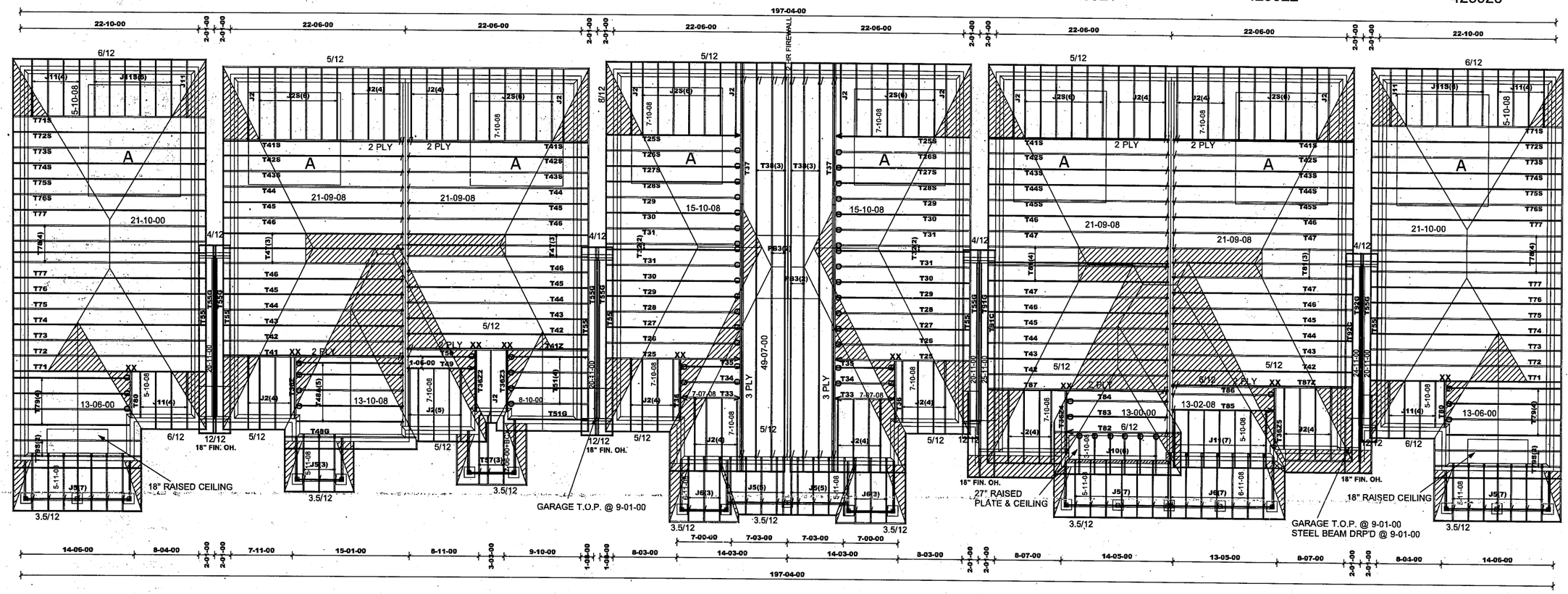
1	2	3	4	5	6	7	8
2504/ EL.A 425916	2501/ EL.A1 425917	2501/ EL.A2 425918	2502/ EL.A1 425919	2502/ EL.A2 425920	2503/ EL.A1 425921	2503/ EL.A2 425922	2504/ EL.A 425923



ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD
6" DEDUCTION
(1" RIGID INSULATION)

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)
H2.5T - (/)

DENOTES CONV. FRAMING

DESIGN CONFORMS WITH OBC 2012 (2019 AMENDMENT)
OCCUPANCY: RESIDENTIAL PART: 9
Ss = 27.2 psf Sr = 8.4 psf

DESIGN LOADS:
TCSL = 23.3 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

All conventional framing to conform with Part 9 of O.B.C. 2012. Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

ALL ROOF PITCHES ARE 10/12 UNLESS OTHERWISE NOTED

A: 12" RAISED COFF. CEILING

DWG# T-2406298 to T-2406299,
T-2406302, T-2406303, T-2406306,
T-2406330 to T-2406369, T-2406372,
T-2406478 to T-2406513, T-1800218

LICENSED PROFESSIONAL ENGINEER
03-02-24
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2406551

TRUSS PLACEMENT PLAN.

This is a truss placement plan only, NOT a final roof framing plan. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual truss drawings for each component identified on this placement plan.

Please verify that all dimensions match the dimensions found on the job. The Building designer is responsible for the temporary/ permanent bracing of the roof and floor system and its integration into the bracing of the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer unless otherwise noted in this plan. Building designer to review and approve this plan to ascertain conformity to his overall structural plan.

M16022

TAMARACK ROOF TRUSSES INC.

Job Track: 52749	Builder / Location: ROYAL PINE HOMES / BRAMPTON
Layout ID: 425915	Project: SUMMER RIDGE ESTATES INC.
Plan Log: 206083	Date: 2024-03-01 Designer:

Model / Elevation: BLK 292 / UNITS 1-8 (COFF. IN MASTER BED)
Mitek ver 8.6.3.353

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52749
Builder:	ROYAL PINE HOMES	PlanLog:	206083
Project:	SUMMER RIDGE ESTATES INC.	Layout ID:	425916
Location:	BRAMPTON	Ref #	
Model:	BLK 292	Page:	1 of 2
Lot #:		Date:	02-29-2024
Elevation:	A UNIT 1-BLK 292	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

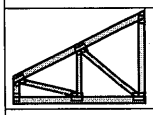
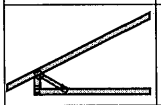
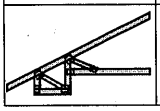
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	T55 Roof Special	4 /12	**20-11-00**	6-04-03	2 x 4	1-03-08 1-09-08	4-10 1-09-15	78.88 50.33		
	1	T55G GABLE	4 /12	**20-11-00**	6-04-03	2 x 4	1-03-08 1-09-08	4-10 1-09-15	81.17 52.17		
	1	T71 Hip Girder	10 /12	**21-10-00**	4-02-03	2 x 6	1-04-08 1-04-08	1-08-08 1-08-08	122.43 74.00		
	1	T71S Hip Girder	10 /12	**21-10-00**	4-02-03	2 x 6	1-04-08 1-04-08	1-08-08 1-08-08	130.3 82.83		
	1	T72 Hip	10 /12	**21-10-00**	5-01-12	2 x 4	1-04-08 1-04-08	1-08-08 1-08-08	90.83 57.83		
	1	T72S Hip	10 /12	**21-10-00**	5-01-12	2 x 4	1-04-08 1-04-08	1-08-08 1-08-08	103.97 67.83		
	1	T73 Hip	10 /12	**21-10-00**	6-01-12	2 x 4	1-04-08 1-04-08	1-08-08 1-08-08	95.56 60.33		
	1	T73S Hip	10 /12	**21-10-00**	6-01-12	2 x 4	1-04-08 1-04-08	1-08-08 1-08-08	109.38 71.33		
	1	T74 Hip	10 /12	**21-10-00**	7-01-12	2 x 4	1-04-08 1-04-08	1-08-08 1-08-08	109.55 68.17		
	1	T74S Hip	10 /12	**21-10-00**	7-01-12	2 x 4	1-04-08 1-04-08	1-08-08 1-08-08	112.24 71.33		
	1	T75 Hip	10 /12	**21-10-00**	8-01-12	2 x 4	1-04-08 1-04-08	1-08-08 1-08-08	104.92 66.33		
	1	T75S Hip	10 /12	**21-10-00**	8-01-12	2 x 4	1-04-08 1-04-08	1-08-08 1-08-08	109.84 71.33		
	1	T76 Hip	10 /12	**21-10-00**	9-01-12	2 x 4	1-04-08 1-04-08	1-08-08 1-08-08	110.74 69.67		
	1	T76S Hip	10 /12	**21-10-00**	9-01-12	2 x 4	1-04-08 1-04-08	1-08-08 1-08-08	123.92 78.67		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52749
Builder:	ROYAL PINE HOMES	PlanLog:	206083
Project:	SUMMER RIDGE ESTATES INC.	Layout ID:	425916
Location:	BRAMPTON	Ref #	
Model:	BLK 292	Page:	2 of 2
Lot #:		Date:	02-29-2024
Elevation:	A UNIT 1-BLK 292	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	2	T77 Hip	10 /12	21-10-00	10-01-12	2 x 4	1-04-08 1-04-08	1-08-08 1-08-08	232.98 148.00		
	4	T78 Common	10 /12	21-10-00	10-09-11	2 x 4	1-04-08 1-04-08	1-08-08 1-08-08	455.58 280.00		
	4	T79 Common	10 /12	13-06-00	7-04-00	2 x 4	1-04-08	1-08-08 1-08-08	250.76 162.00		
	2	T79S Roof Special	10 /12	13-06-00	7-04-00	2 x 4	1-04-08 1-04-08	1-08-08 1-08-08	142.58 92.00		
	1 2-ply	T80 Monopitch Girder	6 /12	5-10-08	4-01-12	2 x 4		1-02-08 4-01-12	51.57 34.00		
	7	J5 Jack-Open	3.5 /12	5-11-08	2-05-00	2 x 4	1-03-08	3-14 2-00-12	108.52 65.33		
	9	J11 Jack-Open	6 /12	5-10-08	4-01-12	2 x 4	1-04-08	1-02-08 4-01-12	164.16 105.00		
	6	J11S Jack-Open	6 /12	5-10-08	4-01-12	2 x 4	1-04-08	1-02-08 3-01-12	126.71 88.00		

TOTAL # TRUSS= **50** TOTAL BFT OF ALL TRUSSES= **1916.48** BFT. TOTAL WEIGHT OF ALL TRSSES **3016.58** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
7	Hardware	H2.5T	
1	Hardware	HGUS26-2	
3	Hardware	LUS24	

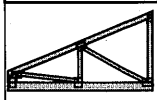
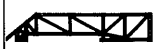
TOTAL NUMBER OF ITEMS= **11**

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52749
Builder:	ROYAL PINE HOMES	PlanLog:	206083
Project:	SUMMER RIDGE ESTATES INC.	Layout ID:	425917
Location:	BRAMPTON	Ref #	
Model:	BLK 292	Page:	1 of 2
Lot #:		Date:	02-29-2024
Elevation:	A1 UNIT 2-BLK 292	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

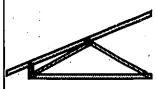
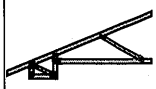
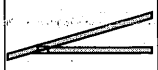
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T36Z Monopitch Girder	5 /12	7-10-08	4-04-06	2 x 4 2 x 6		1-01-00 4-04-06	73.36 46.33		
	1 2-ply	T41 Half Hip Girder	10 /12	21-09-08	4-04-06	2 x 4 2 x 6	1-04-08	1-08-08 4-04-06	217.25 134.67		
	1 2-ply	T41S Half Hip Girder	10 /12	21-09-08	4-04-06	2 x 4	1-04-08	1-08-08 4-04-06	208.58 134.67		
	1	T42 Half Hip	10 /12	21-09-08	5-02-06	2 x 4	1-04-08	1-08-08 5-02-06	92.45 58.67		
	1	T42S Half Hip	10 /12	21-09-08	5-02-06	2 x 4	1-04-08	1-08-08 5-02-06	105.87 67.83		
	1	T43 Half Hip	10 /12	21-09-08	6-00-06	2 x 4	1-04-08	1-08-08 6-00-06	97.36 61.33		
	1	T43S Half Hip	10 /12	21-09-08	6-00-06	2 x 4	1-04-08	1-08-08 6-00-06	112.24 73.17		
	2	T44 Half Hip	10 /12	21-09-08	6-10-06	2 x 4	1-04-08	1-08-08 6-10-06	213.4 134.00		
	2	T45 Half Hip	10 /12	21-09-08	7-08-06	2 x 4	1-04-08	1-08-08 7-08-06	204.45 128.67		
	2	T46 Half Hip	10 /12	21-09-08	8-06-06	2 x 4	1-04-08	1-08-08 8-06-06	227.19 141.00		
	3	T47 Half Hip	10 /12	21-09-08	9-04-06	2 x 4	1-04-08	1-08-08 9-04-06	355.03 218.00		
	5	T48A Common	10 /12	13-10-08	7-06-15	2 x 4	1-04-08	1-08-08 1-10-09	322.85 210.83		
	1	T48G GABLE	10 /12	14-01-00	7-06-15	2 x 4	1-04-08 1-04-08	1-08-08 1-08-08	70.4 46.17		
	1	T55 Roof Special	4 /12	20-11-00	6-04-03	2 x 4	1-03-08 1-09-08	4-10 1-09-15	78.88 50.33		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52749
Builder:	ROYAL PINE HOMES	PlanLog:	206083
Project:	SUMMER RIDGE ESTATES INC.	Layout ID:	425917
Location:	BRAMPTON	Ref #	
Model:	BLK 292	Page:	2 of 2
Lot #:		Date:	02-29-2024
Elevation:	A1 UNIT 2-BLK 292	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	T55G GABLE	4 /12	20-11-00	6-04-03	2 x 4	1-03-08 1-09-08	4-10 1-09-15	81.17 52.17		
	9	J2 Jack-Partial	5 /12	7-10-08	4-04-06	2 x 4	1-04-08	1-01-00 4-04-06	254.42 165.00		
	6	J2S Jack-Partial	5 /12	7-10-08	4-04-06	2 x 4	1-04-08	1-01-00 3-04-06	157.83 110.00		
	3	J5 Jack-Open	3.5 /12	5-11-08	2-05-00	2 x 4	1-03-08	3-14 2-00-12	46.51 28.00		

TOTAL # TRUSS= **45** TOTAL BFT OF ALL TRUSSES= **1860.84** BFT. TOTAL WEIGHT OF ALL TRSSES **2919.24** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
25	Hardware	H2.5T	
1	Hardware	HGUS26-2	
4	Hardware	LUS24	

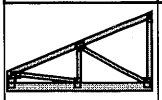
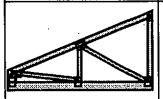
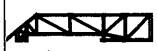
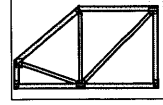
TOTAL NUMBER OF ITEMS= **30**

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52749
Builder:	ROYAL PINE HOMES	PlanLog:	206083
Project:	SUMMER RIDGE ESTATES INC.	Layout ID:	425918
Location:	BRAMPTON	Ref #	
Model:	BLK 292	Page:	1 of 2
Lot #:		Date:	02-29-2024
Elevation:	A2 UNIT 3-BLK 292	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

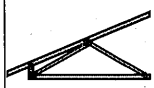
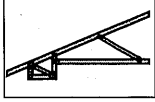
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T36Z2 Monopitch Girder	5 /12	7-10-08	4-04-06	2 x 4 2 x 6		1-01-00 4-04-06	73.36 46.33		
	1 2-ply	T36Z3 Monopitch Girder	5 /12	7-10-08	4-04-06	2 x 4 2 x 6		1-01-00 4-04-06	73.36 46.33		
	1 2-ply	T41S Half Hip Girder	10 /12	21-09-08	4-04-06	2 x 4	1-04-08	1-08-08 4-04-06	208.58 134.67		
	1 2-ply	T41Z Half Hip Girder	10 /12	21-09-08	4-04-06	2 x 4 2 x 6	1-04-08	1-08-08 4-04-06	220.84 135.67		
	1	T42 Half Hip	10 /12	21-09-08	5-02-06	2 x 4	1-04-08	1-08-08 5-02-06	92.45 58.67		
	1	T42S Half Hip	10 /12	21-09-08	5-02-06	2 x 4	1-04-08	1-08-08 5-02-06	105.87 67.83		
	1	T43 Half Hip	10 /12	21-09-08	6-00-06	2 x 4	1-04-08	1-08-08 6-00-06	97.36 61.33		
	1	T43S Half Hip	10 /12	21-09-08	6-00-06	2 x 4	1-04-08	1-08-08 6-00-06	112.24 73.17		
	2	T44 Half Hip	10 /12	21-09-08	6-10-06	2 x 4	1-04-08	1-08-08 6-10-06	213.4 134.00		
	2	T45 Half Hip	10 /12	21-09-08	7-08-06	2 x 4	1-04-08	1-08-08 7-08-06	204.45 128.67		
	2	T46 Half Hip	10 /12	21-09-08	8-06-06	2 x 4	1-04-08	1-08-08 8-06-06	227.19 141.00		
	3	T47 Half Hip	10 /12	21-09-08	9-04-06	2 x 4	1-04-08	1-08-08 9-04-06	355.03 218.00		
	1	T49 Half Hip Girder	10 /12	8-08-08	4-04-06	2 x 4		1-08-08 4-04-06	44.19 29.00		
	1	T50 Half Hip	10 /12	8-08-08	4-11-14	2 x 4		1-08-08 4-11-14	39.71 26.00		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52749
Builder:	ROYAL PINE HOMES	PlanLog:	206083
Project:	SUMMER RIDGE ESTATES INC.	Layout ID:	425918
Location:	BRAMPTON	Ref #	
Model:	BLK 292	Page:	2 of 2
Lot #:		Date:	02-29-2024
Elevation:	A2 UNIT 3-BLK 292	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	4	T51 Common	10 /12	8-10-00	5-04-11	2 x 4	1-04-08	1-08-08 1-08-08	157.66 102.00		
	1	T51G GABLE	10 /12	8-10-00	5-04-11	2 x 4	1-04-08 1-04-08	1-08-08 1-08-08	43.59 28.83		
	1	T55 Roof Special	4 /12	20-11-00	6-04-03	2 x 4	1-03-08 1-09-08	4-10 1-09-15	78.88 50.33		
	1	T55G GABLE	4 /12	20-11-00	6-04-03	2 x 4	1-03-08 1-09-08	4-10 1-09-15	81.17 52.17		
	3	T57 Monopitch	3.5 /12	7-06-00	2-10-07	2 x 4	1-03-08	3-14 2-06-02	79.84 51.50		
	11	J2 Jack-Partial	5 /12	7-10-08	4-04-06	2 x 4	1-04-08	1-01-00 4-04-06	310.96 201.67		
	6	J2S Jack-Partial	5 /12	7-10-08	4-04-06	2 x 4	1-04-08	1-01-00 3-04-06	157.83 110.00		

TOTAL # TRUSS= **50** TOTAL BFT OF ALL TRUSSES= **1897.17** BFT. TOTAL WEIGHT OF ALL TRSSES **2977.95** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
25	Hardware	H2.5T	
2	Hardware	HGUS26-2	
1	Hardware	LJS26DS	
5	Hardware	LUS24	

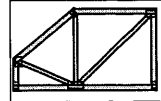
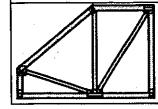
TOTAL NUMBER OF ITEMS= **33**

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52749
Builder:	ROYAL PINE HOMES	PlanLog:	206083
Project:	SUMMER RIDGE ESTATES INC.	Layout ID:	425919
Location:	BRAMPTON	Ref #	
Model:	BLK 292	Page:	1 of 2
Lot #:		Date:	02-29-2024
Elevation:	A1 UNIT 4-BLK 292	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

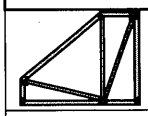
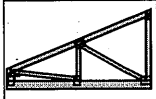
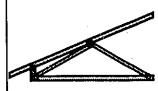
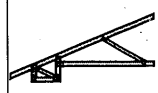
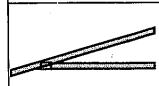
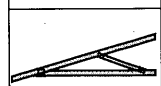
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	T25 Half Hip Girder	10 /12	15-10-08	4-04-06	2 x 4 2 x 6	1-04-08	1-08-08 4-04-06	82.93 52.33		
	1	T25S Half Hip Girder	10 /12	15-10-08	4-04-06	2 x 4	1-04-08	1-08-08 4-04-06	84.77 55.67		
	1	T26 Half Hip	10 /12	15-10-08	5-02-06	2 x 4	1-04-08	1-08-08 5-02-06	69.22 44.17		
	1	T26S Half Hip	10 /12	15-10-08	5-02-06	2 x 4	1-04-08	1-08-08 5-02-06	81.19 52.50		
	1	T27 Half Hip	10 /12	15-10-08	6-00-06	2 x 4	1-04-08	1-08-08 6-00-06	73.01 46.33		
	1	T27S Half Hip	10 /12	15-10-08	6-00-06	2 x 4	1-04-08	1-08-08 6-00-06	86.47 56.83		
	1	T28 Half Hip	10 /12	15-10-08	6-10-06	2 x 4	1-04-08	1-08-08 6-10-06	81.21 51.50		
	1	T28S Half Hip	10 /12	15-10-08	6-10-06	2 x 4	1-04-08	1-08-08 6-10-06	87.83 57.83		
	2	T29 Half Hip	10 /12	15-10-08	7-08-06	2 x 4	1-04-08	1-08-08 7-08-06	171.64 110.67		
	2	T30 Half Hip	10 /12	15-10-08	8-06-06	2 x 4	1-04-08	1-08-08 8-06-06	168.99 105.67		
	2	T31 Half Hip	10 /12	15-10-08	9-04-06	2 x 4	1-04-08	1-08-08 9-04-06	177.35 111.00		
	2	T32 Half Hip	10 /12	15-10-08	10-02-06	2 x 4	1-04-08	1-08-08 10-02-06	178.39 110.33		
	1	T33 Half Hip Girder	10 /12	7-07-08	4-04-06	2 x 4		1-08-08 4-04-06	34.64 23.00		
	1	T34 Half Hip	10 /12	7-07-08	5-02-06	2 x 4		1-08-08 5-02-06	37.38 24.67		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52749
Builder:	ROYAL PINE HOMES	PlanLog:	206083
Project:	SUMMER RIDGE ESTATES INC.	Layout ID:	425919
Location:	BRAMPTON	Ref #	
Model:	BLK 292	Page:	2 of 2
Lot #:		Date:	02-29-2024
Elevation:	A1 UNIT 4-BLK 292	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	T35 Half Hip	10 /12	7-07-08	6-00-06	2 x 4		1-08-08 6-00-06	40.39 26.33		
	1 2-ply	T36 Monopitch Girder	5 /12	7-10-08	4-04-06	2 x 4 2 x 6		1-01-00 4-04-06	73.36 46.33		
	1 3-ply	T37 Hip Girder	5 /12	49-07-00	8-00-12	2 x 6	1-04-08 1-04-08	1-01-00 1-01-00	1021.52 611.00		
	3	T38 Hip	5 /12	49-07-00	9-08-12	2 x 6	1-04-08 1-04-08	1-01-00 1-01-00	949.91 568.00		
	1	T55 Roof Special	4 /12	20-11-00	6-04-03	2 x 4	1-03-08 1-09-08	4-10 1-09-15	78.88 50.33		
	1	T55G GABLE	4 /12	20-11-00	6-04-03	2 x 4	1-03-08 1-09-08	4-10 1-09-15	81.17 52.17		
	2	PB3 Piggyback	5 /12	8-01-00	1-08-03	2 x 4			34.69 22.33		
	10	J2 Jack-Partial	5 /12	7-10-08	4-04-06	2 x 4	1-04-08	1-01-00 4-04-06	282.69 183.33		
	6	J2S Jack-Partial	5 /12	7-10-08	4-04-06	2 x 4	1-04-08	1-01-00 3-04-06	157.83 110.00		
	5	J5 Jack-Open	3.5 /12	5-11-08	2-05-00	2 x 4	1-03-08	3-14 2-00-12	77.51 46.67		
	3	J6 Jack-Partial	3.5 /12	6-11-08	2-08-08	2 x 4	1-03-08	3-14 2-04-04	60.66 36.50		

TOTAL # TRUSS= **55** TOTAL BFT OF ALL TRUSSES= **2655.49** BFT. TOTAL WEIGHT OF ALL TRSSES **4273.63** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
24	Hardware	H2.5T	
1	Hardware	HGUS26-2	
4	Hardware	LJS26DS	
18	Hardware	LUS24	

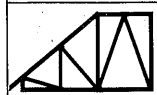
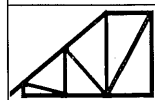
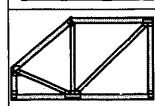
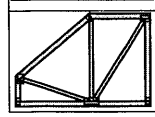
TOTAL NUMBER OF ITEMS= **47**

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52749
Builder:	ROYAL PINE HOMES	PlanLog:	206083
Project:	SUMMER RIDGE ESTATES INC.	Layout ID:	425920
Location:	BRAMPTON	Ref #	
Model:	BLK 292	Page:	1 of 2
Lot #:		Date:	02-29-2024
Elevation:	A2 UNIT 5-BLK 292	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

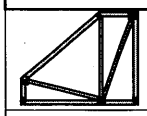
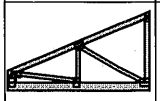
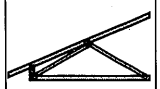
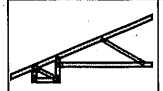
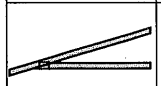
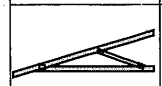
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	T25 Half Hip Girder	10 /12	15-10-08	4-04-06	2 x 4 2 x 6	1-04-08	1-08-08 4-04-06	82.93 52.33		
	1	T25S Half Hip Girder	10 /12	15-10-08	4-04-06	2 x 4	1-04-08	1-08-08 4-04-06	84.77 55.67		
	1	T26 Half Hip	10 /12	15-10-08	5-02-06	2 x 4	1-04-08	1-08-08 5-02-06	69.22 44.17		
	1	T26S Half Hip	10 /12	15-10-08	5-02-06	2 x 4	1-04-08	1-08-08 5-02-06	81.19 52.50		
	1	T27 Half Hip	10 /12	15-10-08	6-00-06	2 x 4	1-04-08	1-08-08 6-00-06	73.01 46.33		
	1	T27S Half Hip	10 /12	15-10-08	6-00-06	2 x 4	1-04-08	1-08-08 6-00-06	86.47 56.83		
	1	T28 Half Hip	10 /12	15-10-08	6-10-06	2 x 4	1-04-08	1-08-08 6-10-06	81.21 51.50		
	1	T28S Half Hip	10 /12	15-10-08	6-10-06	2 x 4	1-04-08	1-08-08 6-10-06	87.83 57.83		
	2	T29 Half Hip	10 /12	15-10-08	7-08-06	2 x 4	1-04-08	1-08-08 7-08-06	171.64 110.67		
	2	T30 Half Hip	10 /12	15-10-08	8-06-06	2 x 4	1-04-08	1-08-08 8-06-06	168.99 105.67		
	2	T31 Half Hip	10 /12	15-10-08	9-04-06	2 x 4	1-04-08	1-08-08 9-04-06	177.35 111.00		
	2	T32 Half Hip	10 /12	15-10-08	10-02-06	2 x 4	1-04-08	1-08-08 10-02-06	178.39 110.33		
	1	T33 Half Hip Girder	10 /12	7-07-08	4-04-06	2 x 4		1-08-08 4-04-06	34.64 23.00		
	1	T34 Half Hip	10 /12	7-07-08	5-02-06	2 x 4		1-08-08 5-02-06	37.38 24.67		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52749
Builder:	ROYAL PINE HOMES	PlanLog:	206083
Project:	SUMMER RIDGE ESTATES INC.	Layout ID:	425920
Location:	BRAMPTON	Ref #	
Model:	BLK 292	Page:	2 of 2
Lot #:		Date:	02-29-2024
Elevation:	A2 UNIT 5-BLK 292	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	T35 Half Hip	10 /12	**7-07-08**	6-00-06	2 x 4		1-08-08 6-00-06	40.39 26.33		
	1 2-ply	T36 Monopitch Girder	5 /12	**7-10-08**	4-04-06	2 x 4 2 x 6		1-01-00 4-04-06	73.36 46.33		
	1 3-ply	T37 Hip Girder	5 /12	**49-07-00**	8-00-12	2 x 6	1-04-08 1-04-08	1-01-00 1-01-00	1021.52 611.00		
	3	T38 Hip	5 /12	**49-07-00**	9-08-12	2 x 6	1-04-08 1-04-08	1-01-00 1-01-00	949.91 568.00		
	1	T55 Roof Special	4 /12	**20-11-00**	6-04-03	2 x 4	1-03-08 1-09-08	4-10 1-09-15	78.88 50.33		
	1	T55G GABLE	4 /12	**20-11-00**	6-04-03	2 x 4	1-03-08 1-09-08	4-10 1-09-15	81.17 52.17		
	2	PB3 Piggyback	5 /12	**8-01-00**	1-08-03	2 x 4			34.69 22.33		
	10	J2 Jack-Partial	5 /12	**7-10-08**	4-04-06	2 x 4	1-04-08	1-01-00 4-04-06	282.69 183.33		
	6	J2S Jack-Partial	5 /12	**7-10-08**	4-04-06	2 x 4	1-04-08	1-01-00 3-04-06	157.83 110.00		
	5	J5 Jack-Open	3.5 /12	**5-11-08**	2-05-00	2 x 4	1-03-08	3-14 2-00-12	77.51 46.67		
	3	J6 Jack-Partial	3.5 /12	**6-11-08**	2-08-08	2 x 4	1-03-08	3-14 2-04-04	60.66 36.50		

TOTAL # TRUSS= **55** TOTAL BFT OF ALL TRUSSES= **2655.49** BFT. TOTAL WEIGHT OF ALL TRSSES **4273.63** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
24	Hardware	H2.5T	
1	Hardware	HGUS26-2	
4	Hardware	LJS26DS	
18	Hardware	LUS24	

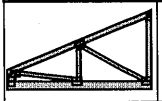
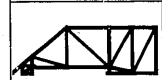
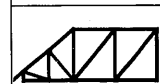
TOTAL NUMBER OF ITEMS= **47**

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52749
Builder:	ROYAL PINE HOMES	PlanLog:	206083
Project:	SUMMER RIDGE ESTATES INC.	Layout ID:	425921
Location:	BRAMPTON	Ref #	
Model:	BLK 292	Page:	1 of 2
Lot #:		Date:	02-29-2024
Elevation:	A1 UNIT 6-BLK 292	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

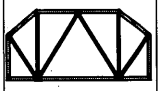
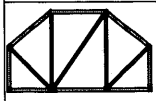
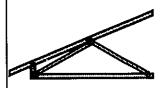
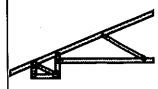
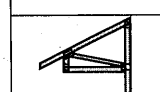
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T36Z4 Monopitch Girder	5 /12	7-10-08	4-04-06	2 x 4 2 x 6		1-01-00 4-04-06	73.36 46.33		
	1 2-ply	T41S Half Hip Girder	10 /12	21-09-08	4-04-06	2 x 4	1-04-08	1-08-08 4-04-06	208.58 134.67		
	1	T42 Half Hip	10 /12	21-09-08	5-02-06	2 x 4	1-04-08	1-08-08 5-02-06	92.45 58.67		
	1	T42S Half Hip	10 /12	21-09-08	5-02-06	2 x 4	1-04-08	1-08-08 5-02-06	105.87 67.83		
	1	T43 Half Hip	10 /12	21-09-08	6-00-06	2 x 4	1-04-08	1-08-08 6-00-06	97.36 61.33		
	1	T43S Half Hip	10 /12	21-09-08	6-00-06	2 x 4	1-04-08	1-08-08 6-00-06	112.24 73.17		
	1	T44 Half Hip	10 /12	21-09-08	6-10-06	2 x 4	1-04-08	1-08-08 6-10-06	106.7 67.00		
	1	T44S Half Hip	10 /12	21-09-08	6-10-06	2 x 4	1-04-08	1-08-08 6-10-06	111.8 73.17		
	1	T45 Half Hip	10 /12	21-09-08	7-08-06	2 x 4	1-04-08	1-08-08 7-08-06	102.22 64.33		
	1	T45S Half Hip	10 /12	21-09-08	7-08-06	2 x 4	1-04-08	1-08-08 7-08-06	117.15 76.00		
	2	T46 Half Hip	10 /12	21-09-08	8-06-06	2 x 4	1-04-08	1-08-08 8-06-06	227.19 141.00		
	2	T47 Half Hip	10 /12	21-09-08	9-04-06	2 x 4	1-04-08	1-08-08 9-04-06	236.69 145.33		
	4	T81 Half Hip	10 /12	21-09-08	10-02-06	2 x 4	1-04-08	1-08-08 10-02-06	493.06 307.33		
	1	T82 Hip Girder	10 /12	13-00-00	5-04-12	2 x 4	1-07-00 1-07-00	4-01-09 4-01-09	74.64 46.50		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52749
Builder:	ROYAL PINE HOMES	PlanLog:	206083
Project:	SUMMER RIDGE ESTATES INC.	Layout ID:	425921
Location:	BRAMPTON	Ref #	
Model:	BLK 292	Page:	2 of 2
Lot #:		Date:	02-29-2024
Elevation:	A1 UNIT 6-BLK 292	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	T83 Hip	10 /12	**13-00-00**	6-04-12	2 x 4		4-01-09 4-01-09	68.33 42.67		
	1	T84 Hip	10 /12	**13-00-00**	7-04-12	2 x 4		4-01-09 4-01-09	67.65 43.50		
	1 2-ply	T87 Half Hip Girder	10 /12	**21-09-08**	4-04-06	2 x 4 2 x 6	1-04-08	1-08-08 4-04-06	217.31 135.67		
	1	T91C Roof Special	4 /12	**25-11-00**	7-07-03	2 x 4	1-03-08 1-09-08	4-10 1-09-15	102.56 63.50		
	1	T91G GABLE	4 /12	**25-11-00**	7-07-03	2 x 4	1-03-08 1-09-08	4-10 1-09-15	105.98 66.50		
	9	J2 Jack-Partial	5 /12	**7-10-08**	4-04-06	2 x 4	1-04-08	1-01-00 4-04-06	254.42 165.00		
	6	J2S Jack-Partial	5 /12	**7-10-08**	4-04-06	2 x 4	1-04-08	1-01-00 3-04-06	157.83 110.00		
	7	J5 Jack-Open	3.5 /12	**5-11-08**	2-05-00	2 x 4	1-03-08	3-14 2-00-12	108.52 65.33		
	6	J10 Jack-Closed	6 /12	**3-10-08**	5-04-12	2 x 4	1-04-08	1-02-08 3-01-12	121.87 84.00		

TOTAL # TRUSS= **55** TOTAL BFT OF ALL TRUSSES= **2138.83** BFT. TOTAL WEIGHT OF ALL TRSSES **3363.8** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
30	Hardware	H2.5T	
1	Hardware	HGUS26-2	
1	Hardware	LJS26DS	
8	Hardware	LUS24	

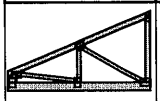
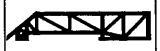
TOTAL NUMBER OF ITEMS= **40**

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52749
Builder:	ROYAL PINE HOMES	PlanLog:	206083
Project:	SUMMER RIDGE ESTATES INC.	Layout ID:	425922
Location:	BRAMPTON	Ref #	
Model:	BLK 292	Page:	1 of 2
Lot #:		Date:	02-29-2024
Elevation:	A2 UNIT 7-BLK 292	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

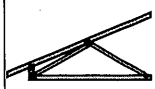
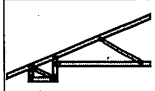
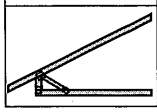
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T36Z5 Monopitch Girder	5 /12	7-10-08	4-04-06	2 x 4 2 x 6		1-01-00 4-04-06	73.36 46.33		
	1 2-ply	T41S Half Hip Girder	10 /12	21-09-08	4-04-06	2 x 4	1-04-08	1-08-08 4-04-06	208.58 134.67		
	1	T42 Half Hip	10 /12	21-09-08	5-02-06	2 x 4	1-04-08	1-08-08 5-02-06	92.45 58.67		
	1	T42S Half Hip	10 /12	21-09-08	5-02-06	2 x 4	1-04-08	1-08-08 5-02-06	105.87 67.83		
	1	T43 Half Hip	10 /12	21-09-08	6-00-06	2 x 4	1-04-08	1-08-08 6-00-06	97.36 61.33		
	1	T43S Half Hip	10 /12	21-09-08	6-00-06	2 x 4	1-04-08	1-08-08 6-00-06	112.24 73.17		
	1	T44 Half Hip	10 /12	21-09-08	6-10-06	2 x 4	1-04-08	1-08-08 6-10-06	106.7 67.00		
	1	T44S Half Hip	10 /12	21-09-08	6-10-06	2 x 4	1-04-08	1-08-08 6-10-06	111.8 73.17		
	1	T45 Half Hip	10 /12	21-09-08	7-08-06	2 x 4	1-04-08	1-08-08 7-08-06	102.22 64.33		
	1	T45S Half Hip	10 /12	21-09-08	7-08-06	2 x 4	1-04-08	1-08-08 7-08-06	117.15 76.00		
	2	T46 Half Hip	10 /12	21-09-08	8-06-06	2 x 4	1-04-08	1-08-08 8-06-06	227.19 141.00		
	2	T47 Half Hip	10 /12	21-09-08	9-04-06	2 x 4	1-04-08	1-08-08 9-04-06	236.69 145.33		
	3	T81 Half Hip	10 /12	21-09-08	10-02-06	2 x 4	1-04-08	1-08-08 10-02-06	369.79 230.50		
	1	T85 Half Hip Girder	10 /12	13-02-08	4-01-12	2 x 4		1-08-08 4-01-12	60.73 39.67		

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52749
Builder:	ROYAL PINE HOMES	PlanLog:	206083
Project:	SUMMER RIDGE ESTATES INC.	Layout ID:	425922
Location:	BRAMPTON	Ref #	
Model:	BLK 292	Page:	2 of 2
Lot #:		Date:	02-29-2024
Elevation:	A2 UNIT 7-BLK 292	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	T86 Half Hip	10 /12	13-02-08	5-01-12	2 x 4		1-08-08 5-01-12	59.29 39.17		
	1 2-ply	T87Z Half Hip Girder	10 /12	21-09-08	4-04-06	2 x 4 2 x 6	1-04-08	1-08-08 4-04-06	217.31 135.67		
	1	T92C Roof Special	4 /12	24-11-00	7-04-03	2 x 4	1-03-08 1-09-08	4-10 1-09-15	100.56 63.33		
	1	T92G GABLE	4 /12	24-11-00	7-04-03	2 x 4	1-03-08 1-09-08	4-10 1-09-15	101.19 65.17		
	9	J2 Jack-Partial	5 /12	7-10-08	4-04-06	2 x 4	1-04-08	1-01-00 4-04-06	254.42 165.00		
	6	J2S Jack-Partial	5 /12	7-10-08	4-04-06	2 x 4	1-04-08	1-01-00 3-04-06	157.83 110.00		
	7	J6 Jack-Partial	3.5 /12	6-11-08	2-08-08	2 x 4	1-03-08	3-14 2-04-04	141.54 85.17		
	7	J11 Jack-Open	6 /12	5-10-08	4-01-12	2 x 4	1-04-08	1-02-08 4-01-12	127.68 81.67		

TOTAL # TRUSS= **54** TOTAL BFT OF ALL TRUSSES= **2024.18** BFT. TOTAL WEIGHT OF ALL TRSSES **3181.97** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
28	Hardware	H2.5T	
1	Hardware	HGUS26-2	
1	Hardware	LJS26DS	
1	Hardware	LUS24	

TOTAL NUMBER OF ITEMS= **31**

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52749
Builder:	ROYAL PINE HOMES	PlanLog:	206083
Project:	SUMMER RIDGE ESTATES INC.	Layout ID:	425923
Location:	BRAMPTON	Ref #	
Model:	BLK 292	Page:	1 of 2
Lot #:		Date:	02-29-2024
Elevation:	A UNIT 8-BLK 292	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

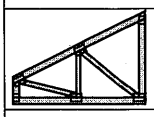
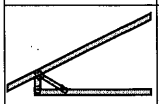
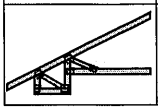
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	T55 Roof Special	4 /12	20-11-00	6-04-03	2 x 4	1-03-08 1-09-08	4-10 1-09-15	78.88 50.33		
	1	T55G GABLE	4 /12	20-11-00	6-04-03	2 x 4	1-03-08 1-09-08	4-10 1-09-15	81.17 52.17		
	1	T71 Hip Girder	10 /12	21-10-00	4-02-03	2 x 6	1-04-08 1-04-08	1-08-08 1-08-08	122.43 74.00		
	1	T71S Hip Girder	10 /12	21-10-00	4-02-03	2 x 6	1-04-08 1-04-08	1-08-08 1-08-08	130.3 82.83		
	1	T72 Hip	10 /12	21-10-00	5-01-12	2 x 4	1-04-08 1-04-08	1-08-08 1-08-08	90.83 57.83		
	1	T72S Hip	10 /12	21-10-00	5-01-12	2 x 4	1-04-08 1-04-08	1-08-08 1-08-08	103.97 67.83		
	1	T73 Hip	10 /12	21-10-00	6-01-12	2 x 4	1-04-08 1-04-08	1-08-08 1-08-08	95.56 60.33		
	1	T73S Hip	10 /12	21-10-00	6-01-12	2 x 4	1-04-08 1-04-08	1-08-08 1-08-08	109.38 71.33		
	1	T74 Hip	10 /12	21-10-00	7-01-12	2 x 4	1-04-08 1-04-08	1-08-08 1-08-08	109.55 68.17		
	1	T74S Hip	10 /12	21-10-00	7-01-12	2 x 4	1-04-08 1-04-08	1-08-08 1-08-08	112.24 71.33		
	1	T75 Hip	10 /12	21-10-00	8-01-12	2 x 4	1-04-08 1-04-08	1-08-08 1-08-08	104.92 66.33		
	1	T75S Hip	10 /12	21-10-00	8-01-12	2 x 4	1-04-08 1-04-08	1-08-08 1-08-08	109.84 71.33		
	1	T76 Hip	10 /12	21-10-00	9-01-12	2 x 4	1-04-08 1-04-08	1-08-08 1-08-08	110.74 69.67		
	1	T76S Hip	10 /12	21-10-00	9-01-12	2 x 4	1-04-08 1-04-08	1-08-08 1-08-08	123.92 78.67		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52749
Builder:	ROYAL PINE HOMES	PlanLog:	206083
Project:	SUMMER RIDGE ESTATES INC.	Layout ID:	425923
Location:	BRAMPTON	Ref #	
Model:	BLK 292	Page:	2 of 2
Lot #:		Date:	02-29-2024
Elevation:	A UNIT 8-BLK 292	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	2	T77 Hip	10 /12	21-10-00	10-01-12	2 x 4	1-04-08 1-04-08	1-08-08 1-08-08	232.98 148.00		
	4	T78 Common	10 /12	21-10-00	10-09-11	2 x 4	1-04-08 1-04-08	1-08-08 1-08-08	455.58 280.00		
	4	T79 Common	10 /12	13-06-00	7-04-00	2 x 4	1-04-08	1-08-08 1-08-08	250.76 162.00		
	2	T79S Roof Special	10 /12	13-06-00	7-04-00	2 x 4	1-04-08 1-04-08	1-08-08 1-08-08	142.58 92.00		
	1 2-ply	T80 Monopitch Girder	6 /12	5-10-08	4-01-12	2 x 4		1-02-08 4-01-12	51.57 34.00		
	7	J5 Jack-Open	3.5 /12	5-11-08	2-05-00	2 x 4	1-03-08	3-14 2-00-12	108.52 65.33		
	9	J11 Jack-Open	6 /12	5-10-08	4-01-12	2 x 4	1-04-08	1-02-08 4-01-12	164.16 105.00		
	6	J11S Jack-Open	6 /12	5-10-08	4-01-12	2 x 4	1-04-08	1-02-08 3-01-12	126.71 88.00		

TOTAL # TRUSS= **50** TOTAL BFT OF ALL TRUSSES= **1916.48** BFT. TOTAL WEIGHT OF ALL TRSSES **3016.58** LBS

HARDWARE

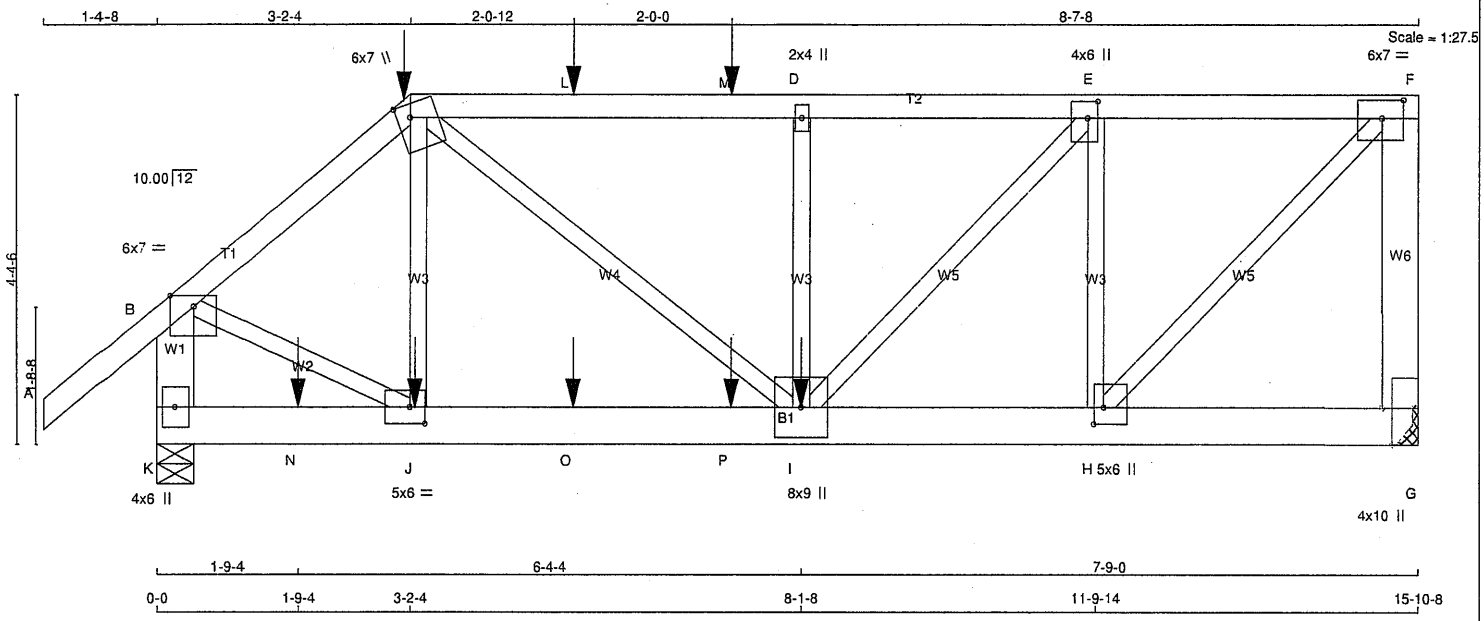
QTY	TYPE	MODEL	LENGTH
7	Hardware	H2.5T	
1	Hardware	HGUS26-2	
3	Hardware	LUS24	

TOTAL NUMBER OF ITEMS= **11**

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425055	T25	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.630 S Aug 30 2023 MiTek Industries, Inc. Wed Feb 28 12:56:32 2024 Page 1
ID:sd0FDfbi38oVqvTaiRkhtSzPAoD-jXEWaaPtPgof3LVqzryTEXFuPIAKUjPnH9Yegrzgcmj



TOTAL WEIGHT = 2 X 83 = 166 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
G - F	2x6	DRY	No.2	SPF
K - B	2x6	DRY	No.2	SPF
K - G	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	6.0	7.0	Edge	
C	TTWW+m	MT20	6.0	7.0	2.00	2.00
D	TMW+w	MT20	2.0	4.0		
E	TMWW+t	MT20	4.0	6.0	2.50	1.50
F	TMVW-t	MT20	6.0	7.0	2.75	3.25
G	BMV1+t	MT20	4.0	10.0	Edge	1.50
H	BMWW+t	MT20	5.0	6.0	2.50	1.50
I	BMWWW+t	MT20	8.0	9.0		
J	BMWW-t	MT20	5.0	6.0	2.50	2.25
K	BMV1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2406330

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1917	0	1917	0	0	MECHANICAL	
K	2728	0	2728	0	0	5-8	3-6

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1356	887 / 0	0 / 0	0 / 0	0 / 0	469 / 0	0 / 0
K	1926	1282 / 0	0 / 0	0 / 0	0 / 0	644 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.42 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-84.9	-84.9	0.14 (1)	10.00	J- C	0 / 113	0.04 (4)	
B- C	-2543 / 0	-84.9	-84.9	0.27 (1)	4.06	C- I	0 / 1344	0.33 (1)	
C- L	-2995 / 0	-84.9	-84.9	0.57 (1)	3.42	I- D	-473 / 0	0.13 (1)	
L- M	-2995 / 0	-84.9	-84.9	0.57 (1)	3.42	B- J	0 / 2091	0.52 (1)	
M- D	-2995 / 0	-84.9	-84.9	0.57 (1)	3.42	H- F	0 / 2467	0.61 (1)	
D- E	-2996 / 0	-84.9	-84.9	0.43 (1)	3.56	I- E	0 / 1821	0.45 (1)	
E- F	-1758 / 0	-84.9	-84.9	0.25 (1)	4.75	H- E	-1664 / 0	0.46 (1)	
G- F	-1878 / 0	0.0	0.0	0.34 (1)	7.28				
K- B	-2658 / 0	0.0	0.0	0.20 (1)	6.35				
K- N	0 / 0	-18.5	-18.5	0.20 (1)	10.00				
N- J	0 / 0	-18.5	-18.5	0.20 (1)	10.00				
J- O	0 / 1948	-18.5	-18.5	0.46 (1)	10.00				
O- P	0 / 1948	-18.5	-18.5	0.46 (1)	10.00				
P- I	0 / 1948	-18.5	-18.5	0.46 (1)	10.00				
I- H	0 / 1758	-18.5	-18.5	0.25 (1)	10.00				
H- G	0 / 0	-18.5	-18.5	0.04 (4)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-2-4	-47	-47	---	FRONT	VERT	DEAD	---	C1
C	3-2-4	-38	-38	---	FRONT	VERT	TOTAL	---	C1
C	3-2-4	-180	-180	---	FRONT	VERT	SNOW	---	C1
I	8-1-8	-954	-954	---	FRONT	VERT	TOTAL	---	C1
J	3-3-0	-187	-187	---	FRONT	VERT	TOTAL	---	C1
L	5-3-0	-32	-32	---	FRONT	VERT	TOTAL	---	C1
M	7-3-0	-32	-32	---	FRONT	VERT	TOTAL	---	C1
N	1-9-4	-187	-187	---	FRONT	VERT	TOTAL	---	C1
O	5-3-0	-187	-187	---	FRONT	VERT	TOTAL	---	C1
P	7-3-0	-187	-187	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.53")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.53")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.57/1.00 (C-D:1) , BC=0.46/1.00 (I-J:1) , WB=0.61/1.00 (F-H:1) , SSI=0.26/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (C) (INPUT = 0.90)
JSI METAL= 0.62 (H) (INPUT = 0.95)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425055	T25S	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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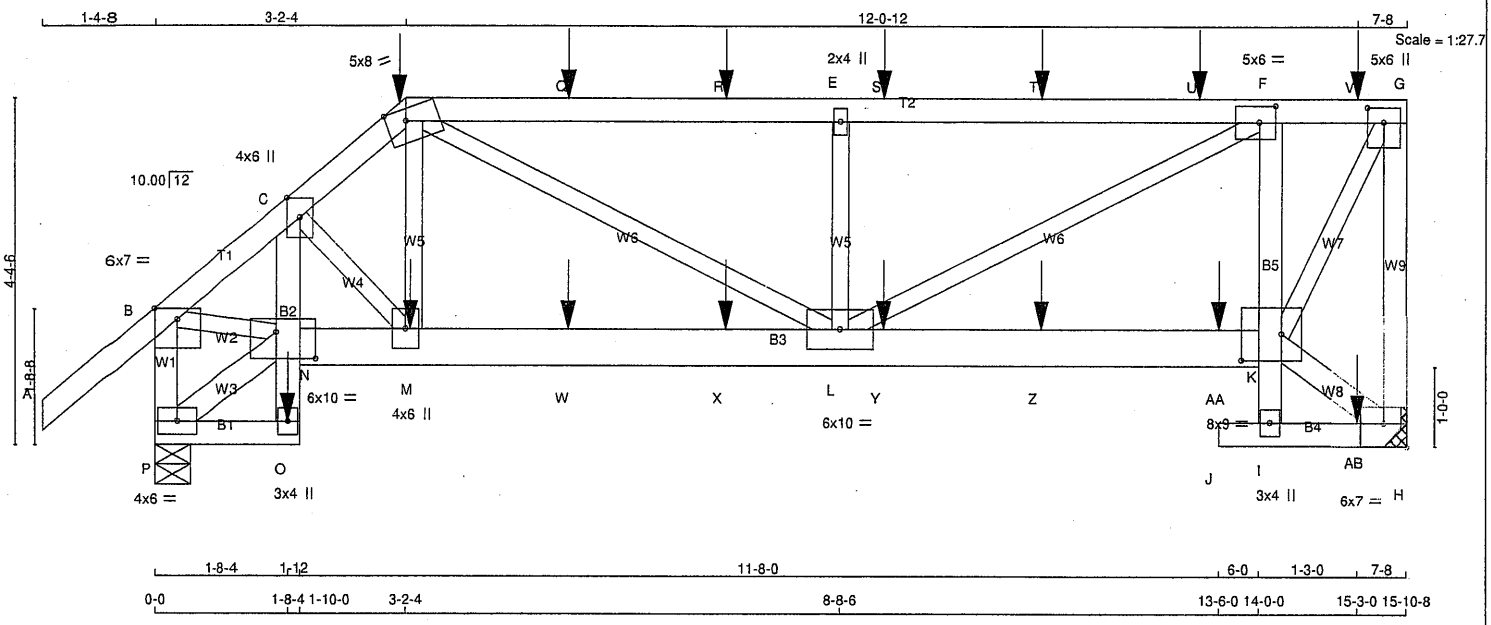

TOTAL WEIGHT = 2 X 84 = 167 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
P - O	2x4	DRY	No.2	SPF
O - C	2x4	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	6.0	7.0	Edge	
C	TMVW+p	MT20	4.0	6.0	Edge	2.00
D	TTWW-m	MT20	5.0	8.0	Edge	3.00
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	5.0	6.0	2.50	2.50
G	TMVW+p	MT20	5.0	6.0	2.25	2.50
H	BMVW1-t	MT20	6.0	7.0	3.50	Edge
I	BMV+p	MT20	3.0	4.0		
K	BVMWW-l	MT20	8.0	9.0	4.00	6.00
L	BMWWW-t	MT20	6.0	10.0		
M	BMWW+t	MT20	4.0	6.0		
N	BVMWW-l	MT20	6.0	10.0	4.00	6.00
O	BMV+p	MT20	3.0	4.0		
P	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2406331

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	2167	0	2167	0	0	MECHANICAL	
P	2322	0	2322	0	0	5-8	2-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1540	969 / 0	0 / 0	0 / 0	0 / 0	571 / 0	0 / 0
P	1644	1069 / 0	0 / 0	0 / 0	0 / 0	575 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.41 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.68 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX	CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO			FR-TO		
A- B	0 / 40	-84.9	-84.9	0.14 (1)	10.00	C- M	-76 / 0	0.01 (1)
B- C	-2796 / 0	-84.9	-84.9	0.24 (1)	3.91	M- D	0 / 666	0.16 (1)
C- D	-2730 / 0	-84.9	-84.9	0.18 (1)	4.01	D- L	0 / 984	0.24 (1)
D- Q	-2990 / 0	-84.9	-84.9	0.62 (1)	3.41	L- E	-543 / 0	0.11 (1)
Q- R	-2990 / 0	-84.9	-84.9	0.62 (1)	3.41	L- F	0 / 2071	0.51 (1)
R- E	-2990 / 0	-84.9	-84.9	0.62 (1)	3.41	P- N	-77 / 0	0.01 (1)
E- S	-2990 / 0	-84.9	-84.9	0.62 (1)	3.41	B- N	0 / 2117	0.52 (1)
S- T	-2990 / 0	-84.9	-84.9	0.62 (1)	3.41	K- H	-65 / 0	0.01 (1)
T- U	-2990 / 0	-84.9	-84.9	0.62 (1)	3.41	K- G	0 / 2241	0.55 (1)
U- F	-2990 / 0	-84.9	-84.9	0.62 (1)	3.41			
F- V	-1152 / 0	-84.9	-84.9	0.25 (1)	5.57			
V- G	-1152 / 0	-84.9	-84.9	0.25 (1)	5.57			
H- G	-1945 / 0	0.0	0.0	0.60 (1)	5.99			
P- B	-2262 / 0	0.0	0.0	0.27 (1)	5.61			
P- O	0 / 63	-18.5	-18.5	0.02 (4)	10.00			
O- N	0 / 287	0.0	0.0	0.16 (1)	10.00			
N- C	0 / 23	0.0	0.0	0.12 (1)	10.00			
N- M	0 / 2147	-18.5	-18.5	0.47 (1)	10.00			
M- W	0 / 2124	-18.5	-18.5	0.54 (1)	10.00			
W- X	0 / 2124	-18.5	-18.5	0.54 (1)	10.00			
X- L	0 / 2124	-18.5	-18.5	0.54 (1)	10.00			
L- Y	0 / 1173	-18.5	-18.5	0.48 (1)	10.00			
Y- Z	0 / 1173	-18.5	-18.5	0.48 (1)	10.00			
Z-AA	0 / 1173	-18.5	-18.5	0.48 (1)	10.00			
AA- K	0 / 1173	-18.5	-18.5	0.48 (1)	10.00			
I- K	0 / 124	0.0	0.0	0.12 (1)	10.00			
K- F	-1409 / 0	0.0	0.0	0.18 (1)	6.68			
J- I	0 / 0	-18.5	-18.5	0.01 (4)	10.00			
I-AB	0 / 53	-18.5	-18.5	0.18 (1)	10.00			
AB- H	0 / 53	-18.5	-18.5	0.18 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	3-2-4	-47	-47	---	FRONT	VERT	DEAD	---	C1
D	3-2-4	-9	-9	---	BACK	VERT	TOTAL	---	C1
D	3-2-4	-180	-180	---	FRONT	VERT	SNOW	---	C1
M	3-3-0	-205	-205	---	BACK	VERT	TOTAL	---	C1

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.53")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.53")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.62/1.00 (D-E:1) , BC=0.54/1.00 (L-M:1) , WB=0.55/1.00 (G-K:1) , SSI=0.37/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
425055	T25S	2	1	TRUSS DESC.	

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
O	1-8-4	-194	-194	---	BACK	VERT	TOTAL	---	C1
Q	5-3-0	-6	-6	---	BACK	VERT	TOTAL	---	C1
R	7-3-0	-6	-6	---	BACK	VERT	TOTAL	---	C1
S	9-3-0	-6	-6	---	BACK	VERT	TOTAL	---	C1
T	11-3-0	-6	-6	---	BACK	VERT	TOTAL	---	C1
U	13-3-0	-6	-6	---	BACK	VERT	TOTAL	---	C1
V	15-3-0	-43	-43	---	BACK	VERT	TOTAL	---	C1
W	5-3-0	-205	-205	---	BACK	VERT	TOTAL	---	C1
X	7-3-0	-205	-205	---	BACK	VERT	TOTAL	---	C1
Y	9-3-0	-205	-205	---	BACK	VERT	TOTAL	---	C1
Z	11-3-0	-205	-205	---	BACK	VERT	TOTAL	---	C1
AA	13-6-0	-206	-206	---	BACK	VERT	TOTAL	---	C1
AB	15-3-0	-190	-190	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

JSI GRIP= 0.80 (F) (INPUT = 0.90)
JSI METAL= 0.45 (B) (INPUT = 0.95)

Structural component only
DWG# T-2406331

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425055	T26	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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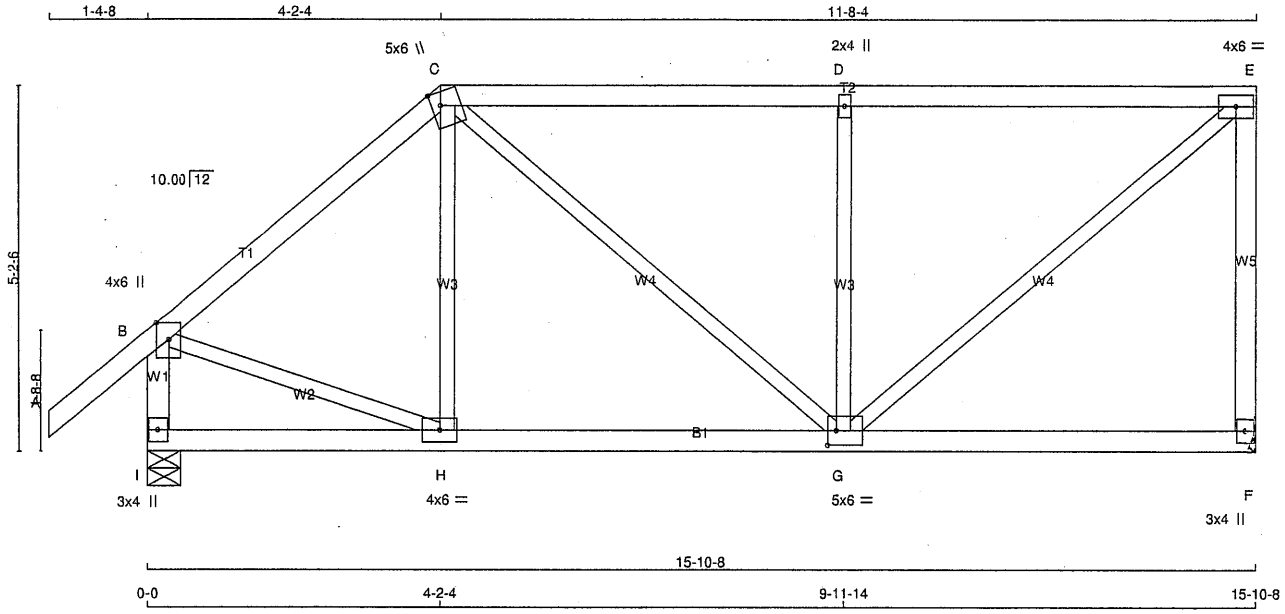

TOTAL WEIGHT = 2 X 69 = 138 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	6.0	2.50	1.50
H	BMWW-t	MT20	4.0	6.0		
I	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	820	0	820	0	0	MECHANICAL	
I	945	0	945	0	0	5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	582	370 / 0	0 / 0	0 / 0	0 / 0	213 / 0	0 / 0
I	668	438 / 0	0 / 0	0 / 0	0 / 0	230 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.24 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-84.9	-84.9	0.13 (1)	10.00	H- C	-70 / 46	0.03 (1)
B- C	-690 / 0	-84.9	-84.9	0.27 (1)	6.25	B- H	0 / 555	0.12 (1)
C- D	-702 / 0	-84.9	-84.9	0.51 (1)	6.24	G- E	0 / 915	0.21 (1)
D- E	-702 / 0	-84.9	-84.9	0.51 (1)	6.24	C- G	0 / 229	0.05 (1)
F- E	-776 / 0	0.0	0.0	0.36 (1)	7.81	G- D	-613 / 0	0.25 (1)
I- B	-915 / 0	0.0	0.0	0.10 (1)	7.81			
I- H	0 / 0	-18.5	-18.5	0.10 (4)	10.00			
H- G	0 / 527	-18.5	-18.5	0.20 (4)	10.00			
G- F	0 / 0	-18.5	-18.5	0.15 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.53")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.53")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.51/1.00 (D-E:1) , BC=0.20/1.00 (G-H:4) , WB=0.25/1.00 (D-G:1) , SSI=0.24/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.71 (G) (INPUT = 0.90)
JSI METAL= 0.37 (B) (INPUT = 0.95)

Structural component only
DWG# T-2406332

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425055	T26S	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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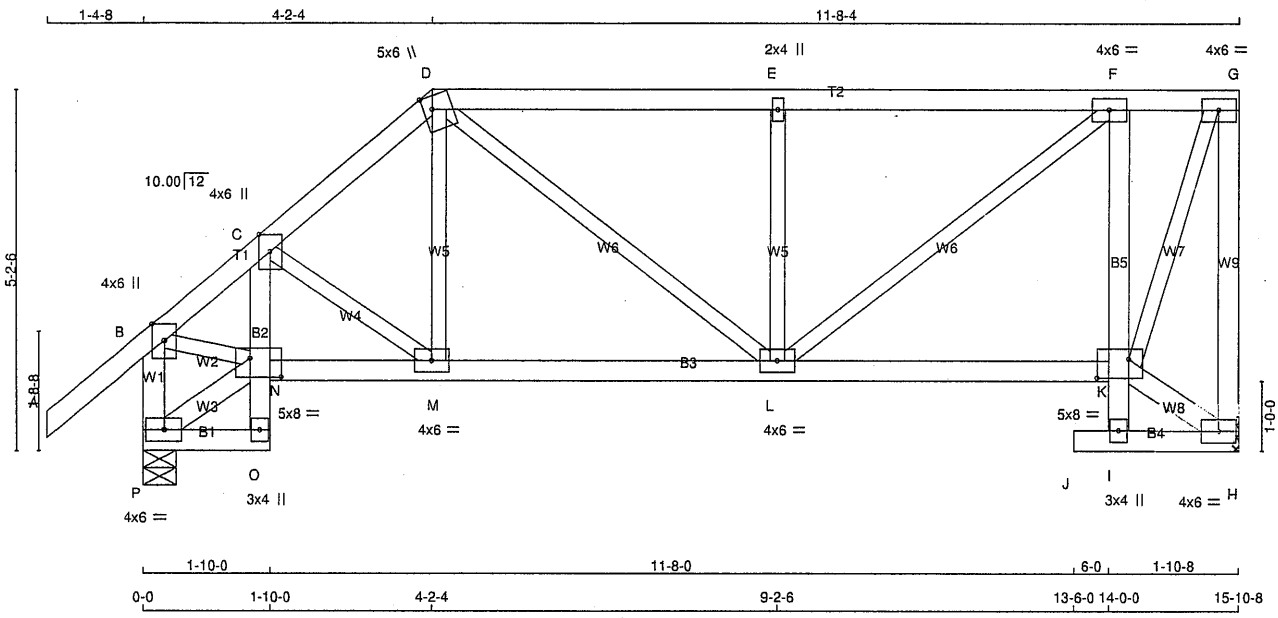

TOTAL WEIGHT = 2 X 81 = 162 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
P - O	2x4	DRY	No.2	SPF
O - C	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMVW+p	MT20	4.0	6.0	Edge	2.00
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	TMVW-t	MT20	4.0	6.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BMV+p	MT20	3.0	4.0		
K	BVMWW-I	MT20	5.0	8.0	3.25	5.50
L	BMWWW-t	MT20	4.0	6.0		
M	BMWW-t	MT20	4.0	6.0		
N	BVMWW-I	MT20	5.0	8.0	3.25	5.50
O	BMV+p	MT20	3.0	4.0		
P	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2406333

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	825	0	825	0	0	MECHANICAL	
P	952	0	952	0	0	5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	587	367 / 0	0 / 0	0 / 0	0 / 0	220 / 0	0 / 0
P	673	441 / 0	0 / 0	0 / 0	0 / 0	232 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.10 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-84.9	-84.9	0.13 (1)		10.00
B- C	-920 / 0	-84.9	-84.9	0.13 (1)		6.25
C- D	-842 / 0	-84.9	-84.9	0.09 (1)		6.25
D- E	-872 / 0	-84.9	-84.9	0.33 (1)		6.10
E- F	-873 / 0	-84.9	-84.9	0.33 (1)		6.10
F- G	-347 / 0	-84.9	-84.9	0.15 (1)		6.25
H- G	-795 / 0	0.0	0.0	0.37 (1)		7.81
P- B	-922 / 0	0.0	0.0	0.10 (1)		7.81
P- O	0 / 22	-18.5	-18.5	0.02 (4)		10.00
O- N	0 / 17	0.0	0.0	0.04 (1)		10.00
N- C	-69 / 0	0.0	0.0	0.03 (1)		7.81
N- M	0 / 700	-18.5	-18.5	0.15 (1)		10.00
M- L	0 / 643	-18.5	-18.5	0.17 (4)		10.00
L- K	0 / 353	-18.5	-18.5	0.14 (4)		10.00
I- K	0 / 31	0.0	0.0	0.04 (1)		10.00
K- F	-732 / 0	0.0	0.0	0.09 (1)		7.81
J- I	0 / 0	-18.5	-18.5	0.01 (4)		10.00
I- H	0 / 24	-18.5	-18.5	0.02 (4)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	CSI (LC)
C- M	-72 / 0	0.01 (1)
M- D	0 / 91	0.03 (4)
D- L	0 / 290	0.07 (1)
L- E	-487 / 0	0.13 (1)
L- F	0 / 661	0.15 (1)
P- N	-26 / 0	0.00 (1)
B- N	0 / 695	0.16 (1)
K- H	-28 / 0	0.00 (1)
K- G	0 / 858	0.19 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.53")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.53")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.37/1.00 (G-H:1) , BC=0.17/1.00 (L-M:4) , WB=0.19/1.00 (G-K:1) , SSI=0.20/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (L) (INPUT = 0.90)
JSI METAL= 0.43 (B) (INPUT = 0.95)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425055	T27	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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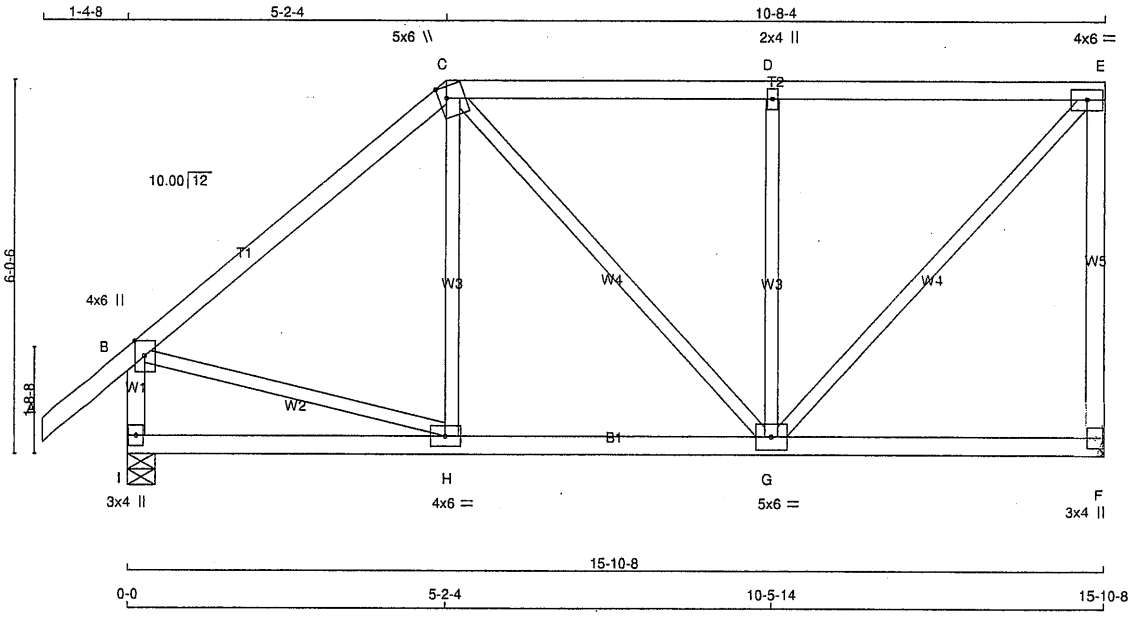

TOTAL WEIGHT = 2 X 73 = 146 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	6.0		
H	BMWW-t	MT20	4.0	6.0		
I	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	820	0	820	0	0	MECHANICAL	
I	945	0	945	0	0	5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	582	370 / 0	0 / 0	0 / 0	0 / 0	213 / 0	0 / 0
I	668	438 / 0	0 / 0	0 / 0	0 / 0	230 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-84.9	-84.9	0.13 (1)		10.00	H- C	-26 / 69	0.02 (4)
B- C	-667 / 0	-84.9	-84.9	0.42 (1)		6.25	B- H	0 / 528	0.12 (1)
C- D	-567 / 0	-84.9	-84.9	0.41 (1)		6.25	G- E	0 / 829	0.19 (1)
D- E	-567 / 0	-84.9	-84.9	0.42 (1)		6.25	C- G	0 / 83	0.02 (1)
F- E	-779 / 0	0.0	0.0	0.54 (1)		7.81	G- D	-561 / 0	0.32 (1)
I- B	-905 / 0	0.0	0.0	0.10 (1)		7.81			
I- H	0 / 0	-18.5	-18.5	0.12 (4)		10.00			
H- G	0 / 511	-18.5	-18.5	0.17 (4)		10.00			
G- F	0 / 0	-18.5	-18.5	0.12 (4)		10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.53")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.53")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.54/1.00 (E-F:1) , BC=0.17/1.00 (G-H:4) , WB=0.32/1.00 (D-G:1) , SSI=0.22/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (G) (INPUT = 0.90)
JSI METAL= 0.37 (B) (INPUT = 0.95)

Structural component only
DWG# T-2406334

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425055	T27S	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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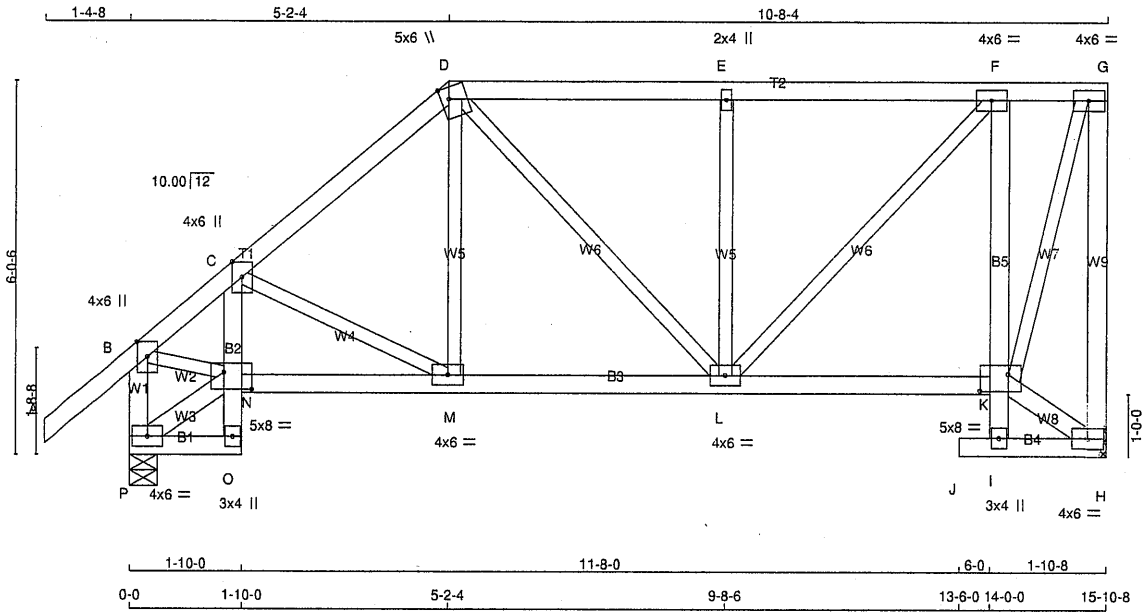

TOTAL WEIGHT = 2 X 86 = 173 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
P - O	2x4	DRY	No.2	SPF
O - C	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMVW+p	MT20	4.0	6.0	Edge	2.00
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	TMVW-t	MT20	4.0	6.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BMV+p	MT20	3.0	4.0		
K	BVMWW-l	MT20	5.0	8.0	3.25	5.50
L	BMWWW-t	MT20	4.0	6.0		
M	BMWW-t	MT20	4.0	6.0		
N	BVMWW-l	MT20	5.0	8.0	3.25	5.50
O	BMV+p	MT20	3.0	4.0		
P	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2406335

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	825	0	825	0	0	MECHANICAL	
P	952	0	952	0	0	5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	587	367 / 0	0 / 0	0 / 0	0 / 0	220 / 0	0 / 0
P	673	441 / 0	0 / 0	0 / 0	0 / 0	232 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.23 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-84.9	-84.9	0.13 (1)	10.00
B- C	-940 / 0	-84.9	-84.9	0.13 (1)	6.23
C- D	-795 / 0	-84.9	-84.9	0.15 (1)	6.25
D- E	-693 / 0	-84.9	-84.9	0.26 (1)	6.25
E- F	-693 / 0	-84.9	-84.9	0.26 (1)	6.25
F- G	-281 / 0	-84.9	-84.9	0.11 (1)	6.25
H- G	-796 / 0	0.0	0.0	0.55 (1)	7.81
P- B	-922 / 0	0.0	0.0	0.10 (1)	7.81
P- O	0 / 23	-18.5	-18.5	0.02 (4)	10.00
O- N	0 / 17	0.0	0.0	0.04 (1)	10.00
N- C	-69 / 6	0.0	0.0	0.03 (1)	7.81
N- M	0 / 730	-18.5	-18.5	0.15 (1)	10.00
M- L	0 / 602	-18.5	-18.5	0.14 (4)	10.00
L- K	0 / 286	-18.5	-18.5	0.11 (4)	10.00
I- K	0 / 31	0.0	0.0	0.04 (1)	10.00
K- F	-721 / 0	0.0	0.0	0.16 (1)	7.81
J- I	0 / 0	-18.5	-18.5	0.01 (4)	10.00
I- H	0 / 23	-18.5	-18.5	0.02 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- M	-145 / 0	0.04 (1)
M- D	0 / 125	0.04 (4)
D- L	0 / 131	0.03 (1)
L- E	-439 / 0	0.16 (1)
L- F	0 / 595	0.13 (1)
P- N	-27 / 0	0.00 (1)
B- N	0 / 724	0.16 (1)
K- H	-27 / 0	0.00 (1)
K- G	0 / 820	0.18 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.53")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.53")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.55/1.00 (G-H:1) , BC=0.16/1.00 (F-K:1) , WB=0.18/1.00 (G-K:1) , SSI=0.18/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.72 (B) (INPUT = 0.90)
JSI METAL= 0.44 (B) (INPUT = 0.95)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425055	T28	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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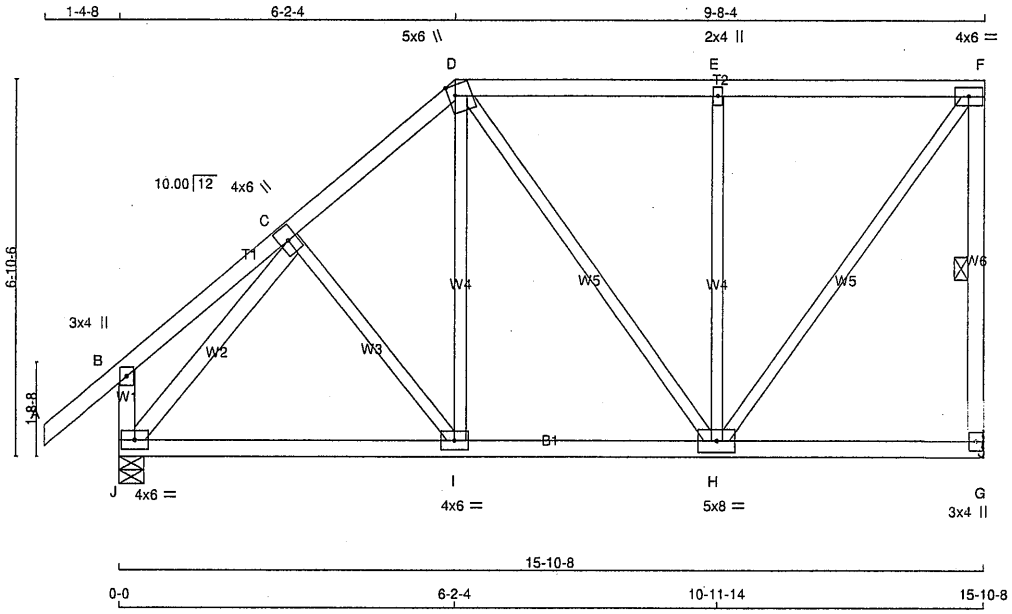

TOTAL WEIGHT = 2 X 81 = 162 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
J - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW+t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BMWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
G	820	0	820	0	0	MECHANICAL	
J	945	0	945	0	0	5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	582	370 / 0	0 / 0	0 / 0	0 / 0	213 / 0	0 / 0
J	668	438 / 0	0 / 0	0 / 0	0 / 0	230 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-84.9	-84.9	0.13 (1)	10.00	C- I	-69 / 11	0.03 (1)
B- C	0 / 19	-84.9	-84.9	0.13 (1)	10.00	I- D	0 / 172	0.04 (4)
C- D	-646 / 0	-84.9	-84.9	0.13 (1)	6.25	J- C	-853 / 0	0.24 (1)
D- E	-463 / 0	-84.9	-84.9	0.33 (1)	6.25	H- F	0 / 776	0.17 (1)
E- F	-463 / 0	-84.9	-84.9	0.33 (1)	6.25	D- H	-34 / 0	0.05 (1)
G- F	-781 / 0	0.0	0.0	0.17 (1)	6.25	H- E	-507 / 0	0.40 (1)
J- B	-224 / 0	0.0	0.0	0.02 (1)	7.81			
J- I	0 / 524	-18.5	-18.5	0.19 (4)	10.00			
I- H	0 / 483	-18.5	-18.5	0.20 (4)	10.00			
H- G	0 / 0	-18.5	-18.5	0.09 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.53")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.53")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.33/1.00 (E-F:1) , BC=0.20/1.00 (H-I:4) , WB=0.40/1.00 (E-H:1) , SSI=0.20/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.66 (F) (INPUT = 0.90)
JSI METAL= 0.37 (C) (INPUT = 0.95)

Structural component only
DWG# T-2406336

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425055	T28S	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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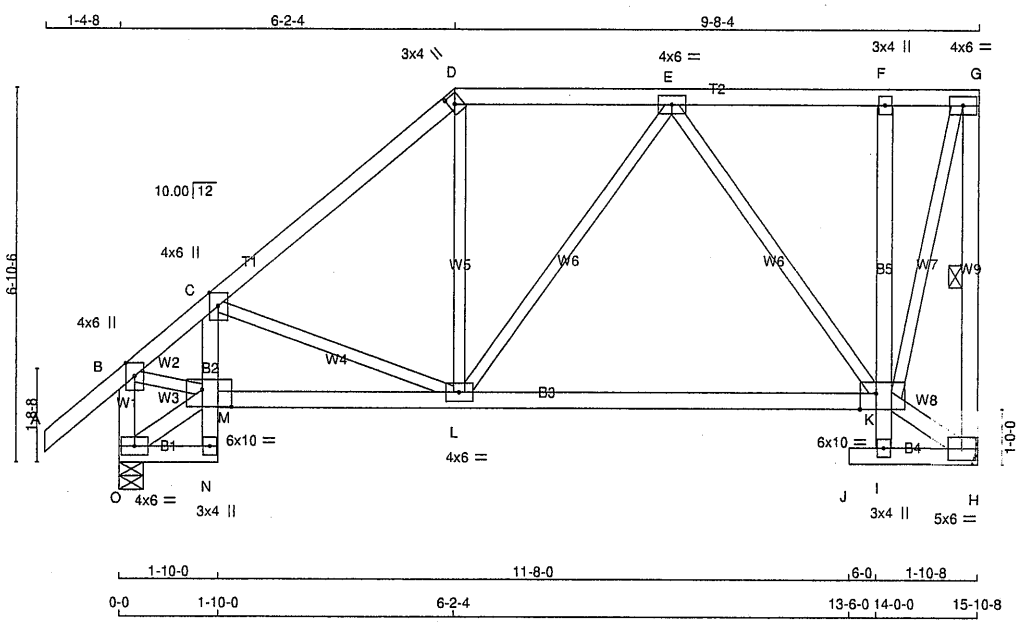


TOTAL WEIGHT = 2 X 88 = 176 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
N - C	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF

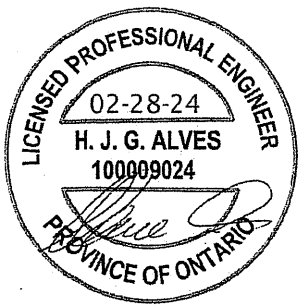
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMVW+p	MT20	4.0	6.0	Edge	2.00
D	TTW+h	MT20	3.0	4.0	2.00	1.25
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
G	TMVW-t	MT20	4.0	6.0		
H	BMVW1-t	MT20	5.0	6.0		
I	BMV+p	MT20	3.0	4.0		
K	BVMWWW-I	MT20	6.0	10.0	Edge	3.50
L	BMWWW-t	MT20	4.0	6.0		
M	BVMWW-I	MT20	6.0	10.0	3.75	6.50
N	BMV+p	MT20	3.0	4.0		
O	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only
DWG# T-2406337

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	825	0	825	0	0	MECHANICAL	
O	952	0	952	0	0	5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	587	367 / 0	0 / 0	0 / 0	0 / 0	220 / 0	0 / 0
O	673	441 / 0	0 / 0	0 / 0	0 / 0	232 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.07 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B		0 / 40	-84.9	-84.9	0.13 (1)	10.00	C- L		-220 / 0	0.09 (1)
B- C		-966 / 0	-84.9	-84.9	0.20 (1)	6.07	L- D		0 / 177	0.04 (4)
C- D		-758 / 0	-84.9	-84.9	0.23 (1)	6.25	O- M		-28 / 0	0.00 (1)
D- E		-569 / 0	-84.9	-84.9	0.21 (1)	6.25	B- M		0 / 765	0.17 (1)
E- F		-239 / 0	-84.9	-84.9	0.20 (1)	6.25	K- H		-26 / 0	0.00 (1)
F- G		-235 / 0	-84.9	-84.9	0.08 (1)	6.25	K- G		0 / 793	0.18 (1)
H- G		-796 / 0	0.0	0.0	0.17 (1)	6.25	L- E		0 / 71	0.02 (4)
O- B		-921 / 0	0.0	0.0	0.10 (1)	7.81	E- K		-527 / 0	0.46 (1)
O- N		0 / 23	-18.5	-18.5	0.02 (4)	10.00				
N- M		0 / 17	0.0	0.0	0.04 (1)	10.00				
M- C		-85 / 0	0.0	0.0	0.03 (1)	7.81				
M- L		0 / 770	-18.5	-18.5	0.32 (4)	10.00				
L- K		0 / 544	-18.5	-18.5	0.29 (4)	10.00				
I- K		0 / 31	0.0	0.0	0.03 (1)	10.00				
K- F		-253 / 0	0.0	0.0	0.07 (1)	7.81				
J- I		0 / 0	-18.5	-18.5	0.01 (4)	10.00				
I- H		0 / 22	-18.5	-18.5	0.02 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.53")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.53")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.23/1.00 (C-D:1) , BC=0.32/1.00 (L-M:4) , WB=0.46/1.00 (E-K:1) , SSI=0.16/1.00 (D-E:1) -

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (B) (INPUT = 0.90)
JSI METAL= 0.45 (B) (INPUT = 0.95)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425055	T29	4	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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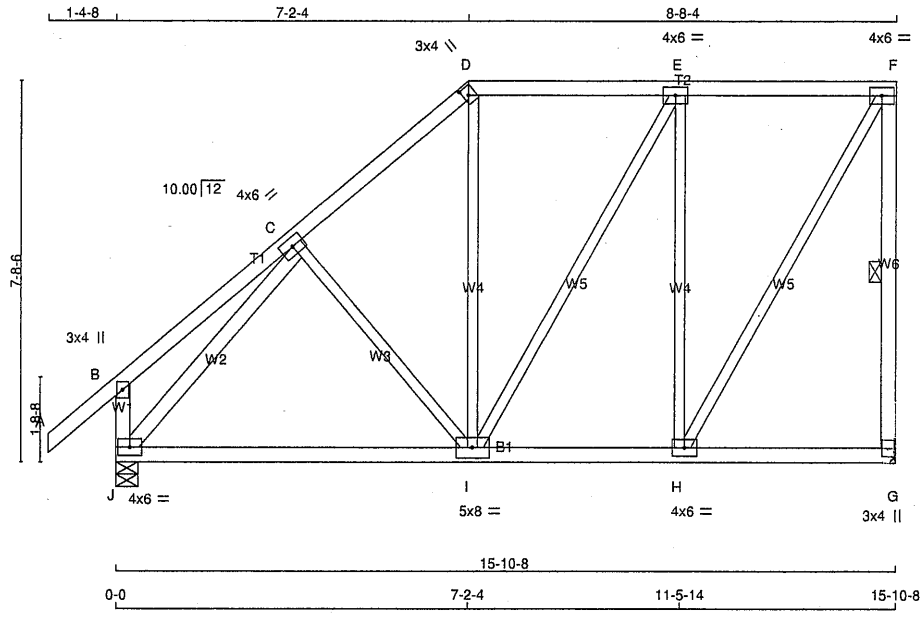

TOTAL WEIGHT = 4 X 86 = 343 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
J - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW+h	MT20	3.0	4.0	2.00	1.25
E	TMWW-t	MT20	4.0	6.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	6.0		
I	BMWWW-t	MT20	5.0	8.0		
J	BMVW1-t	MT20	4.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
G	820	0	820	0	0	MECHANICAL	
J	945	0	945	0	0	5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	582	370 / 0	0 / 0	0 / 0	0 / 0	213 / 0	0 / 0
J	668	438 / 0	0 / 0	0 / 0	0 / 0	230 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-84.9	-84.9	0.13 (1)	10.00	C- I	-133 / 0	0.07 (1)
B- C	0 / 23	-84.9	-84.9	0.18 (1)	10.00	I- D	0 / 115	0.03 (4)
C- D	-605 / 0	-84.9	-84.9	0.18 (1)	6.25	J- C	-840 / 0	0.31 (1)
D- E	-447 / 0	-84.9	-84.9	0.27 (1)	6.25	H- F	0 / 745	0.17 (1)
E- F	-380 / 0	-84.9	-84.9	0.26 (1)	6.25	I- E	0 / 137	0.03 (1)
G- F	-784 / 0	0.0	0.0	0.21 (1)	6.25	H- E	-573 / 0	0.63 (1)
J- B	-239 / 0	0.0	0.0	0.03 (1)	7.81			
J- I	0 / 533	-18.5	-18.5	0.25 (4)	10.00			
I- H	0 / 380	-18.5	-18.5	0.24 (4)	10.00			
H- G	0 / 0	-18.5	-18.5	0.08 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.53")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.53")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.27/1.00 (D-E:1) , BC=0.25/1.00 (I-J:4) , WB=0.63/1.00 (E-H:1) , SSI=0.18/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (H) (INPUT = 0.90)
JSI METAL= 0.19 (C) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
02-28-24
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2406338

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425055	T30	4	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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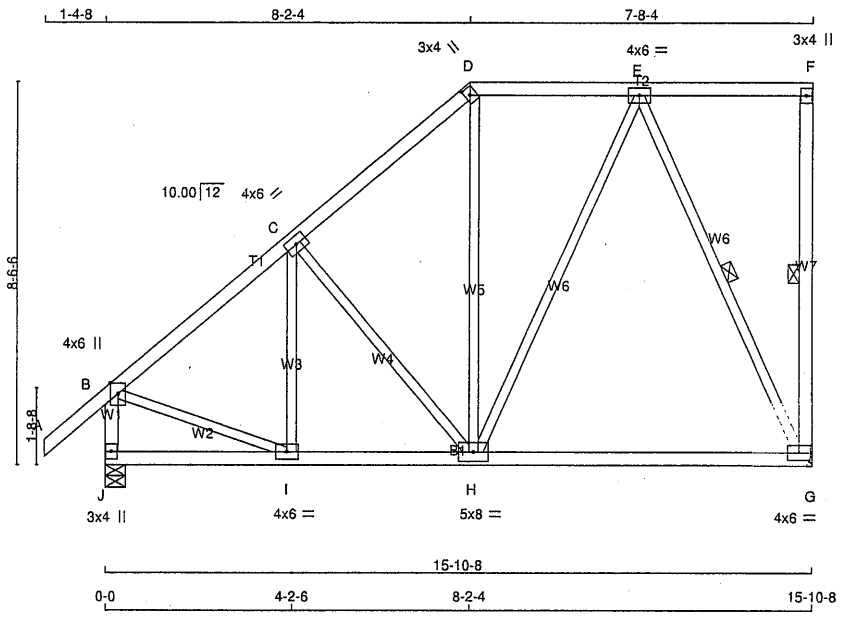

Scale = 1:49.1

TOTAL WEIGHT = 4 X 84 = 338 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMWW-t	MT20	4.0	6.0		
D	TTW+h	MT20	3.0	4.0	2.00	1.25
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	6.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BMWW-t	MT20	4.0	6.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	820	0	820	0	0	MECHANICAL	
J	945	0	945	0	0	5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	582	370 / 0	0 / 0	0 / 0	0 / 0	213 / 0	0 / 0
J	668	438 / 0	0 / 0	0 / 0	0 / 0	230 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, E-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-84.9	-84.9	0.13 (1)	10.00	I- C	-124 / 2	0.05 (1)
B- C	-699 / 0	-84.9	-84.9	0.19 (1)	6.25	C- H	-238 / 0	0.17 (1)
C- D	-555 / 0	-84.9	-84.9	0.18 (1)	6.25	H- D	0 / 89	0.03 (4)
D- E	-405 / 0	-84.9	-84.9	0.16 (1)	6.25	B- I	0 / 586	0.13 (1)
E- F	0 / 0	-84.9	-84.9	0.21 (1)	10.00	H- E	0 / 257	0.06 (1)
G- F	-126 / 0	0.0	0.0	0.04 (1)	6.25	E- G	-704 / 0	0.40 (1)
J- B	-908 / 0	0.0	0.0	0.10 (1)	7.81			
J- I	0 / 0	-18.5	-18.5	0.08 (4)	10.00			
I- H	0 / 557	-18.5	-18.5	0.27 (4)	10.00			
H- G	0 / 300	-18.5	-18.5	0.25 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.53")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.53")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.21/1.00 (E-F:1) , BC=0.27/1.00 (H-I:4) , WB=0.40/1.00 (E-G:1) , SSI=0.16/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.58 (D) (INPUT = 0.90)
JSI METAL= 0.37 (B) (INPUT = 0.95)

Structural component only
DWG# T-2406339

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425055	T31	4	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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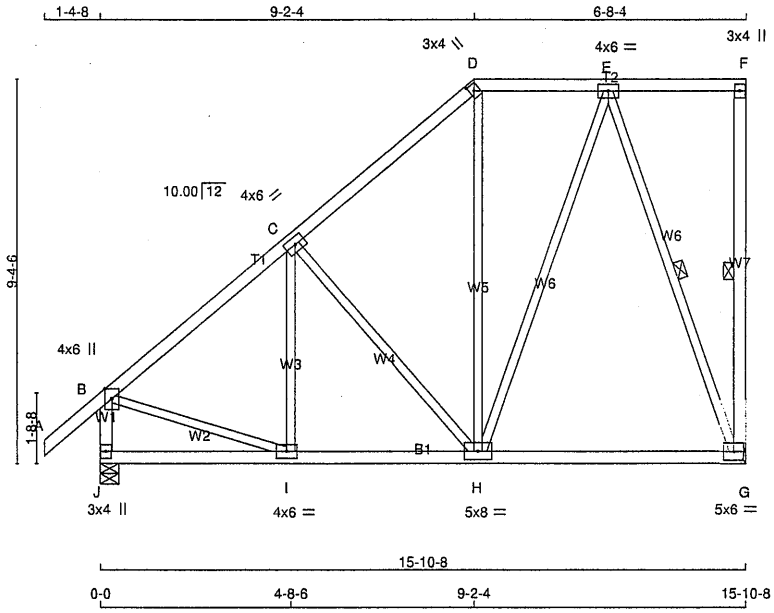

Scale = 1:53.7

TOTAL WEIGHT = 4 X 89 = 355 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMWW-t	MT20	4.0	6.0		
D	TTW+h	MT20	3.0	4.0	2.00	1.25
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	5.0	6.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BMWW-t	MT20	4.0	6.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	820	0	820	0	0	MECHANICAL	
J	945	0	945	0	0	5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	582	370 / 0	0 / 0	0 / 0	0 / 0	213 / 0	0 / 0
J	668	438 / 0	0 / 0	0 / 0	0 / 0	230 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, E-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-84.9	-84.9	0.13 (1)	10.00	I- C	-82 / 35	0.04 (1)
B- C	-698 / 0	-84.9	-84.9	0.24 (1)	6.25	C- H	-311 / 0	0.29 (1)
C- D	-495 / 0	-84.9	-84.9	0.23 (1)	6.25	H- D	0 / 63	0.02 (4)
D- E	-355 / 0	-84.9	-84.9	0.12 (1)	6.25	B- I	0 / 583	0.13 (1)
E- F	0 / 0	-84.9	-84.9	0.15 (1)	10.00	H- E	0 / 329	0.07 (1)
G- F	-110 / 0	0.0	0.0	0.05 (1)	6.25	E- G	-704 / 0	0.47 (1)
J- B	-907 / 0	0.0	0.0	0.10 (1)	7.81			
J- I	0 / 0	-18.5	-18.5	0.09 (4)	10.00			
I- H	0 / 559	-18.5	-18.5	0.22 (4)	10.00			
H- G	0 / 246	-18.5	-18.5	0.19 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.53")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.53")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.24/1.00 (B-C:1) , BC=0.22/1.00 (H-I:4) , WB=0.47/1.00 (E-G:1) , SSI=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.57 (B) (INPUT = 0.90)
JSI METAL= 0.38 (B) (INPUT = 0.95)

Structural component only
DWG# T-2406340

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425055	T32	4	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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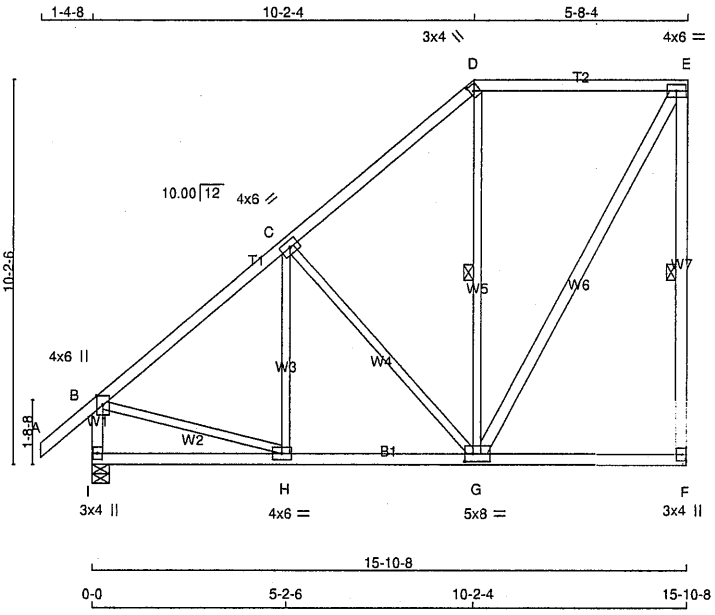

Scale = 1:58.3

TOTAL WEIGHT = 4 X 89 = 357 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMWW-t	MT20	4.0	6.0		
D	TTW+h	MT20	3.0	4.0	2.00	1.25
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	8.0	2.50	2.50
H	BMWW-t	MT20	4.0	6.0		
I	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2406341

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	820	0	820	0	0	MECHANICAL	
I	945	0	945	0	0	5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	582	370 / 0	0 / 0	0 / 0	0 / 0	213 / 0	0 / 0
I	668	438 / 0	0 / 0	0 / 0	0 / 0	230 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, D-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-84.9	-84.9	0.13 (1)	10.00	H- C	-46 / 61	0.03 (1)
B- C	-692 / 0	-84.9	-84.9	0.29 (1)	6.25	C- G	-380 / 0	0.45 (1)
C- D	-432 / 0	-84.9	-84.9	0.29 (1)	6.25	G- D	-145 / 16	0.10 (1)
D- E	-303 / 0	-84.9	-84.9	0.35 (1)	6.25	G- E	0 / 617	0.10 (1)
F- E	-778 / 0	0.0	0.0	0.40 (1)	6.25	B- H	0 / 577	0.13 (1)
I- B	-905 / 0	0.0	0.0	0.10 (1)	7.81			
I- H	0 / 0	-18.5	-18.5	0.11 (4)	10.00			
H- G	0 / 558	-18.5	-18.5	0.18 (4)	10.00			
G- F	0 / 0	-18.5	-18.5	0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.53")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.53")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.40/1.00 (E-F:1) , BC=0.18/1.00 (G-H:4) , WB=0.45/1.00 (C-G:1) , SSI=0.19/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.56 (B) (INPUT = 0.90)
JSI METAL= 0.38 (B) (INPUT = 0.95)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425055	T33	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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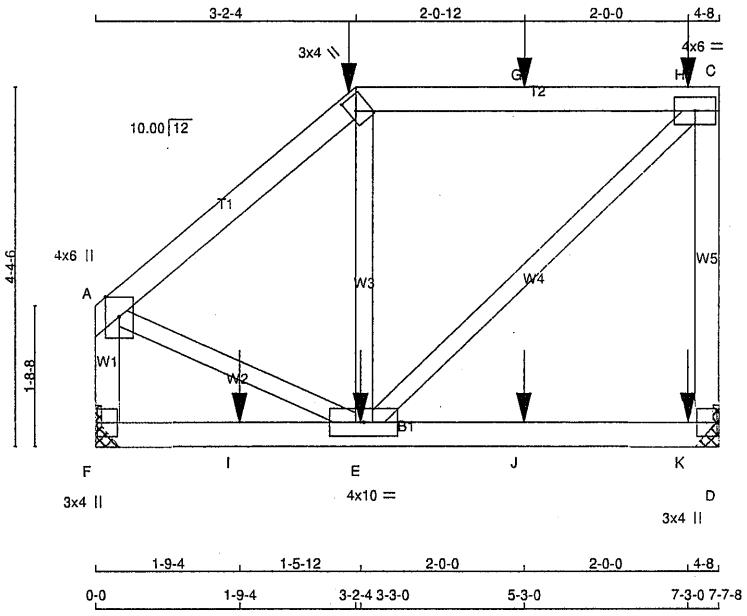

Scale = 1:26.7

TOTAL WEIGHT = 2 X 35 = 69 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B	TTW+h	MT20	3.0	4.0	2.00	1.00
C	TMVW-t	MT20	4.0	6.0		
D	BMV1+p	MT20	3.0	4.0		
E	BMWWW-t	MT20	4.0	10.0		
F	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2406342

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
D	1266	0	1266	0	0	MECHANICAL	
F	1081	0	1081	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D, F. MINIMUM BEARING LENGTH AT JOINT D = 1-8, JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	896	587 / 0	0 / 0	0 / 0	0 / 0	309 / 0	0 / 0
F	764	504 / 0	0 / 0	0 / 0	0 / 0	260 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.24 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FR-TO	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	FR-TO	MAX CSI (LC)
A- B	-901 / 0		-84.9	-84.9	0.18 (1)	6.24	E- B	-161 / 42		0.05 (1)
B- G	-685 / 0		-84.9	-84.9	0.42 (1)	6.25	E- C	0 / 941		0.23 (1)
G- H	-685 / 0		-84.9	-84.9	0.42 (1)	6.25	A- E	0 / 747		0.18 (1)
H- C	-685 / 0		-84.9	-84.9	0.42 (1)	6.25				
D- C	-918 / 0		0.0	0.0	0.28 (1)	7.81				
F- A	-1002 / 0		0.0	0.0	0.12 (1)	7.78				
F- I	0 / 0		-18.5	-18.5	0.36 (1)	10.00				
I- E	0 / 0		-18.5	-18.5	0.36 (1)	10.00				
E- J	0 / 0		-18.5	-18.5	0.36 (1)	10.00				
J- K	0 / 0		-18.5	-18.5	0.36 (1)	10.00				
K- D	0 / 0		-18.5	-18.5	0.36 (1)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
B	3-2-4	-47	-47	---	FRONT	VERT	DEAD	---	C1
B	3-2-4	-38	-38	---	BACK	VERT	TOTAL	---	C1
B	3-2-4	-180	-180	---	FRONT	VERT	SNOW	---	C1
E	3-3-0	-187	-187	---	BACK	VERT	TOTAL	---	C1
G	5-3-0	-32	-32	---	BACK	VERT	TOTAL	---	C1
H	7-3-0	-50	-50	---	BACK	VERT	TOTAL	---	C1
I	1-9-4	-187	-187	---	BACK	VERT	TOTAL	---	C1
J	5-3-0	-187	-187	---	BACK	VERT	TOTAL	---	C1
K	7-3-0	-192	-192	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.25")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.25")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.42/1.00 (B-C:1) , BC=0.36/1.00 (E-F:1) , WB=0.23/1.00 (C-E:1) , SSI=0.30/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.68 (B) (INPUT = 0.90)
JSI METAL= 0.41 (A) (INPUT = 0.95)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425055	T34	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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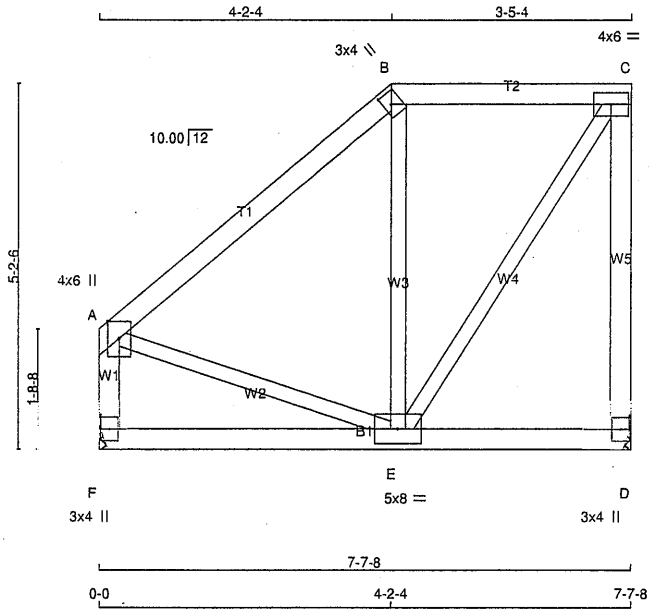

Scale = 1:31.3

TOTAL WEIGHT = 2 X 37 = 75 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B	TTW+h	MT20	3.0	4.0	2.00	1.25
C	TMVW-t	MT20	4.0	6.0		
D	BMV1+p	MT20	3.0	4.0		
E	BMWWW-t	MT20	5.0	8.0		
F	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
D	394	0	394	0	0	IN-SX	IN-SX
F	394	0	394	0	0	MECHANICAL	
						MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D, F. MINIMUM BEARING LENGTH AT JOINT D = 1-8, JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	280	178 / 0	0 / 0	0 / 0	0 / 0	102 / 0	0 / 0
F	280	178 / 0	0 / 0	0 / 0	0 / 0	102 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-207 / 0	-84.9	-84.9	0.19 (1)	6.25	E- B	-192 / 0	0.08 (1)
B- C	-154 / 0	-84.9	-84.9	0.13 (1)	6.25	E- C	0 / 275	0.06 (1)
D- C	-372 / 0	0.0	0.0	0.17 (1)	7.81	A- E	0 / 166	0.04 (1)
F- A	-361 / 0	0.0	0.0	0.04 (1)	7.81			
F- E	0 / 0	-18.5	-18.5	0.08 (4)	10.00			
E- D	0 / 0	-18.5	-18.5	0.08 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.25")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.25")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.19/1.00 (A-B:1) , BC=0.08/1.00 (D-E:4) , WB=0.08/1.00 (B-E:1) , SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.28 (B) (INPUT = 0.90)
JSI METAL= 0.13 (A) (INPUT = 0.95)

Structural component only
DWG# T-2406343

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425055	T35	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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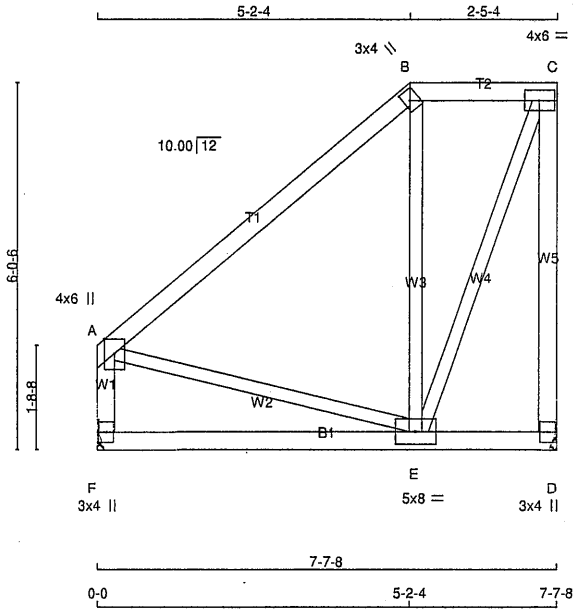

Scale = 1:36.2

TOTAL WEIGHT = 2 X 40 = 81 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B	TTW+h	MT20	3.0	4.0	2.00	1.25
C	TMVW-t	MT20	4.0	6.0		
D	BMV1+p	MT20	3.0	4.0		
E	BMWWW-t	MT20	5.0	8.0		
F	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
D	394	0	394	0	0	MECHANICAL	
F	394	0	394	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D, F. MINIMUM BEARING LENGTH AT JOINT D = 1-8, JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	280	178 / 0	0 / 0	0 / 0	0 / 0	102 / 0	0 / 0
F	280	178 / 0	0 / 0	0 / 0	0 / 0	102 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-158 / 0	-84.9	-84.9	0.29 (1)	6.25	E- B	-223 / 0	0.13 (1)
B- C	-115 / 0	-84.9	-84.9	0.06 (1)	6.25	E- C	0 / 311	0.07 (1)
D- C	-392 / 0	0.0	0.0	0.27 (1)	7.81	A- E	0 / 125	0.03 (1)
F- A	-353 / 0	0.0	0.0	0.04 (1)	7.81			
F- E	0 / 0	-18.5	-18.5	0.11 (4)	10.00			
E- D	0 / 0	-18.5	-18.5	0.11 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.25")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.25")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.29/1.00 (A-B:1) , BC=0.11/1.00 (E-F:4) , WB=0.13/1.00 (B-E:1) , SSI=0.13/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.41 (C) (INPUT = 0.90)
JSI METAL= 0.12 (A) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
02-28-24
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2406344

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425055	T36	2	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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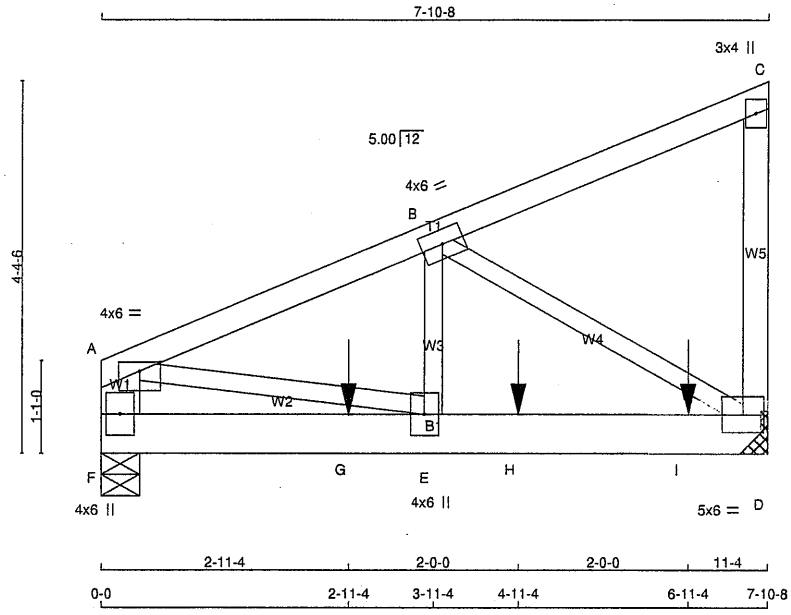

TOTAL WEIGHT = 4 X 37 = 147 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x6	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS		
A- C 1	12	TOP
C- D 1	12	TOP
F- A 2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS		
F- D 2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	Edge	
B	TMWW-t	MT20	4.0	6.0		

Structural component only
DWG# T-2406345

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	1370	0	1370	0	0	MECHANICAL	
F	1258	0	1258	0	0	5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	968	639 / 0	0 / 0	0 / 0	0 / 0	329 / 0	0 / 0
F	889	587 / 0	0 / 0	0 / 0	0 / 0	302 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-1673 / 0	-84.9	-84.9	0.11 (1)	6.25	E- B	0 / 1171	0.14 (1)
B- C	-13 / 0	-84.9	-84.9	0.10 (1)	6.25	B- D	-1810 / 0	0.30 (1)
D- C	-134 / 0	0.0	0.0	0.02 (1)	7.81	A- E	0 / 1580	0.20 (1)
F- A	-1057 / 0	0.0	0.0	0.04 (1)	7.81			
F- G	0 / 0	-18.5	-18.5	0.19 (1)	10.00			
G- E	0 / 0	-18.5	-18.5	0.19 (1)	10.00			
E- H	0 / 1556	-18.5	-18.5	0.25 (1)	10.00			
H- I	0 / 1556	-18.5	-18.5	0.25 (1)	10.00			
I- D	0 / 1556	-18.5	-18.5	0.25 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	2-11-4	-749	-749	---	BACK	VERT	TOTAL	---	C1
H	4-11-4	-265	-265	---	BACK	VERT	TOTAL	---	C1
I	6-11-4	-265	-265	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.26")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.26")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.11/1.00 (A-B:1) , BC=0.25/1.00 (D-E:1) , WB=0.30/1.00 (B-D:1) , SSI=0.31/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.65 (E) (INPUT = 0.90)
JSI METAL= 0.35 (E) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
425055	T36	2	2	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
C	TMV+p	MT20	3.0	4.0		
D	BMVW1-t	MT20	5.0	6.0		
E	BMWW+t	MT20	4.0	6.0		
F	BMV1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
02-28-24
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2406345

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425055	T36Z	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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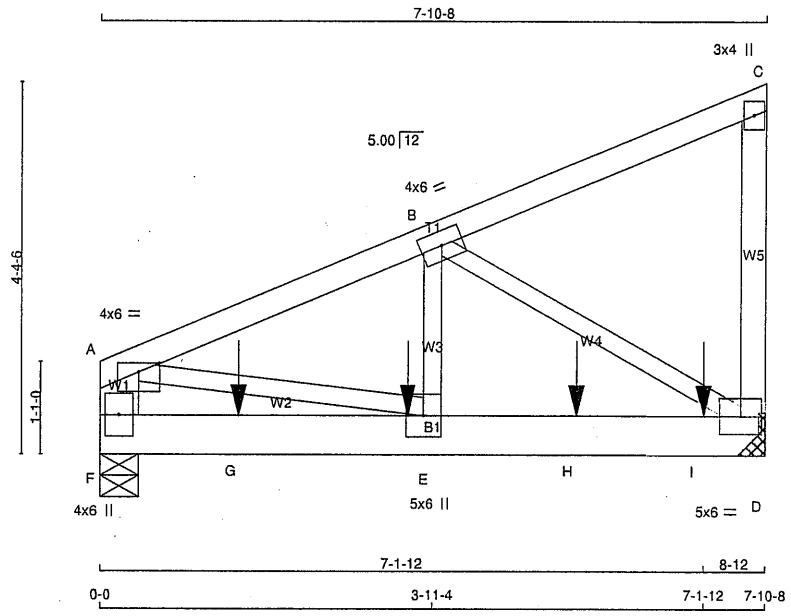

TOTAL WEIGHT = 2 X 37 = 73 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x6	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)		LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- D	1	12	TOP
F- A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
B- E	1	6	SIDE(31.7)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	Edge	

Structural component only
DWG# T-2406346

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
D	2330	0	2330	0	0	MECHANICAL	
F	1779	0	1779	0	0	5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	1645	1097 / 0	0 / 0	0 / 0	0 / 0	548 / 0	0 / 0
F	1256	836 / 0	0 / 0	0 / 0	0 / 0	420 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.80 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-2293 / 0	-84.9	-84.9	0.11 (1)	5.80	E- B	0 / 1755	0.22 (1)
B- C	-12 / 0	-84.9	-84.9	0.09 (1)	6.25	B- D	-2474 / 0	0.41 (1)
D- C	-137 / 0	0.0	0.0	0.02 (1)	7.81	A- E	0 / 2161	0.27 (1)
F- A	-1400 / 0	0.0	0.0	0.05 (1)	7.81			
F- G	0 / 0	-18.5	-18.5	0.22 (1)	10.00			
G- E	0 / 0	-18.5	-18.5	0.22 (1)	10.00			
E- H	0 / 2128	-18.5	-18.5	0.43 (1)	10.00			
H- I	0 / 2128	-18.5	-18.5	0.43 (1)	10.00			
I- D	0 / 2128	-18.5	-18.5	0.43 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	3-7-12	-580	-580	---	FRONT	VERT	TOTAL	---	C1
G	1-7-12	-580	-580	---	FRONT	VERT	TOTAL	---	C1
H	5-7-12	-580	-580	---	FRONT	VERT	TOTAL	---	C1
I	7-1-12	-582	-582	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.26")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.26")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.11/1.00 (A-B:1) , BC=0.43/1.00 (D-E:1) , WB=0.41/1.00 (B-D:1) , SSI=0.31/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.68 (B) (INPUT = 0.90)
JSI METAL= 0.38 (E) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
425055	T36Z	1	2	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	4.0	6.0		
C	TMV+p	MT20	3.0	4.0		
D	BMVW1-t	MT20	5.0	6.0		
E	BMWW+t	MT20	5.0	6.0		
F	BMV1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2406346

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425055	T36Z2	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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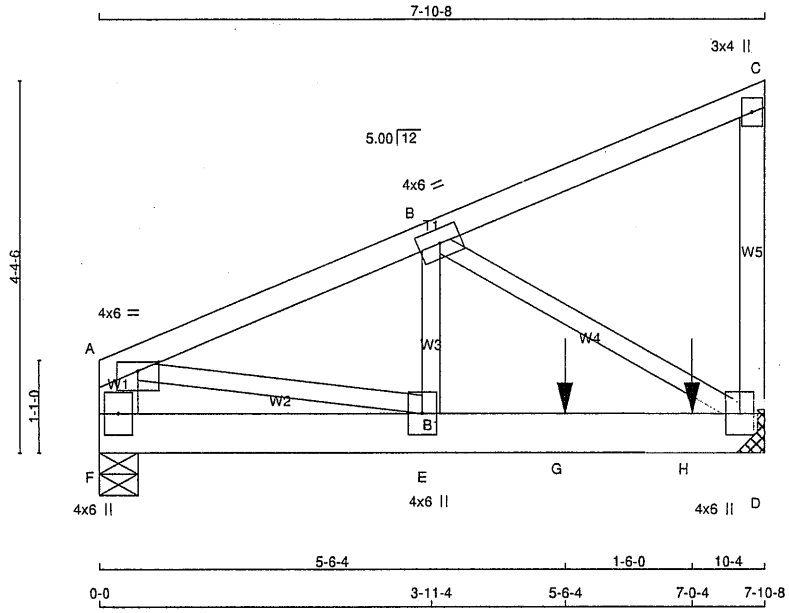

Scale = 1:25.8

TOTAL WEIGHT = 2 X 37 = 73 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x6	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- D	1	12	TOP
F- A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(21.5)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	Edge	
B	TMWW-t	MT20	4.0	6.0		

Structural component only
DWG# T-2406347

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
D	1743	0	1743	0	0	MECHANICAL	
F	978	0	978	0	0	5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	1232	811 / 0	0 / 0	0 / 0	0 / 0	420 / 0	0 / 0
F	692	450 / 0	0 / 0	0 / 0	0 / 0	243 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-1536 / 0	-84.9	-84.9	0.11 (1)	6.25		E- B	0 / 1042	0.13 (1)
B- C	-13 / 0	-84.9	-84.9	0.10 (1)	6.25		B- D	-1663 / 0	0.28 (1)
D- C	-133 / 0	0.0	0.0	0.02 (1)	7.81		A- E	0 / 1452	0.18 (1)
F- A	-982 / 0	0.0	0.0	0.03 (1)	7.81				
F- E	0 / 0	-61.6	-61.6	0.18 (1)	10.00				
E- G	0 / 1430	-61.6	-61.6	0.49 (1)	10.00				
G- H	0 / 1430	-18.5	-18.5	0.49 (1)	10.00				
H- D	0 / 1430	-18.5	-18.5	0.49 (1)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	5-6-4	-872	-872	---	BACK	VERT	TOTAL	---	C1
H	7-0-4	-306	-306	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 4-0-0
END DISTANCE = 5-6-4
END SPAN CARRIED = 4-0-0
END WALL WIDTH = 4-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.26")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.26")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.11/1.00 (A-B:1) , BC=0.49/1.00 (D-E:1) , WB=0.28/1.00 (B-D:1) , SSI=0.35/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (D) (INPUT = 0.90)
JSI METAL= 0.32 (D) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
425055	T36Z2	1	2	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMWW+t	MT20	4.0	6.0		
F	BMV1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2406347

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
425055	T36Z3	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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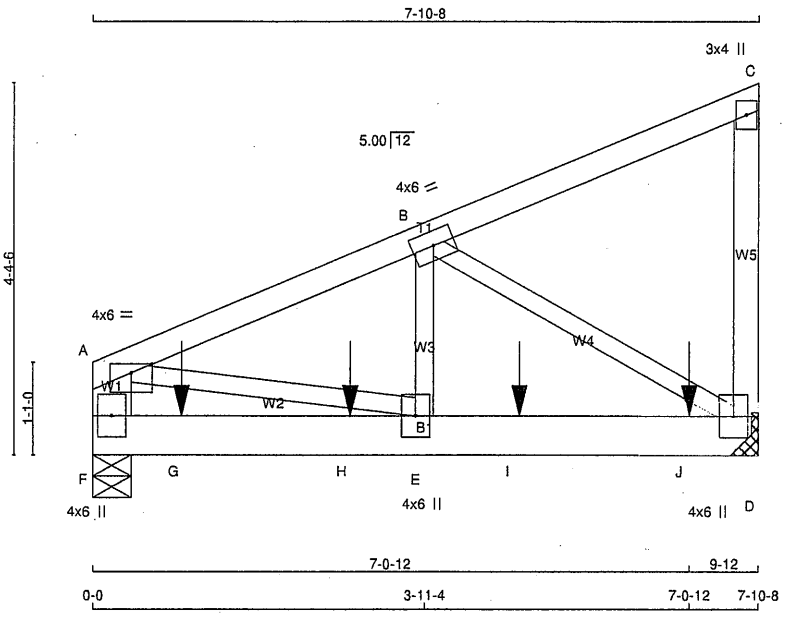

Scale = 1:25.8

TOTAL WEIGHT = 2 X 37 = 73 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x6	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS		
A- C 1	12	TOP
C- D 1	12	TOP
F- A 2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS		
F- D 2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	Edge	
B	TMWW-t	MT20	4.0	6.0		

Structural component only
DWG# T-2406348

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	1312	0	1312	0	0	MECHANICAL	
F	1255	0	1255	0	0	5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	928	608 / 0	0 / 0	0 / 0	0 / 0	320 / 0	0 / 0
F	888	582 / 0	0 / 0	0 / 0	0 / 0	306 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-1381 / 0	-84.9	-84.9	0.11 (1)	6.25	E- B	0 / 897	0.11 (1)
B- C	-13 / 0	-84.9	-84.9	0.10 (1)	6.25	B- D	-1497 / 0	0.25 (1)
D- C	-132 / 0	0.0	0.0	0.02 (1)	7.81	A- E	0 / 1307	0.16 (1)
F- A	-896 / 0	0.0	0.0	0.03 (1)	7.81			
F- G	0 / 0	-18.5	-18.5	0.14 (1)	10.00			
G- H	0 / 0	-18.5	-18.5	0.14 (1)	10.00			
H- E	0 / 0	-18.5	-18.5	0.14 (1)	10.00			
E- I	0 / 1287	-18.5	-18.5	0.22 (1)	10.00			
I- J	0 / 1287	-18.5	-18.5	0.22 (1)	10.00			
J- D	0 / 1287	-18.5	-18.5	0.22 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-0-12	-309	-309	---	FRONT	VERT	TOTAL	---	C1
H	3-0-12	-309	-309	---	FRONT	VERT	TOTAL	---	C1
I	5-0-12	-309	-309	---	FRONT	VERT	TOTAL	---	C1
J	7-0-12	-311	-311	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.26")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.26")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.11/1.00 (A-B:1) , BC=0.22/1.00 (D-E:1) , WB=0.25/1.00 (B-D:1) , SSI=0.20/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.71 (D) (INPUT = 0.90)
JSI METAL= 0.29 (D) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
425055	T36Z3	1	2	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMWW+t	MT20	4.0	6.0		
F	BMV1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2406348

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425915	T36Z4	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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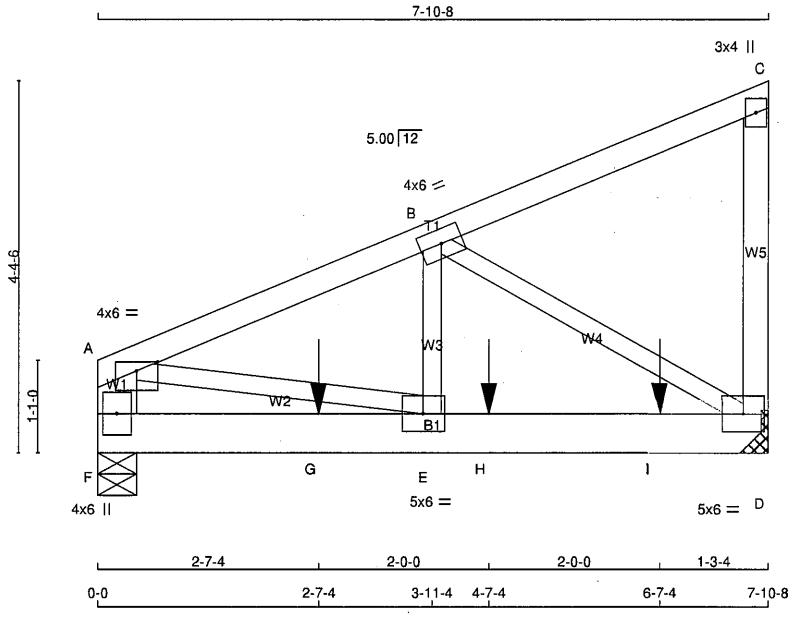

Scale = 1:25.8

TOTAL WEIGHT = 2 X 37 = 73 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x6	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS		
A- C 1	12	TOP
C- D 1	12	TOP
F- A 2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS		
F- D 2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	Edge	
B	TMWW-t	MT20	4.0	6.0		

Structural component only
DWG# T-2406481

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
D	1808	0	1808	0	0	MECHANICAL	
F	1737	0	1737	0	0	5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	1277	845 / 0	0 / 0	0 / 0	0 / 0	433 / 0	0 / 0
F	1224	824 / 0	0 / 0	0 / 0	0 / 0	400 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.82 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-2274 / 0	-84.9	-84.9	0.11 (1)	5.82	E- B	0 / 1737	0.21 (1)
B- C	-12 / 0	-84.9	-84.9	0.09 (1)	6.25	B- D	-2454 / 0	0.41 (1)
D- C	-137 / 0	0.0	0.0	0.02 (1)	7.81	A- E	0 / 2143	0.27 (1)
F- A	-1389 / 0	0.0	0.0	0.05 (1)	7.81			
F- G	0 / 0	-18.5	-18.5	0.31 (1)	10.00			
G- E	0 / 0	-18.5	-18.5	0.31 (1)	10.00			
E- H	0 / 2110	-18.5	-18.5	0.38 (1)	10.00			
H- I	0 / 2110	-18.5	-18.5	0.38 (1)	10.00			
I- D	0 / 2110	-18.5	-18.5	0.38 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	2-7-4	-999	-999	---	BACK	VERT	TOTAL	---	C1
H	4-7-4	-462	-462	---	FRONT	VERT	TOTAL	---	C1
I	6-7-4	-462	-462	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.26")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.26")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.11/1.00 (A-B:1) , BC=0.38/1.00 (D-E:1) , WB=0.41/1.00 (B-D:1) , SSI=0.38/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.67 (B) (INPUT = 0.90)
JSI METAL= 0.24 (B) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425915	T36Z4	1	2	ROYAL PINE HOMES TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
C	TMV+p	MT20	3.0	4.0		
D	BMVW1-t	MT20	5.0	6.0		
E	BMWW-t	MT20	5.0	6.0		
F	BMV1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2406481

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425915	T36Z5	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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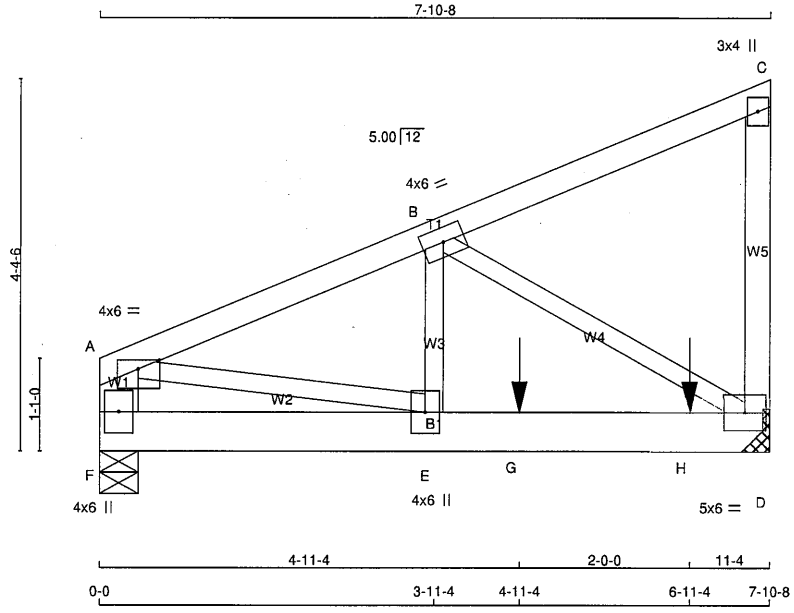

Scale = 1:25.8

TOTAL WEIGHT = 2 X 37 = 73 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x6	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- D	1	12	TOP
F- A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(21.5)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	Edge	
B	TMWW-t	MT20	4.0	6.0		

Structural component only
DWG# T-2406482

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	1905	0	1905	0	0	MECHANICAL	
F	1135	0	1135	0	0	5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	1350	870 / 0	0 / 0	0 / 0	0 / 0	480 / 0	0 / 0
F	805	515 / 0	0 / 0	0 / 0	0 / 0	290 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-1817 / 0	-84.9	-84.9	0.11 (1)	6.25	E- B	0 / 1307	0.16 (1)
B- C	-13 / 0	-84.9	-84.9	0.10 (1)	6.25	B- D	-1964 / 0	0.33 (1)
D- C	-135 / 0	0.0	0.0	0.02 (1)	7.81	A- E	0 / 1715	0.21 (1)
F- A	-1137 / 0	0.0	0.0	0.04 (1)	7.81			
F- E	0 / 0	-61.6	-61.6	0.18 (1)	10.00			
E- G	0 / 1689	-61.6	-61.6	0.42 (1)	10.00			
G- H	0 / 1689	-18.5	-18.5	0.42 (1)	10.00			
H- D	0 / 1689	-18.5	-18.5	0.42 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	4-11-4	-956	-956	---	BACK	VERT	TOTAL	---	C1
H	6-11-4	-470	-470	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 4-0-0
END DISTANCE = 4-11-4
END SPAN CARRIED = 4-0-0
END WALL WIDTH = 4-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.26")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.26")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.11/1.00 (A-B:1) , BC=0.42/1.00 (D-E:1) , WB=0.33/1.00 (B-D:1) , SSI=0.45/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.71 (E) (INPUT = 0.90)
JSI METAL= 0.38 (E) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425915	T36Z5	1	2	ROYAL PINE HOMES TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
C	TMV+p	MT20	3.0	4.0		
D	BMVW1-t	MT20	5.0	6.0		
E	BMWW+t	MT20	4.0	6.0		
F	BMV1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2406482

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425055	T37	2	3	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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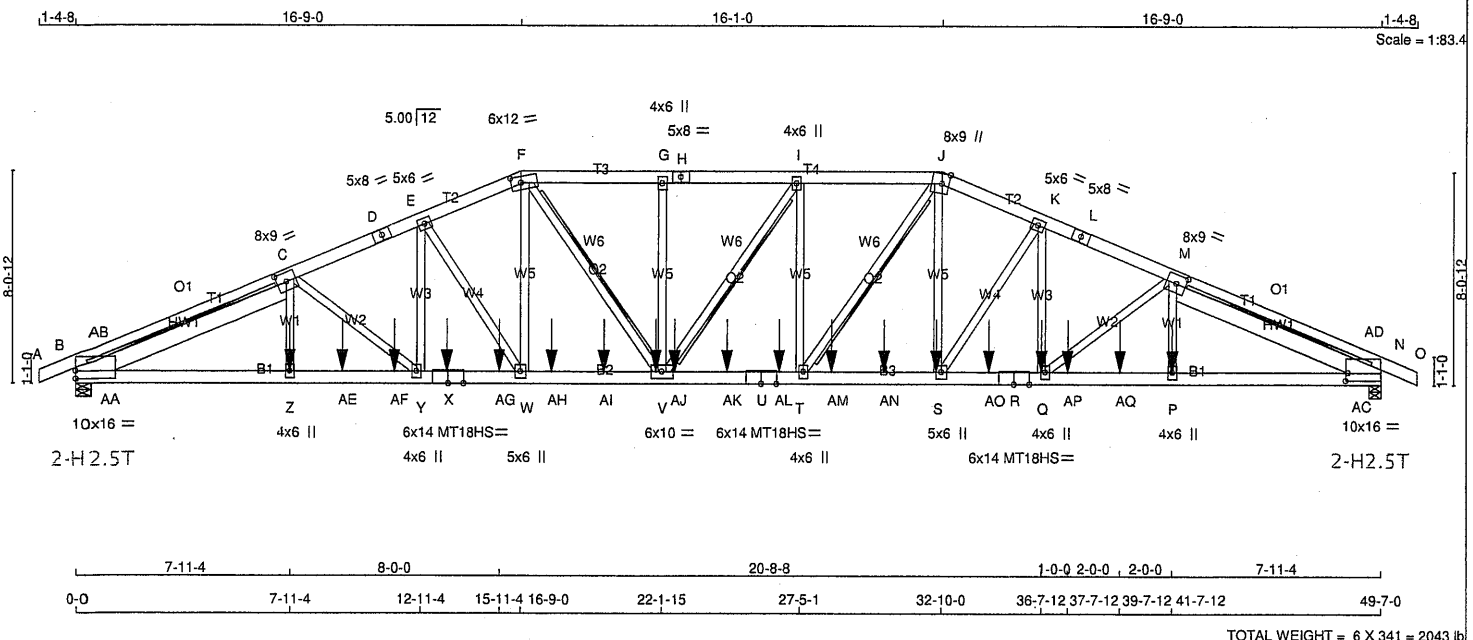

TOTAL WEIGHT = 6 X 341 = 2043 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
J - L	2x6	DRY	No.2	SPF
L - O	2x6	DRY	No.2	SPF
B - X	2x6	DRY	2100F 1.8E	SPF
X - U	2x6	DRY	2100F 1.8E	SPF
U - R	2x6	DRY	2100F 1.8E	SPF
R - N	2x6	DRY	2100F 1.8E	SPF
REINFORCING MEMBERS				
HW1	2x8	DRY	No.2	SPF
HW2	2x8	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-D	2	12	TOP
D-F	2	12	TOP
F-H	2	12	TOP
H-J	2	12	TOP
J-L	2	12	TOP
L-O	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
B-X	2	11	SIDE(454.9)
X-U	2	11	SIDE(454.9)
U-R	2	12	SIDE(341.2)
R-N	2	12	SIDE(341.2)
WEBS : (0.122"X3") SPIRAL NAILS			
C-Z	1	5	SIDE(220.0)
2x4	1	6	
K-Q	1	6	SIDE(264.7)
M-P	1	6	SIDE(38.5)
2x8	2	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

Structural component only
DWG# T-2406349

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	12593	0	12720	192	-1158	5-8	4-10
N	12178	0	12305	0	-1158	5-8	4-7

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 1158 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT N FOR 1158 LBS FACTORED UPLIFT

PROVIDE FOR 192 LBS FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	9051	5721 / 0	521 / 0	0 / 0	0 / -1155	2894 / 0	0 / 0
N	8750	5570 / 0	521 / 0	0 / 0	0 / -1155	2743 / 0	0 / 0
HORIZONTAL REACTIONS							
B	---	0 / 0	0 / 0	0 / 0	137 / -137	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, N

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 2.84 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x6 DRY SPF No.2 T-BRACE AT J-T, F-V, I-V, C-AA, M-AC

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

LOADING

TOTAL LOAD CASES: (18)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	CSI	LC	MAX. UNBRAC LENGTH
A-B	0 / 0	-105.2	-105.2	0.03	(2)	10.00
B-AB	-16904 / 1457	-105.2	-105.2	0.27	(2)	3.53
AB-C	-12907 / 1352	-105.2	-105.2	0.25	(2)	4.04
C-D	-24466 / 2001	-105.2	-105.2	0.45	(2)	2.85
D-E	-24466 / 2001	-105.2	-105.2	0.45	(2)	2.85
E-F	-22433 / 1881	-105.2	-105.2	0.34	(2)	3.06
F-G	-23087 / 1917	-105.2	-105.2	0.40	(2)	2.97
G-H	-23087 / 1917	-105.2	-105.2	0.37	(1)	3.00
H-I	-23087 / 1917	-105.2	-105.2	0.37	(1)	3.00
I-J	-22989 / 1921	-105.2	-105.2	0.39	(3)	2.98
J-K	-22445 / 1881	-105.2	-105.2	0.34	(3)	3.06
K-L	-24535 / 2001	-105.2	-105.2	0.45	(3)	2.84
L-M	-24535 / 2001	-105.2	-105.2	0.45	(3)	2.84
M-AD	-12516 / 1352	-105.2	-105.2	0.24	(3)	4.11
AD-N	-16397 / 1457	-105.2	-105.2	0.26	(3)	3.59
N-O	0 / 0	-105.2	-105.2	0.03	(3)	10.00
B-AA	-1277 / 11890	-39.5	-39.5	0.24	(2)	6.25
AA-Z	-1948 / 24213	-39.5	-39.5	0.44	(2)	6.25
Z-AE	-1949 / 24153	-39.5	-39.5	0.52	(2)	6.25
AE-AF	-1949 / 24153	-39.5	-39.5	0.52	(2)	6.25
AF-Y	-1949 / 24153	-39.5	-39.5	0.52	(2)	6.25
Y-X	-1702 / 22585	-39.5	-39.5	0.46	(1)	6.25
X-AG	-1702 / 22585	-39.5	-39.5	0.46	(1)	6.25
AG-W	-1702 / 22585	-39.5	-39.5	0.46	(1)	6.25
W-AH	-1479 / 20786	-39.5	-39.5	0.43	(1)	6.25
AH-AI	-1479 / 20786	-39.5	-39.5	0.43	(1)	6.25
AI-V	-1479 / 20786	-39.5	-39.5	0.43	(1)	6.25
V-AJ	-1554 / 22989	-39.5	-39.5	0.49	(1)	6.25

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	CSI	LC
Z-C	0 / 3017	0.16	(2)
C-Y	-2014 / 316	0.32	(2)
Y-E	-148 / 3225	0.17	(2)
E-W	-3571 / 417	0.79	(2)
W-F	-323 / 5043	0.27	(2)
S-J	-319 / 5211	0.28	(3)
S-K	-3654 / 418	0.81	(3)
Q-K	-150 / 3334	0.18	(3)
Q-M	-862 / 317	0.14	(3)
P-M	0 / 1819	0.10	(3)
T-J	-445 / 4254	0.23	(2)
F-V	-436 / 4450	0.24	(3)
V-I	-200 / 543	0.03	(2)
T-I	-1097 / 410	0.29	(2)
V-G	-625 / 323	0.16	(2)
AA-AB	-236 / 6550	0.00	(1)
AA-C	-14153 / 768	0.91	(1)
M-AC	-13573 / 769	0.87	(1)
AC-AD	-236 / 6348	0.00	(1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	30.1	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	54.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 5.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF BCBC 2018 , NBC-2019AE
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR NOT USED

(80 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 30.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.35")
ALLOWABLE DEFL.(TL)= L/180 (3.31")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.54")

CSI: TC=0.45/1.00 (K-M:3) , BC=0.52/1.00 (Y-Z:2) , WB=0.91/1.00 (C-AA:1) , SSI=0.27/1.00 (T-V:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873
MT18HS	586	403	2455	1382	3163	3004

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (F) (INPUT = 0.90)
JSI METAL= 0.80 (F) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425055	T37	2	3	ROYAL PINE HOMES TRUSS DESC.	

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMW1-I	MT20	10.0	16.0	3.50	
C	TMWWW-t	MT20	8.0	9.0	4.00	4.25
D, H, L						
D	TS-t	MT20	5.0	8.0		
E	TMWW-t	MT20	5.0	6.0		
F	TTWW-m	MT20	6.0	12.0	2.75	4.50
G	TMW+w	MT20	4.0	6.0		
I	TMWW+t	MT20	4.0	6.0		
J	TTWW+m	MT20	8.0	9.0	Edge	3.25
K	TMWW-t	MT20	5.0	6.0		
M	TMWWW-t	MT20	8.0	9.0	4.00	4.25
N	TMBMW1-I	MT20	10.0	16.0	3.50	1.00
P	BMW+w	MT20	4.0	6.0		
Q, T, Y						
Q	BMWW+t	MT20	4.0	6.0		
R, U, X						
R	BS-t	MT18HS	6.0	14.0		
S	BMWW+t	MT20	5.0	6.0		
V	BMWWW-t	MT20	6.0	10.0		
W	BMWW+t	MT20	5.0	6.0		
Z	BMW+w	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LOADING
TOTAL LOAD CASES: (18)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
AJ-AK	-1554 / 22989	-39.5	-39.5	0.49 (1)	6.25
AK- U	-1554 / 22989	-39.5	-39.5	0.49 (1)	6.25
U-AL	-1554 / 22989	-39.5	-39.5	0.49 (1)	6.25
AL- T	-1554 / 22989	-39.5	-39.5	0.49 (1)	6.25
T-AM	-1296 / 20803	-39.5	-39.5	0.45 (1)	6.25
AM-AN	-1296 / 20803	-39.5	-39.5	0.45 (1)	6.25
AN- S	-1296 / 20803	-39.5	-39.5	0.45 (1)	6.25
S-AO	-1510 / 22643	-39.5	-39.5	0.45 (1)	6.25
AO- R	-1510 / 22643	-39.5	-39.5	0.45 (1)	6.25
R- Q	-1510 / 22643	-39.5	-39.5	0.45 (1)	6.25
Q-AP	-1758 / 23312	-39.5	-39.5	0.45 (3)	6.25
AP-AQ	-1758 / 23312	-39.5	-39.5	0.45 (3)	6.25
AQ- P	-1758 / 23312	-39.5	-39.5	0.45 (3)	6.25
P-AC	-1757 / 23349	-39.5	-39.5	0.43 (3)	6.25
AC- N	-1085 / 11532	-39.5	-39.5	0.23 (3)	6.25

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
P	41-7-12	-881	-881	---	BACK	VERT	TOTAL	---	C1
Q	36-7-12	-1341	-1341	---	BACK	VERT	TOTAL	---	C1
S	32-7-12	-568	-568	---	BACK	VERT	TOTAL	---	C1
V	21-11-4	-568	-568	---	BACK	VERT	TOTAL	---	C1
X	13-11-4	-572	-572	---	BACK	VERT	TOTAL	---	C1
Z	7-11-4	-1525	-1525	---	BACK	VERT	TOTAL	---	C1
AE	9-11-4	-572	-572	---	BACK	VERT	TOTAL	---	C1
AF	11-11-4	-572	-572	---	BACK	VERT	TOTAL	---	C1
AG	15-11-4	-568	-568	---	BACK	VERT	TOTAL	---	C1
AH	17-11-4	-568	-568	---	BACK	VERT	TOTAL	---	C1
AI	19-11-4	-568	-568	---	BACK	VERT	TOTAL	---	C1
AJ	22-7-12	-568	-568	---	BACK	VERT	TOTAL	---	C1
AK	24-7-12	-568	-568	---	BACK	VERT	TOTAL	---	C1
AL	26-7-12	-568	-568	---	BACK	VERT	TOTAL	---	C1
AM	28-7-12	-568	-568	---	BACK	VERT	TOTAL	---	C1
AN	30-7-12	-568	-568	---	BACK	VERT	TOTAL	---	C1
AO	34-7-12	-568	-568	---	BACK	VERT	TOTAL	---	C1
AP	37-7-12	-265	-265	---	BACK	VERT	TOTAL	---	C1
AQ	39-7-12	-265	-265	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.2} PSF AT {30-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.

Structural component only
DWG# T-2406349

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425055	T38	6	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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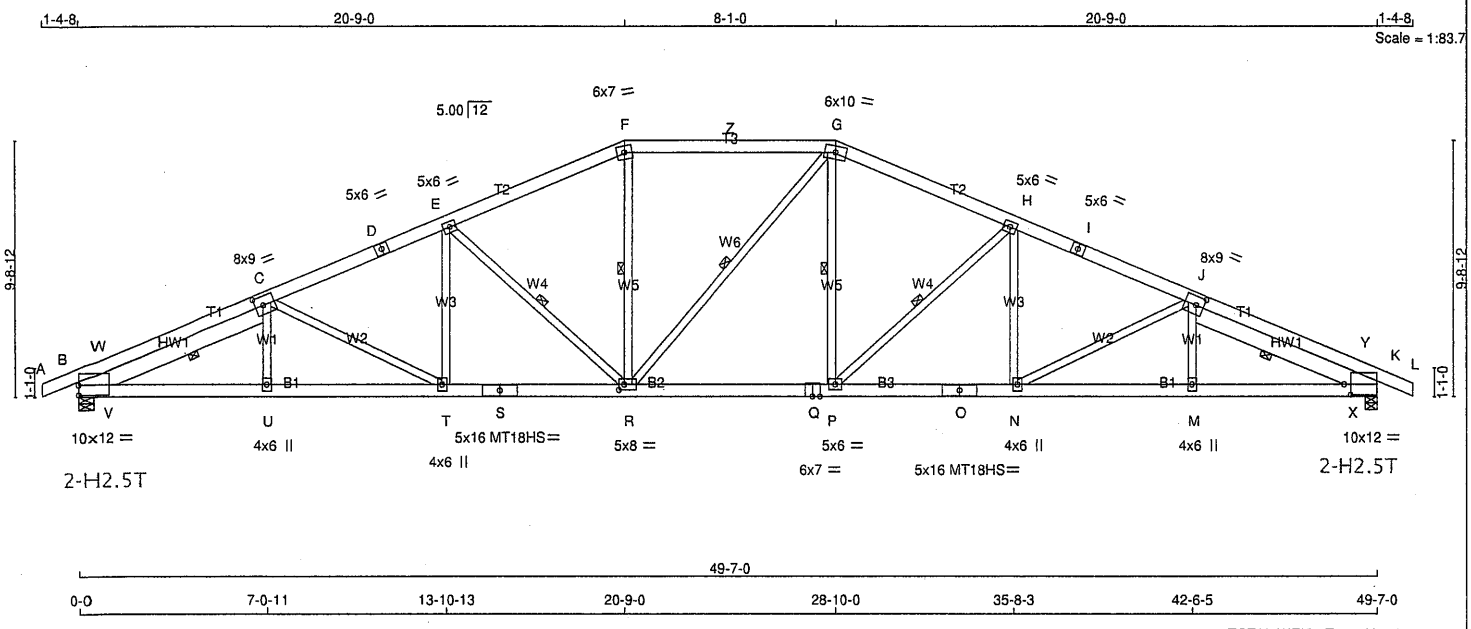

TOTAL WEIGHT = 6 X 317 = 1900 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
F - G	2x6	DRY	No.2	SPF
G - I	2x6	DRY	No.2	SPF
I - L	2x6	DRY	No.2	SPF
B - S	2x6	DRY	No.2	SPF
S - Q	2x6	DRY	No.2	SPF
Q - O	2x6	DRY	No.2	SPF
O - K	2x6	DRY	No.2	SPF
REINFORCING MEMBERS				
HW1	2x8	DRY	No.2	SPF
HW2	2x8	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMW1-I	MT20	10.0	12.0	4.75	
C	TMWWW-t	MT20	8.0	9.0	4.00	3.50
D	TS-t	MT20	5.0	6.0		
E	TMWW-t	MT20	5.0	6.0		
F	TTW-m	MT20	6.0	7.0		
G	TTWW-m	MT20	6.0	10.0		
H	TMWW-t	MT20	5.0	6.0		
I	TS-t	MT20	5.0	6.0		
J	TMWWW-t	MT20	8.0	9.0	4.00	3.50
K	TMBMW1-I	MT20	10.0	12.0	4.75	3.00
M	BMW+w	MT20	4.0	6.0		
N	BMWW+t	MT20	4.0	6.0		
O	BS-t	MT18HS	5.0	16.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BS-t	MT20	6.0	7.0		
R	BMWWW-t	MT20	5.0	8.0	2.50	2.50
S	BS-t	MT18HS	5.0	16.0		
T	BMWW+t	MT20	4.0	6.0		
U	BMW+w	MT20	4.0	6.0		

NOTES (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2406350

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
B	3782	0	3782	234	-1036	5-8	4-2
K	3782	0	3782	0	-1036	5-8	4-2

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 1036 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT K FOR 1036 LBS FACTORED UPLIFT

PROVIDE FOR 234 LBS FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	2814	1579 / 0	521 / 0	0 / 0	0 / -1089	714 / 0	0 / 0
K	2814	1579 / 0	521 / 0	0 / 0	0 / -1089	714 / 0	0 / 0
HORIZONTAL REACTIONS							
B	---	0 / 0	0 / 0	0 / 0	167 / -167	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, K

BRACING
FOR SECTION F-G, MAX. UNBRACED TOP CHORD LENGTH = 2.00 FT.
FOR OTHER SECTIONS, MAX. UNBRACED TOP CHORD LENGTH = 3.06 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 5.97 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-R, F-R, G-R, G-P, H-P, C-V, J-X.

LOADING
TOTAL LOAD CASES: (18)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF) CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 0	-105.2	-105.2	0.09 (2)	10.00
B- W	-4994 / 1302	-105.2	-105.2	0.28 (2)	3.73
W- C	-3836 / 1176	-105.2	-105.2	0.31 (2)	4.18
C- D	-6258 / 1682	-105.2	-105.2	0.64 (1)	3.06
D- E	-6258 / 1682	-105.2	-105.2	0.64 (1)	3.06
E- F	-5196 / 1401	-105.2	-105.2	0.55 (1)	3.42
F- Z	-4788 / 1373	-115.2	-115.2	0.89 (1)	2.00
Z- G	-4788 / 1373	-115.2	-115.2	0.89 (1)	2.00
G- H	-5193 / 1400	-105.2	-105.2	0.55 (1)	3.42
H- I	-6259 / 1683	-105.2	-105.2	0.64 (1)	3.06
I- J	-6259 / 1683	-105.2	-105.2	0.64 (1)	3.06
J- Y	-3837 / 1176	-105.2	-105.2	0.32 (3)	4.18
Y- K	-4995 / 1303	-105.2	-105.2	0.28 (3)	3.73
K- L	0 / 0	-105.2	-105.2	0.09 (3)	10.00
B- V	-1172 / 3544	-39.5	-39.5	0.44 (1)	6.25
V- U	-1763 / 6298	-39.5	-39.5	0.86 (1)	5.97
U- T	-1765 / 6291	-39.5	-39.5	0.84 (1)	5.99
T- S	-1465 / 5788	-39.5	-39.5	0.81 (1)	6.25
S- R	-1465 / 5788	-39.5	-39.5	0.81 (1)	6.25
R- Q	-867 / 4786	-39.5	-39.5	0.69 (1)	6.25
Q- P	-867 / 4786	-39.5	-39.5	0.69 (1)	6.25
P- O	-1231 / 5789	-39.5	-39.5	0.80 (1)	6.25
O- N	-1231 / 5789	-39.5	-39.5	0.80 (1)	6.25
N- M	-1532 / 6291	-39.5	-39.5	0.84 (1)	6.25
M- X	-1531 / 6298	-39.5	-39.5	0.86 (1)	6.25
X- K	-938 / 3544	-39.5	-39.5	0.44 (1)	6.25

WEBS MEMB.	FORCE (LBS)	MAX. FACTORED CSI (LC)
U- C	0 / 338	0.06 (17)
C- T	-775 / 339	0.64 (2)
T- E	-96 / 635	0.10 (5)
E- R	-1609 / 638	0.70 (2)
R- F	-289 / 1238	0.20 (1)
R- G	-730 / 735	0.59 (3)
P- G	-364 / 1383	0.22 (3)
P- H	-1612 / 638	0.70 (3)
N- H	-97 / 639	0.10 (6)
N- J	-774 / 341	0.64 (3)
M- J	0 / 336	0.06 (17)
V- W	-227 / 1678	0.00 (1)
V- C	-3181 / 683	0.41 (1)
J- X	-3180 / 685	0.41 (1)
X- Y	-228 / 1678	0.00 (1)

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	30.1	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	54.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 5.00/12 AND -5.00/12 AND RESPECTIVE HEEL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 4.0 P.S.F.

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF BCBC 2018 , NBC-2019AE
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR NOT USED

(80 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 30.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.29")
ALLOWABLE DEFL.(TL)= L/180 (3.31")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.43")

CSI: TC=0.89/1.00 (F-G:1) , BC=0.86/1.00 (M-X:1) , WB=0.70/1.00 (H-P:3) , SSI=0.28/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873
MT18HS	586	403	2455	1382	3163	3004

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (K) (INPUT = 0.90)
JSI METAL= 0.72 (Q) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
425055	T38	6	1	TRUSS DESC.	

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.2} PSF AT {30-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.

Structural component only
DWG# T-2406350

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425055	T41	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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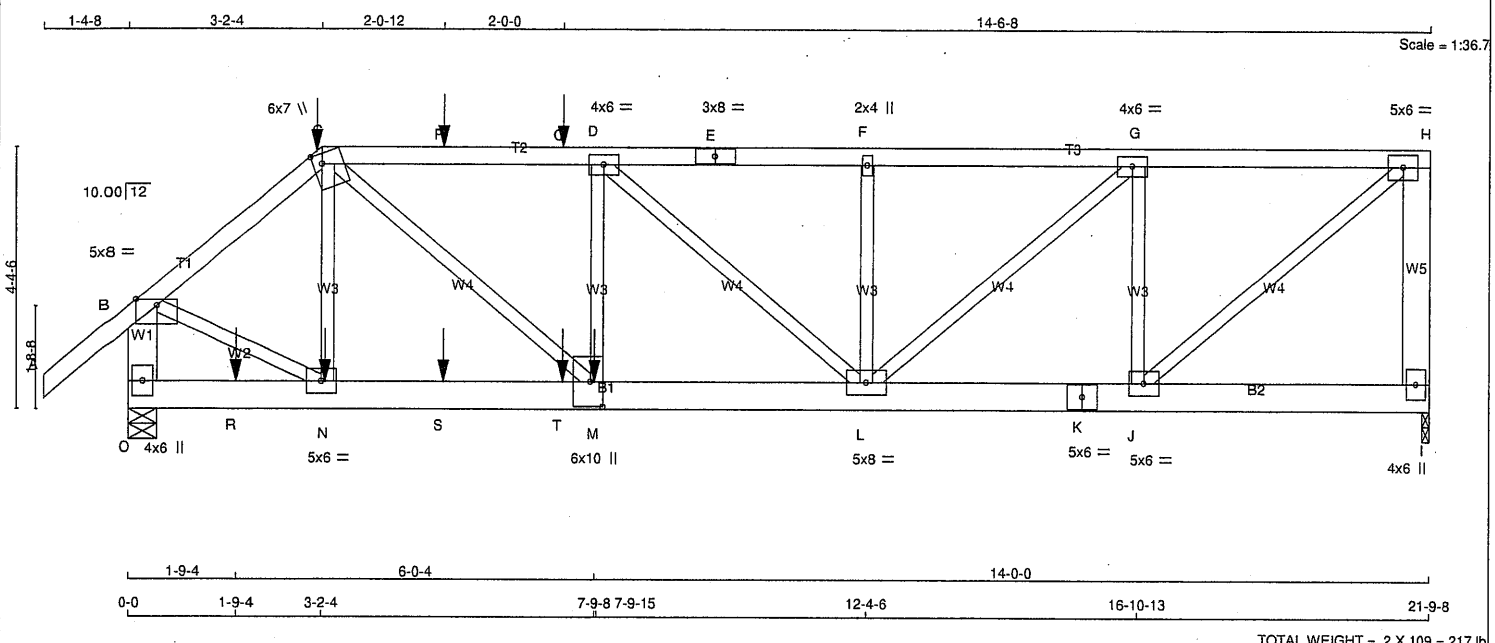

TOTAL WEIGHT = 2 X 109 = 217 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
I - H	2x6	DRY	No.2	SPF
O - B	2x6	DRY	No.2	SPF
O - K	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- E	1	12	SIDE(61.0)
E- H	1	12	TOP
H- I	2	12	TOP
O- B	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
O- K	2	12	SIDE(183.1)
K- I	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
M- D	1	5	SIDE(419.9)

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

Structural component only
DWG# T-2406351

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	2251	0	2251	0	0	1-8	1-8
O	3965	0	3965	0	0	5-8	2-2

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1593	1040 / 0	0 / 0	0 / 0	0 / 0	553 / 0	0 / 0
O	2799	1862 / 0	0 / 0	0 / 0	0 / 0	938 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.95 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-84.9	-84.9	0.07 (1)	10.00	N- C	-336 / 0	0.05 (1)
B- C	-3857 / 0	-84.9	-84.9	0.14 (1)	4.71	B- N	0 / 3171	0.39 (1)
C- P	-5380 / 0	-84.9	-84.9	0.30 (1)	3.95	J- H	0 / 3226	0.40 (1)
P- Q	-5380 / 0	-84.9	-84.9	0.30 (1)	3.95	C- M	0 / 3206	0.40 (1)
Q- D	-5380 / 0	-84.9	-84.9	0.30 (1)	3.95	J- G	-1937 / 0	0.27 (1)
D- E	-4179 / 0	-84.9	-84.9	0.22 (1)	4.48	M- D	0 / 576	0.07 (1)
E- F	-4179 / 0	-84.9	-84.9	0.22 (1)	4.48	L- G	0 / 2236	0.28 (1)
F- G	-4179 / 0	-84.9	-84.9	0.23 (1)	4.47	D- L	-1600 / 0	0.50 (1)
G- H	-2500 / 0	-84.9	-84.9	0.18 (1)	5.54	L- F	-347 / 0	0.05 (1)
I- H	-2207 / 0	0.0	0.0	0.20 (1)	7.81			
O- B	-3897 / 0	0.0	0.0	0.15 (1)	7.17			
O- R	0 / 0	-18.5	-18.5	0.10 (1)	10.00			
R- N	0 / 0	-18.5	-18.5	0.10 (1)	10.00			
N- S	0 / 2944	-18.5	-18.5	0.29 (1)	10.00			
S- T	0 / 2944	-18.5	-18.5	0.29 (1)	10.00			
T- M	0 / 2944	-18.5	-18.5	0.29 (1)	10.00			
M- L	0 / 5380	-18.5	-18.5	0.39 (1)	10.00			
L- K	0 / 2500	-18.5	-18.5	0.19 (1)	10.00			
K- J	0 / 2500	-18.5	-18.5	0.19 (1)	10.00			
J- I	0 / 0	-18.5	-18.5	0.03 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-2-4	-47	-47	---	FRONT	VERT	DEAD	---	C1
C	3-2-4	-38	-38	---	FRONT	VERT	TOTAL	---	C1
C	3-2-4	-180	-180	---	FRONT	VERT	SNOW	---	C1
M	7-9-8	-1630	-1630	---	FRONT	VERT	TOTAL	---	C1
N	3-3-0	-187	-187	---	FRONT	VERT	TOTAL	---	C1
P	5-3-0	-32	-32	---	FRONT	VERT	TOTAL	---	C1
Q	7-3-0	-32	-32	---	FRONT	VERT	TOTAL	---	C1
R	1-9-4	-187	-187	---	FRONT	VERT	TOTAL	---	C1
S	5-3-0	-187	-187	---	FRONT	VERT	TOTAL	---	C1
T	7-3-0	-187	-187	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.30/1.00 (C-D:1) , BC=0.39/1.00 (L-M:1) , WB=0.50/1.00 (D-L:1) , SSI=0.13/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (N) (INPUT = 0.90)
JSI METAL= 0.37 (M) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
425055	T41	1	2	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	6.0	7.0	2.00	1.75
D	TMWW-t	MT20	4.0	6.0		
E	TS-t	MT20	3.0	8.0		
F	TMW+w	MT20	2.0	4.0		
G	TMWW-t	MT20	4.0	6.0		
H	TMVW-t	MT20	5.0	6.0		
I	BMV1+p	MT20	4.0	6.0		
J	BMWW-t	MT20	5.0	6.0		
K	BS-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	5.0	8.0		
M	BMWW+t	MT20	6.0	10.0	5.00	2.50
N	BMWW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2406351

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425055	T41S	2	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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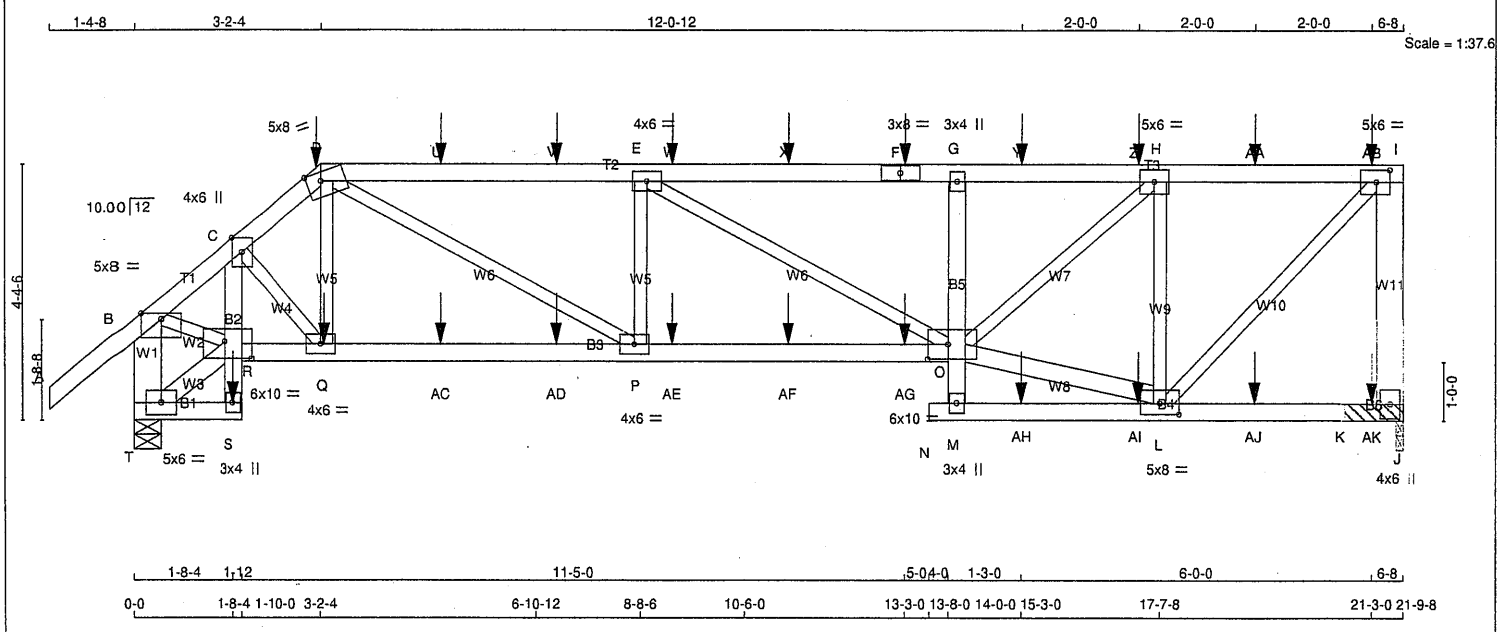

TOTAL WEIGHT = 4 X 104 = 417 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
J - I	2x6	DRY	No.2	SPF
T - B	2x6	DRY	No.2	SPF
T - S	2x4	DRY	No.2	SPF
S - C	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
M - G	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	SIDE(61.0)
D- F	1	12	SIDE(61.0)
F- I	1	12	SIDE(61.0)
I- J	2	12	SIDE(8.7)
T- B	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
T- S	1	12	SIDE(61.0)
S- C	1	12	SIDE(9.8)
R- O	1	12	SIDE(61.0)
G- M	1	12	TOP
N- J	1	12	SIDE(61.0)
WEBS : (0.122"X3") SPIRAL NAILS			
Q- D	1	6	SIDE(13.9)
2x3	1	6	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

Structural component only
DWG# T-2406352

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	2932	0	2932	0	0	1-8	1-8 & BLOCK
T	3099	0	3099	0	0	5-8	1-11

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	2081	1323 / 0	0 / 0	0 / 0	0 / 0	758 / 0	0 / 0
T	2196	1419 / 0	0 / 0	0 / 0	0 / 0	777 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, T

2x4 DRY SPF No.2 BEARING BLOCK 12" LONG AT JT. J ATTACHED TO FRONT SIDE WITH 2 ROWS OF (0.122"X3") SPIRAL NAILS SPACED 4" C.C. 4 NAILS TOTAL.

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-84.9	-84.9	0.07 (1)	10.00
B- C	-3678 / 0	-84.9	-84.9	0.11 (1)	4.82
C- D	-3710 / 0	-84.9	-84.9	0.10 (1)	4.82
D- U	-5149 / 0	-84.9	-84.9	0.35 (1)	4.00
U- V	-5149 / 0	-84.9	-84.9	0.35 (1)	4.00
V- E	-5149 / 0	-84.9	-84.9	0.35 (1)	4.00
E- W	-4641 / 0	-84.9	-84.9	0.32 (1)	4.20
W- X	-4641 / 0	-84.9	-84.9	0.32 (1)	4.20
X- F	-4641 / 0	-84.9	-84.9	0.32 (1)	4.20
F- G	-4641 / 0	-84.9	-84.9	0.32 (1)	4.20
G- Y	-4597 / 0	-84.9	-84.9	0.19 (1)	4.32
Y- Z	-4597 / 0	-84.9	-84.9	0.19 (1)	4.32
Z- H	-4597 / 0	-84.9	-84.9	0.19 (1)	4.32
H-AA	-2424 / 0	-84.9	-84.9	0.16 (1)	5.62
AA-AB	-2424 / 0	-84.9	-84.9	0.16 (1)	5.62
AB- I	-2424 / 0	-84.9	-84.9	0.16 (1)	5.62
J- I	-2628 / 0	0.0	0.0	0.26 (1)	7.81
T- B	-3031 / 0	0.0	0.0	0.12 (1)	7.81
T- S	0 / 79	-18.5	-18.5	0.01 (4)	10.00
S- R	0 / 287	0.0	0.0	0.09 (1)	10.00
R- C	-121 / 0	0.0	0.0	0.07 (1)	7.81
R- Q	0 / 2828	-18.5	-18.5	0.43 (1)	10.00
Q-AC	0 / 2870	-18.5	-18.5	0.62 (1)	10.00
AC-AD	0 / 2870	-18.5	-18.5	0.62 (1)	10.00
AD- P	0 / 2870	-18.5	-18.5	0.62 (1)	10.00
P-AE	0 / 5149	-18.5	-18.5	0.82 (1)	10.00
AE-AF	0 / 5149	-18.5	-18.5	0.82 (1)	10.00
AF-AG	0 / 5149	-18.5	-18.5	0.82 (1)	10.00
AG- O	0 / 5149	-18.5	-18.5	0.82 (1)	10.00
M- O	0 / 180	0.0	0.0	0.12 (1)	10.00
O- G	-395 / 0	0.0	0.0	0.10 (1)	7.81
N- M	0 / 0	-18.5	-18.5	0.00 (4)	10.00
M-AH	0 / 134	-18.5	-18.5	0.21 (1)	10.00
AH-AI	0 / 134	-18.5	-18.5	0.21 (1)	10.00
AI- L	0 / 134	-18.5	-18.5	0.21 (1)	10.00
L-AJ	0 / 0	-18.5	-18.5	0.20 (1)	10.00
AJ- K	0 / 0	-18.5	-18.5	0.20 (1)	10.00
K-AK	0 / 0	-18.5	-18.5	0.20 (1)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
C- Q	0 / 32	0.00 (1)
Q- D	0 / 489	0.06 (1)
L- H	-2345 / 0	0.35 (1)
T- R	-95 / 0	0.01 (1)
B- R	0 / 2810	0.35 (1)
O- L	0 / 2421	0.21 (1)
O- H	0 / 2845	0.35 (1)
L- I	0 / 3418	0.42 (1)
D- P	0 / 2605	0.32 (1)
P- E	-254 / 12	0.03 (1)
E- O	-583 / 0	0.20 (1)

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 957 (0.27")

CSI: TC=0.35/1.00 (D-E:1) , BC=0.82/1.00 (O-P:1) , WB=0.42/1.00 (I-L:1) , SSI=0.26/1.00 (O-P:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMVW+p	MT20	4.0	6.0	Edge	2.00
D	TTWW-m	MT20	5.0	8.0	Edge	3.00
E	TMWW-t	MT20	4.0	6.0		
F	TS-t	MT20	3.0	8.0		
G	TMV+p	MT20	3.0	4.0		
H	TMWW-t	MT20	5.0	6.0		
I	TMVW-t	MT20	5.0	6.0	2.50	2.75
J	BMV1+p	MT20	4.0	6.0		
L	BMWWW-t	MT20	5.0	8.0	2.25	4.00
M	BMV+p	MT20	3.0	4.0		
O	BVMWWW-l	MT20	6.0	10.0	3.00	4.25
P	BMWW-t	MT20	4.0	6.0		
Q	BMWW-t	MT20	4.0	6.0		
R	BVMWW-l	MT20	6.0	10.0	3.50	5.50
S	BMV+p	MT20	3.0	4.0		
T	BMVW1-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LOADING
TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM	TO		LENGTH	FR-TO			
AK- J	0 / 0	-18.5	-18.5	0.20 (1)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	3-2-4	-47	-47	---	FRONT	VERT	DEAD	---	C1
D	3-2-4	-9	-9	---	FRONT	VERT	TOTAL	---	C1
D	3-2-4	-180	-180	---	FRONT	VERT	SNOW	---	C1
F	13-3-0	-6	-6	---	FRONT	VERT	TOTAL	---	C1
Q	3-3-0	-205	-205	---	FRONT	VERT	TOTAL	---	C1
S	1-8-4	-194	-194	---	FRONT	VERT	TOTAL	---	C1
U	5-3-0	-6	-6	---	FRONT	VERT	TOTAL	---	C1
V	7-3-0	-6	-6	---	FRONT	VERT	TOTAL	---	C1
W	9-3-0	-6	-6	---	FRONT	VERT	TOTAL	---	C1
X	11-3-0	-6	-6	---	FRONT	VERT	TOTAL	---	C1
Y	15-3-0	-32	-32	---	FRONT	VERT	TOTAL	---	C1
Z	17-3-0	-32	-32	---	FRONT	VERT	TOTAL	---	C1
AA	19-3-0	-32	-32	---	FRONT	VERT	TOTAL	---	C1
AB	21-3-0	-46	-46	---	FRONT	VERT	TOTAL	---	C1
AC	5-3-0	-205	-205	---	FRONT	VERT	TOTAL	---	C1
AD	7-3-0	-205	-205	---	FRONT	VERT	TOTAL	---	C1
AE	9-3-0	-205	-205	---	FRONT	VERT	TOTAL	---	C1
AF	11-3-0	-205	-205	---	FRONT	VERT	TOTAL	---	C1
AG	13-3-0	-206	-206	---	FRONT	VERT	TOTAL	---	C1
AH	15-3-0	-187	-187	---	FRONT	VERT	TOTAL	---	C1
AI	17-3-0	-187	-187	---	FRONT	VERT	TOTAL	---	C1
AJ	19-3-0	-187	-187	---	FRONT	VERT	TOTAL	---	C1
AK	21-3-0	-191	-191	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

JSI GRIP= 0.80 (L) (INPUT = 0.90)
JSI METAL= 0.38 (O) (INPUT = 0.95)

Structural component only
DWG# T-2406352

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425055	T41Z	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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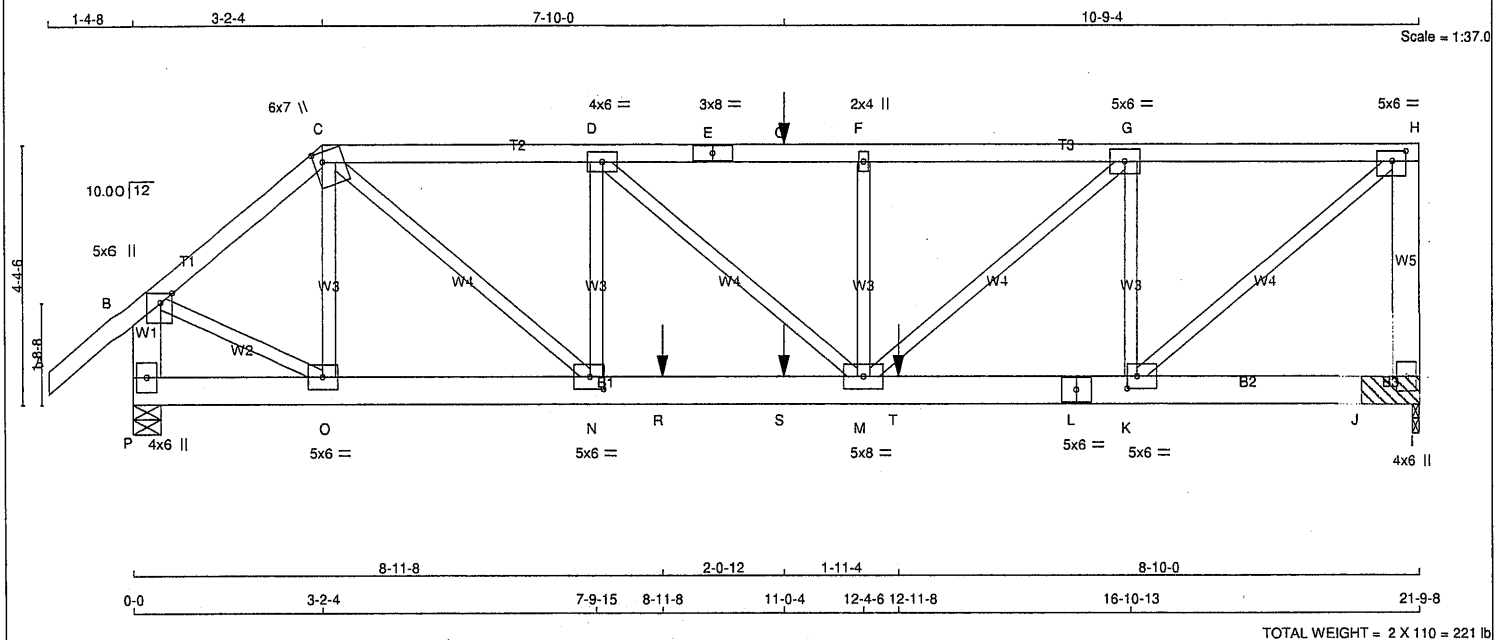

TOTAL WEIGHT = 2 X 110 = 221 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
I - H	2x6	DRY	No.2	SPF
P - B	2x6	DRY	No.2	SPF
P - L	2x6	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- E	1	12	TOP
E- H	1	12	SIDE(0.0)
H- I	2	12	TOP
P- B	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
P- L	2	12	SIDE(0.0)
L- I	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

Structural component only
DWG# T-2406353

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	2840	0	2840	0	0	1-8	1-8 & BLOCK
P	2865	0	2865	0	0	5-8	1-9

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	2009	1311 / 0	0 / 0	0 / 0	0 / 0	698 / 0	0 / 0
P	2025	1333 / 0	0 / 0	0 / 0	0 / 0	692 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, P

2x6 DRY SPF No.2 BEARING BLOCK 12" LONG AT JT. I ATTACHED TO FRONT SIDE WITH 3 ROWS OF (0.122"X3") SPIRAL NAILS SPACED 4" C.C. 6 NAILS TOTAL.

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.94 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-84.9	-84.9	0.07 (1)	10.00	O- C	-830 / 0	0.12 (1)
B- C	-2722 / 0	-84.9	-84.9	0.11 (1)	5.43	B- O	0 / 2238	0.28 (1)
C- D	-4710 / 0	-84.9	-84.9	0.24 (1)	4.25	K- H	0 / 4182	0.52 (1)
D- E	-5463 / 0	-84.9	-84.9	0.27 (1)	3.96	C- N	0 / 3485	0.43 (1)
E- Q	-5463 / 0	-84.9	-84.9	0.27 (1)	3.96	K- G	-2415 / 0	0.34 (1)
Q- F	-5463 / 0	-84.9	-84.9	0.27 (1)	3.96	N- D	-1100 / 0	0.15 (1)
F- G	-5463 / 0	-84.9	-84.9	0.28 (1)	3.94	M- G	0 / 2960	0.37 (1)
G- H	-3240 / 0	-84.9	-84.9	0.20 (1)	4.99	D- M	0 / 1002	0.12 (1)
I- H	-2812 / 0	0.0	0.0	0.25 (1)	7.81	M- F	-376 / 0	0.05 (1)
P- B	-2827 / 0	0.0	0.0	0.11 (1)	7.81			
P- O	0 / 0	-18.5	-18.5	0.01 (1)	10.00			
O- N	0 / 2062	-18.5	-18.5	0.26 (1)	10.00			
N- R	0 / 4710	-18.5	-18.5	0.61 (1)	10.00			
R- S	0 / 4710	-18.5	-18.5	0.61 (1)	10.00			
S- M	0 / 4710	-18.5	-18.5	0.61 (1)	10.00			
M- T	0 / 3240	-18.5	-18.5	0.41 (1)	10.00			
T- L	0 / 3240	-18.5	-18.5	0.41 (1)	10.00			
L- K	0 / 3240	-18.5	-18.5	0.41 (1)	10.00			
K- J	0 / 0	-18.5	-18.5	0.03 (4)	10.00			
J- I	0 / 0	-18.5	-18.5	0.03 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
Q	11-0-4	-32	-32	---	BACK	VERT	TOTAL	---	C1
R	8-11-8	-913	-913	---	BACK	VERT	TOTAL	---	C1
S	11-0-4	-187	-187	---	BACK	VERT	TOTAL	---	C1
T	12-11-8	-1217	-1217	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.28/1.00 (F-G:1) , BC=0.61/1.00 (M-N:1) , WB=0.52/1.00 (H-K:1) , SSI=0.54/1.00 (K-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (K) (INPUT = 0.90)
JSI METAL= 0.37 (K) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
425055	T41Z	1	2	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTWW+m	MT20	6.0	7.0	2.00	1.75
D	TMWW-t	MT20	4.0	6.0		
E	TS-t	MT20	3.0	8.0		
F	TMW+w	MT20	2.0	4.0		
G	TMWW-t	MT20	5.0	6.0		
H	TMVW-t	MT20	5.0	6.0	2.00	2.75
I	BMV1+p	MT20	4.0	6.0		
K	BMWW-t	MT20	5.0	6.0	2.50	2.00
L	BS-t	MT20	5.0	6.0		
M	BMWWW-t	MT20	5.0	8.0		
N	BMWW-t	MT20	5.0	6.0	2.50	2.75
O	BMWW-t	MT20	5.0	6.0		
P	BMV1+p	MT20	4.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2406353

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425055	T42	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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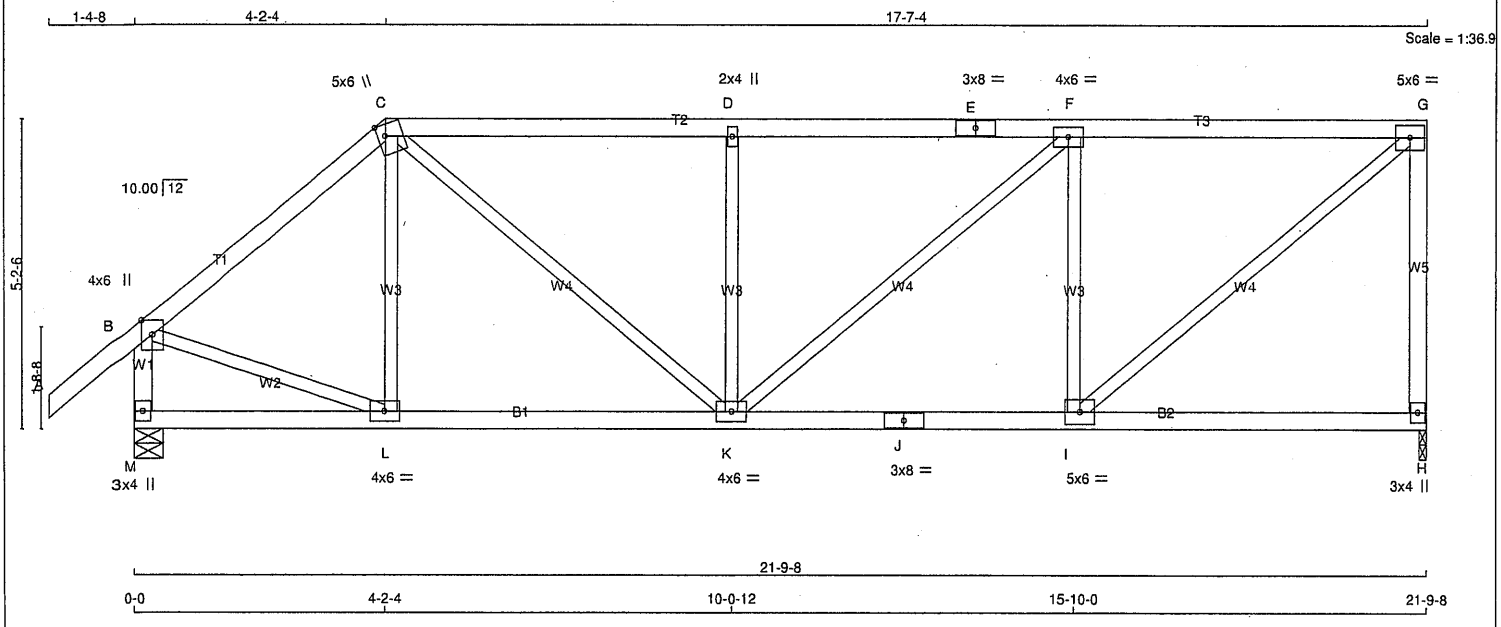

TOTAL WEIGHT = 2 X 92 = 185 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	8.0		
F	TMWW-t	MT20	4.0	6.0		
G	TMVW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	3.0	8.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	1126	0	1126	0	0	1-8	1-8
M	1251	0	1251	0	0	5-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	799	507 / 0	0 / 0	0 / 0	0 / 0	292 / 0	0 / 0
M	885	576 / 0	0 / 0	0 / 0	0 / 0	310 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.07 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)		FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40		-84.9	-84.9	0.13 (1)	10.00	L- C	-148 / 23	0.06 (1)
B- C	-1037 / 0		-84.9	-84.9	0.28 (1)	5.80	B- L	0 / 834	0.19 (1)
C- D	-1307 / 0		-84.9	-84.9	0.44 (1)	5.10	I- G	0 / 1379	0.31 (1)
D- E	-1307 / 0		-84.9	-84.9	0.45 (1)	5.07	C- K	0 / 671	0.15 (1)
E- F	-1307 / 0		-84.9	-84.9	0.45 (1)	5.07	I- F	-754 / 0	0.30 (1)
F- G	-1065 / 0		-84.9	-84.9	0.44 (1)	5.50	K- D	-534 / 0	0.21 (1)
H- G	-1081 / 0		0.0	0.0	0.50 (1)	7.59	K- F	0 / 318	0.07 (1)
M- B	-1222 / 0		0.0	0.0	0.13 (1)	7.25			
M- L	0 / 0		-18.5	-18.5	0.11 (4)	10.00			
L- K	0 / 791		-18.5	-18.5	0.19 (1)	10.00			
K- J	0 / 1065		-18.5	-18.5	0.25 (1)	10.00			
J- I	0 / 1065		-18.5	-18.5	0.25 (1)	10.00			
I- H	0 / 0		-18.5	-18.5	0.15 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.50/1.00 (G-H:1) , BC=0.25/1.00 (I-K:1) , WB=0.31/1.00 (G-I:1) , SSI=0.23/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (K) (INPUT = 0.90)
JSI METAL= 0.51 (B) (INPUT = 0.95)

Structural component only
DWG# T-2406354

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425055	T42S	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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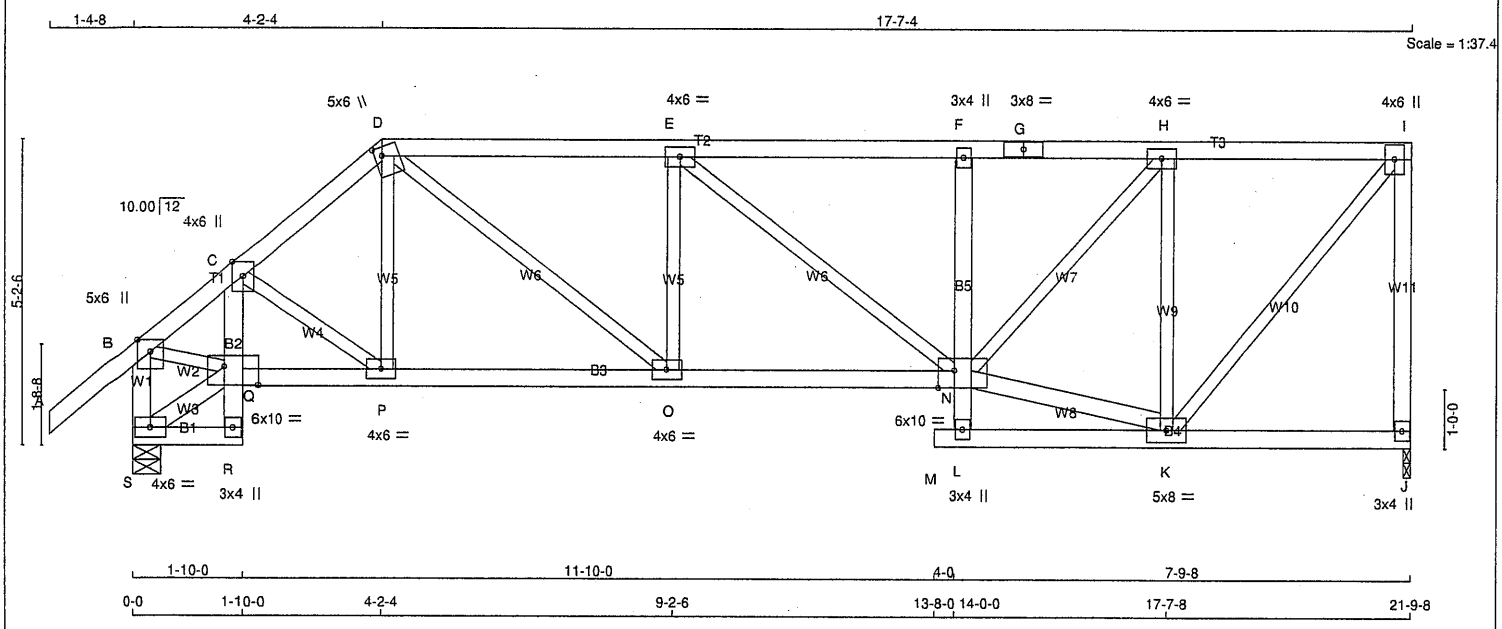

TOTAL WEIGHT = 2 X 106 = 212 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
S - R	2x4	DRY	No.2	SPF
R - C	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
L - F	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMVW+p	MT20	4.0	6.0	Edge	2.00
D	TTWW+m	MT20	5.0	6.0	1.75	1.50
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
G	TS-t	MT20	3.0	8.0		
H	TMWW-t	MT20	4.0	6.0		
I	TMVW+p	MT20	4.0	6.0		
J	BMV1+p	MT20	3.0	4.0		
K	BMWWW-t	MT20	5.0	8.0		
L	BMV+p	MT20	3.0	4.0		
N	BVMWWW-I	MT20	6.0	10.0	Edge	3.25
O	BMWW-t	MT20	4.0	6.0		
P	BMWW-t	MT20	4.0	6.0		
Q	BVMWW-I	MT20	6.0	10.0	3.75	6.50
R	BMV+p	MT20	3.0	4.0		
S	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1)

Structural component only
DWG# T-2406355

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1128	0	1128	0	0	1-8	1-8
S	1258	0	1258	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	801	505 / 0	0 / 0	0 / 0	0 / 0	296 / 0	0 / 0
S	891	578 / 0	0 / 0	0 / 0	0 / 0	313 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, S

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.82 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-84.9	-84.9	0.13 (1)	10.00
B- C	-1311 / 0	-84.9	-84.9	0.14 (1)	5.49
C- D	-1274 / 0	-84.9	-84.9	0.10 (1)	5.60
D- E	-1592 / 0	-84.9	-84.9	0.37 (1)	4.82
E- F	-1459 / 0	-84.9	-84.9	0.36 (1)	4.99
F- G	-1449 / 0	-84.9	-84.9	0.20 (1)	5.21
G- H	-1449 / 0	-84.9	-84.9	0.20 (1)	5.21
H- I	-793 / 0	-84.9	-84.9	0.19 (1)	6.25
J- I	-1097 / 0	0.0	0.0	0.51 (1)	7.55
S- B	-1222 / 0	0.0	0.0	0.13 (1)	7.25
S- R	0 / 31	-18.5	-18.5	0.02 (4)	10.00
R- Q	0 / 17	0.0	0.0	0.05 (1)	10.00
Q- C	-112 / 0	0.0	0.0	0.05 (1)	7.81
Q- P	0 / 1005	-18.5	-18.5	0.20 (1)	10.00
P- O	0 / 974	-18.5	-18.5	0.20 (1)	10.00
O- N	0 / 1593	-18.5	-18.5	0.31 (1)	10.00
L- N	0 / 37	0.0	0.0	0.06 (1)	10.00
N- F	-325 / 0	0.0	0.0	0.06 (1)	7.81
M- L	0 / 0	-18.5	-18.5	0.00 (4)	10.00
L- K	0 / 39	-18.5	-18.5	0.08 (4)	10.00
K- J	0 / 0	-18.5	-18.5	0.08 (4)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- P	-39 / 0	0.01 (1)
P- D	0 / 77	0.03 (4)
D- O	0 / 783	0.18 (1)
O- E	-380 / 0	0.10 (1)
E- N	-170 / 0	0.12 (1)
K- H	-1081 / 0	0.43 (1)
K- I	0 / 1237	0.28 (1)
S- Q	-37 / 0	0.00 (1)
B- Q	0 / 997	0.22 (1)
N- K	0 / 799	0.13 (1)
N- H	0 / 964	0.22 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.51/1.00 (I-J:1) , BC=0.31/1.00 (N-O:1) , WB=0.43/1.00 (H-K:1) , SSI=0.20/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (D) (INPUT = 0.90)
JSI METAL= 0.48 (B) (INPUT = 0.95)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425055	T43	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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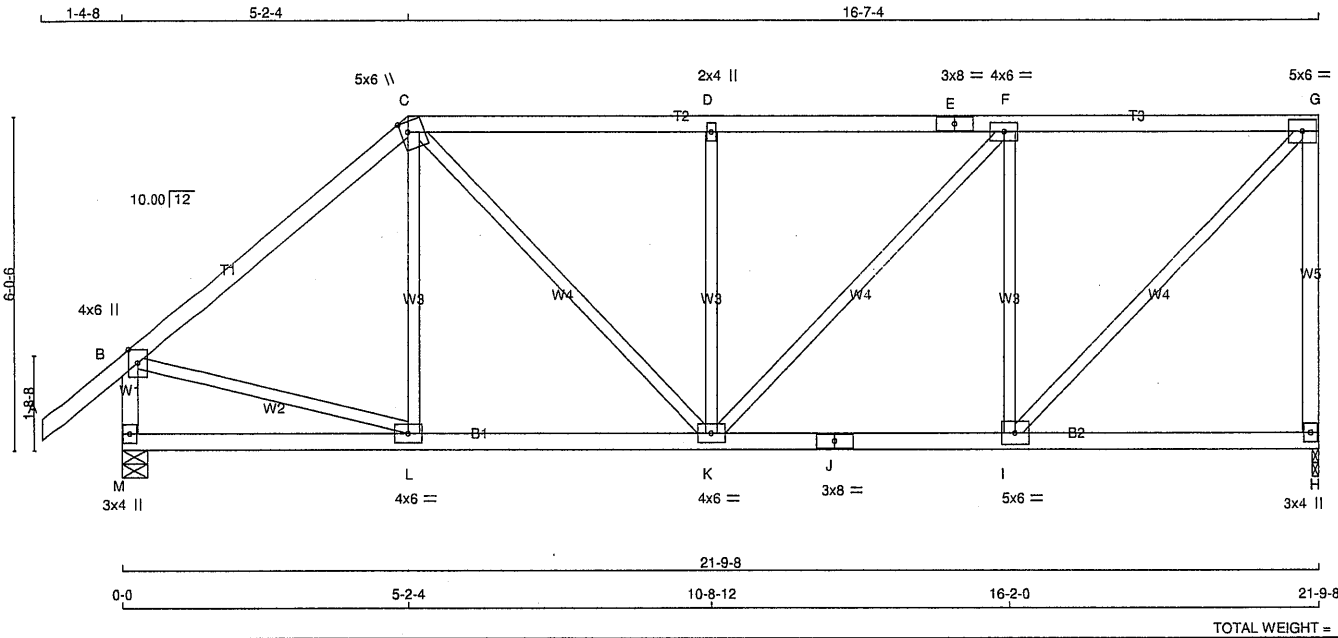

TOTAL WEIGHT = 2 X 97 = 195 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	8.0		
F	TMWW-t	MT20	4.0	6.0		
G	TMVW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	3.0	8.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	1126	0	1126	0	0	1-8	1-8
M	1251	0	1251	0	0	5-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	799	507 / 0	0 / 0	0 / 0	0 / 0	292 / 0	0 / 0
M	885	576 / 0	0 / 0	0 / 0	0 / 0	310 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.47 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 40	-84.9	-84.9	0.13 (1)	10.00	L- C	-91 / 51	0.05 (1)
B- C	-1032 / 0	-84.9	-84.9	0.45 (1)	5.57	B- L	0 / 817	0.18 (1)
C- D	-1116 / 0	-84.9	-84.9	0.38 (1)	5.51	I- G	0 / 1247	0.28 (1)
D- E	-1117 / 0	-84.9	-84.9	0.39 (1)	5.47	C- K	0 / 470	0.11 (1)
E- F	-1117 / 0	-84.9	-84.9	0.39 (1)	5.47	I- F	-774 / 0	0.44 (1)
F- G	-873 / 0	-84.9	-84.9	0.38 (1)	6.02	K- D	-504 / 0	0.29 (1)
H- G	-1084 / 0	0.0	0.0	0.76 (1)	7.58	K- F	0 / 354	0.08 (1)
M- B	-1212 / 0	0.0	0.0	0.13 (1)	7.27			
M- L	0 / 0	-18.5	-18.5	0.13 (4)	10.00			
L- K	0 / 789	-18.5	-18.5	0.20 (4)	10.00			
K- J	0 / 873	-18.5	-18.5	0.22 (4)	10.00			
J- I	0 / 873	-18.5	-18.5	0.22 (4)	10.00			
I- H	0 / 0	-18.5	-18.5	0.14 (4)	10.00			

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.76/1.00 (G-H:1) , BC=0.22/1.00 (I-K:4) , WB=0.44/1.00 (F-I:1) , SSI=0.22/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (B) (INPUT = 0.90)
JSI METAL= 0.52 (B) (INPUT = 0.95)

Structural component only
DWG# T-2406356

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425055	T43S	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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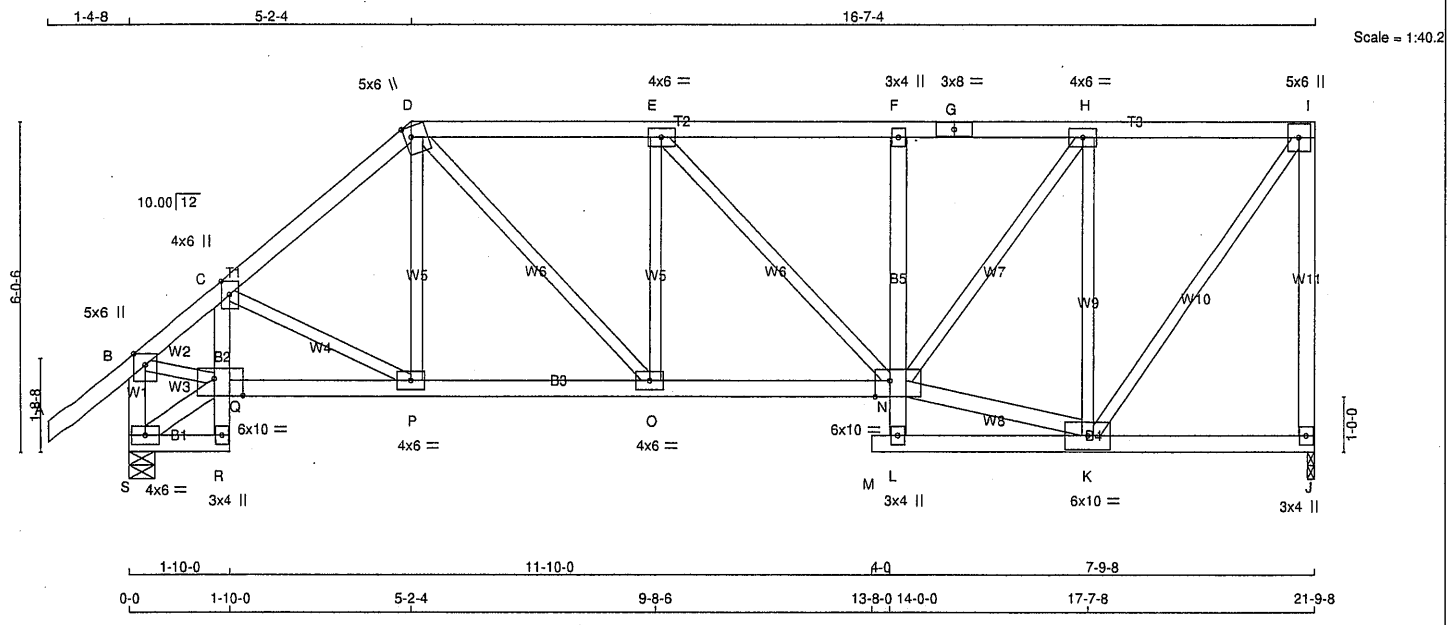


TOTAL WEIGHT = 2 X 112 = 224 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
S - R	2x4	DRY	No.2	SPF
R - C	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
L - F	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

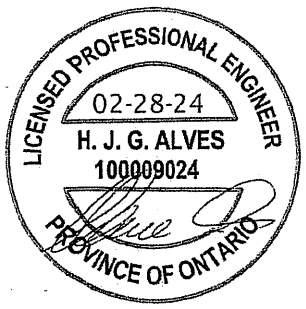
PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMVW+p	MT20	4.0	6.0	Edge	2.00
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
G	TS-t	MT20	3.0	8.0		
H	TMWW-t	MT20	4.0	6.0		
I	TMVW+p	MT20	5.0	6.0		
J	BMV1+p	MT20	3.0	4.0		
K	BMWWW-t	MT20	6.0	10.0		
L	BMV+p	MT20	3.0	4.0		
N	BVMWWW-I	MT20	6.0	10.0	Edge	3.25
O	BMWW-t	MT20	4.0	6.0		
P	BMWW-t	MT20	4.0	6.0		
Q	BVMWW-I	MT20	6.0	10.0	3.75	6.50
R	BMV+p	MT20	3.0	4.0		
S	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1)



Structural component only
DWG# T-2406357

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	1128	0	1128	0	0	1-8	1-8
S	1258	0	1258	0	0	5-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	801	505 / 0	0 / 0	0 / 0	0 / 0	296 / 0	0 / 0
S	891	578 / 0	0 / 0	0 / 0	0 / 0	313 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, S

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.29 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	MAX TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		(PLF)						
A- B	0 / 40	-84.9	-84.9	0.13 (1)	10.00	C- P	-109 / 0	0.03 (1)
B- C	-1330 / 0	-84.9	-84.9	0.14 (1)	5.46	P- D	0 / 101	0.03 (4)
C- D	-1235 / 0	-84.9	-84.9	0.16 (1)	5.60	D- O	0 / 550	0.12 (1)
D- E	-1319 / 0	-84.9	-84.9	0.28 (1)	5.29	O- E	-309 / 0	0.12 (1)
E- F	-1199 / 0	-84.9	-84.9	0.28 (1)	5.49	E- N	-175 / 0	0.13 (1)
F- G	-1193 / 0	-84.9	-84.9	0.20 (1)	5.62	K- H	-1051 / 0	0.60 (1)
G- H	-1193 / 0	-84.9	-84.9	0.20 (1)	5.62	K- I	0 / 1169	0.26 (1)
H- I	-680 / 0	-84.9	-84.9	0.18 (1)	6.25	S- Q	-38 / 0	0.00 (1)
J- I	-1097 / 0	0.0	0.0	0.76 (1)	7.55	B- Q	0 / 1027	0.23 (1)
S- B	-1222 / 0	0.0	0.0	0.13 (1)	7.25	N- K	0 / 687	0.11 (1)
						N- H	0 / 847	0.19 (1)
S- R	0 / 31	-18.5	-18.5	0.02 (4)	10.00			
R- Q	0 / 17	0.0	0.0	0.05 (1)	10.00			
Q- C	-115 / 0	0.0	0.0	0.05 (1)	7.81			
Q- P	0 / 1034	-18.5	-18.5	0.20 (1)	10.00			
P- O	0 / 939	-18.5	-18.5	0.19 (1)	10.00			
O- N	0 / 1319	-18.5	-18.5	0.26 (1)	10.00			
L- N	0 / 36	0.0	0.0	0.05 (1)	10.00			
N- F	-301 / 0	0.0	0.0	0.06 (1)	7.81			
M- L	0 / 0	-18.5	-18.5	0.00 (4)	10.00			
L- K	0 / 31	-18.5	-18.5	0.08 (4)	10.00			
K- J	0 / 0	-18.5	-18.5	0.08 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.76/1.00 (I-J:1) , BC=0.26/1.00 (N-O:1) , WB=0.60/1.00 (H-K:1) , SSI=0.18/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.71 (H) (INPUT = 0.90)
JSI METAL= 0.48 (B) (INPUT = 0.95)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425055	T44	4	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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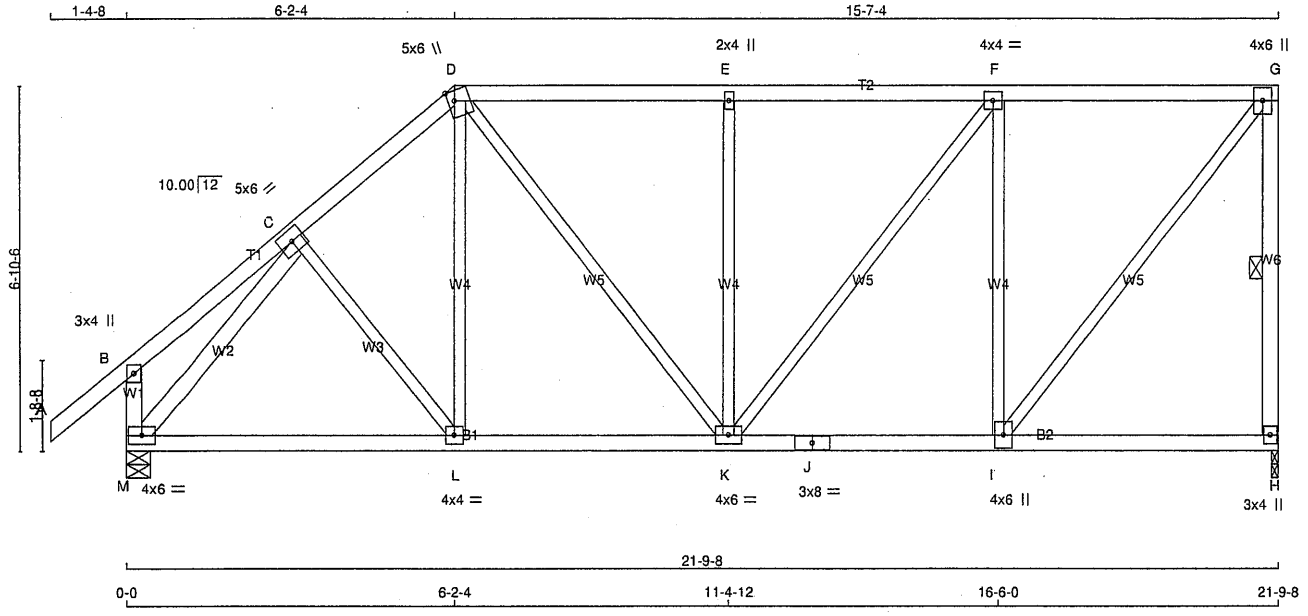

TOTAL WEIGHT = 4 X 107 = 427 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
M - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW+p	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWW+t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	8.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMVW1-t	MT20	4.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	1126	0	1126	0	0	IN-SX	IN-SX
M	1251	0	1251	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	799	507 / 0	0 / 0	0 / 0	0 / 0	292 / 0	0 / 0
M	885	576 / 0	0 / 0	0 / 0	0 / 0	310 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.86 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-84.9	-84.9	0.13 (1)	10.00	C- L	0 / 41	0.01 (4)
B- C	0 / 19	-84.9	-84.9	0.12 (1)	10.00	L- D	0 / 115	0.04 (4)
C- D	-1026 / 0	-84.9	-84.9	0.14 (1)	6.03	M- C	-1240 / 0	0.35 (1)
D- E	-967 / 0	-84.9	-84.9	0.32 (1)	5.89	I- G	0 / 1162	0.26 (1)
E- F	-967 / 0	-84.9	-84.9	0.34 (1)	5.86	D- K	0 / 312	0.07 (1)
F- G	-729 / 0	-84.9	-84.9	0.33 (1)	6.25	I- F	-793 / 0	0.63 (1)
H- G	-1087 / 0	0.0	0.0	0.23 (1)	6.06	K- E	-473 / 0	0.38 (1)
M- B	-225 / 0	0.0	0.0	0.02 (1)	7.81	K- F	0 / 389	0.09 (1)
M- L	0 / 763	-18.5	-18.5	0.23 (4)	10.00			
L- K	0 / 773	-18.5	-18.5	0.23 (4)	10.00			
K- J	0 / 729	-18.5	-18.5	0.19 (4)	10.00			
J- I	0 / 729	-18.5	-18.5	0.19 (4)	10.00			
I- H	0 / 0	-18.5	-18.5	0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.34/1.00 (E-F:1) , BC=0.23/1.00 (K-L:4) , WB=0.63/1.00 (F-I:1) , SSI=0.21/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.68 (G) (INPUT = 0.90)
JSI METAL= 0.33 (I) (INPUT = 0.95)

Structural component only
DWG# T-2406358

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425915	T44S	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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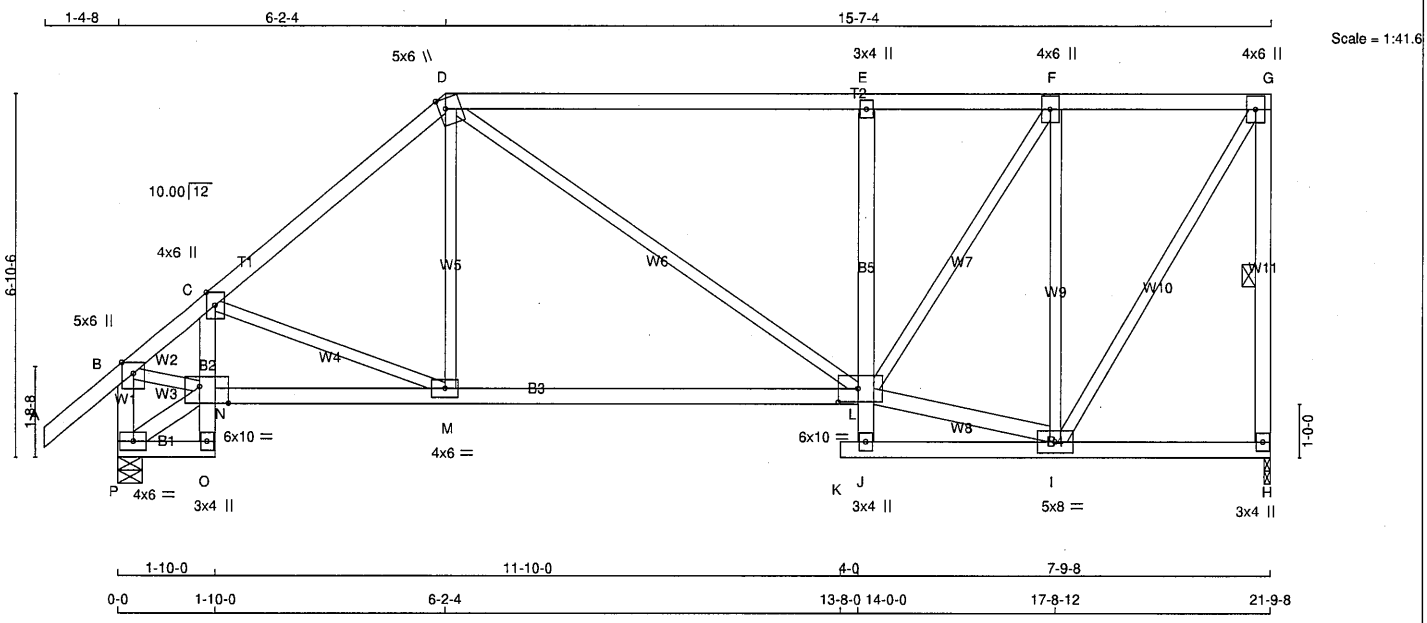

TOTAL WEIGHT = 2 X 112 = 224 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
P - O	2x4	DRY	No.2	SPF
O - C	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
J - E	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMVW+p	MT20	4.0	6.0	Edge	2.00
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMV+p	MT20	3.0	4.0		
F	TMWW+t	MT20	4.0	6.0		
G	TMVW+p	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWWW-t	MT20	5.0	8.0		
J	BMV+p	MT20	3.0	4.0		
L	BVMWWW-I	MT20	6.0	10.0	3.00	4.50
M	BMWW-t	MT20	4.0	6.0		
N	BVMWW-I	MT20	6.0	10.0	3.75	6.50
O	BMV+p	MT20	3.0	4.0		
P	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2406483

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1128	0	1128	0	0	1-8	1-8
P	1258	0	1258	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	801	505 / 0	0 / 0	0 / 0	0 / 0	296 / 0	0 / 0
P	891	578 / 0	0 / 0	0 / 0	0 / 0	313 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, P

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO					FR-TO		
A- B	0 / 40	-84.9 -84.9	0.13 (1)	10.00	C- M	-180 / 0	0.07 (1)
B- C	-1357 / 0	-84.9 -84.9	0.20 (1)	5.33	M- D	0 / 193	0.06 (4)
C- D	-1203 / 0	-84.9 -84.9	0.25 (1)	5.55	D- L	0 / 221	0.05 (1)
D- E	-1092 / 0	-84.9 -84.9	0.80 (1)	4.75	I- F	-999 / 0	0.80 (1)
E- F	-1088 / 0	-84.9 -84.9	0.69 (1)	4.76	I- G	0 / 1094	0.25 (1)
F- G	-569 / 0	-84.9 -84.9	0.22 (1)	6.25	P- N	-38 / 0	0.00 (1)
H- G	-1098 / 0	0.0 0.0	0.24 (1)	6.04	B- N	0 / 1068	0.24 (1)
P- B	-1222 / 0	0.0 0.0	0.13 (1)	7.25	L- I	0 / 572	0.09 (1)
					L- F	0 / 942	0.21 (1)
P- O	0 / 32	-18.5 -18.5	0.02 (4)	10.00			
O- N	0 / 17	0.0 0.0	0.05 (1)	10.00			
N- C	-133 / 0	0.0 0.0	0.05 (1)	7.81			
N- M	0 / 1075	-18.5 -18.5	0.34 (4)	10.00			
M- L	0 / 911	-18.5 -18.5	0.34 (4)	10.00			
J- L	0 / 38	0.0 0.0	0.04 (1)	10.00			
L- E	-674 / 0	0.0 0.0	0.29 (1)	7.81			
K- J	0 / 0	-18.5 -18.5	0.00 (4)	10.00			
J- I	0 / 27	-18.5 -18.5	0.08 (4)	10.00			
I- H	0 / 0	-18.5 -18.5	0.08 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.80/1.00 (D-E:1) , BC=0.34/1.00 (M-N:4) , WB=0.80/1.00 (F-I:1) , SSI=0.31/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.71 (B) (INPUT = 0.90)
JSI METAL= 0.49 (B) (INPUT = 0.95)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425055	T45	4	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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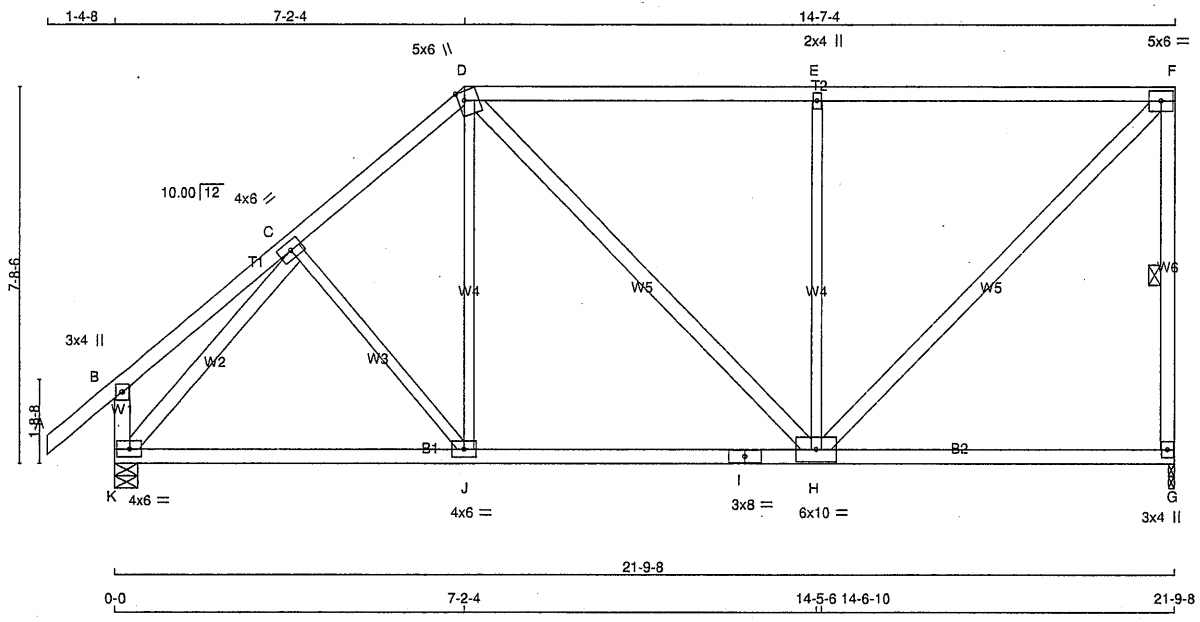

TOTAL WEIGHT = 4 X 109 = 435 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
K - C	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	5.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	6.0	10.0		
I	BS-t	MT20	3.0	8.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMVW1-t	MT20	4.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1126	0	1126	0	0	1-8	1-8
K	1251	0	1251	0	0	5-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	799	507 / 0	0 / 0	0 / 0	0 / 0	292 / 0	0 / 0
K	885	576 / 0	0 / 0	0 / 0	0 / 0	310 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.93 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-84.9	-84.9	0.13 (1)	10.00	C- J	-59 / 21	0.03 (1)
B- C	0 / 22	-84.9	-84.9	0.17 (1)	10.00	J- D	0 / 191	0.05 (4)
C- D	-995 / 0	-84.9	-84.9	0.19 (1)	6.02	K- C	-1236 / 0	0.46 (1)
D- E	-828 / 0	-84.9	-84.9	0.84 (1)	4.93	H- F	0 / 1176	0.19 (1)
E- F	-828 / 0	-84.9	-84.9	0.84 (1)	4.93	D- H	0 / 114	0.02 (1)
G- F	-1071 / 0	0.0	0.0	0.28 (1)	6.09	H- E	-770 / 0	0.84 (1)
K- B	-240 / 0	0.0	0.0	0.03 (1)	7.81			
K- J	0 / 784	-18.5	-18.5	0.29 (4)	10.00			
J- I	0 / 748	-18.5	-18.5	0.30 (4)	10.00			
I- H	0 / 748	-18.5	-18.5	0.30 (4)	10.00			
H- G	0 / 0	-18.5	-18.5	0.23 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.84/1.00 (E-F:1) , BC=0.30/1.00 (H-J:4) , WB=0.84/1.00 (E-H:1) , SSI=0.30/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (C) (INPUT = 0.90)
JSI METAL= 0.28 (C) (INPUT = 0.95)

Structural component only
DWG# T-2406359

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425915	T45S	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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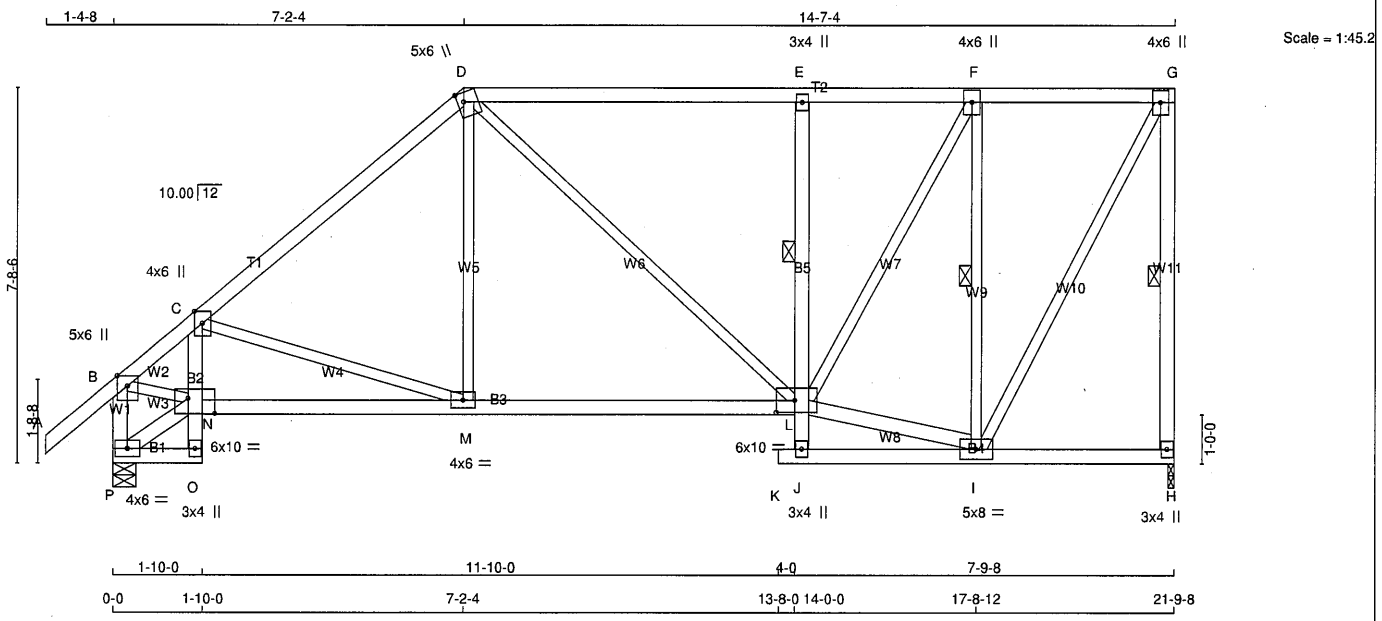

Scale = 1:45.2

TOTAL WEIGHT = 2 X 117 = 234 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
P - O	2x4	DRY	No.2	SPF
O - C	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
J - E	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMVW+p	MT20	4.0	6.0	Edge	2.00
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMV+p	MT20	3.0	4.0		
F	TMWW+t	MT20	4.0	6.0		
G	TMVW+p	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWWW-t	MT20	5.0	8.0		
J	BMV+p	MT20	3.0	4.0		
L	BVMWWW-I	MT20	6.0	10.0	3.00	4.50
M	BMWW-t	MT20	4.0	6.0		
N	BVMWW-I	MT20	6.0	10.0	3.75	6.50
O	BMV+p	MT20	3.0	4.0		
P	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2406484

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	1128	0	1128	0	0	1-8	1-8
P	1258	0	1258	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	801	505 / 0	0 / 0	0 / 0	0 / 0	296 / 0	0 / 0
P	891	578 / 0	0 / 0	0 / 0	0 / 0	313 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, P

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.10 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-H, E-L, F-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-84.9	-84.9	0.13 (1)		10.00
B- C	-1392 / 0	-84.9	-84.9	0.32 (1)		5.10
C- D	-1141 / 0	-84.9	-84.9	0.37 (1)		5.51
D- E	-931 / 0	-84.9	-84.9	0.56 (1)		5.57
E- F	-927 / 0	-84.9	-84.9	0.51 (1)		5.57
F- G	-509 / 0	-84.9	-84.9	0.20 (1)		6.25
H- G	-1098 / 0	0.0	0.0	0.29 (1)		6.04
P- B	-1221 / 0	0.0	0.0	0.13 (1)		7.25
P- O	0 / 33	-18.5	-18.5	0.02 (4)		10.00
O- N	0 / 17	0.0	0.0	0.05 (1)		10.00
N- C	-129 / 0	0.0	0.0	0.05 (1)		7.81
N- M	0 / 1128	-18.5	-18.5	0.31 (4)		10.00
M- L	0 / 859	-18.5	-18.5	0.29 (4)		10.00
J- L	0 / 38	0.0	0.0	0.04 (1)		10.00
L- E	-580 / 0	0.0	0.0	0.05 (1)		6.25
K- J	0 / 0	-18.5	-18.5	0.00 (4)		10.00
J- I	0 / 23	-18.5	-18.5	0.08 (4)		10.00
I- H	0 / 0	-18.5	-18.5	0.08 (4)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- M	-287 / 0	0.17 (1)
M- D	0 / 211	0.06 (4)
D- L	0 / 98	0.02 (1)
I- F	-990 / 0	0.37 (1)
I- G	0 / 1070	0.24 (1)
P- N	-39 / 0	0.00 (1)
B- N	0 / 1121	0.25 (1)
L- I	0 / 512	0.08 (1)
L- F	0 / 841	0.19 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.56/1.00 (D-E:1) , BC=0.31/1.00 (M-N:4) , WB=0.37/1.00 (F-I:1) , SSI=0.27/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (B) (INPUT = 0.90)
JSI METAL= 0.50 (B) (INPUT = 0.95)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425055	T46	4	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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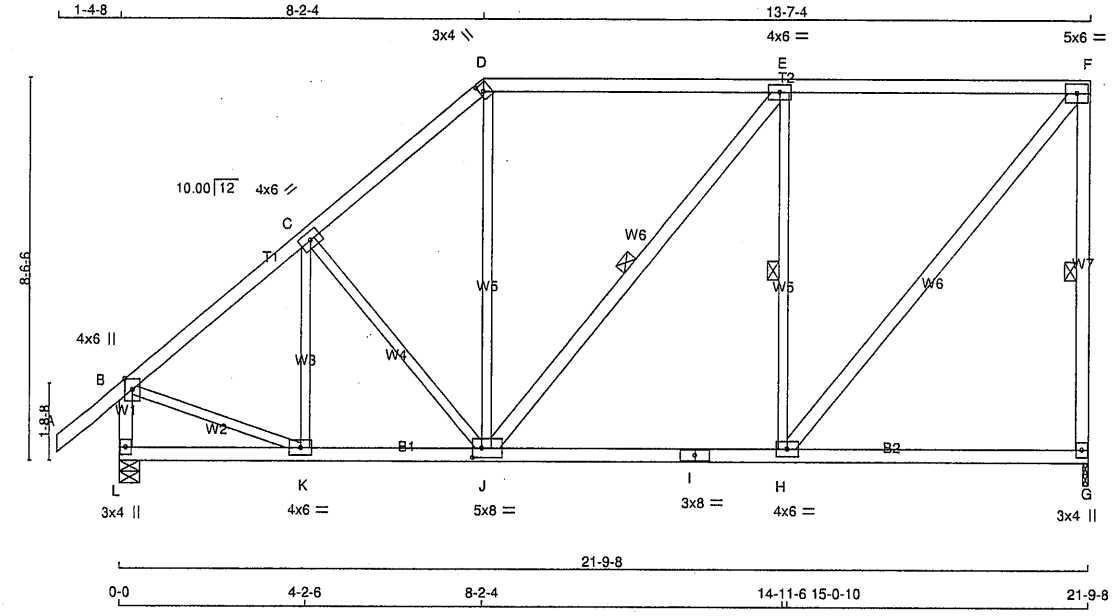

TOTAL WEIGHT = 4 X 114 = 454 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMWW-t	MT20	4.0	6.0		
D	TTW+h	MT20	3.0	4.0	2.00	1.00
E	TMWW-t	MT20	4.0	6.0		
F	TMVW-t	MT20	5.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	8.0		
J	BMWWW-t	MT20	5.0	8.0	2.50	2.50
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1126	0	1126	0	0	1-8	1-8
L	1251	0	1251	0	0	5-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	799	507 / 0	0 / 0	0 / 0	0 / 0	292 / 0	0 / 0
L	885	576 / 0	0 / 0	0 / 0	0 / 0	310 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.68 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, E-J, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)		FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40		-84.9	-84.9	0.13 (1)	10.00	K- C	-193 / 0	0.08 (1)
B- C	-1045 / 0		-84.9	-84.9	0.26 (1)	5.82	C- J	-185 / 0	0.13 (1)
C- D	-945 / 0		-84.9	-84.9	0.25 (1)	6.05	J- D	0 / 245	0.06 (1)
D- E	-706 / 0		-84.9	-84.9	0.71 (1)	5.69	B- K	0 / 865	0.19 (1)
E- F	-711 / 0		-84.9	-84.9	0.71 (1)	5.68	H- F	0 / 1113	0.18 (1)
G- F	-1076 / 0		0.0	0.0	0.36 (1)	6.09	J- E	-8 / 6	0.00 (1)
L- B	-1217 / 0		0.0	0.0	0.13 (1)	7.26	H- E	-711 / 0	0.33 (1)
L- K	0 / 0		-18.5	-18.5	0.07 (4)	10.00			
K- J	0 / 822		-18.5	-18.5	0.19 (1)	10.00			
J- I	0 / 711		-18.5	-18.5	0.28 (4)	10.00			
I- H	0 / 711		-18.5	-18.5	0.28 (4)	10.00			
H- G	0 / 0		-18.5	-18.5	0.22 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.71/1.00 (E-F:1) , BC=0.28/1.00 (H-J:4) , WB=0.33/1.00 (E-H:1) , SSI=0.28/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (B) (INPUT = 0.90)
JSI METAL= 0.52 (B) (INPUT = 0.95)

Structural component only
DWG# T-2406360

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425055	T47	6	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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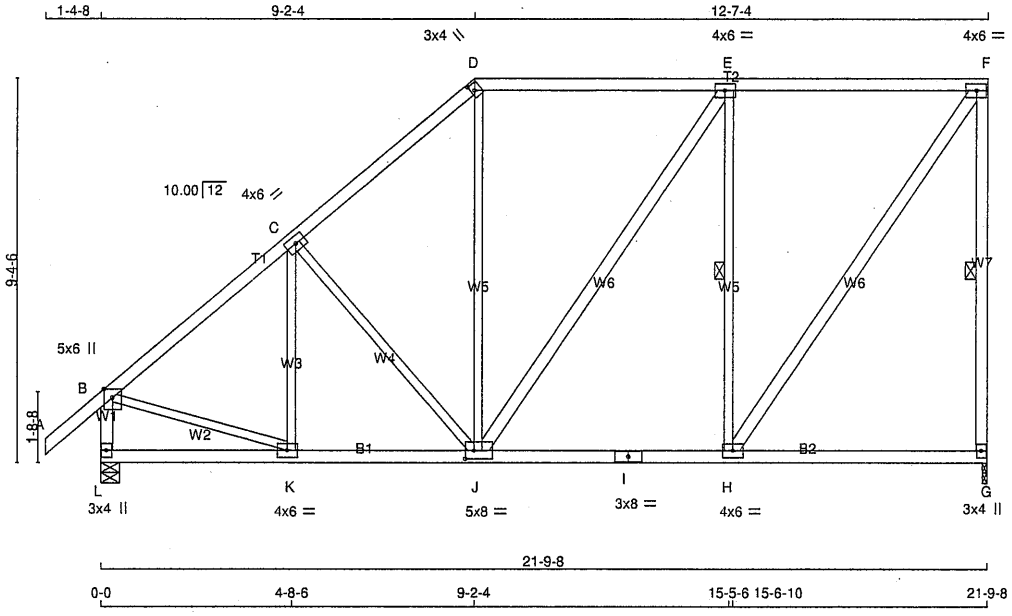

TOTAL WEIGHT = 6 X 118 = 710 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	6.0		
D	TTW+h	MT20	3.0	4.0	2.00	1.00
E	TMWW-t	MT20	4.0	6.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	8.0		
J	BMWWW-t	MT20	5.0	8.0	2.50	2.50
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2406361

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1126	0	1126	0	0	1-8	1-8
L	1251	0	1251	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	799	507 / 0	0 / 0	0 / 0	0 / 0	292 / 0	0 / 0
L	885	576 / 0	0 / 0	0 / 0	0 / 0	310 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.70 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-84.9	-84.9	0.13 (1)	10.00	K- C	-151 / 17	0.07 (1)
B- C	-1053 / 0	-84.9	-84.9	0.34 (1)	5.70	C- J	-258 / 0	0.24 (1)
C- D	-895 / 0	-84.9	-84.9	0.32 (1)	6.06	J- D	0 / 211	0.05 (1)
D- E	-665 / 0	-84.9	-84.9	0.60 (1)	6.13	B- K	0 / 866	0.19 (1)
E- F	-612 / 0	-84.9	-84.9	0.60 (1)	6.25	H- F	0 / 1068	0.17 (1)
G- F	-1080 / 0	0.0	0.0	0.45 (1)	6.08	J- E	0 / 94	0.02 (1)
L- B	-1214 / 0	0.0	0.0	0.13 (1)	7.26	H- E	-741 / 0	0.43 (1)
L- K	0 / 0	-18.5	-18.5	0.09 (4)	10.00			
K- J	0 / 832	-18.5	-18.5	0.18 (1)	10.00			
J- I	0 / 612	-18.5	-18.5	0.24 (4)	10.00			
I- H	0 / 612	-18.5	-18.5	0.24 (4)	10.00			
H- G	0 / 0	-18.5	-18.5	0.19 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.60/1.00 (D-E:1) , BC=0.24/1.00 (H-J:4) , WB=0.43/1.00 (E-H:1) , SSI=0.26/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (D) (INPUT = 0.90)
JSI METAL= 0.42 (B) (INPUT = 0.95)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425055	T48A	5	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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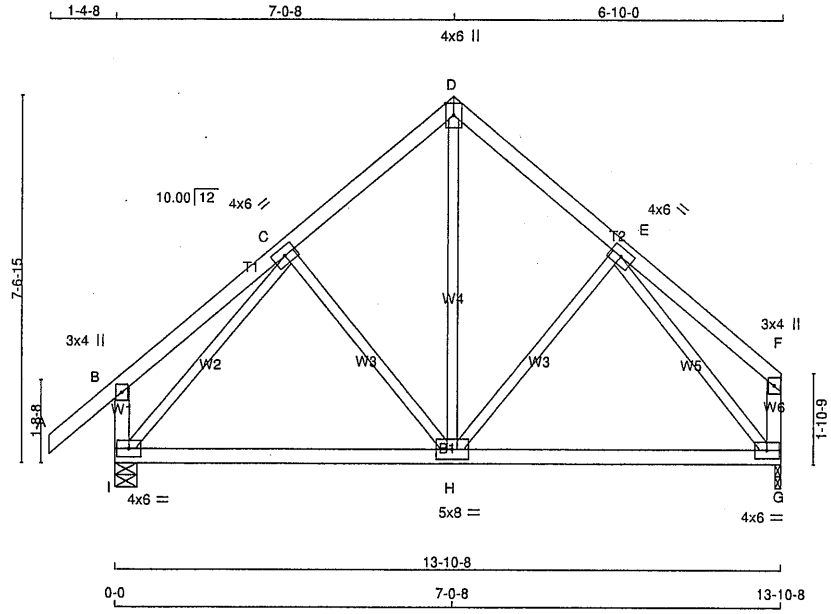

TOTAL WEIGHT = 5 X 65 = 323 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	6.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
I	842	0	842	0	0	5-8	1-8
G	717	0	717	0	0	1-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	595	391 / 0	0 / 0	0 / 0	0 / 0	204 / 0	0 / 0
G	509	323 / 0	0 / 0	0 / 0	0 / 0	186 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO						FR-TO			
A- B	0 / 40	-84.9	-84.9	0.13 (1)	10.00	C- H	-146 / 0	0.08 (1)	
B- C	0 / 22	-84.9	-84.9	0.17 (1)	10.00	H- D	0 / 361	0.08 (1)	
C- D	-488 / 0	-84.9	-84.9	0.13 (1)	6.25	H- E	-117 / 0	0.06 (1)	
D- E	-486 / 0	-84.9	-84.9	0.12 (1)	6.25	I- C	-714 / 0	0.36 (1)	
E- F	0 / 22	-84.9	-84.9	0.16 (1)	10.00	E- G	-711 / 0	0.34 (1)	
I- B	-238 / 0	0.0	0.0	0.03 (1)	7.81				
G- F	-105 / 0	0.0	0.0	0.01 (1)	7.81				
I- H	0 / 451	-18.5	-18.5	0.29 (4)	10.00				
H- G	0 / 433	-18.5	-18.5	0.29 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.46")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.46")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.17/1.00 (B-C:1) , BC=0.29/1.00 (H-I:4) , WB=0.36/1.00 (C-I:1) , SSI=0.11/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.48 (E) (INPUT = 0.90)
JSI METAL= 0.16 (C) (INPUT = 0.95)

Structural component only
DWG# T-2406362

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425055	T48G	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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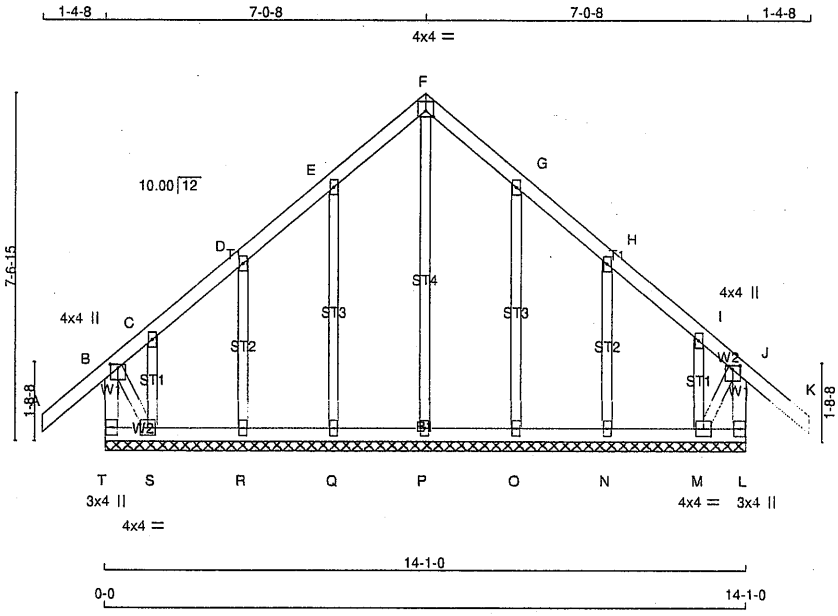

TOTAL WEIGHT = 70 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
T - B	2x4	DRY	No.2	SPF
A - F	2x4	DRY	No.2	SPF
F - K	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, E, G, H, I						
C	TMW+w	MT20	2.0	4.0		
F	TTW-p	MT20	4.0	4.0	1.50	2.00
J	TMVW+p	MT20	4.0	4.0	1.00	2.00
L	BMV1+p	MT20	3.0	4.0		
M	BMWW1-t	MT20	4.0	4.0		
N, O, P, Q, R						
N	BMW1+w	MT20	2.0	4.0		
S	BMWW1-t	MT20	4.0	4.0		
T	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
T- B	-276 / 0	0.0	0.0	0.03 (1)		7.81
A- B	0 / 40	-84.9	-84.9	0.13 (1)		10.00
B- C	-70 / 0	-84.9	-84.9	0.12 (1)		6.25
C- D	-8 / 0	-84.9	-84.9	0.04 (1)		10.00
D- E	-7 / 0	-84.9	-84.9	0.05 (1)		10.00
E- F	-16 / 0	-84.9	-84.9	0.05 (1)		6.25
F- G	-16 / 0	-84.9	-84.9	0.05 (1)		6.25
G- H	-7 / 0	-84.9	-84.9	0.05 (1)		10.00
H- I	-8 / 0	-84.9	-84.9	0.04 (1)		10.00
I- J	-70 / 0	-84.9	-84.9	0.12 (1)		6.25
J- K	0 / 40	-84.9	-84.9	0.13 (1)		10.00
L- J	-276 / 0	0.0	0.0	0.03 (1)		7.81

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- F	-137 / 0	0.14 (1)
Q- E	-191 / 0	0.10 (1)
R- D	-173 / 0	0.05 (1)
S- C	-29 / 0	0.00 (1)
O- G	-191 / 0	0.10 (1)
N- H	-173 / 0	0.05 (1)
M- I	-29 / 0	0.00 (1)
B- S	0 / 20	0.00 (1)
M- J	0 / 20	0.00 (1)

T- S	0 / 0	-18.5	-18.5	0.01 (4)	10.00
S- R	0 / 11	-18.5	-18.5	0.02 (4)	10.00
R- Q	0 / 7	-18.5	-18.5	0.02 (4)	10.00
Q- P	0 / 4	-18.5	-18.5	0.01 (4)	10.00
P- O	0 / 4	-18.5	-18.5	0.01 (4)	10.00
O- N	0 / 7	-18.5	-18.5	0.02 (4)	10.00
N- M	0 / 11	-18.5	-18.5	0.02 (4)	10.00
M- L	0 / 0	-18.5	-18.5	0.01 (4)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13/1.00 (A-B:1) , BC=0.02/1.00 (R-S:4) , WB=0.14/1.00 (F-P:1) , SSI=0.08/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (F) (INPUT = 0.90)
JSI METAL= 0.10 (G) (INPUT = 0.95)

Structural component only
DWG# T-2406363

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425055	T49	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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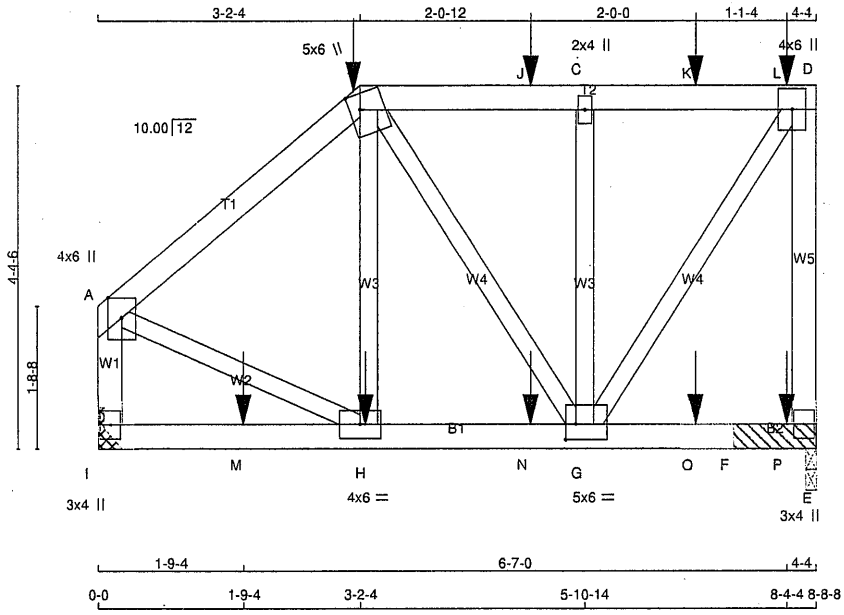

Scale = 1:26.5

TOTAL WEIGHT = 44 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
I - A	2x4	DRY	No.2	SPF
I - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B	TTWW+m	MT20	5.0	6.0	2.25	1.50
C	TMW+w	MT20	2.0	4.0		
D	TMVW+p	MT20	4.0	6.0		
E	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	6.0	2.25	1.50
H	BMWW-t	MT20	4.0	6.0		
I	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2406364

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	1516	0	1516	0	0	1-8	1-8 & BLOCK
I	1254	0	1254	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	1073	699 / 0	0 / 0	0 / 0	0 / 0	374 / 0	0 / 0
I	886	584 / 0	0 / 0	0 / 0	0 / 0	302 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

2x4 DRY SPF No.2 BEARING BLOCK 12" LONG AT JT. E ATTACHED TO FRONT SIDE WITH 2 ROWS OF (0.122"X3") SPIRAL NAILS SPACED 2" C.C. 12 NAILS TOTAL.

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.92 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-1037 / 0	-84.9	-84.9	0.18 (1)	5.92	H- B	0 / 218	0.05 (4)
B- J	-687 / 0	-84.9	-84.9	0.14 (1)	6.25	A- H	0 / 861	0.21 (1)
J- C	-687 / 0	-84.9	-84.9	0.14 (1)	6.25	G- D	0 / 1212	0.30 (1)
C- K	-687 / 0	-84.9	-84.9	0.14 (1)	6.25	B- G	-202 / 0	0.08 (1)
K- L	-687 / 0	-84.9	-84.9	0.14 (1)	6.25	G- C	-362 / 0	0.11 (1)
L- D	-687 / 0	-84.9	-84.9	0.14 (1)	6.25			
E- D	-1173 / 0	0.0	0.0	0.36 (1)	7.33			
I- A	-1134 / 0	0.0	0.0	0.13 (1)	7.43			
I- M	0 / 0	-18.5	-18.5	0.28 (1)	10.00			
M- H	0 / 0	-18.5	-18.5	0.28 (1)	10.00			
H- N	0 / 800	-18.5	-18.5	0.33 (1)	10.00			
N- G	0 / 800	-18.5	-18.5	0.33 (1)	10.00			
G- O	0 / 0	-18.5	-18.5	0.28 (1)	10.00			
O- F	0 / 0	-18.5	-18.5	0.28 (1)	10.00			
F- P	0 / 0	-18.5	-18.5	0.28 (1)	10.00			
P- E	0 / 0	-18.5	-18.5	0.28 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
B	3-2-4	-47	-47	---	FRONT	VERT	DEAD	---	C1
B	3-2-4	-38	-38	---	BACK	VERT	TOTAL	---	C1
B	3-2-4	-180	-180	---	FRONT	VERT	SNOW	---	C1
H	3-3-0	-187	-187	---	BACK	VERT	TOTAL	---	C1
J	5-3-0	-32	-32	---	BACK	VERT	TOTAL	---	C1
K	7-3-0	-32	-32	---	BACK	VERT	TOTAL	---	C1
L	8-4-4	-51	-51	---	BACK	VERT	TOTAL	---	C1
M	1-9-4	-187	-187	---	BACK	VERT	TOTAL	---	C1
N	5-3-0	-187	-187	---	BACK	VERT	TOTAL	---	C1
O	7-3-0	-187	-187	---	BACK	VERT	TOTAL	---	C1
P	8-4-4	-192	-192	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP	CH.	LL =	23.3	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL	LOAD	=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.29")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.29")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.36/1.00 (D-E:1) , BC=0.33/1.00 (G-H:1) , WB=0.30/1.00 (D-G:1) , SSI=0.29/1.00 (E-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (G) (INPUT = 0.90)
JSI METAL= 0.47 (A) (INPUT = 0.95)

CONTINUED ON PAGE 2

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

Structural component only
DWG# T-2406364

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425055	T50	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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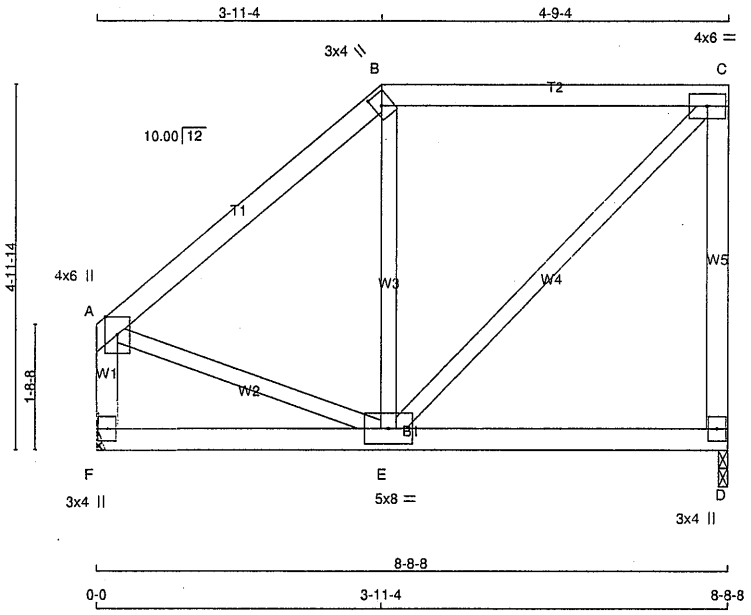

Scale = 1:30.1

TOTAL WEIGHT = 40 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B	TTW+h	MT20	3.0	4.0	2.00	1.25
C	TMVW-t	MT20	4.0	6.0		
D	BMV1+p	MT20	3.0	4.0		
E	BMWWW-t	MT20	5.0	8.0		
F	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
D	450	0	450	0	0	1-8	1-8
F	450	0	450	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	319	203 / 0	0 / 0	0 / 0	0 / 0	117 / 0	0 / 0
F	319	203 / 0	0 / 0	0 / 0	0 / 0	117 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
A- B	-283 / 0	-84.9	-84.9	0.23 (1)	6.25	E- B	-189 / 0	0.07 (1)	
B- C	-212 / 0	-84.9	-84.9	0.33 (1)	6.25	E- C	0 / 301	0.07 (1)	
D- C	-415 / 0	0.0	0.0	0.17 (1)	7.81	A- E	0 / 229	0.05 (1)	
F- A	-422 / 0	0.0	0.0	0.05 (1)	7.81				
F- E	0 / 0	-18.5	-18.5	0.10 (4)	10.00				
E- D	0 / 0	-18.5	-18.5	0.10 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.29")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.29")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.33/1.00 (B-C:1) , BC=0.10/1.00 (D-E:4) , WB=0.07/1.00 (B-E:1) , SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.39 (B) (INPUT = 0.90)
JSI METAL= 0.16 (A) (INPUT = 0.95)

Structural component only
DWG# T-2406365

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425055	T51	4	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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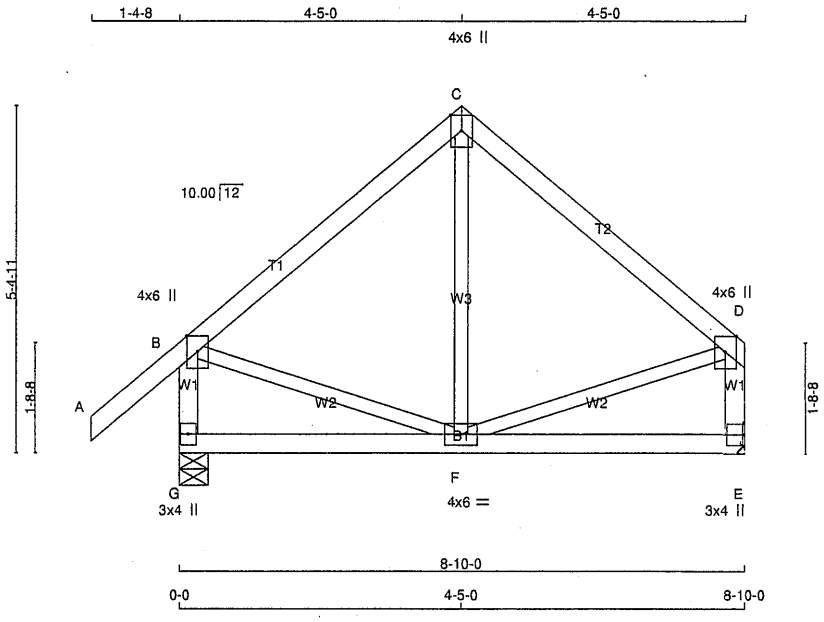

TOTAL WEIGHT = 4 X 39 = 158 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
G - B	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMVW+p	MT20	4.0	6.0	Edge	
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2406366

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	581	0	581	0	0	5-8	1-8
E	457	0	457	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	410	274 / 0	0 / 0	0 / 0	0 / 0	136 / 0	0 / 0
E	324	206 / 0	0 / 0	0 / 0	0 / 0	118 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-84.9	-84.9	0.13 (1)	10.00	F- C	-29 / 63	0.02 (4)
B- C	-270 / 0	-84.9	-84.9	0.21 (1)	6.25	B- F	0 / 217	0.05 (1)
C- D	-270 / 0	-84.9	-84.9	0.21 (1)	6.25	F- D	0 / 217	0.05 (1)
G- B	-550 / 0	0.0	0.0	0.06 (1)	7.81			
E- D	-425 / 0	0.0	0.0	0.05 (1)	7.81			
G- F	0 / 0	-18.5	-18.5	0.10 (4)	10.00			
F- E	0 / 0	-18.5	-18.5	0.10 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.29")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.29")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.21/1.00 (C-D:1) , BC=0.10/1.00 (E-F:4) , WB=0.05/1.00 (B-F:1) , SSI=0.11/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.34 (B) (INPUT = 0.90)
JSI METAL= 0.20 (D) (INPUT = 0.95)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425055	T51G	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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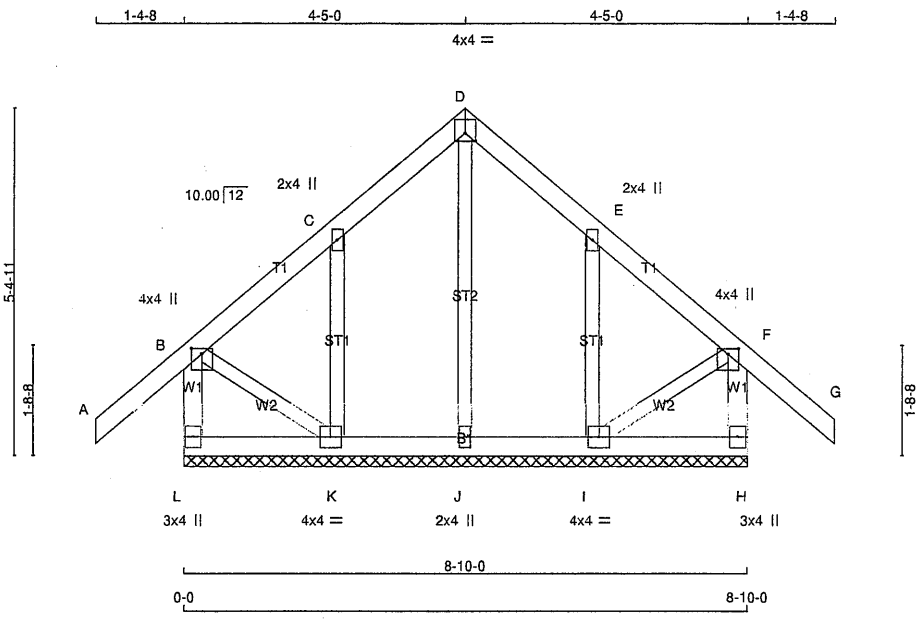

TOTAL WEIGHT = 44 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
L - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMW+w	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	4.0	1.50	2.00
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	4.0	1.00	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMWW1-t	MT20	4.0	4.0		
J	BMW1+w	MT20	2.0	4.0		
K	BMWW1-t	MT20	4.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- B	-224 / 0	0.0	0.0	0.02 (1)	7.81	J- D	-113 / 0	0.05 (1)
A- B	0 / 40	-84.9	-84.9	0.13 (1)	10.00	K- C	-228 / 0	0.05 (1)
B- C	-7 / 0	-84.9	-84.9	0.07 (1)	10.00	I- E	-228 / 0	0.05 (1)
C- D	-28 / 0	-84.9	-84.9	0.07 (1)	6.25	B- K	0 / 19	0.00 (1)
D- E	-28 / 0	-84.9	-84.9	0.07 (1)	6.25	I- F	0 / 19	0.00 (1)
E- F	-7 / 0	-84.9	-84.9	0.07 (1)	10.00			
F- G	0 / 40	-84.9	-84.9	0.13 (1)	10.00			
H- F	-224 / 0	0.0	0.0	0.02 (1)	7.81			
L- K	0 / 0	-18.5	-18.5	0.03 (4)	10.00			
K- J	0 / 10	-18.5	-18.5	0.03 (4)	10.00			
J- I	0 / 10	-18.5	-18.5	0.03 (4)	10.00			
I- H	0 / 0	-18.5	-18.5	0.03 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13/1.00 (A-B:1) , BC=0.03/1.00 (I-J:4) , WB=0.05/1.00 (E-I:1) , SSI=0.07/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (D) (INPUT = 0.90)
JSI METAL= 0.12 (C) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
02-28-24
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2406367

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425055	T55	4	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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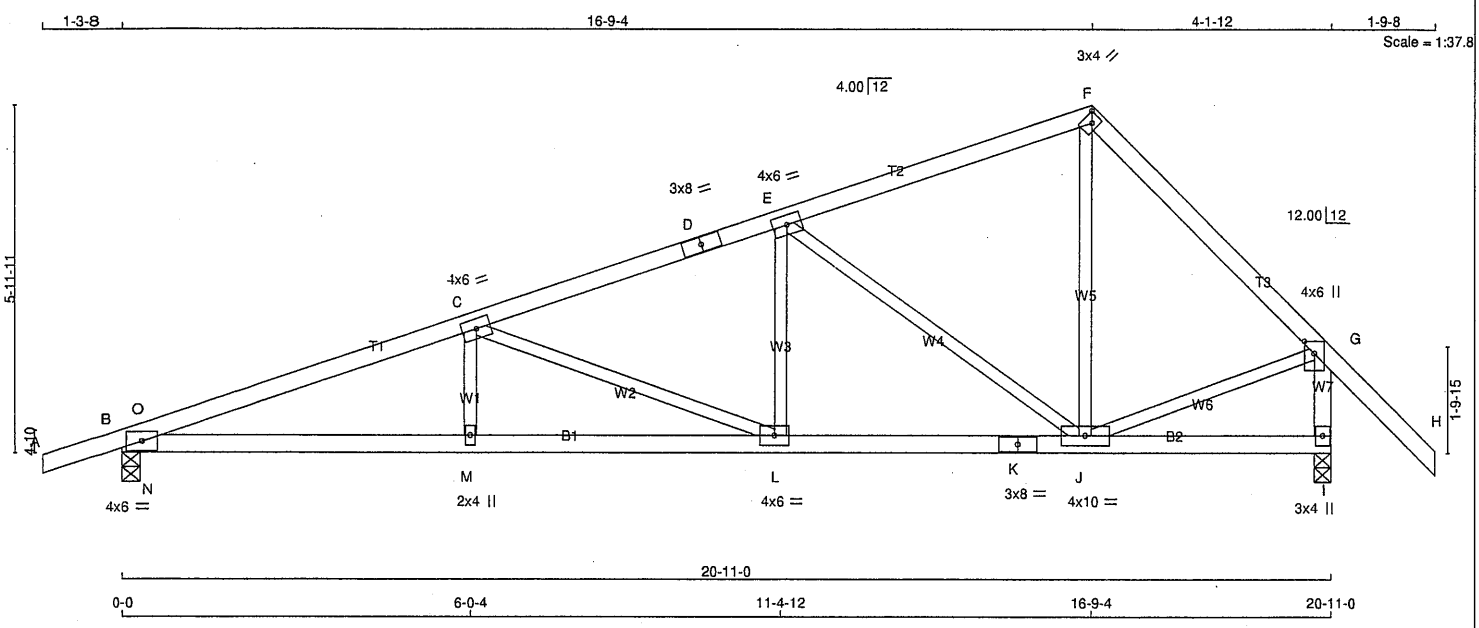

TOTAL WEIGHT = 4 X 79 = 316 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
B - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-t	MT20	4.0	6.0		
C	TMWW-t	MT20	4.0	6.0		
D	TS-t	MT20	3.0	8.0		
E	TMWW-t	MT20	4.0	6.0		
F	TTW+h	MT20	3.0	4.0	Edge	1.75
G	TMVW+p	MT20	4.0	6.0	2.50	2.00
I	BMV1+p	MT20	3.0	4.0		
J	BMWWW-t	MT20	4.0	10.0		
K	BS-t	MT20	3.0	8.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMW+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
B	1195	0	1195	0	0	3-8	1-8
I	1242	0	1242	0	0	3-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	846	549 / 0	0 / 0	0 / 0	0 / 0	296 / 0	0 / 0
I	878	575 / 0	0 / 0	0 / 0	0 / 0	303 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.13 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			FR-TO		
A- B	0 / 15	-84.9 -84.9	0.11 (1)	10.00	M- C	0 / 120	0.04 (4)
B- O	-2497 / 0	-84.9 -84.9	0.19 (1)	4.22	C- L	-864 / 0	0.50 (1)
O- C	-2444 / 0	-84.9 -84.9	0.34 (1)	4.13	L- E	0 / 411	0.09 (1)
C- D	-1595 / 0	-84.9 -84.9	0.30 (1)	4.93	E- J	-1069 / 0	0.83 (1)
D- E	-1595 / 0	-84.9 -84.9	0.30 (1)	4.93	J- F	0 / 509	0.11 (1)
E- F	-705 / 0	-84.9 -84.9	0.28 (1)	6.25	J- G	0 / 685	0.15 (1)
F- G	-917 / 0	-84.9 -84.9	0.20 (1)	6.22	N- O	-75 / 84	0.00 (1)
G- H	0 / 57	-84.9 -84.9	0.22 (1)	10.00			
I- G	-1212 / 0	0.0 0.0	0.13 (1)	7.27			
B- N	0 / 2322	-18.5 -18.5	0.45 (1)	10.00			
N- M	0 / 2322	-18.5 -18.5	0.49 (1)	10.00			
M- L	0 / 2322	-18.5 -18.5	0.42 (1)	10.00			
L- K	0 / 1517	-18.5 -18.5	0.30 (1)	10.00			
K- J	0 / 1517	-18.5 -18.5	0.30 (1)	10.00			
J- I	0 / 0	-18.5 -18.5	0.10 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.34/1.00 (C-O:1) , BC=0.49/1.00 (M-N:1) , WB=0.83/1.00 (E-J:1) , SSI=0.20/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (F) (INPUT = 0.90)
JSI METAL= 0.52 (B) (INPUT = 0.95)

Structural component only
DWG# T-2406368

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
425055	T55G	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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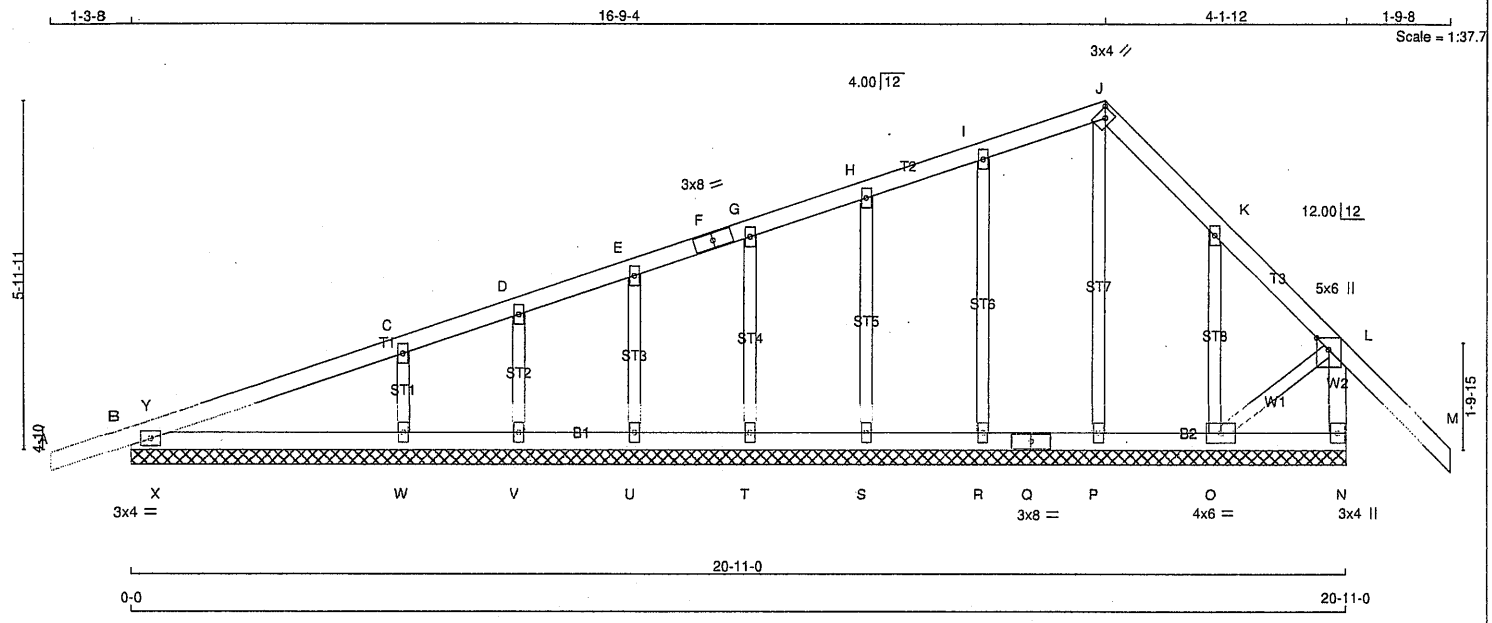

TOTAL WEIGHT = 4 X 81 = 325 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - F	2x4	DRY	No.2	SPF
F - J	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
B - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C, D, E, G, H, I, K						
C	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	8.0		
J	TTW+h	MT20	3.0	4.0	Edge	1.75
L	TMVW+p	MT20	5.0	6.0	Edge	
N	BMV1+p	MT20	3.0	4.0		
O	BMWW1-t	MT20	4.0	6.0		
P, R, S, T, U, V, W						
P	BMW1+w	MT20	2.0	4.0		
Q	BS-t	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 15	-84.9	-84.9	0.11 (1)	10.00
B- Y	-40 / 0	-84.9	-84.9	0.03 (4)	6.25
Y- C	-37 / 0	-84.9	-84.9	0.19 (1)	6.25
C- D	-58 / 0	-84.9	-84.9	0.19 (1)	6.25
D- E	-29 / 0	-84.9	-84.9	0.05 (1)	6.25
E- F	-32 / 0	-84.9	-84.9	0.05 (1)	6.25
F- G	-32 / 0	-84.9	-84.9	0.05 (1)	6.25
G- H	-29 / 0	-84.9	-84.9	0.04 (1)	6.25
H- I	-26 / 0	-84.9	-84.9	0.05 (1)	6.25
I- J	-31 / 0	-84.9	-84.9	0.05 (1)	6.25
J- K	-40 / 0	-84.9	-84.9	0.05 (1)	6.25
K- L	-89 / 0	-84.9	-84.9	0.21 (1)	6.25
L- M	0 / 57	-84.9	-84.9	0.22 (1)	10.00
N- L	-368 / 0	0.0	0.0	0.04 (1)	7.81
B- X	0 / 45	-18.5	-18.5	0.13 (1)	10.00
X- W	0 / 45	-18.5	-18.5	0.13 (1)	10.00
W- V	0 / 34	-18.5	-18.5	0.10 (1)	10.00
V- U	0 / 33	-18.5	-18.5	0.02 (1)	10.00
U- T	0 / 30	-18.5	-18.5	0.02 (4)	10.00
T- S	0 / 28	-18.5	-18.5	0.02 (4)	10.00
S- R	0 / 26	-18.5	-18.5	0.02 (4)	10.00
R- Q	0 / 24	-18.5	-18.5	0.02 (4)	10.00
Q- P	0 / 24	-18.5	-18.5	0.02 (4)	10.00
P- O	0 / 26	-18.5	-18.5	0.02 (4)	10.00
O- N	0 / 0	-18.5	-18.5	0.02 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- J	-121 / 0	0.07 (1)
R- I	-198 / 0	0.08 (1)
S- H	-164 / 0	0.05 (1)
T- G	-166 / 0	0.04 (1)
U- E	-188 / 0	0.04 (1)
V- D	-85 / 0	0.01 (1)
W- C	-356 / 0	0.05 (1)
O- K	-109 / 0	0.03 (1)
O- L	0 / 35	0.01 (1)
X- Y	-180 / 10	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.22/1.00 (L-M:1) , BC=0.13/1.00 (W-X:1) , WB=0.08/1.00 (I-R:1) , SSI=0.15/1.00 (C-Y:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.61 (J) (INPUT = 0.90)
JSI METAL= 0.10 (C) (INPUT = 0.95)

Structural component only
DWG# T-2406369

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425055	T57	3	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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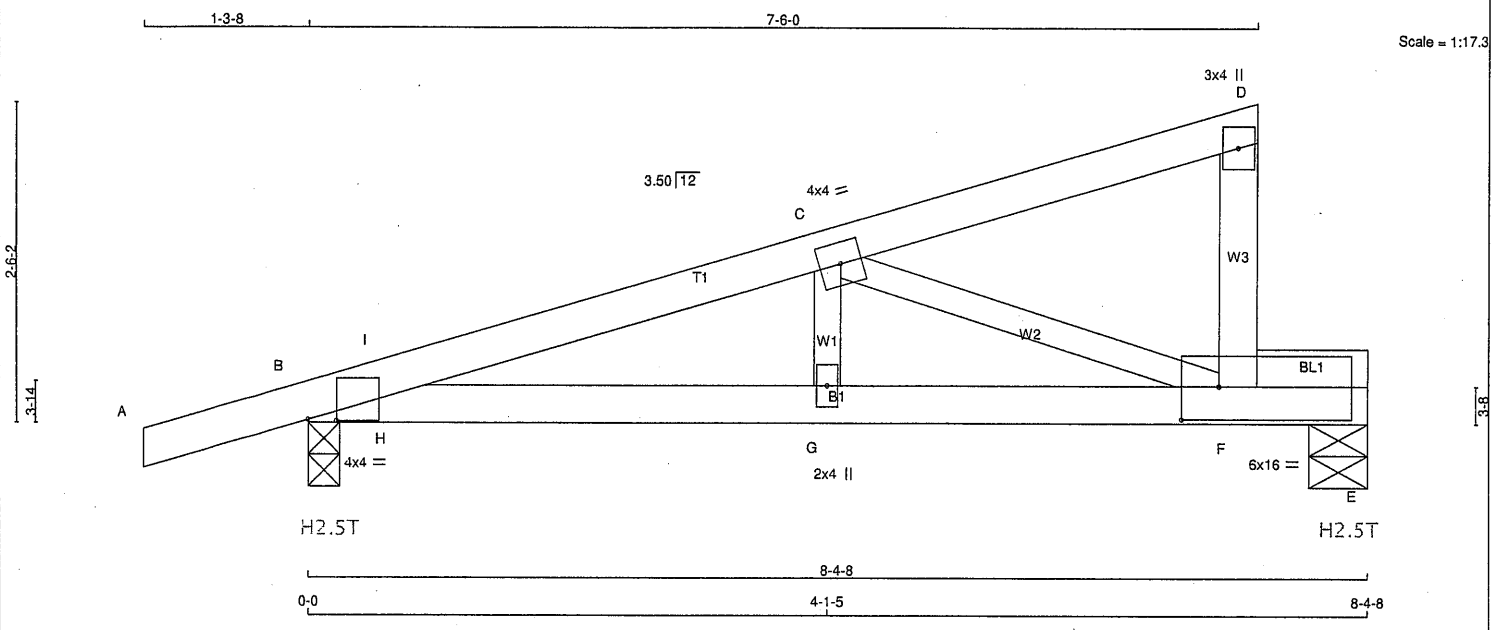

TOTAL WEIGHT = 3 X 27 = 80 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	4.0	4.0	0.25	2.75
C	TMWW-t	MT20	4.0	4.0		
D	TMV+p	MT20	3.0	4.0		
F	BMVWK-t	MT20	6.0	16.0	3.00	3.50
G	BMW+w	MT20	2.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	542	0	542	168	-461	3-0	1-8
E	362	0	362	0	-369	5-8	1-8

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 461 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT E FOR 369 LBS FACTORED UPLIFT

PROVIDE FOR 168 LBS FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	383	255 / 0	0 / 0	0 / 0	0 / -411	128 / 0	0 / 0
E	259	156 / 0	0 / 0	0 / 0	0 / -330	102 / 0	0 / 0
HORIZONTAL REACTIONS							
B	---	0 / 0	0 / 0	0 / 0	120 / 0	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.19 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (12)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 15	-84.9	-84.9	0.10 (1)	10.00	G- C	-339 / 255	0.06 (1)
B- I	-925 / 842	-84.9	-84.9	0.13 (5)	6.25	C- F	-950 / 960	0.21 (1)
I- C	-911 / 867	-84.9	-84.9	0.20 (5)	6.25	H- I	-130 / 67	0.00 (1)
C- D	-51 / 25	-84.9	-84.9	0.12 (1)	6.25			
F- D	-120 / 76	0.0	0.0	0.04 (5)	7.81			
B- H	-848 / 881	-18.5	-18.5	0.25 (1)	6.25			
H- G	-848 / 881	-18.5	-18.5	0.28 (1)	6.25			
G- F	-848 / 881	-18.5	-18.5	0.63 (1)	6.19			
F- E	0 / 0	-18.5	-18.5	0.20 (1)	10.00			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.2} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.28")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.28")
CALCULATED VERT. DEFL.(TL) = L/ 838 (0.12")

CSI: TC=0.20/1.00 (C-I:5) , BC=0.63/1.00 (F-G:1) , WB=0.21/1.00 (C-F:1) , SSI=0.15/1.00 (F-G:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (C) (INPUT = 0.90)
JSI METAL= 0.36 (B) (INPUT = 0.95)

Structural component only
DWG# T-2406372

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425915	T71	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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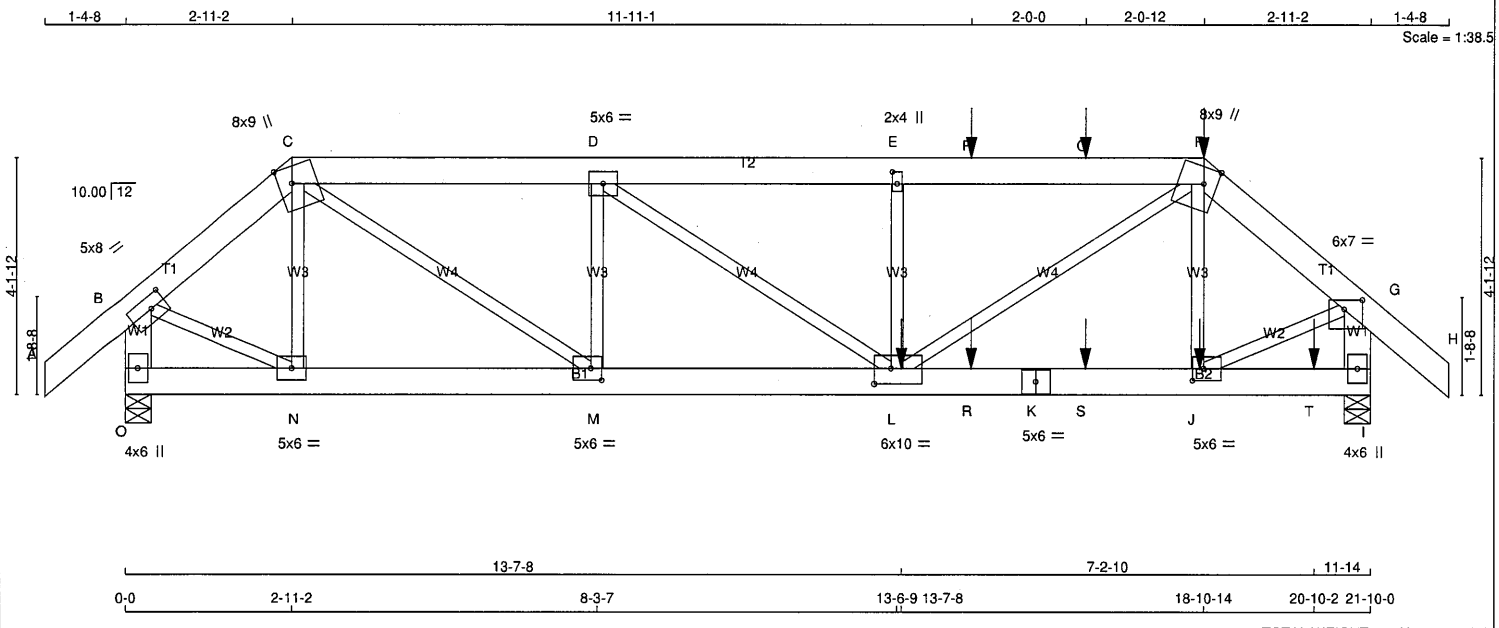

TOTAL WEIGHT = 2 X 122 = 245 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
O - B	2x6	DRY	No.2	SPF
I - G	2x6	DRY	No.2	SPF
O - K	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.50	3.25
C	TTWW+m	MT20	8.0	9.0	Edge	2.75
D	TMWW-t	MT20	5.0	6.0		
E	TMW+w	MT20	2.0	4.0	2.50	1.00
F	TTWW+m	MT20	8.0	9.0	Edge	2.75
G	TMVW-p	MT20	6.0	7.0	2.00	3.75
I	BMV1+p	MT20	4.0	6.0		
J	BMWW-t	MT20	5.0	6.0	2.50	2.25
K	BS-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	6.0	10.0	3.25	3.50
M	BMWW-t	MT20	5.0	6.0	2.50	2.25
N	BMWW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2406485

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1932	0	1932	0	0	5-8	2-1
I	2774	0	2774	0	0	5-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1367	893 / 0	0 / 0	0 / 0	0 / 0	474 / 0	0 / 0
I	1964	1276 / 0	0 / 0	0 / 0	0 / 0	689 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.76 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 41	-84.9	-84.9	0.08 (1)	10.00	N- C	-417 / 0	0.10 (1)
B- C	-1774 / 0	-84.9	-84.9	0.08 (1)	5.87	C- M	0 / 2169	0.54 (1)
C- D	-3136 / 0	-84.9	-84.9	0.24 (1)	4.55	M- D	-1116 / 0	0.27 (1)
D- E	-4092 / 0	-84.9	-84.9	0.30 (1)	4.00	D- L	0 / 1167	0.29 (1)
E- P	-4091 / 0	-84.9	-84.9	0.48 (1)	3.76	L- E	-674 / 0	0.16 (1)
P- Q	-4091 / 0	-84.9	-84.9	0.48 (1)	3.76	L- F	0 / 2508	0.62 (1)
Q- F	-4091 / 0	-84.9	-84.9	0.48 (1)	3.76	J- F	-549 / 0	0.13 (1)
F- G	-2664 / 0	-84.9	-84.9	0.10 (1)	5.00	B- N	0 / 1455	0.36 (1)
G- H	0 / 41	-84.9	-84.9	0.08 (1)	10.00	J- G	0 / 2184	0.54 (1)
O- B	-1927 / 0	0.0	0.0	0.14 (1)	7.20			
I- G	-2766 / 0	0.0	0.0	0.20 (1)	6.24			
O- N	0 / 0	-18.5	-18.5	0.06 (4)	10.00			
N- M	0 / 1343	-18.5	-18.5	0.23 (1)	10.00			
M- L	0 / 3136	-18.5	-18.5	0.50 (1)	10.00			
L- R	0 / 2019	-18.5	-18.5	0.37 (1)	10.00			
R- K	0 / 2019	-18.5	-18.5	0.37 (1)	10.00			
K- S	0 / 2019	-18.5	-18.5	0.37 (1)	10.00			
S- J	0 / 2019	-18.5	-18.5	0.37 (1)	10.00			
J- T	0 / 0	-18.5	-18.5	0.11 (4)	10.00			
T- I	0 / 0	-18.5	-18.5	0.11 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	18-10-14	-30	-30	---	FRONT	VERT	DEAD	---	C1
F	18-10-14	-119	-119	---	BACK	VERT	TOTAL	---	C1
F	18-10-14	-117	-117	---	FRONT	VERT	SNOW	---	C1
J	18-10-2	-29	-29	---	BACK	VERT	TOTAL	---	C1
L	13-7-8	-943	-943	---	BACK	VERT	TOTAL	---	C1
P	14-10-2	-113	-113	---	BACK	VERT	TOTAL	---	C1
Q	16-10-2	-113	-113	---	BACK	VERT	TOTAL	---	C1
R	14-10-2	-29	-29	---	BACK	VERT	TOTAL	---	C1
S	16-10-2	-29	-29	---	BACK	VERT	TOTAL	---	C1
T	20-10-2	-29	-29	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.48/1.00 (E-F:1) , BC=0.50/1.00 (L-M:1) , WB=0.62/1.00 (F-L:1) , SSI=0.31/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (B) (INPUT = 0.90)
JSI METAL= 0.46 (J) (INPUT = 0.95)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425915	T71S	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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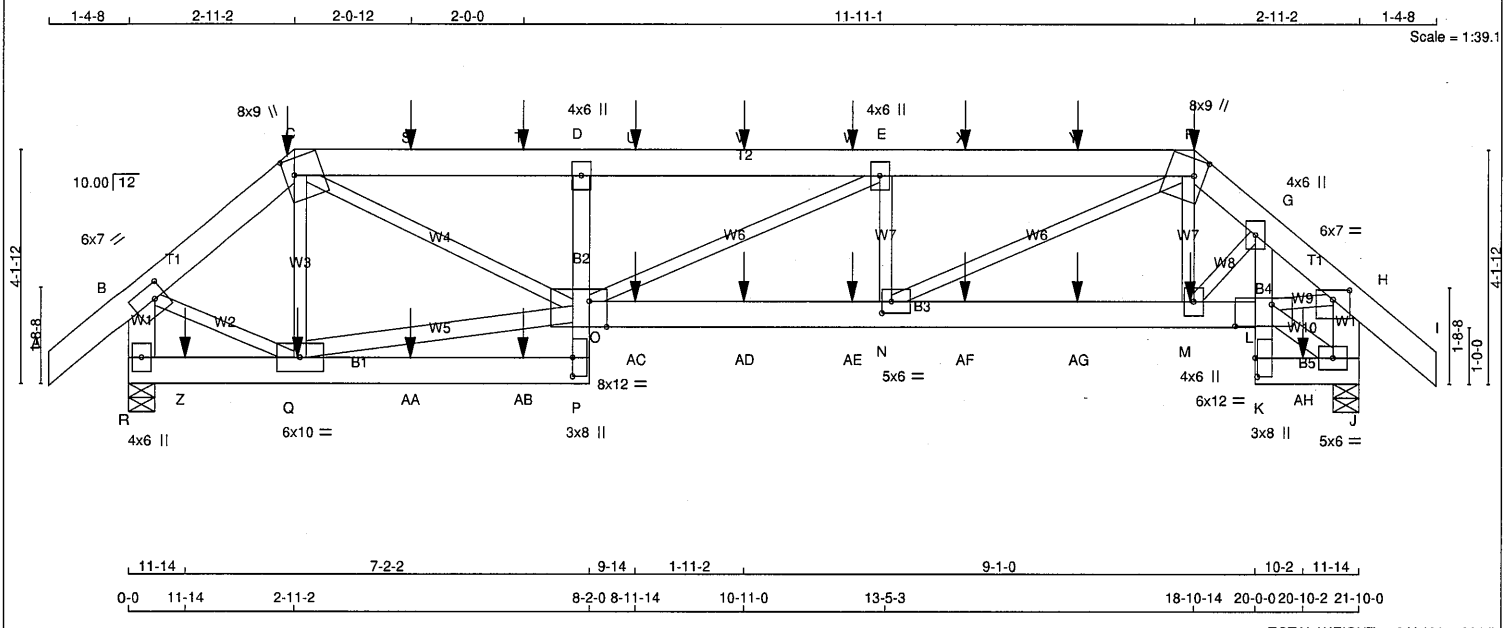

TOTAL WEIGHT = 2 X 130 = 261 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - I	2x6	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
R - P	2x6	DRY	No.2	SPF
P - D	2x4	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
K - G	2x4	DRY	No.2	SPF
K - J	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	6.0	7.0	3.00	2.25
C	TTWW+m	MT20	8.0	9.0	3.50	2.00
D	TMV+p	MT20	4.0	6.0		
E	TMWW+t	MT20	4.0	6.0		
F	TTWW+m	MT20	8.0	9.0	3.50	2.25
G	TMVW+p	MT20	4.0	6.0		
H	TMVW-p	MT20	6.0	7.0	2.00	3.50
J	BMVW1-t	MT20	5.0	6.0		
K	BMV+p	MT20	3.0	8.0	4.00	0.50
L	BVMWW-l	MT20	6.0	12.0	4.50	7.75
M	BMWW+t	MT20	4.0	6.0		
N	BMWW-t	MT20	5.0	6.0	2.50	2.00
O	BVMWWW-l	MT20	8.0	12.0	Edge	3.75
P	BMV+p	MT20	3.0	8.0	4.00	
Q	BMWWW-t	MT20	6.0	10.0		
R	BMV1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1)

Structural component only
DWG# T-2406486

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
R	2378	0	2378	0	0	5-8	2-9
J	2364	0	2364	0	0	5-8	2-9

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1686	1082 / 0	0 / 0	0 / 0	0 / 0	604 / 0	0 / 0
J	1677	1073 / 0	0 / 0	0 / 0	0 / 0	603 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.72 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 41	-84.9	-84.9	0.08 (1)	10.00	
B- C	-2205 / 0	-84.9	-84.9	0.09 (1)	5.40	
C- S	-4333 / 0	-84.9	-84.9	0.44 (1)	3.72	
S- T	-4333 / 0	-84.9	-84.9	0.44 (1)	3.72	
T- D	-4333 / 0	-84.9	-84.9	0.44 (1)	3.72	
D- U	-4385 / 0	-84.9	-84.9	0.34 (1)	3.84	
U- V	-4385 / 0	-84.9	-84.9	0.34 (1)	3.84	
V- W	-4385 / 0	-84.9	-84.9	0.34 (1)	3.84	
W- E	-4385 / 0	-84.9	-84.9	0.34 (1)	3.84	
E- X	-4445 / 0	-84.9	-84.9	0.42 (1)	3.72	
X- Y	-4445 / 0	-84.9	-84.9	0.42 (1)	3.72	
Y- F	-4445 / 0	-84.9	-84.9	0.42 (1)	3.72	
F- G	-2956 / 0	-84.9	-84.9	0.18 (1)	4.70	
G- H	-2959 / 0	-84.9	-84.9	0.18 (1)	4.70	
H- I	0 / 41	-84.9	-84.9	0.08 (1)	10.00	
R- B	-2363 / 0	0.0	0.0	0.17 (1)	6.65	
J- H	-2294 / 0	0.0	0.0	0.17 (1)	6.73	
R- Z	0 / 0	-18.5	-18.5	0.11 (4)	10.00	
Z- Q	0 / 0	-18.5	-18.5	0.11 (4)	10.00	
Q-AA	0 / 138	-18.5	-18.5	0.12 (4)	10.00	
AA-AB	0 / 138	-18.5	-18.5	0.12 (4)	10.00	
AB- P	0 / 138	-18.5	-18.5	0.12 (4)	10.00	
P- O	0 / 82	0.0	0.0	0.22 (1)	10.00	
O- D	-804 / 0	0.0	0.0	0.23 (1)	7.81	
O-AC	0 / 4446	-18.5	-18.5	0.75 (1)	10.00	
AC-AD	0 / 4446	-18.5	-18.5	0.75 (1)	10.00	
AD-AE	0 / 4446	-18.5	-18.5	0.75 (1)	10.00	
AE- N	0 / 4446	-18.5	-18.5	0.75 (1)	10.00	
N-AF	0 / 2319	-18.5	-18.5	0.45 (1)	10.00	
AF-AG	0 / 2319	-18.5	-18.5	0.45 (1)	10.00	
AG- M	0 / 2319	-18.5	-18.5	0.45 (1)	10.00	
M- L	0 / 2229	-18.5	-18.5	0.44 (1)	10.00	
K- L	0 / 43	0.0	0.0	0.10 (1)	10.00	
L- G	-68 / 0	0.0	0.0	0.10 (1)	7.81	
K-AH	0 / 59	-18.5	-18.5	0.03 (4)	10.00	
AH- J	0 / 59	-18.5	-18.5	0.03 (4)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-784 / 0	0.19 (1)
O- E	-68 / 0	0.04 (4)
N- E	-713 / 0	0.13 (1)
N- F	0 / 2363	0.58 (1)
M- F	0 / 150	0.05 (4)
M- G	0 / 123	0.03 (1)
B- Q	0 / 1819	0.45 (1)
L- J	-70 / 0	0.01 (1)
L- H	0 / 2193	0.54 (1)
Q- O	0 / 1581	0.28 (1)
C- O	0 / 2980	0.74 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-11-2	-30	-30	---	FRONT	VERT	DEAD	---	C1
C	2-11-2	-119	-119	---	FRONT	VERT	TOTAL	---	C1

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 945 (0.28")

CSI: TC=0.44/1.00 (C-D:1) , BC=0.75/1.00 (N-O:1) , WB=0.74/1.00 (C-O:1) , SSI=0.31/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-11-2	-117	-117	---	FRONT	VERT	SNOW	---	C1
F	18-10-14	-30	-30	---	FRONT	VERT	DEAD	---	C1
F	18-10-14	-73	-73	---	FRONT	VERT	TOTAL	---	C1
F	18-10-14	-117	-117	---	FRONT	VERT	SNOW	---	C1
M	18-10-2	-65	-65	---	FRONT	VERT	TOTAL	---	C1
Q	2-11-14	-29	-29	---	FRONT	VERT	TOTAL	---	C1
S	4-11-14	-113	-113	---	FRONT	VERT	TOTAL	---	C1
T	6-11-14	-113	-113	---	FRONT	VERT	TOTAL	---	C1
U	8-11-14	-67	-67	---	FRONT	VERT	TOTAL	---	C1
V	10-11-0	-67	-67	---	FRONT	VERT	TOTAL	---	C1
W	12-10-2	-67	-67	---	FRONT	VERT	TOTAL	---	C1
X	14-10-2	-67	-67	---	FRONT	VERT	TOTAL	---	C1
Y	16-10-2	-67	-67	---	FRONT	VERT	TOTAL	---	C1
Z	11-14	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AA	4-11-14	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AB	6-11-14	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AC	8-11-14	-65	-65	---	FRONT	VERT	TOTAL	---	C1
AD	10-11-0	-65	-65	---	FRONT	VERT	TOTAL	---	C1
AE	12-10-2	-65	-65	---	FRONT	VERT	TOTAL	---	C1
AF	14-10-2	-65	-65	---	FRONT	VERT	TOTAL	---	C1
AG	16-10-2	-65	-65	---	FRONT	VERT	TOTAL	---	C1
AH	20-10-2	-31	-31	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

JSI GRIP= 0.80 (B) (INPUT = 0.90)
JSI METAL= 0.50 (O) (INPUT = 0.95)

Structural component only
DWG# T-2406486

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425915	T72	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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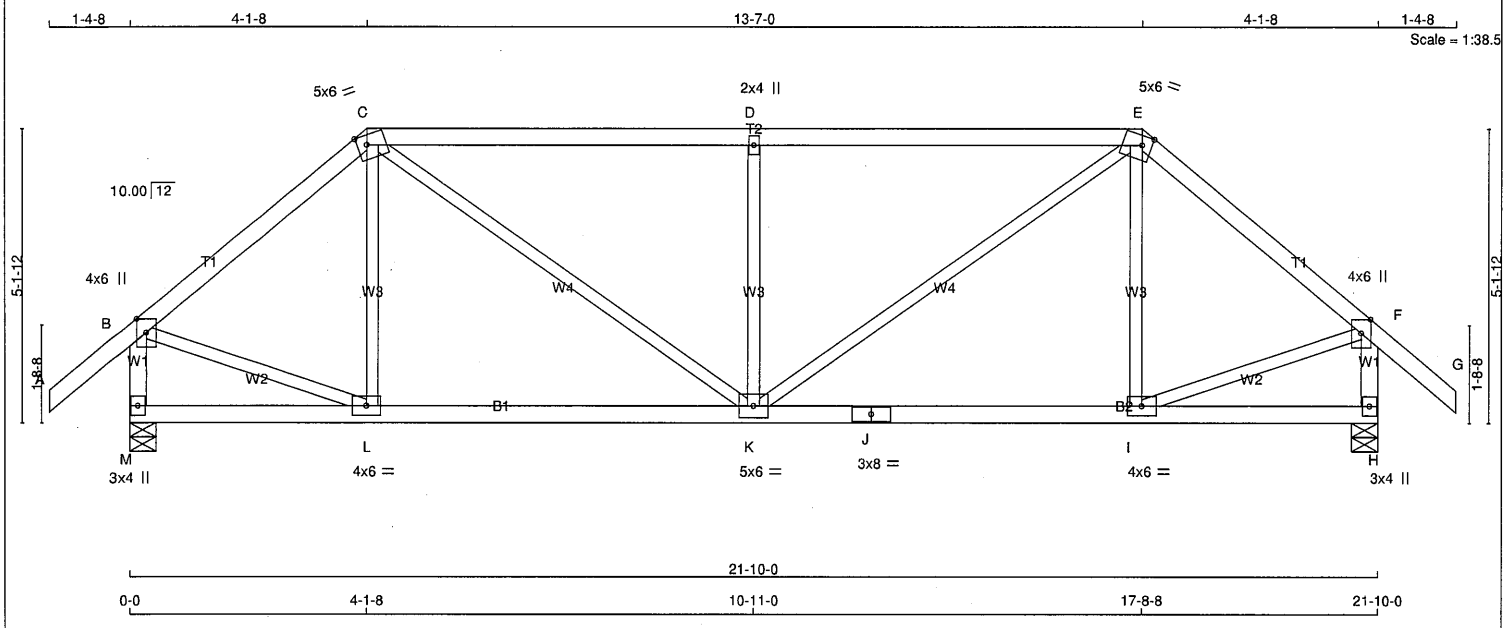

TOTAL WEIGHT = 2 X 91 = 182 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW-m	MT20	5.0	6.0	2.00	2.00
D	TMW+w	MT20	2.0	4.0		
E	TTWW-m	MT20	5.0	6.0	2.00	2.00
F	TMVW+p	MT20	4.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	8.0		
K	BMWWW-t	MT20	5.0	6.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1253	0	1253	0	0	5-8	1-8
H	1253	0	1253	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	887	577 / 0	0 / 0	0 / 0	0 / 0	310 / 0	0 / 0
H	887	577 / 0	0 / 0	0 / 0	0 / 0	310 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.35 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO							FR-TO		
A- B	0 / 40	-84.9	-84.9	0.13	(1)	10.00	L- C	-145 / 29	0.06 (1)
B- C	-1041 / 0	-84.9	-84.9	0.28	(1)	5.80	C- K	0 / 718	0.16 (1)
C- D	-1379 / 0	-84.9	-84.9	0.77	(1)	4.35	K- D	-707 / 0	0.28 (1)
D- E	-1379 / 0	-84.9	-84.9	0.77	(1)	4.35	K- E	0 / 718	0.16 (1)
E- F	-1041 / 0	-84.9	-84.9	0.28	(1)	5.80	I- E	-145 / 29	0.06 (1)
F- G	0 / 40	-84.9	-84.9	0.13	(1)	10.00	B- L	0 / 839	0.19 (1)
M- B	-1227 / 0	0.0	0.0	0.13	(1)	7.23	I- F	0 / 839	0.19 (1)
H- F	-1227 / 0	0.0	0.0	0.13	(1)	7.23			
M- L	0 / 0	-18.5	-18.5	0.13	(4)	10.00			
L- K	0 / 794	-18.5	-18.5	0.24	(4)	10.00			
K- J	0 / 794	-18.5	-18.5	0.24	(4)	10.00			
J- I	0 / 794	-18.5	-18.5	0.24	(4)	10.00			
I- H	0 / 0	-18.5	-18.5	0.13	(4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.77/1.00 (C-D:1) , BC=0.24/1.00 (I-K:4) , WB=0.28/1.00 (D-K:1) , SSI=0.28/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (C) (INPUT = 0.90)
JSI METAL= 0.51 (B) (INPUT = 0.95)

Structural component only
DWG# T-2406487

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425915	T72S	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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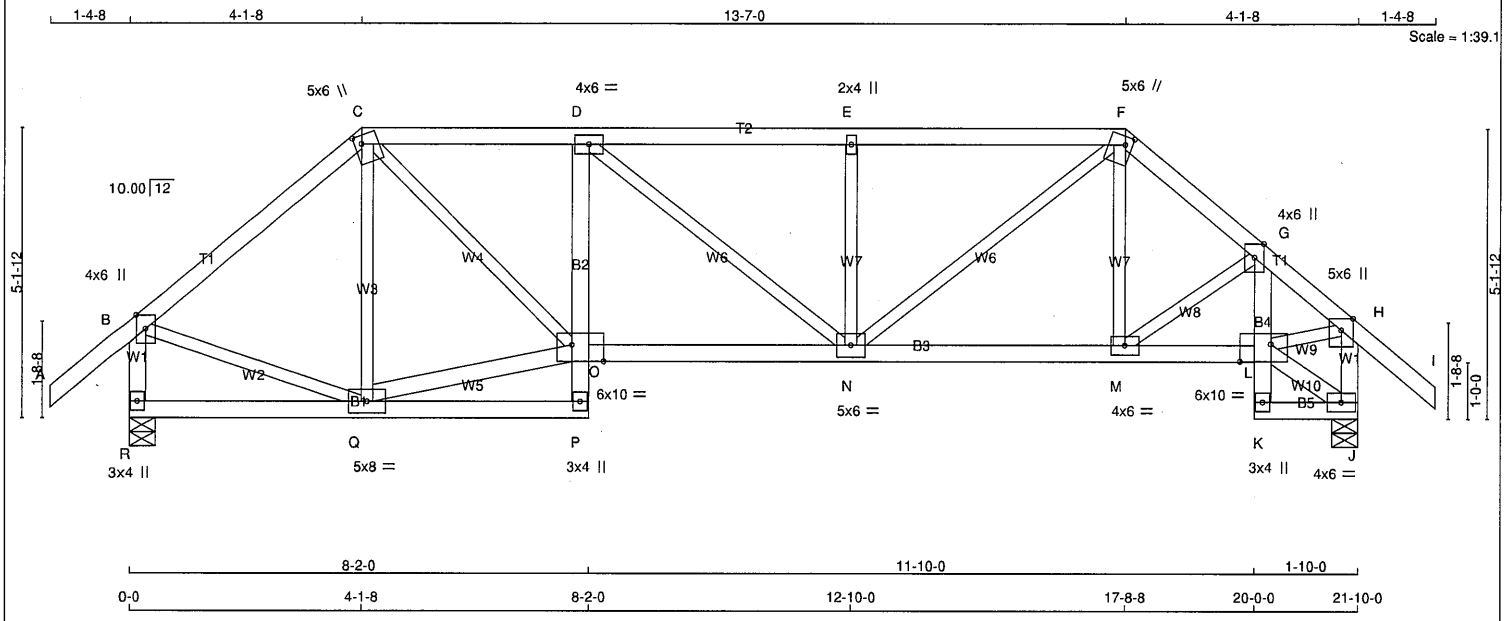

TOTAL WEIGHT = 2 X 104 = 208 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - D	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
K - G	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	1.75	1.50
D	TMVW-t	MT20	4.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	1.75	1.50
G	TMVW+p	MT20	4.0	6.0	Edge	2.00
H	TMVW+p	MT20	5.0	6.0	Edge	
J	BMVW1-t	MT20	4.0	6.0		
K	BMV+p	MT20	3.0	4.0		
L	BVMWW-l	MT20	6.0	10.0	3.75	6.50
M	BMWW-t	MT20	4.0	6.0		
N	BMWWW-t	MT20	5.0	6.0		
O	BVMWW-l	MT20	6.0	10.0	Edge	6.75
P	BMV+p	MT20	3.0	4.0		
Q	BMWWW-t	MT20	5.0	8.0		
R	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1)

Structural component only
DWG# T-2406488

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	1249	0	1249	0	0	5-8	1-8
J	1257	0	1257	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	884	575 / 0	0 / 0	0 / 0	0 / 0	310 / 0	0 / 0
J	890	579 / 0	0 / 0	0 / 0	0 / 0	311 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.87 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-84.9	-84.9	0.13 (1)	10.00
B- C	-1021 / 0	-84.9	-84.9	0.28 (1)	5.85
C- D	-1515 / 0	-84.9	-84.9	0.22 (1)	5.10
D- E	-1594 / 0	-84.9	-84.9	0.32 (1)	4.87
E- F	-1594 / 0	-84.9	-84.9	0.32 (1)	4.87
F- G	-1274 / 0	-84.9	-84.9	0.10 (1)	5.61
G- H	-1309 / 0	-84.9	-84.9	0.14 (1)	5.49
H- I	0 / 40	-84.9	-84.9	0.13 (1)	10.00
R- B	-1220 / 0	0.0	0.0	0.13 (1)	7.26
J- H	-1221 / 0	0.0	0.0	0.13 (1)	7.25
R- Q	0 / 0	-18.5	-18.5	0.09 (4)	10.00
Q- P	0 / 34	-18.5	-18.5	0.09 (4)	10.00
P- O	0 / 31	0.0	0.0	0.05 (1)	10.00
O- D	-440 / 0	0.0	0.0	0.07 (1)	7.81
O- N	0 / 1524	-18.5	-18.5	0.30 (1)	10.00
N- M	0 / 974	-18.5	-18.5	0.20 (1)	10.00
M- L	0 / 1002	-18.5	-18.5	0.20 (1)	10.00
K- L	0 / 17	0.0	0.0	0.05 (1)	10.00
L- G	-112 / 0	0.0	0.0	0.05 (1)	7.81
K- J	0 / 31	-18.5	-18.5	0.02 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-368 / 0	0.14 (1)
D- N	0 / 90	0.02 (1)
N- E	-457 / 0	0.12 (1)
N- F	0 / 788	0.18 (1)
M- F	0 / 74	0.03 (4)
M- G	-35 / 0	0.01 (1)
B- Q	0 / 826	0.19 (1)
Q- O	0 / 778	0.13 (1)
C- O	0 / 1018	0.23 (1)
L- J	-37 / 0	0.00 (1)
L- H	0 / 994	0.22 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.32/1.00 (E-F:1) , BC=0.30/1.00 (N-O:1) , WB=0.23/1.00 (C-O:1) , SSI=0.19/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (C) (INPUT = 0.90)
JSI METAL= 0.50 (B) (INPUT = 0.95)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425915	T73	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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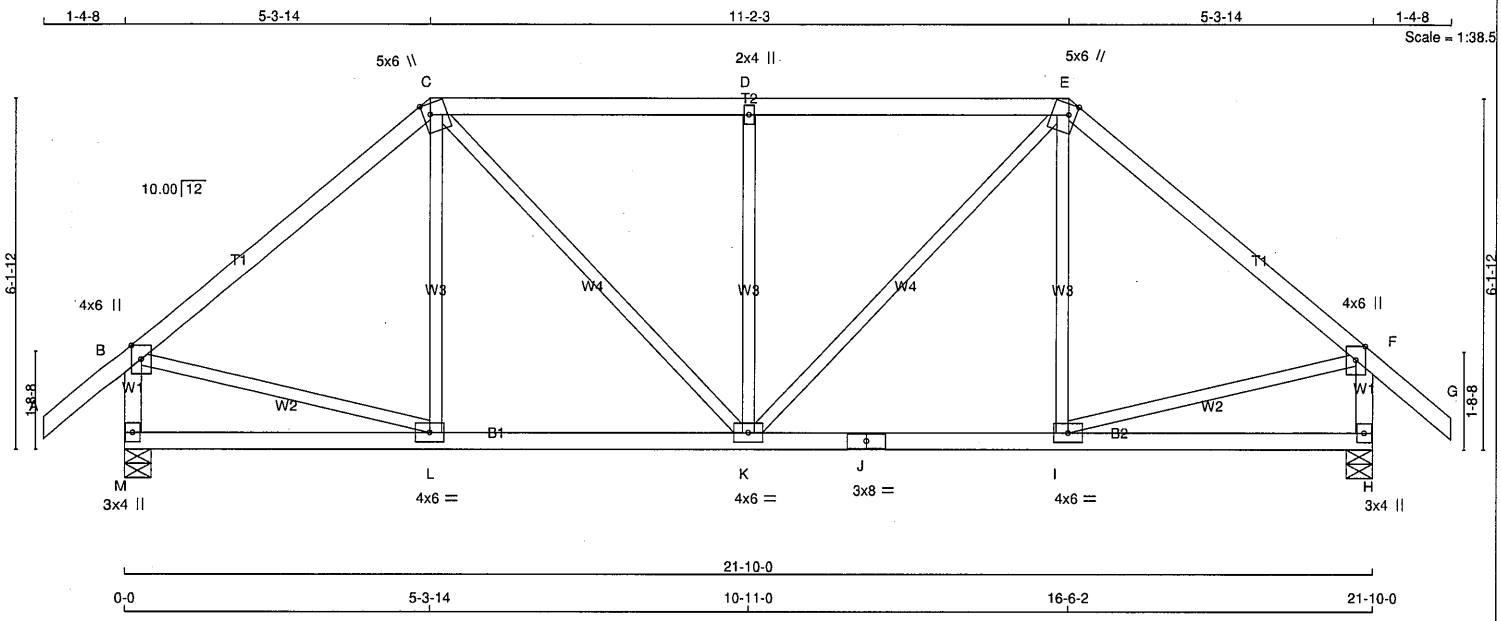

TOTAL WEIGHT = 2 X 96 = 191 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMVW+p	MT20	4.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	8.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2406489

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1253	0	1253	0	0	5-8	1-8
H	1253	0	1253	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	887	577 / 0	0 / 0	0 / 0	0 / 0	310 / 0	0 / 0
H	887	577 / 0	0 / 0	0 / 0	0 / 0	310 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.31 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-84.9	-84.9	0.13	(1)	10.00	L- C	-84 / 54	0.05 (1)
B- C	-1032 / 0	-84.9	-84.9	0.47	(1)	5.52	C- K	0 / 466	0.10 (1)
C- D	-1112 / 0	-84.9	-84.9	0.48	(1)	5.31	K- D	-580 / 0	0.35 (1)
D- E	-1112 / 0	-84.9	-84.9	0.48	(1)	5.31	K- E	0 / 466	0.10 (1)
E- F	-1032 / 0	-84.9	-84.9	0.47	(1)	5.52	I- E	-84 / 54	0.05 (1)
F- G	0 / 40	-84.9	-84.9	0.13	(1)	10.00	B- L	0 / 816	0.18 (1)
M- B	-1213 / 0	0.0	0.0	0.13	(1)	7.27	I- F	0 / 816	0.18 (1)
H- F	-1213 / 0	0.0	0.0	0.13	(1)	7.27			
M- L	0 / 0	-18.5	-18.5	0.13	(4)	10.00			
L- K	0 / 790	-18.5	-18.5	0.21	(4)	10.00			
K- J	0 / 790	-18.5	-18.5	0.21	(4)	10.00			
J- I	0 / 790	-18.5	-18.5	0.21	(4)	10.00			
I- H	0 / 0	-18.5	-18.5	0.13	(4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.48/1.00 (D-E:1) , BC=0.21/1.00 (I-K:4) , WB=0.35/1.00 (D-K:1) , SSI=0.23/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (B) (INPUT = 0.90)
JSI METAL= 0.52 (B) (INPUT = 0.95)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425915	T73S	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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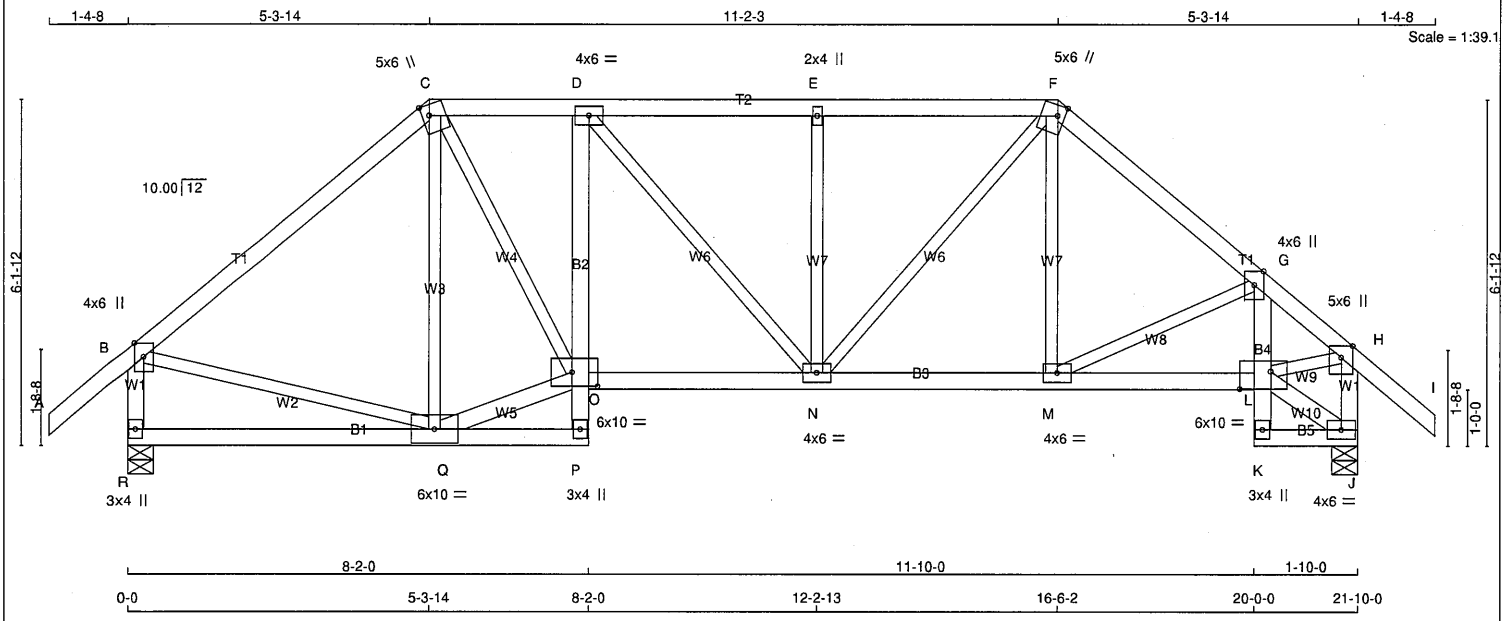

TOTAL WEIGHT = 2 X 109 = 219 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - D	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
K - G	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMVW-t	MT20	4.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMVW+p	MT20	4.0	6.0	Edge	2.00
H	TMVW+p	MT20	5.0	6.0	Edge	
J	BMVW1-t	MT20	4.0	6.0		
K	BMV+p	MT20	3.0	4.0		
L	BVMWW-l	MT20	6.0	10.0	3.75	6.50
M	BMWW-t	MT20	4.0	6.0		
N	BMWWW-t	MT20	4.0	6.0		
O	BVMWW-l	MT20	6.0	10.0	3.00	5.50
P	BMV+p	MT20	3.0	4.0		
Q	BMWWW-t	MT20	6.0	10.0		
R	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1)

Structural component only
DWG# T-2406490

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	1249	0	1249	0	0	5-8	1-8
J	1257	0	1257	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	884	575 / 0	0 / 0	0 / 0	0 / 0	310 / 0	0 / 0
J	890	579 / 0	0 / 0	0 / 0	0 / 0	311 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.41 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-84.9	-84.9	0.13 (1)	10.00
B- C	-1033 / 0	-84.9	-84.9	0.47 (1)	5.52
C- D	-1197 / 0	-84.9	-84.9	0.13 (1)	5.69
D- E	-1277 / 0	-84.9	-84.9	0.24 (1)	5.41
E- F	-1277 / 0	-84.9	-84.9	0.24 (1)	5.41
F- G	-1227 / 0	-84.9	-84.9	0.17 (1)	5.60
G- H	-1332 / 0	-84.9	-84.9	0.14 (1)	5.45
H- I	0 / 40	-84.9	-84.9	0.13 (1)	10.00
R- B	-1207 / 0	0.0	0.0	0.13 (1)	7.29
J- H	-1221 / 0	0.0	0.0	0.13 (1)	7.25
R- Q	0 / 0	-18.5	-18.5	0.13 (4)	10.00
Q- P	0 / 25	-18.5	-18.5	0.13 (4)	10.00
P- O	-2 / 0	0.0	0.0	0.04 (1)	10.00
O- D	-382 / 0	0.0	0.0	0.07 (1)	7.81
O- N	0 / 1203	-18.5	-18.5	0.23 (1)	10.00
N- M	0 / 932	-18.5	-18.5	0.19 (1)	10.00
M- L	0 / 1038	-18.5	-18.5	0.20 (1)	10.00
K- L	0 / 17	0.0	0.0	0.05 (1)	10.00
L- G	-115 / 0	0.0	0.0	0.05 (1)	7.81
K- J	0 / 31	-18.5	-18.5	0.02 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-414 / 0	0.25 (1)
D- N	0 / 113	0.03 (1)
N- E	-408 / 0	0.16 (1)
N- F	0 / 520	0.12 (1)
M- F	0 / 105	0.03 (4)
M- G	-120 / 0	0.03 (1)
B- Q	0 / 815	0.18 (1)
Q- O	0 / 814	0.13 (1)
C- O	0 / 871	0.20 (1)
L- J	-38 / 0	0.00 (1)
L- H	0 / 1031	0.23 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.47/1.00 (B-C:1) , BC=0.23/1.00 (N-O:1) , WB=0.25/1.00 (C-Q:1) , SSI=0.17/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (B) (INPUT = 0.90)
JSI METAL= 0.52 (B) (INPUT = 0.95)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425915	T74	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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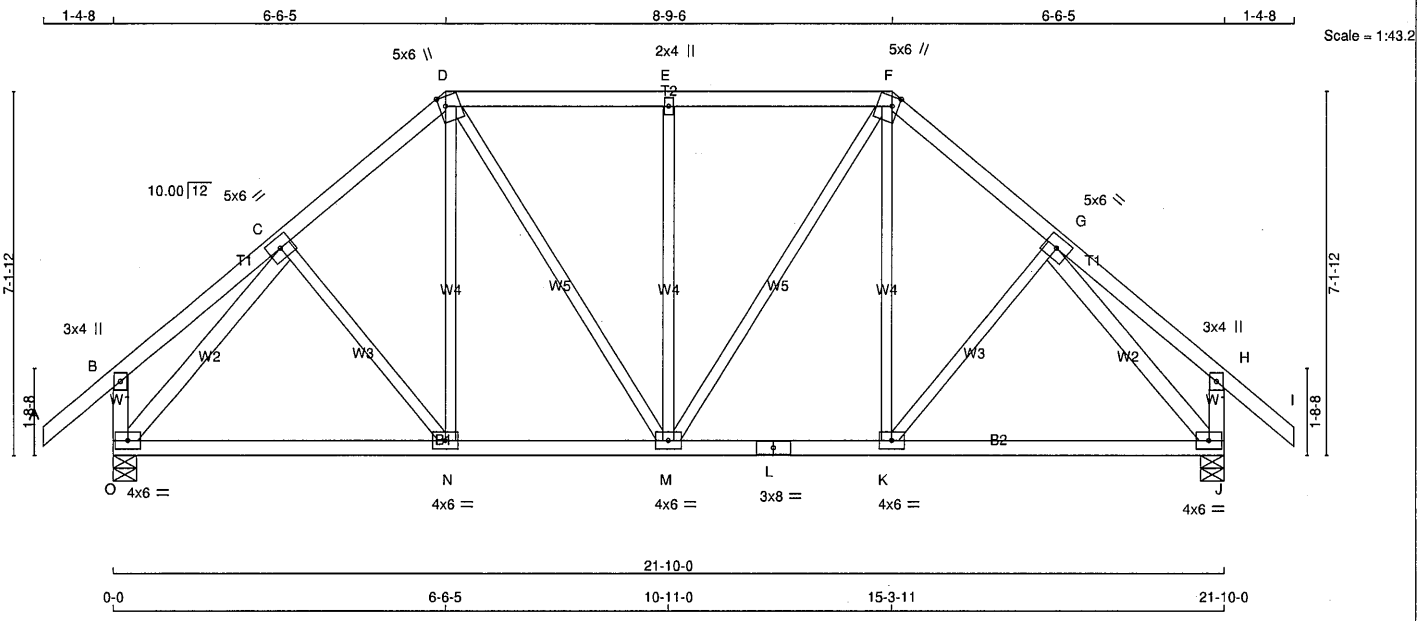

TOTAL WEIGHT = 2 X 110 = 219 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
O - C	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	5.0	6.0		
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	4.0	6.0		
K	BMWW-t	MT20	4.0	6.0		
L	BS-t	MT20	3.0	8.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BMWW-t	MT20	4.0	6.0		
O	BMVW1-t	MT20	4.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1253	0	1253	0	0	5-8	1-8
J	1253	0	1253	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	887	577 / 0	0 / 0	0 / 0	0 / 0	310 / 0	0 / 0
J	887	577 / 0	0 / 0	0 / 0	0 / 0	310 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.09 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 40	-84.9	-84.9	0.13 (1)		10.00
B- C	0 / 20	-84.9	-84.9	0.14 (1)		10.00
C- D	-1019 / 0	-84.9	-84.9	0.12 (1)		6.09
D- E	-926 / 0	-84.9	-84.9	0.21 (1)		6.17
E- F	-926 / 0	-84.9	-84.9	0.21 (1)		6.17
F- G	-1019 / 0	-84.9	-84.9	0.12 (1)		6.09
G- H	0 / 20	-84.9	-84.9	0.14 (1)		10.00
H- I	0 / 40	-84.9	-84.9	0.13 (1)		10.00
O- B	-230 / 0	0.0	0.0	0.02 (1)		7.81
J- H	-230 / 0	0.0	0.0	0.02 (1)		7.81
O- N	0 / 772	-18.5	-18.5	0.24 (4)		10.00
N- M	0 / 767	-18.5	-18.5	0.25 (4)		10.00
M- L	0 / 767	-18.5	-18.5	0.25 (4)		10.00
L- K	0 / 767	-18.5	-18.5	0.25 (4)		10.00
K- J	0 / 772	-18.5	-18.5	0.24 (4)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI
C- N	-11 / 34	0.01 (4)
N- D	0 / 138	0.04 (4)
D- M	0 / 295	0.07 (1)
M- E	-451 / 0	0.40 (1)
M- F	0 / 295	0.07 (1)
K- F	0 / 138	0.04 (4)
K- G	-11 / 34	0.01 (4)
O- C	-1242 / 0	0.39 (1)
G- J	-1242 / 0	0.39 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.21/1.00 (D-E:1) , BC=0.25/1.00 (K-M:4) , WB=0.40/1.00 (E-M:1) , SSI=0.18/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.57 (O) (INPUT = 0.90)
JSI METAL= 0.28 (G) (INPUT = 0.95)

Structural component only
DWG# T-2406491

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425915	T74S	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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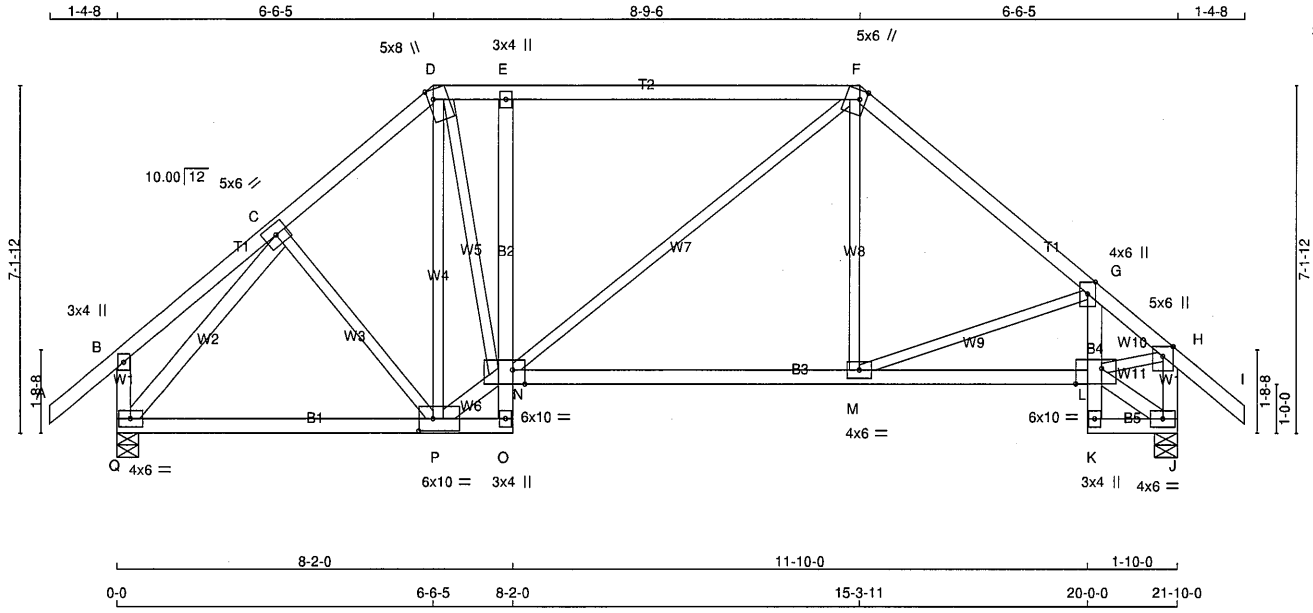

TOTAL WEIGHT = 2 X 112 = 224 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - E	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
K - G	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
Q - C	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	8.0	Edge	1.25
E	TMV+p	MT20	3.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMVW+p	MT20	4.0	6.0	Edge	2.00
H	TMVW+p	MT20	5.0	6.0	Edge	
J	BMVW1-t	MT20	4.0	6.0		
K	BMV+p	MT20	3.0	4.0		
L	BVMWW-I	MT20	6.0	10.0	3.75	6.50
M	BMWW-t	MT20	4.0	6.0		
N	BVMWWW-I	MT20	6.0	10.0	Edge	3.00
O	BMV+p	MT20	3.0	4.0		
P	BMWWW-t	MT20	6.0	10.0	3.00	3.50
Q	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1)

Structural component only
DWG# T-2406492

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
Q	1249	0	1249	0	0	5-8	1-8
J	1257	0	1257	0	0	5-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	884	575 / 0	0 / 0	0 / 0	0 / 0	310 / 0	0 / 0
J	890	579 / 0	0 / 0	0 / 0	0 / 0	311 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.35 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-84.9	-84.9	0.13 (1)	10.00	C- P	-13 / 34	0.01 (4)
B- C	0 / 21	-84.9	-84.9	0.14 (1)	10.00	P- D	-350 / 0	0.31 (1)
C- D	-1019 / 0	-84.9	-84.9	0.12 (1)	6.08	N- F	0 / 201	0.05 (1)
D- E	-1038 / 0	-84.9	-84.9	0.47 (1)	5.49	M- F	0 / 194	0.06 (4)
E- F	-1048 / 0	-84.9	-84.9	0.55 (1)	5.35	M- G	-216 / 0	0.10 (1)
F- G	-1179 / 0	-84.9	-84.9	0.26 (1)	5.58	Q- C	-1243 / 0	0.39 (1)
G- H	-1367 / 0	-84.9	-84.9	0.19 (1)	5.36	P- N	0 / 891	0.14 (1)
H- I	0 / 40	-84.9	-84.9	0.13 (1)	10.00	D- N	0 / 1136	0.26 (1)
Q- B	-229 / 0	0.0	0.0	0.02 (1)	7.81	L- J	-39 / 0	0.00 (1)
J- H	-1220 / 0	0.0	0.0	0.13 (1)	7.26	L- H	0 / 1084	0.24 (1)
Q- P	0 / 773	-18.5	-18.5	0.27 (4)	10.00			
P- O	0 / 36	-18.5	-18.5	0.21 (4)	10.00			
O- N	-58 / 0	0.0	0.0	0.04 (1)	7.81			
N- E	-711 / 0	0.0	0.0	0.40 (1)	7.81			
N- M	0 / 891	-18.5	-18.5	0.30 (4)	10.00			
M- L	0 / 1091	-18.5	-18.5	0.31 (4)	10.00			
K- L	0 / 17	0.0	0.0	0.05 (1)	10.00			
L- G	-129 / 0	0.0	0.0	0.04 (1)	7.81			
K- J	0 / 32	-18.5	-18.5	0.02 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.55/1.00 (E-F:1) , BC=0.40/1.00 (E-N:1) , WB=0.39/1.00 (C-Q:1) , SSI=0.29/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.73 (H) (INPUT = 0.90)
JSI METAL= 0.50 (H) (INPUT = 0.95)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425915	T75	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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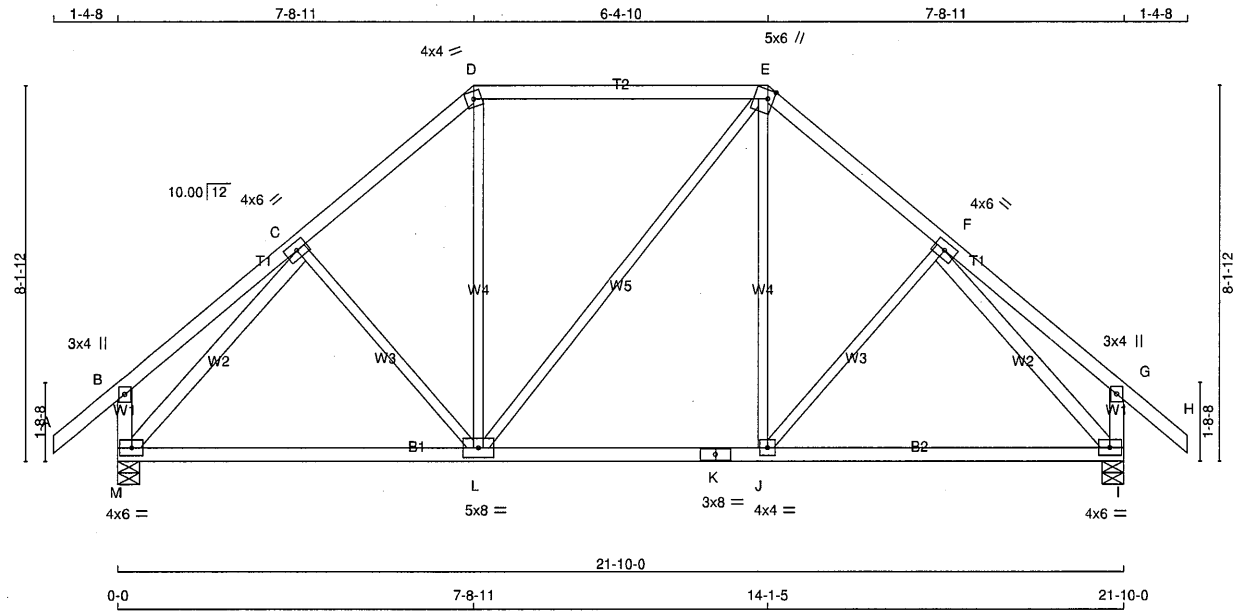

TOTAL WEIGHT = 2 X 105 = 210 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
M - C	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMWW-t	MT20	4.0	6.0		
G	TMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	8.0		
L	BMWWW-t	MT20	5.0	8.0		
M	BMVW1-t	MT20	4.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1253	0	1253	0	0	5-8	1-8
I	1253	0	1253	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	887	577 / 0	0 / 0	0 / 0	0 / 0	310 / 0	0 / 0
I	887	577 / 0	0 / 0	0 / 0	0 / 0	310 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.12 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-84.9	-84.9	0.13 (1)	10.00
B- C	0 / 25	-84.9	-84.9	0.20 (1)	10.00
C- D	-976 / 0	-84.9	-84.9	0.17 (1)	6.12
D- E	-732 / 0	-84.9	-84.9	0.45 (1)	6.25
E- F	-976 / 0	-84.9	-84.9	0.17 (1)	6.12
F- G	0 / 25	-84.9	-84.9	0.20 (1)	10.00
G- H	0 / 40	-84.9	-84.9	0.13 (1)	10.00
M- B	-249 / 0	0.0	0.0	0.03 (1)	7.81
I- G	-249 / 0	0.0	0.0	0.03 (1)	7.81
M- L	0 / 792	-18.5	-18.5	0.30 (4)	10.00
L- K	0 / 732	-18.5	-18.5	0.29 (4)	10.00
K- J	0 / 732	-18.5	-18.5	0.29 (4)	10.00
J- I	0 / 792	-18.5	-18.5	0.30 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- L	-95 / 11	0.06 (1)
L- D	0 / 213	0.05 (4)
L- E	0 / 0	0.00 (1)
J- E	0 / 213	0.05 (4)
J- F	-95 / 11	0.06 (1)
M- C	-1232 / 0	0.53 (1)
F- I	-1232 / 0	0.53 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.45/1.00 (D-E:1) , BC=0.30/1.00 (I-J:4) , WB=0.53/1.00 (C-M:1) , SSI=0.21/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (C) (INPUT = 0.90)
JSI METAL= 0.33 (K) (INPUT = 0.95)

Structural component only
DWG# T-2406493

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425915	T75S	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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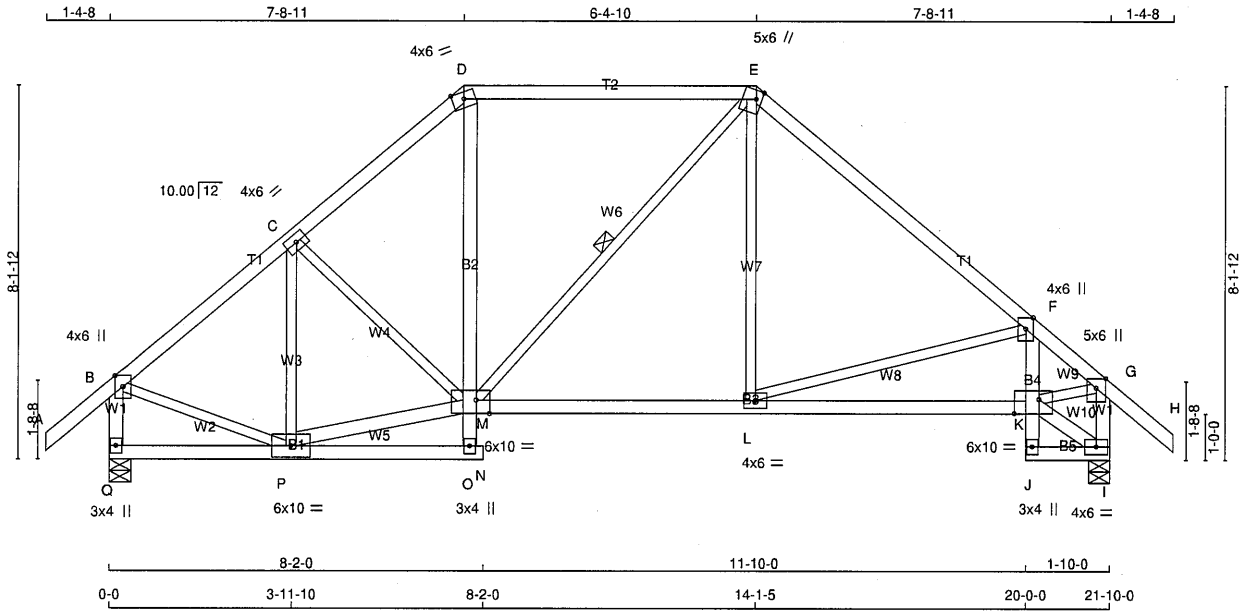

TOTAL WEIGHT = 2 X 110 = 220 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
O - D	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
J - F	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMWW-t	MT20	4.0	6.0		
D	TTV-m	MT20	4.0	6.0	Edge	
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMVW+p	MT20	4.0	6.0	Edge	2.00
G	TMVW+p	MT20	5.0	6.0	Edge	
I	BMVW1-t	MT20	4.0	6.0		
J	BMV+p	MT20	3.0	4.0		
K	BVMWW-I	MT20	6.0	10.0	3.75	6.50
L	BMWW-t	MT20	4.0	6.0		
M	BVMWWW-I	MT20	6.0	10.0	Edge	3.50
O	BMV+p	MT20	3.0	4.0		
P	BMWWW-t	MT20	6.0	10.0		
Q	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2406494

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	1252	0	1252	0	0	5-8	1-8
I	1259	0	1259	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	887	575 / 0	0 / 0	0 / 0	0 / 0	312 / 0	0 / 0
I	891	579 / 0	0 / 0	0 / 0	0 / 0	312 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.12 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI
A- B	0 / 40	-84.9	-84.9	0.13 (1)	10.00	P- C	-405 / 0	0.15 (1)
B- C	-1033 / 0	-84.9	-84.9	0.17 (1)	5.99	M- E	-16 / 0	0.01 (4)
C- D	-1091 / 0	-84.9	-84.9	0.17 (1)	5.87	L- E	0 / 219	0.06 (4)
D- E	-825 / 0	-84.9	-84.9	0.45 (1)	6.04	L- F	-349 / 0	0.26 (1)
E- F	-1109 / 0	-84.9	-84.9	0.40 (1)	5.52	B- P	0 / 863	0.19 (1)
F- G	-1415 / 0	-84.9	-84.9	0.32 (1)	5.12	P- M	0 / 835	0.13 (1)
G- H	0 / 40	-84.9	-84.9	0.13 (1)	10.00	C- M	-13 / 0	0.01 (1)
Q- B	-1224 / 0	0.0	0.0	0.13 (1)	7.25	K- I	-40 / 0	0.00 (1)
I- G	-1222 / 0	0.0	0.0	0.13 (1)	7.25	K- G	0 / 1156	0.26 (1)
Q- P	0 / 0	-18.5	-18.5	0.08 (4)	10.00			
P- O	0 / 15	-18.5	-18.5	0.08 (4)	10.00			
O- N	0 / 0	-18.5	-18.5	0.00 (4)	10.00			
O- M	0 / 39	0.0	0.0	0.03 (1)	10.00			
M- D	0 / 295	0.0	0.0	0.07 (1)	10.00			
M- L	0 / 831	-18.5	-18.5	0.28 (4)	10.00			
L- K	0 / 1163	-18.5	-18.5	0.30 (1)	10.00			
J- K	0 / 17	0.0	0.0	0.05 (1)	10.00			
K- F	-129 / 0	0.0	0.0	0.04 (1)	7.81			
J- I	0 / 34	-18.5	-18.5	0.02 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
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- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.45/1.00 (D-E:1) , BC=0.30/1.00 (K-L:1) , WB=0.26/1.00 (G-K:1) , SSI=0.21/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (B) (INPUT = 0.90)
JSI METAL= 0.51 (G) (INPUT = 0.95)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425915	T76	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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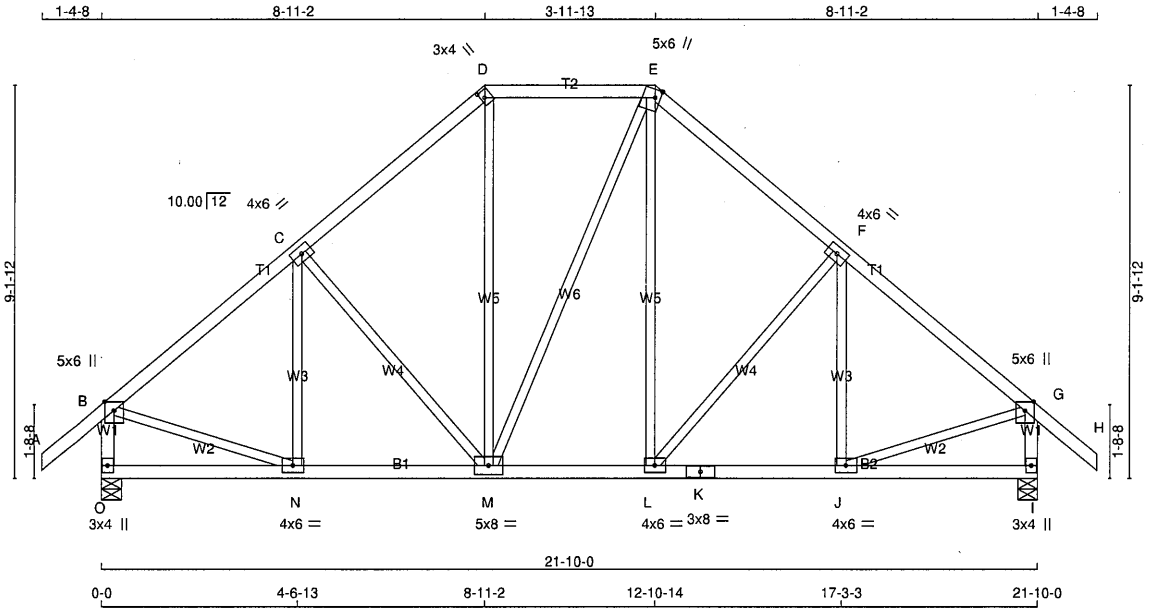

TOTAL WEIGHT = 2 X 111 = 221 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	6.0		
D	TTW+h	MT20	3.0	4.0	2.00	1.00
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMWW-t	MT20	4.0	6.0		
G	TMVW+p	MT20	5.0	6.0	Edge	
I	BMV1+p	MT20	3.0	4.0		
J, L, N						
J	BMWW-t	MT20	4.0	6.0		
K	BS-t	MT20	3.0	8.0		
M	BMWWW-t	MT20	5.0	8.0		
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1253	0	1253	0	0	5-8	1-8
I	1253	0	1253	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	887	577 / 0	0 / 0	0 / 0	0 / 0	310 / 0	0 / 0
I	887	577 / 0	0 / 0	0 / 0	0 / 0	310 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.87 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-84.9	-84.9	0.13 (1)	10.00	N- C	-159 / 15	0.07 (1)
B- C	-1055 / 0	-84.9	-84.9	0.23 (1)	5.87	C- M	-242 / 0	0.21 (1)
C- D	-910 / 0	-84.9	-84.9	0.23 (1)	6.20	M- D	0 / 258	0.06 (1)
D- E	-677 / 0	-84.9	-84.9	0.18 (1)	6.25	M- E	0 / 2	0.00 (1)
E- F	-909 / 0	-84.9	-84.9	0.23 (1)	6.20	L- E	0 / 256	0.06 (1)
F- G	-1055 / 0	-84.9	-84.9	0.23 (1)	5.87	L- F	-243 / 0	0.21 (1)
G- H	0 / 40	-84.9	-84.9	0.13 (1)	10.00	J- F	-157 / 16	0.07 (1)
O- B	-1218 / 0	0.0	0.0	0.13 (1)	7.26	B- N	0 / 869	0.20 (1)
I- G	-1218 / 0	0.0	0.0	0.13 (1)	7.26	J- G	0 / 869	0.20 (1)
O- N	0 / 0	-18.5	-18.5	0.09 (4)	10.00			
N- M	0 / 832	-18.5	-18.5	0.18 (1)	10.00			
M- L	0 / 677	-18.5	-18.5	0.14 (1)	10.00			
L- K	0 / 832	-18.5	-18.5	0.18 (1)	10.00			
K- J	0 / 832	-18.5	-18.5	0.18 (1)	10.00			
J- I	0 / 0	-18.5	-18.5	0.09 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.23/1.00 (F-G:1) , BC=0.18/1.00 (J-L:1) , WB=0.21/1.00 (F-L:1) , SSI=0.14/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (D) (INPUT = 0.90)
JSI METAL= 0.42 (G) (INPUT = 0.95)

Structural component only
DWG# T-2406495

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425915	T76S	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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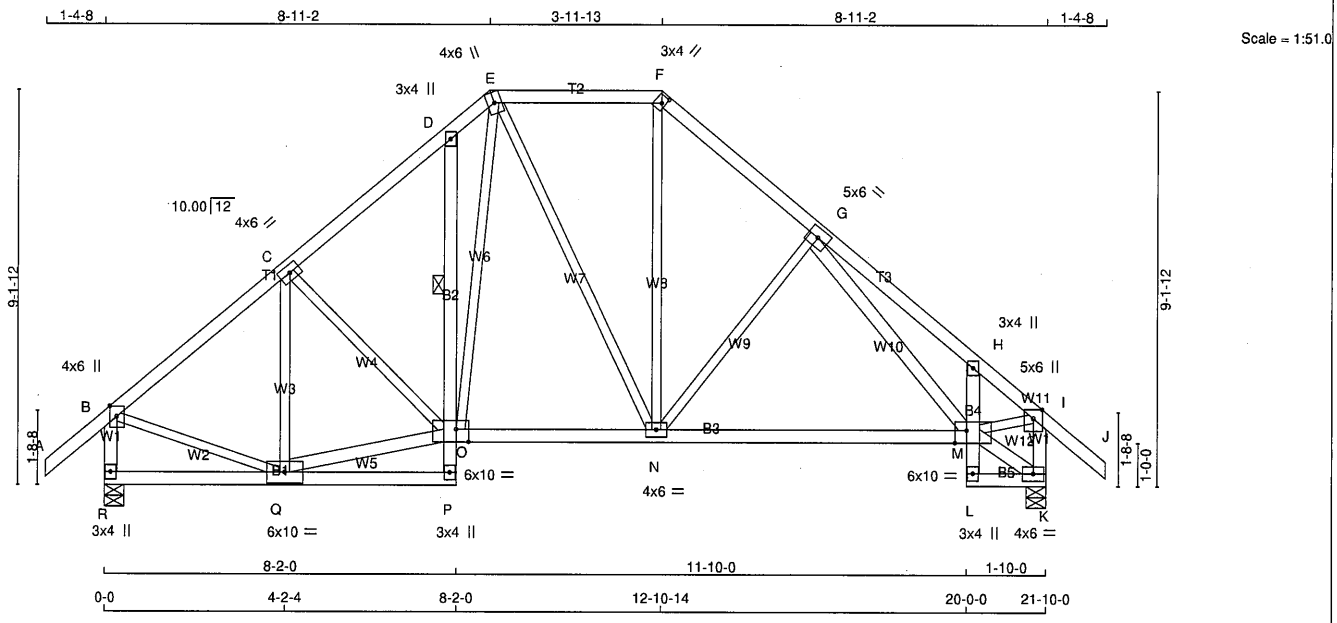

TOTAL WEIGHT = 2 X 124 = 248 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - J	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - D	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
G - M	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMWW-t	MT20	4.0	6.0		
D	TMV+p	MT20	3.0	4.0		
E	TTWW+m	MT20	4.0	6.0		
F	TTW+h	MT20	3.0	4.0	2.00	1.00
G	TMWW-t	MT20	5.0	6.0		
H	TMV+p	MT20	3.0	4.0		
I	TMVW+p	MT20	5.0	6.0	Edge	
K	BMVW1-t	MT20	4.0	6.0		
L	BMV+p	MT20	3.0	4.0		
M	BVMWWW-I	MT20	6.0	10.0	Edge	3.25
N	BMWWW-t	MT20	4.0	6.0		
O	BVMWWW-I	MT20	6.0	10.0	Edge	3.50
P	BMV+p	MT20	3.0	4.0		
Q	BMWWW-t	MT20	6.0	10.0		
R	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1)

Structural component only
DWG# T-2406496

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
R	1249	0	1249	0	0	IN-SX	IN-SX
K	1257	0	1257	0	0	5-8	1-8
						5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	884	575 / 0	0 / 0	0 / 0	0 / 0	310 / 0	0 / 0
K	890	579 / 0	0 / 0	0 / 0	0 / 0	311 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.65 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-84.9	-84.9	0.13 (1)	10.00	
B- C	-1033 / 0	-84.9	-84.9	0.18 (1)	5.98	
C- D	-1077 / 0	-84.9	-84.9	0.18 (1)	5.89	
D- E	-1072 / 0	-84.9	-84.9	0.04 (1)	6.06	
E- F	-766 / 0	-84.9	-84.9	0.17 (1)	6.25	
F- G	-1016 / 0	-84.9	-84.9	0.15 (1)	6.05	
G- H	-1213 / 0	-84.9	-84.9	0.16 (1)	5.66	
H- I	-1247 / 0	-84.9	-84.9	0.11 (1)	5.65	
I- J	0 / 40	-84.9	-84.9	0.13 (1)	10.00	
R- B	-1219 / 0	0.0	0.0	0.13 (1)	7.26	
K- I	-1223 / 0	0.0	0.0	0.13 (1)	7.25	
R- Q	0 / 0	-18.5	-18.5	0.08 (4)	10.00	
Q- P	0 / 13	-18.5	-18.5	0.08 (4)	10.00	
P- O	0 / 32	0.0	0.0	0.02 (1)	10.00	
O- D	-197 / 0	0.0	0.0	0.02 (1)	6.25	
O- N	0 / 753	-18.5	-18.5	0.29 (4)	10.00	
N- M	0 / 944	-18.5	-18.5	0.30 (4)	10.00	
L- M	0 / 17	0.0	0.0	0.05 (1)	10.00	
M- H	-157 / 0	0.0	0.0	0.03 (1)	7.81	
L- K	0 / 31	-18.5	-18.5	0.02 (4)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-391 / 0	0.16 (1)
N- F	0 / 346	0.08 (1)
N- G	-293 / 0	0.19 (1)
G- M	0 / 33	0.01 (4)
B- Q	0 / 858	0.19 (1)
M- K	-36 / 0	0.00 (1)
M- I	0 / 960	0.22 (1)
Q- O	0 / 837	0.13 (1)
C- O	-20 / 0	0.01 (1)
E- N	0 / 37	0.01 (4)
O- E	0 / 472	0.11 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.18/1.00 (B-C:1) , BC=0.30/1.00 (M-N:4) , WB=0.22/1.00 (I-M:1) , SSI=0.13/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (B) (INPUT = 0.90)
JSI METAL= 0.51 (B) (INPUT = 0.95)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425915	T77	4	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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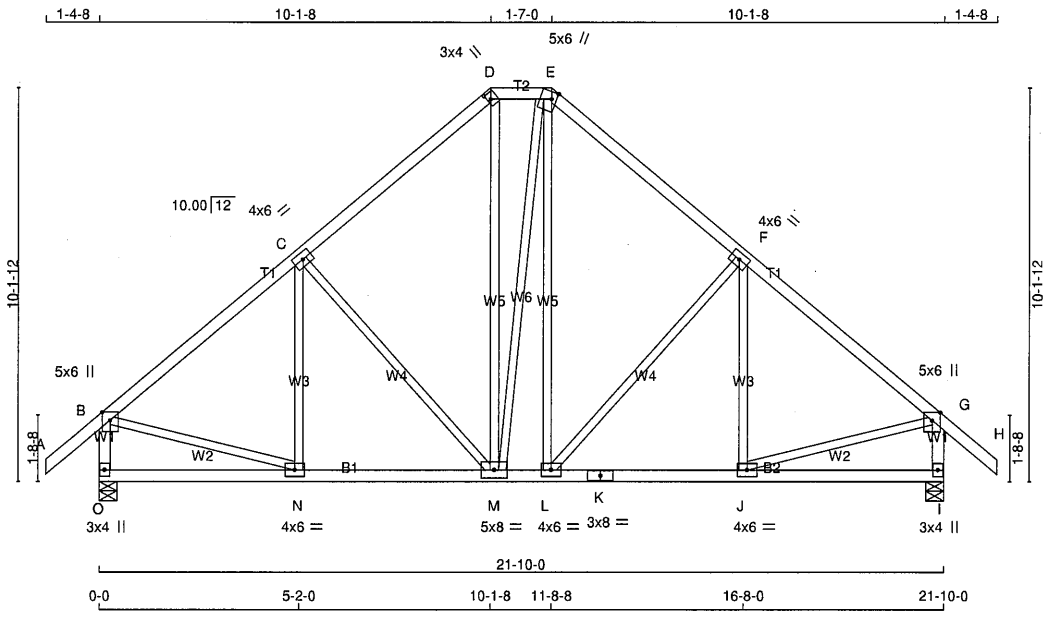

TOTAL WEIGHT = 4 X 116 = 466 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	6.0		
D	TTW+h	MT20	3.0	4.0	2.00	1.00
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMWW-t	MT20	4.0	6.0		
G	TMVW+p	MT20	5.0	6.0	Edge	
I	BMV1+p	MT20	3.0	4.0		
J, L, N						
J	BMWW-t	MT20	4.0	6.0		
K	BS-t	MT20	3.0	8.0		
M	BMWWW-t	MT20	5.0	8.0		
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1253	0	1253	0	0	5-8	1-8
I	1253	0	1253	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	887	577 / 0	0 / 0	0 / 0	0 / 0	310 / 0	0 / 0
I	887	577 / 0	0 / 0	0 / 0	0 / 0	310 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.76 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-84.9	-84.9	0.13 (1)	10.00	N- C	-118 / 41	0.07 (1)
B- C	-1058 / 0	-84.9	-84.9	0.30 (1)	5.76	C- M	-322 / 0	0.37 (1)
C- D	-848 / 0	-84.9	-84.9	0.29 (1)	6.25	M- D	0 / 298	0.07 (1)
D- E	-627 / 0	-84.9	-84.9	0.03 (1)	6.25	M- E	0 / 16	0.00 (1)
E- F	-845 / 0	-84.9	-84.9	0.29 (1)	6.25	L- E	0 / 281	0.06 (1)
F- G	-1059 / 0	-84.9	-84.9	0.30 (1)	5.76	L- F	-326 / 0	0.38 (1)
G- H	0 / 40	-84.9	-84.9	0.13 (1)	10.00	J- F	-112 / 44	0.06 (1)
O- B	-1213 / 0	0.0	0.0	0.13 (1)	7.27	B- N	0 / 867	0.20 (1)
I- G	-1214 / 0	0.0	0.0	0.13 (1)	7.26	J- G	0 / 868	0.20 (1)
O- N	0 / 0	-18.5	-18.5	0.12 (4)	10.00			
N- M	0 / 838	-18.5	-18.5	0.19 (1)	10.00			
M- L	0 / 624	-18.5	-18.5	0.15 (1)	10.00			
L- K	0 / 839	-18.5	-18.5	0.20 (1)	10.00			
K- J	0 / 839	-18.5	-18.5	0.20 (1)	10.00			
J- I	0 / 0	-18.5	-18.5	0.12 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.30/1.00 (F-G:1) , BC=0.20/1.00 (J-L:1) , WB=0.38/1.00 (F-L:1) , SSI=0.16/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (D) (INPUT = 0.90)
JSI METAL= 0.43 (G) (INPUT = 0.95)

Structural component only
DWG# T-2406497

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425915	T78	8	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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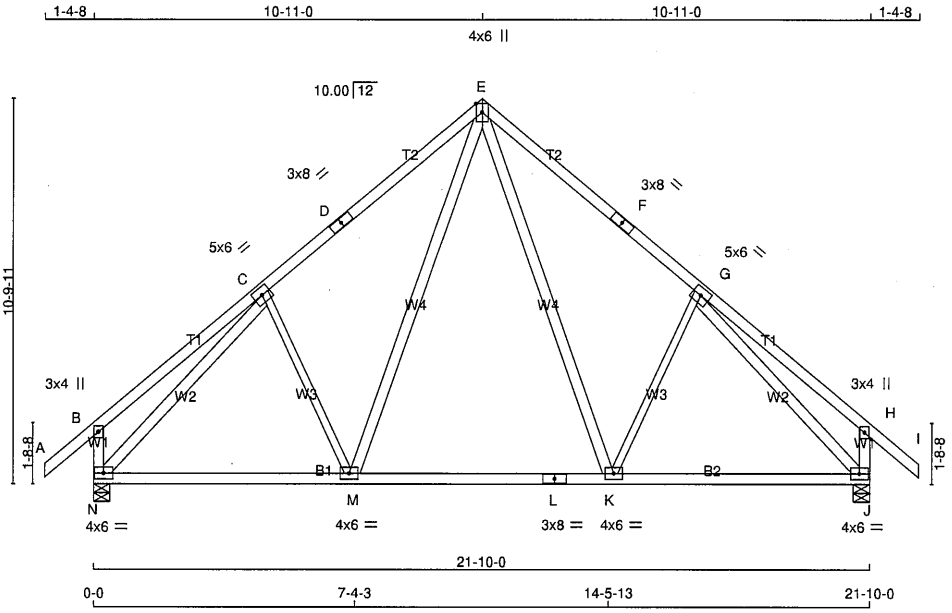

TOTAL WEIGHT = 8 X 114 = 911 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
K - G	2x3	DRY	No.2	SPF
C - M	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TS-t	MT20	3.0	8.0		
E	TTWW+p	MT20	4.0	6.0	Edge	
F	TS-t	MT20	3.0	8.0		
G	TMWW-t	MT20	5.0	6.0		
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	4.0	6.0		
K	BMWW-t	MT20	4.0	6.0		
L	BS-t	MT20	3.0	8.0		
M	BMWW-t	MT20	4.0	6.0		
N	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1253	0	1253	0	0	5-8	1-8
J	1253	0	1253	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	887	577 / 0	0 / 0	0 / 0	0 / 0	310 / 0	0 / 0
J	887	577 / 0	0 / 0	0 / 0	0 / 0	310 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-84.9	-84.9	0.13 (1)	10.00	E- K	0 / 409	0.07 (1)
B- C	0 / 45	-84.9	-84.9	0.45 (1)	10.00	K- G	-267 / 0	0.16 (1)
C- D	-969 / 0	-84.9	-84.9	0.42 (1)	5.77	M- E	0 / 409	0.07 (1)
D- E	-969 / 0	-84.9	-84.9	0.42 (1)	5.77	C- M	-267 / 0	0.16 (1)
E- F	-969 / 0	-84.9	-84.9	0.42 (1)	5.77	N- C	-1263 / 0	0.84 (1)
F- G	-969 / 0	-84.9	-84.9	0.42 (1)	5.77	G- J	-1263 / 0	0.84 (1)
G- H	0 / 45	-84.9	-84.9	0.45 (1)	10.00			
H- I	0 / 40	-84.9	-84.9	0.13 (1)	10.00			
N- B	-253 / 0	0.0	0.0	0.03 (1)	7.81			
J- H	-253 / 0	0.0	0.0	0.03 (1)	7.81			
N- M	0 / 838	-18.5	-18.5	0.30 (4)	10.00			
M- L	0 / 586	-18.5	-18.5	0.28 (4)	10.00			
L- K	0 / 586	-18.5	-18.5	0.28 (4)	10.00			
K- J	0 / 838	-18.5	-18.5	0.30 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.45/1.00 (G-H:1) , BC=0.30/1.00 (J-K:4) , WB=0.84/1.00 (C-N:1) , SSI=0.19/1.00 (E-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.64 (G) (INPUT = 0.90)
JSI METAL= 0.30 (C) (INPUT = 0.95)

Structural component only
DWG# T-2406498

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425915	T79	8	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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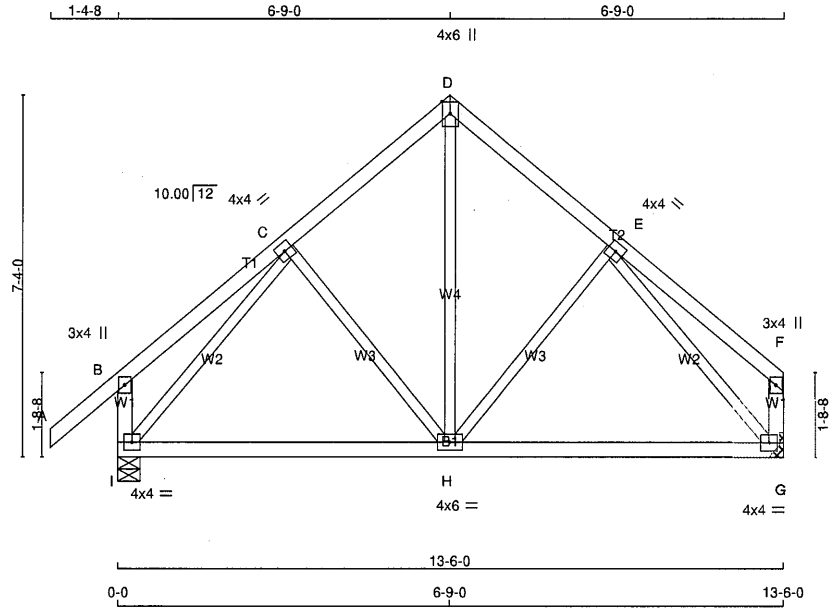

TOTAL WEIGHT = 8 X 63 = 502 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BMWWW-t	MT20	4.0	6.0		
I	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	822	0	822	0	0	5-8	1-8
G	698	0	698	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	581	383 / 0	0 / 0	0 / 0	0 / 0	199 / 0	0 / 0
G	495	314 / 0	0 / 0	0 / 0	0 / 0	181 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-84.9	-84.9	0.13 (1)	10.00	H- D	0 / 360	0.08 (1)
B- C	0 / 21	-84.9	-84.9	0.15 (1)	10.00	H- E	-133 / 0	0.06 (1)
C- D	-477 / 0	-84.9	-84.9	0.12 (1)	6.25	C- H	-133 / 0	0.06 (1)
D- E	-477 / 0	-84.9	-84.9	0.12 (1)	6.25	I- C	-694 / 0	0.32 (1)
E- F	0 / 21	-84.9	-84.9	0.15 (1)	10.00	E- G	-694 / 0	0.32 (1)
I- B	-234 / 0	0.0	0.0	0.03 (1)	7.81			
G- F	-109 / 0	0.0	0.0	0.01 (1)	7.81			
I- H	0 / 435	-18.5	-18.5	0.28 (4)	10.00			
H- G	0 / 435	-18.5	-18.5	0.28 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.45")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.45")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.15/1.00 (B-C:1) , BC=0.28/1.00 (H-I:4) , WB=0.32/1.00 (C-I:1) , SSI=0.11/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (C) (INPUT = 0.90)
JSI METAL= 0.23 (E) (INPUT = 0.95)

Structural component only
DWG# T-2406499

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425915	T79S	4	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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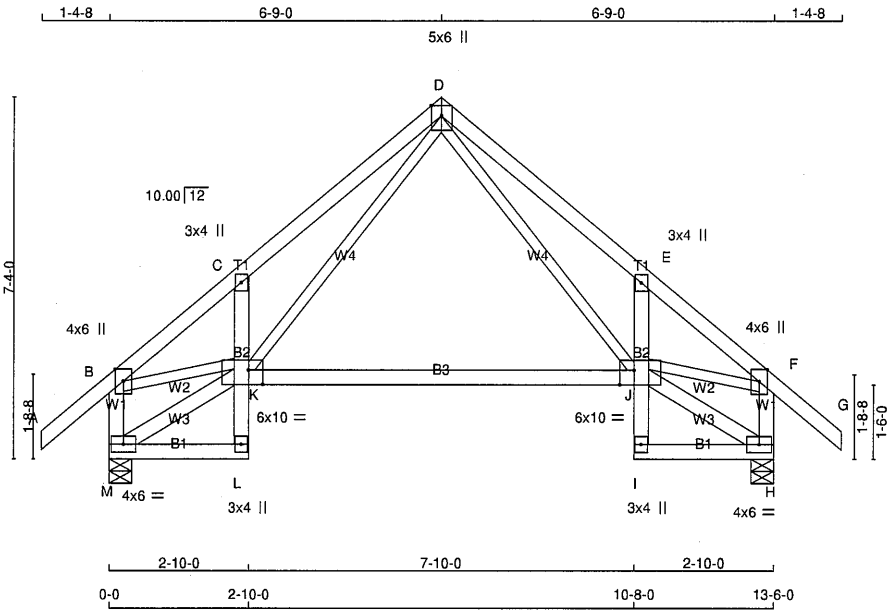

TOTAL WEIGHT = 4 X 71 = 285 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
L - C	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
I - E	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMV+p	MT20	3.0	4.0		
D	TTWW+p	MT20	5.0	6.0	Edge	
E	TMV+p	MT20	3.0	4.0		
F	TMVW+p	MT20	4.0	6.0	Edge	
H	BMVW1-t	MT20	4.0	6.0		
I	BMV+p	MT20	3.0	4.0		
J	BVMWWW-I	MT20	6.0	10.0	Edge	3.50
K	BVMWWW-I	MT20	6.0	10.0	Edge	3.50
L	BMV+p	MT20	3.0	4.0		
M	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2406500

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	822	0	822	0	0	5-8	1-8
H	822	0	822	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	581	383 / 0	0 / 0	0 / 0	0 / 0	199 / 0	0 / 0
H	581	383 / 0	0 / 0	0 / 0	0 / 0	199 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.04 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-84.9	-84.9	0.13 (1)	10.00	D- J	0 / 597	0.13 (1)
B- C	-963 / 0	-84.9	-84.9	0.13 (1)	6.20	K- D	0 / 597	0.13 (1)
C- D	-1000 / 0	-84.9	-84.9	0.19 (1)	6.04	M- K	-16 / 0	0.00 (1)
D- E	-1000 / 0	-84.9	-84.9	0.19 (1)	6.04	B- K	0 / 762	0.17 (1)
E- F	-963 / 0	-84.9	-84.9	0.13 (1)	6.20	J- H	-16 / 0	0.00 (1)
F- G	0 / 40	-84.9	-84.9	0.13 (1)	10.00	J- F	0 / 762	0.17 (1)
M- B	-790 / 0	0.0	0.0	0.08 (1)	7.81			
H- F	-790 / 0	0.0	0.0	0.08 (1)	7.81			
M- L	0 / 14	-18.5	-18.5	0.04 (4)	10.00			
L- K	0 / 28	0.0	0.0	0.03 (1)	10.00			
K- C	-351 / 0	0.0	0.0	0.02 (1)	7.81			
K- J	0 / 401	-18.5	-18.5	0.39 (4)	10.00			
I- J	0 / 28	0.0	0.0	0.03 (1)	10.00			
J- E	-351 / 0	0.0	0.0	0.02 (1)	7.81			
I- H	0 / 14	-18.5	-18.5	0.04 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.45")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.45")
CALCULATED VERT. DEFL.(TL) = L/ 687 (0.24")

CSI: TC=0.19/1.00 (C-D:1) , BC=0.39/1.00 (J-K:4) , WB=0.17/1.00 (B-K:1) , SSI=0.12/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.61 (F) (INPUT = 0.90)
JSI METAL= 0.46 (B) (INPUT = 0.95)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425915	T80	2	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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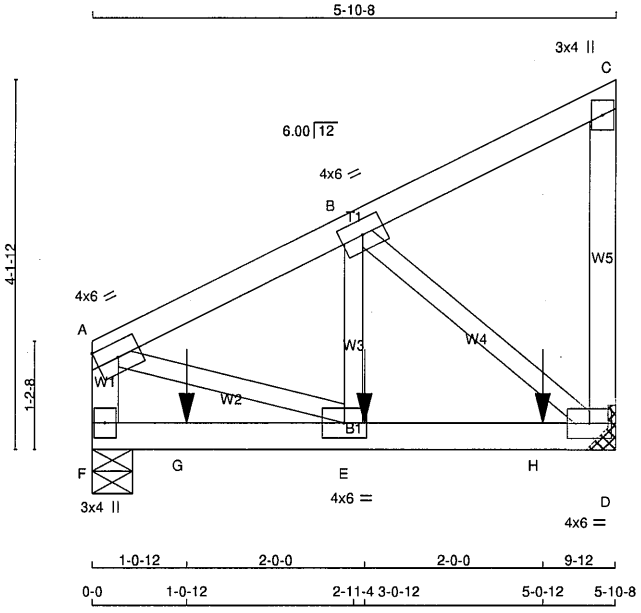

TOTAL WEIGHT = 4 X 26 = 103 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS		SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- D	1	12	TOP
F- A	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	1	12	SIDE(61.0)
WEBS : (0.122"X3") SPIRAL NAILS			
B- E	1	6	SIDE(116.2)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0		Edge

Structural component only
DWG# T-2406501

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
D	1353	0	1353	0	0	MECHANICAL	
F	1294	0	1294	0	0	5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	958	622 / 0	0 / 0	0 / 0	0 / 0	336 / 0	0 / 0
F	916	595 / 0	0 / 0	0 / 0	0 / 0	321 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-1129 / 0	-84.9	-84.9	0.05 (1)	6.25	E- B	0 / 1036	0.13 (1)
B- C	-10 / 0	-84.9	-84.9	0.05 (1)	6.25	B- D	-1309 / 0	0.16 (1)
D- C	-102 / 0	0.0	0.0	0.01 (1)	7.81	A- E	0 / 1065	0.13 (1)
F- A	-923 / 0	0.0	0.0	0.05 (1)	7.81			
F- G	0 / 0	-18.5	-18.5	0.29 (1)	10.00			
G- E	0 / 0	-18.5	-18.5	0.29 (1)	10.00			
E- H	0 / 1019	-18.5	-18.5	0.35 (1)	10.00			
H- D	0 / 1019	-18.5	-18.5	0.35 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	3-0-12	-480	-480	---	FRONT	VERT	TOTAL	---	C1
G	1-0-12	-480	-480	---	FRONT	VERT	TOTAL	---	C1
H	5-0-12	-482	-482	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.05/1.00 (A-B:1) , BC=0.35/1.00 (D-E:1) , WB=0.16/1.00 (B-D:1) , SSI=0.18/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.63 (E) (INPUT = 0.90)
JSI METAL= 0.16 (E) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
425915	T80	2	2	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	4.0	6.0		
C	TMV+p	MT20	3.0	4.0		
D	BMVW1-t	MT20	4.0	6.0		
E	BMWW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2406501

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425915	T81	7	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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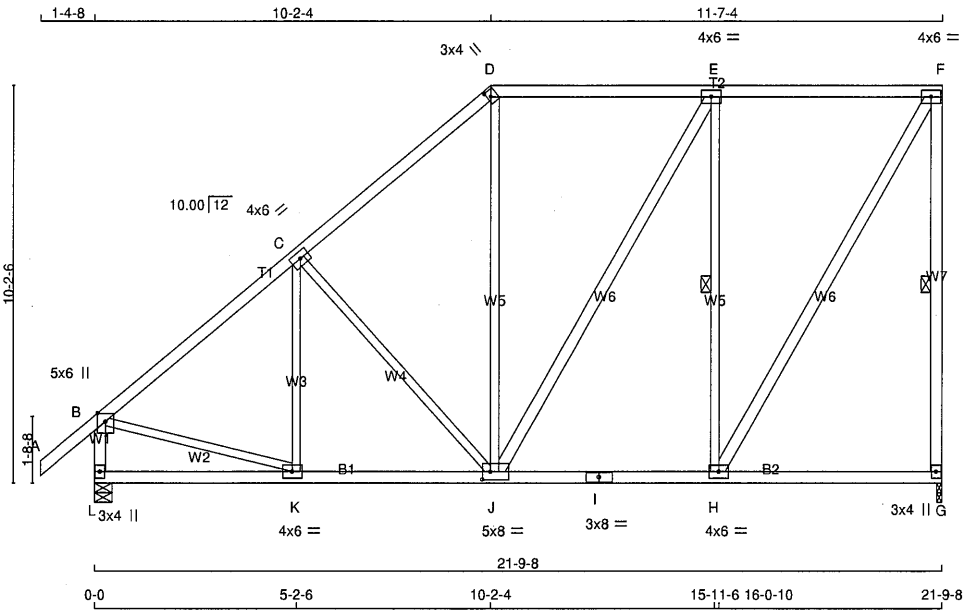

Scale = 1:56.5

TOTAL WEIGHT = 7 X 123 = 863 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	6.0		
D	TTW+h	MT20	3.0	4.0	2.00	1.00
E	TMWW-t	MT20	4.0	6.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	8.0		
J	BMWWW-t	MT20	5.0	8.0	2.50	2.50
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2406502

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
G	1126	0	1126	0	0	1-8	1-8
L	1251	0	1251	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	799	507 / 0	0 / 0	0 / 0	0 / 0	292 / 0	0 / 0
L	885	576 / 0	0 / 0	0 / 0	0 / 0	310 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.57 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-84.9	-84.9	0.13 (1)	10.00	K- C	-115 / 40	0.07 (1)
B- C	-1055 / 0	-84.9	-84.9	0.42 (1)	5.57	C- J	-327 / 0	0.39 (1)
C- D	-841 / 0	-84.9	-84.9	0.40 (1)	6.07	J- D	0 / 174	0.04 (4)
D- E	-620 / 0	-84.9	-84.9	0.50 (1)	6.25	B- K	0 / 865	0.19 (1)
E- F	-528 / 0	-84.9	-84.9	0.49 (1)	6.25	H- F	0 / 1039	0.17 (1)
G- F	-1083 / 0	0.0	0.0	0.55 (1)	6.07	J- E	0 / 187	0.03 (1)
L- B	-1211 / 0	0.0	0.0	0.13 (1)	7.28	H- E	-772 / 0	0.55 (1)
L- K	0 / 0	-18.5	-18.5	0.11 (4)	10.00			
K- J	0 / 836	-18.5	-18.5	0.19 (1)	10.00			
J- I	0 / 528	-18.5	-18.5	0.20 (4)	10.00			
I- H	0 / 528	-18.5	-18.5	0.20 (4)	10.00			
H- G	0 / 0	-18.5	-18.5	0.15 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.55/1.00 (F-G:1) , BC=0.20/1.00 (H-J:4) , WB=0.55/1.00 (E-H:1) , SSI=0.24/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (D) (INPUT = 0.90)
JSI METAL= 0.42 (B) (INPUT = 0.95)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425915	T82	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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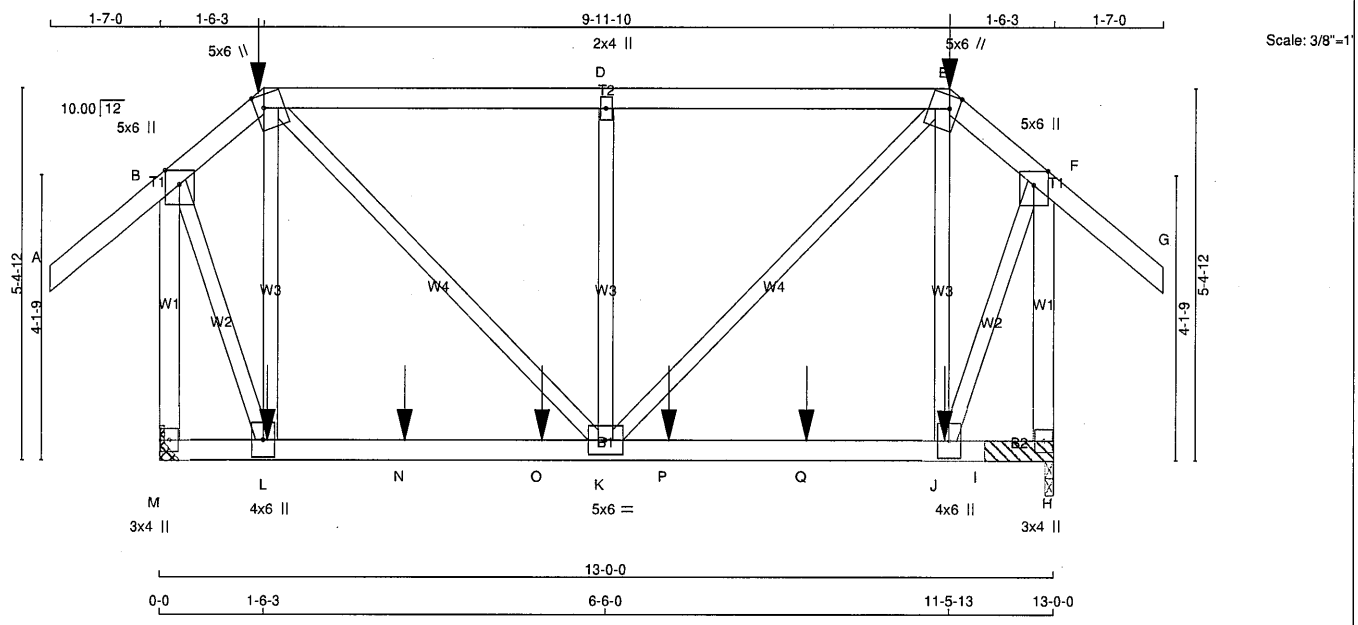

TOTAL WEIGHT = 75 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMVW+p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
J	BMWW+t	MT20	4.0	6.0		
K	BMWWW-t	MT20	5.0	6.0		
L	BMWW+t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2406503

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
M	1442	0	1442	0	0	MECHANICAL	
H	1442	0	1442	0	0	1-8	1-8 & BLOCK

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT M. MINIMUM BEARING LENGTH AT JOINT M = 1-9.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1014	697 / 0	0 / 0	0 / 0	0 / 0	318 / 0	0 / 0
H	1014	697 / 0	0 / 0	0 / 0	0 / 0	318 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

2x4 DRY SPF No.2 BEARING BLOCK 12" LONG AT JT. H ATTACHED TO FRONT SIDE WITH 2 ROWS OF (0.122"X3") SPIRAL NAILS SPACED 2" C.C. 12 NAILS TOTAL.

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.85 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 46	-84.9	-84.9	0.19 (1)	10.00	L- C	-457 / 0	0.21 (1)
B- C	-552 / 0	-84.9	-84.9	0.18 (1)	6.25	C- K	0 / 767	0.19 (1)
C- D	-910 / 0	-84.9	-84.9	0.40 (1)	5.85	K- D	-514 / 0	0.23 (1)
D- E	-910 / 0	-84.9	-84.9	0.40 (1)	5.85	K- E	0 / 767	0.19 (1)
E- F	-552 / 0	-84.9	-84.9	0.18 (1)	6.25	J- E	-457 / 0	0.21 (1)
F- G	0 / 46	-84.9	-84.9	0.19 (1)	10.00	B- L	0 / 974	0.24 (1)
M- B	-1505 / 0	0.0	0.0	0.45 (1)	6.65	J- F	0 / 974	0.24 (1)
H- F	-1505 / 0	0.0	0.0	0.45 (1)	6.65			
M- L	0 / 0	-18.5	-18.5	0.19 (1)	10.00			
L- N	0 / 373	-18.5	-18.5	0.44 (1)	10.00			
N- O	0 / 373	-18.5	-18.5	0.44 (1)	10.00			
O- K	0 / 373	-18.5	-18.5	0.44 (1)	10.00			
K- P	0 / 373	-18.5	-18.5	0.44 (1)	10.00			
P- Q	0 / 373	-18.5	-18.5	0.44 (1)	10.00			
Q- J	0 / 373	-18.5	-18.5	0.44 (1)	10.00			
J- I	0 / 0	-18.5	-18.5	0.19 (1)	10.00			
I- H	0 / 0	-18.5	-18.5	0.19 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	1-6-3	-12	-12	---	FRONT	VERT	DEAD	---	C1
C	1-6-3	-45	-45	---	FRONT	VERT	SNOW	---	C1
E	11-5-13	-12	-12	---	FRONT	VERT	DEAD	---	C1
E	11-5-13	-45	-45	---	FRONT	VERT	SNOW	---	C1
J	11-5-1	-127	-127	---	FRONT	VERT	TOTAL	---	C1
L	1-6-15	-127	-127	---	FRONT	VERT	TOTAL	---	C1
N	3-6-15	-127	-127	---	FRONT	VERT	TOTAL	---	C1
O	5-6-15	-127	-127	---	FRONT	VERT	TOTAL	---	C1
P	7-5-1	-127	-127	---	FRONT	VERT	TOTAL	---	C1
Q	9-5-1	-127	-127	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.45/1.00 (F-H:1) , BC=0.44/1.00 (J-K:1) , WB=0.24/1.00 (F-J:1) , SSI=0.25/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
425915	T82	1	1	TRUSS DESC.	

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

JSI GRIP= 0.70 (K) (INPUT = 0.90)
JSI METAL= 0.25 (B) (INPUT = 0.95)

Structural component only
DWG# T-2406503

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425915	T83	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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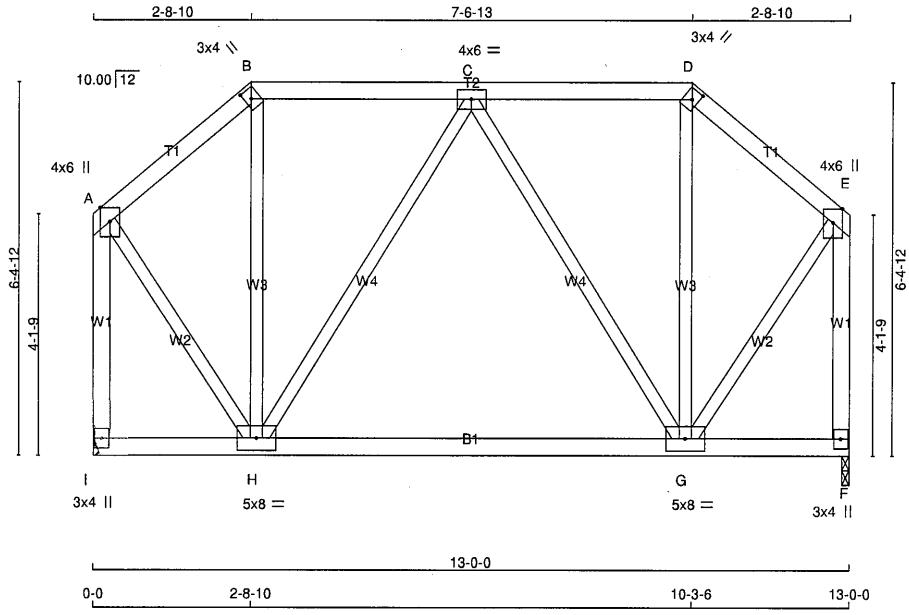

TOTAL WEIGHT = 68 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
I - A	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B	TTW+h	MT20	3.0	4.0	2.00	1.25
C	TMWW-t	MT20	4.0	6.0		
D	TTW+h	MT20	3.0	4.0	2.00	1.25
E	TMVW+p	MT20	4.0	6.0	Edge	
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	8.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	672	0	672	0	0	MECHANICAL	
F	672	0	672	0	0	1-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	477	303 / 0	0 / 0	0 / 0	0 / 0	174 / 0	0 / 0
F	477	303 / 0	0 / 0	0 / 0	0 / 0	174 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-331 / 0	-84.9	-84.9	0.11 (1)	6.25	H- B	-25 / 37	0.02 (1)
B- C	-253 / 0	-84.9	-84.9	0.20 (1)	6.25	G- D	-25 / 37	0.02 (1)
C- D	-253 / 0	-84.9	-84.9	0.20 (1)	6.25	A- H	0 / 425	0.10 (1)
D- E	-331 / 0	-84.9	-84.9	0.11 (1)	6.25	G- E	0 / 425	0.10 (1)
I- A	-669 / 0	0.0	0.0	0.19 (1)	7.81	H- C	-231 / 0	0.23 (1)
F- E	-669 / 0	0.0	0.0	0.19 (1)	7.81	C- G	-231 / 0	0.23 (1)
I- H	0 / 0	-18.5	-18.5	0.16 (4)	10.00			
H- G	0 / 372	-18.5	-18.5	0.19 (4)	10.00			
G- F	0 / 0	-18.5	-18.5	0.16 (4)	10.00			

DESIGN CRITERIA [M]

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.20/1.00 (C-D:1) , BC=0.19/1.00 (G-H:4) , WB=0.23/1.00 (C-H:1) , SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.42 (E) (INPUT = 0.90)
JSI METAL= 0.17 (A) (INPUT = 0.95)

Structural component only
DWG# T-2406504

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425915	T84	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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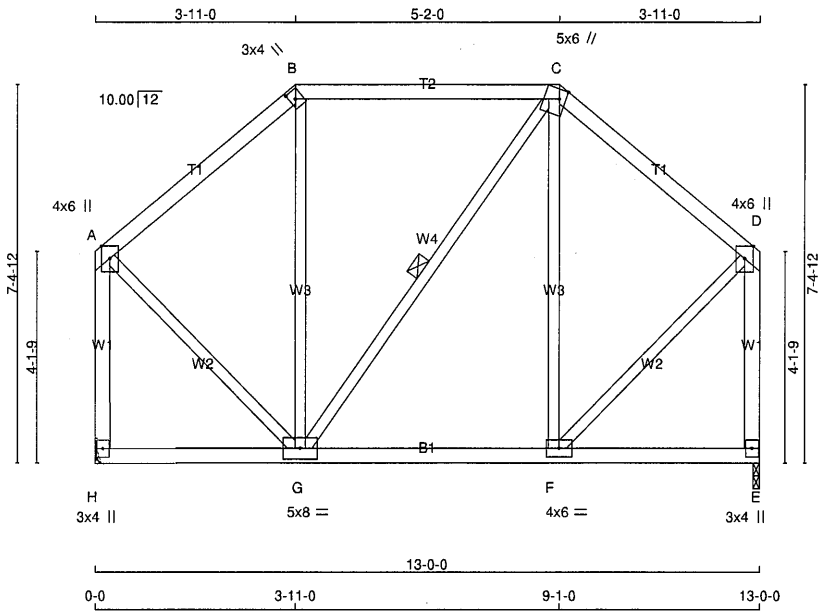

Scale = 1:43.0

TOTAL WEIGHT = 68 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B	TTW+h	MT20	3.0	4.0	2.00	1.25
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMVW+p	MT20	4.0	6.0	Edge	
E	BMV1+p	MT20	3.0	4.0		
F	BMWW-t	MT20	4.0	6.0		
G	BMWWW-t	MT20	5.0	8.0		
H	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2406505

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	672	0	672	0	0	MECHANICAL	
E	672	0	672	0	0	1-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	477	303 / 0	0 / 0	0 / 0	0 / 0	174 / 0	0 / 0
E	477	303 / 0	0 / 0	0 / 0	0 / 0	174 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-349 / 0	-84.9	-84.9	0.17 (1)	6.25	G- B	-161 / 8	0.16 (1)
B- C	-265 / 0	-84.9	-84.9	0.29 (1)	6.25	G- C	0 / 0	0.00 (1)
C- D	-349 / 0	-84.9	-84.9	0.17 (1)	6.25	F- C	-160 / 8	0.16 (1)
H- A	-643 / 0	0.0	0.0	0.18 (1)	7.81	A- G	0 / 368	0.08 (1)
E- D	-644 / 0	0.0	0.0	0.19 (1)	7.81	F- D	0 / 368	0.08 (1)
H- G	0 / 0	-18.5	-18.5	0.09 (4)	10.00			
G- F	0 / 265	-18.5	-18.5	0.11 (4)	10.00			
F- E	0 / 0	-18.5	-18.5	0.09 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.29/1.00 (B-C:1) , BC=0.11/1.00 (F-G:4) , WB=0.16/1.00 (B-G:1) , SSI=0.17/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.45 (B) (INPUT = 0.90)
JSI METAL= 0.19 (D) (INPUT = 0.95)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425915	T85	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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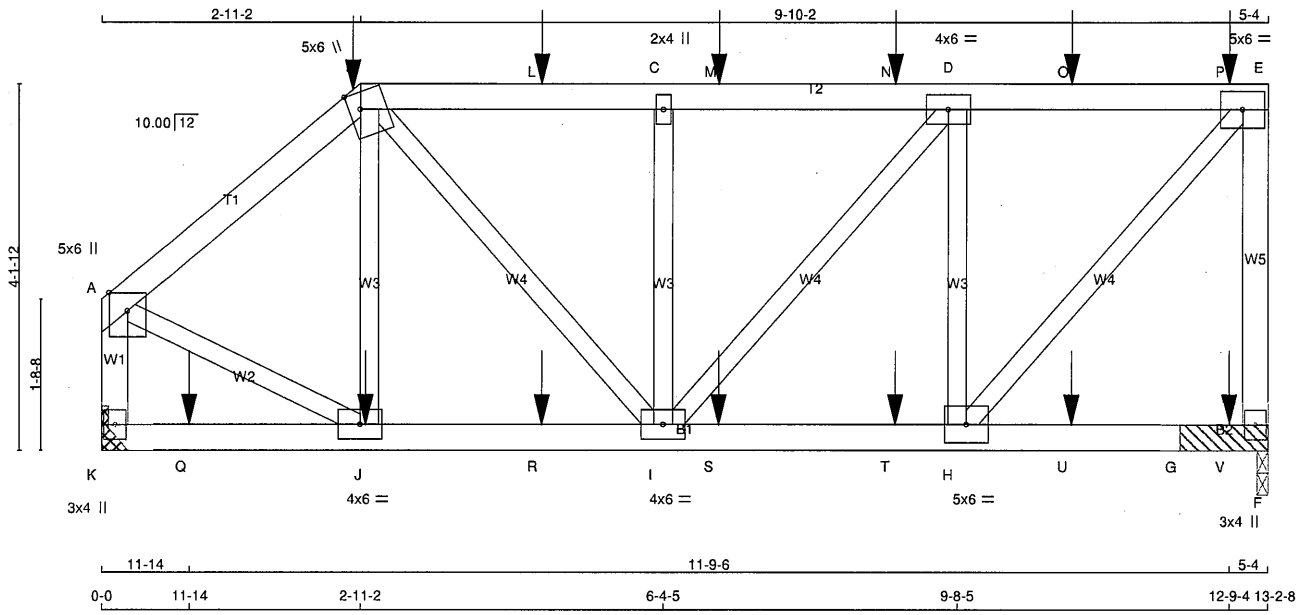

TOTAL WEIGHT = 61 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
K - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	Edge	
B	TTWW+m	MT20	5.0	6.0	2.25	1.50
C	TMW+w	MT20	2.0	4.0		
D	TMWW-t	MT20	4.0	6.0		
E	TMVW-t	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	5.0	6.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2406506

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	1485	0	1485	0	0	1-8	1-8 & BLOCK
K	1367	0	1367	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K. MINIMUM BEARING LENGTH AT JOINT K = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1054	672 / 0	0 / 0	0 / 0	0 / 0	382 / 0	0 / 0
K	971	615 / 0	0 / 0	0 / 0	0 / 0	356 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

2x4 DRY SPF No.2 BEARING BLOCK 12" LONG AT JT. F ATTACHED TO FRONT SIDE WITH 2 ROWS OF (0.122"X3") SPIRAL NAILS SPACED 2" C.C. 12 NAILS TOTAL.

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.24 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI	MAX (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-1213 / 0	-84.9	-84.9	0.16	(1)	5.61	J- B	-283 / 0	0.08 (1)
B- L	-1265 / 0	-84.9	-84.9	0.28	(1)	5.34	A- J	0 / 1019	0.25 (1)
L- C	-1265 / 0	-84.9	-84.9	0.28	(1)	5.34	H- E	0 / 1488	0.37 (1)
C- M	-1265 / 0	-84.9	-84.9	0.34	(1)	5.24	B- I	0 / 516	0.13 (1)
M- N	-1265 / 0	-84.9	-84.9	0.34	(1)	5.24	H- D	-953 / 0	0.26 (1)
N- D	-1265 / 0	-84.9	-84.9	0.34	(1)	5.24	I- C	-583 / 0	0.16 (1)
D- O	-1003 / 0	-84.9	-84.9	0.33	(1)	5.75	I- D	0 / 401	0.10 (1)
O- P	-1003 / 0	-84.9	-84.9	0.33	(1)	5.75			
P- E	-1003 / 0	-84.9	-84.9	0.33	(1)	5.75			
F- E	-1415 / 0	0.0	0.0	0.39	(1)	6.82			
K- A	-1325 / 0	0.0	0.0	0.16	(1)	6.99			
K- Q	0 / 0	-18.5	-18.5	0.08	(4)	10.00			
Q- J	0 / 0	-18.5	-18.5	0.08	(4)	10.00			
J- R	0 / 921	-18.5	-18.5	0.21	(1)	10.00			
R- I	0 / 921	-18.5	-18.5	0.21	(1)	10.00			
I- S	0 / 1003	-18.5	-18.5	0.24	(1)	10.00			
S- T	0 / 1003	-18.5	-18.5	0.24	(1)	10.00			
T- H	0 / 1003	-18.5	-18.5	0.24	(1)	10.00			
H- U	0 / 0	-18.5	-18.5	0.12	(4)	10.00			
U- G	0 / 0	-18.5	-18.5	0.12	(4)	10.00			
G- V	0 / 0	-18.5	-18.5	0.12	(4)	10.00			
V- F	0 / 0	-18.5	-18.5	0.12	(4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
B	2-11-2	-30	-30	---	FRONT	VERT	DEAD	---	C1
B	2-11-2	-119	-119	---	BACK	VERT	TOTAL	---	C1
B	2-11-2	-117	-117	---	FRONT	VERT	SNOW	---	C1
J	2-11-14	-29	-29	---	BACK	VERT	TOTAL	---	C1
L	4-11-14	-113	-113	---	BACK	VERT	TOTAL	---	C1
M	6-11-14	-113	-113	---	BACK	VERT	TOTAL	---	C1
N	8-11-14	-113	-113	---	BACK	VERT	TOTAL	---	C1
O	10-11-14	-113	-113	---	BACK	VERT	TOTAL	---	C1
P	12-9-4	-130	-130	---	BACK	VERT	TOTAL	---	C1

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.44")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.44")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.39/1.00 (E-F:1) , BC=0.24/1.00 (H-I:1) , WB=0.37/1.00 (E-H:1) , SSI=0.29/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (E) (INPUT = 0.90)
JSI METAL= 0.43 (A) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
425915	T85	1	1	TRUSS DESC.	

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
Q	11-14	-29	-29	---	BACK	VERT	TOTAL	---	C1
R	4-11-14	-29	-29	---	BACK	VERT	TOTAL	---	C1
S	6-11-14	-29	-29	---	BACK	VERT	TOTAL	---	C1
T	8-11-14	-29	-29	---	BACK	VERT	TOTAL	---	C1
U	10-11-14	-29	-29	---	BACK	VERT	TOTAL	---	C1
V	12-9-4	-33	-33	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

Structural component only
DWG# T-2406506

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425915	T86	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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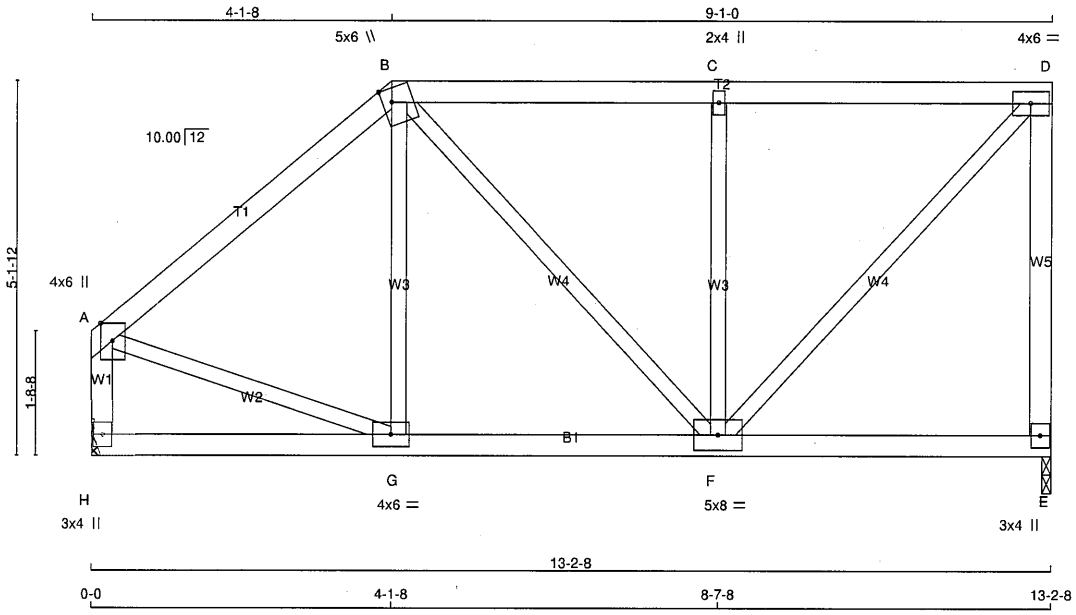

TOTAL WEIGHT = 59 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B	TTWW+m	MT20	5.0	6.0	2.25	1.50
C	TMW+w	MT20	2.0	4.0		
D	TMVW-t	MT20	4.0	6.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	5.0	8.0		
G	BMWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
E	683	0	683	0	0	1-8	1-8
H	683	0	683	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	485	308 / 0	0 / 0	0 / 0	0 / 0	177 / 0	0 / 0
H	485	308 / 0	0 / 0	0 / 0	0 / 0	177 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-534 / 0	-84.9	-84.9	0.26 (1)	6.25	G- B	-45 / 47	0.02 (1)
B- C	-470 / 0	-84.9	-84.9	0.29 (1)	6.25	A- G	0 / 430	0.10 (1)
C- D	-470 / 0	-84.9	-84.9	0.29 (1)	6.25	F- D	0 / 685	0.15 (1)
E- D	-647 / 0	0.0	0.0	0.29 (1)	7.81	B- F	0 / 91	0.02 (1)
H- A	-651 / 0	0.0	0.0	0.07 (1)	7.81	F- C	-475 / 0	0.19 (1)
H- G	0 / 0	-18.5	-18.5	0.08 (4)	10.00			
G- F	0 / 408	-18.5	-18.5	0.12 (4)	10.00			
F- E	0 / 0	-18.5	-18.5	0.08 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.44")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.44")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.29/1.00 (C-D:1) , BC=0.12/1.00 (F-G:4) , WB=0.19/1.00 (C-F:1) , SSI=0.19/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.48 (D) (INPUT = 0.90)
JSI METAL= 0.27 (A) (INPUT = 0.95)

Structural component only
DWG# T-2406507

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425915	T87	1	2	ROYAL PINE HOMES TRUSS DESC.	

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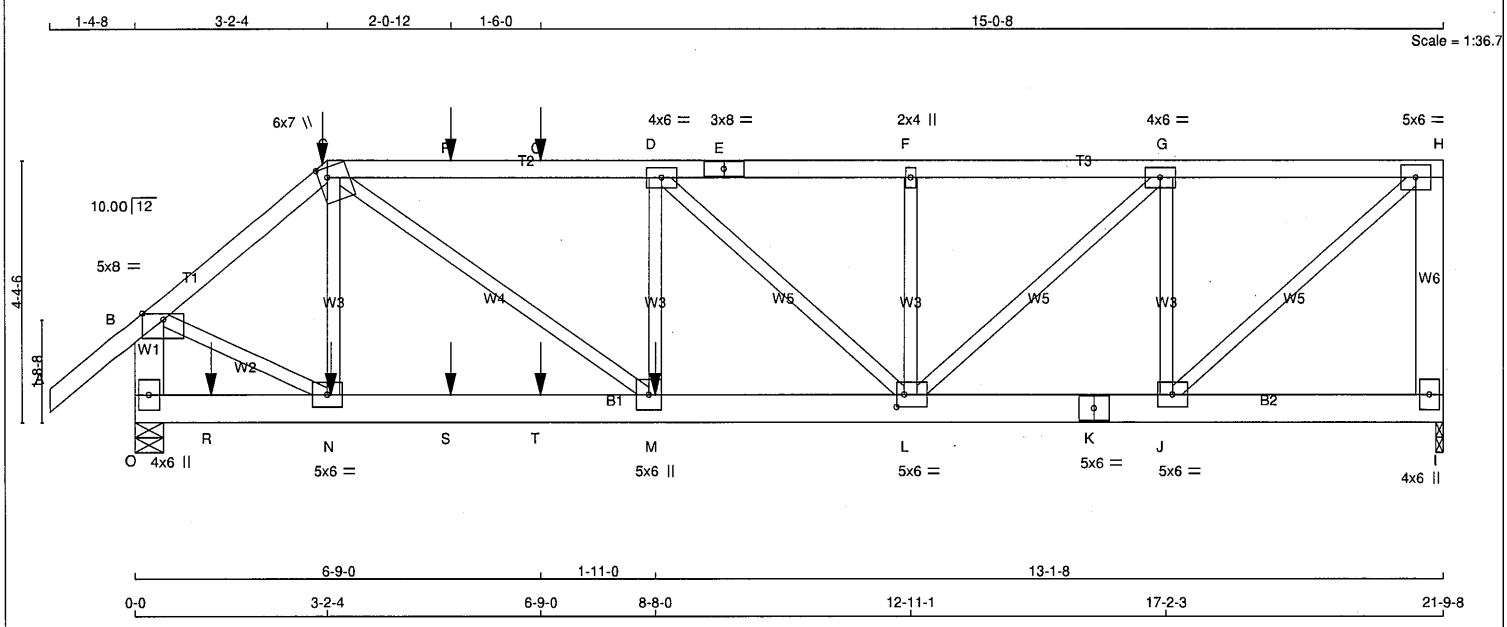

TOTAL WEIGHT = 2 X 109 = 217 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
I - H	2x6	DRY	No.2	SPF
O - B	2x6	DRY	No.2	SPF
O - K	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- E	1	12	SIDE(61.0)
E- H	1	12	TOP
H- I	2	12	TOP
O- B	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
O- K	2	12	SIDE(183.1)
K- I	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
D- M	1	6	SIDE(283.6)

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

Structural component only
DWG# T-2406508

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	2119	0	2119	0	0	1-8	1-8
O	3574	0	3574	0	0	5-8	1-15

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1500	976 / 0	0 / 0	0 / 0	0 / 0	524 / 0	0 / 0
O	2525	1673 / 0	0 / 0	0 / 0	0 / 0	851 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.08 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	CSI	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-84.9	-84.9	0.07 (1)	10.00	N- C	-154 / 47	0.02 (1)	
B- C	-3426 / 0	-84.9	-84.9	0.13 (1)	4.95	B- N	0 / 2817	0.35 (1)	
C- P	-4814 / 0	-84.9	-84.9	0.37 (1)	4.08	M- D	0 / 524	0.06 (1)	
P- Q	-4814 / 0	-84.9	-84.9	0.37 (1)	4.08	J- H	0 / 2936	0.36 (1)	
Q- D	-4814 / 0	-84.9	-84.9	0.37 (1)	4.08	D- L	-1546 / 0	0.45 (1)	
D- E	-3687 / 0	-84.9	-84.9	0.26 (1)	4.66	J- G	-1815 / 0	0.25 (1)	
E- F	-3687 / 0	-84.9	-84.9	0.26 (1)	4.66	L- F	-289 / 0	0.04 (1)	
F- G	-3687 / 0	-84.9	-84.9	0.19 (1)	4.74	L- G	0 / 2009	0.25 (1)	
G- H	-2221 / 0	-84.9	-84.9	0.16 (1)	5.81	C- M	0 / 2711 '	0.34 (1)	
I- H	-2079 / 0	0.0	0.0	0.19 (1)	7.81				
O- B	-3491 / 0	0.0	0.0	0.13 (1)	7.48				
O- R	0 / 0	-18.5	-18.5	0.13 (1)	10.00				
R- N	0 / 0	-18.5	-18.5	0.13 (1)	10.00				
N- S	0 / 2619	-18.5	-18.5	0.32 (1)	10.00				
S- T	0 / 2619	-18.5	-18.5	0.32 (1)	10.00				
T- M	0 / 2619	-18.5	-18.5	0.32 (1)	10.00				
M- L	0 / 4814	-18.5	-18.5	0.34 (1)	10.00				
L- K	0 / 2221	-18.5	-18.5	0.18 (1)	10.00				
K- J	0 / 2221	-18.5	-18.5	0.18 (1)	10.00				
J- I	0 / 0	-18.5	-18.5	0.02 (4)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-2-4	-47	-47	---	FRONT	VERT	DEAD	---	C1
C	3-2-4	-38	-38	---	FRONT	VERT	TOTAL	---	C1
C	3-2-4	-180	-180	---	FRONT	VERT	SNOW	---	C1
M	8-8-0	-1262	-1262	---	FRONT	VERT	TOTAL	---	C1
N	3-3-0	-187	-187	---	FRONT	VERT	TOTAL	---	C1
P	5-3-0	-32	-32	---	FRONT	VERT	TOTAL	---	C1
Q	6-9-0	-32	-32	---	FRONT	VERT	TOTAL	---	C1
R	1-3-0	-187	-187	---	FRONT	VERT	TOTAL	---	C1
S	5-3-0	-187	-187	---	FRONT	VERT	TOTAL	---	C1
T	6-9-0	-187	-187	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.37/1.00 (C-D:1) , BC=0.34/1.00 (L-M:1) , WB=0.45/1.00 (D-L:1) , SSI=0.14/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (L) (INPUT = 0.90)
JSI METAL= 0.40 (M) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
425915	T87	1	2	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	6.0	7.0	2.00	1.75
D	TMWW-t	MT20	4.0	6.0		
E	TS-t	MT20	3.0	8.0		
F	TMW+w	MT20	2.0	4.0		
G	TMWW-t	MT20	4.0	6.0		
H	TMVW-t	MT20	5.0	6.0		
I	BMV1+p	MT20	4.0	6.0		
J	BMWW-t	MT20	5.0	6.0		
K	BS-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	5.0	6.0	2.50	1.50
M	BMWW+t	MT20	5.0	6.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2406508

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425915	T87Z	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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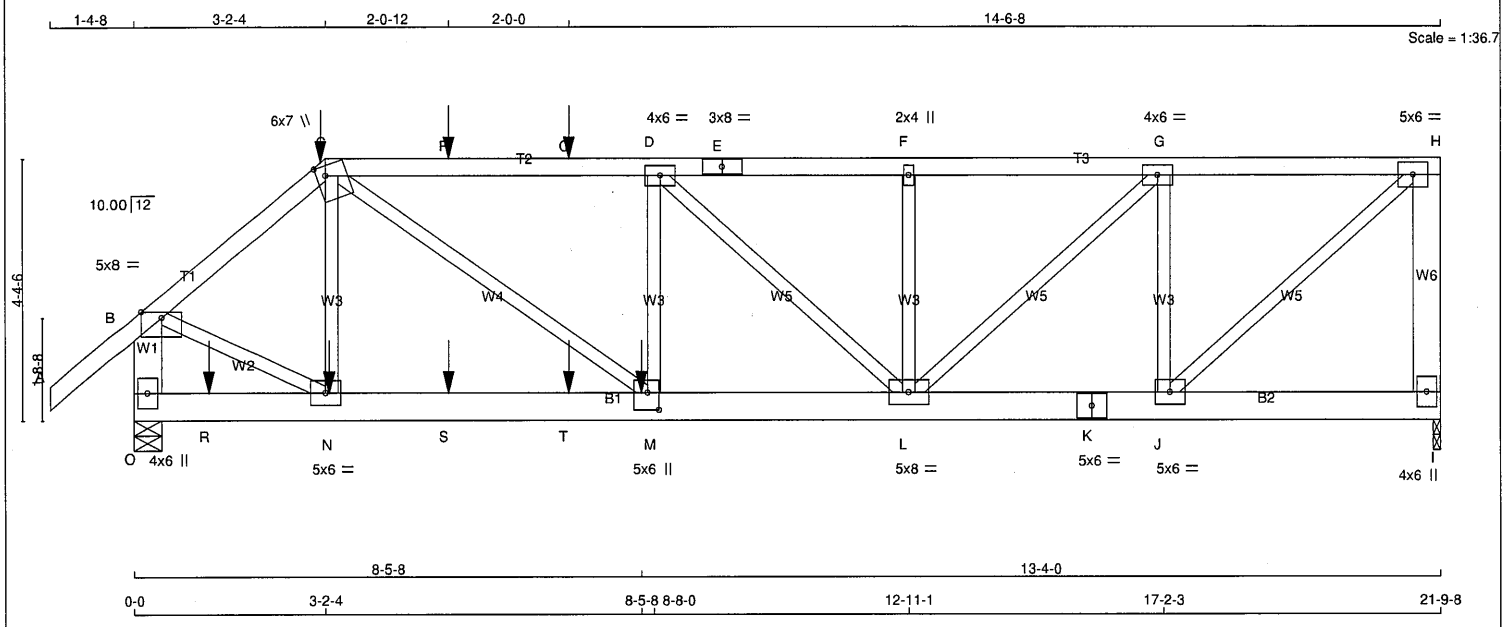

TOTAL WEIGHT = 2 X 109 = 217 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
I - H	2x6	DRY	No.2	SPF
O - B	2x6	DRY	No.2	SPF
O - K	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- E	1	12	SIDE(61.0)
E- H	1	12	TOP
H- I	2	12	TOP
O- B	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
O- K	2	12	SIDE(183.1)
K- I	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
D- M	1	6	SIDE(309.0)

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

Structural component only
DWG# T-2406509

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	2165	0	2165	0	0	1-8	1-8
O	3626	0	3626	0	0	5-8	1-15

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1534	989 / 0	0 / 0	0 / 0	0 / 0	545 / 0	0 / 0
O	2564	1686 / 0	0 / 0	0 / 0	0 / 0	878 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.03 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-84.9	-84.9	0.07 (1)	10.00
B- C	-3474 / 0	-84.9	-84.9	0.13 (1)	4.92
C- P	-4959 / 0	-84.9	-84.9	0.37 (1)	4.03
P- Q	-4959 / 0	-84.9	-84.9	0.37 (1)	4.03
Q- D	-4959 / 0	-84.9	-84.9	0.37 (1)	4.03
D- E	-3789 / 0	-84.9	-84.9	0.26 (1)	4.61
E- F	-3789 / 0	-84.9	-84.9	0.26 (1)	4.61
F- G	-3789 / 0	-84.9	-84.9	0.20 (1)	4.68
G- H	-2273 / 0	-84.9	-84.9	0.16 (1)	5.76
I- H	-2125 / 0	0.0	0.0	0.19 (1)	7.81
O- B	-3537 / 0	0.0	0.0	0.13 (1)	7.45
O- R	0 / 0	-18.5	-18.5	0.13 (1)	10.00
R- N	0 / 0	-18.5	-18.5	0.13 (1)	10.00
N- S	0 / 2655	-18.5	-18.5	0.30 (1)	10.00
S- T	0 / 2655	-18.5	-18.5	0.30 (1)	10.00
T- M	0 / 2655	-18.5	-18.5	0.30 (1)	10.00
M- L	0 / 4959	-18.5	-18.5	0.35 (1)	10.00
L- K	0 / 2273	-18.5	-18.5	0.18 (1)	10.00
K- J	0 / 2273	-18.5	-18.5	0.18 (1)	10.00
J- I	0 / 0	-18.5	-18.5	0.02 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- C	-199 / 25	0.03 (1)
B- N	0 / 2857	0.35 (1)
M- D	0 / 562	0.07 (1)
J- H	0 / 3006	0.37 (1)
D- L	-1603 / 0	0.46 (1)
J- G	-1861 / 0	0.26 (1)
L- F	-291 / 0	0.04 (1)
L- G	0 / 2078	0.26 (1)
C- M	0 / 2845	0.35 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-2-4	-47	-47	---	FRONT	VERT	DEAD	---	C1
C	3-2-4	-38	-38	---	BACK	VERT	TOTAL	---	C1
C	3-2-4	-180	-180	---	FRONT	VERT	SNOW	---	C1
M	8-5-8	-1335	-1335	---	BACK	VERT	TOTAL	---	C1
N	3-3-0	-187	-187	---	BACK	VERT	TOTAL	---	C1
P	5-3-0	-32	-32	---	BACK	VERT	TOTAL	---	C1
Q	7-3-0	-32	-32	---	BACK	VERT	TOTAL	---	C1
R	1-3-0	-187	-187	---	BACK	VERT	TOTAL	---	C1
S	5-3-0	-187	-187	---	BACK	VERT	TOTAL	---	C1
T	7-3-0	-187	-187	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.37/1.00 (C-D:1) , BC=0.35/1.00 (L-M:1) , WB=0.46/1.00 (D-L:1) , SSI=0.14/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (M) (INPUT = 0.90)
JSI METAL= 0.42 (M) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
425915	T87Z	1	2	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	6.0	7.0	2.00	1.75
D	TMWW-t	MT20	4.0	6.0		
E	TS-t	MT20	3.0	8.0		
F	TMW+w	MT20	2.0	4.0		
G	TMWW-t	MT20	4.0	6.0		
H	TMVW-t	MT20	5.0	6.0		
I	BMV1+p	MT20	4.0	6.0		
J	BMWW-t	MT20	5.0	6.0		
K	BS-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	5.0	8.0		
M	BMWW+t	MT20	5.0	6.0	3.50	2.25
N	BMWW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2406509

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425915	T91C	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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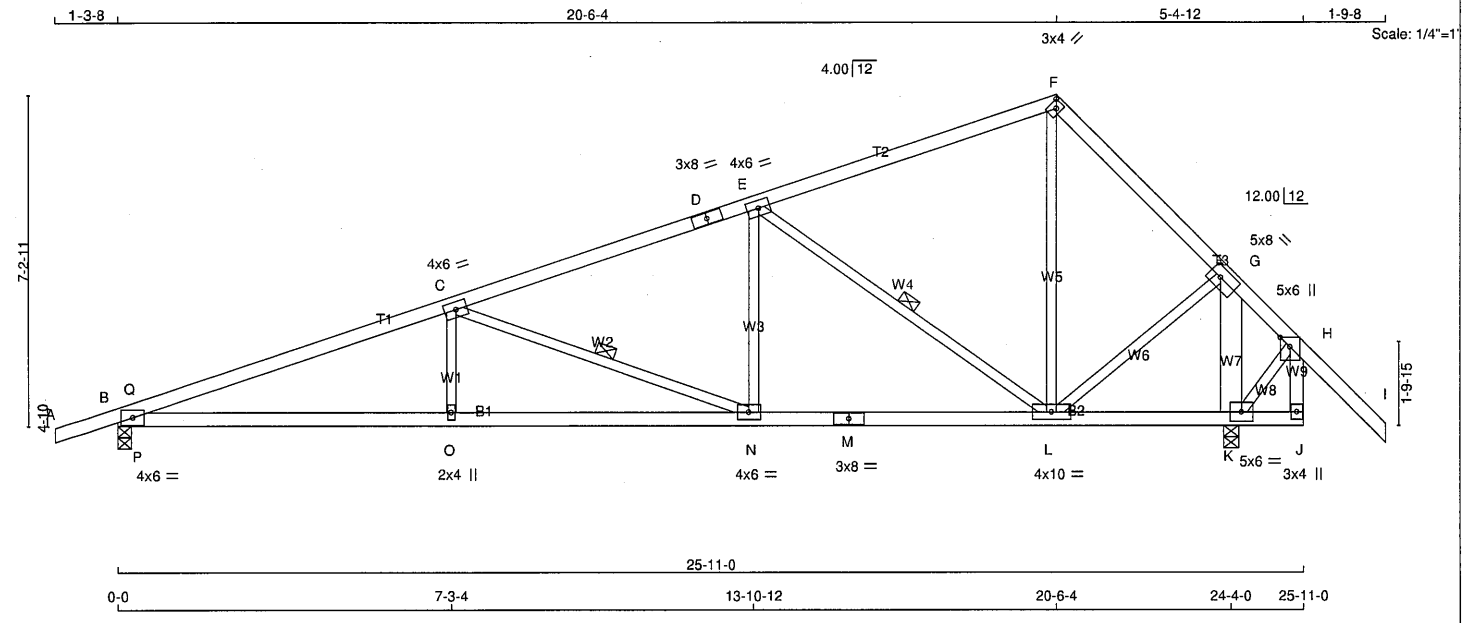

TOTAL WEIGHT = 103 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
B - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
K - G	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-t	MT20	4.0	6.0		
C	TMWW-t	MT20	4.0	6.0		
D	TS-t	MT20	3.0	8.0		
E	TMWW-t	MT20	4.0	6.0		
F	TTW+h	MT20	3.0	4.0	Edge	1.75
G	TMWW-t	MT20	5.0	8.0	2.50	3.00
H	TMVW+p	MT20	5.0	6.0	Edge	
J	BMV+p	MT20	3.0	4.0		
K	BMWW1-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	4.0	10.0		
M	BS-t	MT20	3.0	8.0		
N	BMWW-t	MT20	4.0	6.0		
O	BMW+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	1349	0	1349	0	0	3-8	1-8
K	1604	0	1604	0	0	3-15	1-12

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	956	617 / 0	0 / 0	0 / 0	0 / 0	338 / 0	0 / 0
K	1135	740 / 0	0 / 0	0 / 0	0 / 0	395 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.70 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-N, E-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 15	-84.9	-84.9	0.11 (1)	10.00	O- C	0 / 153	0.05 (4)
B- Q	-2818 / 0	-84.9	-84.9	0.26 (1)	3.94	C- N	-1101 / 0	0.38 (1)
Q- C	-2783 / 0	-84.9	-84.9	0.56 (1)	3.70	N- E	0 / 509	0.11 (1)
C- D	-1701 / 0	-84.9	-84.9	0.47 (1)	4.60	E- L	-1328 / 0	0.58 (1)
D- E	-1701 / 0	-84.9	-84.9	0.47 (1)	4.60	L- F	0 / 357	0.08 (1)
E- F	-587 / 0	-84.9	-84.9	0.44 (1)	6.25	K- H	-185 / 0	0.03 (1)
F- G	-778 / 0	-84.9	-84.9	0.16 (1)	6.25	K- G	-1416 / 0	0.13 (1)
G- H	0 / 166	-84.9	-84.9	0.23 (1)	10.00	L- G	0 / 851	0.19 (1)
H- I	0 / 57	-84.9	-84.9	0.22 (1)	10.00	P- Q	-159 / 99	0.00 (1)
J- H	-2 / 6	0.0	0.0	0.00 (4)	7.81			
B- P	0 / 2647	-18.5	-18.5	0.56 (1)	10.00			
P- O	0 / 2647	-18.5	-18.5	0.61 (1)	10.00			
O- N	0 / 2647	-18.5	-18.5	0.53 (1)	10.00			
N- M	0 / 1617	-18.5	-18.5	0.34 (1)	10.00			
M- L	0 / 1617	-18.5	-18.5	0.34 (1)	10.00			
L- K	-135 / 0	-18.5	-18.5	0.09 (4)	6.25			
K- J	0 / 0	-18.5	-18.5	0.04 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.26")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.56/1.00 (C-Q:1) , BC=0.61/1.00 (O-P:1) , WB=0.58/1.00 (E-L:1) , SSI=0.25/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.71 (B) (INPUT = 0.90)
JSI METAL= 0.59 (B) (INPUT = 0.95)

Structural component only
DWG# T-2406510

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425915	T91G	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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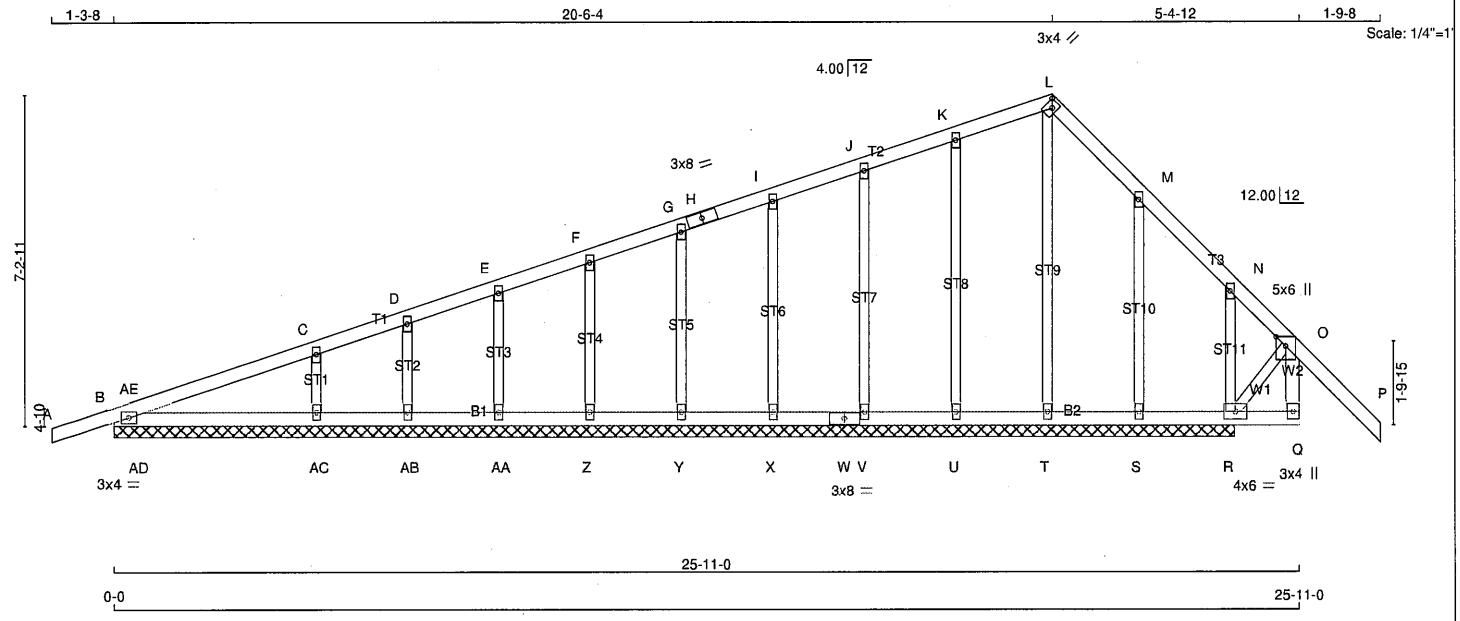

TOTAL WEIGHT = 106 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - H	2x4	DRY	No.2	SPF
H - L	2x4	DRY	No.2	SPF
L - P	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
B - W	2x4	DRY	No.2	SPF
W - Q	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C, D, E, F, G, I, J, K, M, N						
C	TMW+w	MT20	2.0	4.0		
H	TS-t	MT20	3.0	8.0		
L	TTW+h	MT20	3.0	4.0	Edge	1.75
O	TMVW+p	MT20	5.0	6.0	Edge	
Q	BMV+p	MT20	3.0	4.0		
R	BMWW1-t	MT20	4.0	6.0		
S, T, U, V, X, Y, Z, AA, AB, AC						
S	BMW1+w	MT20	2.0	4.0		
W	BS-t	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 15	-84.9	-84.9	0.11 (1)	10.00
B-AE	0 / 157	-84.9	-84.9	0.04 (4)	10.00
AE- C	0 / 157	-84.9	-84.9	0.20 (1)	10.00
C- D	0 / 139	-84.9	-84.9	0.19 (1)	10.00
D- E	0 / 164	-84.9	-84.9	0.07 (1)	10.00
E- F	0 / 162	-84.9	-84.9	0.07 (1)	10.00
F- G	0 / 165	-84.9	-84.9	0.07 (1)	10.00
G- H	0 / 167	-84.9	-84.9	0.07 (1)	10.00
H- I	0 / 167	-84.9	-84.9	0.07 (1)	10.00
I- J	0 / 168	-84.9	-84.9	0.07 (1)	10.00
J- K	0 / 173	-84.9	-84.9	0.09 (1)	10.00
K- L	0 / 165	-84.9	-84.9	0.08 (1)	10.00
L- M	0 / 210	-84.9	-84.9	0.09 (1)	10.00
M- N	0 / 225	-84.9	-84.9	0.09 (1)	10.00
N- O	0 / 147	-84.9	-84.9	0.23 (1)	10.00
O- P	0 / 57	-84.9	-84.9	0.22 (1)	10.00
Q- O	-14 / 6	0.0	0.0	0.00 (1)	7.81
B-AD	-140 / 0	-18.5	-18.5	0.12 (1)	6.25
AD-AC	-140 / 0	-18.5	-18.5	0.12 (1)	6.25
AC-AB	-150 / 0	-18.5	-18.5	0.09 (1)	6.25
AB-AA	-152 / 0	-18.5	-18.5	0.02 (4)	6.25
AA- Z	-155 / 0	-18.5	-18.5	0.02 (4)	6.25
Z- Y	-157 / 0	-18.5	-18.5	0.01 (4)	6.25
Y- X	-159 / 0	-18.5	-18.5	0.01 (4)	6.25
X- W	-160 / 0	-18.5	-18.5	0.01 (4)	6.25
W- V	-160 / 0	-18.5	-18.5	0.01 (4)	6.25
V- U	-162 / 0	-18.5	-18.5	0.01 (4)	6.25
U- T	-163 / 0	-18.5	-18.5	0.02 (4)	6.25
T- S	-158 / 0	-18.5	-18.5	0.02 (4)	6.25
S- R	-154 / 0	-18.5	-18.5	0.06 (1)	6.25
R- Q	0 / 0	-18.5	-18.5	0.06 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- L	-349 / 0	0.32 (1)
U- K	-205 / 0	0.14 (1)
V- J	-160 / 0	0.08 (1)
X- I	-171 / 0	0.07 (1)
Y- G	-170 / 0	0.05 (1)
Z- F	-165 / 0	0.04 (1)
AA- E	-186 / 0	0.03 (1)
AB- D	-95 / 0	0.02 (1)
AC- C	-339 / 0	0.05 (1)
S- M	-195 / 0	0.08 (1)
R- N	-35 / 0	0.01 (1)
R- O	-214 / 0	0.03 (1)
AD-AE	-178 / 5	0.00 (1)

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.23/1.00 (N-O:1) , BC=0.12/1.00 (AC-AD:1) , WB=0.32/1.00 (L-T:1) , SSI=0.15/1.00 (C-AE:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.61 (L) (INPUT = 0.90)
JSI METAL= 0.13 (L) (INPUT = 0.95)

Structural component only
DWG# T-2406511

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425915	T92C	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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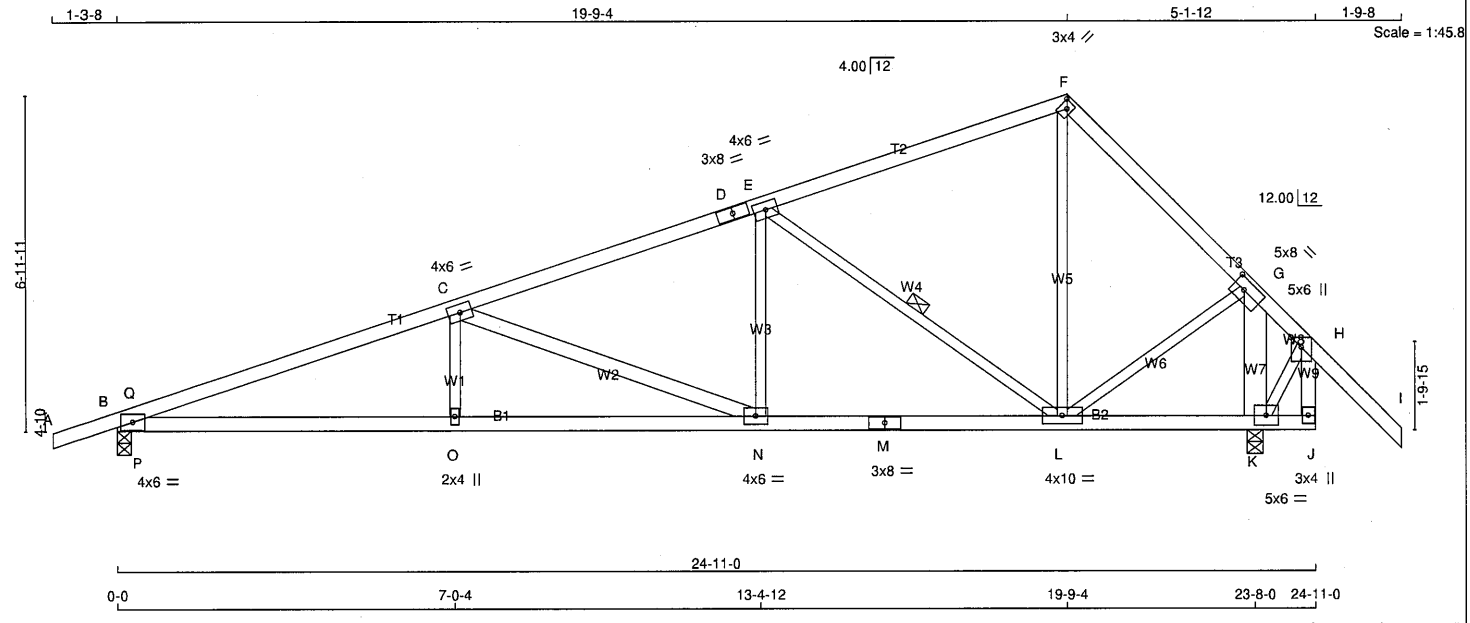

TOTAL WEIGHT = 101 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
B - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - N	2x4	DRY	No.2	SPF
K - G	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	4.0	6.0		
C	TMWW-t	MT20	4.0	6.0		
D	TS-t	MT20	3.0	8.0		
E	TMWW-t	MT20	4.0	6.0		
F	TTW+h	MT20	3.0	4.0	Edge	1.75
G	TMWW-t	MT20	5.0	8.0	2.50	3.00
H	TMVW+p	MT20	5.0	6.0	Edge	
J	BMV+p	MT20	3.0	4.0		
K	BMWW1-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	4.0	10.0		
M	BS-t	MT20	3.0	8.0		
N	BMWW-t	MT20	4.0	6.0		
O	BMW+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	1318	0	1318	0	0	3-8	1-8
K	1531	0	1531	0	0	3-15	1-11

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	934	604 / 0	0 / 0	0 / 0	0 / 0	330 / 0	0 / 0
K	1084	707 / 0	0 / 0	0 / 0	0 / 0	376 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.79 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 15	-84.9	-84.9	0.11 (1)	10.00
B- Q	-2757 / 0	-84.9	-84.9	0.24 (1)	3.99
Q- C	-2718 / 0	-84.9	-84.9	0.51 (1)	3.79
C- D	-1680 / 0	-84.9	-84.9	0.43 (1)	4.68
D- E	-1680 / 0	-84.9	-84.9	0.43 (1)	4.68
E- F	-611 / 0	-84.9	-84.9	0.40 (1)	6.25
F- G	-812 / 0	-84.9	-84.9	0.16 (1)	6.25
G- H	0 / 129	-84.9	-84.9	0.23 (1)	10.00
H- I	0 / 57	-84.9	-84.9	0.22 (1)	10.00
J- H	-12 / 0	0.0	0.0	0.00 (1)	7.81
B- P	0 / 2585	-18.5	-18.5	0.53 (1)	10.00
P- O	0 / 2585	-18.5	-18.5	0.58 (1)	10.00
O- N	0 / 2585	-18.5	-18.5	0.51 (1)	10.00
N- M	0 / 1597	-18.5	-18.5	0.33 (1)	10.00
M- L	0 / 1597	-18.5	-18.5	0.33 (1)	10.00
L- K	-109 / 0	-18.5	-18.5	0.09 (4)	6.25
K- J	0 / 0	-18.5	-18.5	0.04 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- C	0 / 147	0.05 (4)
C- N	-1057 / 0	0.66 (1)
N- E	0 / 490	0.11 (1)
E- L	-1276 / 0	0.52 (1)
L- F	0 / 394	0.09 (1)
K- G	-1336 / 0	0.11 (1)
L- G	0 / 807	0.18 (1)
K- H	-169 / 0	0.03 (1)
P- Q	-140 / 97	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.79")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.79")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.23")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.51/1.00 (C-Q:1) , BC=0.58/1.00 (O-P:1) , WB=0.66/1.00 (C-N:1) , SSI=0.24/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.70 (B) (INPUT = 0.90)
JSI METAL= 0.58 (B) (INPUT = 0.95)

Structural component only
DWG# T-2406512

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425915	T92G	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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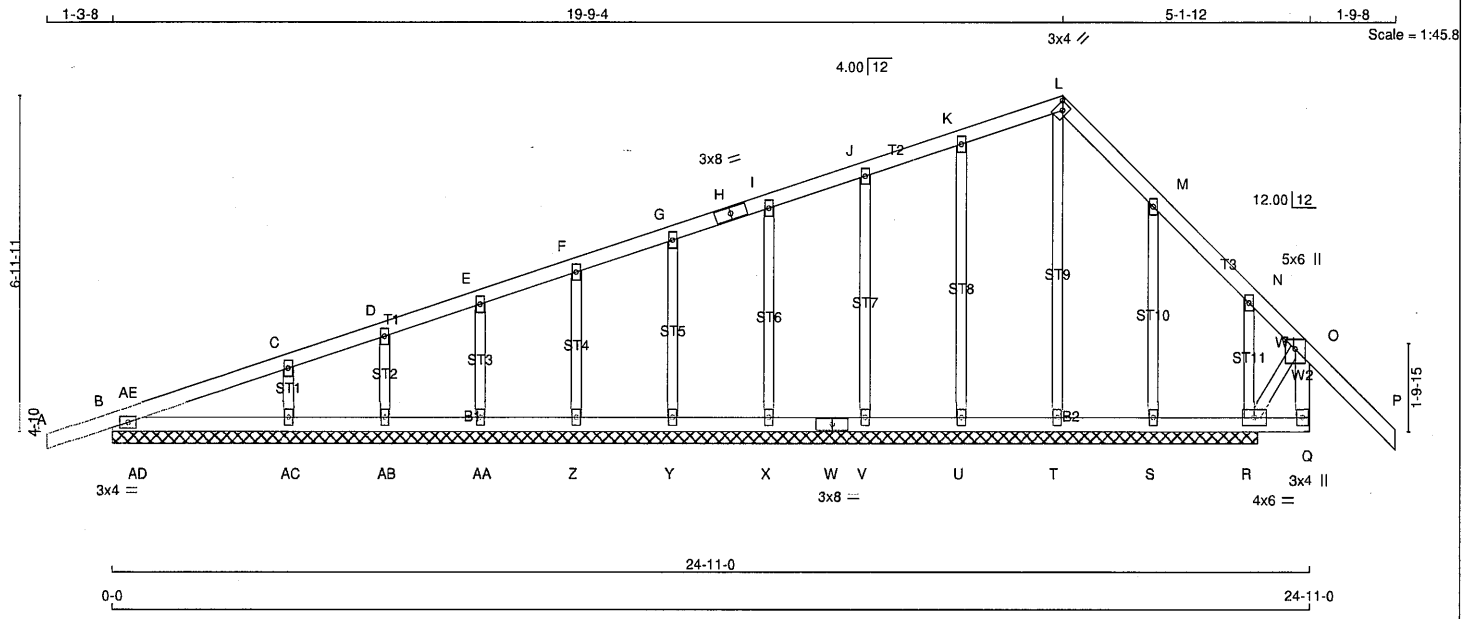

TOTAL WEIGHT = 101 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - H	2x4	DRY	No.2	SPF
H - L	2x4	DRY	No.2	SPF
L - P	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
B - W	2x4	DRY	No.2	SPF
W - Q	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C, D, E, F, G, I, J, K, M, N						
C	TMW+w	MT20	2.0	4.0		
H	TS-t	MT20	3.0	8.0		
L	TTW+h	MT20	3.0	4.0	Edge	1.75
O	TMVW+p	MT20	5.0	6.0	Edge	
Q	BMV+p	MT20	3.0	4.0		
R	BMWW1-t	MT20	4.0	6.0		
S, T, U, V, X, Y, Z, AA, AB, AC						
S	BMW1+w	MT20	2.0	4.0		
W	BS-t	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2406513

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI	MAX (LC)	MAX. UNBRAC LENGTH
A- B	0 / 15	-84.9	-84.9	0.11	(1)	10.00
B-AE	0 / 131	-84.9	-84.9	0.03	(4)	10.00
AE- C	0 / 139	-84.9	-84.9	0.14	(1)	10.00
C- D	0 / 130	-84.9	-84.9	0.14	(1)	10.00
D- E	0 / 146	-84.9	-84.9	0.07	(1)	10.00
E- F	0 / 147	-84.9	-84.9	0.07	(1)	10.00
F- G	0 / 150	-84.9	-84.9	0.06	(1)	10.00
G- H	0 / 152	-84.9	-84.9	0.06	(1)	10.00
H- I	0 / 152	-84.9	-84.9	0.06	(1)	10.00
I- J	0 / 153	-84.9	-84.9	0.06	(1)	10.00
J- K	0 / 157	-84.9	-84.9	0.08	(1)	10.00
K- L	0 / 150	-84.9	-84.9	0.08	(1)	10.00
L- M	0 / 190	-84.9	-84.9	0.08	(1)	10.00
M- N	0 / 203	-84.9	-84.9	0.09	(1)	10.00
N- O	0 / 115	-84.9	-84.9	0.23	(1)	10.00
O- P	0 / 57	-84.9	-84.9	0.22	(1)	10.00
Q- O	-22 / 2	0.0	0.0	0.00	(1)	7.81
B-AD	-123 / 0	-18.5	-18.5	0.09	(1)	6.25
AD-AC	-123 / 0	-18.5	-18.5	0.09	(1)	6.25
AC-AB	-133 / 0	-18.5	-18.5	0.06	(1)	6.25
AB-AA	-136 / 0	-18.5	-18.5	0.02	(4)	6.25
AA- Z	-140 / 0	-18.5	-18.5	0.02	(4)	6.25
Z- Y	-142 / 0	-18.5	-18.5	0.01	(4)	6.25
Y- X	-144 / 0	-18.5	-18.5	0.01	(4)	6.25
X- W	-146 / 0	-18.5	-18.5	0.01	(4)	6.25
W- V	-146 / 0	-18.5	-18.5	0.01	(4)	6.25
V- U	-147 / 0	-18.5	-18.5	0.01	(4)	6.25
U- T	-149 / 0	-18.5	-18.5	0.02	(4)	6.25
T- S	-143 / 0	-18.5	-18.5	0.02	(4)	6.25
S- R	-139 / 0	-18.5	-18.5	0.06	(1)	6.25
R- Q	0 / 0	-18.5	-18.5	0.06	(1)	10.00

WEBS

MEMB. FR-TO	MAX. FORCE (LBS)	MAX CSI (LC)
T- L	-330 / 0	0.27 (1)
U- K	-205 / 0	0.13 (1)
V- J	-160 / 0	0.08 (1)
X- I	-171 / 0	0.06 (1)
Y- G	-169 / 0	0.05 (1)
Z- F	-167 / 0	0.04 (1)
AA- E	-179 / 0	0.03 (1)
AB- D	-125 / 0	0.02 (1)
AC- C	-279 / 0	0.04 (1)
S- M	-192 / 0	0.07 (1)
R- N	-8 / 0	0.00 (1)
R- O	-215 / 0	0.03 (1)
AD-AE	-130 / 3	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.23/1.00 (N-O:1) , BC=0.09/1.00 (AC-AD:1) , WB=0.27/1.00 (L-T:1) , SSI=0.12/1.00 (C-AE:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.61 (L) (INPUT = 0.90)
JSI METAL= 0.12 (L) (INPUT = 0.95)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425055	PB3	4	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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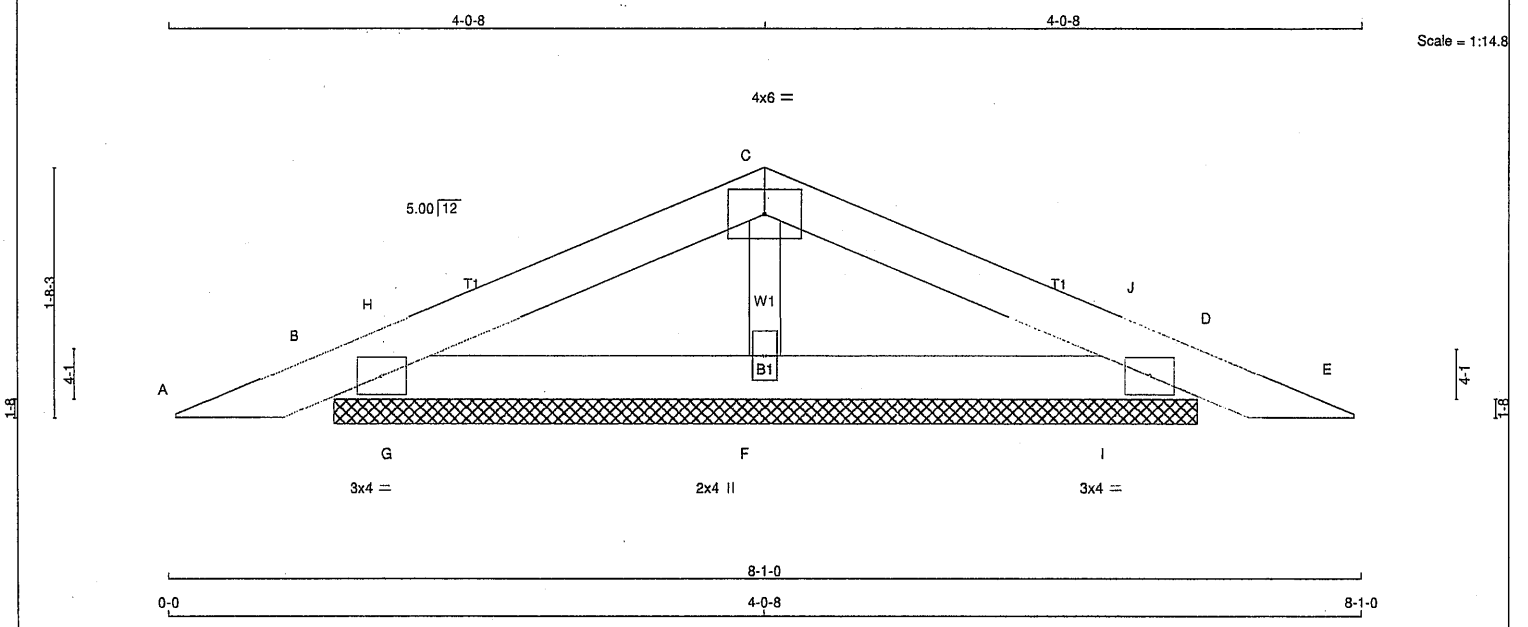

TOTAL WEIGHT = 4 X 17 = 69 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW-p	MT20	4.0	6.0		
D	TMB1-I	MT20	3.0	4.0		
F	BMW1+w	MT20	2.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	239	0	239	0	0	5-10-6	1-8
D	239	0	239	0	0	5-10-6	1-8
F	300	0	300	0	0	5-10-6	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	167	118 / 0	0 / 0	0 / 0	0 / 0	49 / 0	0 / 0
D	167	118 / 0	0 / 0	0 / 0	0 / 0	49 / 0	0 / 0
F	214	131 / 0	0 / 0	0 / 0	0 / 0	83 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 15	-84.9	-84.9	0.06 (1)	10.00	F- C	-163 / 0	0.02 (1)
B- H	-44 / 0	-84.9	-84.9	0.01 (1)	6.25	G- H	-142 / 0	0.00 (1)
H- C	-51 / 0	-84.9	-84.9	0.08 (1)	6.25	I- J	-142 / 0	0.00 (1)
C- J	-51 / 0	-84.9	-84.9	0.08 (1)	6.25			
J- D	-44 / 0	-84.9	-84.9	0.01 (1)	6.25			
D- E	0 / 15	-84.9	-84.9	0.06 (1)	10.00			
B- G	0 / 46	-18.5	-18.5	0.09 (1)	10.00			
G- F	0 / 46	-18.5	-18.5	0.09 (1)	10.00			
F- I	0 / 46	-18.5	-18.5	0.09 (1)	10.00			
I- D	0 / 46	-18.5	-18.5	0.09 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.08/1.00 (C-J:1) , BC=0.09/1.00 (F-I:1) , WB=0.02/1.00 (C-F:1) , SSI=0.10/1.00 (D-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (D) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 0.95)

Structural component only
DWG# T-2406306

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425055	J2	40	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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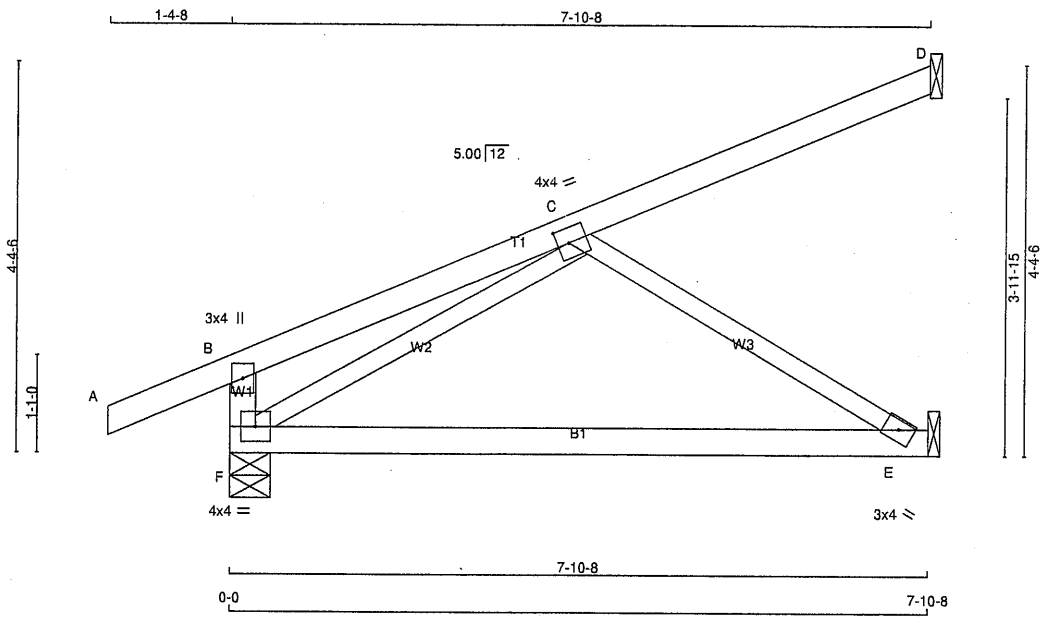

TOTAL WEIGHT = 40 X 28 = 1131 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
E	BMW1+w	MT20	3.0	4.0		
F	BMVW1-t	MT20	4.0	4.0		

NOTES (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	516	0	516	0	0	5-8	1-8
D	131	0	131	0	0	1-8	1-8
E	281	0	281	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) D , E

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	364	245 / 0	0 / 0	0 / 0	0 / 0	119 / 0	0 / 0
D	91	72 / 0	0 / 0	0 / 0	0 / 0	19 / 0	0 / 0
E	202	116 / 0	0 / 0	0 / 0	0 / 0	86 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-245 / 0	0.0	0.0	0.02 (1)	7.81	F- C	-384 / 0	0.13 (1)
A- B	0 / 23	-84.9	-84.9	0.12 (1)	10.00	C- E	-389 / 0	0.14 (1)
B- C	0 / 16	-84.9	-84.9	0.22 (1)	10.00			
C- D	-15 / 0	-84.9	-84.9	0.16 (1)	6.25			
F- E	0 / 327	-18.5	-18.5	0.32 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.25")
CALCULATED VERT. DEFL.(TL) = L/ 620 (0.15")

CSI: TC=0.22/1.00 (B-C:1) , BC=0.32/1.00 (E-F:4) , WB=0.14/1.00 (C-E:1) , SSI=0.15/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.36 (C) (INPUT = 0.90)
JSI METAL= 0.14 (E) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
02-28-24
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2406298

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425055	J2S	24	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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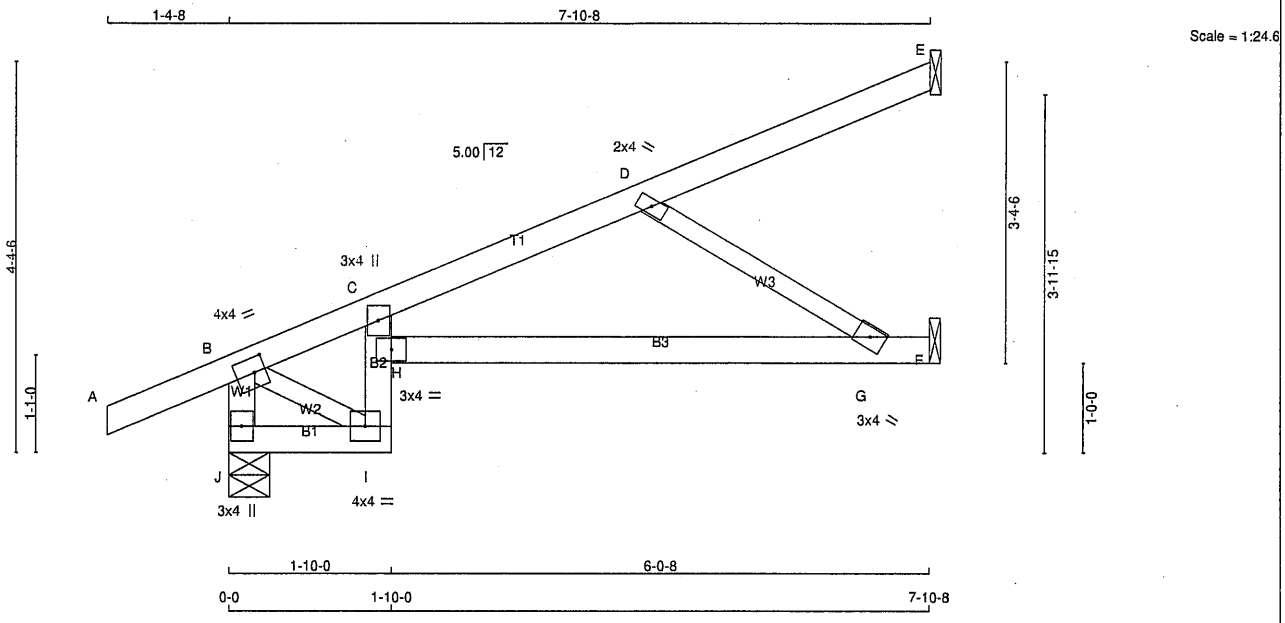

TOTAL WEIGHT = 24 X 26 = 631 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
J - B	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - C	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.50
C	TMV+p	MT20	3.0	4.0		
D	TMW+w	MT20	2.0	4.0		
G	BMW+w	MT20	3.0	4.0		
H	BVM-l	MT20	3.0	4.0		
I	BMVW-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	539	0	539	0	0	5-8	1-8
E	90	0	90	0	0	1-8	1-8
F	306	0	306	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) E , F

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	380	256 / 0	0 / 0	0 / 0	0 / 0	124 / 0	0 / 0
E	62	53 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0
F	220	124 / 0	0 / 0	0 / 0	0 / 0	95 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- B	-560 / 0	0.0	0.0	0.05 (4)	7.81	D- G	-484 / 0	0.11 (1)
A- B	0 / 23	-84.9	-84.9	0.12 (1)	10.00	B- I	0 / 223	0.05 (1)
B- C	-323 / 0	-84.9	-84.9	0.24 (1)	6.25			
C- D	-381 / 0	-84.9	-84.9	0.36 (1)	6.25			
D- E	-17 / 0	-84.9	-84.9	0.19 (1)	6.25			
J- I	0 / 32	-18.5	-18.5	0.08 (1)	10.00			
I- H	-42 / 8	0.0	0.0	0.24 (1)	7.81			
H- C	0 / 88	0.0	0.0	0.22 (1)	10.00			
H- G	0 / 399	-18.5	-18.5	0.35 (1)	10.00			
G- F	0 / 0	-18.5	-18.5	0.27 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.26")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.26")
CALCULATED VERT. DEFL.(TL) = L/ 424 (0.22")

CSI: TC=0.36/1.00 (C-D:1) , BC=0.35/1.00 (G-H:1) , WB=0.11/1.00 (D-G:1) , SSI=0.24/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.37 (I) (INPUT = 0.90)
JSI METAL= 0.22 (D) (INPUT = 0.95)

Structural component only
DWG# T-2406299

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425055	J5	13	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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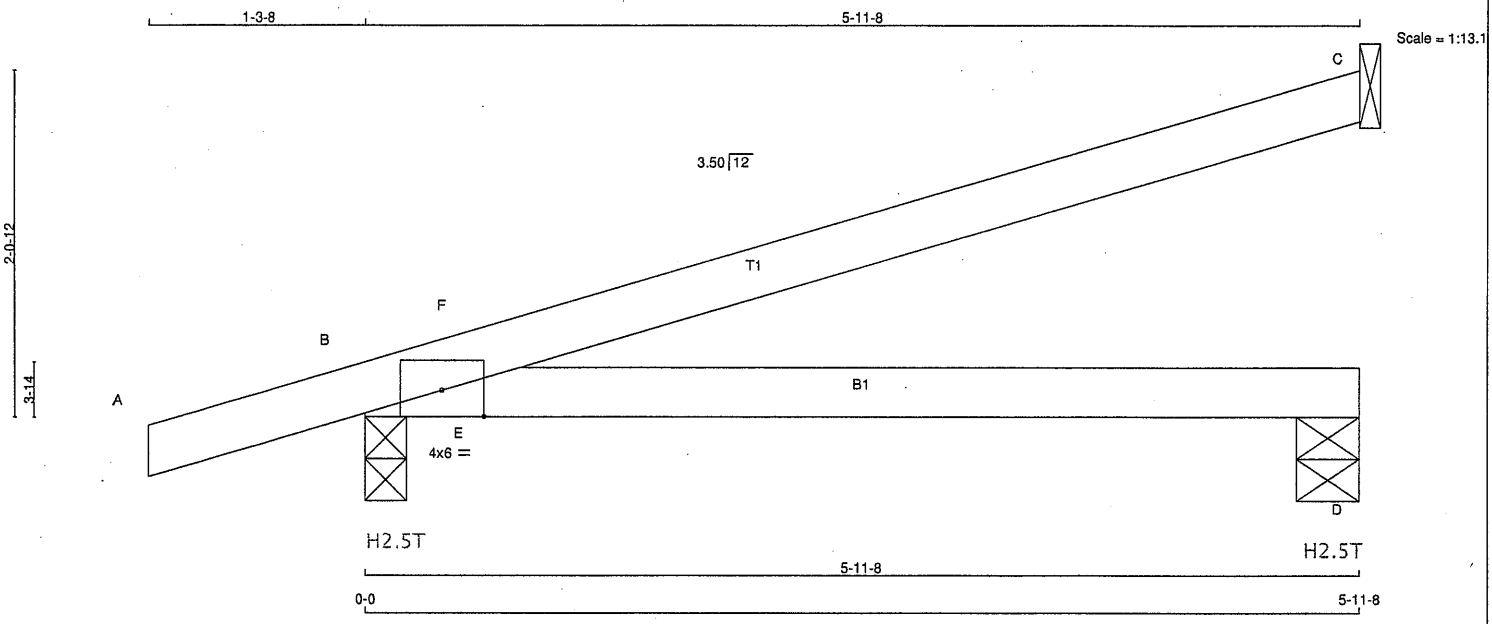

TOTAL WEIGHT = 13 X 16 = 202 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	4.0	6.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
C	221	0	221	0	-152	1-8	1-8
B	421	0	421	123	-277	3-0	1-8
D	87	0	87	0	-109	4-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 152 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT B FOR 277 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT

PROVIDE FOR 123 LBS FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	153	117 / 0	0 / 0	0 / 0	0 / -132	37 / 0	0 / 0
B	297	201 / 0	0 / 0	0 / 0	0 / -260	96 / 0	0 / 0
D	65	22 / 0	0 / 0	0 / 0	0 / -106	43 / 0	0 / 0
HORIZONTAL REACTIONS							
B	---	0 / 0	0 / 0	0 / 0	88 / 0	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (12)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 15	-84.9	-84.9	0.10 (1)	10.00	E- F	-291 / 105	0.00 (1)
B- F	-85 / 8	-84.9	-84.9	0.06 (12)	6.25			
F- C	-50 / 2	-84.9	-84.9	0.38 (1)	6.25			
B- E	0 / 0	-18.5	-18.5	0.27 (1)	10.00			
E- D	0 / 0	-18.5	-18.5	0.28 (1)	10.00			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.2} PSF AT {15-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 639 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 496 (0.14")

CSI: TC=0.38/1.00 (C-F:1) , BC=0.28/1.00 (D-E:1) , WB=0.00/1.00 (E-F:1) , SSI=0.23/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.11 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 0.95)

Structural component only
DWG# T-2406302

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425055	J6	6	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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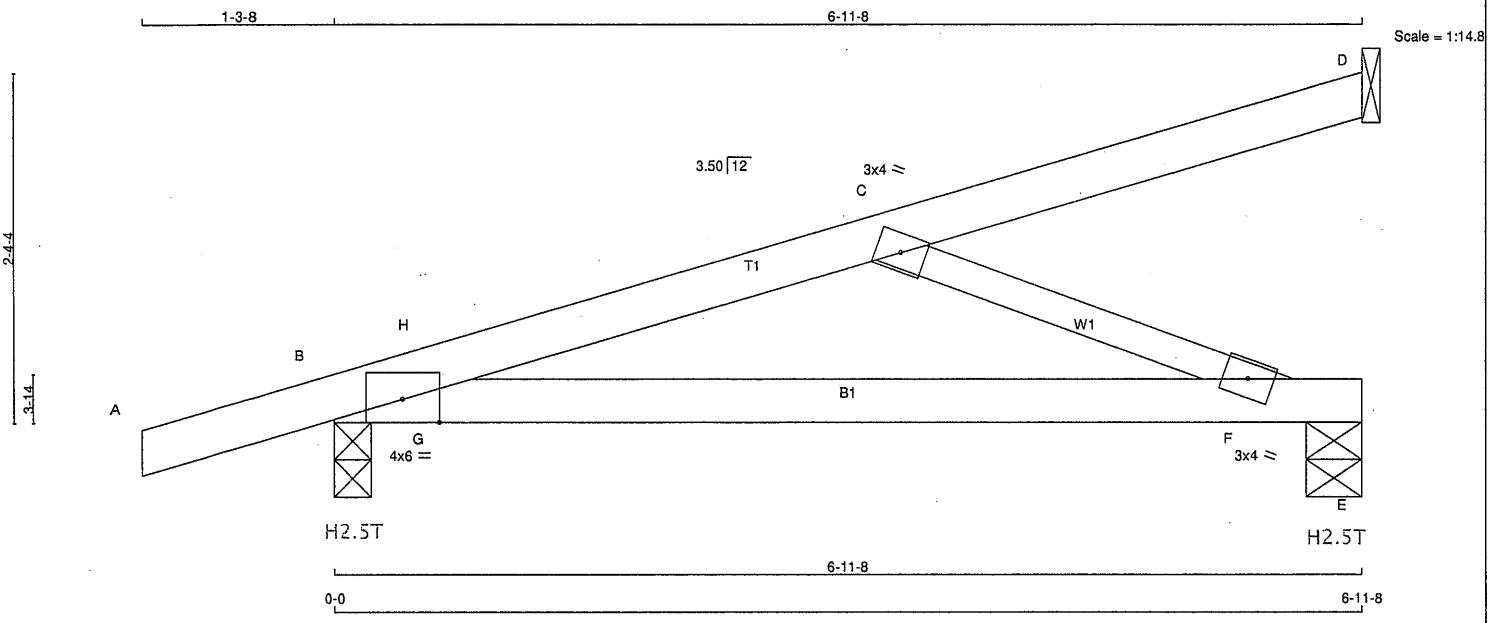

TOTAL WEIGHT = 6 X 20 = 121 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	4.0	6.0	Edge	
C	TMW+w	MT20	3.0	4.0		
F	BMW+w	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	107	0	107	0	-67	1-8	1-8
B	473	0	473	141	-316	3-0	1-8
E	253	0	253	0	-238	4-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) D

PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT B FOR 316 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT E FOR 238 LBS FACTORED UPLIFT

PROVIDE FOR 141 LBS FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	74	59 / 0	0 / 0	0 / 0	0 / -57	14 / 0	0 / 0
B	333	224 / 0	0 / 0	0 / 0	0 / -296	109 / 0	0 / 0
E	182	103 / 0	0 / 0	0 / 0	0 / -221	79 / 0	0 / 0
HORIZONTAL REACTIONS							
B	---	0 / 0	0 / 0	0 / 0	100 / 0	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, B, E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (12)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 15	-84.9	-84.9	0.10 (1)	10.00	C- F	-529 / 385	0.11 (1)
B- H	-567 / 322	-84.9	-84.9	0.25 (5)	6.25	G- H	-233 / 146	0.00 (1)
H- C	-478 / 255	-84.9	-84.9	0.24 (5)	6.25			
C- D	-24 / 0	-84.9	-84.9	0.09 (1)	6.25			
B- G	-348 / 478	-18.5	-18.5	0.12 (12)	6.25			
G- F	-348 / 478	-18.5	-18.5	0.34 (1)	6.25			
F- E	0 / 0	-18.5	-18.5	0.26 (1)	10.00			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.2} PSF AT {15-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

[M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/ 546 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/ 514 (0.16")

CSI: TC=0.25/1.00 (B-H:5) , BC=0.34/1.00 (F-G:1) , WB=0.11/1.00 (C-F:1) , SSI=0.20/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.35 (F) (INPUT = 0.90)
JSI METAL= 0.20 (F) (INPUT = 0.95)

Structural component only
DWG# T-2406303

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425915	J10	6	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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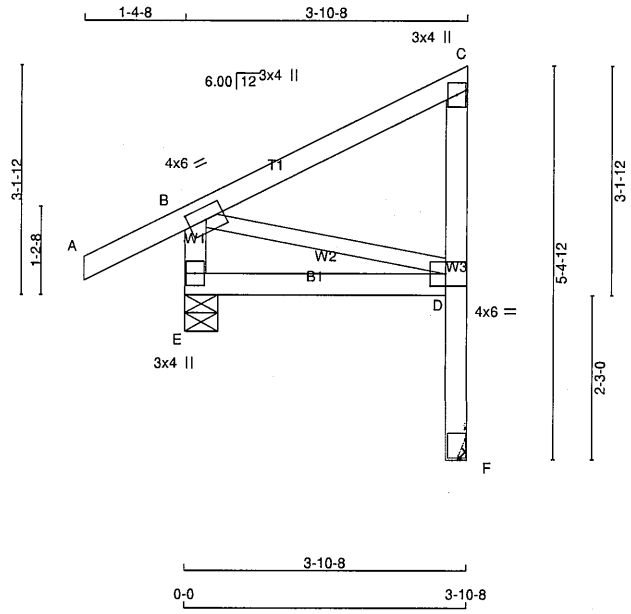

Scale = 1:30.2

TOTAL WEIGHT = 6 X 20 = 122 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	2.00	3.00
B	TP+p	MT20	3.0	4.0	3.50	3.50
C	TMV+p	MT20	3.0	4.0		
D	BVMW-I	MT20	4.0	6.0	2.00	2.50
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	322	0	322	0	0	5-8	1-8
F	200	0	200	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-12.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	227	157 / 0	0 / 0	0 / 0	0 / 0	69 / 0	0 / 0
F	142	90 / 0	0 / 0	0 / 0	0 / 0	52 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-287 / 0	0.0	0.0	0.03 (1)	7.81	B- D	0 / 0	0.00 (1)
A- B	0 / 27	-84.9	-84.9	0.14 (5)	10.00			
B- C	0 / 0	-84.9	-84.9	0.22 (1)	10.00			
F- D	-200 / 0	0.0	0.0	0.03 (1)	7.81			
D- C	-164 / 0	0.0	0.0	0.03 (1)	7.81			
E- D	0 / 0	-18.5	-18.5	0.08 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.22/1.00 (B-C:1) , BC=0.08/1.00 (D-E:4) , WB=0.00/1.00 (B-D:1) , SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (B) (INPUT = 0.90)
JSI METAL= 0.05 (C) (INPUT = 0.95)

Structural component only
DWG# T-2406478

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425915	J11	25	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.630 S Aug 30 2023 MiTek Industries, Inc. Wed Feb 28 17:09:58 2024 Page 1
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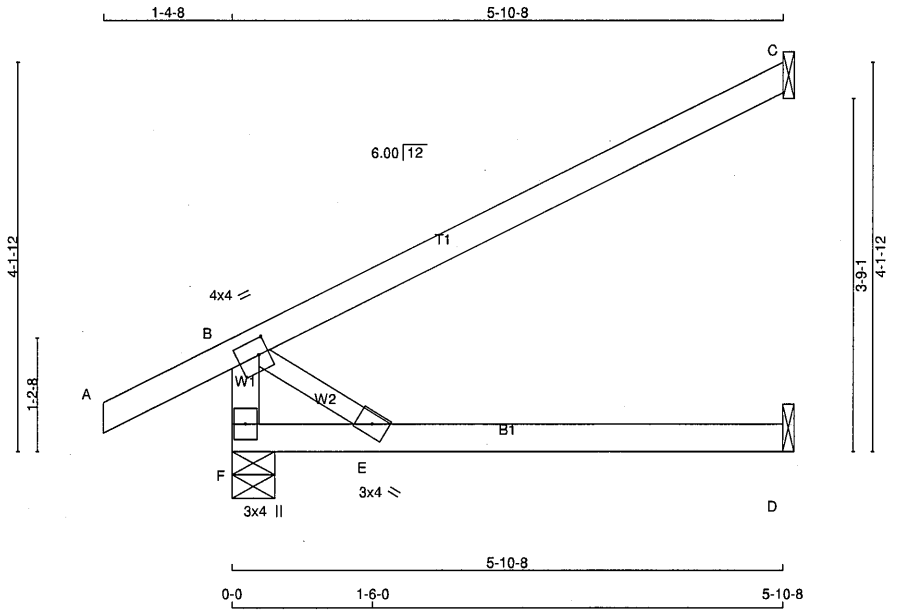

TOTAL WEIGHT = 25 X 18 = 456 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
E	BMW+w	MT20	3.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	426	0	426	0	0	5-8	1-8
C	249	0	249	0	0	1-8	1-8
D	54	0	61	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	300	204 / 0	0 / 0	0 / 0	0 / 0	96 / 0	0 / 0
C	172	137 / 0	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0
D	43	0 / 0	0 / 0	0 / 0	0 / 0	43 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-371 / 0	0.0	0.0	0.04 (1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 27	-84.9	-84.9	0.12 (1)	10.00			
B- C	0 / 0	-84.9	-84.9	0.50 (1)	10.00			
F- E	0 / 0	-18.5	-18.5	0.14 (4)	10.00			
E- D	0 / 0	-18.5	-18.5	0.19 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.50/1.00 (B-C:1) , BC=0.19/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.17/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 0.95)

Structural component only
DWG# T-2406479

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425915	J11S	12	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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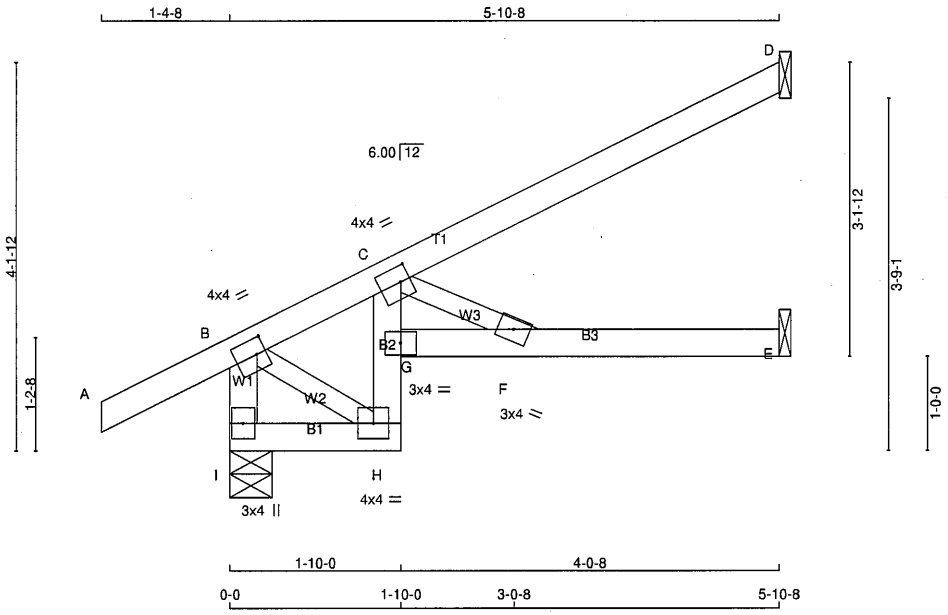


TOTAL WEIGHT = 12 X 21 = 253 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
I - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
H - C	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TMVW-t	MT20	4.0	4.0	2.00	1.25
F	BMW+w	MT20	3.0	4.0		
G	BVM-I	MT20	3.0	4.0		
H	BMVW-t	MT20	4.0	4.0		
I	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	441	0	441	0	0	5-8	1-8
D	180	0	180	0	0	1-8	1-8
E	108	0	108	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) D , E

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	310	212 / 0	0 / 0	0 / 0	0 / 0	98 / 0	0 / 0
D	125	95 / 0	0 / 0	0 / 0	0 / 0	31 / 0	0 / 0
E	80	34 / 0	0 / 0	0 / 0	0 / 0	46 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- B	-425 / 0	0.0	0.0	0.04 (1)	7.81	B- H	0 / 189	0.04 (1)
A- B	0 / 27	-84.9	-84.9	0.12 (1)	10.00	C- F	-446 / 0	0.07 (1)
B- C	-213 / 0	-84.9	-84.9	0.12 (1)	6.25			
C- D	0 / 4	-84.9	-84.9	0.26 (1)	10.00			
I- H	0 / 0	-18.5	-18.5	0.02 (4)	10.00			
H- G	-73 / 0	0.0	0.0	0.23 (1)	7.81			
G- C	0 / 111	0.0	0.0	0.24 (1)	10.00			
G- F	0 / 391	-18.5	-18.5	0.38 (1)	10.00			
F- E	0 / 0	-18.5	-18.5	0.32 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 619 (0.11")

CSI: TC=0.26/1.00 (C-D:1) , BC=0.38/1.00 (F-G:1) , WB=0.07/1.00 (C-F:1) , SSI=0.18/1.00 (C-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

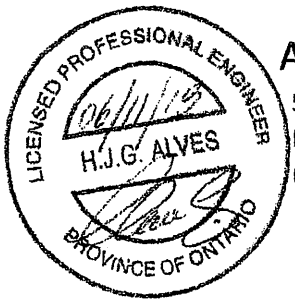
PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.33 (C) (INPUT = 0.90)
JSI METAL= 0.17 (F) (INPUT = 0.95)

Structural component only
DWG# T-2406480



Alves Engineering Services Inc.
5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components

2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.

3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.

4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.

5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.

2- Lumber is to be the sizes and grade specified on the truss drawing.

3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.

4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings

5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.

6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)

7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.

8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

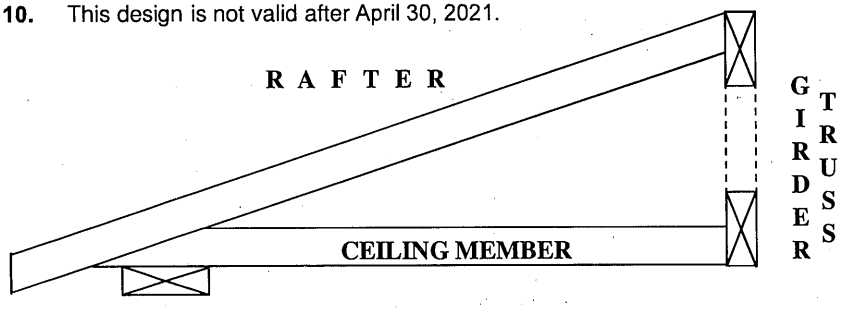
T-1900215 Feb 09, 2018

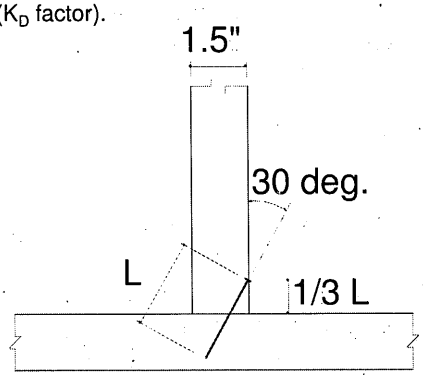
BEARING ANCHORAGE BY TOE-NAILS FOR LATERAL CAPACITY B97791H1

NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL LATERAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	132	147
	3.25	0.144	132	147
	3.50	0.160	159	177
COMMON SPIRAL	3.00	0.122	97	108
	3.25	0.122	97	108
	3.50	0.152	145	162

NOTES:

1. Rafter and ceiling members may be anchored to top and bottom chords of girder truss by toe-nailing rafter and ceiling members to girder chords provided the reaction does not exceed the lateral capacities in the table. Hangers (specified by others) are required for reactions higher than the maximum toe-nail capacity. Reactions are based on factored loads.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.4.1.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in tables below.
5. Nail values in table are based on the following relative lumber densities: G = 0.42 (SPF), G = 0.49 (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See next page for nailing on bearing plate).
7. For loads due to **wind** the nail lateral capacity in this table may be multiplied by 1.15 (K_D factor).
8. Lumber must be dry (< 19% moisture content) at the time of nail installation.
9. Nail values in this table comply with CSA O86-14, section 12.9.4
10. This design is not valid after April 30, 2021.

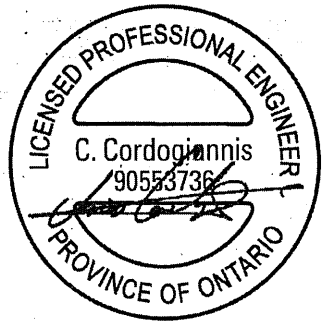




TOE-NAIL INSTALLATION

Nail type	Common wire	Common spiral	Common wire	Common spiral
Nail dia. (in)	0.160	0.152	0.144	0.122
	(3.5" nail)		(3" and 3.25" nail)	
LUMBER SIZE	MAXIMUM NUMBER OF TOE-NAILS			
2X4 SPF	2	2	3	3
2X4 D. Fir	2	2	2	2
2X6 SPF	4	4	4	5
2X6 D. Fir	3	3	3	4

PEO
Certificate No. 10889485



MiTek®

MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

April 2, 2020

BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING B97791H2

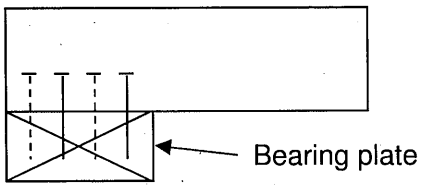
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	30	42
	3.25	0.144	32	45
	3.50	0.160	38	52
COMMON SPIRAL	3.00	0.122	26	36
	3.25	0.122	28	40
	3.50	0.152	36	50

Note: If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

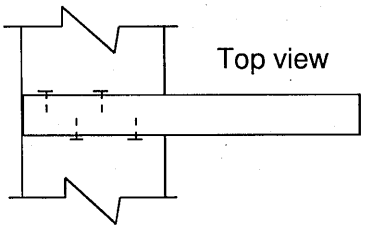
NOTES:

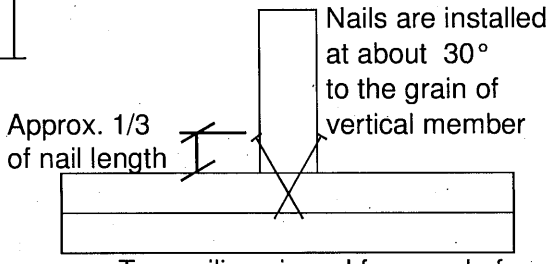
1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to **wind** or **earthquake** load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.5.2.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities: G = 0.42(SPF), G = 0.49(D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37579H1).
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-14, section 12.9.5
9. This design is not valid after April 30, 2021.

Toe-nailing on 2x6 Bearing Plate



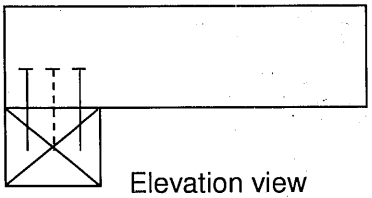
Elevation view



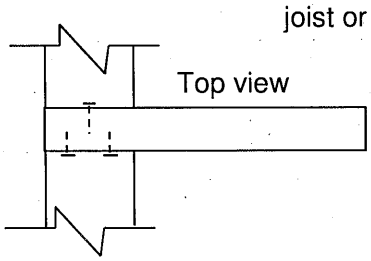


Toe-nailing viewed from end of joist or truss

Toe-nailing on 2x4 Bearing Plate



Elevation view



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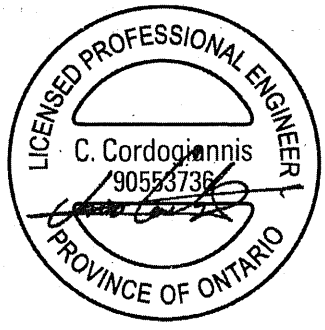


MiTek®

MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

April 2, 2020

MiTek®

STANDARD DETAIL MSD2015-P

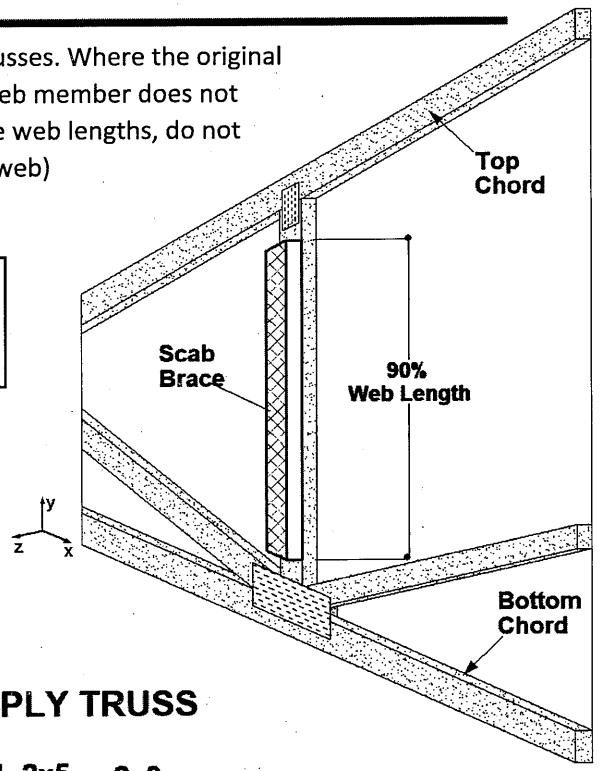
Issued: **APRIL 27, 2022**
Expiry: **APRIL 30, 2024**

ALTERNATIVE WEB BRACING SOLUTIONS

The scab brace detail shown on this page provides an alternative method of bracing compression webs of single ply trusses. Where the original design calls for web bracing, the scab-brace is an acceptable alternative provided that the factored axial force in the web member does not exceed the tabulated values shown below. This detail applies to web lengths of 4.0 ft. to 10.0 ft. only. For intermediate web lengths, do not interpolate, use the tabulated value of the longer length. (ex. For a 6.25 ft. web, use the tabulated values for a 6.5 ft. web)

	Maximum factored web force, lbs (1-Ply Truss)				
WEB LENGTH (Ft)	2x3	2x4	2x5	2x6	2x8+
4.0	4331	6064	7796	9529	12561
4.5	3794	5312	6829	8347	11003
5.0	3285	4599	5913	7227	9527
5.5	2823	3952	5081	6210	8186
6.0	2415	3381	4347	5313	7003
6.5	2063	2888	3713	4538	5982
7.0	1763	2468	3174	3879	5113
7.5	1510	2114	2718	3322	4379
8.0	1297	1816	2335	2854	3762
8.5	1117	1564	2011	2458	3240
9.0	966	1353	1740	2126	2803
9.5	840	1176	1512	1848	2436
10.0	733	1027	1320	1614	2127

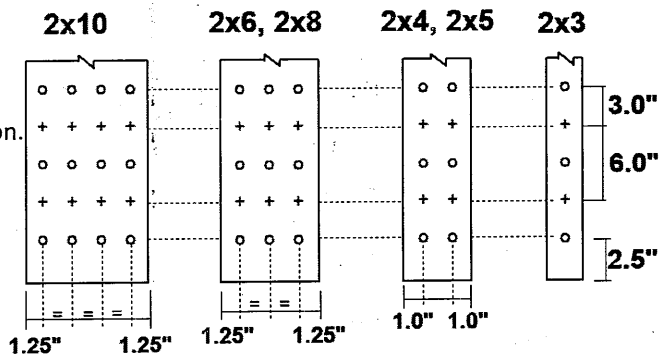
SCAB BRACE DETAIL
1-PLY TRUSS



NOTES:

1. This detail **CANNOT** be used to repair damaged webs.
2. Scab and web sizes must be equal (i.e. use a 2x6 scab on a 2x6 web, etc.).
3. Scab & web lumber must be DRY (≤ 19% moisture content) at time of installation.
4. Scab must cover minimum 90% of the entire length of web.
5. For 2x12 webs use 2x10 nail pattern, but with 5 rows of nails instead of 4 rows.
6. This detail is for webs loaded axially only (not for axial/bending members).
7. Web and scab lumber shall be SPF No. 2 (or better) grade.
8. Tabulated resistances are for standard load duration only (K_D=1.0) and DRY service conditions (K_S =1.0). Do not use detail for WET service applications.
9. This detail shall be used only in conjunction with sealed MiTek truss drawings.

SCAB CONNECTION: 1-PLY TRUSS



+ 0.122" dia. x 3.0" nail driven from front face
o 0.122" dia. x 3.0" nail driven from back face

Note: Connect scabs to truss along their entire length.

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Certificate No. 10889485

MiTek®

STANDARD DETAIL MSD2015-P

Issued: **APRIL 27, 2022**
Expiry: **APRIL 30, 2024**

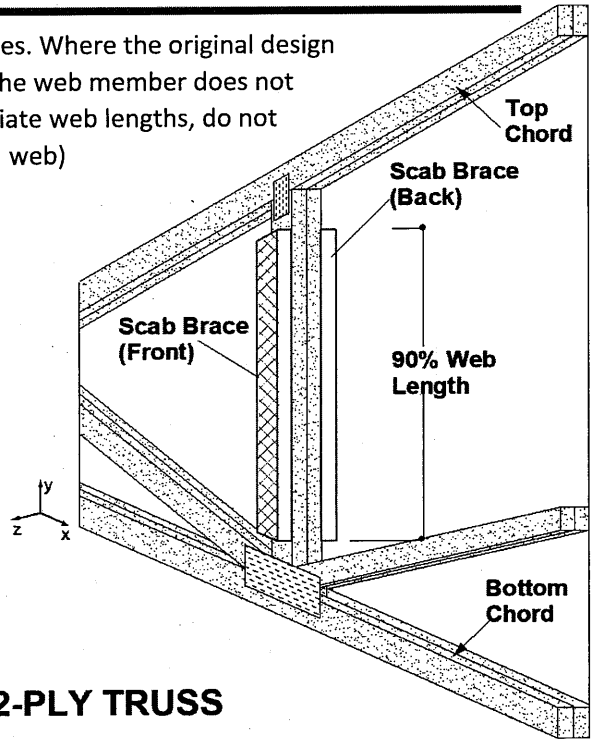
ALTERNATIVE WEB BRACING SOLUTIONS

The scab brace detail shown on this page provides an alternative method of bracing compression webs of 2-PLY trusses. Where the original design calls for web bracing, the scab-brace is an acceptable alternative provided that the maximum factored axial force in the web member does not exceed the tabulated values shown below. This detail applies to web lengths of 4.0 Ft. to 10.0 Ft. only. For intermediate web lengths, do not interpolate, use the tabulated value of the longer length. (ex. For a 6.25 ft. web, use the tabulated values for a 6.5 ft. web)

Maximum factored web force, lbs (2-Ply Truss)

WEB LENGTH (Ft.)	Web size	2x3	2x4	2x5	2x6	2x8+
4.0		8663	12128	15593	19058	25122
4.5		7588	10623	13659	16694	22006
5.0		6570	9198	11826	14455	19054
5.5		5645	7903	10162	12420	16371
6.0		4830	6762	8694	10626	14007
6.5		4126	5776	7426	9077	11965
7.0		3526	4937	6347	7758	10226
7.5		3020	4228	5436	6644	8758
8.0		2594	3632	4670	5708	7524
8.5		2235	3128	4022	4916	6480
9.0		1933	2706	3479	4253	5606
9.5		1680	2352	3024	3696	4872
10.0		1467	2054	2640	3227	4254

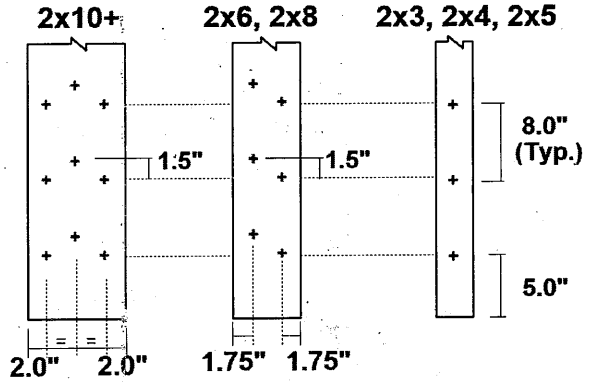
SCAB BRACE DETAIL
2-PLY TRUSS



NOTES:

1. This detail **CANNOT** be used to repair damaged webs.
2. Scab sizes must be equal to web size (i.e. use a 2x6 scab on a 2x6 web, etc.).
3. Scabs &web lumber must be DRY (≤ 19% moisture content) at time of installation.
4. Scabs must cover 90% of the entire length of web and installed on both faces.
5. This detail shall NOT apply to vertical webs used for girder load transfer.
6. Web & scab lumber to be SPF No. 2 (or better) grade.
7. This detail is for webs loaded axially only (not for axial/bending members).
8. Ensure scabs will not interfere with incoming trusses, prior to using this detail.
9. Tabulated resistances are for standard load duration only (K_D=1.0) and DRY service conditions (K_S =1.0). Do not use detail for WET service applications.
10. This detail shall be used only in conjunction with sealed MiTek truss drawings.

SCAB CONNECTION: 2-PLY TRUSS



+ MITEK MIFLK006 Screw @ 8 in. cc
Note: Connect scabs to truss along their entire length.

PEO
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TECHNICAL BULLETIN

LUS — Double-Shear Joist Hangers

SIMPSON Strong-Tie

All LUS hangers have double-shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

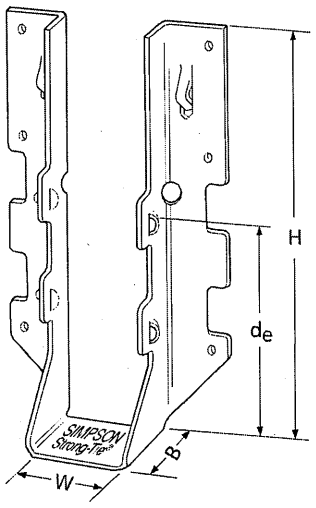
- Factored resistances are in accordance with CSA O86-14 and CSA O86:19.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

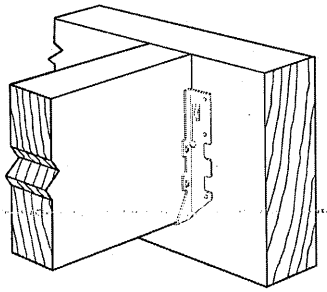
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire
- Double-shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

- These hangers cannot be modified

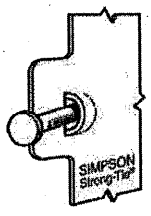


LUS28


Typical LUS Installation

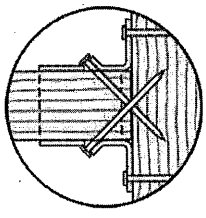
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LUS24	18	1 9/16	3 1/8	1 3/4	1 15/16	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 13/16	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1 9/16	4 3/4	1 3/4	3 5/8	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 7/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4 5/8	4 3/16	2	3 1/4	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 9/16	6 5/8	1 3/4	3 3/4	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4 5/8	6 1/4	2	3 1/4	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 9/16	7 13/16	1 3/4	3 7/8	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4 5/8	8 3/16	2	5 1/4	(8) 16d	(6) 16d	2580	3345	2320	2375

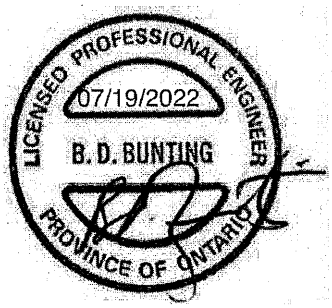
1. d_e is the distance from the seat of the hanger to the highest joist nail.



Dome double-shear nailing prevents tabs breaking off (available on some models).

US Patent 5,603,580


Double-shear nailing top view.



This technical bulletin is effective until December 31, 2024, and reflects information available as of July 1, 2022. This information is updated periodically and should not be relied upon after December 31, 2024. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HUS/LJS — Double-Shear Joist Hangers

SIMPSON Strong-Tie

All hangers have double-shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

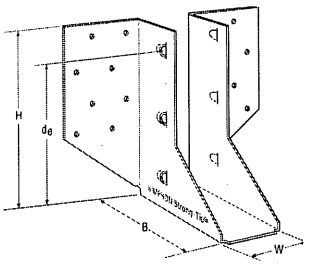
- Factored resistances are in accordance with CSA O86-14 and CSA O86:19.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

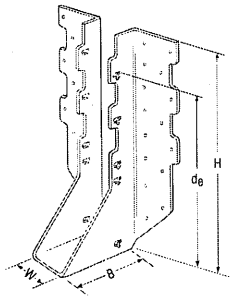
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double-shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

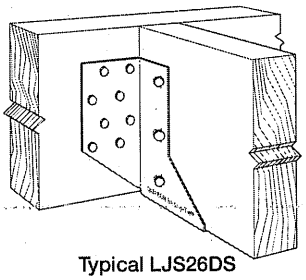
- See current catalogue for options



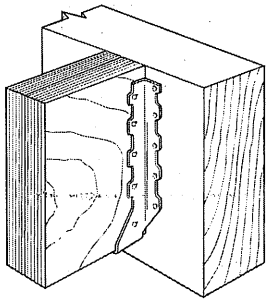
LJS26DS



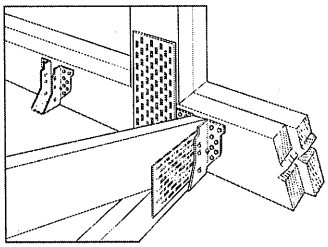
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS Installation



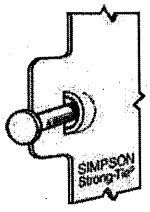
Typical HUS Installation



Typical HUS Installation
(Truss designer to provide fastener quantity for connecting multiple members together)

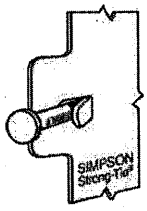
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir–L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
								lb.	lb.	lb.	lb.
LJS26DS	18	1 9/16	5	3½	4⅝	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1⅝	5⅜	3	3 15/16	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1⅝	7 3/32	3	6 3/32	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1⅝	9 3/32	3	7 31/32	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1 13/16	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

1. d_e is the distance from the seat of the hanger to the highest joist nail.

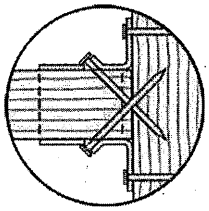


Dome double-shear nailing prevents tabs breaking off (available on some models).

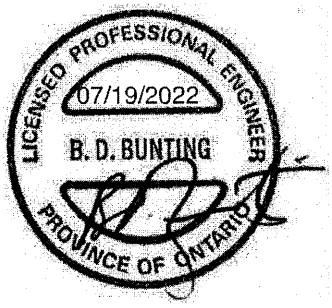
US Patent 5,603,580



Double-shear nailing side view. Do not bend tab back.



Double-shear nailing top view.



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T-SPECHUS22 7/22 exp. 12/24

(800) 999-5099
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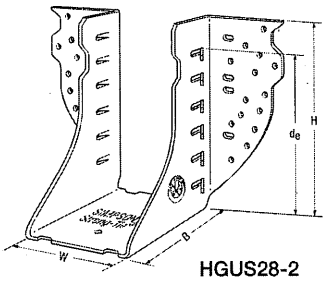
TECHNICAL BULLETIN

HGUS — Double-Shear Joist Hangers

SIMPSON Strong-Tie

All HGUS hangers have double-shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.



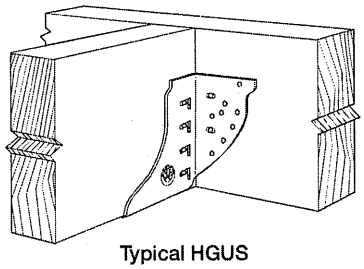
HGUS28-2

Material: 12 gauge

Finish: G90 galvanized

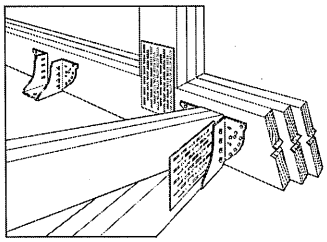
Design:

- Factored resistances are in accordance with CSA O86-14 and CSA O86:19.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.


Typical HGUS Installation

Installation:

- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double-shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

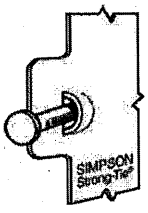

Typical HGUS Installation
(Truss designer to provide fastener quantity for connecting multiple members together)

Options:

- See current catalogue for options

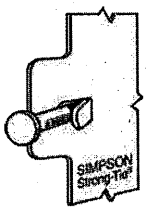
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir–L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
HGUS26	12	1⅝	5⅜	5	4 5/32	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3 5/16	5 7/16	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4 15/16	5½	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6 9/16	5 7/16	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1⅝	7⅛	5	6⅛	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3 5/16	7 3/16	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4 15/16	7¼	4	6⅜	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6 9/16	7 3/16	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210-2	12	3 5/16	9 3/16	4	8⅛	(46) 16d	(16) 16d	6840	14015	4855	10270
HGUS210-3	12	4 15/16	9¼	4	8⅜	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6 9/16	9 3/16	4	8⅛	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6 9/16	10⅝	4	10⅛	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6 9/16	12⅝	4	11⅛	(66) 16d	(22) 16d	10130	16400	7195	11645

1. d_e is the distance from the seat of the hanger to the highest joist nail.

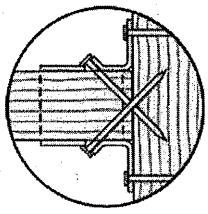


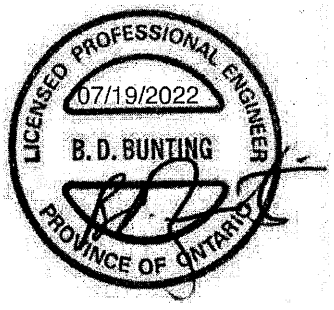
Dome double-shear nailing prevents tabs breaking off (available on some models).

US Patent 5,603,580



Double-shear nailing side view. Do not bend tab back.


Double-shear nailing top view.



This technical bulletin is effective until December 31, 2024, and reflects information available as of July 1, 2022. This information is updated periodically and should not be relied upon after December 31, 2024. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

 T-SPECHGUS22 7/22 exp. 12/24

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TECHNICAL BULLETIN

HGUS — Double-Shear Joist Hangers

SIMPSON Strong-Tie

All HGUS hangers have double-shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

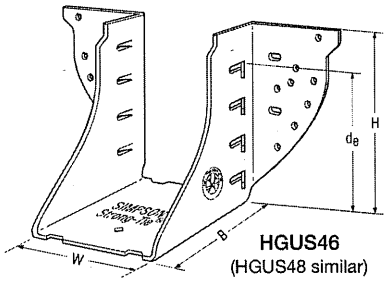
- Factored resistances are in accordance with CSA O86-14 and CSA O86:19.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

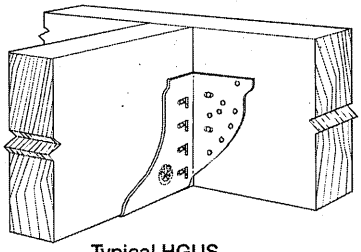
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double-shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

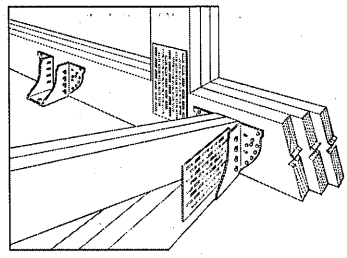
Options:

- See current catalogue for options



HGUS46
(HGUS48 similar)

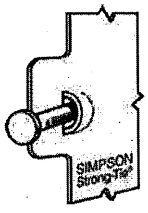

Typical HGUS Installation


Typical HGUS Installation

(Truss designer to provide fastener quantity for connecting multiple members together)

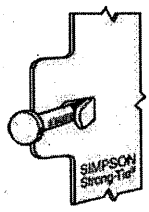
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
HGUS48	12	3⅝	7⅛	4	6 1/16	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS410	12	3⅝	9	4	8 1/16	(46) 16d	(16) 16d	6840	14015	4855	10270
HGUS412	12	3⅝	10 7/16	4	10 1/16	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS414	12	3⅝	12 7/16	4	11 1/16	(66) 16d	(22) 16d	10130	16400	7195	11645
HGUS5.50/8	12	5½	6 15/16	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS5.50/10	12	5½	8 15/16	4	8⅛	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS5.50/12	12	5½	10½	4	10⅛	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS5.50/14	12	5½	12½	4	11⅛	(66) 16d	(22) 16d	10130	16400	7195	11645
HGUS7.25/8	12	7¼	7 7/32	4	6 13/32	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS7.25/10	12	7¼	8⅝	4	8¼	(46) 16d	(16) 16d	6840	15760	4855	11190
HGUS7.25/12	12	7¼	10⅝	4	10¼	(56) 16d	(20) 16d	7640	16110	5425	11435
HGUS7.25/14	12	7¼	12⅝	4	11¼	(66) 16d	(22) 16d	10130	18200	7195	12920

1. d_e is the distance from the seat of the hanger to the highest joist nail.

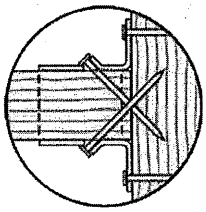


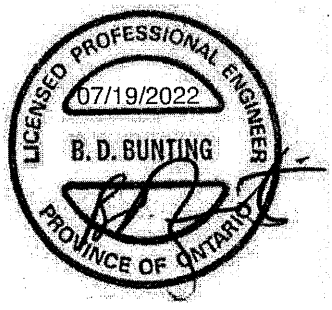
Dome double-shear nailing prevents tabs breaking off (available on some models).

US Patent 5,603,580



Double-shear nailing side view. Do not bend tab back.


Double-shear nailing top view.



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TECHNICAL BULLETIN

H — Seismic and Hurricane Ties

SIMPSON Strong-Tie

The H connector series provides wind and seismic ties for trusses and rafters.

Material: 18 gauge

Finish: G90 galvanized

Design:

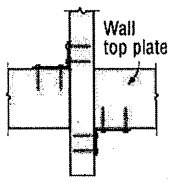
- Factored resistances are in accordance with CSA O86-14 and CSA O86:19.
- Factored resistances have been increased 15%. No further increase is permitted.

Installation:

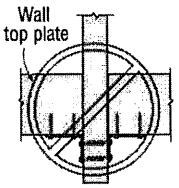
- Use all specified fasteners
- Nails: 8d = 0.131" dia. x 2½" long common wire, 8d x 1½" = 0.131" x 1½ long, 10d x 1½" = 0.146" x 1½" long
- H1 can be installed with flanges facing outwards
- Hurricane ties do not replace solid blocking

Factored resistances for more than one direction for a single connection cannot be added together. A factored load which can be divided into components in the directions given must be evaluated as follows: Factored Shear/Resisting Shear + Factored Tension/Resisting Tension ≤ 1.0.

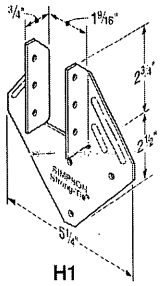
Hurricane Tie Installations to Achieve Twice the Load (Top View)



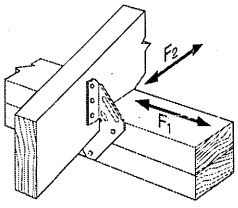
Install diagonally across from each other for minimum 2x truss.



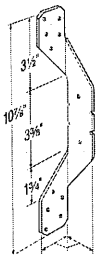
Nailing into both sides of a single ply 2x truss may cause the wood to split.



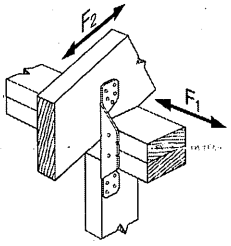
H1



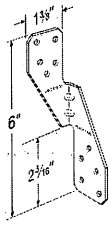
H1 Installation



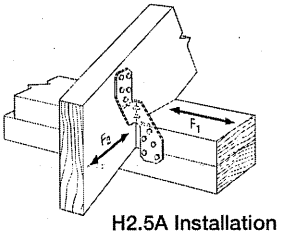
H2A



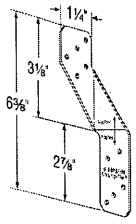
H2A Installation



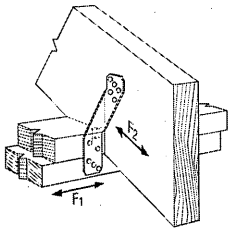
H2.5A



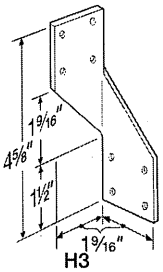
H2.5A Installation



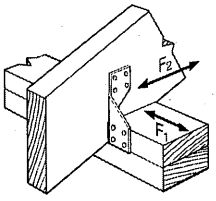
H2.5T



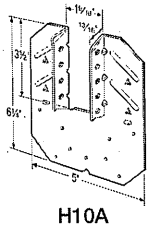
H2.5T Installation (Nails into both top plates)



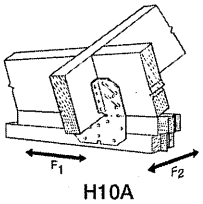
H3



H3 Installation



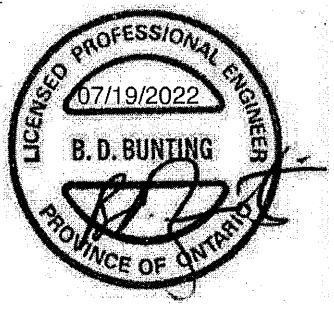
H10A



H10A Installation

Model No.	Ga.	Fasteners			Factored Resistance (lb.)					
					D.Fir-L			S-P-F		
					Uplift	Normal		Uplift	Normal	
		To Rafter	To Plates	To Studs		F_1	F_2		F_1	F_2
					(K_D=1.15)			(K_D=1.15)		
H1	18	(6) 8d x 1½"	(4) 8d	—	740	685	300	680	485	215
H2A	18	(5) 8d x 1½"	(2) 8d x 1½"	(5) 8d x 1½"	830	220	75	590	155	55
H2.5A	18	(5) 8d	(5) 8d	—	805	160	160	755	160	160
H2.5T	18	(5) 8d	(5) 8d	—	835	175	240	740	160	210
H3	18	(4) 8d	(4) 8d	—	740	180	265	615	125	190
H10A	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1735	795	410	1505	565	290

1. Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.
2. Factored resistances are for one anchor. A minimum rafter thickness of 2½" must be used when framing anchors are installed on each side of the joist and on the same side of the plate.
3. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.
4. Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a Continuous Load Path, connections must be on same side of the wall.



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ONTARIO WOOD TRUSS FABRICATORS ASSOCIATION

TECH-NOTES

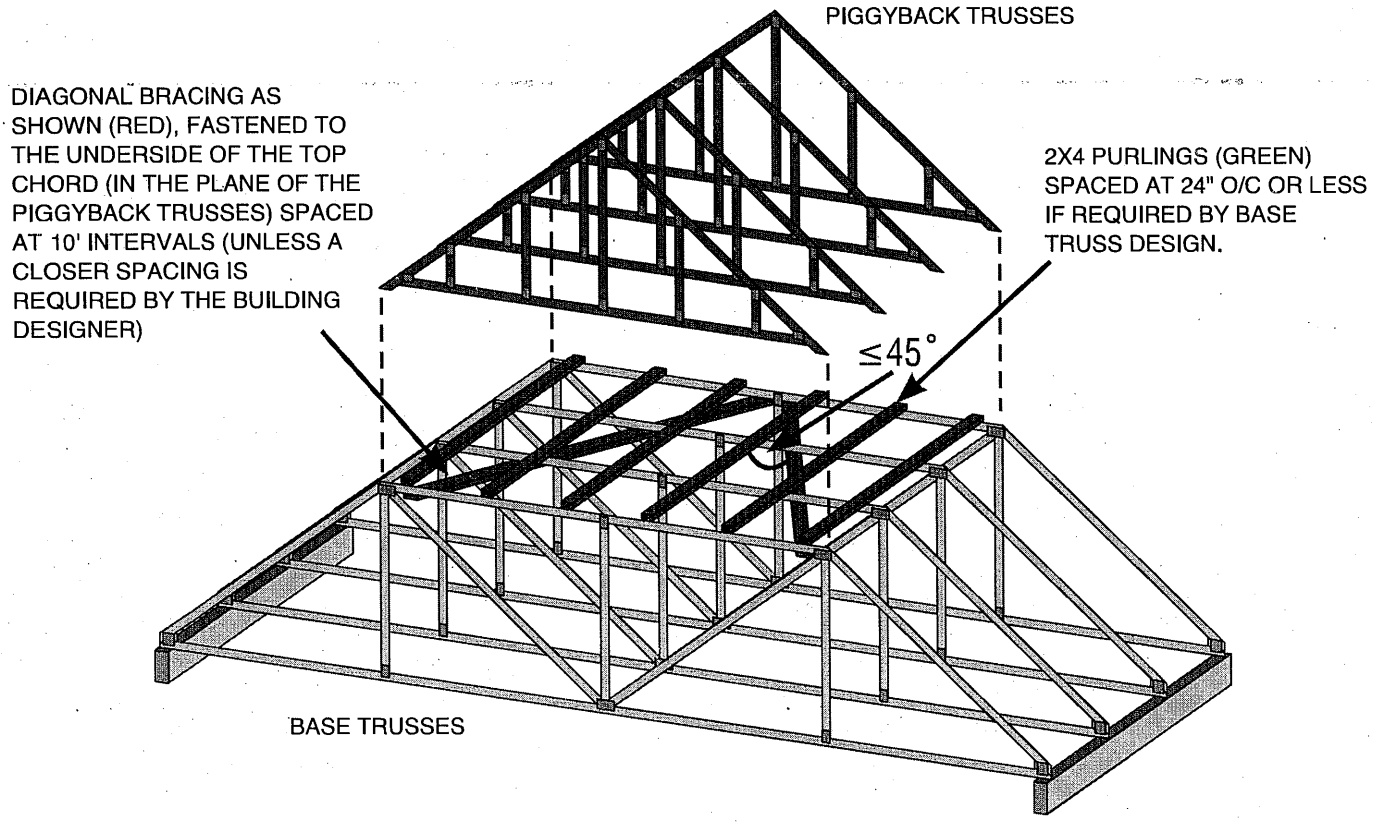
TN 15-001
Piggyback Bracing

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:

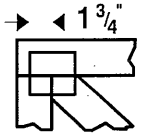


NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

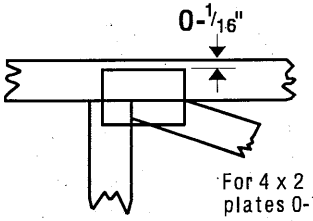
SKETCH FROM BCSI-CANADA 2013

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

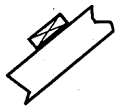
*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

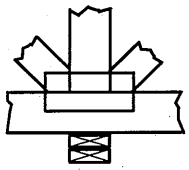
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

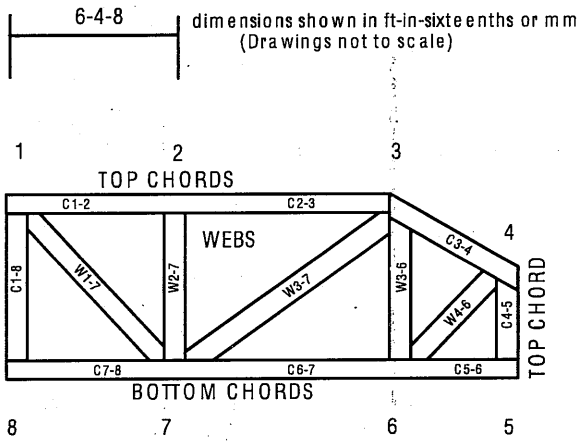


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

MiTek®
POWER TO PERFORM.™

⚠ General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.