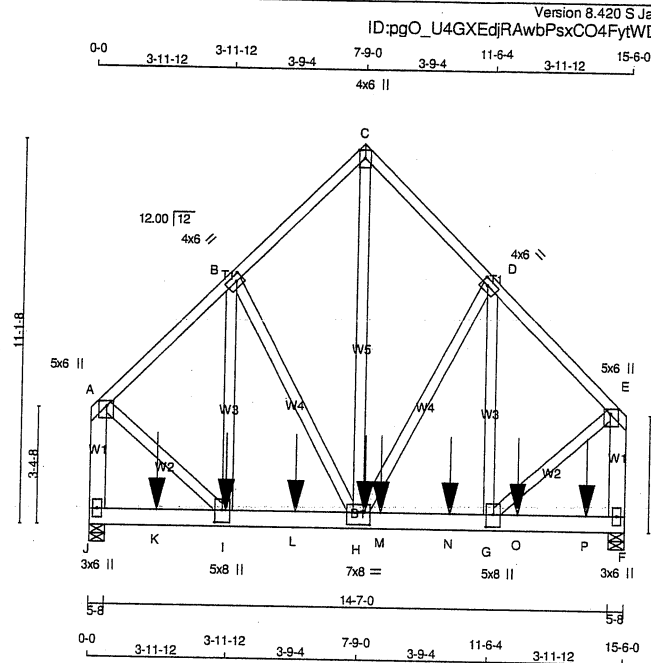
JSI GRIP = 0.66 (E) (INPUT = 0.90)
JSI METAL = 0.22 (E) (INPUT = 1.00)



Structural component only
DWG# T-2126679

Structural component only
DWG# T-2126680

JOB NAME 420542	TRUSS NAME T37	QUANTITY 1	PLY 3	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4 DRY No.2	SPF			
C - E	2x4 DRY No.2	SPF			
J - A	2x6 DRY No.2	SPF			
F - E	2x6 DRY No.2	SPF			
J - F	2x6 DRY No.2	SPF			
ALL WEBS	2x4 DRY No.2	SPF			
EXCEPT					

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C 1	12	TOP
C - E 1	12	TOP
J - A 2	12	TOP
F - E 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
J - F 2	7	SIDE(612.9)
WEBS : (0.122"x3") SPIRAL NAILS		
2x4 1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT REQRD BRG
JT	VERT 8689	DOWN 8689	IN-SX 5-8
F	HORZ 0	HORZ 0	IN-SX 5-8

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
JT	COMBINED SNOW LIVE	PERM. LIVE		
J	6162 3947 / 0	0 / 0	0 / 0	2215 / 0
F	6768 4336 / 0	0 / 0	0 / 0	2433 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.59 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH	WEBS	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH
MEMB.				MEMB.		
FR-TO				FR-TO		
A-B	-6248 / 0	-84.9 -84.9 0.13 (1)	4.63	H-C	0 / 7495	0.40 (1)
B-C	-5509 / 0	-84.9 -84.9 0.11 (1)	4.88	H-D	-1301 / 0	0.39 (1)
C-D	-5509 / 0	-84.9 -84.9 0.11 (1)	4.88	G-D	0 / 1390	0.07 (1)
D-E	-6357 / 0	-84.9 -84.9 0.14 (1)	4.59	B-H	-1141 / 0	0.35 (1)
J-A	-7848 / 0	0.0 0.0 0.33 (1)	6.42	I-B	0 / 1172	0.06 (1)
F-E	-7982 / 0	0.0 0.0 0.33 (1)	6.39	A-I	0 / 5507	0.30 (1)
				G-E	0 / 5603	0.30 (1)
J-K	0 / 0	-18.5 -18.5 0.36 (1)	10.00			
K-I	0 / 0	-18.5 -18.5 0.36 (1)	10.00			
I-L	0 / 4430	-18.5 -18.5 0.41 (1)	10.00			
L-H	0 / 4430	-18.5 -18.5 0.41 (1)	10.00			
H-M	0 / 4506	-18.5 -18.5 0.56 (1)	10.00			
M-N	0 / 4506	-18.5 -18.5 0.56 (1)	10.00			
N-G	0 / 4506	-18.5 -18.5 0.56 (1)	10.00			
G-O	0 / 0	-18.5 -18.5 0.38 (1)	10.00			
O-P	0 / 0	-18.5 -18.5 0.38 (1)	10.00			
P-F	0 / 0	-18.5 -18.5 0.38 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
H	7-11-4	-1474	-1474	---	BACK	VERT	TOTAL	---	C1
I	3-11-4	-1474	-1474	---	BACK	VERT	TOTAL	---	C1
K	1-11-4	-1474	-1474	---	BACK	VERT	TOTAL	---	C1
L	5-11-4	-1474	-1474	---	BACK	VERT	TOTAL	---	C1
M	8-4-12	-1474	-1474	---	BACK	VERT	TOTAL	---	C1
N	10-4-12	-1474	-1474	---	BACK	VERT	TOTAL	---	C1
O	12-4-12	-1474	-1474	---	BACK	VERT	TOTAL	---	C1
P	14-4-12	-1474	-1474	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3 PSF
DL = 6.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.4 PSF	
TOTAL LOAD = 36.7 PSF	

SPACING = 24.0 IN/C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.52")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.52")
CALCULATED VERT. DEFL.(TL) = L/999 (0.08")

CSI: TC=0.33/1.00 (E-F:1), BC=0.56/1.00 (G-H:1), WB=0.40/1.00 (C-H:1), SSI=0.54/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.61 (E) (INPUT = 1.00)



Structural component only
DWG# T-2126681

CONTINUED ON PAGE 2

JOB NAME 420542	TRUSS NAME T37	QUANTITY 1	PLY 3	JOB DESC. ROYAL PINE HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Aug 5 10:41:45 2021 Page 2
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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	1.75	2.75
B	TMVW-t	MT20	4.0	6.0	2.00	2.25
C	TTW+p	MT20	4.0	6.0	2.75	2.00
D	TMVW-t	MT20	4.0	6.0	2.00	2.25
E	TMVW+p	MT20	5.0	6.0	1.75	2.75
F	BMV1+p	MT20	3.0	6.0		
G	BMVW-t	MT20	5.0	8.0	4.25	2.50
H	BMVW-t	MT20	7.0	8.0	4.25	4.00
I	BMVW-t	MT20	5.0	8.0	4.25	2.50
J	BMV1+p	MT20	3.0	6.0		

NOTES- (1)

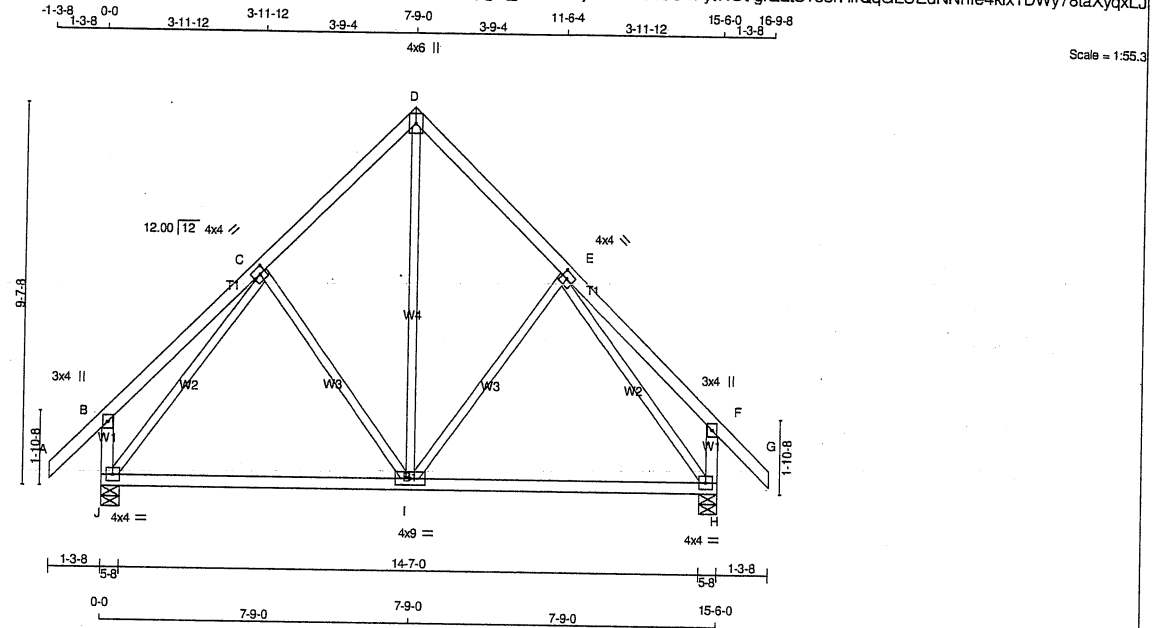
1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only
DWG# T-2126681 *m*

JOB NAME 420542	TRUSS NAME T38	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Thu Aug 5 10:41:46 2021 Page 1
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TOTAL WEIGHT = 79 b [M/F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
J - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	4.0	4.0	2.00	1.75
D	TTW+p	MT20	4.0	6.0		
E	TMVW-t	MT20	4.0	4.0	2.00	1.75
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMVW1-t	MT20	4.0	9.0		
J	BMVW1-t	MT20	4.0	4.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
J	919	0	919	0	5-8	5-8
H	919	0	919	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
J	650	426 / 0	0 / 0	0 / 0	0 / 0	224 / 0	0 / 0
H	650	426 / 0	0 / 0	0 / 0	0 / 0	224 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1) (LC)	MAX. FACTORED VERT. LOAD (LC1) (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC1) (LC)
FR-TO					FR-TO		
A-B	0 / 42	-84.9	-84.9 0.12 (1)	10.00	I-D	0 / 442	0.10 (1)
B-C	0 / 28	-84.9	-84.9 0.21 (1)	10.00	I-E	-164 / 0	0.13 (1)
C-D	-520 / 0	-84.9	-84.9 0.16 (1)	6.25	C-I	-164 / 0	0.13 (1)
D-E	-520 / 0	-84.9	-84.9 0.16 (1)	6.25	J-C	-766 / 0	0.60 (1)
E-F	0 / 28	-84.9	-84.9 0.21 (1)	10.00	E-H	-766 / 0	0.60 (1)
F-G	0 / 42	-84.9	-84.9 0.12 (1)	10.00			
J-B	-242 / 0	0.0	0.0 0.03 (1)	7.81			
H-F	-242 / 0	0.0	0.0 0.03 (1)	7.81			
J-I	0 / 446	-18.5	-18.5 0.36 (4)	10.00			
I-H	0 / 446	-18.5	-18.5 0.36 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.52")

CALCULATED VERT. DEFL.(LL) = L/999 (0.01")

ALLOWABLE DEFL.(TL)= L/360 (0.52")

CALCULATED VERT. DEFL.(TL) = L/999 (0.08")

CSI: TC=0.21/1.00 (B-C:1), BC=0.36/1.00 (H-I:4), WB=0.60/1.00 (C-J:1), SSI=0.12/1.00 (I-J:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (E) (INPUT = 0.90)

JSI METAL= 0.27 (C) (INPUT = 1.00)

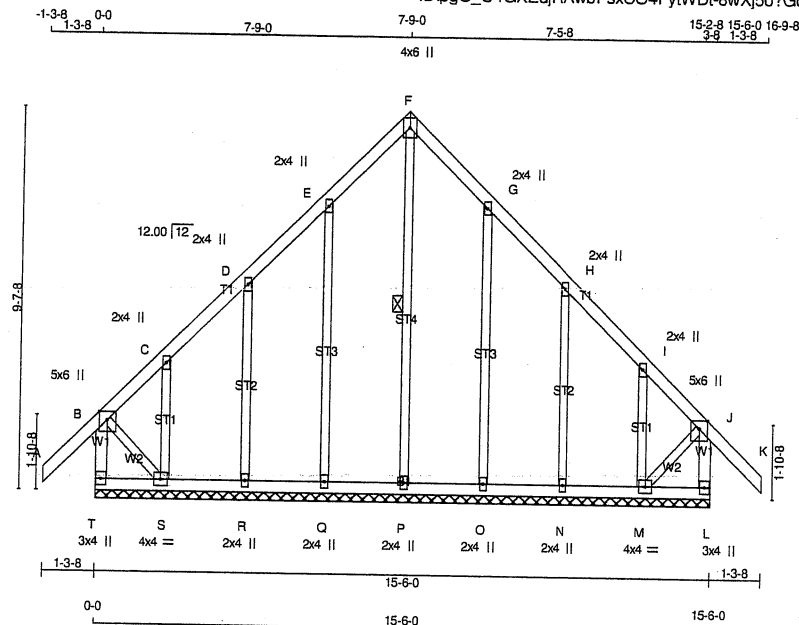


Structural component only
DWG# T-2126682

JOB NAME 420542	TRUSS NAME T38G	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 86 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER
T - B	2x4	DRY	No.2	
A - F	2x4	DRY	No.2	
F - K	2x4	DRY	No.2	
L - J	2x4	DRY	No.2	
T - L	2x4	DRY	No.2	

ALL WEBS 2x3 DRY No.2
ALL GABLE WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2'-0" OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	2.50
C, D, E, G, H, I						
C	TMVW+w	MT20	2.0	4.0		
F	TTW+p	MT20	4.0	6.0		
J	TMVW+p	MT20	5.0	6.0	Edge	2.50
L	BMV1+p	MT20	3.0	4.0		
M	BMVW1-t	MT20	4.0	4.0		
N, O, P, Q, R						
N	BMV1+w	MT20	2.0	4.0		
S	BMVW1-t	MT20	4.0	4.0		
T	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	CHORDS			MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
		FR-TO	FROM TO	VERT. LOAD LC1 MAX (PLF)			
T-B	-275 / 0		0.0	0.0	0.03 (1)	7.81	
A-B	0 / 42		-84.9	-84.9	0.12 (1)	10.00	
B-C	-63 / 0		-84.9	-84.9	0.11 (1)	6.25	
C-D	-21 / 0		-84.9	-84.9	0.04 (1)	6.25	
D-E	-21 / 0		-84.9	-84.9	0.05 (1)	6.25	
E-F	-31 / 0		-84.9	-84.9	0.05 (1)	6.25	
F-G	-31 / 0		-84.9	-84.9	0.04 (1)	6.25	
G-H	-21 / 0		-84.9	-84.9	0.05 (1)	6.25	
H-I	-21 / 0		-84.9	-84.9	0.04 (1)	6.25	
I-J	-63 / 0		-84.9	-84.9	0.11 (1)	6.25	
J-K	0 / 42		-84.9	-84.9	0.12 (1)	10.00	
L-J	-275 / 0		0.0	0.0	0.03 (1)	7.81	
T-S	0 / 0		-18.5	-18.5	0.02 (4)	10.00	
S-R	0 / 20		-18.5	-18.5	0.02 (4)	10.00	
R-Q	0 / 16		-18.5	-18.5	0.02 (4)	10.00	
Q-P	0 / 14		-18.5	-18.5	0.02 (4)	10.00	
P-O	0 / 14		-18.5	-18.5	0.02 (4)	10.00	
O-N	0 / 16		-18.5	-18.5	0.02 (4)	10.00	
N-M	0 / 20		-18.5	-18.5	0.02 (4)	10.00	
M-L	0 / 0		-18.5	-18.5	0.02 (4)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
DL = 6.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.4	PSF	
TOTAL LOAD = 36.7	PSF	

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.12/1.00 (A-B:1), BC=0.02/1.00 (R-S:4), WB=0.20/1.00 (E-Q:1), SSI=0.07/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788

PLATE PLACEMENT TOL. = 0.250 inches

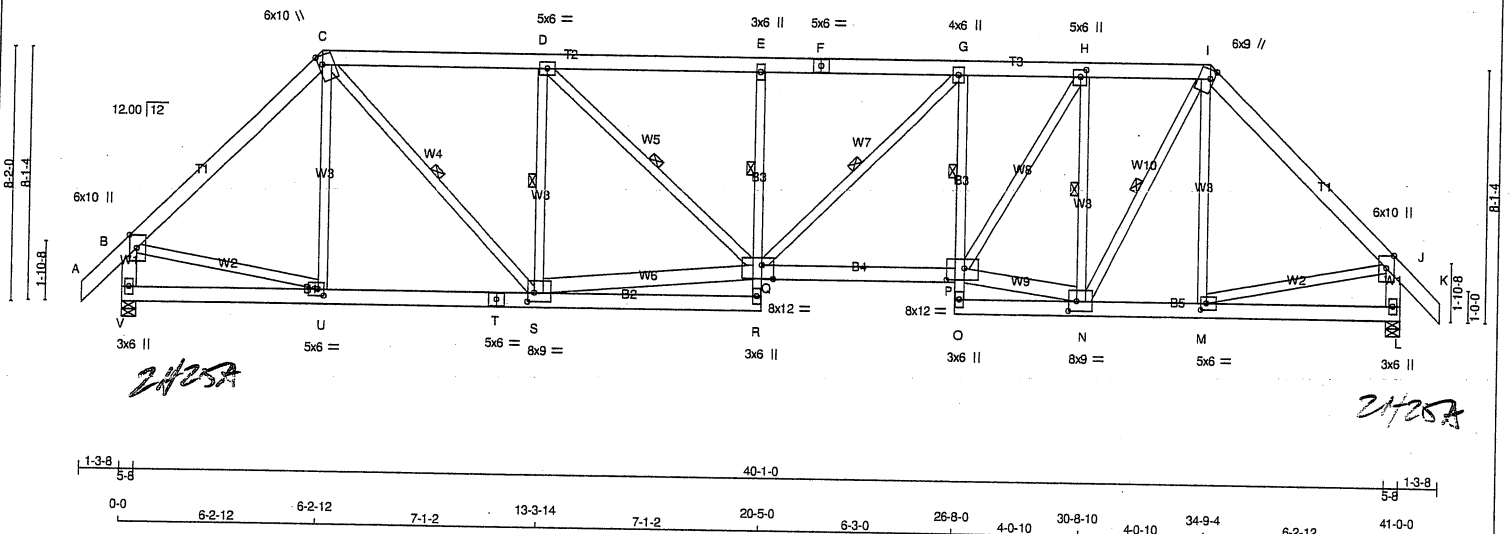
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (G) (INPUT = 0.90)
JSI METAL= 0.10 (G) (INPUT = 1.00)



Structural component only
DWG# T-2126683

JOB NAME 420547	TRUSS NAME T50S	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Thu Aug 5 11:04:44 2021 Page 1	
ID: X496mqPCjhQnqfx7YDh0Lytvsh-KrEH5eg?YEuE_CA54czmEzr2LY33pDVLqWyoxyqx?n				Scale = 1:70.5	



LUMBER			
N.L.G.A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x6	DRY	No.2
C - F	2x6	DRY	No.2
F - I	2x6	DRY	No.2
I - K	2x6	DRY	No.2
V - B	2x6	DRY	No.2
L - J	2x6	DRY	No.2
V - T	2x6	DRY	No.2
T - R	2x6	DRY	2100F 1.8E
R - E	2x6	DRY	2100F 1.8E
Q - P	2x4	DRY	No.2
O - G	2x6	DRY	2100F 1.8E
O - L	2x6	DRY	No.2
O - L	2x6	DRY	2100F 1.8E
ALL WEBS	2x4	DRY	No.2
EXCEPT			
S - Q	2x6	DRY	2100F 1.8E
P - N	2x6	DRY	2100F 1.8E

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	6.0	10.0	Edge	2.75
C TTWW+m	MT20	6.0	10.0	3.50	1.50
D TMVW-t	MT20	5.0	6.0		
E TMV+p	MT20	3.0	6.0		
F TS-t	MT20	5.0	6.0		
G TMVW+p	MT20	4.0	6.0		
H TMVW-t	MT20	5.0	6.0	2.75	2.25
I TTWW+m	MT20	6.0	9.0	3.50	1.50
J TMVW+p	MT20	6.0	10.0	Edge	
L BMV1+p	MT20	3.0	6.0		
M BMVW-t	MT20	5.0	6.0	2.50	2.25
N BMVW-t	MT20	8.0	9.0	3.75	3.00
O BMV+p	MT20	3.0	6.0		
P BVWW-t	MT20	8.0	12.0	4.50	6.75
Q BVWW-t	MT20	8.0	12.0	5.25	4.50
R BMV+p	MT20	3.0	6.0		
S BMVW-t	MT20	8.0	9.0	3.50	2.50
T BS-t	MT20	5.0	6.0		
U BMVW-t	MT20	5.0	6.0	2.50	2.25
V BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
V	3120	0	3427	-388
L	3120	0	3427	-1091

PROVIDE ANCHORAGE AT BEARING JOINT V FOR 1091 LBS FACTORED UPLIFT

PROVIDE ANCHORAGE AT BEARING JOINT L FOR 1091 LBS FACTORED UPLIFT

PROVIDE FOR 388 LBS FACTORED HORIZONTAL REACTION AT JOINT V

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
V	2318	1525 / 0	431 / 0	0 / 0	293 / -1052	567 / 0
L	2318	1525 / 0	431 / 0	0 / 0	143 / -1052	567 / 0

HORIZONTAL REACTIONS

V	0 / 0	0 / 0	0 / 0	277 / -277	0 / 0	0 / 0
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BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, L

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 3.36 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 5.74 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-Q, G-P, C-S, D-S, D-Q, G-Q, H-N, I-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (18)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)	FACTORED MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 62	-105.2 -105.2	0.09 (2)	U-C	-273 / 170	0.22 (9)	
B-C	-3238 / 1083	-105.2 -105.2	0.53 (2)	C-S	-837 / 2495	0.44 (14)	
C-D	-3874 / 1488	-105.2 -105.2	0.55 (2)	S-D	-2070 / 811	0.56 (3)	
D-E	-4986 / 1824	-105.2 -105.2	0.53 (2)	S-Q	-1128 / 3878	0.30 (13)	
E-F	-4996 / 1827	-105.2 -105.2	0.46 (3)	D-Q	-487 / 1629	0.26 (3)	
F-G	-4996 / 1827	-105.2 -105.2	0.46 (3)	Q-G	-272 / 765	0.12 (2)	
G-H	-4547 / 1663	-105.2 -105.2	0.31 (3)	P-N	-869 / 3341	0.16 (3)	
H-I	-3261 / 1298	-105.2 -105.2	0.20 (3)	P-H	-809 / 2556	0.63 (13)	
I-J	-3234 / 1084	-105.2 -105.2	0.53 (3)	N-H	-2636 / 918	0.72 (2)	
J-K	0 / 62	-105.2 -105.2	0.09 (3)	N-I	-757 / 2245	0.36 (2)	
V-B	-3326 / 1114	0.0 0.0	0.23 (2)	M-I	-310 / 165	0.24 (10)	
L-J	-3323 / 1115	0.0 0.0	0.23 (3)	B-U	-538 / 2330	0.37 (2)	
				M-J	-539 / 2326	0.37 (3)	
V-U	-362 / 376	-39.5 -39.5	0.08 (17)				
U-T	-618 / 2277	-39.5 -39.5	0.17 (5)				
T-S	-618 / 2277	-39.5 -39.5	0.17 (5)				
S-R	-19 / 61	-39.5 -39.5	0.09 (17)				
R-Q	0 / 145	0.0 0.0	0.10 (3)				
Q-E	-835 / 402	0.0 0.0	0.12 (3)				
Q-P	-1272 / 4555	-39.5 -39.5	0.29 (3)				
O-P	0 / 84	0.0 0.0	0.08 (3)				
P-G	-1203 / 523	0.0 0.0	0.16 (2)				
O-N	-16 / 52	-39.5 -39.5	0.03 (17)				
N-M	-498 / 2274	-39.5 -39.5	0.16 (3)				
M-L	-12 / 27	-39.5 -39.5	0.07 (17)				

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 30.1	PSF
DL = 6.0	PSF	
BOT CH.	LL = 10.5	PSF
DL = 7.4	PSF	
TOTAL LOAD = 54.0	PSF	

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 4 OF BCBC 2018, OBC 2012, ABC 2019
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- SLOPE REDUCTION FACTOR NOT USED

(80% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 30.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.37")
CALCULATED VERT. DEFL.(LL) = L/999 (0.20")
ALLOWABLE DEFL.(TL) = L/180 (2.73")
CALCULATED VERT. DEFL.(TL) = L/999 (0.28")

CSI: TC=0.63/1.00 (D-E-2), BC=0.29/1.00 (P-Q-3), WB=0.72/1.00 (H-N-2), SSI=0.31/1.00 (C-D-2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE ROTATION TOL. = 0.250 inches

PLATE PLACEMENT TOL. = 5.0 Deg.

JSI GRIP=0.89 (H) (INPUT = 0.90)
JSI METAL=0.83 (B) (INPUT = 1.00)



Structural component only
DWG# T-2126690

JOB NAME 420547	TRUSS NAME T50S	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			

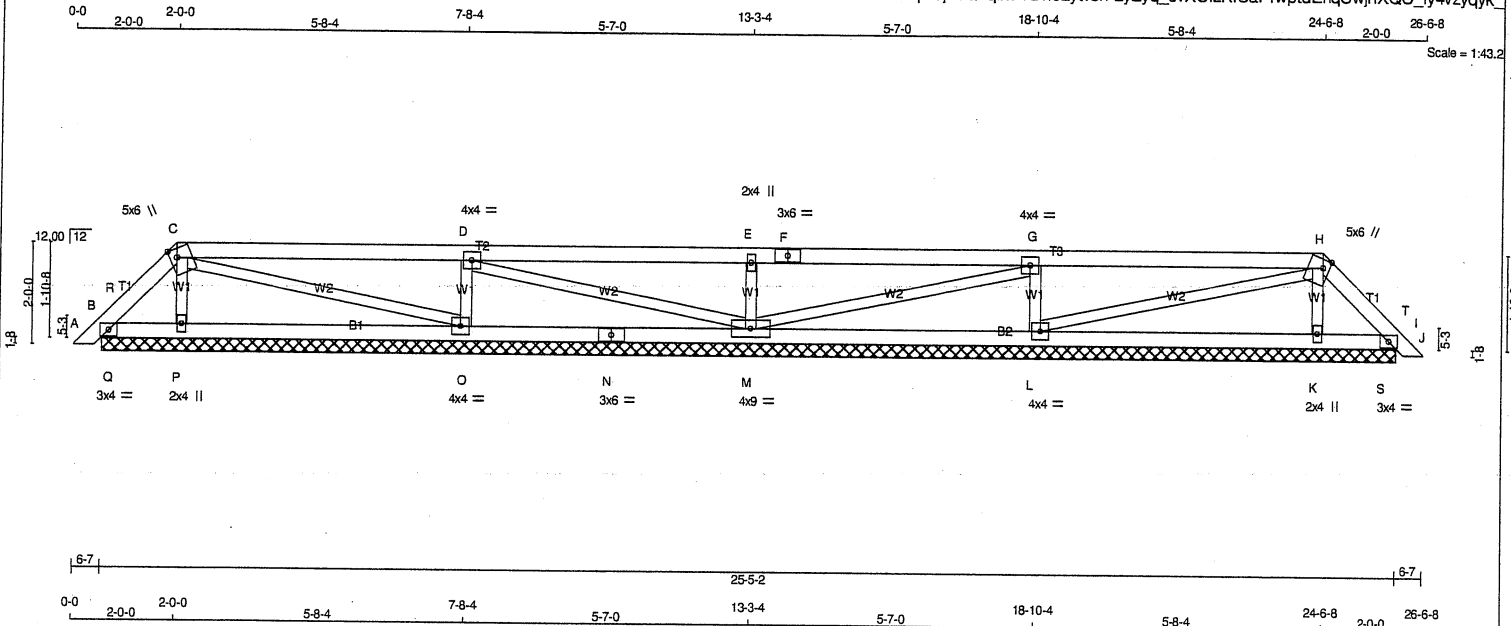
Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Aug 5 11:04:44 2021 Page 2
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Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.2) PSF AT
(40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK
COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL
WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON
(OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY
FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF
AND 5.0 PSF RESPECTIVELY.



Structural component only
DWG# T-2126690 *mm*



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - J	2x4	DRY	No.2
B - N	2x4	DRY	No.2
N - I	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			SPF

PLATES (table is in inches)					
JL	TYPE	PLATES	W	LEN	Y X
B	TMB1-t	MT20	3.0	4.0	
C	TTVVW+m	MT20	5.0	6.0	2.00 1.50
D	TMWw-t	MT20	4.0	4.0	
E	TMW+w	MT20	2.0	4.0	
F	TS-t	MT20	3.0	6.0	
G	TMMW-t	MT20	4.0	4.0	
H	TTVVW+m	MT20	5.0	6.0	2.00 1.50
I	TMB1-t	MT20	3.0	4.0	
K	BMW1+w	MT20	2.0	4.0	
L	BMWVW1-t	MT20	4.0	4.0	
M	BMWVW1-t	MT20	4.0	9.0	
N	BS-t	MT20	3.0	6.0	
O	BMWVW1-t	MT20	4.0	4.0	
P	BMW1+w	MT20	2.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARING DESIGNER								
FACTORED			MAXIMUM FACTORED			INPUT	REQRD	
GROSS REACTION			GROSS REACTION			BRG	BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
JT	132	0	132	0	0	25-5-2	15-5-2	25-5-2
P	292	0	292	0	0	25-5-2	15-5-2	25-5-2
O	650	0	650	0	0	25-5-2	15-5-2	25-5-2
M	554	0	554	0	0	25-5-2	15-5-2	25-5-2
L	650	0	650	0	0	25-5-2	15-5-2	25-5-2
K	292	0	292	0	0	25-5-2	15-5-2	25-5-2
I	132	0	132	0	0	25-5-2	15-5-2	25-5-2

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS							
1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	90	79 / 0	0 / 0	0 / 0	0 / 0	10 / 0	0 / 0
P	212	110 / 0	0 / 0	0 / 0	0 / 0	101 / 0	0 / 0
O	460	299 / 0	0 / 0	0 / 0	0 / 0	162 / 0	0 / 0
M	394	248 / 0	0 / 0	0 / 0	0 / 0	145 / 0	0 / 0
L	460	299 / 0	0 / 0	0 / 0	0 / 0	162 / 0	0 / 0
K	212	110 / 0	0 / 0	0 / 0	0 / 0	101 / 0	0 / 0
I	90	79 / 0	0 / 0	0 / 0	0 / 0	10 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, P, O, M, L, K, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM TO		LENGTH	FR-TO			
A-B	0 / 12	-84.9	-84.9	0.01 (1)	10.00	P-C	-187 / 0	0.03 (1)
B-R	-22 / 21	-84.9	-84.9	0.02 (4)	6.25	C-O	-17 / 0	0.01 (1)
R-C	-65 / 0	-84.9	-84.9	0.02 (4)	6.25	C-D	-540 / 0	0.08 (1)
C-D	-12 / 0	-84.9	-84.9	0.40 (1)	6.25	D-M	-29 / 0	0.02 (1)
D-E	0 / 16	-84.9	-84.9	0.40 (1)	10.00	M-E	-436 / 0	0.06 (1)
E-F	0 / 16	-84.9	-84.9	0.40 (1)	10.00	M-G	-29 / 0	0.02 (1)
F-G	0 / 16	-84.9	-84.9	0.40 (1)	10.00	L-G	-540 / 0	0.08 (1)
G-H	-12 / 0	-84.9	-84.9	0.40 (1)	6.25	L-H	-17 / 0	0.01 (1)
H-T	-65 / 0	-84.9	-84.9	0.02 (4)	6.25	K-H	-187 / 0	0.03 (1)
T-I	-22 / 21	-84.9	-84.9	0.02 (4)	6.25	Q-R	-115 / 0	0.00 (1)
I-J	0 / 12	-84.9	-84.9	0.01 (1)	10.00	S-T	-115 / 0	0.00 (1)
B-Q	0 / 41	-18.5	-18.5	0.03 (1)	10.00			
Q-P	0 / 41	-18.5	-18.5	0.09 (4)	10.00			
P-O	0 / 28	-18.5	-18.5	0.12 (4)	10.00			
O-N	0 / 12	-18.5	-18.5	0.12 (4)	10.00			
N-M	0 / 12	-18.5	-18.5	0.12 (4)	10.00			
M-L	0 / 12	-18.5	-18.5	0.12 (4)	10.00			
L-K	0 / 28	-18.5	-18.5	0.12 (4)	10.00			
K-S	0 / 41	-18.5	-18.5	0.09 (4)	10.00			
S-I	0 / 41	-18.5	-18.5	0.03 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR
SMALL BUILDING REQUIREMENTS OF PART 9,
NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.40/1.00 (D-E:1), BC=0.12/1.00 (O-P:4),
WB=0.08/1.00 (D-O:1), SSI=0.23/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES					
PLATE	GRIP(DRY)		SHEAR		SECTION
	(PSI)		(PLI)		(PLI)
	MAX	MIN	MAX	MIN	MAX MIN
MT20	650	371	1747	788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

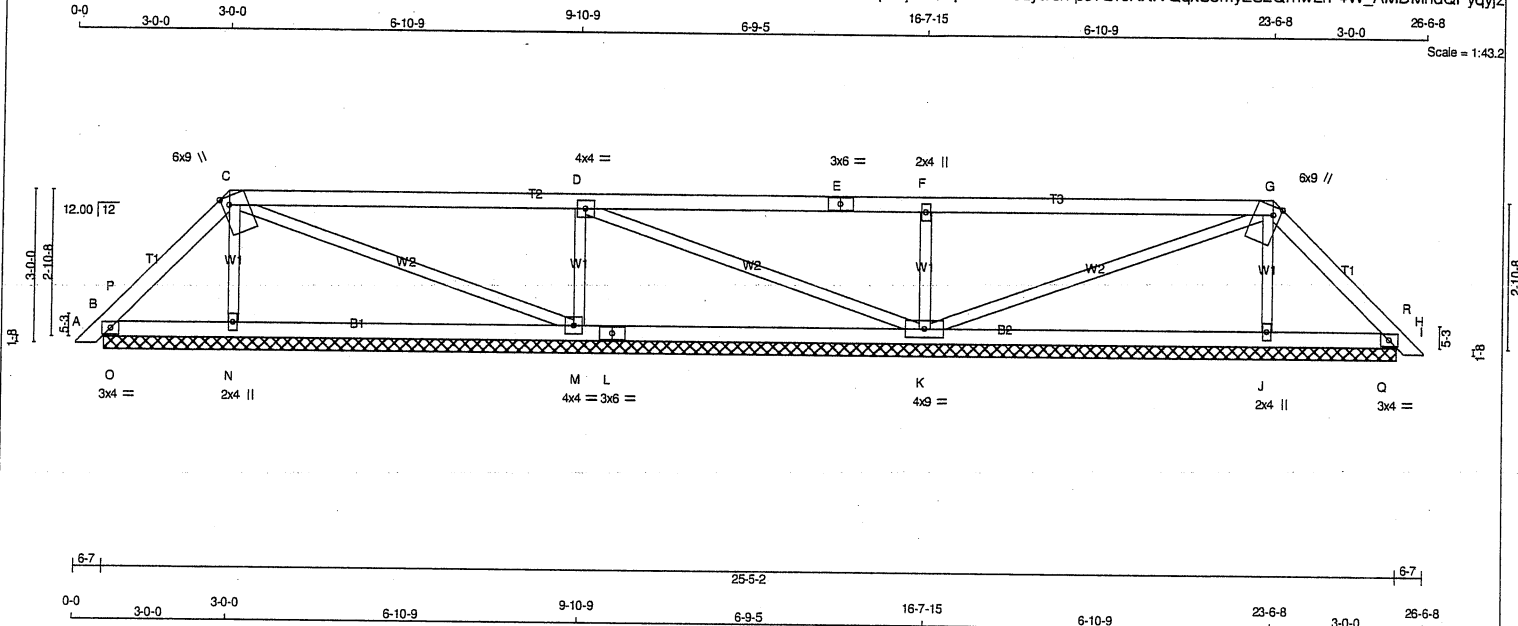
JSI GRIP= 0.66 (O) (INPUT = 0.90)
JSI METAL= 0.15 (F) (INPUT = 1.00)



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
420538	PB2	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER				
N. L. G. A. RULES				
CHORDS		SIZE	LUMBER	DESCR
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
B - L	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS		2x3	DRY	No.2
DRY:		SEASONED LUMBER.		SPF

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-t	MT20	3.0	4.0		
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TTWW+m	MT20	6.0	9.0	Edge	1.75
H	TMB1-t	MT20	3.0	4.0		
J	BMW1-w	MT20	2.0	4.0		
K	BMWW1-t	MT20	4.0	9.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW1-t	MT20	4.0	4.0		
N	BMW1-w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES-

- (1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	VERT	HORZ	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT		REQD	
			DOWN	HORZ	DOWN	HORZ	IN-SX	IN-SX	BRG	BRG
B	214	0	214	0	0	0	25-5-2 (15-5-2)	25-5-2 (15-5-2)		
N	363	0	363	0	0	0	25-5-2 (15-5-2)	25-5-2 (15-5-2)		
M	760	0	760	0	0	0	25-5-2 (15-5-2)	25-5-2 (15-5-2)		
K	798	0	798	0	0	0	25-5-2 (15-5-2)	25-5-2 (15-5-2)		
J	370	0	370	0	0	0	25-5-2 (15-5-2)	25-5-2 (15-5-2)		
H	198	0	198	0	0	0	25-5-2 (15-5-2)	25-5-2 (15-5-2)		

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	148	118 / 0	0 / 0	0 / 0	0 / 0	30 / 0	0 / 0
N	263	140 / 0	0 / 0	0 / 0	0 / 0	123 / 0	0 / 0
M	538	347 / 0	0 / 0	0 / 0	0 / 0	191 / 0	0 / 0
K	565	368 / 0	0 / 0	0 / 0	0 / 0	197 / 0	0 / 0
J	267	144 / 0	0 / 0	0 / 0	0 / 0	124 / 0	0 / 0
H	137	109 / 0	0 / 0	0 / 0	0 / 0	28 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, N, M, K, J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1		MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
			FROM	TO				
FR-TO								
A-B	0 / 12	-84.9	-84.9	0.01 (1)	10.00	N-C	-236 / 0	0.04 (1)
B-P	-5 / 23	-84.9	-84.9	0.04 (1)	10.00	C-M	-30 / 0	0.03 (1)
P-C	-108 / 0	-84.9	-84.9	0.05 (1)	6.25	M-D	-622 / 0	0.11 (1)
C-D	-31 / 0	-84.9	-84.9	0.54 (1)	6.25	D-K	-42 / 0	0.04 (1)
D-E	0 / 7	-84.9	-84.9	0.54 (1)	10.00	K-F	-637 / 0	0.11 (1)
E-F	0 / 7	-84.9	-84.9	0.54 (1)	10.00	K-G	-55 / 0	0.06 (1)
F-G	0 / 8	-84.9	-84.9	0.54 (1)	10.00	J-G	-243 / 0	0.04 (1)
G-R	-86 / 0	-84.9	-84.9	0.05 (1)	6.25	O-P	-230 / 0	0.00 (1)
R-H	0 / 28	-84.9	-84.9	0.04 (1)	10.00	Q-R	-234 / 0	0.00 (1)
H-I	0 / 12	-84.9	-84.9	0.01 (1)	10.00			
B-O	0 / 69	-18.5	-18.5	0.07 (1)	10.00			
O-N	0 / 69	-18.5	-18.5	0.13 (4)	10.00			
N-M	0 / 60	-18.5	-18.5	0.18 (4)	10.00			
M-L	0 / 32	-18.5	-18.5	0.17 (4)	10.00			
L-K	0 / 32	-18.5	-18.5	0.17 (4)	10.00			
K-J	0 / 44	-18.5	-18.5	0.17 (4)	10.00			
J-Q	0 / 54	-18.5	-18.5	0.13 (4)	10.00			
Q-H	0 / 54	-18.5	-18.5	0.07 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
DL	=	6.0	PSF	
BOT CH.	LL	=	0.0	PSF
DL	=	7.4	PSF	
TOTAL LOAD	=	36.7	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.54/1.00 (C-D:1), BC=0.18/1.00 (M-N:4), WB=0.11/1.00 (F-K:1), SS=0.27/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650	371	1747
	788	1987	1873

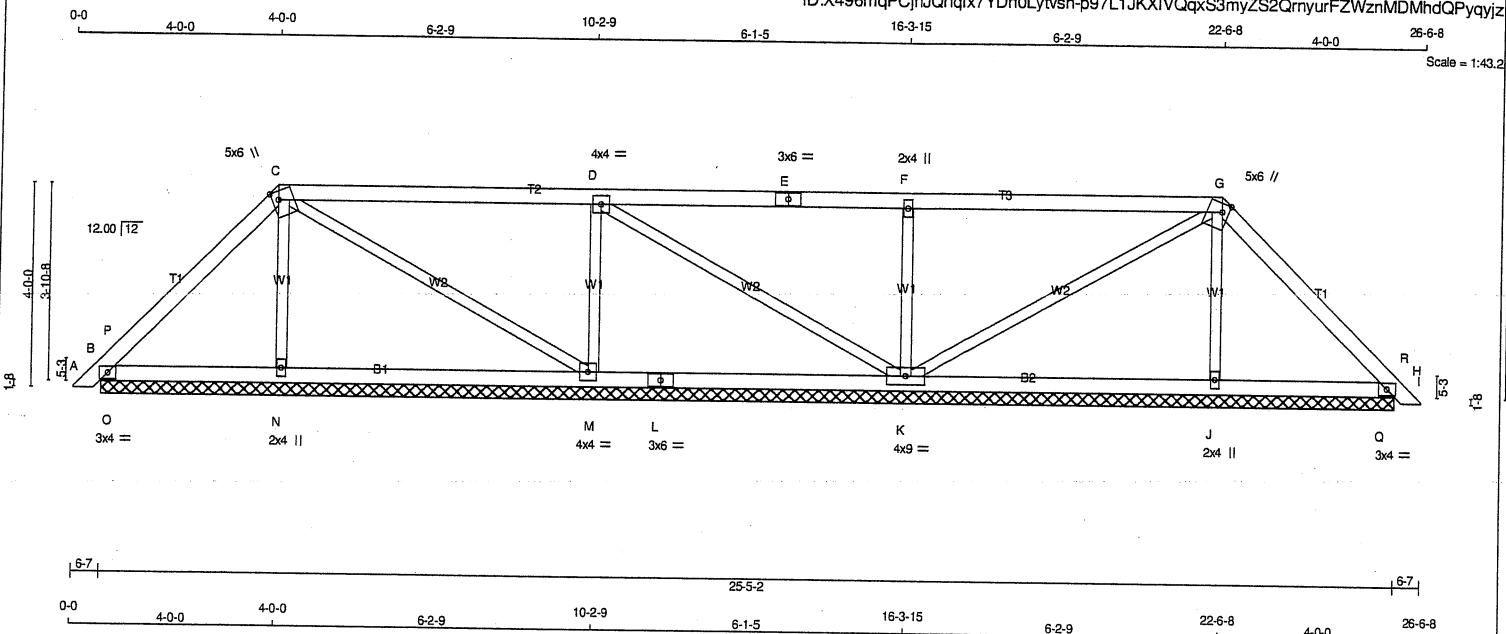
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.68 (M) (INPUT = 0.90)
JSI METAL= 0.17 (L) (INPUT = 1.00)



Structural component only
DWG# T-2126622



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - E	2x4	DRY No.2	SPF
E - G	2x4	DRY No.2	SPF
G - I	2x4	DRY No.2	SPF
B - L	2x4	DRY No.2	SPF
L - H	2x4	DRY No.2	SPF
ALL WEBS	2x3	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-t	MT20	3.0	4.0		
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW+w	MT20	2.0	4.0		
G	TTWW+m	MT20	5.0	6.0	2.00	1.50
H	TMB1-t	MT20	3.0	4.0		
J	BMW1+w	MT20	2.0	4.0		
K	BMWW1-t	MT20	4.0	9.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW1-t	MT20	4.0	4.0		
N	BMW1+w	MT20	2.0	4.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
B	306	0	306	0	25-5-2 (14-5-25-5-2)	
N	335	0	335	0	25-5-2 (14-5-25-5-2)	
M	672	0	672	0	25-5-2 (14-5-25-5-2)	
K	774	0	774	0	25-5-2 (14-5-25-5-2)	
J	339	0	339	0	25-5-2 (14-5-25-5-2)	
H	279	0	279	0	25-5-2 (14-5-25-5-2)	

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	COMBINED	MAX/MIN. COMPONENT REACTIONS				DEAD	SOIL
		SNOW	LIVE	PERM. LIVE	WIND		
B	213	156 / 0	0 / 0	0 / 0	0 / 0	57 / 0	0 / 0
N	242	130 / 0	0 / 0	0 / 0	0 / 0	112 / 0	0 / 0
M	476	305 / 0	0 / 0	0 / 0	0 / 0	171 / 0	0 / 0
K	547	361 / 0	0 / 0	0 / 0	0 / 0	186 / 0	0 / 0
J	245	132 / 0	0 / 0	0 / 0	0 / 0	113 / 0	0 / 0
H	195	141 / 0	0 / 0	0 / 0	0 / 0	53 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, N, M, K, J, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)		
FR-TO		FROM	TO		FR-TO				
A-B	0 / 12	-84.9	-84.9	0.01 (1)	10.00	N-C	-207 / 0	0.05 (1)	
B-P	-6 / 30	-84.9	-84.9	0.07 (1)	10.00	C-M	-48 / 0	0.05 (1)	
P-C	-157 / 0	-84.9	-84.9	0.12 (1)	6.25	M-D	-535 / 0	0.13 (1)	
C-D	-54 / 0	-84.9	-84.9	0.44 (1)	6.25	D-K	-78 / 0	0.07 (1)	
D-E	0 / 13	-84.9	-84.9	0.44 (1)	10.00	K-F	-575 / 0	0.13 (1)	
E-F	0 / 13	-84.9	-84.9	0.44 (1)	10.00	K-G	-95 / 0	0.09 (1)	
F-G	0 / 13	-84.9	-84.9	0.44 (1)	10.00	J-G	-210 / 0	0.05 (1)	
G-R	-119 / 0	-84.9	-84.9	0.12 (1)	6.25	O-P	-375 / 0	0.00 (1)	
R-H	0 / 71	-84.9	-84.9	0.08 (1)	10.00	Q-R	-380 / 0	0.00 (1)	
H-I	0 / 12	-84.9	-84.9	0.01 (1)	10.00				
B-O	0 / 102	-18.5	-18.5	0.12 (1)	10.00				
O-N	0 / 102	-18.5	-18.5	0.12 (4)	10.00				
N-M	0 / 96	-18.5	-18.5	0.14 (4)	10.00				
M-L	0 / 54	-18.5	-18.5	0.14 (4)	10.00				
L-K	0 / 54	-18.5	-18.5	0.14 (4)	10.00				
K-J	0 / 70	-18.5	-18.5	0.14 (4)	10.00				
J-Q	0 / 76	-18.5	-18.5	0.12 (4)	10.00				
Q-H	0 / 76	-18.5	-18.5	0.12 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.44/1.00 (D-F:1), BC=0.14/1.00 (M-N:4), WB=0.13/1.00 (F-K:1), SI=0.29/1.00 (H-Q:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

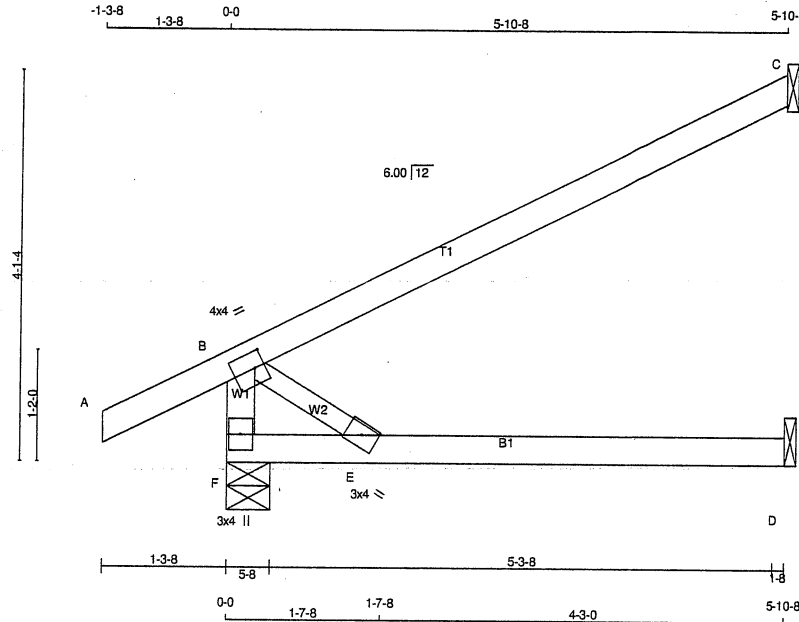
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.66 (G) (INPUT = 0.90)
JSI METAL= 0.14 (E) (INPUT = 1.00)



JOB NAME 420538	TRUSS NAME J1	QUANTITY 22	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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TOTAL WEIGHT = 22 X 18 = 396 lb [M]

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
F - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-t	MT20	4.0	4.0	2.00 1.25
E	BMW+w	MT20	3.0	4.0	
F	BMV1+p	MT20	3.0	4.0	

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX
F	419	0	419	0	5-8	5-8
C	249	0	249	0	1-8	1-8
D	54	0	61	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
F	295	200/0	0/0	0/0	0/0	95/0	0/0
C	172	137/0	0/0	0/0	0/0	35/0	0/0
D	43	0/0	0/0	0/0	0/0	43/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	
FR-TO		FROM	TO	FR-TO			
F-B	-364/0	0.0	0.0 0.04 (1)	B-E	0/0	0.00 (1)	
A-B	0/26	-84.9	-84.9 0.11 (1)				
B-C	0/0	-84.9	-84.9 0.50 (1)				
F-E	0/0	-18.5	-18.5 0.14 (4)				
E-D	0/0	-18.5	-18.5 0.19 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCG 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.50/1.00 (B-C:1), BC=0.19/1.00 (D-E:4), WB=0.00/1.00 (B-E:1), SSI=0.17/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

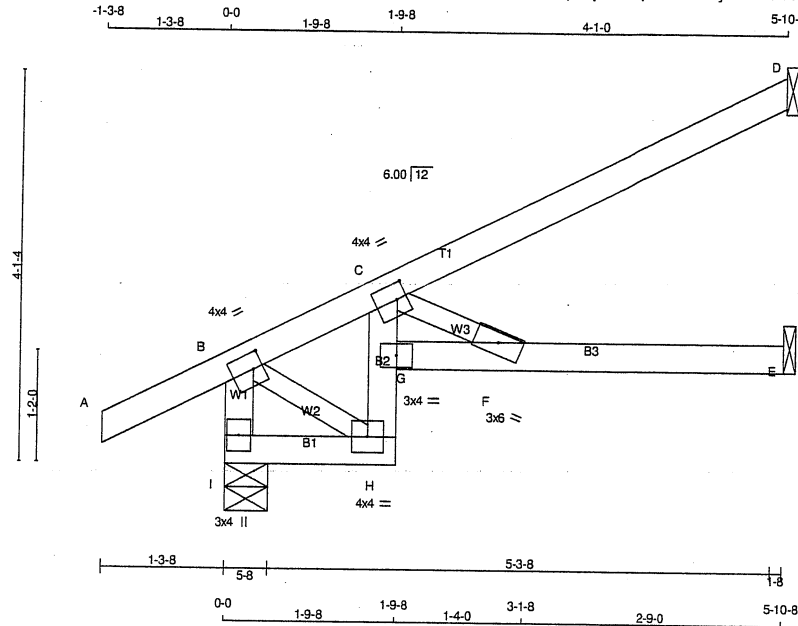


Structural component only
DWG# T-2126619

JOB NAME 420540	TRUSS NAME J1S	QUANTITY 9	PLY 1	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 9 X 21 = 186 lb [M]

LUMBER				DESCR.
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	
I - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
H - C	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B TMVW-t	MT20	4.0	4.0	2.00	1.25
C TMVW-t	MT20	4.0	4.0	2.00	1.25
F BMW+w	MT20	3.0	6.0		
G BVMH	MT20	3.0	4.0		
H BMVW-t	MT20	4.0	4.0		
I BMV1+p	MT20	3.0	4.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
JT	432	0	432	0	5-8	5-8
I	184	0	184	0	1-8	1-8
D	107	0	107	0	1-8	1-8
E						

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) D, E

UNFACTORED REACTIONS

	1ST LCASE COMBINED		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	WIND			
JT	304	207 / 0	0 / 0	0 / 0	97 / 0	0 / 0	0 / 0
I	128	96 / 0	0 / 0	0 / 0	32 / 0	0 / 0	0 / 0
D	79	33 / 0	0 / 0	0 / 0	45 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)		FACTORED VERT. LOAD (PLF)		MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	WEBS MAX. FACTORED FORCE (LBS)		MAX. CSI (LC)
	FR-TO	FROM TO	FROM TO	TO				FR-TO	FROM TO	
I-B	-417 / 0	0.0	0.0	0.04 (1)	7.81		B-H	0 / 192	0.04 (1)	
A-B	0 / 26	-84.9	-84.9	0.11 (1)	10.00		C-F	-481 / 0	0.07 (1)	
B-C	-216 / 0	-84.9	-84.9	0.11 (1)	6.25					
C-D	0 / 4	-84.9	-84.9	0.27 (1)	10.00					
I-H	0 / 0	-18.5	-18.5	0.01 (4)	10.00					
H-G	-73 / 0	0.0	0.0	0.23 (1)	7.81					
G-C	0 / 134	0.0	0.0	0.25 (1)	10.00					
G-F	0 / 419	-18.5	-18.5	0.39 (1)	10.00					
F-E	0 / 0	-18.5	-18.5	0.32 (1)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3 PSF
DL = 6.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.4 PSF	
TOTAL LOAD = 36.7 PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL)= L/999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL)= L/587 (0.12")

CSI: TC=0.27/1.00 (C-D:1), BC=0.39/1.00 (F-G:1), WB=0.07/1.00 (C-F:1), SS=0.19/1.00 (C-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PL)
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

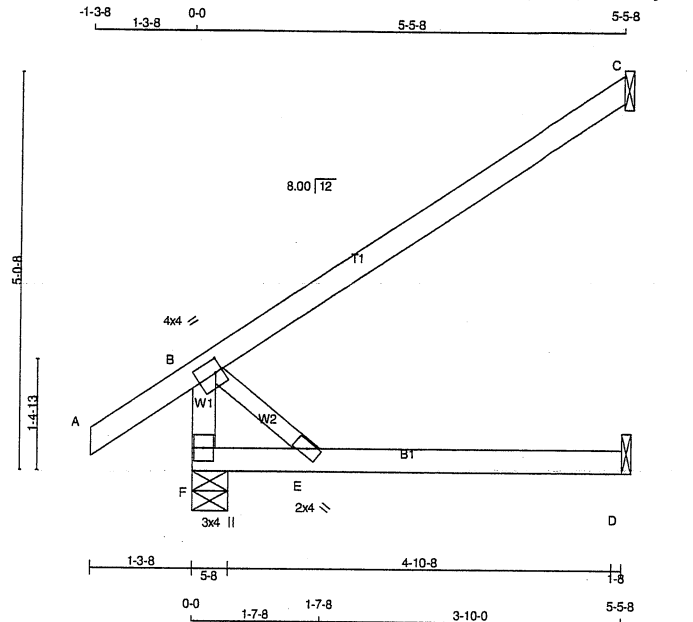
JSI GRIP= 0.36 (C) (INPUT = 0.90)
JSI METAL= 0.15 (G) (INPUT = 1.00)



Structural component only
DWG# T-2126649

JOB NAME 420538	TRUSS NAME J2	QUANTITY 4	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1:27.9

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
F - B	2x4	DRY No.2	SPF
A - C	2x4	DRY No.2	SPF
F - D	2x4	DRY No.2	SPF
ALL WEBS	2x3	DRY No.2	SPF
DRY: SEASONED LUMBER.			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.00
E	BMW-w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES:

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
F	399	0	399	0	5-8	5-8
C	232	0	232	0	1-8	1-8
D	50	0	57	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	281	191/0	0/0	0/0	0/0	90/0	0/0
C	160	127/0	0/0	0/0	0/0	33/0	0/0
D	40	0/0	0/0	0/0	0/0	40/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		MAX. UNBRAC LENGTH	MEMB.	WEBS	
	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX. CSI (LC)			MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM	TO			FR-TO		
F-B	-348/0	0.0	0.0	0.04 (1)	7.81	B-E	0/0	0.00 (1)
A-B	0/32	-84.9	-84.9	0.11 (1)	10.00			
B-C	0/0	-84.9	-84.9	0.43 (1)	10.00			
F-E	0/0	-18.5	-18.5	0.13 (4)	10.00			
E-D	0/0	-18.5	-18.5	0.16 (4)	10.00			

TOTAL WEIGHT = 4 X 18 = 72 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3 PSF
DL = 6.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.4 PSF	
TOTAL LOAD = 36.7 PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.43/1.00 (B-C:1), BC=0.16/1.00 (D-E:4), WB=0.00/1.00 (B-E:1), SSI=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20 650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

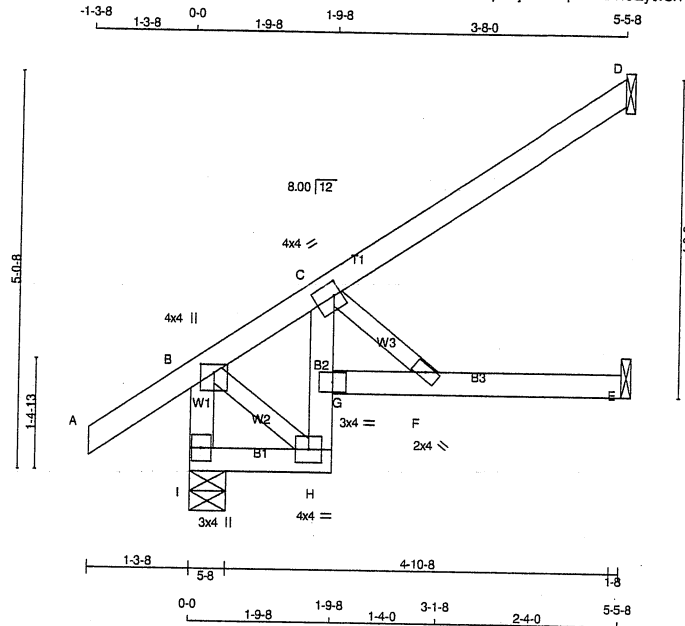


Structural component only
DWG# T-2126620

JOB NAME 420540	TRUSS NAME J2S	QUANTITY 3	PLY 1	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER
I - B	2x4	DRY No.2
A - D	2x4	DRY No.2
I - H	2x4	DRY No.2
H - C	2x4	DRY No.2
G - E	2x4	DRY No.2

ALL WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

DESCR.

SPF	SPF	SPF	SPF	SPF	SPF
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DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
I	413	0	413	0	5-8	5-8
D	162	0	162	0	1-8	1-8
E	105	0	105	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) D, E

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
I	291	199/0	0/0	0/0	0/0	92/0	0/0
D	113	85/0	0/0	0/0	0/0	27/0	0/0
E	77	34/0	0/0	0/0	0/0	44/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED				WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO			FR-TO		
I-B	-398/0	0.0	0.0	0.04 (1)	7.81	B-H	0 / 147	0.03 (1)
A-B	0 / 32	-84.9	-84.9	0.11 (1)	10.00	C-F	-305 / 0	0.05 (1)
B-C	-175/0	-84.9	-84.9	0.11 (1)	6.25			
C-D	0 / 4	-84.9	-84.9	0.21 (1)	10.00			
I-H	0 / 0	-18.5	-18.5	0.01 (4)	10.00			
H-G	-65 / 0	0.0	0.0	0.17 (1)	7.81			
G-C	0 / 103	0.0	0.0	0.18 (1)	10.00			
G-F	0 / 227	-18.5	-18.5	0.32 (1)	10.00			
F-E	0 / 0	-18.5	-18.5	0.28 (1)	10.00			

TOTAL WEIGHT = 3 X 22 = 66 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.04")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/792$ (0.08")

CSI: TC=0.21/1.00 (C-D:1), BC=0.32/1.00 (F-G:1), WB=0.05/1.00 (C-F:1), SS=0.13/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354
	1667	788
	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.28 (B) (INPUT = 0.90)
JSI METAL = 0.16 (F) (INPUT = 1.00)

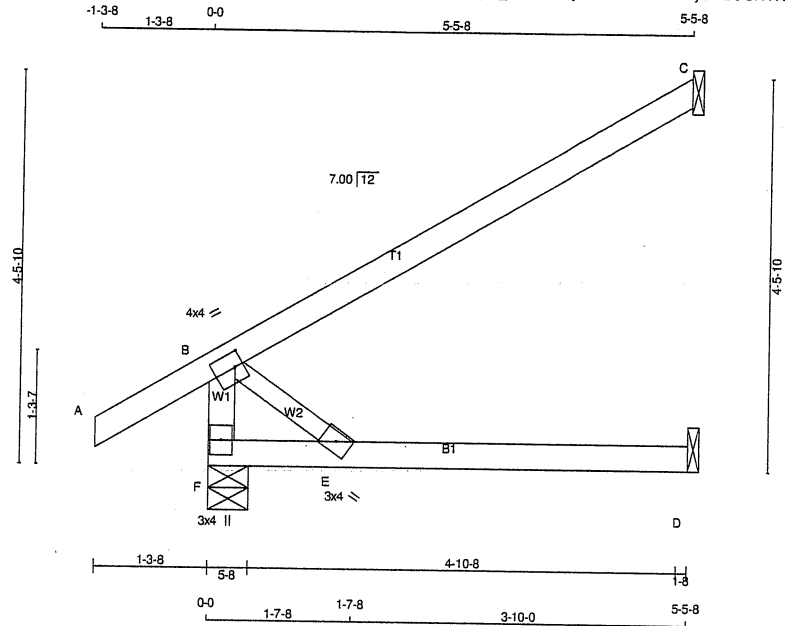


Structural component only
DWG# T-2126650

JOB NAME 420542	TRUSS NAME J3	QUANTITY 4	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1:25.0



LUMBER				TOTAL WEIGHT = 4 X 18 = 70 lb			
N. L. G. A. RULES							
CHORDS	SIZE	LUMBER	DESCR.				
F - B	2x4	DRY	No.2	SPF			
A - C	2x4	DRY	No.2	SPF			
F - D	2x4	DRY	No.2	SPF			
ALL WEBS	2x3	DRY	No.2	SPF			
DRY: SEASONED LUMBER.							

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B TMVW-t	MT20	4.0	4.0	2.00	1.25
E BMW+w	MT20	3.0	4.0		
F BMV1+p	MT20	3.0	4.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
F	398	0	398	0	5-8	5-8
C	232	0	232	0	1-8	1-8
D	50	0	57	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS		PERM. LIVE	WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE					
F	280	191 / 0	0 / 0	0 / 0	0 / 0	0 / 0	90 / 0	0 / 0
C	160	127 / 0	0 / 0	0 / 0	0 / 0	0 / 0	33 / 0	0 / 0
D	40	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	40 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		MAX. UNBRAC LENGTH	WEBS		MAX. FACTORED
	MAX. FACTORED	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX		MEMB. FORCE (LBS)	MAX	
FR-TO			FROM	TO				
F-B	-347 / 0		0.0	0.0	0.04 (1)	7.81	B-E	0 / 0
A-B	0 / 29		-84.9	-84.9	0.11 (1)	10.00		
B-C	0 / 0		-84.9	-84.9	0.43 (1)	10.00		
F-E	0 / 0		-18.5	-18.5	0.13 (4)	10.00		
E-D	0 / 0		-18.5	-18.5	0.16 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.43/1.00 (B-C:1), BC=0.16/1.00 (D-E:4), WB=0.00/1.00 (B-E:1), SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

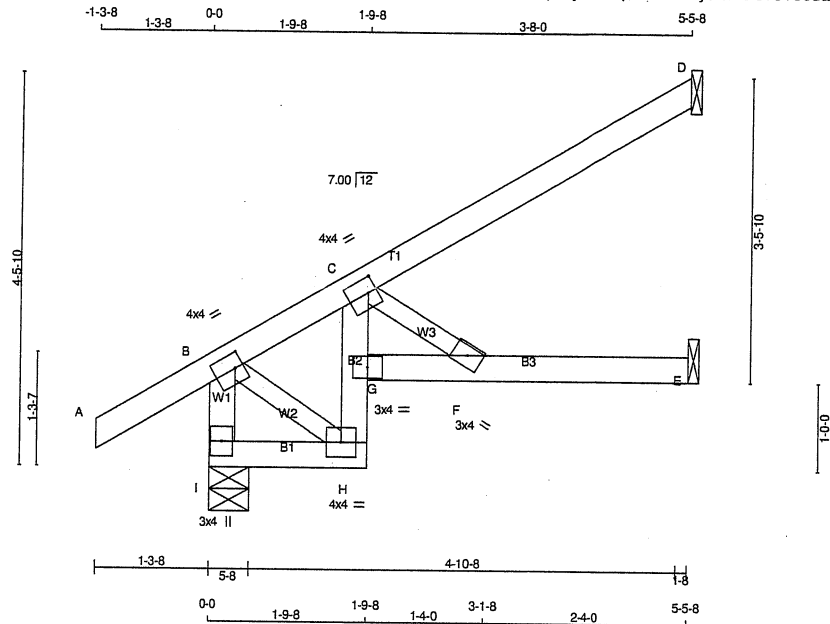
JSI GRIP = 0.21 (B) (INPUT = 0.90)
JSI METAL = 0.06 (B) (INPUT = 1.00)



Structural component only
DWG# T-2126665

JOB NAME 420547	TRUSS NAME J3S	QUANTITY 3	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1:25.0

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
I - B	2x4	DRY No.2	SPF
A - D	2x4	DRY No.2	SPF
I - H	2x4	DRY No.2	SPF
H - C	2x4	DRY No.2	SPF
G - E	2x4	DRY No.2	SPF
ALL WEBS	2x3	DRY No.2	SPF
DRY: SEASONED LUMBER.			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TMVW-t	MT20	4.0	4.0	2.00	1.25
F	BMVW-w	MT20	3.0	4.0		
G	BMVH-i	MT20	3.0	4.0		
H	BMVW-t	MT20	4.0	4.0		
I	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
I	412	0	412	0	5-8	5-8
D	164	0	164	0	1-8	1-8
E	104	0	104	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) D, E

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
I	290	199 / 0	0 / 0	0 / 0	0 / 0	92 / 0	0 / 0
D	114	86 / 0	0 / 0	0 / 0	0 / 0	28 / 0	0 / 0
E	76	33 / 0	0 / 0	0 / 0	0 / 0	43 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)		LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM	TO			FR-TO			
I-B	-397 / 0	0.0	0.0	0.04 (1)	7.81	B-H	0 / 157	0.04 (1)	
A-B	0 / 29	-84.9	-84.9	0.11 (1)	10.00	C-F	-346 / 0	0.05 (1)	
B-C	-183 / 0	-84.9	-84.9	0.11 (1)	6.25				
C-D	0 / 4	-84.9	-84.9	0.22 (1)	10.00				
I-H	0 / 0	-18.5	-18.5	0.01 (4)	10.00				
H-G	-64 / 0	0.0	0.0	0.18 (1)	7.81				
G-C	0 / 106	0.0	0.0	0.20 (1)	10.00				
G-F	0 / 279	-18.5	-18.5	0.32 (1)	10.00				
F-E	0 / 0	-18.5	-18.5	0.28 (1)	10.00				

TOTAL WEIGHT = 3 X 21 = 62 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = 1/788 (0.08")

CSI: TC=0.22/1.00 (C-D:1), BC=0.32/1.00 (F-G:1),
WB=0.05/1.00 (C-F:1), SS=0.13/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.28 (C) (INPUT = 0.90)
JSI METAL= 0.13 (F) (INPUT = 1.00)



Structural component only
DWG# T-2127361



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering tem and General Safety notes.

T-1900218

Feb 09, 2018

TOE-NAIL CAPACITY DETAILS

LATERAL AND WITHDRAWAL RESISTANCE OF BEARING ANCHORAGE BY TOE-NAILS

			SPF	D. FIR	SPF	D. FIR
COMMON WIRE	3.00	0.144	122	139	30	42
	3.25	0.144	127	144	32	45
	3.50	0.160	152	173	38	52
COMMON SPIRAL	3.00	0.122	96	108	26	36
	3.25	0.122	97	108	28	40
	3.50	0.152	142	161	36	50
3.25" Gun nail	3.25	0.120	94	105	28	39

Note: If using truss with D. Fir lumber and SPF bearing plate, use tabulated SPF values in table.

Nail type:	Common wire	Common spiral	Common wire	Common spiral	Gun Nail
Diameter (in.)	0.160	0.152	0.144	0.122	0.120
Length (in.)	3.50	3.50	3.00	3.00	3.25
MAXIMUM NUMBER OF TOE NAIL					
2x4 SPF	2	2	3	3	3
2x6 SPF	4	4	4	5	5
2x4 D. FIR	2	2	2	2	2
2x6 D. FIR	3	3	3	4	4

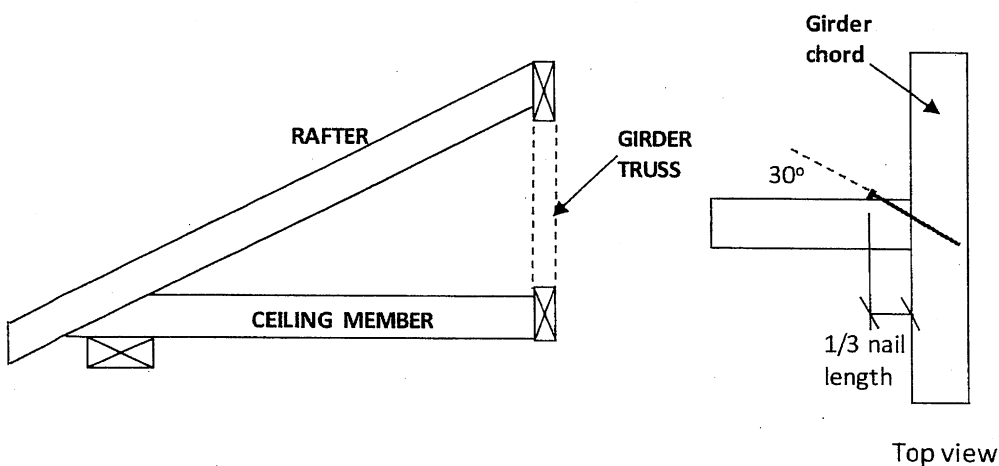


Figure 1: Toe-Nailing Rafter / Ceiling Member to Girder Truss

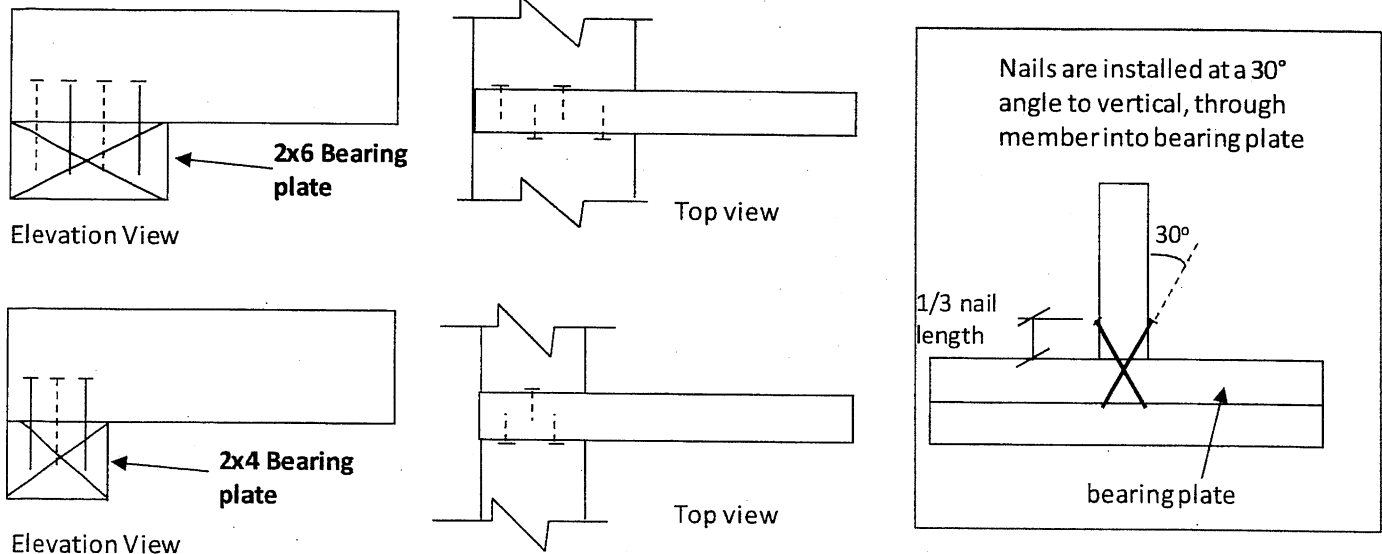
PEO
Certificate No. 10889485



December 21, 2020

TOE-NAIL CAPACITY DETAILS

Figure 2: Toe-Nail Anchorage to Bearing Plate for Uplift



NOTES:

1. Rafter and ceiling members may be connected to top and bottom chords of girder truss by toe-nailing the members into the girder chords (see fig. 1), provided the factored vertical reactions of the supported members do not exceed the lateral resistance of the toe-nails. Mechanical connectors (hangers) are required if factored vertical reactions exceed the toe-nail capacity, or if the connection must resist horizontal loads (loads perpendicular to the face of girder or rafter).
2. Trusses, rafters or ceiling members may be anchored to the bearing plate with toe-nails (see fig. 2), provided that the factored uplift reactions due to **wind or earthquake loads** do not exceed the **withdrawal resistance of the toe-nails**. Mechanical anchors (tie-downs) are required for reactions that exceed the toe-nail withdrawal capacity. Toe-nail anchorage to bearing plates is **NOT** permitted if uplift reactions are generated from gravity loads (snow, floor live, dead).
3. Tabulated toe-nail resistances on page 1 are for **one** toe-nail. Multiply unit values by the number of nails used in the connection. Maximum number of nails in a connection shall not exceed the tabulated limits shown on page 1 for a given lumber size /species.
4. Nail values are based on specific gravity of $G = 0.42$ (SPF) and $G = 0.49$ (D. Fir).
5. Toe-nails shall be driven at approximately $1/3$ the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member.
6. For wind / earthquake loads, tabulated lateral resistances may be multiplied by 1.15 (K_D factor). No increases are permitted for tabulated withdrawal resistances.
7. Lumber must be dry ($< 19\%$ moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-19, Clause 12.9.

PEO
Certificate No. 10889485



LUL/LUS/LJS/HUS/HHUS/HGUS

Standard and Double-Shear Joist Hangers



This product is preferable to similar connectors because of
a) easier installation, b) higher capacities, c) lower installed
cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 217–218.

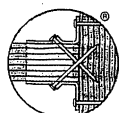
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 18–20.

Installation:

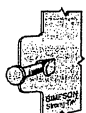
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

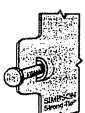
- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on pp. 105–107.



Double-Shear
Nailing
Top View

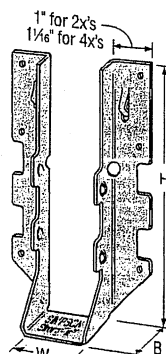
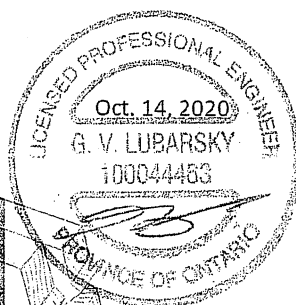
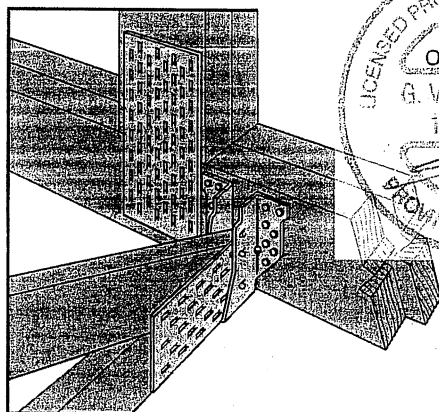


Double-Shear
Nailing
Side View;
Do not
bend tab

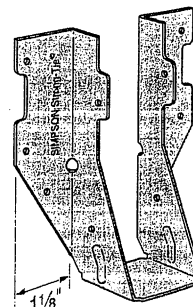


Dome Double-Shear
Nailing
Side View
(available on
some models)

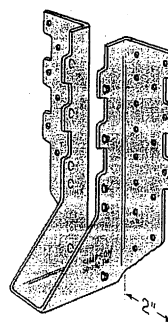
**Typical HUS26
Installation
with Reduced
Heel Height**
(Truss Designer
to provide
fastener quantity
for connecting
multiple members
together)



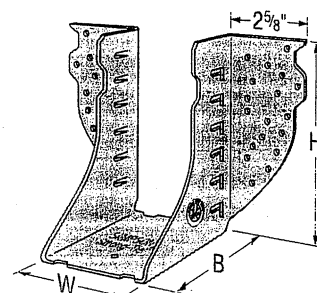
LUS28



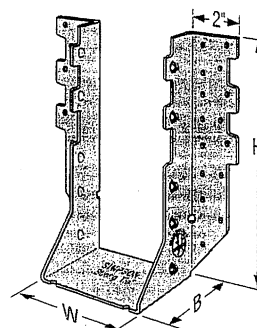
LU26L



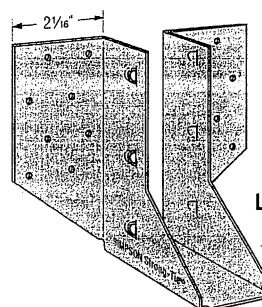
HUS210
(HUS26, HUS28,
and HHUS similar)



HGUS28-2



HHUS210-2



LJS26DS

TECHNICAL BULLETIN

LUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

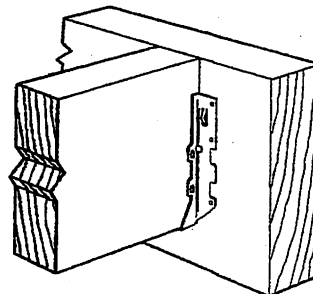
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

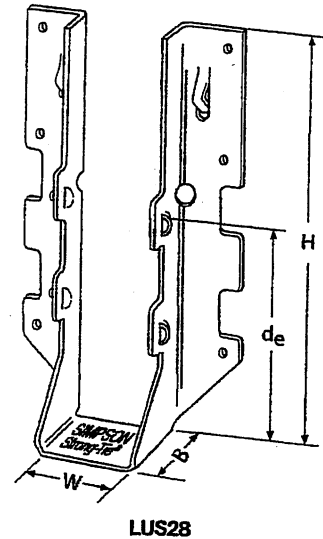
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire,
10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified

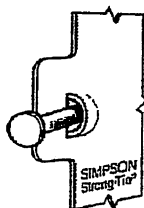


Typical LUS
Installation



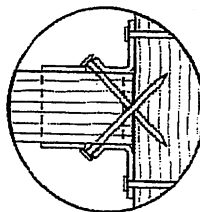
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _u =1.15)	Normal (K _p =1.00)	Uplift (K _u =1.15)	Normal (K _p =1.00)
LUS24	18	1⅞	3⅞	1¾	1⅝	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3⅞	3⅞	2	1⅝	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1⅞	4⅞	1¾	3⅞	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3⅞	4⅞	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4⅞	4⅞	2	3⅞	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1⅞	6⅞	1¾	3⅞	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3⅞	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4⅞	6⅞	2	3⅞	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1⅞	7⅞	1¾	3⅞	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3⅞	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4⅞	8⅞	2	5⅞	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.

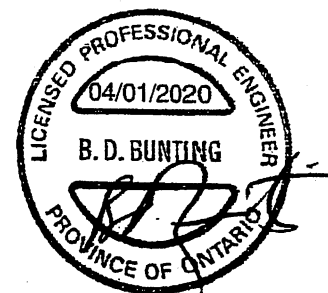


Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).

U.S. Patent
5,603,580



Double
Shear
Nailing
Top View.



**LIMIT
STATES
DESIGN**

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECLUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HUS/LJS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

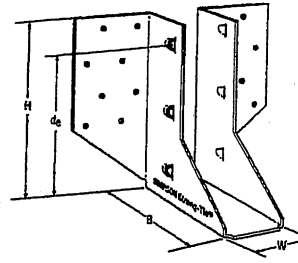
- Factored resistances are in accordance with CSA O86 -14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

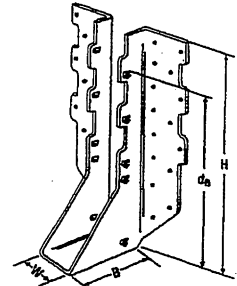
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

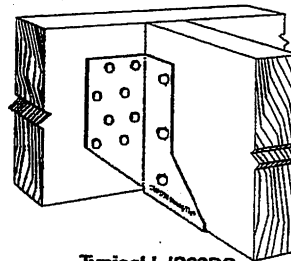
- See current catalogue for options



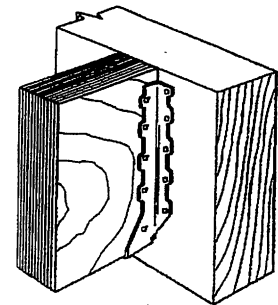
LJS26DS



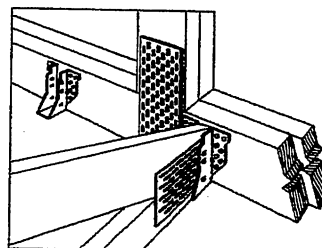
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



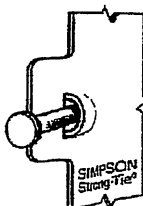
Typical HUS
Installation



Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

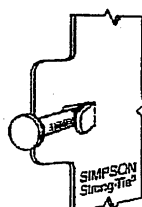
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e	Face	Joist	D-Fir-L		S-P-F	
								Uplift (K _u =1.15)	Normal (K _o =1.00)	Uplift (K _u =1.15)	Normal (K _o =1.00)
LJS26DS	18	1 1/16	5	3 1/2	4 5/8	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1 1/8	5 3/8	3	3 15/16	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1 1/8	7 3/8	3	6 3/32	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1 1/8	9 3/8	3	7 31/32	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1 1/16	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

1. d_e is the distance from the seat of the hanger to the highest joist nail.

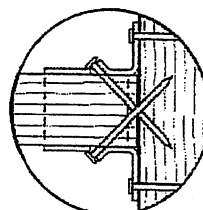


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

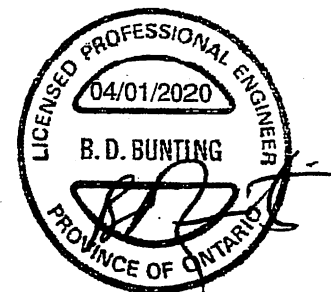
U.S. Patent
5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



**LIMIT
STATES
DESIGN**

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

© 2020 Simpson Strong-Tie Company Inc.

T-SPECHUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

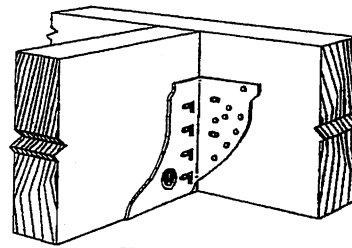
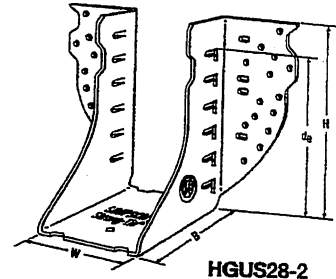
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

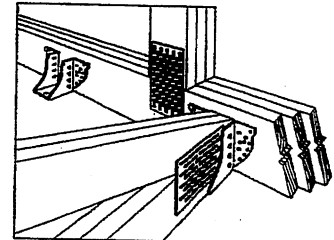
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

- See current catalogue for options



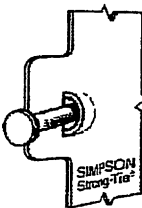
Typical HGUS Installation



Typical HGUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

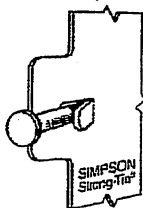
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _g ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _g =1.15)	Normal (K _g =1.00)	Uplift (K _g =1.15)	Normal (K _g =1.00)
HGUS26	12	1½	5½	5	4½	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3½	5½	4	4½	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4½	5½	4	4½	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6½	5½	4	4½	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1½	7½	5	6½	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3½	7½	4	6½	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4½	7½	4	6½	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6½	7½	4	6½	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210	12	1½	9½	5	7½	(46) 16d	(16) 16d	3535	11070	2510	8090
HGUS210-2	12	3½	9½	4	8½	(46) 16d	(16) 16d	6840	14015	4855	10270
HGUS210-3	12	4½	9½	4	8½	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6½	9½	4	8½	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6½	10½	4	10½	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6½	12½	4	11½	(66) 16d	(22) 16d	10130	16400	7195	11645

1. d_g is the distance from the seat of the hanger to the highest joist nail.

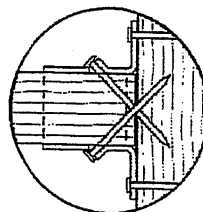


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

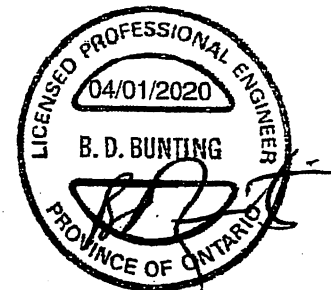
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

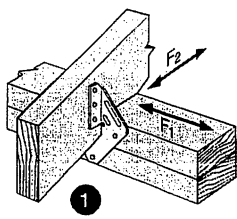
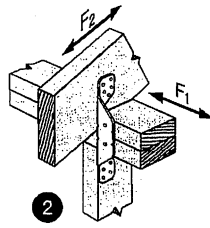
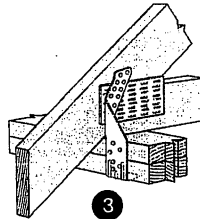
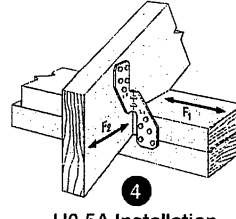
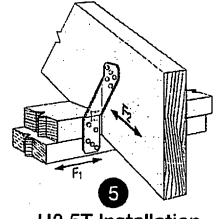
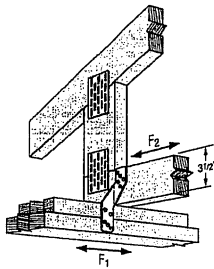
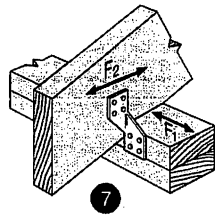
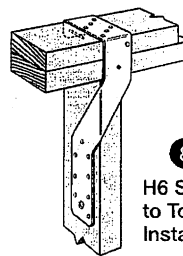
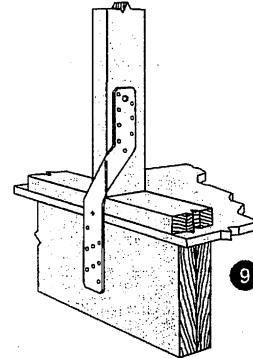
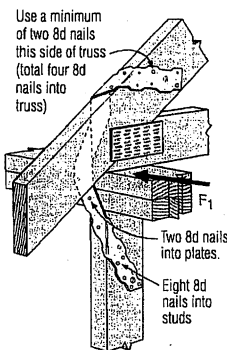
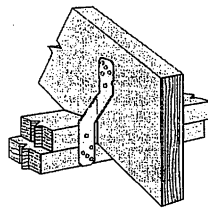
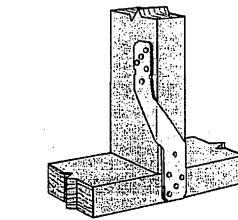
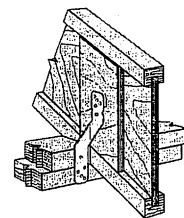
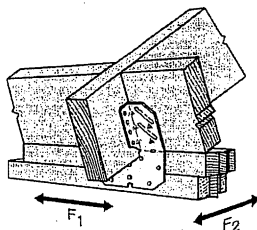
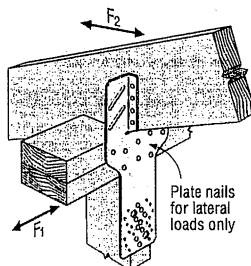
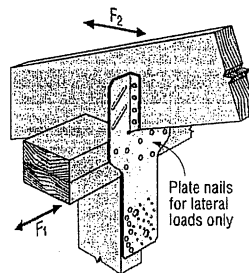
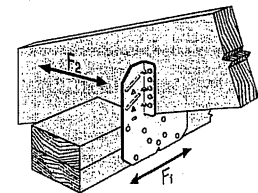
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T-SPECHGUS20 3/20 exp. 6/22

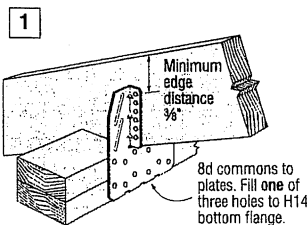
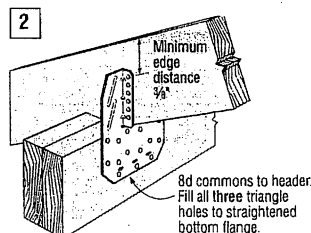
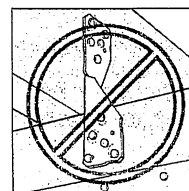
(800) 999-5099
strongtie.com

H/TSP

Seismic and Hurricane Ties (cont.)

**1** H1 Installation**2** H2A Installation**3** TSP Installation**4** H2.5A Installation
(Nails into both top plates)**5** H2.5T Installation
(Nails into both top plates)**6** H2.5T Installation**7** H3 Installation
(Nails into upper top plate)**8** H6 Stud to Top Plate Installation**9** H6 Stud to Band Joist Installation**10** H7Z Installation**11** H8 Attaching Rafter to Double Top Plates**12** H8 attaching Stud to Sill
((4) 8d into plate, (5) 8d into stud)**13** H8 attaching I-Joist to Double Top Plates**14** H10A Field-Bent Installation**15** H10S Installation**16** H10S Installation with Stud Offset**17** H10A Installation

H10A optional positive angle nailing connects shear blocking to rafter. Use 8d common nails. Slot allows maximum field-bending up to a pitch of 6/12, use 75% of the table uplift value; bend one time only.

**18** H14 Installation to Double Top Plates**19** H14 Installation to Double 2x Header**Avoid a Misinstallation**

Do not make new holes or overdrive nails.

H/TSP

Seismic and Hurricane Ties

Simpson Strong-Tie® hurricane ties provide a positive connection between truss/rafter and the wall of the structure to resist wind and seismic forces. New additions to the line provide even more options.

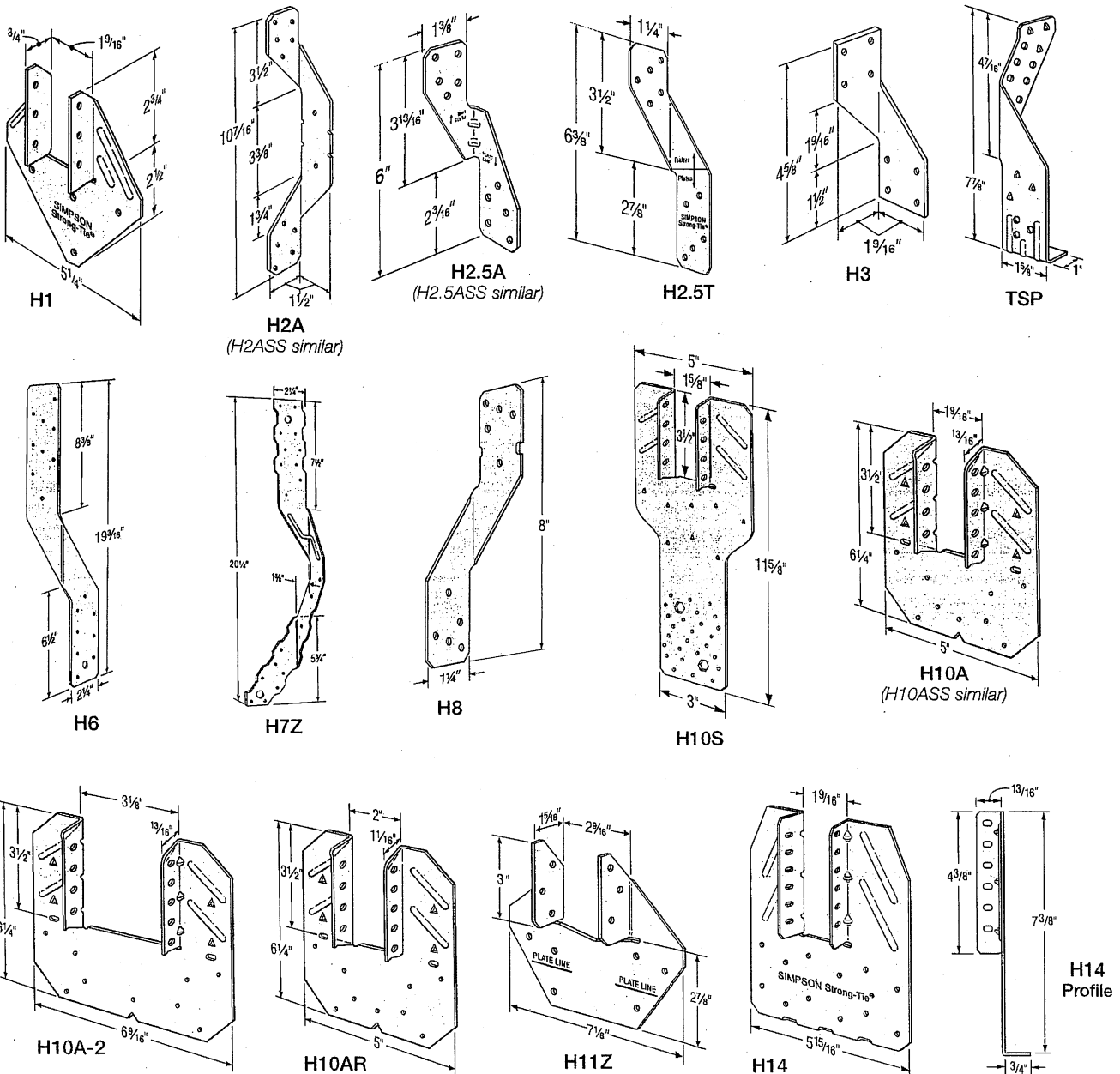
- H10AR — The heavy-duty design of the H10A available with a 2" wide throat to accommodate rough lumber
- H10A-2 — The H10A design with a 3" throat for double 2x members
- H2ASS, H2.5ASS and H10ASS — Popular ties now available in stainless steel

Material: See table

Finish: Galvanized. H7Z and H11Z — ZMAX® coating. Some models available in stainless steel or ZMAX; see Corrosion Information, pp. 20–24 or visit strongtie.com.

Installation:

- Use all specified fasteners; see General Notes.
- H1 can be installed with flanges facing inward (reverse of H1 installation drawing; number 1).
- H2.5T, H3 and H6 ties are shipped in equal quantities of right and left versions (right versions shown).
- Hurricane ties do not replace solid blocking.
- When installing ties on plated trusses (on the side opposite the truss plate) do not fasten through the truss plate from behind. This can force the truss plate off of the truss and compromise truss performance.
- H10A optional nailing to connect shear blocking, use 8d nails. Slots allow maximum field bending up to a pitch of 6:12, use H10A sloped loads for field bent installation.



TECHNICAL BULLETIN

H – Seismic and Hurricane Ties

SIMPSON
Strong-Tie

The H connector series provides wind and seismic ties for trusses and rafters.

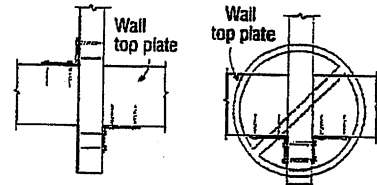
Material: 18 gauge **Finish:** G90 galvanized

Design: • Factored resistances are in accordance with CSA O86-14
• Factored resistances have been increased 15%. No further increase is permitted.

Installation: • Use all specified fasteners
• Nails: 8d = 0.131" dia. x 2½" long common wire, 8d x 1½" = 0.131" x 1½" long, 10d x 1½" = 0.146" x 1½" long
• H1 can be installed with flanges facing outwards
• Hurricane ties do not replace solid blocking

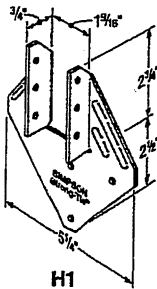
Factored resistances for more than one direction for a single connection cannot be added together. A factored load which can be divided into components in the directions given must be evaluated as follows: Factored Shear/Resisting Shear + Factored Tension/Resisting Tension ≤ 1.0.

Hurricane Tie Installations to Achieve Twice the Load (Top View)

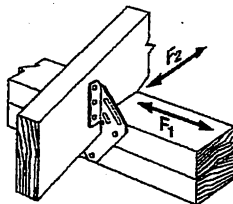


Install diagonally across from each other for minimum 2x truss.

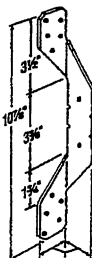
Nailing into both sides of a single ply 2x truss may cause the wood to split.



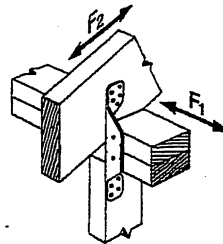
H1



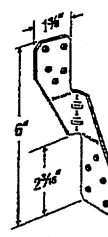
H1 Installation



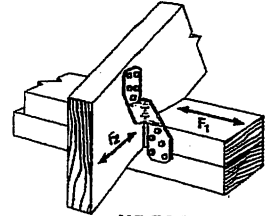
H2A



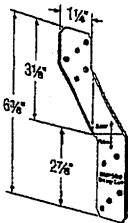
H2A Installation



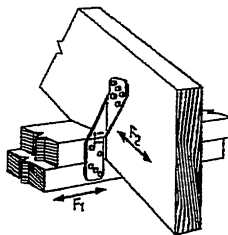
H2.5A



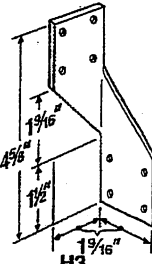
H2.5A Installation



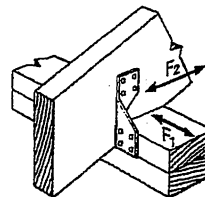
H2.5T



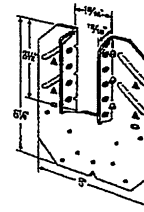
H2.5T Installation
(Nails into both top plates)



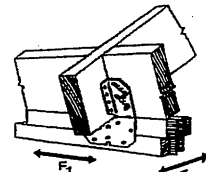
H3



H3 Installation



H10A



H10A Installation

Model No.	Ga.	Fasteners			Factored Resistance (lb.)					
					D.Fir-L			S-P-F		
		To Rafter	To Plates	To Studs	Uplift	Normal		Uplift	Normal	
						F ₁	F ₂		F ₁	F ₂
					(K _p =1.15)					(K _p =1.15)
H1	18	(6) 8d x 1½"	(4) 8d	—	740	685	300	680	485	215
H2A	18	(5) 8d x 1½"	(2) 8d x 1½"	(5) 8d x 1½"	830	220	75	590	155	55
H2.5A	18	(5) 8d	(5) 8d	—	805	160	160	755	160	160
H2.5T	18	(5) 8d	(5) 8d	—	835	175	240	740	160	210
H3	18	(4) 8d	(4) 8d	—	740	180	265	615	125	190
H10A	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1735	795	410	1505	565	290

1. Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.

2. Factored resistances are for one anchor. A minimum after thickness of 2½" must be used when framing anchors are installed on each side of the joist and on the same side of the plate.

3. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.

4. Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a Continuous Load Path, connections must be on same side of the wall.

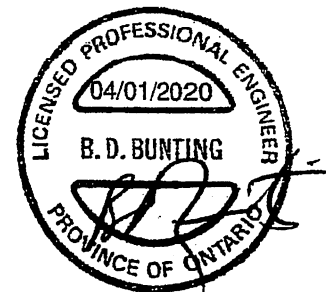


**LIMIT
STATES
DESIGN**

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECH20 3/20 exp. 6/22



(800) 999-5099
strongtie.com



ONTARIO WOOD TRUSS
FABRICATORS ASSOCIATION

TECH-NOTES

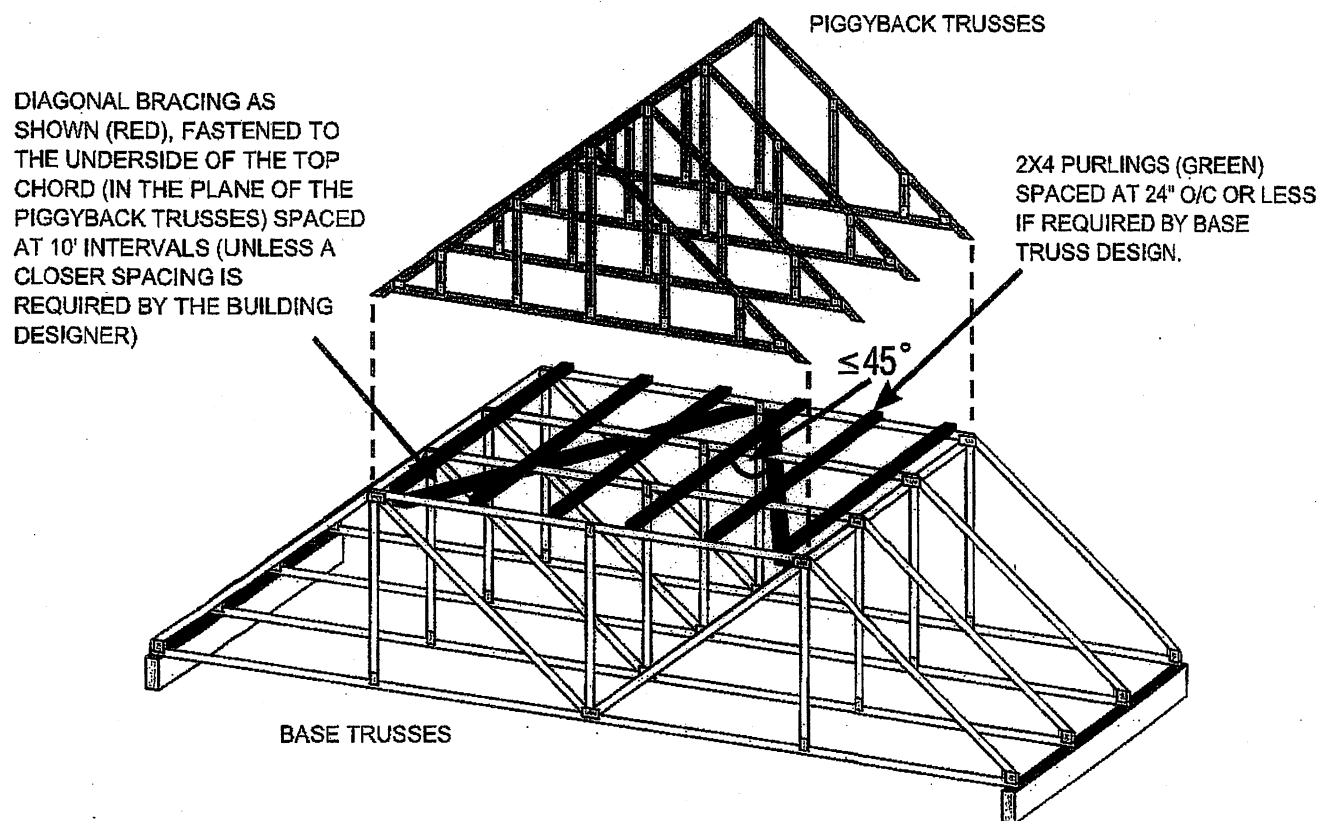
TN 15-001 Piggyback Bracing

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

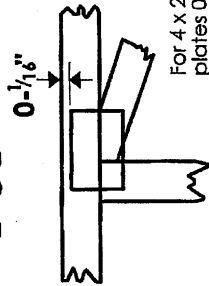
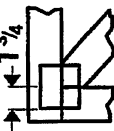
Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

Symbols

PLATE LOCATION AND ORIENTATION

Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

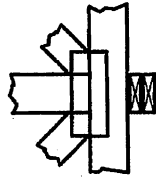
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

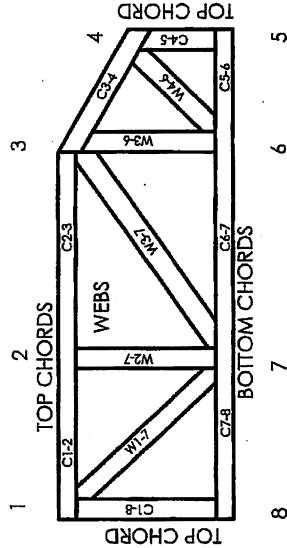


Industry Standards:

- TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
- DSB-89: Design Standard for Bracing
- BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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MiTek
POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MI-7473C rev. 10-'08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.